Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
 Document ID:1SXW487-Z0808155617

Truss Fabricator:	Anderson Truss Company
Job Identification:	6-228--Mike Todd Construction Zebra 1 -- , **
Truss Count:	25
Model Code:	Florida Building Code 2004
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Version 7.24.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-02 -Closed

Seal Date: 06/08/2006

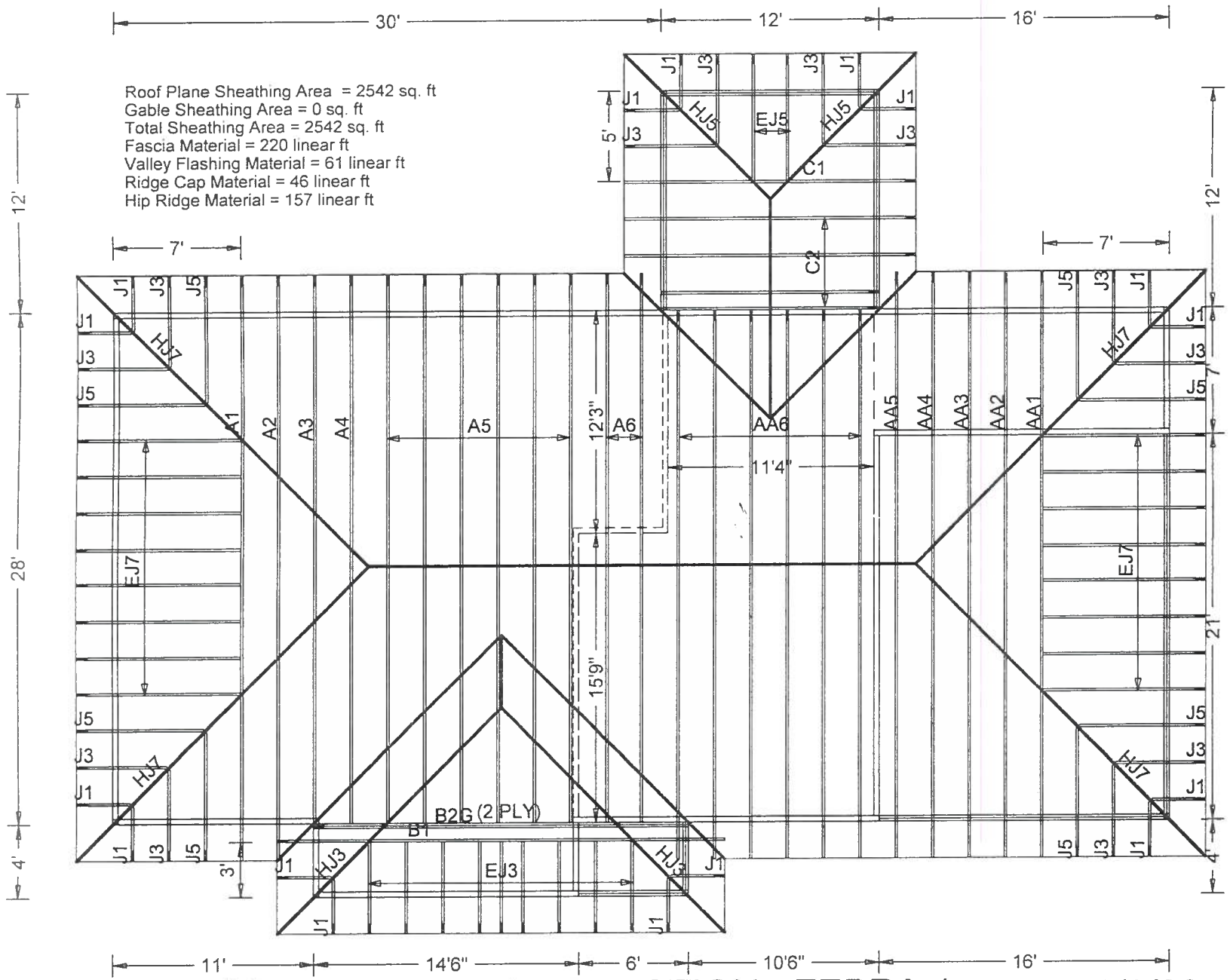
-Truss Design Engineer-
Arthur R. Fisher
Florida License Number: 59687
1950 Marley Drive
Haines City, FL 33844

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: TCFILLER-BCFILLER-

#	Ref Description	Drawing#	Date
1	86959--A1	06159106	06/08/06
2	86960--A2	06159098	06/08/06
3	86961--A3	06159099	06/08/06
4	86962--A4	06159107	06/08/06
5	86963--A5	06159108	06/08/06
6	86964--A6	06159100	06/08/06
7	86965--AA1	06159109	06/08/06
8	86966--AA2	06159110	06/08/06
9	86967--AA3	06159111	06/08/06
10	86968--AA4	06159112	06/08/06
11	86969--AA5	06159114	06/08/06
12	86970--AA6	06159101	06/08/06
13	86971--B1	06159115	06/08/06
14	86972--B2G	06159116	06/08/06
15	86973--C1	06159117	06/08/06
16	86974--C2	06159102	06/08/06
17	86975--HJ7	06159006	06/08/06
18	86976--EJ7	06159103	06/08/06
19	86977--J5	06159104	06/08/06
20	86978--J3	06159008	06/08/06
21	86979--J1	06159009	06/08/06
22	86980--HJ5	06159007	06/08/06
23	86981--EJ5	06159010	06/08/06
24	86982--HJ3	06159118	06/08/06
25	86983--EJ3	06159105	06/08/06



#6-228 MIKE TODD CONSTRUCTION - ZEBRA 1

6/8/06

754-4387
867-0477 cell

Scale: 1/8" = 1'

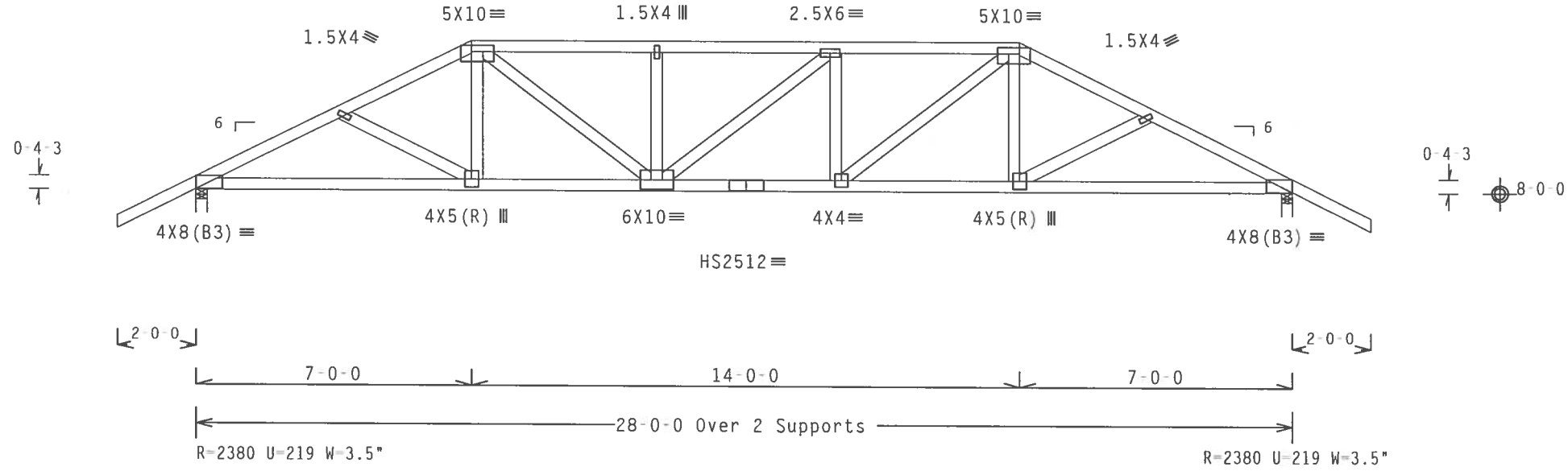
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE

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Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (H, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER
LICENSE
No.59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 08 '06

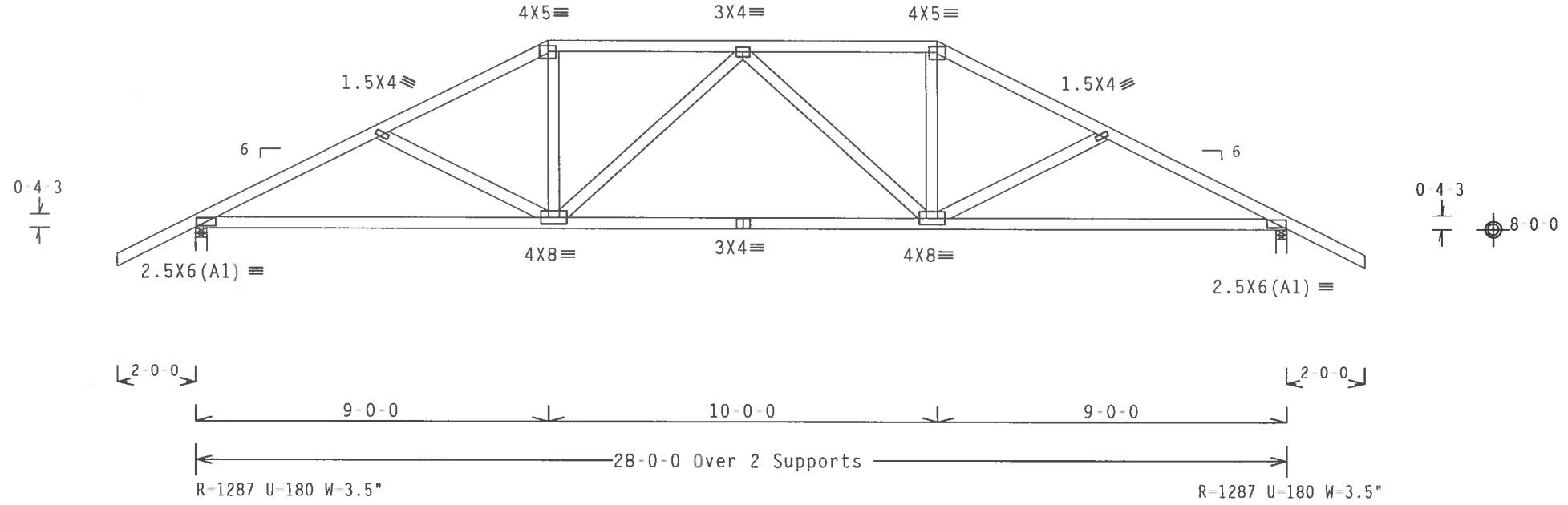
TC LL	20.0 PSF	REF R487-- 86959
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159106
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 8125
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXW487 Z08

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

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ARTHUR A. FISHER LICENSE No.58687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jun 08 '06

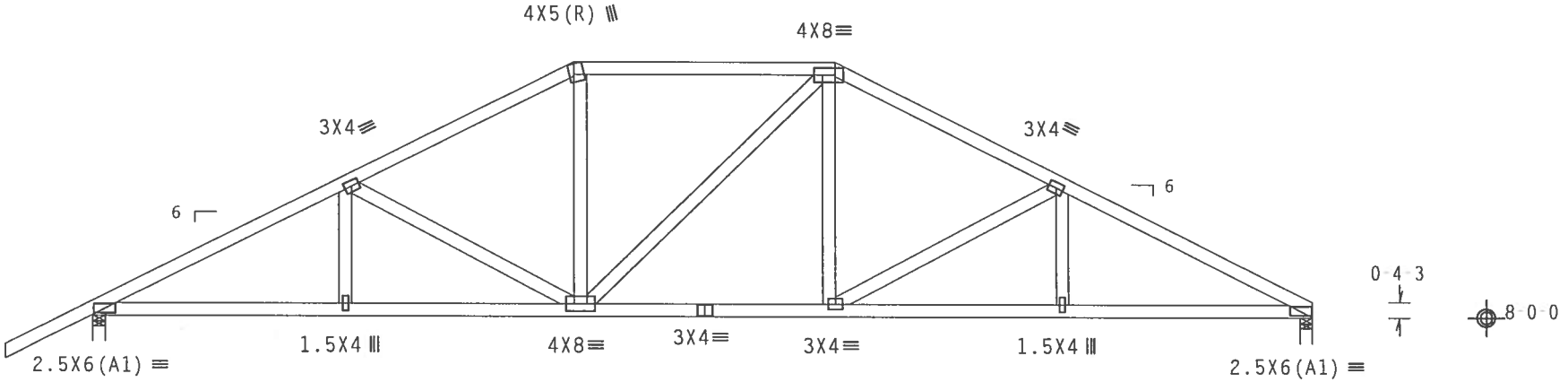
TC LL	20.0 PSF	REF R487-- 86960
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159098
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	40.0 PSF	SEQN- 8138
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXW487 Z08

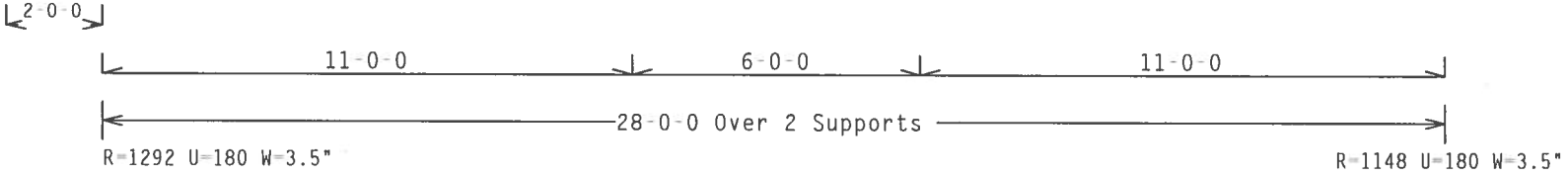
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.12 TY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

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STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 08 '06

TC LL	20.0 PSF	REF	R487-- 86961
TC DL	10.0 PSF	DATE	06/08/06
BC DL	10.0 PSF	DRW	HCUSR487 06159099
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	40.0 PSF	SEQN-	8152
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SXW487 Z08

