Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1SXW487-Z0808155617

Truss Fabricator: Anderson Truss Company

Job Identification: 6-228--Mike Todd Construction Zebra 1 -- , **

Truss Count: 25

Model Code: Florida Building Code 2004

Truss Criteria: ANSI/TPI-2002(STD)/FBC

Pring Software: Albina Software Vension 7.2

Engineering Software: Alpine Software, Version 7.24.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-02 -Closed

Notes:

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

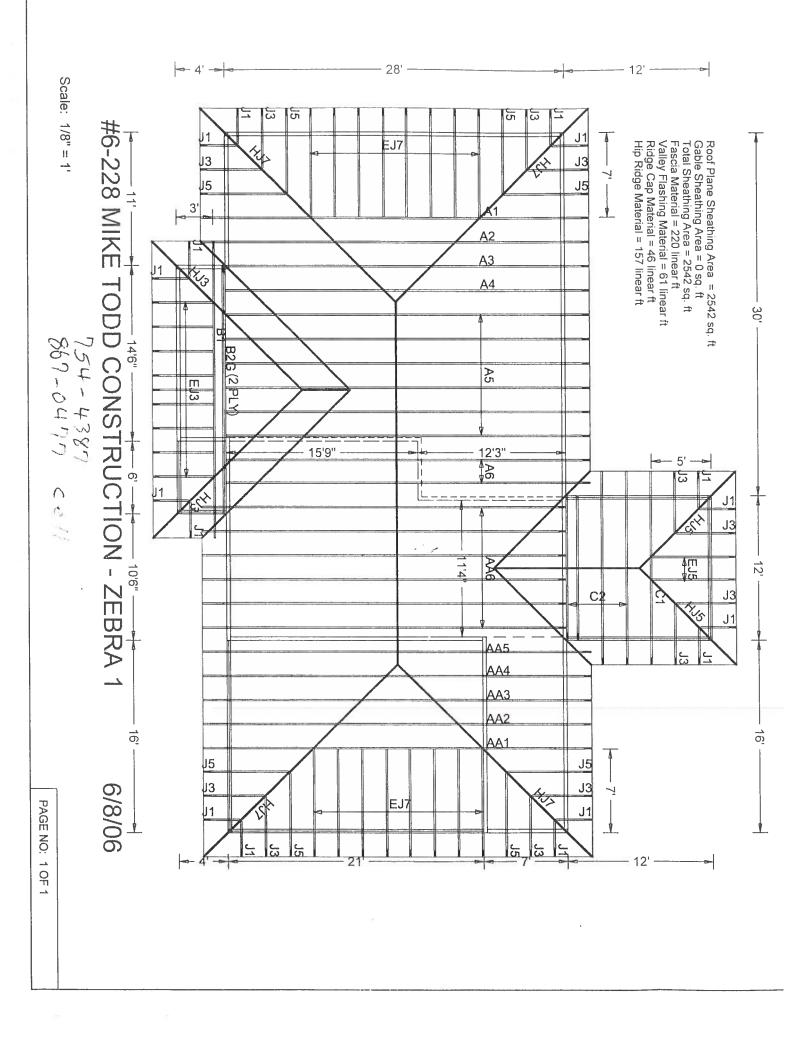
Details: TCFILLER-BCFILLER-

	#	Ref Description	Drawing#	Date
	1	86959 A1	06159106	06/08/06
i	2	86960 A2	06159098	06/08/06
	2	86961 A3	06159099	06/08/06
ı	4	86962 A4	06159107	06/08/06
į	5	86963A5	06159108	06/08/06
Ì	6	86964 A6	06159100	06/08/06
١	7	86965 AA1	06159109	06/08/06
ı	8	86966 AA2	06159110	06/08/06
Į	9	86967 AA3	06159111	06/08/06
۱	10	86968AA4	06159112	06/08/06
ı	11	86969AA5	06159114	06/08/06
I	12	86970 AA6	06159101	06/08/06
	13	86971B1	06159115	06/08/06
ı	14	86972B2G	06159116	06/08/06
	15	86973C1	06159117	06/08/06
ı	16	86974C2	06159102	06/08/06
l	17	86975HJ7	06159006	06/08/06
١	18	86976EJ7	06159103	06/08/06
l	19	86977 J5	06159104	06/08/06
I	20	86978J3	06159008	06/08/06
	21	86979J1	06159009	06/08/06
	22	86980HJ5	06159007	06/08/06
	23	86981EJ5	06159010	06/08/06
١	24	86982HJ3	06159118	06/08/06
	25	86983EJ3	06159105	06/08/06



-Truss Design Engineer-Arthur R. Fisher Florida License Number: 59687 1950 Marley Drive Haines City, FL 33844





I n 24" Bot chord 2x4 SP
Webs 2x4 SP PLT TYP. lieu of structural panels or rigid ceiling use purlins to brace TC @ " 0C , BC @ 24" 0C. 0-4-3 ALPINE 20 Gauge HS, Wave **1**2-0-0 世代2 4X8(B3) ≡ Dense R-2380 U-219 W-3.5" 6 **IMPORTANT**rurrish a copy of this design to the installation contractor.

ARPOINTS, INC. SMALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE RESOURCES, INC. SMALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. BY FAILURE TO BUILD THE RUSS IN COMPONENCE WITH HET:

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Alpine Engineered Products, Inc. 1959 Marley Drive Hames City, FL 33844

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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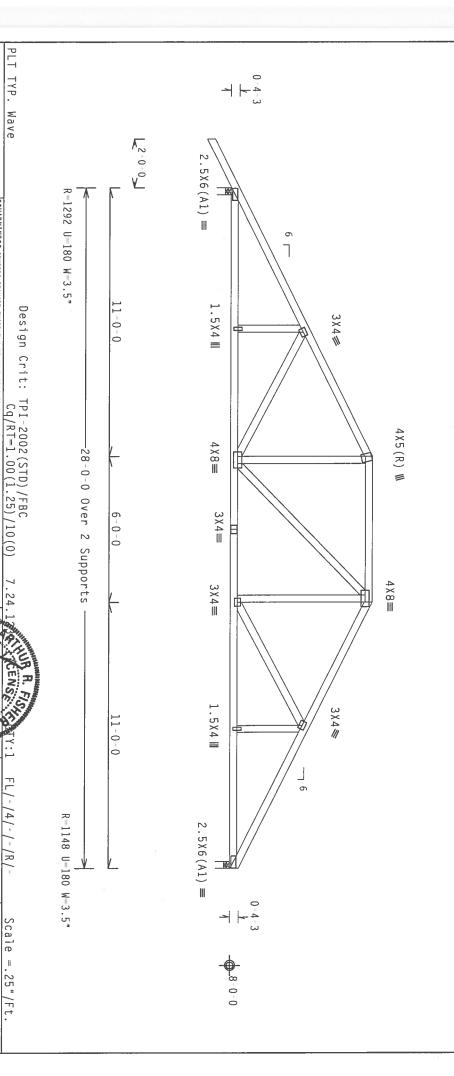
Bot chord 2x4 Webs 2x4 In 3 PLT TYP. Alpine Engineered Products, Inc. 1950 Marley Drive lieu of structural panels or rigid ceiling use purlins to brace TC @ " OC, BC @ 24" OC. 0-4-3 Haines City, FL 33844 Ficate of on # 567 ALPINE Wave \$\$\$ K2-0-0 327 $2.5 \times 6 (A1) =$ Dense Dense R=1287 U-180 W-3.5" ***IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FAILINE TO BUILD THE PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILINE TO BUILD THE TRUSS IN CONTRACHANCE WITH HE! BEAR FOR THE FOR THIS FOR THE FOR THE FOR THE FOR THE FOR THIS FOR THIS FOR THIS FOR THE FORE DEATH HE FORE THE FORE DEATH HE FORE THE FORE **WARNING** IRUSSE'S REQUIRE EXTREME CARE HI FABRICATION. MANDELHG. SHIPPING, INSTALLING AND BRACING, RETER TO BEST 1 0.3 (BUILDING COMPONENT SACTY INFORMATION), PUBLICATION OF THE (RAUSS PLATE INSTITUTE, SB3 D'OHOFRIO BA. SUITE ZOO, MADISON, AT 18.5319) AND MICA (HOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LIN, MADISON, AT 18.5319) AND MICA (HOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LIN, THANISON, AT 15.719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNITES OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGION CELLING. BUILDING DESIGNER PER ANSI/TP1 1 SEC. 2 9 9-0-0 1.5X4₩ Design Crit: 4 X 5 == TPI-2002 (STD) /FBC 4 X 8 == Cq/RT=1.00(1.25)/10(0) -28-0-0 Over 2 Supports BUILDING IS THE RESPONSIBILITY OF THE 3 X 4 ≡ 10-0-0 3 X 4≡ 110 mph wind, 15.00 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. 4 X 8≡ 4 X 5 ≡ .5X4 € 9-0-0 SPACING ВС ВС TC DL TC LL DUR.FAC. TOT.LD. FL/-/4/-/-/R/-민 R-1287 U-180 W-3.5" 9 $2.5 \times 6 (A1) \equiv$ 40.0 20.0 PSF 24.0" 1.25 10.0 PSF 10.0 PSF 0.0 PSF K2-0-0V PSF DATE REF JRFF-SEQN-HC-ENG DRW HCUSR487 06159098 Scale = .25"/Ft. R487--1SXW487 JB/AF 8138 06/08/06 86960 208

Bot chord 2x4 SP
Webs 2x4 SP In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ 24" OC. ##Z Dense

IIU mph w1nd, 15.00 ft mean hgt, ASCL /-02, CL05LD bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

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Alpine Engineered Products, Inc. 1950 Marley Drive Hanes City, FL 33844 Teatle of / 'on # 567

ALPINE

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

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REF DATE

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DRW HCUSR487 06159099

06/08/06

JB/AF 8152

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Bot H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. PLT TYP. Alpine Engineered Products, Inc. p cnord ZX4 SP L t chord ZX4 SP L Webs ZX4 SP L 0 Haines City, FL icate of 43 ALPINE Wave 33844 on # 567 K2-0-0 V #2 Dense #3 $.5 \times 6 (A1) \equiv$ R-1293 U-180 W-3.5" **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FAILURE TO BUILD THE PRODUCTS. THE. SMALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE RESS IN COMPORANCE WITH THE THE THE PRODUCTION. THE STATE OF THIS PRODUCT OF THE PRODUCT OF THE PROPERTY OF THE PROPE ****WARNING** IRUSELS REQUIRE EXTREME CARE IN FARRICATION, IMADIENT, SUPPING, INSTALLING AND BRACING, RETER TO BEST I DOS JOHUDING COMPORED SAFETY INFORMATION), PUBLISHED BY THE UNUSS PLATE INSTITUTE, 503 DO FORTRADISC HAMBOON, STATE TO BEST I DOS JOHNSTON, ALSO FORTRADISC HAMBOON, MISSIED BY THE ZOO, MADISON, MISSIED BY THE ADDITION FOR THE PRACTICES PRIOR TO PROPERTY ATTACHED TO THE COMPANY OF THE CONTROL AND BOTTOM CORDS SHALL HAVE PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE A PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE A PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE A PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE A PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE A PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE A PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE A PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE A PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE A PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE A PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE A PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE A PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE A PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CORDS SHALL HAVE A PROPERTY ATTACHED TO THE CONTROL AND BOTTOM CONTROL AND THE CONTR RIGID CEILING. 6 Design Crit: TPI-2002(STD)/FBC 3-0-0 1.5X4 III 5×4/ Cq/RT=1.00(1.25)/10(0) 28-0-0 3 X 4≡ 4 X 5 ≡ Over 2 Supports 3 X 4 ≡ 0-0 3 X 4 ≡ 4 X 5 == In lieu of structural panels or rigid ceiling use @ 24" OC, BC @ 24" OC. 110 mph wind, 15.00 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. CENSE 6 TATE OF 1.59687 1.5X4 Ⅲ 3×4₩ 13 - 0 - 0R-1147 U-180 H-Simpson LUS26 4 is (2)2X6 min. 10d Common, 0.148"x3.0" nails in Truss 10d Common, 0.148"x3.0" ВС TC DL TC LL ВС SPACING DUR.FAC. TOT.LD. FL/-/4/ 2 $2.5 \times 6 (A1) =$ So.Pine 40.0 /-/R/-1.25 10.0 PSF 20.0 PSF 24.0" 10.0 PSF 0.0 purlins to brace TC PSF PSF nails in Girder SEQN-DATE REF HC-ENG JRFF-DRW HCUSR487 06159107 Scal R487--1SXW487 Z08 =.25"/Ft. JB/AF 8169 06/08/06 8-0-0 86962

