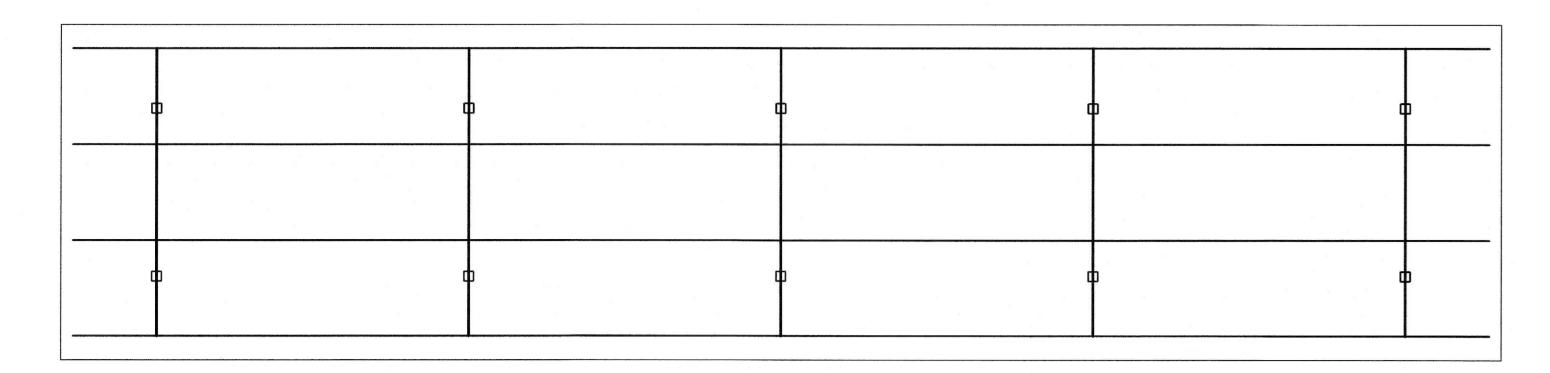
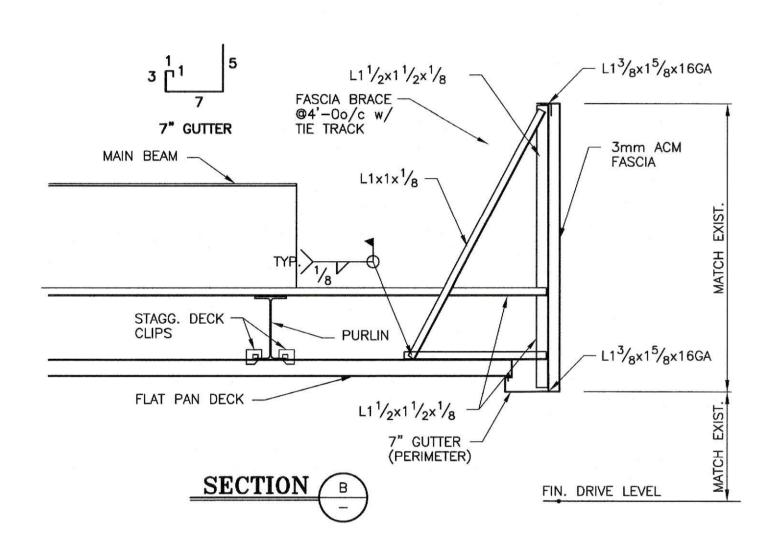
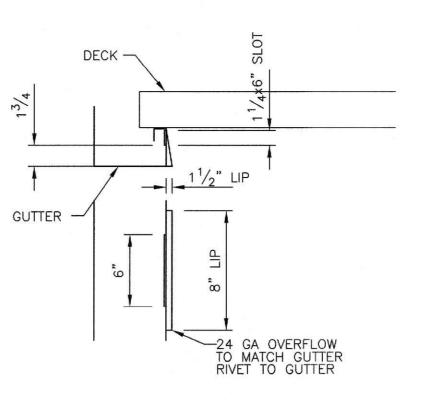
FASCIA TO BE 3mm A.C.M. AROUND ENTIRE CANOPY