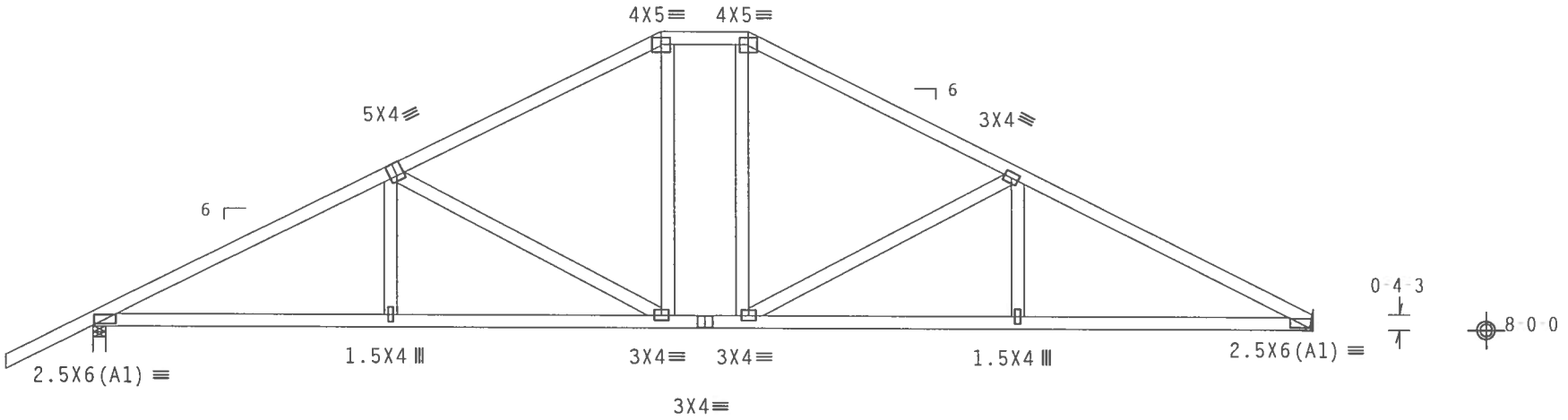
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

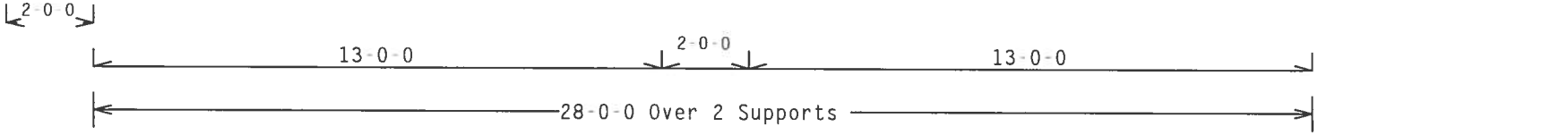
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.





R=1293 U=180 W=3.5"

R=1147 U=180 H=Simpson LUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
...der is (2)2X6 min. So.Pine

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.12 ...TY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
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...icate of ... ...on # 567

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ARTHUR R. FISHER
LICENSE
No.59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 08 '06

TC LL	20.0 PSF	REF R487-- 86962
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159107
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 8169
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXW487 Z08

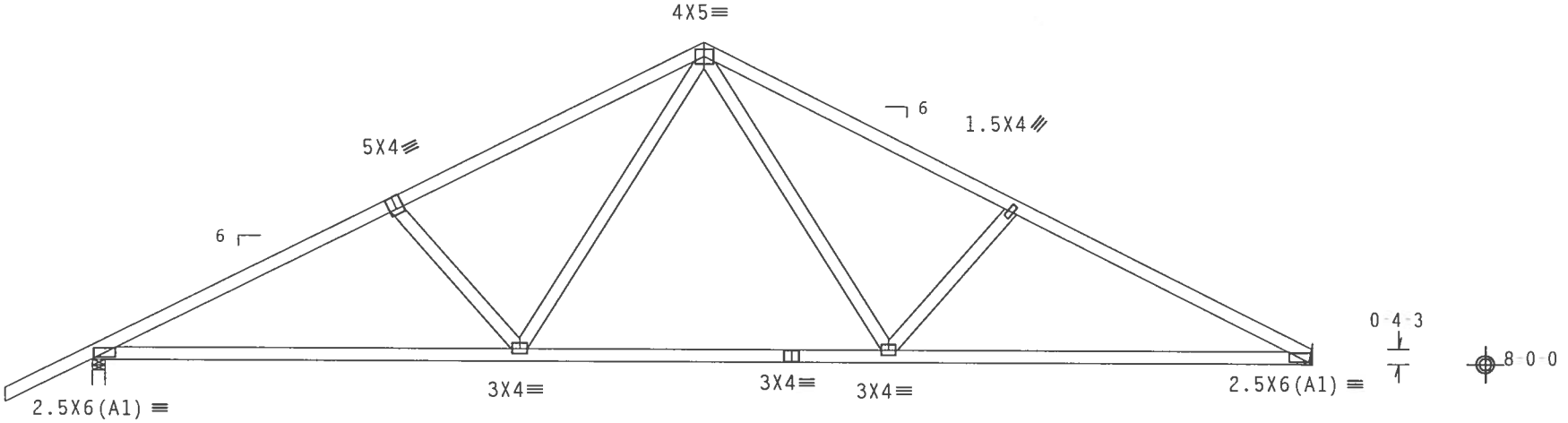
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

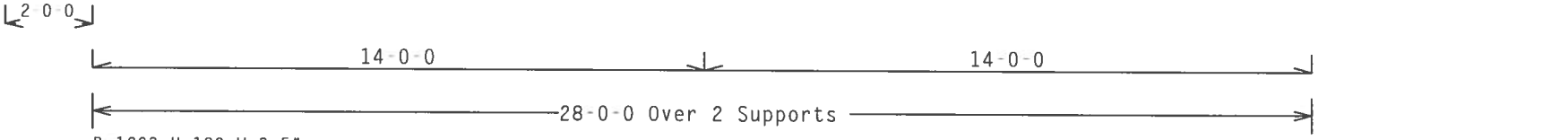
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.





R=1293 U=180 W=3.5"

R=1147 U=180 H=Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

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ARTHUR R. FISHER
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STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 08 '06

TC LL	20.0 PSF	REF R487-- 86963
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159108
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 8194
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXW487 Z08

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Filler 2x4 SP #3

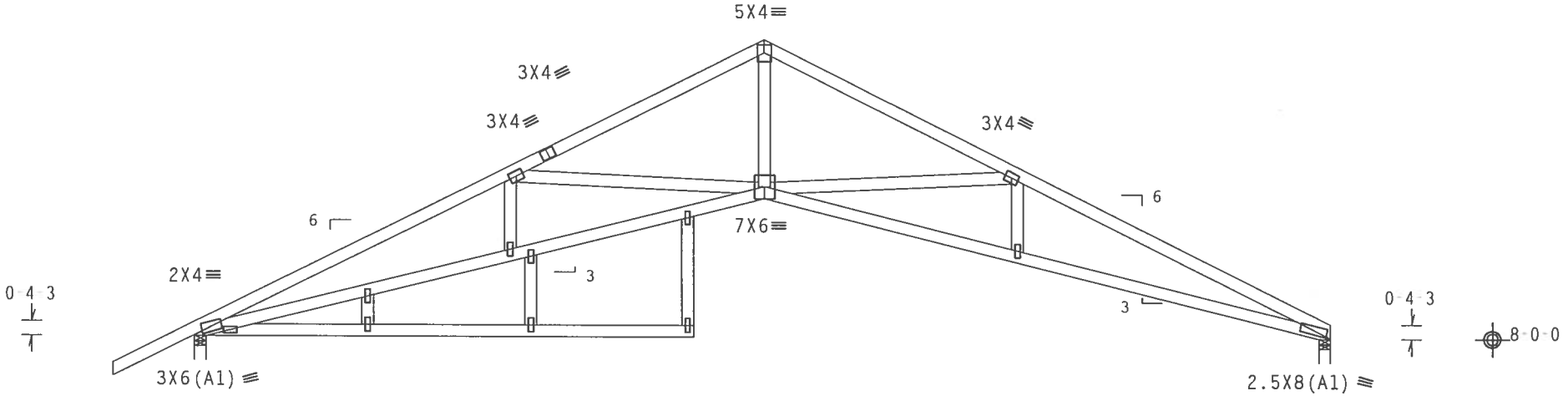
SEE DWGS TCFILLER1103 AND BCFILLER1103 FOR FILLER DETAILS. LATERALLY BRACE BOTTOM CHORD ABOVE FILLER AT 24" O.C,AND TOP CHORD UNDER FILLER AT 24" OC INCLUDING A LATERAL BRACE AT CHORD ENDS.

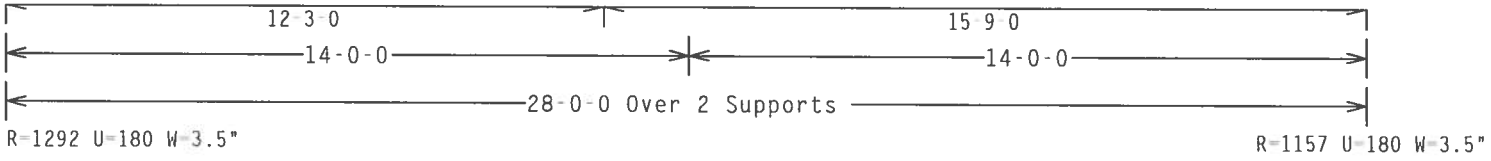
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.19" due to live load and 0.29" due to dead load.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.





Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

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Certificate of Authorization # 567

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ARTHUR R. FISHER
LICENSE
No.59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 06 06

TC LL	20.0 PSF	REF	R487-- 86964
TC DL	10.0 PSF	DATE	06/08/06
BC DL	10.0 PSF	DRW	HCUSR487 06159100
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	40.0 PSF	SEQN-	8219
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SXW487 Z08

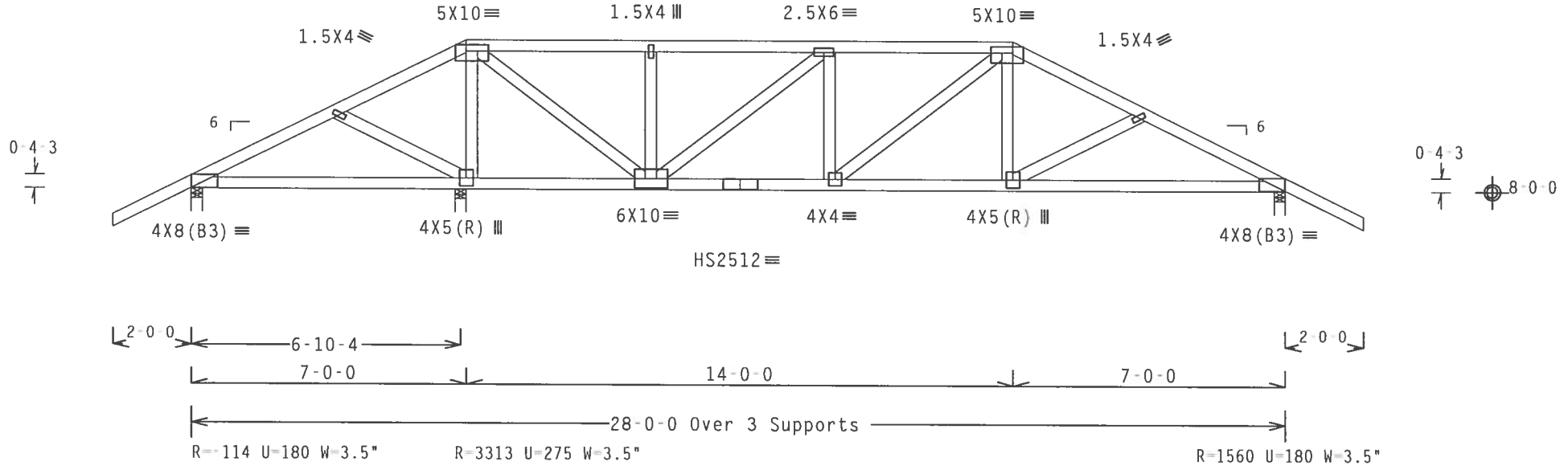
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave

Cq/RT=1.00(1.25)/10(0) 7.24.12 [illegible] QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
[illegible]icate of [illegible]on # 567

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jun 08 '06

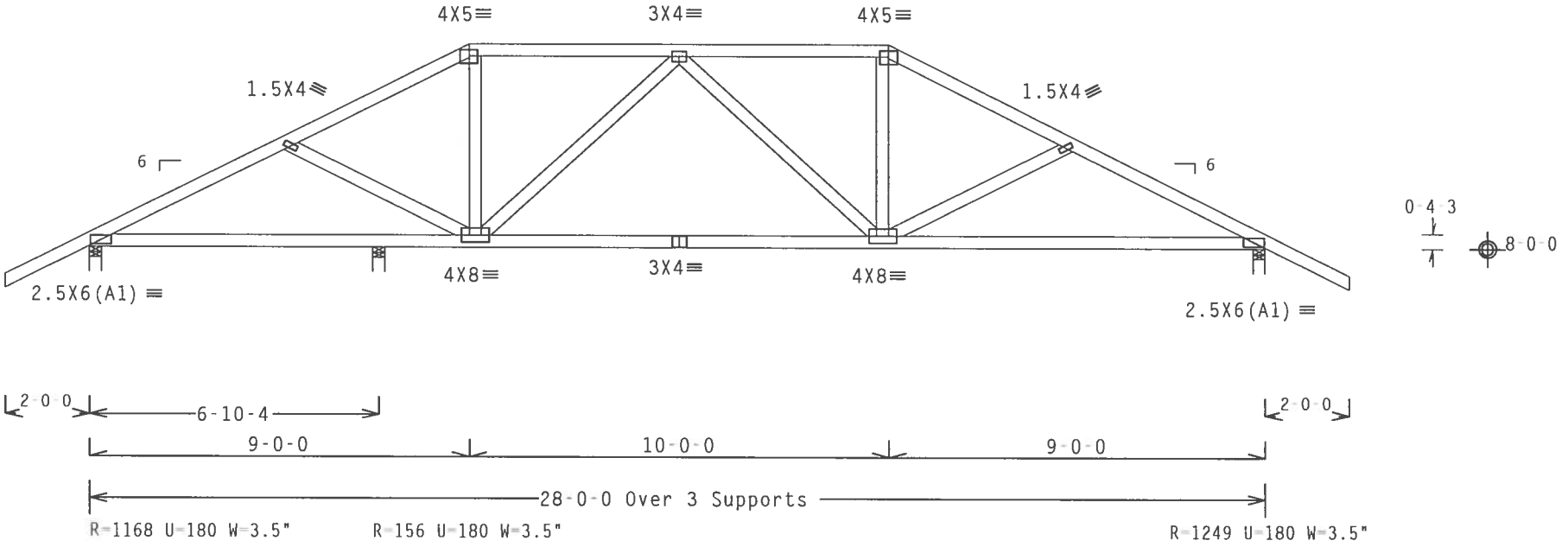
TC LL	20.0 PSF	REF R487-- 86965
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159109
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 8129
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXW487 Z08