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Bot chord 2x4 SP
Webs 2x4 SP PLT TYP. Alpine Engineered Products, Inc. Haines City, FL 33844 icate of on # 567 <u>4</u> ـ ALPINE Wave L2-0-0 ##Z .5X6(A1) =Dense R-1293 U-180 W-3.5" **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FAILURE TO BUILD THE PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSCES IN CONFORMACE WITH TPI:

OF SHART AND THE PRODUCT OF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF FPIT 2 DRAWING INDICALES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY DESIGN SHOWN. THE SULFABLILITY AND USE OF THIS COMPONENT FOR ANY BUILD BUILDING DESIGNER PER ANSI/TPI I SEC. 2. RIGID CEILING. 6 Design Crit: TPI-2002(STD)/FBC 5×4/ 14-0-0 Cq/RT=1.00(1.25)/10(0) 3 X 4≡ ANY BUILDING IS THE RESPONSIBILITY OF THE -28-0-0 Over 2 Supports 4 X 5 ≡ **®** 110 mph wind, 15.00 ft mean hgt, ASCE /-02, CLOSED bidg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. In lieu of structural panels or rigid ceiling use @ 24″ OC, BC @ 24″ OC. 3 X 4 ≡ $3 \times 4 \equiv$ CPNS 90, 80 6 6.59687 1.5X4 // 4-0-0 R-1147 U-180 H-Simpson HUS26 der is (1)2X6 min. So.Pine (4) 10d Common, 0.148"x3.0" nails in Truss (14) 10d Common, 0.148"x3.0" BC DL ВС TC DL TC LL SPACING DUR.FAC. TOT.LD. FL/-/4/-/-/R/ $2.5 \times 6 (A1) =$ 40.0 20.0 24.0" 10.0 PSF 1.25 10.0 PSF 0.0 purlins to brace TC PSF PSF PSF nails in Girder DATE REF SEQN-JRFF-HC-ENG DRW HCUSR487 06159108 æ R487--1SXW487 Z08 =.25"/Ft. JB/AF 8194 06/08/06 86963

Bot chord 2x4 SP | Webs 2x4 SP | Filler 2x4 SP | #2 Dense #3 #3

SEE DWGS TCFILLER1103 AND BCFILLER1103 FOR FILLER DETAILS. LATERALLY BRACE BOTTOM CHORD ABOVE FILLER AT 24" O.C., AND TOP CHORD UNDER FILLER AT 24" OC INCLUDING A LATERAL BRACE AT CHORD ENDS.

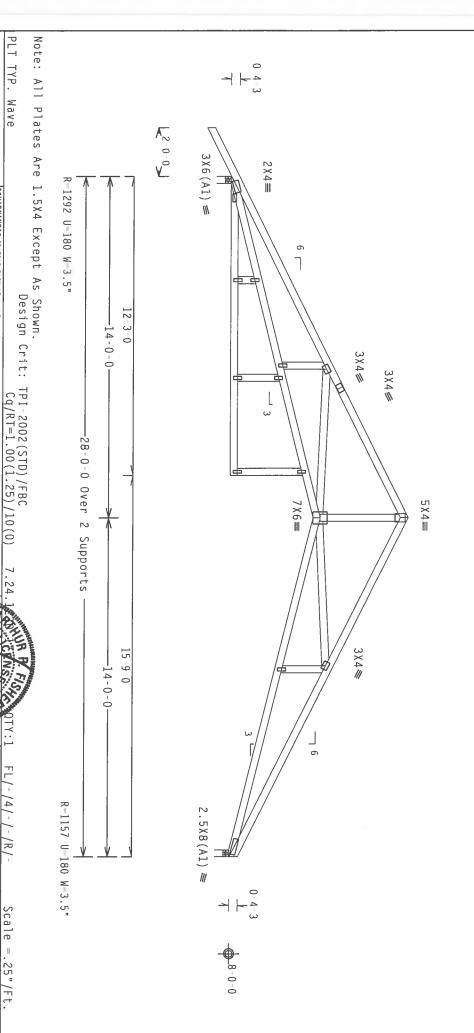
110 mph wind, 15.00 ft mean hgt, ASCE /-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.19" due to live load and 0.29'

due to dead load

In lieu of structural panels use purlins to brace \mathcal{I} **@**

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Alpine Engineered Products, Inc.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FAILURE TO RECONSTRUCT THE PRODUCTS. INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAIL ON FROM THIS DESIGN. ANY FAILURE TO BOULD THE ROUSE IN COMMONANCE WITH HET:

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SEQN-HC-ENG

ALPINE

Haines City, FL

33844 on # 567

Bot chord 2x4 SP
Webs 2x4 SP ##2 Dense

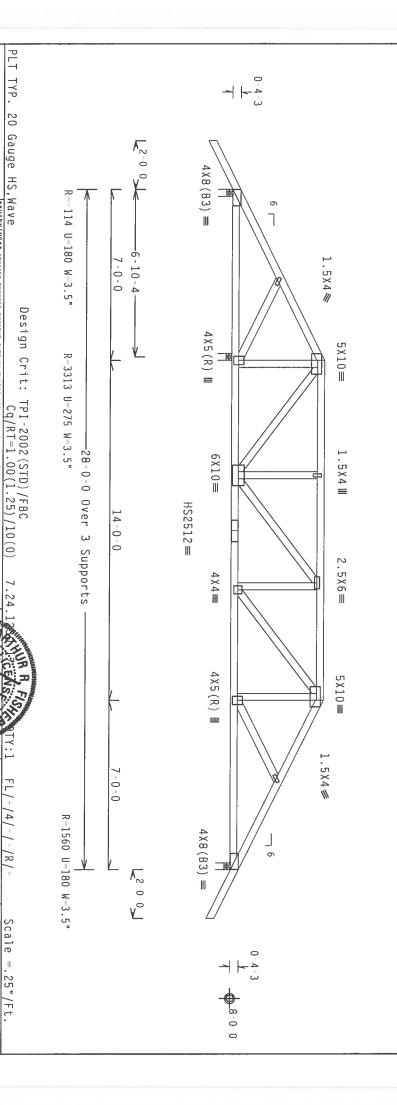
In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ 24" OC.

IIU mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0

#1 hip supports 7.0-0 jacks with no webs

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Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Alpine Engineered Products, Inc.

ALPINE

RIGIO CEILING

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPTHE ENGINEERED PRODUCTS, INC. SMALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FALLURE TO BUILD THE IRUSES IN CONFORMACE WITH ITE!

OF FARBICALTHE, MANDLING, SHIPPING, INSTALLING A BRACING OF PRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MOS (MAIDONAL DESIGN SPEC, MY FAFA) AND TP!. ALPHY CONFICENCY PALAIES ARE HADO OF ZOTUBLING AND MAID SPECIAL PROPERTY OF THE SPECIAL PROPERTY OF THE SPECIAL PROPERTY OF THIS DESIGN AND MAILS OF THE SPECIAL PROPERTY OF THIS DESIGN OF PALAIES TOLLOWED BY C1) SHALL BE FER AURILY AD OF THIS DESIGN. PARY AND MAILS OF THE SPECIAL PROPERTY OF THE SPECIAL PROPERTY.

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Haines City, FL 33844 icate of / on # 567

BUILDING DESIGNER PER ANSI/TPI 1 SEC.

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR

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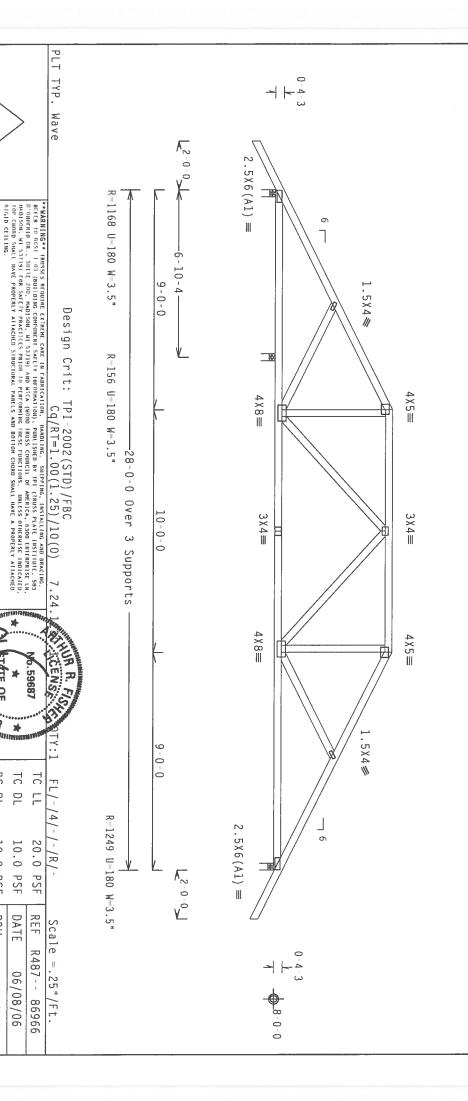
1SXW4R7 Z08

1.25

In 24" Bot chord 2x4 SP
Webs 2x4 SP lieu of structural panels or rigid ceiling use purlins to brace TC $^{\circ}$ OC, BC @ 24" OC. ##Z Dense e

110 mph wind, 15.00 ft mean hgt, ASCE /-02, CLOSED bidg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Alpine Engineered Products, Inc.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ARY FAILURE TO BUILD THE PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DELIVATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE RUSS IN COMPONANCE WITH PET:

RUSS IN COMPONANCE WITH PET:

DESIGN CONTRONS WITH APPLICABLE PROPYSIONS OF MOS (INSTONAL DESIGN SPEC, BY ATERA) AND TPI.

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SPACING DUR.FAC. TOT.LD.

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> DATE REF

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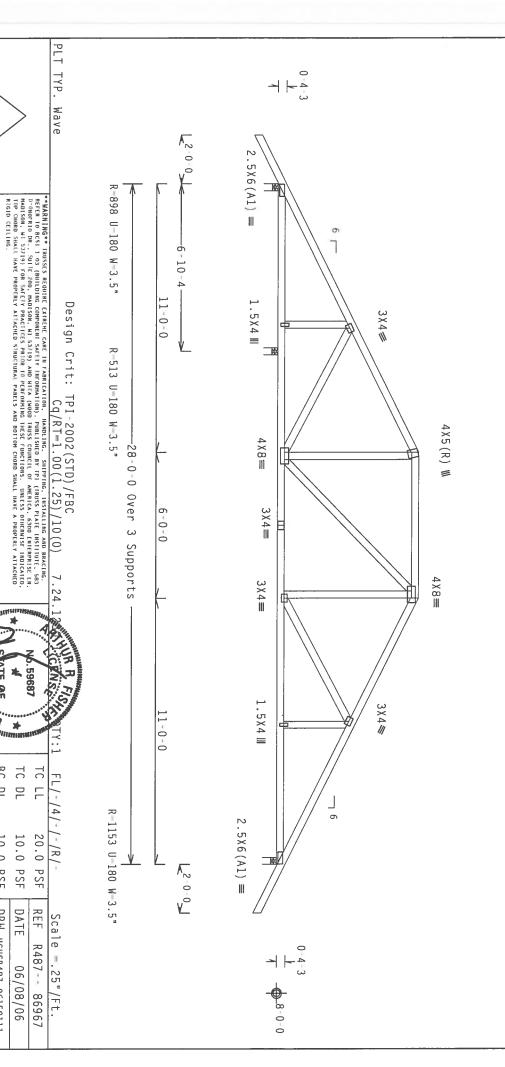
ALPINE

Haines City, FL 33844 Ficate of / on # 567

Bot chord 2x4
Webs 2x4 In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ 24" OC. \$\$\$ #2 Dense **@**

IIU mph wind, 15.00 ft mean hgt, ASCE /-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Alpine Engineered Products, Inc.

IMPORTANTTURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPINE ENGINEERED PRODUCTS, THE SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION ROW THIS DESIGN. ANY FAILURE TO BUILD THE RESULT OF THE SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION ROW THIS DESIGN. FOR THIS ARE HADED OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THIS (MATIONAL DESIGN SPEC, BY AFRA) AND TPI.