ROOF FRAMING PLAN

SCALE: $\frac{1}{8} = 1'-0$





OVERFLOW

SCALE: $1^{1}/_{2} = 1'-0$

GENERAL NOTES

FOUNDATIONS:

THE SOIL BEARING VALUE SHALL NOT BE LESS THAN 2000 PSF. THE BEARING VALUE SHALL BE VERIFIED BY THE G.C. FOOTINGS SHALL BEAR IN UNDISTURBED SOIL. ALL T/FOOT. ELEVATION ARE TO BE THE SAME UNLESS SHOWN OTHERWISE. IF ELEVATIONS ARE NOT THE SAME, CHAPMAN CANOPIES MUST BE INSTRUCTED OF ELEV.'S IN WRITING WHEN CANOPY ORDER IS PLACED. FOOTINGS DESIGN COMPLIES W/ACI-318.

REINFORCING STEEL:

ALL DEFORMED BARS SHALL COMPLY W/ASTM A615 FY=60 AND SHALL BE WIRE TIED AT ALL JOINTS. RUSTY, OILY, OR DIRTY STEEL SHALL NOT BE USED.

ANCHOR BOLTS:

ANCHOR BOLTS MUST BE INSTALLED WITH A TEMPLATE AND WITHIN 1/8-INCH OF MEASUREMENTS OF THE BASE PLATE OR COLUMN WILL NOT FIT. CONCRETE CONTRACTOR IS RESPONSIBLE FOR RECESSING FOOTINGS 10 - INCHES BELOW FINISH GRADE AND FOR EXTENDING ANCHOR BOLTS 5 - INCHES ABOVE FOOTINGS IN ORDER FOR CANOPY TO ERECT PROPERLY. ANCHOR BOLTS SHALL BE ASTM F1554 GRADE 36.

CONCRETE:

ALL CONCRETE SHALL BE 3000 PSI IN 28 DAYS. ALL CONC. SHALL BE PLACED IN ACCORDANCE WITH ACI-318.

STRUCTURAL STEEL:

ALL STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AISC SPECIFICATIONS. W-SHAPES SHALL BE ASTM A992. MISCELLANEOUS STEEL, PLATES, ANGLES, AND CHANNELS ARE TO BE ASTM A36. HSS SHAPES SHALL BE ASTM A500 GR. C. PROOF OF WELDERS CERTIFICATION SHALL BE AVAILABLE ON REQUEST. ALL BOLTS SHALL BE IN ACCORDANCE w/ASTM A325.

OTHER CONCRETE ITEMS:

OTHER CONCRETE ITEMS SUCH AS DRIVE THRU SLAB, BUILDING SLAB, PERIMETER FOOTING AND LOAD BEARING FOOTINGS NOT USED FOR THE CANOPY ARE TO BE AT THE SAME ELEVATION UNLESS SHOWN OTHERWISE. IF ELEVATIONS ARE TO VARY, CHAPMAN CANOPIES MUST BE INSTRUCTED OF ELEVATIONS IN WRITING WHEN ORDER IS PLACED.

FOUNDATION NOTE:

PARAMETER

4.0 PSF (plus SELFWEIGHT)

20.0 PSF

1.0 PSF + DRIFTS

Vult=122.0(Vasd=94.5)MPH 3s Gust qz=28.9 PSF

FOOTINGS PROVIDED BY CUSTOMER. CHAPMAN CAN. IS NOT RESPONSIBLE OR LIABLE FOR THE USE OF EXIST. FOOTINGS.

(w/ APP. RED.)

Case B Cnw Cnl

Cfx

1.84(53.11)

-1.10(-28.61) -.10(-2.60)

CODE REFERENCE

FBC 2023

8th Ed.