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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ARTHUR R. FISHER
LICENSE
No.59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 08 '06

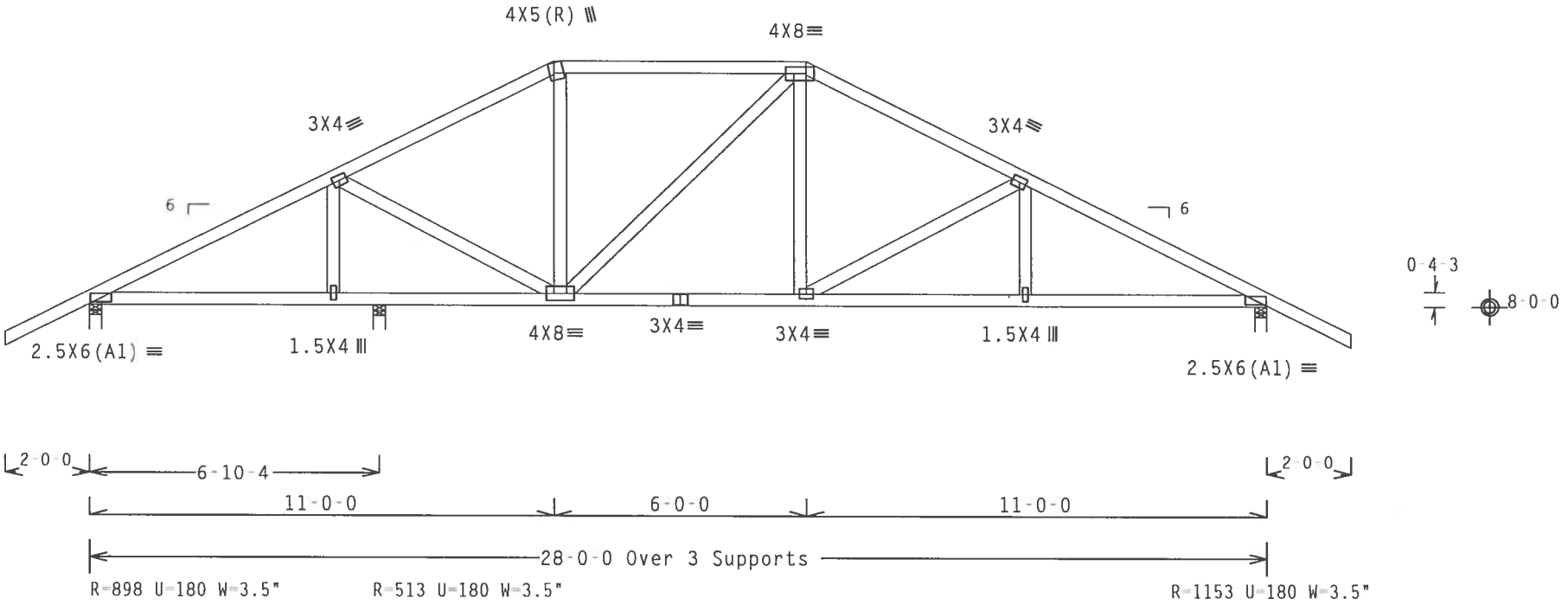
TC LL	20.0 PSF	REF	R487-- 86966
TC DL	10.0 PSF	DATE	06/08/06
BC DL	10.0 PSF	DRW	HCUSR487 06159110
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	8144
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SXW487 Z08

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 08 '06

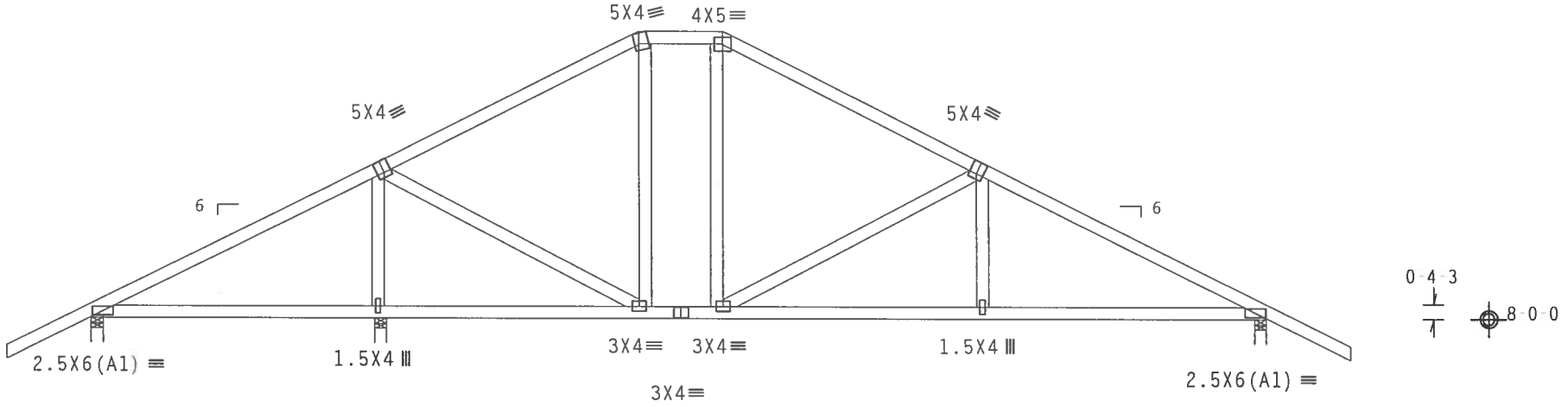
TC LL	20.0 PSF	REF	R487-- 86967
TC DL	10.0 PSF	DATE	06/08/06
BC DL	10.0 PSF	DRW	HCUSR487 06159111
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	8159
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SXW487 Z08

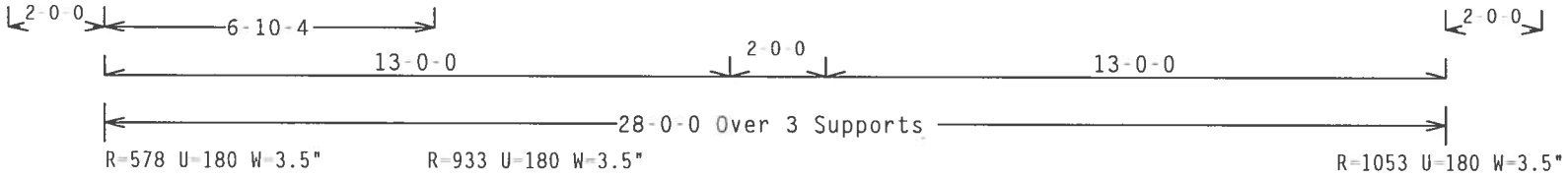
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.





PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

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ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 08 06

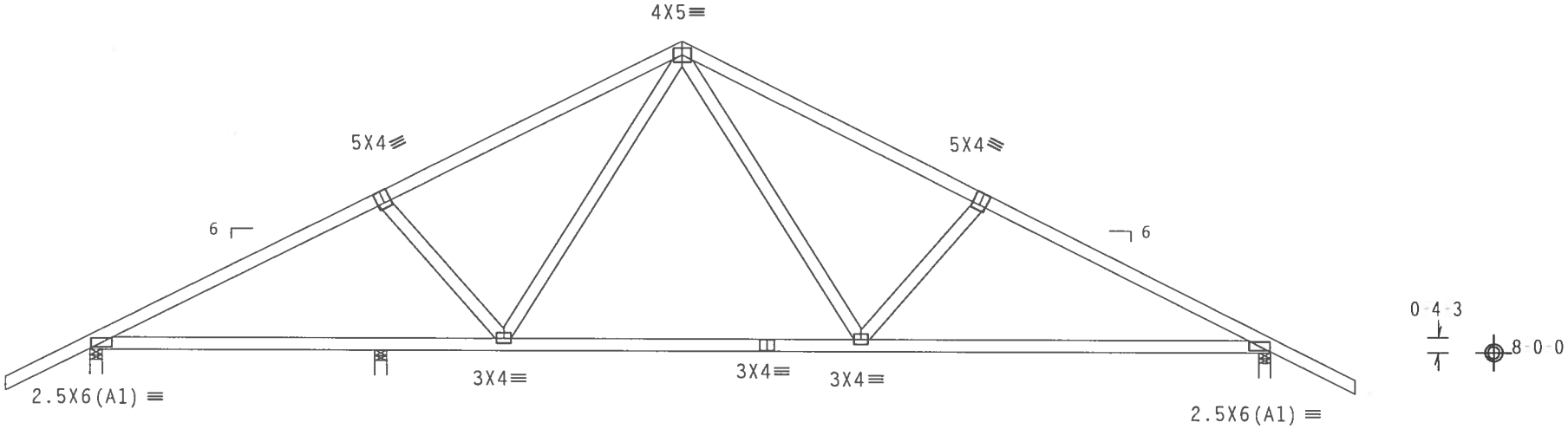
TC LL	20.0 PSF	REF R487-- 86968
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159112
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 8180
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXW487 Z08

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

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ARTHUR R. FISHER
LICENSE
No. 29687
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
Jun 08 '06

TC LL	20.0 PSF	REF	R487-- 86969
TC DL	10.0 PSF	DATE	06/08/06
BC DL	10.0 PSF	DRW	HCUSR487 06159114
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	8201
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SXW487 Z08

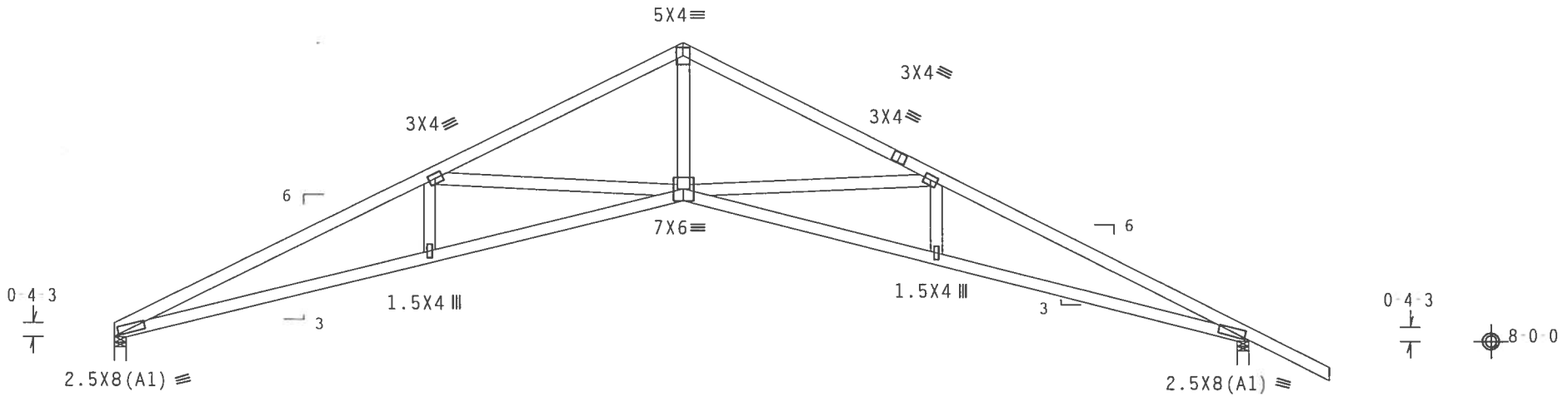
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Calculated horizontal deflection is 0.19" due to live load and 0.30" due to dead load.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
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ARTHUR R. FISHER LICENSE No.50687 STATE OF FLORIDA PROFESSIONAL ENGINEER
JUN 08 '06

TC LL	20.0 PSF	REF	R487-- 86970
TC DL	10.0 PSF	DATE	06/08/06
BC DL	10.0 PSF	DRW	HCUSR487 06159101
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	40.0 PSF	SEQN-	8209
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SXW487 Z08

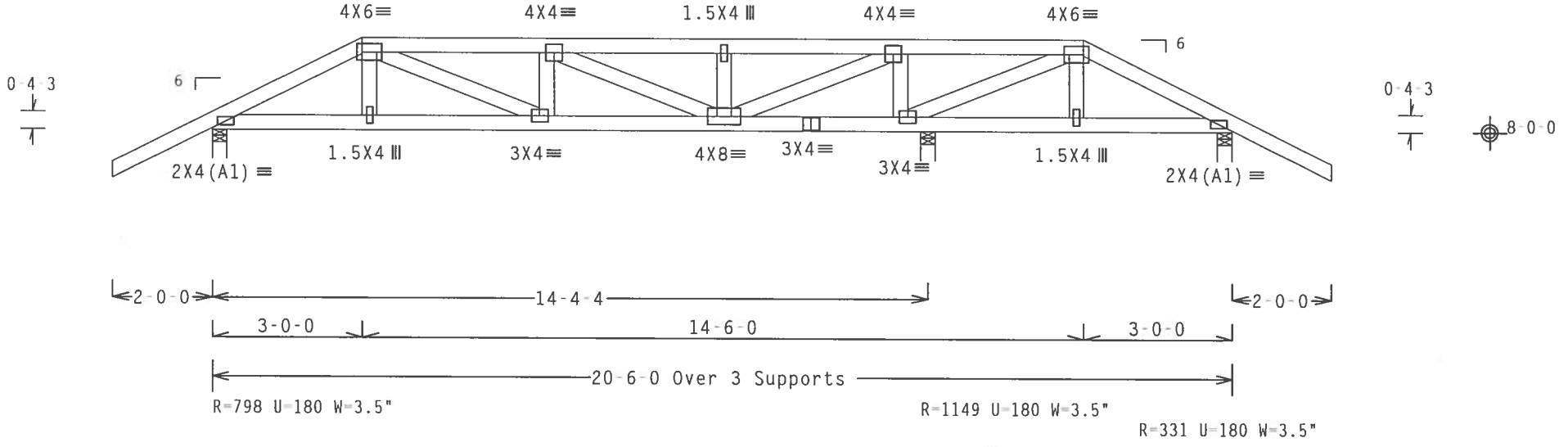
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