CONNECTION PLAITS ARE HADE OF 20/109/160A (M.H/S/S) ASTH MASS GRANDE AD/SO (M.K/H.S) GALV. SIELL. APPLY DALLES OF THE SHALL NOT THE SHALL NOT THE SHALL NOT PROFESSIONAL ENGINEERING AS GRANDE AND THIS DESIGN. POSITION PAR BRAHHIGS 160A Z. ANY THESECTION OF PLAITS SHOULDED BY SHOWN THE SHOWN THE SHIP AND THE SHOULDED WERE PER ANY SHOWN THE SHIP AND THE SHOWN THE SHOWN THE SHIP AND THE SHOWN THE SHIP AND THE SHIP AND THE SHOWN THE SHIP AND THE SHOWN THE SHIP AND THE SHIP AND THE SHIP AND THE SHOWN THE SHIP AND THE SHOWN THE SHIP AND THE SHOWN THE SHIP AND THE SHIP

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HC-ENG

JB/AF 8159

ALPINE

Itaines City, FL 33844

Bot In 24* PLT TYP. Alpine Engineered Products, Inc. t chord 2x4 SP Webs 2x4 SP lieu of structural panels or rigid ceiling use purlins to brace TC °OC, BC @ 24°OC. 0-4-3 Haines City, FL ALPINE Wave 33844 on # 567 L2-0-0 #2 Dense #2 Dense #3 $2.5 \times 6 \text{ (A1)} =$ R=578 U-180 W-3.5" ***IMPORTANT***URNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALP TALURE TO BUILD THE PRODUCTS, INC. SHALL NOT DE RESPONSIBLE FOR ANY DEVIATION ROW THIS DESIGN:

TRUSS IN CONTEMBRANCE ATIN IP:

OESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THIS CALLING, SHIPPING, INSTALLING & BRACTHE OF TRUSSES,

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THIS (MINIODAL DESIGN SPEC, BY METAPA) AND IPI.

CONTECTOR PLAIGE SAE HADE OF 20/189/IGAG, (MINIODAL DESIGN SPEC, BY METAPA) AND IPI.

CONTECTOR PLAIGE SAE HADE OF 20/189/IGAG, (MINIOSAL) ASIA HAS GRADE AND STEEL APPLY

PLAITES TO EACH FACE OF TRUSS AND, UNLESS DIMERNISE DOCALED ON HIS DESIGN, POSITION PER BRAHINGS 160A Z.

ANY INSPECTION OF PLAICE FOLLOWED BY (1) SHALL BE FER ANNY A 30" TPIT 2002 SEC. 3.

AREA ON HIS SOMEWHALL AND STEEL AS A SEAL ON HIS DESIGN TO THE TRUSS COMPONENT **WARNING** IRUSSES REQUIRE EXTREMÉ CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. RETER TO BEST 1 03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE. 883 D'OMOFRIO DR. SUNITE ZOO, HADISON, HI 53719) AND MICA (MODO RHUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LH, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PLEFORNING INESE FUNCTIONS. UNLESS OTHERWISE INDICATED. TOP CORDS SMALL HAVE PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILLING. DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. 6 6-10-4 Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 13 - 0 - 01.5X4 III 5×4/ USE OF THIS COMPONENT FOR ANY BUILDING IS THE R-933 U-180 W-3.5" 28-0-0 **@** 3 X 4≡ 5×4≡ Over 3 Supports RESPONSIBILITY OF 3 X 4 ≡ 3 X 4 ≡ 4 X 5 == 110 mph wind, 15.00 ft mean hgt, ASCE 7 02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. LENS 0. 59687 1.5X4 III 5 X 4 // 13-0-0 TC DL TC LL ВС SDACING ВC DUR.FAC. TOT.LD. FL/-/4/-/-/R/ 민] R=1053 U=180 W=3.5* 2.5X6(A1) =40.0 1.25 20.0 PSF 24.0" 10.0 PSF 10.0 PSF 0.0 PSF PSF L2=0=0v DATE REF JRFF-SEQN-HC-ENG DRW HCUSR487 06159112 Scale =. R487---1SXWAR7 JB/AF 8180 06/08/06 25"/Ft. 86968 208

In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ 24" OC. Bot chord 2x4 SP
Webs 2x4 SP PLT TYP. Alpine Engineered Products, Inc. 0 Haines City, FL - 4 3 ALPINE Wave 33844 nn # 567 L2-0-0 #2 Dense #3 $.5 \times 6 (A1) \equiv$ R-1167 U-180 W-3.5" ***IMPORTANT***TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPTHE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION ROOM HIS DESIGN: ANY FALLURE TO BUILLD THE TRUSS HE CONDOMNAUGE WITH FPT.

OF TABLES, INC. SHALL HOT BE RESPONSIONS OF THIS CREATED SHALL SHEPHIG. HAS LINEAR SHALL HOT BEACHEN OF TRUSSES, DESIGN CONFORNS WITH APPLICABLE PROPVISIONS OF THIS CREATED BELLET. A PAPER CONFECTION PARTS ARE HADE OF ZO/THE PARTS, AND THE CONFECTION PARTS ARE HADE OF ZO/THE PARTS. A CHARMACH AND AND THE PARTS. A PAPER CONFECTION PARTS ARE HADE OF ZO/THE PARTS. OF THE STATE OF THE PARTS OF THE TRUSS OF THE PARTS. DESIGN AND THIS DESIGN. POSITION PER BRANHES 100A 12.

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ANY IMPRECIATION OF PARTS TO LOUGHED BY CLOSED OF THE TRUSS COMPONENTIAL PROPERTY AND THE TRUSS COMPONENTY. PLATES TO EACH FACE OF TRUSS AND. UNITESS OTHE ANY INSPECTION OF PLATES FOLLOWED BY (1) SMALL DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL DESIGN SHOWN THE SUITABILITY AND USE OF IN BUILDING DESIGNER PER ANSI/IPI I SEC. 2. **WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, MANDEING, SUIPPING, INSTALLING AND BRACING, RETER TO BEST 103 (BUILDING COMPORENT SAFETY INFORMATION), PHILISHED BY FPT (TRUSS PLATT INSTITUTE, 583 0 "O"HOPETO DR. S. SUITE ZOO. MADISON, HI 53719) AND MICA (MODO TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE INFORMATION, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNICSS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLLING. 6-10-4 6 Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 5×4# 14-0-0 R=159 U=180 W=3.5" 3 X 4 ≡ 28-0-0 **@** Over 3 Supports 4 X 5≡ RESPONSIBILITY OF ll0 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. $3X4 \equiv$ 3 \ 4 ≡ RIOP ENGINEER 14-0-0 5 X 4 🦊 * ВС 8 C TC DL SPACING DUR.FAC. TC TOT.LD. FL/-/4/-/-/R/ 민 6 R-1239 U-180 W-3.5" $2.5 \times 6 (A1) =$ 40.0 20.0 24.0" 1.25 10.0 PSF 10.0 PSF 0.0 PSF PSF PSF K2 0 0 V DATE REF JRFF-SEQN-HC-ENG DRW HCUSR487 06159114 Scale R487--1SXW4R7 =.25"/Ft. JB/AF 8201 06/08/06 86969 8-0-0 Z08

Bot chord 2x4 SP
Webs 2x4 SP Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. Calculated horizontal deflection is 0.19" due to live load and 0.30" due to dead load. PLT TYP. Alpine Engineered Products, Inc. 7 1 3 Haines City, FL ALPINE 2.5X8(A1) = Wave 33844 on # 567 R=1156 U=180 W=3.5" #2 Dense #3 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY TALLINE TO BUILD HIGH PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETVATION FROM THIS DESIGN: ANY TALLINE TO BUILD HIGH ROBOTH THIS DESIGN. THE STREET HIS THE STREET HIS DESIGN. THE STREET HIS THE 6 14-0-0 1.5X4 III Design Crit: 3X4# TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 28-0-0 Over 2 Supports 7 X 6≡ 5 X 4≡ In lieu of structural panels or rigid ceiling use purlins to brace @ 24" 0C, BC @ 24" 0C. 110 mph wind, 15.00 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 3×4// 1.5X4 W 3X4// WHUR R. 14-0-0 CENSE 90,80 .59687 9 R-1301 U-180 W-3.5" BC LL BC DL IC LL TC DL SPACING DUR.FAC. 2.5X8(A1) ≥ TOT.LD. FL/= /4/-/-/R/-L2-0-0V 40.0 20.0 10.0 PSF 24.0" 1.25 10.0 PSF 0.0 PSF PSF PSF JRFF-SEQN-DATE REF HC-ENG DRW HCUSR487 06159101 Scale =. R487-1SXW4R7 JB/AF TC 8209 06/08/06 .25"/Ft. 86970 Z08

Bot chord 2x4 SP
Webs 2x4 SP In 24" PLT TYP. Alpine Engineered Products, Inc. 1950 Marley Drive lieu of structural panels or rigid ceiling use purlins to brace TC @ " 0C, BC @ 24" 0C. 0-4-3 Itames City, FL ALPINE Wave 33844 3n#567 €2-0-0-> #2 Dense #3 $2X4(A1) \equiv$ **IMPORTANT**FURRISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FAILURE TO BUILD HE PRODUCTS. INC. SMALL NOT BE RESPONSIBLE FOR ANY DETIALID A FRANCISCITION FROM HIS DESIGN. ANY FAILURE TO BUILD HE RUSS IN COMPORMACE WITH HET:

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OESIGN COMPORMS WITH APPLICABLE PROVISIONS OF 1005 (MATIONAL DESIGN SPEC, BY ATRAPA) AND TE!

CONICIONE PAIRTS ARE ANDE OF TOTAL SPANCISCO AND A TOTAL SPECIAL PROVISIONS OF 1005 (MATIONAL DESIGN SPEC) ANY SPECIAL PROVISIONS OF 1005 (MATIONAL DESIGN SPECIAL PROVISIONS OF 1005 (MATIONAL PROVISIONS OF 1005 (MATIONAL PROVIS **WARNING** IRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING. INSTALLING AND BRACING RETER TO BEST 1 03 (BUILDING CORPONENT SAFITY INFORMATION), PUBLISHED BY IPT (TRUSS PLATE INSTITUTE, 583 D'OHOFRIO BR. SITIE 200, HADISON, HI 53719) AND HEAVE AND SON, HI 53719 AND HEAVE A PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED RIGHD CEILLING. R=798 U-180 W-3.5" DESIGN SHOWN. THE SUITABILITY AND BUILDING DESIGNER PER ANSI/IPI 1 SEC. 3-0-0 1.5X4 III 4 X 6≡ Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF $3 \times 4 =$ 4 X 4 ≡ -20-6-0 Over 3 Supports 1.5X4 III 14-6-0 4 X 8≡ $3 \times 4 \equiv$ Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 #1 hip supports 3-0-0 jacks with no webs. $4 \times 4 \equiv$ 3 X 4 莹 - MX R=1149 U-180 W=3.5" . 59687 1.5X4 4×6= = 3-0-0 R=331 U=180 W=3.5" BC DL TC DL TC LL SPACING DUR.FAC. TOT.LD. FL/-/4/-/-/R/ $2X4(A1) \equiv$ 6 40.0 20.0 10.0 PSF 24.0" 1.25 10.0 PSF 0.0 PSF PSF PSF JRFF-SEQN-DATE REF HC-ENG DRW HCUSR487 06159115 Scale = .3125"/Ft. R487 1SXW487 Z08 JB/AF 8235 06/08/06 86971

Bot chord 2x8 SP Webs 2x4 SP TC = From 62 PLF at 0.00 BC = From 20 PLF at 0.00 BC = 1147 LB Conc. Load at 12.06, 14.06 SPECIAL LOADS (LUMBER DUR.FAC.=1.25 / rom 62 PLF at 0.00 t #12 Dense W6 2x4 / PLATE DUR.FAC.-1.25)
) to 62 PLF at 20.50
) to 20 PLF at 20.50
2.06, 4.06, 6.06, 8 SP #2 Dense: 8.06, 10.06 2 COMPLETE TRUSSES REQUIRED

In 24"

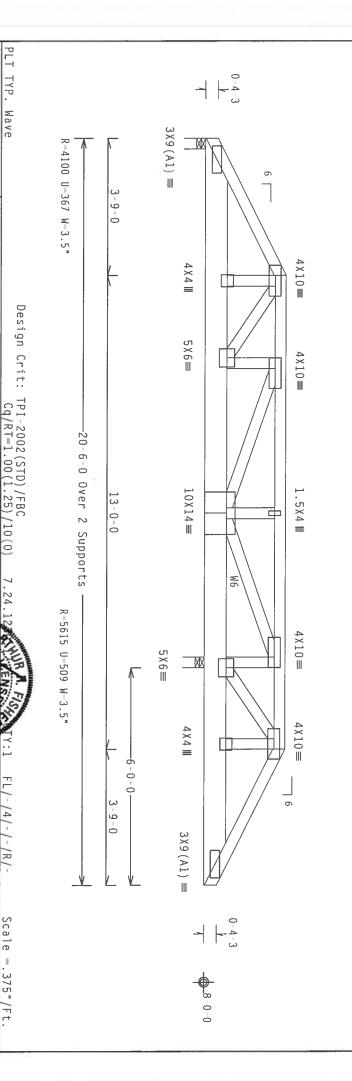
lieu of structural panels or rigid ceiling use purlins to brace TC $^{\circ}$ OC, BC @ 24 $^{\circ}$ OC.

@

Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 4.75" o.c.
Webs : 1 Row @ 4" o.c. Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP.