ASCE 7-22, PART 7.0

ASCE 7, PART 26-29

ASCE 7, FIG. 27.3-4

ASCE 7, FIG. 29.3-1

ASCE 7-22

FIG. 22.1

FIG. 22.2

FIG. 22-14

TAB. 1.5-1

TAB. 1.5-2

TAB. 11.4-1

TAB. 11.4-2

TAB. 20.3-1

EQ. 11.4-1

EQ. 11.4-2

EQ. 11.4-3

EQ. 11.4-4

12.8

12.8

1606.1

0.0790

0.0480

1.00

1.60

2.40

0.1264

0.1152

0.0843

0.0768

V=CsW

0.0281 0.0674 12.8.1.1

3.00 | 1.25 | TAB. 12.2-1

.59 1.42 EQ. 12.8-1

1607.12.2.1

NOTE: THIS CANOPY IS DESIGNED PER ASCE 7-22 SEE FIG. 27.3-4, UNBALANCED WIND LOAD 1.20(31.21) CASE A .30(7.80)DEAD LOAD LIVE LOAD -.10(-2.60)-1.10(-28.61) SNOW LOAD CASE B WIND SPEED * CLEAR WIND FLOW NOTES: 1. THESE LOADS HAVE BEEN APPLIED TO STRUCTURE IN ACCORDANCE WITH ASCE 7-22, CHAPTER 2, 2.4.1 BASIC COMBINATIONS FOR ALLOWABLE STRESS SEISMIC DATA 2. THESE STEEL MEMBERS HAVE BEEN SIZED BASED ON SEISMIC DESIGN CATEGORY TAB. 11.6(1)(2) ASD, AISC 15th EDITION. 3. ANALYSIS OF THIS STRUCTURE HAS BEEN ACCOMPLISHED USING THE LATEST GENERATION OF MATRIX BASED SOFTWARE. 4. COLUMN SLENDERNESS FACTORS ARE BASED ON CHAPTER C, DIRECT ANALYSIS METHOD. Kx = 1.0Kz = 1.05. BASES ARE FIXED.

Digitally signed by Date:

NO. DATE:

CARTER MILLER SANSING: 2024-345 DATE: 8/10/2024

10:30:56 -06'00'

2 3

FLORIDA (CA 31480) No: 50338(Exp. 2/28/2025) BY: Sea ST. LICENCE.

STRUCTURE LOADS

CATEGORY II

Case A & B

RISK CATEGORY

SEISMIC FACTOR, le

SITE COEFFICIENT, Fa

SITE COEFFICIENT, FV

SITE CLASSIFICATION

W, Kips

В

VERT. ROOF PRESSURE Case A Cnw Cnl

1.20(31.21) .30(7.80)

Cfz

.2 SEC. SPECTRUM RESPONSE, Ss

1 SEC. SPECTRUM RESPONSE, S1

LONG PERIOD TRANSITION PERIOD, TI

SITE ADJUSTMENT COEFFICENT, Sms

SITE ADJUSTMENT COEFFICENT, Sm1

DESIGN SPECTRAL RESPONSE, SDS

DESIGN SPECTRAL RESPONSE, SD1

SEISMIC RESPONSE COEFFICIENT, Cs

Non-Detailed Sys./Orid. Cant. Col.

RESPONSE MODIFICATION FACTOR, R

BASIC STRUCTURAL SYSTEM - SEISMIC RESISTING SYSTEM

METHOD OF ANALYSIS - EQUIVALENT LATERAL FORCE

PREPARED BY: CARTER-MILLER-SANSING, LTD., P.O.B 4324, MERIDIAN, MS 39304, 601-483-0601

1.91(55.32)

HORIZ. FASCIA PRESSURE

★140.050338 ★ STATE OF

CERTIFICATION

GREGORY R. MILLER

CHAPMAN CANOPY, INC. 201 PATRICIA STREET/P.O. BOX 3527/HUEYTOWN, AL 35023

PHONE: 1-(205) 491-4845 S&S FOOD STORES

CALE: NOTED LOCATION: 7905 SW US 27 FORT WHITE, FL DRAWING NO: SHEET 1 of 1 24194-123

Gregory R 2024.11.25