#1 hip supports 3-0-0 jacks with no webs.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1 TY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

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Certificate of Authorization # 567

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ARTHUR B. FISHER
LICENSE
No.59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 08 '06

TC LL	20.0 PSF	REF R487-- 86971
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159115
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 8235
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXW487 Z08

Top chord 2x4 SP #2 Dense
Bot chord 2x8 SP #1 Dense
Webs 2x4 SP #3 :W6 2x4 SP #2 Dense:

SPECIAL LOADS
(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 62 PLF at 0.00 to 62 PLF at 20.50
BC - From 20 PLF at 0.00 to 20 PLF at 20.50
BC - 1147 LB Conc. Load at 2.06, 4.06, 6.06, 8.06, 10.06 12.06, 14.06

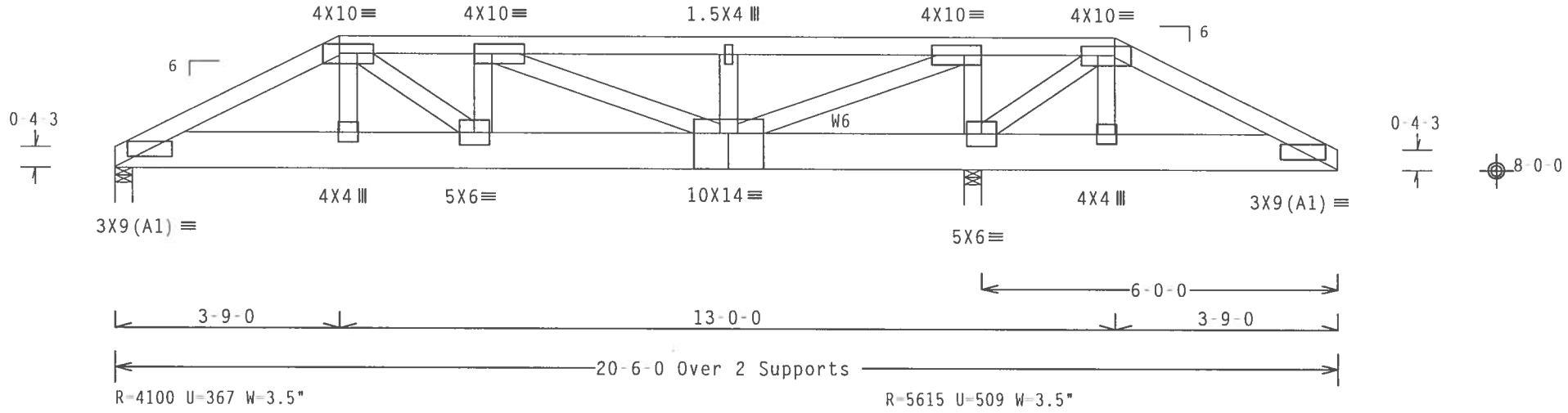
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Common (0.148"x3.25", min.) nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 4.75" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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ARTHUR A. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 08 06

TC LL	20.0 PSF	REF R487-- 86972
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159116
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 8227
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXW487 Z08

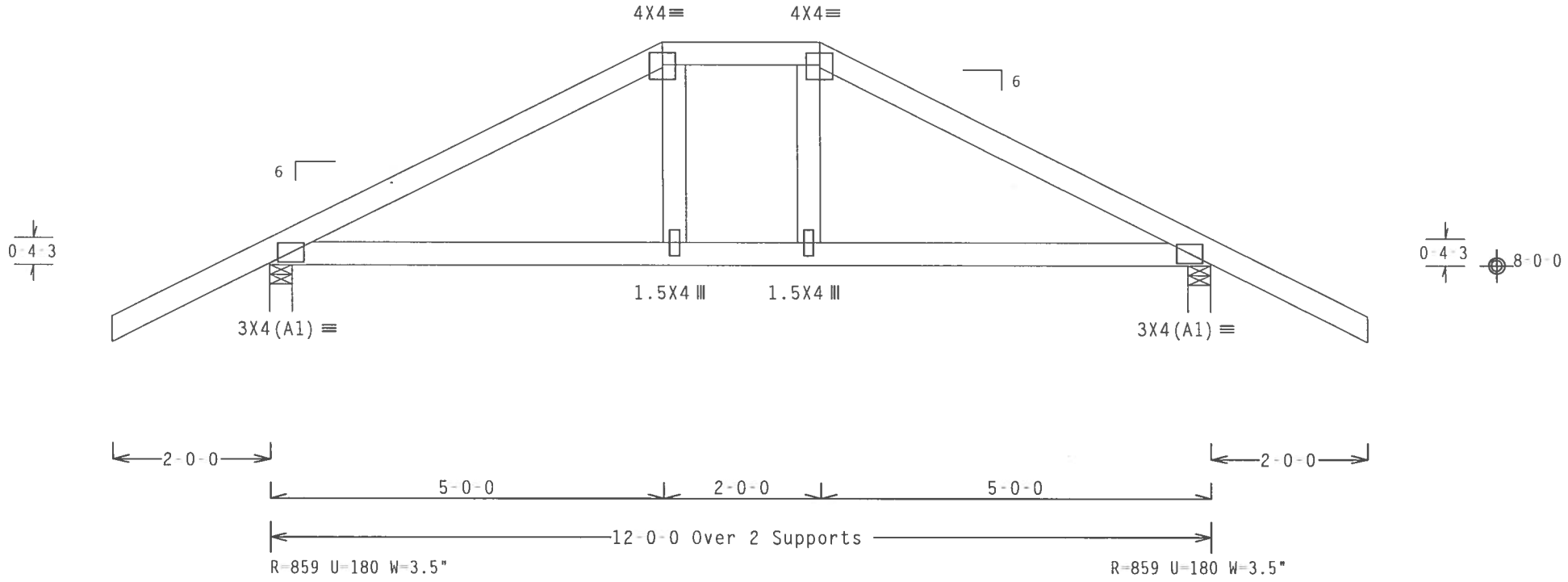
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

#1 hip supports 5-0-0 jacks with no webs.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.12 QTY:1 FL/-/4/-/-/R/-

Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 08, 2006

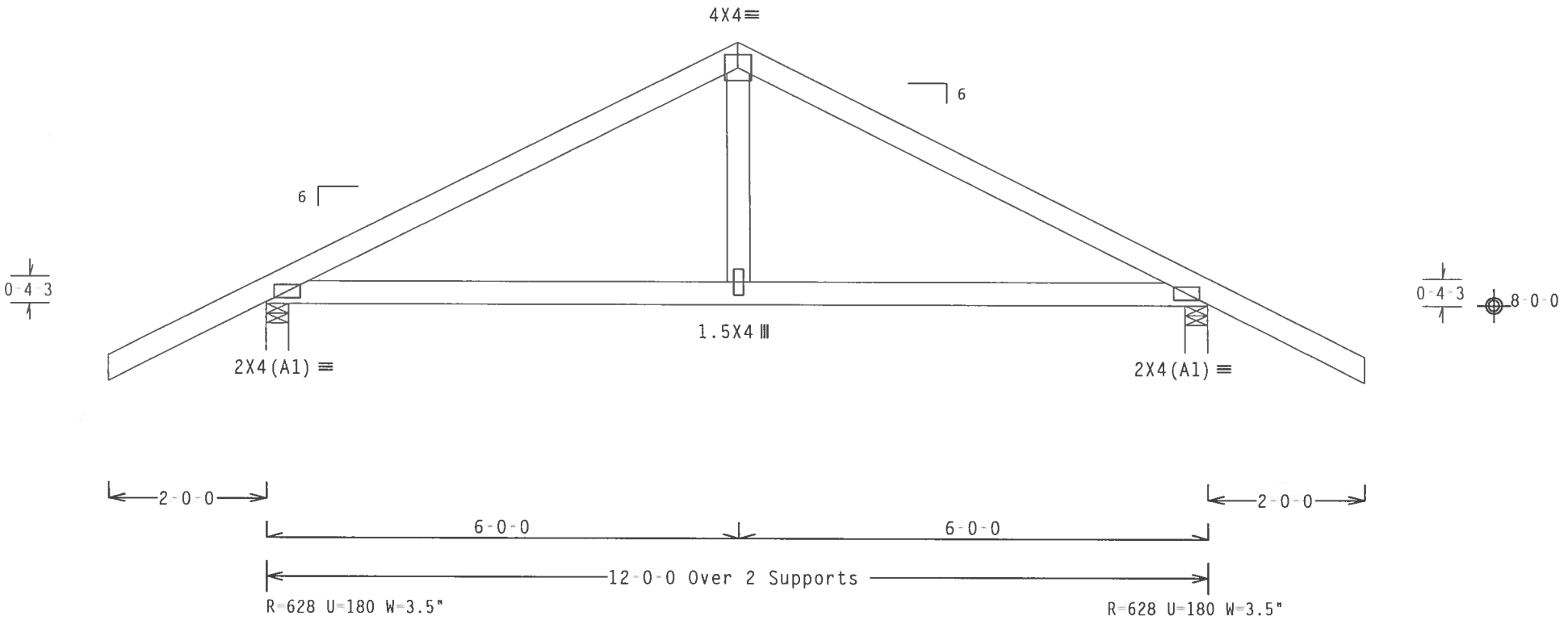
TC LL	20.0 PSF	REF R487-- 86973
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159117
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 8105
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXW487 Z08

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jul 08 '06

TC LL	20.0 PSF	REF	R487-- 86974
TC DL	10.0 PSF	DATE	06/08/06
BC DL	10.0 PSF	DRW	HCUSR487 06159102
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	40.0 PSF	SEQN-	8100
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SXW487 Z08

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

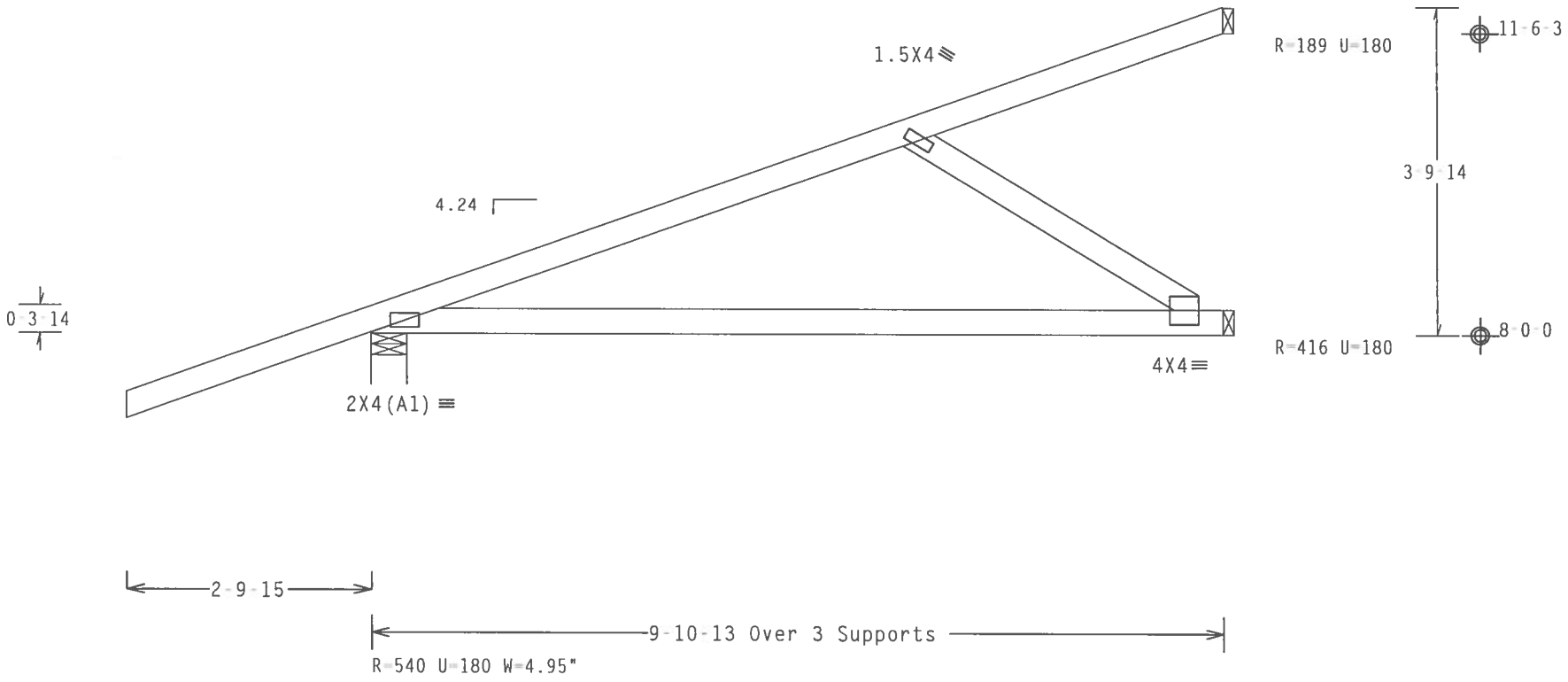
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Hipjack supports 7-0-0 setback jacks with no webs.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 08 '06

TC LL	20.0 PSF	REF R487-- 86975
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159006
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 2126
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXW487 Z08