Wave

Alpine Engineered Products, Inc.

Aley Drive Hv, FL 33844

BUILDING DESIGNER PER ANSI/IP! I SEC.

DRAWING INDICATES

ALPINE

IMPORTANTUPUNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FALLURE FOR DITTOR THE ENGINEERED PROMOCES, HG. STALL HIS DE RESTORMENTE FOR ANY DEVIATION FROM THIS DESIGN. ANY FALLURE TO BUILTO THE RUSS IN CONTRORMACE WITH PIECESTED ON THE RESTORMENT OF THE RESTOR

OZ SEC.J. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT HE TRUS COMPONENT OF THE

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24.0" 1.25

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WARNING IRUSEES REQUIEE EXTREME CARE IN FABRICATION, MANDEING, SHIPPING, INSTALLING AND BRACING RETER TO BECS 1 TO 3 (BULLOTHE COMPORTH SAFETY INFORMATION), PUBLICATED BY FP (FRUSS PLATE INSTITUTE, 583 D'OHOFRIO BR. SUITE 200, MADISON, HI 53719) AND MICA (MODO BRUSS COUNCIL OF ANRICA, 6300 ENTERPRISE UN, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS DIMERMISE INDICATED. FOR CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGED CEILING.

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HC-ENG

JB/AF

FL/-/4/-/-/R/-

æ R487--11

.375"/Ft. 86972

In 24" Bot chord 2x4 SP
Webs 2x4 SP PLT TYP. Alpine Engineered Products, Inc. lieu of structural panels or rigid ceiling use purlins to brace TC @ " OC, BC @ $24\,$ " OC. 0-4-3 Flames City, FL 33844
Ficate of / 2n # 567 ALPINE Wave #2 Dense #2 Dense #3 -2-0-0-**IMPORTANT**TURNISH A COPY OF THIS DISIGN TO THE INSTALLATION CONTRACTOR. ANY FALURE TO BUILD THE PRODUCTS, THC. SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DISIGN: ANY FALURE TO BUILD THE TRUSS IN CONTRAMANCE WITH TPI: OR FARRICATING, HANDLING, SHIPPING, INSTALLING & BRACCING OF TRUSSES.

DESIGN CONTRACT AND THE PROVISIONS OF ADS (MATIONAL DESIGN SPEC, BY AFAPA) AND TPI: ALPINE CONNECTOR PLATES, ARE MADE OF 20/18/166A (M. 1/5/F), ASTH AGS GRADE AD/60 (M. K/H.S) GALV, SHEEL APPLY PLATES TO EXCHE FACE OF TRUSS AND. MILES OTHERISES LOCATED ON HIS DESIGN POSITION FOR REMAINGS 160A Z PLATES TO EXCHED THE ACCOUNTY OF THE ACCO **WARNING** IRUSSES REQUIRE EXTREME CARE IN FABRICATION. MANDLING. SHIPPING, INSTALLING AND BRACING. RETER TO BEST 1 03 (BUILDING COMPONENT SAFETY WINDRANDION), PUBLISHED BY IPI (IRUSS PLAIE INSTITUTE, 583 D'OMOFRIO BR. SHITE ZOD, ANDISON, AU 153719) AND WICA (MODO BRUSS COUNCEL) OF AREIGA, 6300 ENTERPRISE LU. MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED SHALL HAVE A PROPERLY ATTACHED. BUILDING DESIGNER PER ANSI/TPI I SEC. DRAWING INDICATES 3X4(A1) =R-859 U-180 W-3.5" W Design Crit: 0-0 THIS COMPONENT FOR ANY BUILDING IS TPI 2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 12-0-0 Over 2 Supports 4 X 4 ≡ 1.5X4 III SOLELY FOR THE TRUSS COMPONENT 듑 THE RESPONSIBILITY OF 2-0-0 1.5X4 Ⅲ AND ADJUSTED AND THE MEAN NGT, AND TO DEST, WIND BC DL-5.0 anywhere in roof, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. #1 hip supports 5-0-0 jacks with no webs. 4 X 4≡ ф 5-0-0 6 R-859 U-180 W-3.5" BC LL BC DL TC DL TC LL SPACING DUR.FAC. TOT.LD. 3X4(A1) =FL/-/4/-/ M 40.0 20.0 PSF 1.25 10.0 PSF 10.0 PSF -2-0-0-V 24.0" -/R/ 0.0 PSF PSF JRFF -DATE REF SEQN-HC-ENG JB/AF DRW HCUSR487 06159117 Scale =.5"/Ft. R487--1SXW4R7 8105 06/08/06 86973 208

Alpine Engineered Products, Inc. 1950 Marley Drive Hames City, FL 33844 on # 567 In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ 24" OC. Bot PLT TYP. t chord 2x4 SP Webs 2x4 SP 0 4 3 ALPINE Wave #2 Dense #2 Dense #3 -2 - 0 - 0 -**IMPORTANT**TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ANY FAILURE TO BUILD THE PRODUCTS, INC. SAY FAILURE TO BUILD THE PRODUCTS, INC. SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION RORS THIS DESIGN. ANY FAILURE TO BUILD THE ROSS IN CONTRAMENT HIP PIL.

BESTON CONFORMS WITH APPLICABLE PROVISIONS OF 1005 (MATIONAL DESIGN SPEC, BY AFRA) AND TPI.

CONTROL OF A CONTRACT OF THE STATE AND A CONTROL OF THE STATE ***MARNING** HRISSIS REDNIBE EXTREME CARE IN FABRICATION, MANDING, SUPPHIG, INSTALLING AND BRACHEG.
RETER TO REST I DO 3 (BULDING COMPORIE SALEY MODBATION), PUBLISHED BY FPF (1888) PACATE HASTINE, SAS
D'ONDÉRIO DR. SUITE ZOD, MADISON, ALF \$37.99 AND MICA (ADOD FRES COUNCIL OF AMERICA, GADO EMERPRISE LU,
MADISON, AL \$37.99 FOR \$AVETY PRACTICES PRIOR TO PEFFORMING HEST O'UNCTIONS. UNILESS O'MERNISE INDECATED,
TOP CHORD SMALL HAVE PROPERTY ATTACHED STRUCTURAL PARIES AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED. BUILDING DESIGNER PER ANSI/IPI I SEC. RIGIO CEILING. 2X4(A1) =MD R-628 U-180 W-3.5" 9 Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) THIS COMPONENT FOR 6-0-0 ANY BUILDING 12-0-0 Over 2 Supports IRY AFRAY AND IPI. ALPHE (W. KJM.S) GALY. SIEEL. APPLY IGM. POSITION PER DRAWINGS 160A Z. 2002 SEC.3. A SEAL ON THIS Y SOLELY FOR HIE TRUSS COMPONENT IN STREET OF THE RESPONSIBILITY OF THE **@** 1.5X4 Ⅲ 4 X 4≡ ф 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is $1.50\,\mathrm{.}$.24.1 TOENS, 6-0-0 6 TATE OF 6.59687 R-628 U-180 W-3.5" ВС 8 C TC DL SPACING DUR.FAC. TC TOT.LD. FL/-/4/-/-/R/ $2 \times 4 (A1) =$ 민 M 20.0 PSF 24.0" 1.25 40.0 PSF 10.0 PSF 10.0 PSF 0.0 PSF -2-0-0--REF JRFF-SEQN DATE HC-ENG JB/AF DRW HCUSR487 06159102 Scale = .5" R487--1SXW4R7 8100 06/08/06 86974 Z08

Bot chord 2x4 SP Webs 2x4 SP Provide Provide In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ 24" OC. PLT TYP. Alpine Engineered Products, Inc. 0 3 14 ALPINE Aarley Drive $\omega \sim$ Wave 16d common nails(0.162"x3.5"),
16d common nails(0.162"x3.5"), #2 Dense #2 Dense #3 -2 - 9 - 15**WARNING** IRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING RETER TO BEST I DA (BULCING COMPONEN) SAFITY INTORNALION, PUBLISHED BY PPI (TRUSS PLATE INSTITUTE, 583 D'OHOFRIO BR. SUITE ZOO, HADISON, ALL SAFITY NEW BY ALCA (MODO TRUSS COUNCEL) OF ARRICA, 6300 ENTERPRISE UNITADISON, MI 53719) FOR SAFITY PRACIFICES PRIOR TO PERFORMING THESE FUNCTIONS. UNITES OTHERWISE INDICATED, THE CHRON SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING. DESIGN SHOWN. THE SUITABILITY AND BUILDING DESIGNER PER ANSI/TP1 1 SEC. DRAWING INDICATES 2X4(A1) =R-540 U-180 W-4.95* \mathbb{N} Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 4.24 F toe nailed toe nailed THIS COMPONENT FOR at Top chord at Bot chord -9-10-13 Over 3 Supports OF TPIE-2002 SEC.3. A SEAL ON THIS OWSIBILITY SOLELY FOR THE TRUSS COMPONENT ANY BUILDING IS THE RESPONSIBILITY OF THE **@** 110 mph wind, 15.00 ft mean hgt, ASCE 7–02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. Hipjack supports 7-0-0 setback jacks with no webs 1.5X4 7.24 59687 4 X 4 == BC DL TC DL TC LL SNISAGS DUR.FAC. TOT.LD. FL/-/4/-R-416 U-180 R 189 U-180 40.0 10.0 PSF 24.0" 1.25 20.0 PSF /-/R/-10.0 PSF 0.0 PSF PSF 9 14 REF JRFF-DATE SEQN-HC-ENG DRW HCUSR487 06159006 Scale =.5"/Ft. R487--1SXW4R7 Z08 JB/AF 2126 06/08/06 86975

Provide Provide top chord 2x4 SP Bot chord 2x4 SP In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ 24" OC. PLT TYP. Alpine Engineered Products, Inc. Haines City, FL icale of / (2) (2) ALPINE Wave 33844 on # 567 16d common nails $(0.162^*x3.5^*)$, toe nailed at Top chord. 16d common nails $(0.162^*x3.5^*)$, toe nailed at Bot chord. #2 Dense 0-4-3 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE LUSIALLATION CONTRACTOR. ALPTHE ENGINEERED PRODUCTS, THE. SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE RUSS IN COMPORANCE THIN PIL.

BESTON CONTROMANCE THIN PIL.

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COSTON CONTROMANCE THIN PIL.

COSTON CONTROMANCE THIN PIL.

CONTROMANCE THIN PIL. Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)7.
HARNING HRUSSES REQUIRE EXTREME CARE IN FABRICATION, IMADELIGE, SUPPTING, INSTALLING AND BRACCING, BEFOR 10 BCS.1 LO3 (BUILDING COMPONENT SAFETY HIPORNATION), PHRILSIFIE BY THE (TRUSS PLATE INSTITUTE, 583 D'ONORRIO BR., SUTIE 200, MADISON, 41 53719) AND MICA (MODO TRUSS COUNCELL OF AFRICA, 6300 ENTERPRISE LM, MADISON, 41 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNITES OTHERUSE INDICATED. TOP COODS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. -2-0-0-× $2X4(A1) \equiv$ R-450 U-180 W-3.5" W -7-0-0 Over 3 Supports @ 110 mph wind, 15.00 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. . 24 AN HUR R. USENSE 0.59687 90' R-182 U-180 R-77 U-180 BC DL TC DL SDACING DUR.FAC. TC TOT.LD. 3-10 FL/-/4/-/-/R/ ٺ 11-6-11 8-0-0 40.0 1.25 20.0 PSF 24.0" 10.0 PSF 10.0 PSF 0.0 PSF PSF REF SEQN-DATE JRFF-HC-ENG JB/AF DRW HCUSR487 06159103 Scal e R487--1SXWAR7 ZO8 =.5"/Ft. 8071 06/08/06 86976

In lieu 24° 0C, Provide Provide Bot chord 2x4 SP #2 Dense Alpine Engineered Products, Inc. 1950 Marley Drive Haines City, FL 33844 TYP. of structural panels or rigid ceiling use purlins to brace TC @ BC @ $24\,^{\circ}$ OC. ALPINE 2) 16d common nails(0.162"x3.5"),
2) 16d common nails(0.162"x3.5"), Wave 0-4-3 **IMPORTANT**TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ANY FAUNCES, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FALLURE TO BUILD THE TRUSCES.

BESTON CONTONNES HITH APPLICABLE PROVISIONS OF 1805 (INTIONAL DESIGN SPEC, BY ATRA) AND TPI.

CONNECTION PLAISES ARE MADE OF 20/18/19/GAG (M-11/55/2) ASTH ASS GRADE 40/50 (M. KYLES), GALV. SIEEL. APPLY PARES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE TOCATED ON THIS DESIGN. POSITION FOR BRANHINGS 160A Z.

ANY INSPECTION OF PLAIES COLONDED BY (1) SHALL BE FER AIMER AS OF THIS 2002 SEC.3.