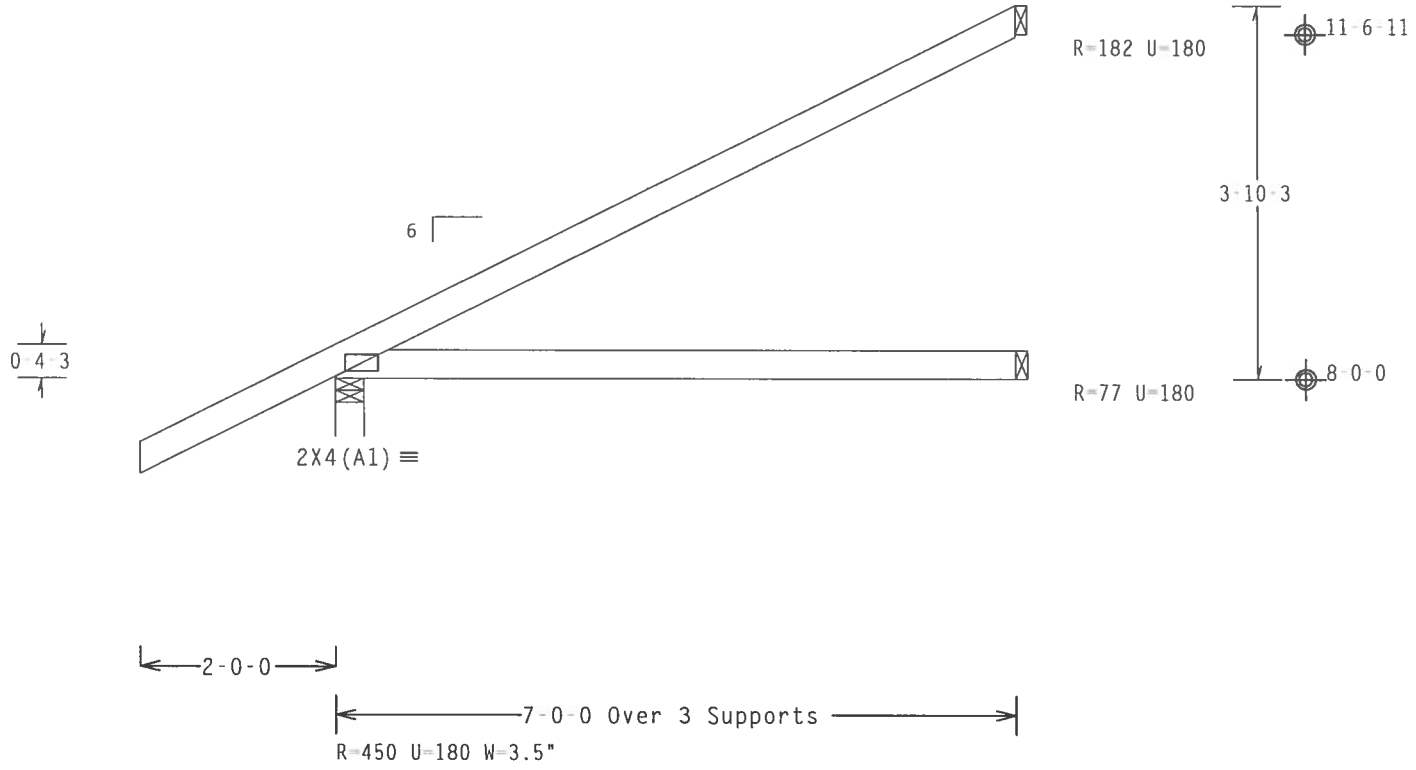
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cg/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
JUL 28 '06

TC LL	20.0 PSF	REF	R487-- 86976
TC DL	10.0 PSF	DATE	06/08/06
BC DL	10.0 PSF	DRW	HCUSR487 06159103
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	40.0 PSF	SEQN-	8071
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SXW487 Z08

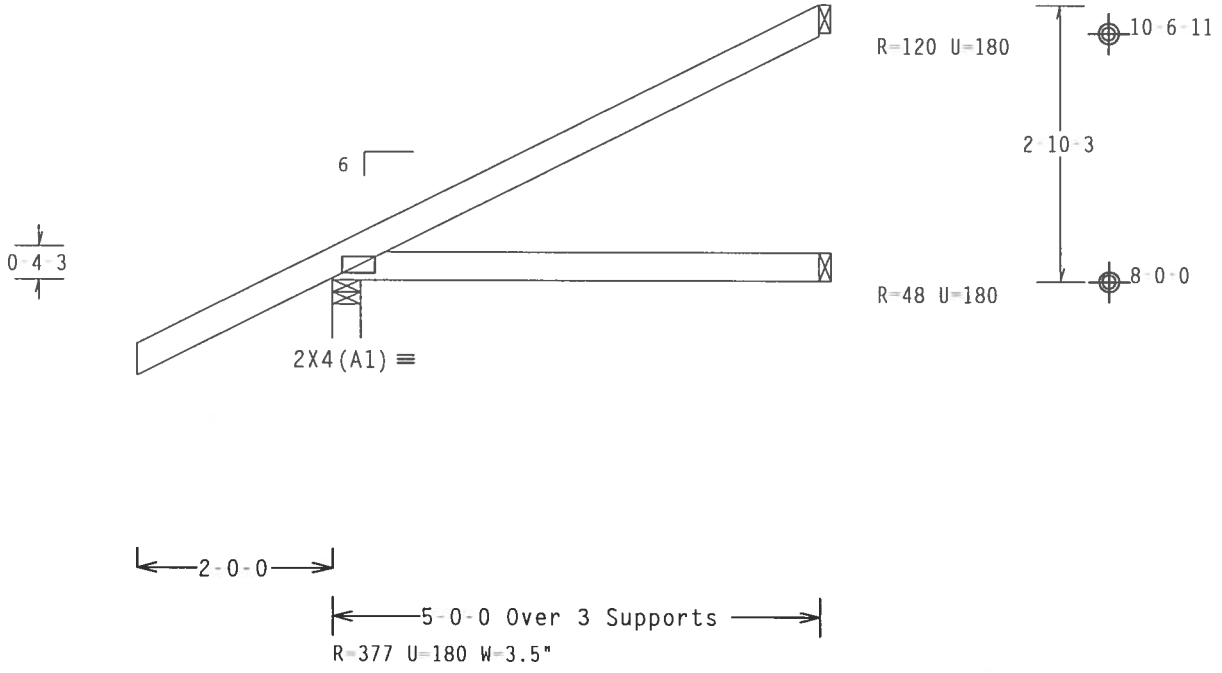
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.[illegible] QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
[illegible]ficate of [illegible]on # 567

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ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 0[illegible] '06

TC LL	20.0 PSF	REF R487-- 86977
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159104
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	40.0 PSF	SEQN- 8075
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXW487 Z08

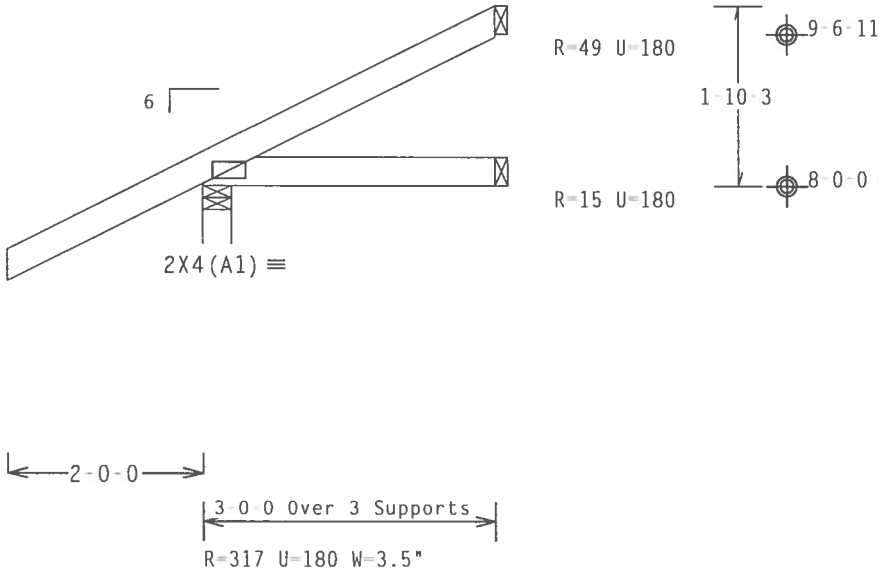
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1 LC... TY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

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Certificate of Authorization # 567

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ARTHUR R. FISHER
LICENSE
No.59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Jun 08 '06

TC LL	20.0 PSF	REF R487-- 86978
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159008
BC LL	0.0 PSF	HC-ENG TCE/AF *
TOT.LD.	40.0 PSF	SEQN- 87850
DUR.FAC.	1.25	
SPACING	24.0"	JRFF- 1SXW487 Z08

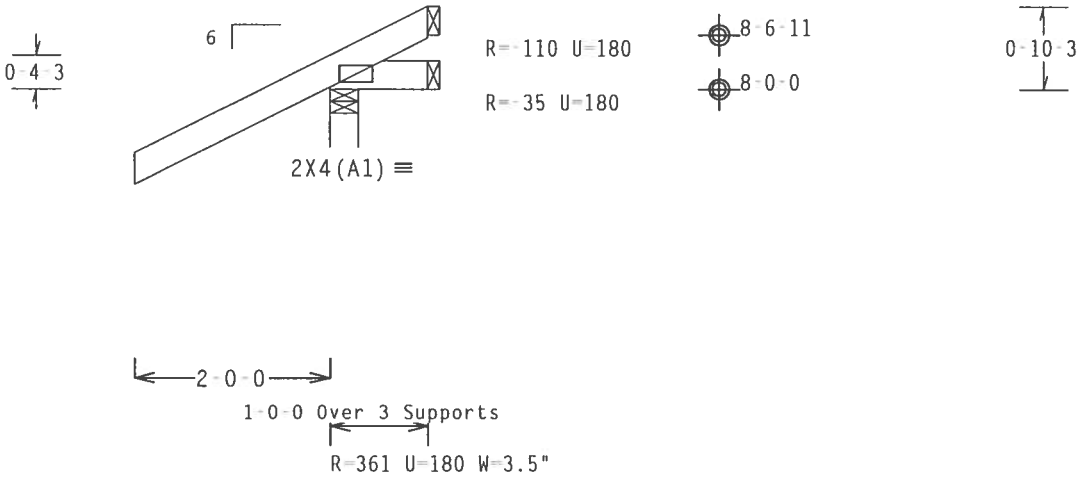
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

JUL 08 '06

TC LL	20.0 PSF	REF R487-- 86979
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159009
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 7002
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXW487 Z08

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

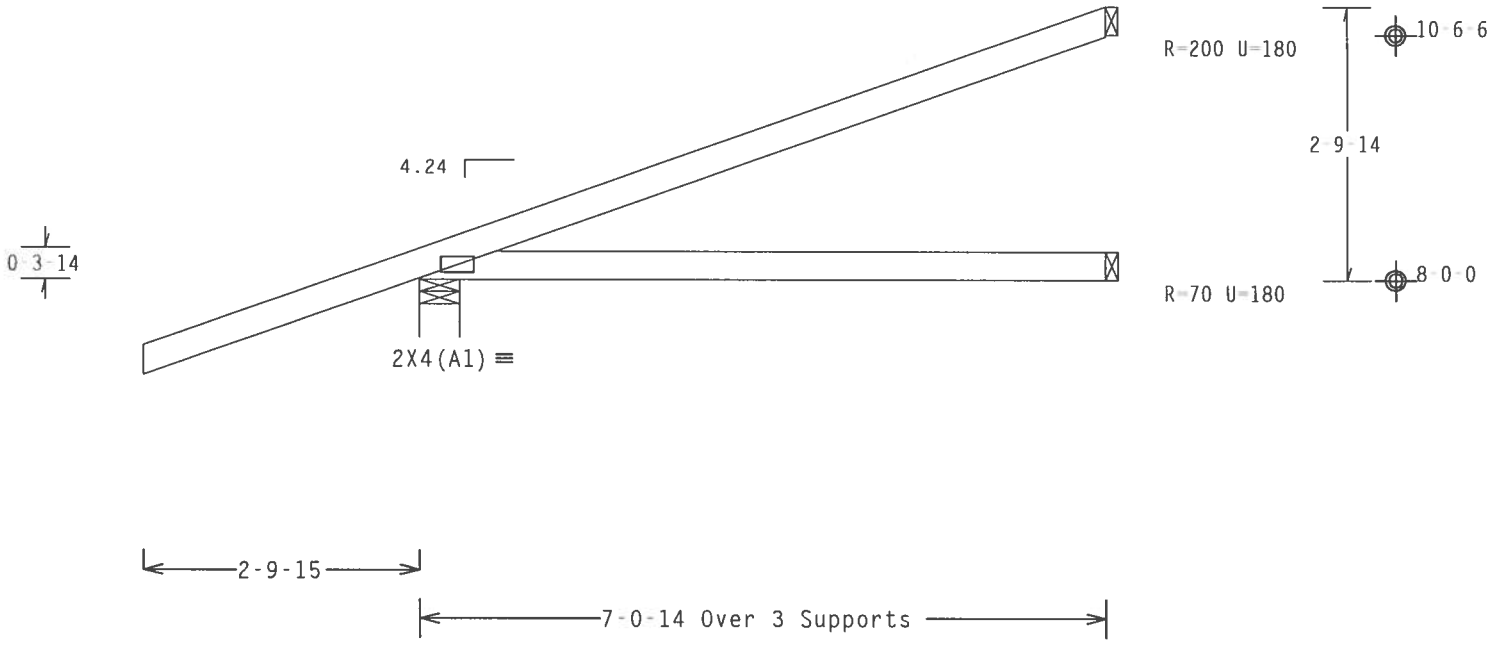
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Hipjack supports 5-0-0 setback jacks with no webs.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1 TY:5 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 08 '06

TC LL	20.0 PSF	REF R487-- 86980
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159007
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 99786
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXW487 Z08