ASSA AND THIS DESIGN SOCIETARES OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT OF SHALL AND SECONDOLLY HERE AS OF THIS 2002 SEC.3.

ANY INSPECTION OF PLAIES COLONDED BY (1) SHALL BE FER AIMER AS OF THIS 2002 SEC.3.

ASSA ANY INSPECTION OF PLAIES TABLETLY AND USE OF THIS 2003 SEC.3.

ANY INSPECTION OF PLAIES TABLETLY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE **MARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. RETER TO BEST I 03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (RRUSS PLATE INSTITUTE, 583 0 "OUDDERTO DR. SUITE 2005, MADISON, HE 53719) AND META (ADDOD RAUSS COUNCIL OF AREATA, 6300 ENTERPRISE LH, HADISON, HE 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNITES OHICRATES INDICATED, TOP CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOSTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING. -2-0-0-Design Crit: 2X4(A1) =toe nailed at Top chord. toe nailed at Bot chord. MD R-377 6 U=180 W-3.5" -5-0-0 Over TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 3 Supports 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. R-48 U-180 R-120 U-180 PENSE 59687 90' ~ 10 ů 10-6-11 BC LL BC DL TC DL DUR.FAC. SPACING TC TOT.LD. FL/-/4/-20.0 24.0" 40.0 /-/R/-1.25 10.0 PSF 10.0 PSF 0.0 PSF PSF PSF SEQN-JRFF-DATE REF HC-ENG DRW HCUSR487 06159104 Scale =.5"/Ft. R487--1SXW4R7 Z08 JB/AF 8075 06/08/06 86977

Provide Provide In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. Bot chord 2x4 SP #2 Dense PLT TYP. Alpine Engineered Products, Inc. ficate of ' 'on # 567 ALPINE 2) 16d common nails (0.162 "x3.5 "), toe nailed at Top chord. 2) 16d common nails (0.162 "x3.5 "), toe nailed at Bot chord. Wave **IMPORTANT**FIRMISH A COPY OF THIS DESIGN TO THE THISTALLATION CONTRACTOR. ALPTHE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE RUSSES IN CONFIDENCE HIS PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR THIS PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR THIS PROPERTY OF THIS PROPERTY OF THE PROPERTY OF THIS PROPERTY OF THE CONFIDENCE OF THE PROPERTY OF ***MARNING** IRUSSES REQUIRE EXTRÊME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST I O3 (BUILDING COMPORIET) RAFETY INFORMATION). PUBLISHED BY IPI (TRUSS PLATE INSTITUTE, 583 0 "OHOMERIO BR. SUITE 200, MADISON, H. 53719) AND MEA MADISON SESSON BETT OF A SOAD ENTERPRISE IN, HADISON, H. 53719) FOR SAFELY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNILESS OHERHISE INDICATED. THE CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CEILING. DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. -2-0-0-Φ Design Crit: 2X4(A1) =R=317 U=180 W=3.5" W Over 3 Supports TPI 2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) R-15 U-180 R-49 U-180 110 mph wind, 15.00 ft mean hgt, ASCE /-02, CLOSED bidg, Located anywhere in roof, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. 7.24 9 6 11 8-0-0 o. 59687 90' SPACING TC DL TC LL DUR.FAC. 8 C ВС TOT.LD. FL/-/4/-/-/R/-PL 24.0" 1.25 20.0 PSF 40.0 PSF 10.0 PSF 10.0 PSF 0.0 PSF JRFF SEQN-DATE REF HC-ENG DRW HCUSR487 06159008

Scale =.5"/Ft. R487--

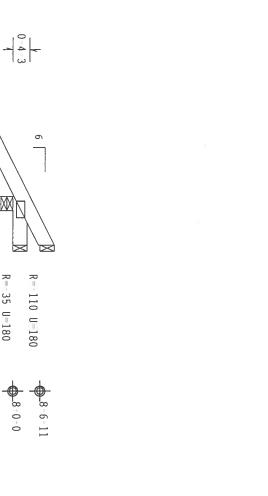
1SXW487

208

TCE/AF 87850

06/08/06 86978

Provide (Provide (In lieu of structural panels or rigid ceiling use purlins to brace TC $24\,\text{m}$ OC, BC @ $24\,\text{m}$ OC. Bot chord 2x4 SP #2 Dense 2) 16d common nails (0.162"x3.5"), toe nailed at Top chord. 2) 16d common nails (0.162"x3.5"), toe nailed at Bot chord. **@**





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 $2X4(A1) \equiv$

0-10-3

WARNING IRUSSES REDUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING. INSTALLING AND BRACING RETER TO BEST 1 D3 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE. 583 D**OHOTRIO BG. STITE ZOO. ANDISON. MI 53719) AND MICA (MODO TRUSS COUNCEL) OF ARREAS, 6300 ENTERPRISE LIN. HADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING INESE FUNCTIONS. UNICESS DIMERMISE INDICATED. TOP CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED. Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

> ZENSE lo. 59687

> > FL/-/4/-/-/R/-

PLT TYP.

Wave

DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

90'

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DRW HCUSR487 06159009

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DATE

06/08/06 86979

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Scale = .5"/Ft. R487---

ВС

SPACING

24.0"

JRFF-

1SXW487 Z08

DUR.FAC. TOT.LD.

40.0 1.25

SEQN

HC-ENG

JB/AF

Alpine Engineered Products, Inc. 1950 Marley Drive Haines City, FL 33844

ALPINE

IIU mph wind, 15.00 ft mean hgt, ASCE 7-02, CLUSED bidg, Located anywhere in roof, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

PLT TYP. Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ 24" OC. Bot chord 2x4 SP #2 Dense Alpine Engineered Products, Inc Haines City, FL 33844 Ticate of / 2n # 567 ALPINE Wave 0-3-14 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FALLURE TO BUILD THE PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY EVILLOR FOR THIS DESIGN. ANY FALLURE TO BUILD THE TRUSS IN COMPORMACE WITH HE!

OF FABRICATION, HE WITH APPLICABLE PROVISIONS OF ANDS (MATIONAL DESIGN SPEC. BY AFRA) AND TP!

CONNECTOR PLATES ARE HADE OF Z07/18/16/36A (M.14/5/4) ASIM AG53 GRADE 40/66 (M. K/M.5) GALV. STEEL. APPLY
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERNISE LOCATED ON THIS DESIGN, POSITION PER DRAWHIGS 160A. J.

ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNER AS OF TP11 2002 SEC.3. A SEAL ON THIS
DRAWHIG INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLICY FOR THE THAS COMPONENT

DESIGN SHOWN. THE SULTABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/IP1 1 SEC. 2. RIGIO CEILING. -2-9-15 Design Crit: 2X4(A1) =4.24 [Ma -392 U-180 W-4.95" TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 7-0-14 **®** 0ver 3 Supports Provide Provide 110 mph wind, 15.00 ft mean hgt, ASCE 7–02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 Hipjack supports 5-0-0 setback jacks with no (2) 16d common nails (0.162"x3.5"), toe nailed at Top chord. (2) 16d common nails (0.162"x3.5"), toe nailed at Bot chord. THE WENSE! 0.59687 TY:5 R-70 U-180 R-200 U-180 SPACING TC DL ВС DUR.FAC ВС T TOT.LD. FL/-/4/-P 2 webs. 9 40.0 20.0 /-/R/ 24.0" 10.0 PSF 1.25 0.0 10.0 PSF 14 **→** 10 6 6 800 PSF PSF PSF SEQN DATE REF JRFF-HC-ENG DRW HCUSR487 06159007 Scal le =.5"/Ft. R487---1SXW4R7 JB/AF 99786 06/08/06 86980 Z08

Provide Provide Bot D In lieu of structural panels or rigid ceiling use purlins to brace TC $24\,\text{"}$ OC, BC @ $24\,\text{"}$ OC. PLT TYP. Alpine Engineered Products, Inc. 1950 Marley Drive chord 2x4 SP #2 Dense Haines City, FL 33844 Trate of / on # 567 ALPINE ~ ~ ___ Wave 16d common nails (0.162"x3.5"), toe nailed 16d common nails (0.162"x3.5"), toe nailed 0 4 3 **WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, INNOLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BEST I 03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, 503)
DO-ONDERIO DR., SUITE 200, MADISON, MI 53719) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE IN, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE TUNCTIONS. UNLESS OTHERHISE INDICATED, TOP CHOBO SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGIO CELLING. ***IMPORTANT***TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FALLURE TO BUILD HE PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN: ANY FALLURE TO BUILD HE RISSS IN CONTRAMANCE WITH HE!.

OF TABRICATION, HOLD, SHIPPING, HISTALLING BRACHEO OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROPUSIONS OF THIS MAIN HOLD, SHIPPING, HESTALLING BRACHEO OF THE LAPINE CONFICENCY PARTS AND HE FOR CONTRACTOR PARTS AND HE FOR CONTRACTOR PARTS AND HE FOR AND HELD, AND HE FOR AND HELD SO THE HIS DESIGN POSITION PER DRAHING. 190A J. ANY HESPECTION OF PLATES FOLLOWED BY (1) SHALL BE FOR ANNEX AS OF TPIT 2002 SEC. J. A SEAL ON HIS BRACHEO OF PLATES FOLLOWED BY (1) SHALL BE FOR ANNEX AS OF TPIT 2002 SEC. J. A SEAL ON HIS BRACHEO IN HIS DESIGN AS CEPTANCE OF PROPESSIONAL ENGINEERISE CONTRACTOR OF THE TRUSS COPPONENT DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI I SEC. 2. -2-0-0-V SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF Design Crit: 2X4(A1) =R-377 U-180 W-3.5" W 6 -5-0-0 Over at Top chord. at Bot chord. TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) ω Supports **@** IIU mph wind, 15.00 ft mean hgt, ASCt /-02, CLUSED bidg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. R-120 U-180 R-48 U-180 TOENS O 6.59687 OF OF 10 ىن 10-6-11 8 0 0 TC DL ВС ВС TC SPACING DUR.FAC. FL/-/4/-/-/R/-TOT.LD. 민 20.0 40.0 1.25 10.0 PSF 24.0" 10.0 PSF 0.0 PSF PSF PSF DATE REF JRFF-SEQN-HC-ENG DRW HCUSR487 06159010 Scale =.5"/Ft. R487--1SXW487 Z08 JB/AF 99551 06/08/06 86981

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ $24\mbox{\,}^{\circ}$ OC. Bot chord 2x4 SP #2 Dense Alpine Engineered Products, Inc PLT TYP. ficate of / ' on # 567 ALPINE Wave **IMPORTANT***URRISH A COPY OF THIS DESIGN TO THE THISTALLATION CONTRACTOR.

ALPHIC CHICAGE
PRODUCTS, INC. SHALL HOT BE RESPONSIBLE FOR ARMY DEVIATION ROW THIS DESIGN. ANY FALLERE TO BUILD THE
RROSE IN CONTRAMANCE WITH HE!

FROMET SHE CONTRAMACE WITH HE!

DESIGN CONFORMS HITH APPLICABLE PROVISIONS OF HOS (MALIGNAL DESIGN SPEC, BY AFREA) AND FP!

APPLICABLE PROVISIONS OF HOS (MALIGNAL DESIGN SPEC, BY AFREA) AND FP!

CONNECTOR PLATES ARE MADIO OF 20/18/16/AGA (M-1/5/27) ASTH AGAS GRADE 40/60 (M. K/H.S) GAVE. STEEL APPLY

PLATES 10 EACH FACE OF TRUSS AND, DHEESS OTHERHISE LOCATED ON THIS DESIGN. POSITION PER BRAHHGS 160A Z.

ANY INSPECTION OF PLATES TOLOWED BY (1) SHALL BE FER ANNY AS OF THIS 2002 SEC. 3.

ASSAL ON HITS SECOND OF PLATES FOLLOWED BY (1) SHALL BE FER ANNY AS OF THIS 2002 SEC. 3.