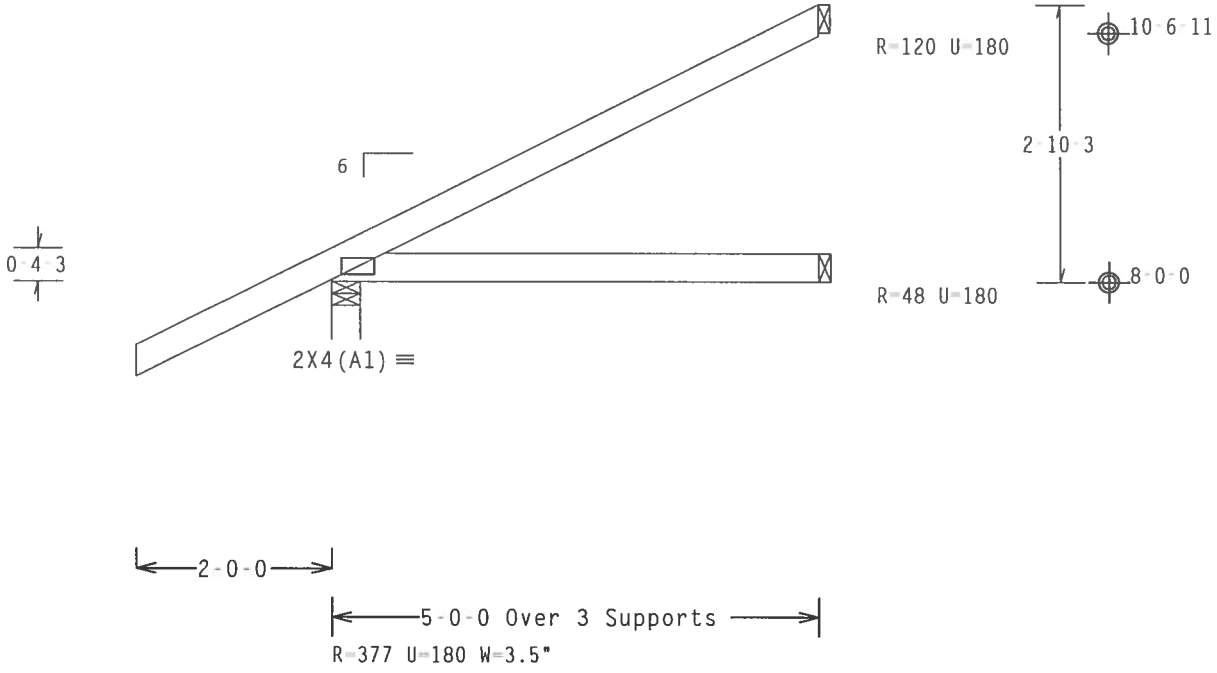
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.12 TY:11 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jun 08 '06

TC LL	20.0 PSF	REF R487-- 86981
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159010
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	40.0 PSF	SEQN- 99551
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXW487 Z08

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

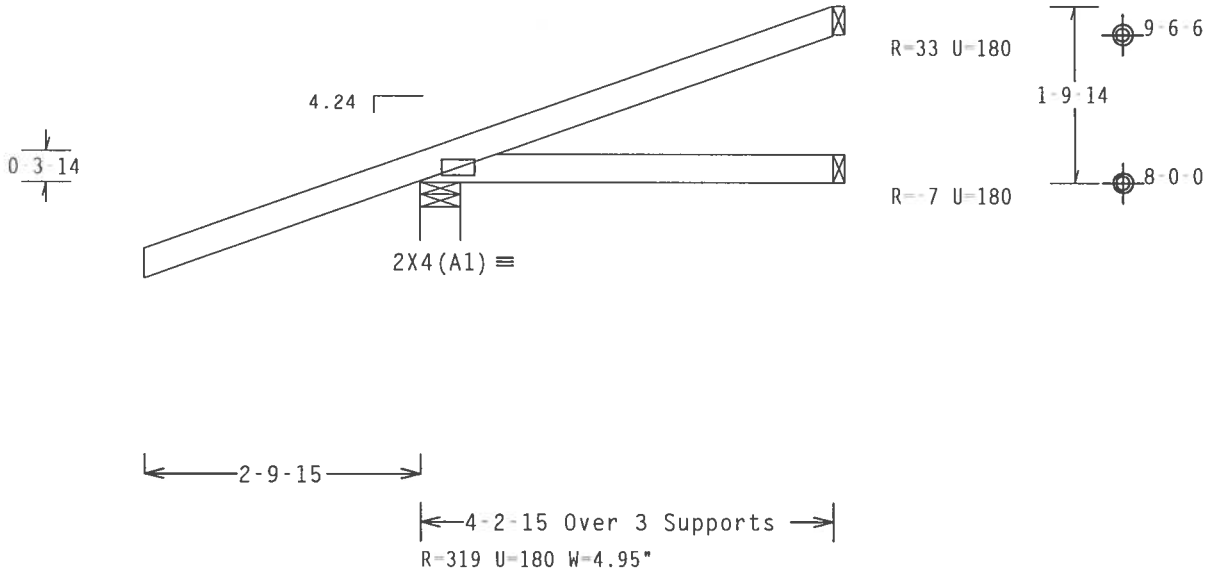
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Hipjack supports 3-0-0 setback jacks with no webs.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1 [illegible] QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
[illegible]ficate of [illegible]on # 567

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ARTHUR R. FISHER
LICENSE
No.59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 08 '06

TC LL	20.0 PSF	REF	R487-- 86982
TC DL	10.0 PSF	DATE	06/08/06
BC DL	10.0 PSF	DRW	HCUSR487 06159118
BC LL	0.0 PSF	HC-ENG	AF/AF
TOT.LD.	40.0 PSF	SEQN-	8240
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SXW487 Z08

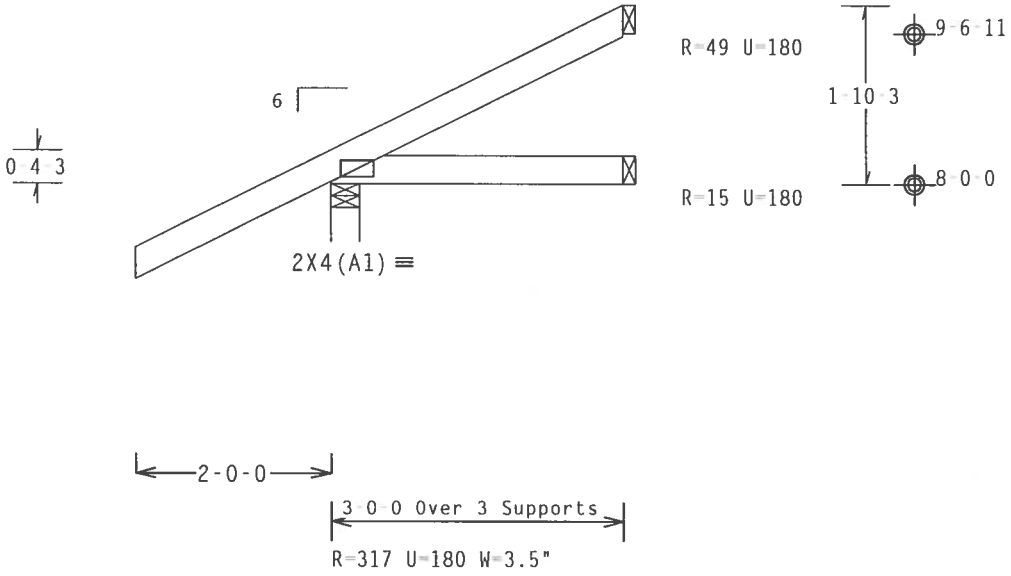
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
JUL 28 '06

TC LL	20.0 PSF	REF R487-- 86983
TC DL	10.0 PSF	DATE 06/08/06
BC DL	10.0 PSF	DRW HCUSR487 06159105
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	40.0 PSF	SEQN- 8108
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXW487 Z08

TOP CHORD FILLER DETAIL

+ 2X4 CONTINUOUS LATERAL BRACING AT 24" OC MAXIMUM SPACING. ATTACH TO EACH TOP CHORD WITH (2) 16d NAILS. BRACING MATERIAL TO BE SUPPLIED AND ATTACHED AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.

++ 2X4 SO. PINE #2 N OR SPF #1/#2 FILLER TOP CHORD.

+++ 2X4 SO. PINE #3 OR SPF #1/#2 VERTICAL WEBS SPACED 48" OC MAXIMUM.

* 8/12 MAXIMUM PITCH.

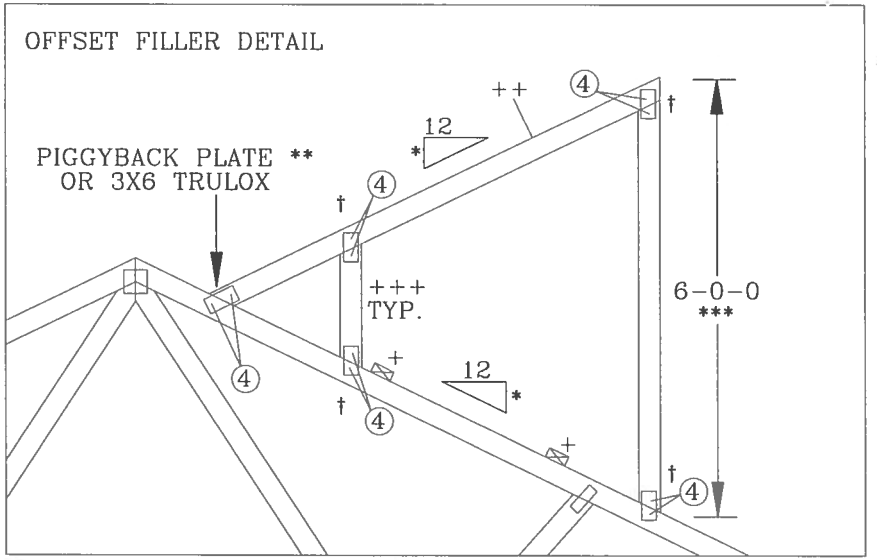
** 2X8.25 PIGGYBACK SPECIAL PLATE. SEE DRAWING PIGBACKB0699 FOR PIGGYBACK SPECIAL PLATE INFORMATION.

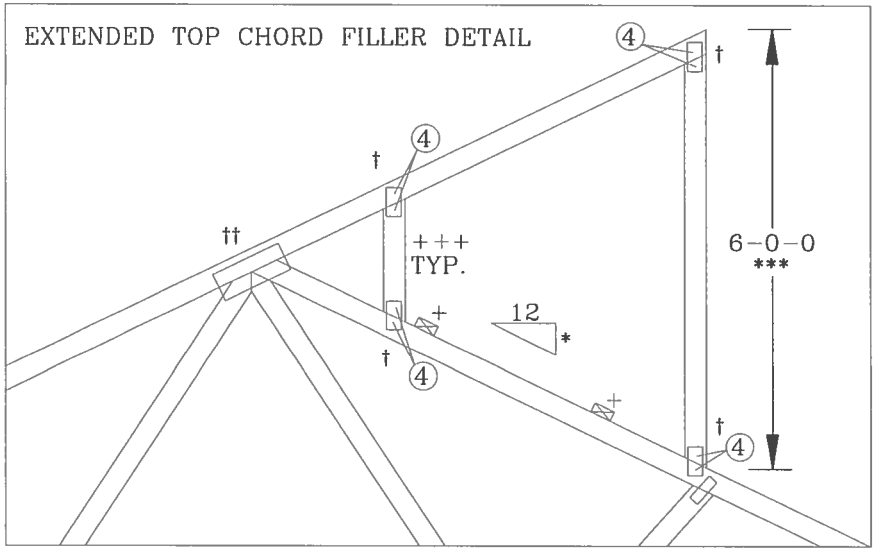
*** 6'0" MAXIMUM HEIGHT.

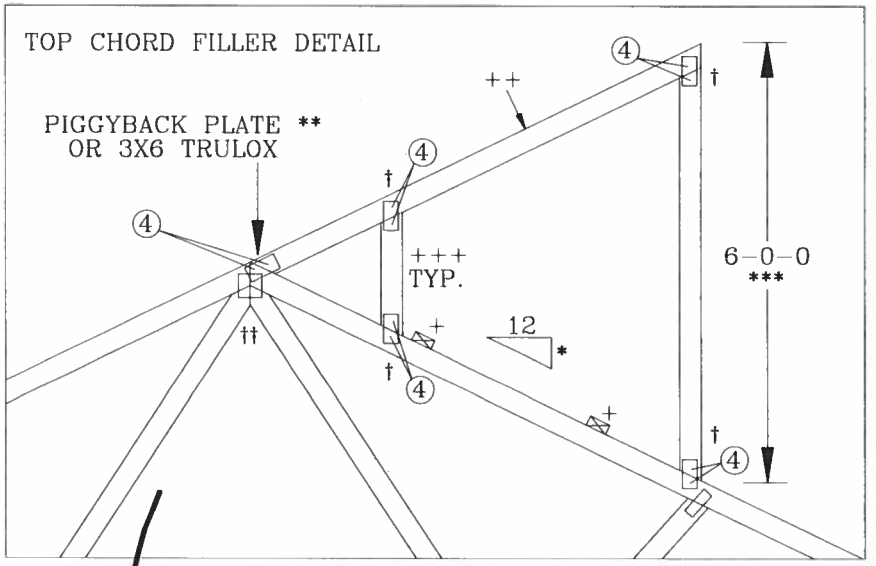
† W2X4 OR 3X6 TRULOX.

†† REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

11 GAUGE (0.120")X1.375" NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO EACH FACE OF EACH TRUSS PLY. SEE DWG 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS.







THIS DRAWING REPLACES DRAWING 884,080

ALPINE

ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

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ARTHUR R. FISHER
LICENSE
NO. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 11,03

TC LL	MAX 30	PSF
TC DL	MAX 15	PSF
BC DL	MAX 10	PSF
BC LL	0	PSF
TOT. LD.	MAX 55	PSF
DUR. FAC.	1.15 OR 1.33	
SPACING	24.0"	

REF	TC-FILLER
DATE	11/26/03
DRWG	TCFILLER1103
-ENG	SJP/KAR

BOTTOM CHORD FILLER DETAIL

* OPTIONAL INTERIOR OR CANTILEVER BEARING. MINIMUM PLATE SIZES (1X3 WAVE) MAY BE USED IF BEARING IS OMITTED. WEDGE OR VERTICAL MEMBER MUST COINCIDE WITH BEARING LOCATION.