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ANY INSPECTION OF PLATES FOR THIS SECOND BY THE PLATE BY THE PLATE SECOND BY THE FER ANY AS OF THE PLATE SECOND BY THE PLATE BY THE PLATE SECOND BY THE PLATE BY THE PLA **WARNING** INISES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SUPPRING, INSTALLIG AND BRACING RETER TO BEST TO SUBULDING COMPORINT SAFETY INFORMATION), PUBLISHED BY TPI (RUSS PLATE INSTITUTE, 583 D CHOPFRIO BR., SUFIE ZOO, HADISON, WI 53719) AND MICA (MODO BRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LIFT, HADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED, UP CHORD SMALL HAVE A PROPERLY ATTACHED RIGHD CELLING. DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. THE SUITABILITY AND USE OF THIS COMPONENT FOR -2-9-15 Design Crit: 4.24 $2X4(A1) \equiv$ Ma R-319 U-180 W-4.95" <--4-2-15 Over 3 Supports → TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) ANY BUILDING IS THE RESPONSIBILITY OF **@** Provide Provide 110 mph wind, 15.00 ft mean hgt, ASCE /-02, CLUSED bidg, Located anywhere in roof, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL=5.0 Hipjack supports 3:0:0 setback jacks with no Ħ R-33 U-180 R--7 U-180 2) 16d common nails (0.162"x3.5"), toe nailed at Top chord 2) 16d common nails (0.162"x3.5"), toe nailed at Bot chord LENSE TATE OF o. 59687 90 1 - 9 - 149 6 6 8-0-0 BC LL SPACING BC DL TC DL TC LL DUR.FAC. TOT.LD. FL/-/4/-/-/R/webs. 20.0 40.0 24.0" 1.25 10.0 PSF 10.0 PSF 0.0 PSF PSF PSF JRFF-REF SEQN DATE HC-ENG DRW HCUSR487 06159118 Scale = .5"/Ft. R487--15XW487 Z08 AF/AF 8240 06/08/06 86982

Bot chord 2x4 SP #2 Dense

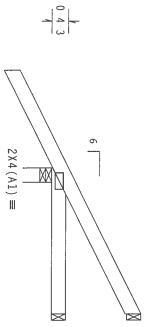
In lieu of structural panels or rigid ceiling use purlins to brace TC $24\,^{\circ}$ OC, BC @ $24\,^{\circ}$ OC.

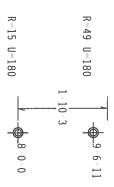
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Provide Provide 2) 16d common nails(0.162"x3.5"), toe nailed at Top chord. 2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7.02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.





2-0-0-R=317 U-180 W=3.5" 3 0 0 Over 3 Supports

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT TYP.

Wave

WARNING IRUSSIS REQUIRE EXIRENE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, RETER TO BCSI. TO GUDILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY FPI (RRUSS PLATE INSTITUTE, 583 0"ONDERIO DR. SUITE 700", MADISON, H. 153719) AND MECA (MADOD RRUSS COUNCIL OF ARRICA, 6300 ENTERPRISE LH, MADISON, H. 153719) FOR SAFETY PRACTICES PRIOR TO PERFORMING INESE FUNCTIONS. UNICES OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FALLINE FOOGLEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN: ANY FALLINE TO BUILD THE RUSS IN CONFORMANCE WITH HE!

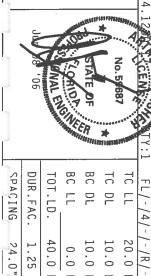
OF FABRICALING, MANDLES HE SADE OF TRUSSES, DESIGN SOFE, IN STATE OF THE SECS.
DESIGN CONTROMS WITH APPLICABLE PROVISIONS OF NOS (MAIJONAL DESIGN SPEC, BY ATRAD, AND IPI. APPLICABLE TO FACIL FACE OF TRUSS AND, DIRECTOR PALATES ARE HADE OF ZO/120 HEAGA, CH.H/S/ZH, ASHING AS GRADE GO/GO, CH. K/H.S) GAV. STEEL. APPLY
PLATES TO FACIL FACE OF TRUSS AND, DIRECTS OTHERWISE (COCATED ON HIS DESIGN, POSITION FOR BRANHOS 150A Z.

ANY HISTORIANO OF PLATES TOLOHOUR BY C1) SHALL BE FER ANNEX AS OF IPI ZOOZ SCC. 3.

AS SEAL ON THIS DESIGNER FER ANSI/IPI I SEC. 2.

Alpine Engineered Products, Inc. ficate of / on # 567

ALPINE



10.0 PSF 10.0 PSF 0.0 PSF 40.0 PSF	BC DL 10.0 PSF BC LL 0.0 PSF	TOT.LD. 40.0 PSF SEQN-	DIIR FAC 1 25	NEST THE PROPERTY OF THE PARTY	TC DL BC DL BC LL TOT.LD.	20.0 PSF 10.0 PSF 10.0 PSF 0.0 PSF 40.0 PSF	R48 HCUSF
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Scale =.5"/Ft.

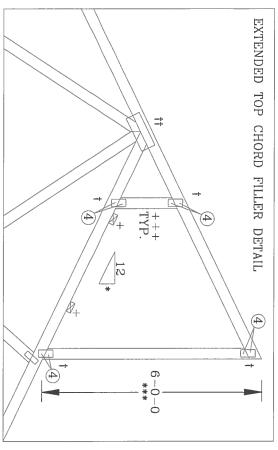
24.0"

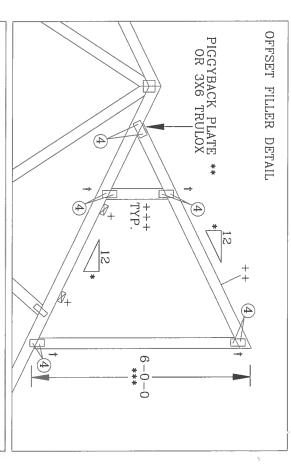
JRFF-

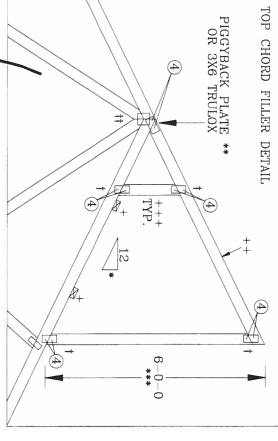
1SXW4R7 Z08

TOP CHORD FILLER DETAIL

- + 2X4 CONTINUOUS LATERAL BRACING AT 24" OC MAXIMUM SPACING. ATTACH TO EACH TOP CHORD WITH (2) 16d NAILS. BRACING MATERIAL TO BE SUPPLIED AND ATTACHED AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.
- ++ 2X4 SO. PINE #2 N OR SPF #1/#2 FILLER TOP CHORD.
- +++ 2X4 SO. PINE #3 48" OC MAXIMUM. OR SPF #1/#2 VERTICAL WEBS SPACED
- * 8/12 MAXIMUM PITCH.
- ** 2X8.25 PIGGYBACK SPECIAL PLATE. SEE I FOR PIGGYBACK SPECIAL PLATE INFORMATION. SEE DRAWING PIGBACKB0699
- *** 6'0" MAXIMUM HEIGHT.
- † W2X4 OR 3X6 TRULOX.
- †† REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN
- EACH FACE OF EACH TRUSS PLY. SEE DWG 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS. 11 GAUGE (0.120")X1.375" NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO





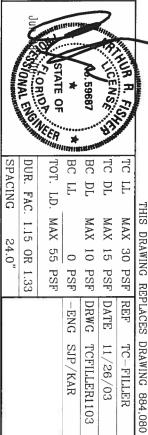




IMPORTANI FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION PROBH THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONVERNANCE WITH THE JOE FABRICATING, HANDLING, SHEPPING, INSTALLING BEACKO OF TRUSSES. DESIGN CONVERNS WITH APPLICABLE PROVISIONS OF HIS CHAITCHAIL DESIGN SEC. BY AFRENA AND TEL ALPINE CONNECTOR PLATES ARE MADE OF 20/18/1564 V. MILESS OTHERWISE LOCATED BY AFRENA AND THE ALPINE CONNECTOR PLATES AND INSPECTION OF PLASS AND, UNICESS OTHERWISE LOCATED IN THIS DESIGN, POSITION PER DRAVINGS 160A-7. ANY INSPECTION OF PLASS AND, UNICESS OTHERWISE LOCATED IN THIS DESIGN, POSITION PER DRAVINGS 160A-7. ANY INSPECTION OF PLASS AND, UNICESS OTHERWISE OF SHALL BE PER ANNEX AS OF TELL APPLY PLATES TO ANY INSPECTION OF PLASS AND SHOWN THE PROFESSIONAL ENGINEERING RESPONSIBILITY SOLL FOR THE PROSE SCHOOLING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLL FOR THE PROSE SCHOOLING TO THE BUILDING IS THE RESPONSIBILITY OF THE BUILDING SUITABLITY AND USE OF THE BUILDING

POMPANO BEACH, FLORIDA

ALPINE



24.0"

55 0

PSF PSF PSF

DRWG DATE

TCFILLER1103

-ENG

SJP/KAR

REF

TC-FILLER

11/26/03

BOTTOM CHORD FILLER DETAIL

SIZES (1X3 WAVE) MAY BE USED IF BEARING IS OMITTED. WEDGE OR VERTICAL MEMBER MUST COINCIDE WITH BEARING LOCATION. OPTIONAL INTERIOR OR CANTILEVER BEARING. MINIMUM PLATE

ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO EACH FACE OF THE TRUSS. SEE DWG 160TL FOR NAILING AND 11 GAUGE (0.120")X1.375" NAILS REQUIRED FOR TRULOX PLATE TRULOX PLATE REQUIREMENTS.

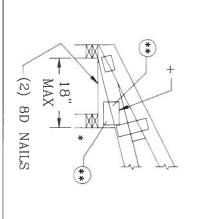
3X4 WAVE OR 4X8 TRULOX

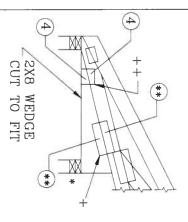
++ 2X4 WAVE OR 3X6 TRULOX

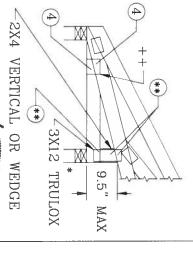
SHOWN DETAIL REFER FOR LUMBER, PLATES, AND OTHER INFORMATION NOT TO ENGINEER'S SEALED DESIGN REFERENCING THIS

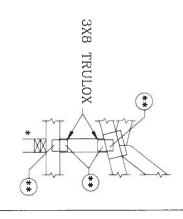
TRULOX PLATES SHOWN ARE MINIMUMS. LARGER PLATES BE REQUIRED TO ACCOMODATE REQUIRED NAILS (**)

FILLER BOTTOM CHORD	MAXIMUM REACTION	EACTION	MINIMIN	** REQUIRED	D NAILS PER	R FACE WITH	1 TRULOX P	PLATES
OR WEDGE SPECIES	DOWNWARD	UPLIFT	EΑ	1.00 D.O.L.	1.15 D.O.L.	1.25 D.O.L.	1.33 D.O.L.	1.60 D.O.L.
DOUGLAS FIR-LARCH	3281#	1656#	1.5" X 3.5"	12	11	10	9	8
HEM-FIR	2126#	1095#	1.5" X 3.5"	9	8	7	7	6
SPRUCE-PINE-FIR	2231#	1192#	1.5" X 3.5"	10	9	8	8	6
SOUTHERN PINE DENSE	3465#	1791#	1.5" X 3.5"	12	11	10	9	8
SOUTHERN PINE	2966#	1492#	1.5" X 3.5"	10	9	8	8	7
SOUTHERN PINE NON-DENSE	2520#	1343#	1.5" X 3.5"	9	8	7	7	o



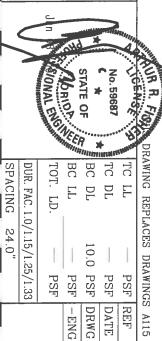






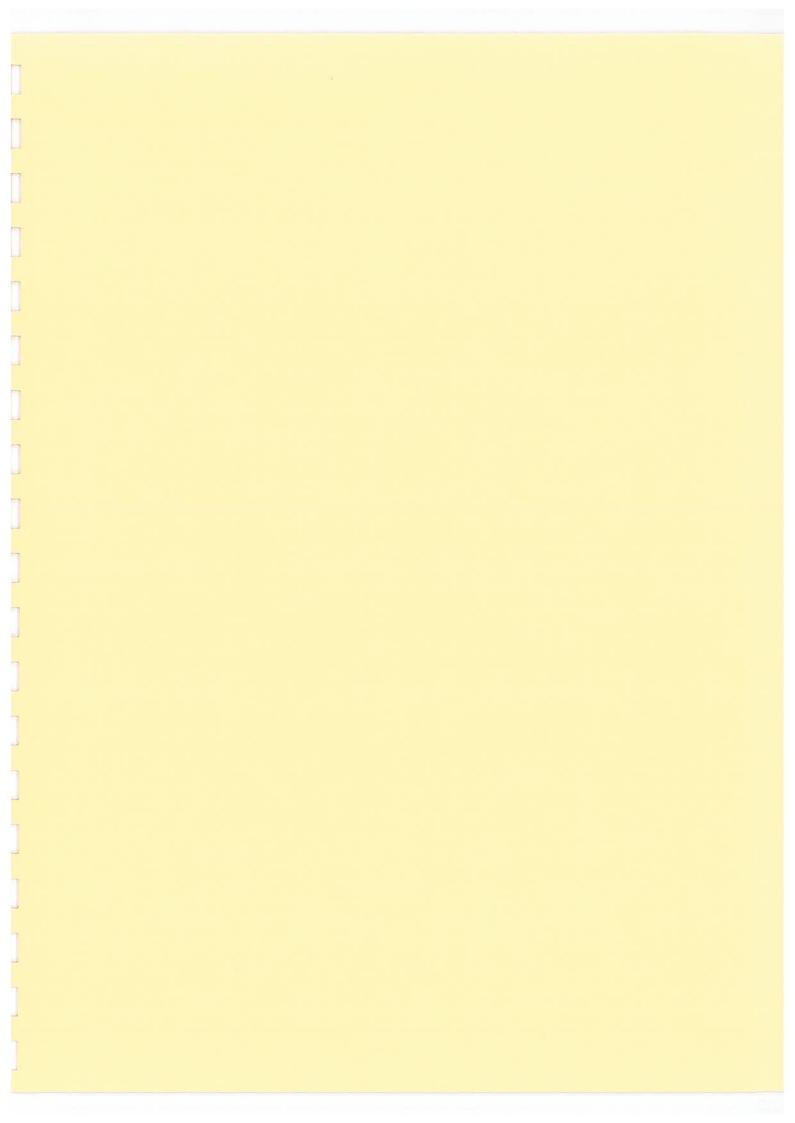


WAVARNIOWA TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESI 1—30 BOILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI CIRUSS PLATE INSTITUTE. 583 D'ROUFRID DR., SUITE 200, HADISON, VI. 53719) AND VTCA (VODIO TRUSS COUNCIL OF AHERICA, 6300 ENTERPRISE LN, HADISON, VI. 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNLESS DIFFERVISE INDICATED, TOP CHARD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