\+ 3X4 WAVE OR 4X8 TRULOX

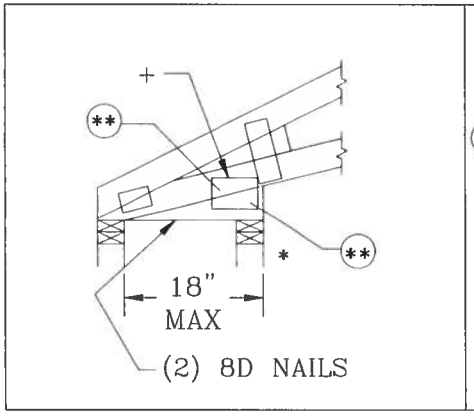
++ 2X4 WAVE OR 3X6 TRULOX

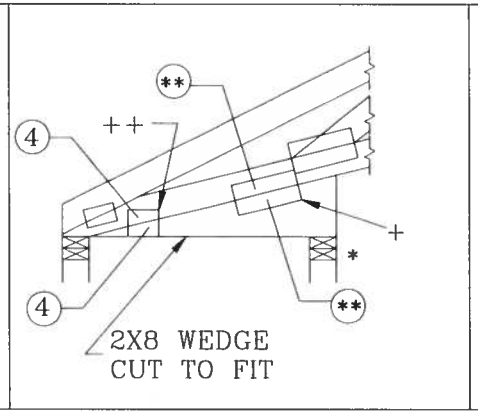
11 GAUGE (0.120")X1.375" NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO EACH FACE OF THE TRUSS. SEE DWG 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS.

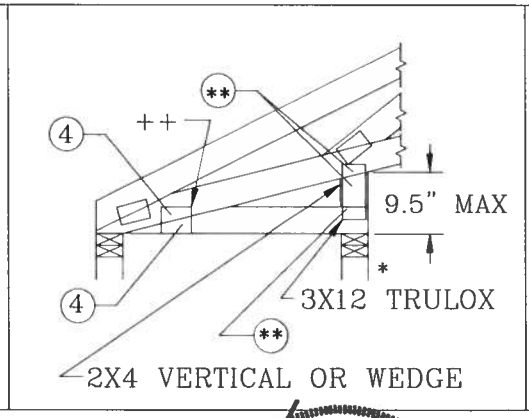
REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

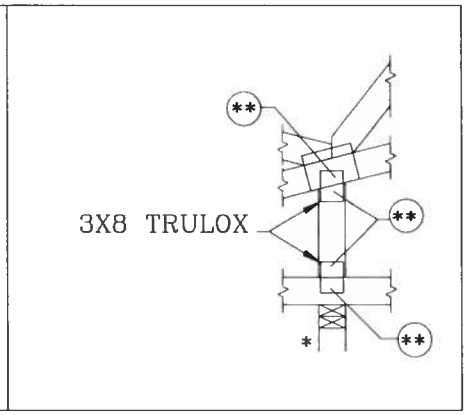
ALL TRULOX PLATES SHOWN ARE MINIMUMS. LARGER PLATES MAY BE REQUIRED TO ACCOMODATE REQUIRED NAILS (**)

FILLER BOTTOM CHORD OR WEDGE SPECIES	MAXIMUM REACTION		MINIMUM BEARING AREA	** REQUIRED NAILS PER FACE WITH TRULOX PLATES				
	DOWNWARD	UPLIFT		1.00 D.O.L.	1.15 D.O.L.	1.25 D.O.L.	1.33 D.O.L.	1.60 D.O.L.
DOUGLAS FIR-LARCH	3281#	1656#	1.5" X 3.5"	12	11	10	9	8
HEM-FIR	2126#	1095#	1.5" X 3.5"	9	8	7	7	6
SPRUCE-PINE-FIR	2231#	1192#	1.5" X 3.5"	10	9	8	8	6
SOUTHERN PINE DENSE	3465#	1791#	1.5" X 3.5"	12	11	10	9	8
SOUTHERN PINE	2966#	1492#	1.5" X 3.5"	10	9	8	8	7
SOUTHERN PINE NON-DENSE	2520#	1343#	1.5" X 3.5"	9	8	7	7	6









DRAWING REPLACES DRAWINGS A115 A115/R & 884,132

ALPINE

ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W,K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER

TC LL	—	PSF
TC DL	—	PSF
BC DL	10.0	PSF
BC LL	—	PSF
TOT. LD.	—	PSF
DUR. FAC. 1.0/1.15/1.25/1.33		
SPACING 24.0"		

REF	BC FILLER
DATE	11/26/03
DRWG	BCFILLER1103
-ENG	DLJ/KAR

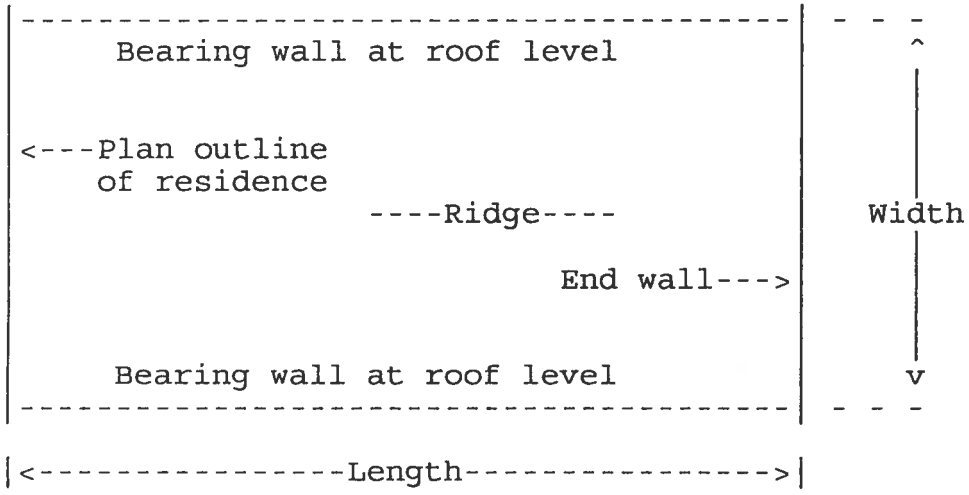
Data entry by: MT Date: 06 01 06

Project name: ZEBRA CT. ~~Bldg~~ - 1
Location : COLUMBIA COUNTY

R E S I D E N T I A L W I N D D E S I G N A N D A N A L Y S I S
A product of EDA Software, Inc.
Based on the Standard Building Code, 1994 edition

**** GENERAL INPUT DATA ****

Permanent construction
Simple rectangular building



Length along bearing walls out to out of studs = 66 feet
Width along end walls out to out of studs = 28 feet
Roof overhang in long direction from outer face of stud = 2 feet generally
Roof overhang at short end wall from outer face of stud = 2 feet generally
Height of exterior wall to top of plate on long side = 8 feet constant
Roof cross slope = 6 /12

Wind velocity = 100 mph

**** DEGREE OF ENCLOSURE ****

Assume that this building is an 'Enclosed building' per Code 1606.2.3.

AR7005 27 Jun 2006

**** STRUCTURAL FRAMING INPUT DATA ****

*** Roof Structural Data ***

Member number 1
Jack truss--hip-ended roof
Span length out to out of supports = 28 feet
Roof cross slope = 6 /12
Truss spacing = 24 inches
Overhang = 2 feet

Member number 2
Jack truss--hip-ended roof
Span length out to out of supports = 22 feet
Roof cross slope = 6 /12
Truss spacing = 24 inches
Overhang = 2 feet

*** Wall Structural Data ***

Spacing of wall studs = 16 inches
Total number of plates = 3
Wall stud number 1 is 8 feet high out to out of plates

COEFFICIENTS AND PRESSURES
Main Wind Force Resisting Systems

Actual pressure = Velocity pressure x Use factor x Coefficient
Wind velocity is 100 mph
Mean roof height is 11.87268 feet
Velocity pressure is 20.4 psf
Use factor is 1.0
Roof cross slope is 6 on 12, which equals 26.56505 degrees to horizontal
End zone width is 6 feet

	Coefficient	Design Pressure (psf)
End zone		
Windward wall (1E)	.7	14.28
Windward roof (2E)	-1	-20.4
Leeward roof (3E)	-1	-20.4
Leeward wall (4E)	-.95	-19.38
Overhang	-1.5	-30.6
Interior zone		
Windward wall (1)	.4	8.16
Windward roof (2)	-.75	-15.3
Leeward roof (3)	-.75	-15.3
Leeward wall (4)	-.7	-14.28
Overhang	-1.5	-30.6

ROOF LOADING--Roof Number 1 (pounds per square foot)

Roof cross slope = 6 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
No insulation	
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.215147
Total roof unit weight on slope	= 6.075148
Cosine of roof cross slope	= .8944272
Roof unit weight on horizontal	= 6.792222
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Ceiling insulation R-30	= .5
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
Total	= 10.59222

Roof Unit Dead Load = 11 psf
Roof dead load supported generally by wall = 159.7911 plf

ROOF LOADING--Roof Number 2 (pounds per square foot)

Roof cross slope = 6 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
No insulation	
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.215147
Total roof unit weight on slope	= 6.075148
Cosine of roof cross slope	= .8944272
Roof unit weight on horizontal	= 6.792222
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Ceiling insulation R-30	= .5
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
Total	= 10.59222

Roof Unit Dead Load = 11 psf
Roof dead load supported generally by wall = 159.7911 plf

ROOF LOADING--Roof Number 3 (pounds per square foot)

Roof cross slope = 6 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
No insulation	
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.215147
Total roof unit weight on slope	= 6.075148
Cosine of roof cross slope	= .8944272
Roof unit weight on horizontal	= 6.792222
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Ceiling insulation R-30	= .5
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
Total	= 10.59222

Roof Unit Dead Load = 11 psf
Roof dead load supported generally by wall = 159.7911 plf

ROOF LOADING--Roof Number 4 (pounds per square foot)

Roof cross slope = 6 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
No insulation	
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.215147
Total roof unit weight on slope	= 6.075148
Cosine of roof cross slope	= .8944272
Roof unit weight on horizontal	= 6.792222
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Ceiling insulation R-30	= .5
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
Total	= 10.59222

Roof Unit Dead Load = 11 psf
Roof dead load supported generally by wall = 159.7911 plf

ROOF MEMBER DEAD LOAD REACTIONS AT BEARINGS
All values are in pounds

Roof member number 1 --Span 28 feet, Slope 6 /12, interior zone---- 319
Roof member number 2 --Span 28 feet, Slope 6 /12, end zone--------- 319
Roof member number 3 --Span 22 feet, Slope 6 /12, interior zone---- 256
Roof member number 4 --Span 22 feet, Slope 6 /12, end zone--------- 256

EXTERIOR WALL LOADING (pounds per linear foot)

Wood frame wall-- 8 ft. out to out plates

3--2 in. x 4 in. plates	= 2.865625
2 in. x 4 in. studs at 16 in. spacing	= 5.462598
R-13 Insulation	= 1.90625
3/8 in. Mineral board siding	= 12.1875
1/2 in. Gypsum board--Total 1 layer---	= 16
Total	= 38.42197

Exterior Wall Unit Dead Load = 39 plf

S U M M A R Y O F H U R R I C A N E A N C H O R A N A L Y S I S

All values of forces are in pounds. Resistances have been increased for wind.
End zone width = 6 feet
Code: C = Compliance N = Non-compliance

Simpson hurricane anchors

Member 1 --Hip roof--Span 28 feet, at 24 inches oc--in interior zone:
Uplift = 771 Dead = 319 Net = 452 Model Special, Resistance = 717 C
Model H9--all nails installed per manufacturers catalog
Data supplied by operator--not from EDA database

Member 2 --Hip roof--Span 28 feet, at 24 inches oc--in end zone:
Uplift = 771 Dead = 319 Net = 452 Model Special, Resistance = 717 C
Model H9--all nails installed per manufacturers catalog
Data supplied by operator--not from EDA database

Member 3 --Hip roof--Span 22 feet, at 24 inches oc--in interior zone:
Uplift = 638 Dead = 256 Net = 382 Model Special, Resistance = 717 C
Model H9--all nails installed per manufacturers catalog
Data supplied by operator--not from EDA database

Member 4 --Hip roof--Span 22 feet, at 24 inches oc--in end zone:
Uplift = 638 Dead = 256 Net = 382 Model Special, Resistance = 717 C
Model H9--all nails installed per manufacturers catalog
Data supplied by operator--not from EDA database

**** ANALYSIS OF ROOF SHEATHING AS SHEAR DIAPHRAGM TRANSVERSE ****
Shear analysis applies along supporting shearwalls.

Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is Oriented Strand Board, 7/16 inch thick
Sheathing has no intermediate blocking
Fasteners on panel ends are 8d nails spaced at 5 inches
Fasteners in panel interior are 8d nails spaced at 10 inches

Total lateral wind force on building = 11911 pounds
Total force transferred through diaphragm to shearwalls = 5955 pounds
Total length of shearwalls = 56 feet
MINIMUM REQUIRED TOTAL SHEARWALL LENGTH = 23.7 FT.--LOCATE EVENLY THROUGHOUT

Actual diaphragm force per unit length of shearwall = 106 plf
Allowable diaphragm force per unit length of shearwall = 251 plf

---

*** Summary of Analysis ***
Roof sheathing diaphragm satisfies Code requirements.

**** ANALYSIS OF ROOF SHEATHING AS SHEAR DIAPHRAGM LONGITUDINAL ****
Shear analysis applies along supporting shearwalls.

Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is Oriented Strand Board, 7/16 inch thick
Sheathing has no intermediate blocking
Fasteners on panel ends are 8d nails spaced at 5 inches
Fasteners in panel interior are 8d nails spaced at 10 inches

Total lateral wind force on building = 4090 pounds
Total force transferred through diaphragm to shearwalls = 2045 pounds
Total length of shearwalls = 132 feet
MINIMUM REQUIRED TOTAL SHEARWALL LENGTH = 7.9 FT.--LOCATE EVENLY THROUGHOUT

Actual diaphragm force per unit length of shearwall = 15 plf
Allowable diaphragm force per unit length of shearwall = 251 plf

---

*** Summary of Analysis ***
Roof sheathing diaphragm satisfies Code requirements.

**** ANALYSIS OF ROOF SHEATHING FOR FASTENER WITHDRAWAL ****

Interior zone (area Ri)
Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is 7/16 inch with no intermediate blocking
Size of sheathing is 48 inches by 96 inches
Fasteners along end trusses are 8d nails spaced at 5 inches
Fasteners along int. trusses are 8d nails spaced at 10 inches
Total outward wind force on sheathing = 656 pounds
Total withdrawal resistance of 40 nails = 3038 pounds (increased for wind)
Fastening of roof sheathing satisfies Code requirements.

Edge strip (area Si) width = 3 feet
Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is 7/16 inch with no intermediate blocking
Size of sheathing is 48 inches by 96 inches
Fasteners along end trusses are 8d nails spaced at 5 inches
Fasteners along int. trusses are 8d nails spaced at 10 inches
Total outward wind force on sheathing = 1024 pounds
Total withdrawal resistance of 40 nails = 3038 pounds (increased for wind)
Fastening of roof sheathing satisfies Code requirements.

End zone (areas Se and C) width = 6 feet
Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is 7/16 inch with no intermediate blocking
Size of sheathing is 48 inches by 96 inches
Fasteners along end truss are 8d nails spaced at 5 inches
Fasteners along end wall are 8d nails spaced at 5 inches
Fasteners along int. trusses are 8d nails spaced at 10 inches
Total outward wind force on sheathing = 1417 pounds
Total withdrawal resistance of 40 nails = 3038 pounds (increased for wind)
Fastening of roof sheathing satisfies Code requirements.

**** ANALYSIS OF WALL STUDS ****

*** Analysis of Wall Stud Number 1 ***

2 in. x 4 in. single studs at 16 in. spacing
Stud height is 7.625 feet--located in interior zone
Top of studs is laterally supported by ceiling diaphragm or other method
Spruce--Pine--Fir lumber----Number 1--Number 2 grade
Sheathing is inch rated OSB, span rating 24/16

Cross-sectional area = 5.25 sq.in.
Moment of inertia = 5.359375 in.^4
Section Modulus = 3.0625 in.^3
Elastic modulus of wood stud = 1400000 in.^2

Total outward force on stud = 268 pounds
Stud moment = 255 ft-lb.

Stresses:
Stud bending vert : Actual = 1000 psi Allowable = 2415 psi (adjusted)
Stud shear : Actual = 35 psi Allowable = 112 psi (adjusted)
Stud tensile : Actual = 33 psi Allowable = 1020 psi (adjusted)
Interaction bending and tension actual/allowable stress ratio total = .4464316
Sheathing bending hor: Actual = 146 psi Allowable = 222 psi(adjusted)
Deflections:
Stud : Actual = .2226 in. Allowable = .5083 in.

*** Summary of Analysis ***

Wall structure satisfies all Code requirements.

**** ANALYSIS OF WALL STUDS ****

*** Analysis of Wall Stud Number 2 ***

2 in. x 4 in. single studs at 16 in. spacing
Stud height is 7.625 feet--located in end zone
Top of studs is laterally supported by ceiling diaphragm or other method
Spruce--Pine--Fir lumber----Number 1--Number 2 grade
Sheathing is inch rated OSB, span rating 24/16

Cross-sectional area	= 5.25 sq.in.
Moment of inertia	= 5.359375 in.^4
Section Modulus	= 3.0625 in.^3
Elastic modulus of wood stud	= 1400000 in.^2
Total outward force on stud	= 309 pounds
Stud moment	= 294 ft-lb.

Stresses:

Stud bending vert	: Actual = 1154 psi	Allowable = 2415 psi	(adjusted)
Stud shear	: Actual = 40 psi	Allowable = 112 psi	(adjusted)
Stud tensile	: Actual = 33 psi	Allowable = 1020 psi	(adjusted)

Interaction bending and tension actual/allowable stress ratio total = .5101997

Sheathing bending hor: Actual = 169 psi Allowable = 222 psi(adjusted)

Deflections:

Stud : Actual = .2567 in. Allowable = .5083 in.

*** Summary of Analysis ***

Wall structure satisfies all Code requirements.

**** ALLOWABLE STRESS PROPERTIES ****

Base stresses (psi):
Wood:
   Bending   = 875
   Tension   = 425
   Shear   = 70
   Elastic modulus = 1400000

Adjustment factors for wood:
   Duration (Du)   = 1.6
   Wet service (Wt)   = 1
   Temperature (Tm)   = 1
   Stability (St)   = 1
   Size (Sz)   = 1.5
   Volume (Vm)   = 1
   Flat use (Fu)   = 1
   Repetitive (Rp)   = 1.15
   Curvature (Cu)   = 1
   Form (Fm)   = 1
   Shear stress (Sh) = 1

Allowable stresses (psi):
Wood:
   Bending = 2415 (Base x Du x Wt x Tm x St x Sz x Vm x Fu x Rp x Cu x Fm)
   Tension = 1020 (Base x Du x Wt x Tm x Sz)
   Shear   = 112 (Base x Du x Wt x Tm x Sh)
   Elastic modulus = 2240000 (Base x Wt x Tm)

Sheathing:
   Bending   = 222 (Base x 1.33)
   Elastic modulus = 61904.76 (Base)

T R A N S V E R S E D R A G S T R U T N A I L A N A L Y S I S

Wall framing is 2 in. x 4 in. studs
Wall stud framing lumber is Spruce--Pine--Fir
Fasteners are 16d common nails
Approximate nail spacing = 20 inches

Total lateral force on building = 11911 pounds
Force applied at top of walls = 5955 pounds
Total dragstrut length = 56 feet
Shear per unit dragstrut length = 106 pounds per linear foot

Actual shear on each nail = 176 pounds
Allowable shear on each nail = 192 pounds

Dragstrut nailing satisfies Code requirements.

L O N G I T U D I N A L D R A G S T R U T N A I L A N A L Y S I S

Wall framing is 2 in. x 4 in. studs
Wall stud framing lumber is Spruce--Pine--Fir
Fasteners are 16d common nails
Approximate nail spacing = 20 inches

Total lateral force on building = 4090 pounds
Force applied at top of walls = 2045 pounds
Total dragstrut length = 132 feet
Shear per unit dragstrut length = 15 pounds per linear foot

Actual shear on each nail = 25 pounds
Allowable shear on each nail = 192 pounds

Dragstrut nailing satisfies Code requirements.

**** T R A N S V E R S E S H E A R W A L L A N A L Y S I S ****

Wall framing is 2 in. x 4 in. studs at 16 inch spacing
Wall stud framing lumber is Spruce--Pine--Fir
Wall shear siding is Oriented Strand Board -- 7/16 inch thick
Wall sheathing has all edges nailed
Fasteners: 8d common nails spaced along edges at 5 inch centers
Fasteners: 8d common nails spaced in interior at 10 inch centers

Total lateral force on building = 11911 pounds
Force applied at top of walls = 5955 pounds
Accumulated total shearwall length = 56 feet

Actual unit shear on shearwalls = 106 pounds per linear foot
Allowable unit shear on shearwalls = 257 pounds per linear foot

Shearwall satisfies Code requirements.

**** L O N G I T U D I N A L S H E A R W A L L A N A L Y S I S ****

Wall framing is 2 in. x 4 in. studs at 16 inch spacing
Wall stud framing lumber is Spruce--Pine--Fir
Wall shear siding is Oriented Strand Board -- 7/16 inch thick
Wall sheathing has all edges nailed
Fasteners: 8d common nails spaced along edges at 5 inch centers
Fasteners: 8d common nails spaced in interior at 10 inch centers

Total lateral force on building = 4090 pounds
Force applied at top of walls = 2045 pounds
Accumulated total shearwall length = 132 feet

Actual unit shear on shearwalls = 15 pounds per linear foot
Allowable unit shear on shearwalls = 257 pounds per linear foot

Shearwall satisfies Code requirements.

*** ANALYSIS OF OUTWARD FORCES ON WALL SHEATHING ***

Wall number 1 : Total outward wind force on sheathing = 804 pounds
: Total withdrawal resistance of 76 nails = 4240 pounds

Wall number 2 : Total outward wind force on sheathing = 927 pounds
: Total withdrawal resistance of 76 nails = 4240 pounds

**** ANALYSIS OF SHEATHING FASTENERS ****

Wall framing is Spruce--Pine--Fir lumber
Sheathing is 7/16 inch Oriented Strand Board
Sheathing extends from bottom of bottom plate to top of top plate
Fasteners are 8d common nails at 5 inch spacing

Total uniform wind uplift in first story at top of wall level = 290 plf
Uniform dead loads per linear foot:
Roof = 159.7911 plf

Total = 159.7911 plf
Total uniform dead load in first story at top of wall level = 159 plf
Net wind uplift in first story at top of wall level = 131 plf

Total uplift force on each nail = 54 pounds
Allowable shear on each nail = 97 pounds (increased for wind)
Sheathing to plate fastening satisfies all Code requirements.

**** ANALYSIS OF SHEATHING FASTENERS ****

Wall framing is Spruce--Pine--Fir lumber
Sheathing is 7/16 inch Oriented Strand Board
Sheathing extends from bottom of bottom plate to top of top plate
Fasteners are 8d common nails at 5 inch spacing

Total uniform wind uplift in first story at floor level = 290 plf
Uniform dead loads per linear foot:
Roof = 159.7911 plf
Wall = 38.42197 plf

Total = 198.2131 plf
Total uniform dead load in first story at floor level = 198 plf
Net wind uplift in first story at floor level = 92 plf

Total uplift force on each nail = 38 pounds
Allowable shear on each nail = 97 pounds (increased for wind)
Sheathing to plate fastening satisfies all Code requirements.

**** ANALYSIS OF FOUNDATION ANCHORAGE ****

Anchor bolts are 1/2 inch A307, with 2 inch round washer at 48 inch centers.

Total uniform wind uplift on foundation = 290 pounds per linear foot
Uniform dead loads in pounds per linear foot:
Roof = 159.7911 plf
Wall = 38.42197 plf

Total = 198.2131 plf
Total uniform dead load times 2/3 = 132 pounds per linear foot
Net uplift force on foundation = 158 pounds per linear foot

Total uplift force on each anchor bolt = 632 pounds
Safe tension value of each anchor bolt = 1634 pounds (increased by 1/3)
Bolt safe tension value is governed by washer failure

*** Summary of Analysis ***
Foundation anchorage satisfies all Code requirements.

**** ANALYSIS OF CORNER HOLD-DOWN REQUIREMENTS ****

Hold-down is one typical anchor bolt with washer, each wall
Normal anchor bolt spacing = 48 inches
Distance from corner to hold-down device = 6 inches
Distance from corner to first interior anchor bolt = 48 inches
Net uplift force on foundation = 158 pounds per linear foot
Tributary distance to corner device = 2.25 feet
Net uplift on corner hold-down device = 355 pounds

Uplift tension due to shearwall action in a transverse shearwall segment:
Distance from corner to hold-down device = 6 inches
Distance from corner to first interior anchor bolt = 48 inches
Total shear from shearwall segment = 319 pounds
Height of wall = 8 feet
Uniform dead load times 2/3 = 25 pounds per linear foot
Shearwall moment at bottom of wall = 2552 foot-pounds
Additional tension at corner device = 1237 pounds
Total uplift tension on corner hold-down devices = 1592 pounds
Allowable tension on corner hold-down devices = 3268 pounds

*** Summary of Analysis ***
Corner hold-down device COMPLIES with Code requirements.

**** ANALYSIS OF FOUNDATION ****

Stemwall is 8 inch concrete masonry, filled with grout, 16 inches high
Footing is 20 inches wide by 10 inches deep
Earth cover over top of footing is 4 inches

Total uniform wind uplift on foundation = 290 pounds per linear foot
Uniform dead loads in pounds per linear foot:
Roof = 159.7911 plf
Wall = 38.42197 plf

Total = 198.2131 plf
Total uniform dead load times 2/3 = 132 pounds per linear foot
Net uplift force at top of foundation = 158 pounds per linear foot
Weight of stemwall footing earth x 2/3 = 261 pounds per linear foot
Net uplift at bottom of footing = 0 pounds per linear foot

*** Summary of Analysis ***
Foundation is stable.

**** ANALYSIS OF REINFORCING STEEL ****

Grade 40 reinforcing steel, Number 5 vert. bars at 72 inch centers

Total uniform wind uplift on foundation = 290 pounds per linear feet
Uniform dead loads in pounds per linear foot:
Roof = 159.7911 plf
Wall = 38.42197 plf

Total = 198.2131 plf
Total uniform dead load times 2/3 = 132 pounds per linear foot
Net uplift force on foundation = 158 pounds per linear foot
Weight of concrete block stemwall x 2/3 = 81 pounds per linear foot
Net uplift at top of footing = 77 pounds per linear foot

Total uplift force on each re-bar = 462 pounds
Safe tension value of each re-bar = 8181 pounds (increased by 1/3)

*** Summary of Analysis ***
Reinforcing steel satisfies all Code requirements.

**** SUMMARY OF REINFORCING DATA ****

Foundation wall data:
Wall is composed of 8 inch concrete masonry, fully grouted.
Wall reinforcing is Grade 40 steel, Number 5 at 72 inch centers
Minimum required lap splice for Number 5 bar is 25 inches.
Minimum required clearance for Number 5 bar is 1.5 inches.
Wall reinf. in footing has a std. A.C.I. hook, 6 inches below top of footing.

Footing data:
Footing is continuous, 20 inches wide by 10 inches deep.
Footing concrete is 2500 psi
Footing reinforcing is Grade 40 steel, 2--#( ) longitudinal.
Minimum required splice length = 25 inches
Reinforcing steel shall have cover as follows:
    Top-------6 inches
    Sides-----3 inches
    Bottom----3 inches