	>	8 MONAL ENGINE	A DA OF VEE	STATE OF	No.59687	A 100 06 100
SPACING 24.0"	DUR. FAC. 1.0/1.15/1.25/1.33	TOT. LD.	BC LL	BC DL	TC DL	TC LL
4.0"	.15/1.25/1.33	- PSF	- PSF	10.0 PSF	- PSF	PSF
			-ENG	DRWG	PSF DATE	PSF REF
			-ENG DLJ/KAR	10.0 PSF DRWG BCFILLER1103	11/26/03	BC FILLER

A115/R & 884,132



Data entry by: MT Date: 06 01 06 Project name: ZEBRA CT. Back-1 Location : COLUMBIA COUNTY RESIDENTIAL WIND DESIGN AND ANALYSIS A product of EDA Software, Inc. Based on the Standard Building Code, 1994 edition **** GENERAL INPUT DATA **** Permanent construction Simple rectangular building ______ Bearing wall at roof level <---Plan outline of residence ----Ridge----Width End wall---> Bearing wall at roof level ______ Length along bearing walls out to out of studs = 66 feet

Length along bearing walls out to out of studs = 66 feet
Width along end walls out to out of studs = 28 feet
Roof overhang in long direction from outer face of stud = 2 feet generally
Roof overhang at short end wall from outer face of stud = 2 feet generally
Height of exterior wall to top of plate on long side = 8 feet constant
Roof cross slope = 6 /12

Wind velocity = 100 mph

**** DEGREE OF ENCLOSURE ****

Assume that this building is an 'Enclosed building' per Code 1606.2.3.

1) 051) W ARTOOS 27 AMZKG

**** STRUCTURAL FRAMING INPUT DATA **** *** Roof Structural Data *** Member number 1 Jack truss--hip-ended roof

Span length out to out of supports = 28 feet Roof cross slope = 6/12

Truss spacing = 24 inches Overhang = 2 feet

Member number 2

Jack truss--hip-ended roof

Span length out to out of supports = 22 feet

Roof cross slope = 6/12Truss spacing = 24 inches Overhang = 2 feet

*** Wall Structural Data ***

Spacing of wall studs = 16 inches Total number of plates = 3

Wall stud number 1 is 8 feet high out to out of plates

COEFFICIENTS AND PRESSURES Main Wind Force Resisting Systems

Actual pressure = Velocity pressure x Use factor x Coefficient Wind velocity is 100 mph Mean roof height is 11.87268 feet Velocity pressure is 20.4 psf Use factor is 1.0

Roof cross slope is 6 on 12, which equals 26.56505 degrees to horizontal End zone width is 6 feet

		Coefficient	Design Pressure (psf)
End zone			
Windward wall	(1E)	. 7	14.28
Windward roof	(2E)	-1	-20.4
Leeward roof	(3E)	-1	-20.4
Leeward wall	(4E)	95	-19.38
Overhang		-1.5	-30.6
Interior zone			
Windward wall	(1)	. 4	8.16
Windward roof	(2)	75	-15.3
Leeward roof	(3)	75	-15.3
Leeward wall	(4)	7	-14.28
Overhang		-1.5	-30.6
============	========		=======================================

```
ROOF LOADING--Roof Number 1 (pounds per square foot)
Roof cross slope = 6 inches per foot
Fiberglass shingles 240 # per square and 1 layer of 15 # felt = 2.55
No insulation
7/16 in. roof sheathing
2 in. x 4 in. wood trusses at 24 in. spacing
_____
Total roof unit weight on slope
                                             = 6.075148
Cosine of roof cross slope
_____
Roof unit weight on horizontal
1 layer of 1/2 in. gypsum board ceiling--plain
                                             = 2
Ceiling insulation R-30
                                             = .5
                                              = 1
Air-conditioning ductwork
Full lighting
Miscellaneous
______
                                              = 10.59222
Roof Unit Dead Load = 11 psf
Roof dead load supported generally by wall = 159.7911 plf
ROOF LOADING--Roof Number 2 (pounds per square foot)
Roof cross slope = 6 inches per foot
______
Fiberglass shingles 240 # per square and 1 layer of 15 # felt = 2.55
No insulation
7/16 in. roof sheathing
2 in. x 4 in. wood trusses at 24 in. spacing
- -
Total roof unit weight on slope
                                             = 6.075148
Cosine of roof cross slope
                    ______
_____
                                             = 6.792222
Roof unit weight on horizontal
                                             = 2
1 layer of 1/2 in. gypsum board ceiling--plain
Ceiling insulation R-30
                                             = .5
Air-conditioning ductwork
Full lighting
Miscellaneous
______
                                              = 10.59222
Total
```

Roof Unit Dead Load = 11 psf

Roof dead load supported generally by wall = 159.7911 plf

```
Roof cross slope = 6 inches per foot
Fiberglass shingles 240 # per square and 1 layer of 15 # felt = 2.55
No insulation
7/16 in. roof sheathing
2 in. x 4 in. wood trusses at 24 in. spacing
_ _ _ _
Total roof unit weight on slope
                                             = 6.075148
Cosine of roof cross slope
Roof unit weight on horizontal
                                             = 6.792222
1 layer of 1/2 in. gypsum board ceiling--plain
                                             = 2
Ceiling insulation R-30
                                             = .5
                                             = 1
Air-conditioning ductwork
Full lighting
Miscellaneous
_______
                                             = 10.59222
Roof Unit Dead Load = 11 psf
Roof dead load supported generally by wall = 159.7911 plf
ROOF LOADING--Roof Number 4 (pounds per square foot)
Roof cross slope = 6 inches per foot
_~~----
Fiberglass shingles 240 # per square and 1 layer of 15 # felt = 2.55
No insulation
7/16 in. roof sheathing
2 in. x 4 in. wood trusses at 24 in. spacing
- -----
Total roof unit weight on slope
                                             = 6.075148
Cosine of roof cross slope
_____
                                             = 6.792222
Roof unit weight on horizontal
1 layer of 1/2 in. gypsum board ceiling--plain
                                             = 2
Ceiling insulation R-30
                                             = .5
                                             = 1
Air-conditioning ductwork
Full lighting
Miscellaneous
_____
                                             = 10.59222
Total
```

ROOF LOADING--Roof Number 3 (pounds per square foot)

Roof Unit Dead Load = 11 psf

Roof dead load supported generally by wall = 159.7911 plf

ROOF MEMBER DEAD LOAD REACTIONS AT BEARINGS All values are in pounds

Roof	member	number	1	Span	28	feet,	Slope 6	/12,	interior zone	319
									end zone	
									interior zone	
Roof	member	number	4	Span	22	feet,	Slope 6	/12,	end zone	256

EXTERIOR WALL LOADING (pounds per linear foot)

Wood frame wall-- 8 ft. out to out plates

32 in. x 4 in. plates 2 in. x 4 in. studs at 16 in. spacing R-13 Insulation	= 2.865625 = 5.462598 = 1.90625
3/8 in. Mineral board siding 1/2 in. Gypsum boardTotal 1 layer	= 12.1875 = 16
Total	= 38.42197

Exterior Wall Unit Dead Load = 39 plf

SUMMARY OF HURRICANE ANCHOR ANALYSIS

All values of forces are in pounds. Resistances have been increased for wind. End zone width = 6 feet

Code: C = Compliance

N = Non-compliance

Simpson hurricane anchors

Member 1 --Hip roof--Span 28 feet, at 24 inches oc--in interior zone: Uplift = 771 Dead = 319 Net = 452 Model Special, Resistance = 717 C Model H9--all nails installed per manufacturers catalog Data supplied by operator--not from EDA database

Member 2 --Hip roof--Span 28 feet, at 24 inches oc--in end zone: Uplift = 771 Dead = 319 Net = 452 Model Special, Resistance = 717 C Model H9--all nails installed per manufacturers catalog Data supplied by operator--not from EDA database

Member 3 --Hip roof--Span 22 feet, at 24 inches oc--in interior zone: Uplift = 638 Dead = 256 Net = 382 Model Special, Resistance = 717 C Model H9--all nails installed per manufacturers catalog Data supplied by operator--not from EDA database

Member 4 --Hip roof--Span 22 feet, at 24 inches oc--in end zone: Uplift = 638 Dead = 256 Net = 382 Model Special, Resistance = 717 C Model H9--all nails installed per manufacturers catalog Data supplied by operator--not from EDA database **** ANALYSIS OF ROOF SHEATHING AS SHEAR DIAPHRAGM TRANSVERSE **** Shear analysis applies along supporting shearwalls.

Roof trusses are Southern Pine lumber, spaced at 24 inches Sheathing is Oriented Strand Board, 7/16 inch thick Sheathing has no intermediate blocking Fasteners on panel ends are 8d nails spaced at 5 inches Fasteners in panel interior are 8d nails spaced at 10 inches

Total lateral wind force on building = 11911 pounds
Total force transferred through diaphragm to shearwalls = 5955 pounds
Total length of shearwalls = 56 feet

MINIMUM REQUIRED TOTAL SHEARWALL LENGTH = 23.7 FT.--LOCATE EVENLY THROUGHOUT

Actual diaphragm force per unit length of shearwall = 106 plf Allowable diaphragm force per unit length of shearwall = 251 plf

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*** Summary of Analysis ***
Roof sheathing diaphragm satisfies Code requirements.

**** ANALYSIS OF ROOF SHEATHING AS SHEAR DIAPHRAGM LONGITUDINAL ****

Shear analysis applies along supporting shearwalls.

Roof trusses are Southern Pine lumber, spaced at 24 inches

Sheathing is Oriented Strand Board, 7/16 inch thick Sheathing has no intermediate blocking Fasteners on panel ends are 8d nails spaced at 5 inches Fasteners in panel interior are 8d nails spaced at 10 inches

Total lateral wind force on building = 4090 pounds
Total force transferred through diaphragm to shearwalls = 2045 pounds
Total length of shearwalls = 132 feet
MINIMUM REQUIRED TOTAL SHEARWALL LENGTH = 7.9 FT.--LOCATE EVENLY THROUGHOUT

Actual diaphragm force per unit length of shearwall = 15 plf Allowable diaphragm force per unit length of shearwall = 251 plf

*** Summary of Analysis ***
Roof sheathing diaphragm satisfies Code requirements.

**** ANALYSIS OF ROOF SHEATHING FOR FASTENER WITHDRAWAL ****

Interior zone (area Ri)
Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is 7/16 inch with no intermediate blocking
Size of sheathing is 48 inches by 96 inches
Fasteners along end trusses are 8d nails spaced at 5 inches
Fasteners along int. trusses are 8d nails spaced at 10 inches
Total outward wind force on sheathing = 656 pounds
Total withdrawal resistance of 40 nails = 3038 pounds (increased for wind)
Fastening of roof sheathing satisfies Code requirements.

Edge strip (area Si) width = 3 feet
Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is 7/16 inch with no intermediate blocking
Size of sheathing is 48 inches by 96 inches
Fasteners along end trusses are 8d nails spaced at 5 inches
Fasteners along int. trusses are 8d nails spaced at 10 inches
Total outward wind force on sheathing = 1024 pounds
Total withdrawal resistance of 40 nails = 3038 pounds (increased for wind)
Fastening of roof sheathing satisfies Code requirements.

End zone (areas Se and C) width = 6 feet
Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is 7/16 inch with no intermediate blocking
Size of sheathing is 48 inches by 96 inches
Fasteners along end truss are 8d nails spaced at 5 inches
Fasteners along end wall are 8d nails spaced at 5 inches
Fasteners along int. trusses are 8d nails spaced at 10 inches
Total outward wind force on sheathing = 1417 pounds
Total withdrawal resistance of 40 nails = 3038 pounds (increased for wind)
Fastening of roof sheathing satisfies Code requirements.

```
*** Analysis of Wall Stud Number 1 ***
2 in. x 4 in. single studs at 16 in. spacing
Stud height is 7.625 feet--located in interior zone
Top of studs is laterally supported by ceiling diaphragm or other method
Spruce--Pine--Fir lumber----Number 1--Number 2 grade
Sheathing is inch rated OSB, span rating 24/16
                               = 5.25 \text{ sq.in.}
Cross-sectional area
Moment of inertia
                               = 5.359375 in.^4
Section Modulus
                               = 3.0625 in.^3
Elastic modulus of wood stud = 1400000 in.^2
Total outward force on stud = 268 pounds
                                = 255 \text{ ft-lb.}
Stud moment
Stresses:
Stud bending vert : Actual = 1000 psi Allowable = 2415 psi (adjusted)
Stud shear : Actual = 35 psi Allowable = 112 psi (adjusted)
Stud tensile : Actual = 33 psi Allowable = 1020 psi (adjusted)
Interaction bending and tension actual/allowable stress ratio total = .4464316
   Sheathing bending hor: Actual = 146 psi Allowable = 222 psi(adjusted)
Deflections:
   Stud : Actual = .2226 in. Allowable = .5083 in.
______
*** Summary of Analysis ***
Wall structure satisfies all Code requirements.
```

**** ANALYSIS OF WALL STUDS ****

```
*** Analysis of Wall Stud Number 2 ***
2 in. x 4 in. single studs at 16 in. spacing
Stud height is 7.625 feet--located in end zone
Top of studs is laterally supported by ceiling diaphragm or other method
Spruce--Pine--Fir lumber----Number 1--Number 2 grade
Sheathing is inch rated OSB, span rating 24/16
Cross-sectional area = 5.25 sq.in.

Moment of inertia = 5.359375 in.

= 3.0625 in.^3
                                  = 5.359375 in.^4
Elastic modulus of wood stud = 1400000 in.^2
Total outward force on stud = 309 pounds
Stud moment
                                   = 294 \text{ ft-lb}
Stresses:
   Stud bending vert : Actual = 1154 psi Allowable = 2415 psi (adjusted)
Stud shear : Actual = 40 psi Allowable = 112 psi (adjusted)
Stud tensile : Actual = 33 psi Allowable = 1020 psi (adjusted)
Interaction bending and tension actual/allowable stress ratio total = .5101997
   Sheathing bending hor: Actual = 169 psi Allowable = 222 psi(adjusted)
Deflections:
   Stud : Actual = .2567 in. Allowable = .5083 in.
*** Summary of Analysis ***
```

Wall structure satisfies all Code requirements.

**** ANALYSIS OF WALL STUDS ****

```
**** ALLOWABLE STRESS PROPERTIES ****
Base stresses (psi):
Wood:
   Bending = 875
Tension = 425
Shear = 70
   Shear
                  = 70
   Elastic modulus = 1400000
Adjustment factors for wood:
   Duration (Du)
   Wet service (Wt) = 1
   Temperature (Tm) = 1
   Stability (St) = 1
   Size (Sz)
                    = 1.5
   Volume (Vm)
                     = 1
   Flat use (Fu)
                     = 1
   Repetitive (Rp) = 1.15
   Curvature (Cu) = 1
   Form (Fm)
                     = 1
   Shear stress (Sh) = 1
Allowable stresses (psi):
Wood:
   Bending = 2415 (Base x Du x Wt x Tm x St x Sz x Vm x Fu x Rp x Cu x Fm)
   Tension = 1020 (Base x Du x Wt x Tm x Sz)
   Shear = 112 (Base x Du x Wt x Tm x Sh)
   Elastic modulus = 2240000 (Base x Wt x Tm)
Sheathing:
           = 222 \text{ (Base x 1.33)}
   Bending
   Elastic modulus = 61904.76 (Base)
```

TRANSVERSE DRAGSTRUT NAIL ANALYSIS

Wall framing is 2 in. x 4 in. studs

Wall stud framing lumber is Spruce--Pine--Fir

Fasteners are 16d common nails

= 20 inches Approximate nail spacing

Total lateral force on building = 11911 pounds Force applied at top of walls = 5955 pounds Total dragstrut length = 56 feet

Shear per unit dragstrut length = 106 pounds per linear foot

Actual shear on each nail = 176 pounds Allowable shear on each nail = 192 pounds

Dragstrut nailing satisfies Code requirements.

LONGITUDINAL DRAGSTRUT NAIL ANALYSIS

Wall framing is 2 in. x 4 in. studs Wall stud framing lumber is Spruce--Pine--Fir

Fasteners are 16d common nails

Approximate nail spacing = 20 inches

Total lateral force on building = 4090 pounds

Force applied at top of walls = 2045 pounds
Total dragstrut length = 132 feet

Shear per unit dragstrut length = 15 pounds per linear foot

Actual shear on each nail = 25 pounds Allowable shear on each nail = 192 pounds

Dragstrut nailing satisfies Code requirements.

**** TRANSVERSE SHEARWALL ANALYSIS ****

Wall framing is 2 in. x 4 in. studs at 16 inch spacing

Wall stud framing lumber is Spruce--Pine--Fir

Wall shear siding is Oriented Strand Board -- 7/16 inch thick

Wall sheathing has all edges nailed

Fasteners: 8d common nails spaced along edges at 5 inch centers Fasteners: 8d common nails spaced in interior at 10 inch centers

Total lateral force on building = 11911 pounds Force applied at top of walls = 5955 pounds

Accumulated total shearwall length = 56 feet

Actual unit shear on shearwalls = 106 pounds per linear foot Allowable unit shear on shearwalls = 257 pounds per linear foot

Shearwall satisfies Code requirements.

**** LONGITUDINAL SHEARWALL ANALYSIS ****

Wall framing is 2 in. x 4 in. studs at 16 inch spacing

Wall stud framing lumber is Spruce--Pine--Fir

Wall shear siding is Oriented Strand Board -- 7/16 inch thick

Wall sheathing has all edges nailed

Fasteners: 8d common nails spaced along edges at 5 inch centers Fasteners: 8d common nails spaced in interior at 10 inch centers

Total lateral force on building = 4090 pounds Force applied at top of walls = 2045 pounds Accumulated total shearwall length = 132 feet

Actual unit shear on shearwalls = 15 pounds per linear foot Allowable unit shear on shearwalls = 257 pounds per linear foot

Shearwall satisfies Code requirements.

*** ANALYSIS OF OUTWARD FORCES ON WALL SHEATHING ***

Wall number 1 : Total outward wind force on sheathing = 804 pounds : Total withdrawal resistance of 76 nails = 4240 pounds

Wall number 2 : Total outward wind force on sheathing = 927 pounds : Total withdrawal resistance of 76 nails = 4240 pounds

**** ANALYSIS OF SHEATHING FASTENERS ****

Wall framing is Spruce--Pine--Fir lumber Sheathing is 7/16 inch Oriented Strand Board Sheathing extends from bottom of bottom plate to top of top plate Fasteners are 8d common nails at 5 inch spacing

Total uniform wind uplift in first story at top of wall level = 290 plf Uniform dead loads per linear foot:

Roof = 159.7911 plf

Total = 159.7911 plf

Total uniform dead load in first story at top of wall level = 159 plf
Net wind uplift in first story at top of wall level = 131 plf

Total uplift force on each nail = 54 pounds

Allowable shear on each nail = 97 pounds (increased for wind)

Sheathing to plate fastening satisfies all Code requirements.

**** ANALYSIS OF SHEATHING FASTENERS ****

Wall framing is Spruce--Pine--Fir lumber Sheathing is 7/16 inch Oriented Strand Board Sheathing extends from bottom of bottom plate to top of top plate Fasteners are 8d common nails at 5 inch spacing

Fotal uniform wind uplift in first story at floor level = 290 plf
Jniform dead loads per linear foot:

Roof = 159.7911 plf Wall = 38.42197 plf

Total = 198.2131 plf

Fotal uniform dead load in first story at floor level = 198 plf
Net wind uplift in first story at floor level = 92 plf

Fotal uplift force on each nail = 38 pounds
Allowable shear on each nail = 97 pounds (increased for wind)
Sheathing to plate fastening satisfies all Code requirements.

**** ANALYSIS OF FOUNDATION ANCHORAGE **** Anchor bolts are 1/2 inch A307, with 2 inch round washer at 48 inch centers. Total uniform wind uplift on foundation = 290 pounds per linear foot Uniform dead loads in pounds per linear foot: Roof = 159.7911 plf Wall = 38.42197 plf Total = 198.2131 plf Total uniform dead load times 2/3 = 132 pounds per linear foot Net uplift force on foundation = 158 pounds per linear foot Total uplift force on each anchor bolt = 632 pounds Safe tension value of each anchor bolt = 1634 pounds (increased by 1/3) Bolt safe tension value is governed by washer failure *** Summary of Analysis *** Foundation anchorage satisfies all Code requirements. **** ANALYSIS OF CORNER HOLD-DOWN REQUIREMENTS **** Hold-down is one typical anchor bolt with washer, each wall Normal anchor bolt spacing = 48 inches Distance from corner to hold-down device Distance from corner to first interior anchor bolt = 48 inches Net uplift force on foundation = 158 pounds per linear foot Tributary distance to corner device = 2.25 feet Net uplift on corner hold-down device = 355 pounds Uplift tension due to shearwall action in a transverse shearwall segment: Distance from corner to hold-down device = 6 inches Distance from corner to first interior anchor bolt = 48 inches Total shear from shearwall segment = 319 pounds Height of wall = 8 feet Uniform dead load times 2/3 = 25 pounds per linear foot Shearwall moment at bottom of wall = 2552 foot-pounds Additional tension at corner device = 1237 pounds Total uplift tension on corner hold-down devices = 1592 pounds Allowable tension on corner hold-down devices = 3268 pounds

*** Summary of Analysis ***

Corner hold-down device COMPLIES with Code requirements.

**** ANALYSIS OF FOUNDATION ****

Stemwall is 8 inch concrete masonry, filled with grout, 16 inches high Footing is 20 inches wide by 10 inches deep Earth cover over top of footing is 4 inches

Total uniform wind uplift on foundation = 290 pounds per linear foot Uniform dead loads in pounds per linear foot:

Roof = 159.7911 plf Wall = 38.42197 plf

Total = 198.2131 plf

Total uniform dead load times 2/3 = 132 pounds per linear foot
Net uplift force at top of foundation = 158 pounds per linear foot
Weight of stemwall footing earth x 2/3 = 261 pounds per linear foot
Net uplift at bottom of footing = 0 pounds per linear foot

*** Summary of Analysis ***
Foundation is stable.

**** ANALYSIS OF REINFORCING STEEL ****

Grade 40 reinforcing steel, Number 5 vert. bars at 72 inch centers

Total uniform wind uplift on foundation = 290 pounds per linear feet Uniform dead loads in pounds per linear foot:

Roof = 159.7911 plf Wall = 38.42197 plf

Total = 198.2131 plf

Total uniform dead load times 2/3 = 132 pounds per linear foot Net uplift force on foundation = 158 pounds per linear foot Weight of concrete block stemwall x 2/3 = 81 pounds per linear foot Net uplift at top of footing = 77 pounds per linear foot

Total uplift force on each re-bar = 462 pounds
Safe tension value of each re-bar = 8181 pounds (increased by 1/3)

*** Summary of Analysis ***
Reinforcing steel satisfies all Code requirements.

**** SUMMARY OF REINFORCING DATA ****

Wall reinforcing is Grade 40 steel, Number 5 at 72 inch centers Minimum required lap splice for Number 5 bar is 25 inches. Minimum required clearance for Number 5 bar is 1.5 inches.
Wall reinf. in footing has a std. A.C.I. hook, 6 inches below top of footing. Footing data: Footing is continuous, 20 inches wide by 10 inches deep. Footing concrete is 2500 psi Footing reinforcing is Grade 40 steel, 2--#() longitudinal. Minimum required splice length = 25 inches Reinforcing steel shall have cover as follows:

Wall is composed of 8 inch concrete masonry, fully grouted.

Top----6 inches Sides----3 inches Bottom---3 inches

Foundation wall data: