DATE 09/17/2007

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000026241

APPLICANT HUGO ESCALANTE PHONE 386.288.8666

ADDRESS POB 280 FT. WHITE FL 32038

OWNER VERLENE GRIFFIN PHONE 386.497.2956

ADDRESS 141 SCRUBTOWN ROAD FT. WHITE FL 32038

CONTRACTOR HUGO ESCALANTE PHONE 386.288.8666

LOCATION OF PROPERTY 441-S TO C-18,TR TO TUSTENUGGEE AVE,TL TO OLD BELLAMY,TL TO SCRUBTOWN,TR AND IT'S THE CORNER LOT ON L.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 79000.00

HEATED FLOOR AREA 1580.00 TOTAL AREA 2116.00 HEIGHT 18.60 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 05-7S-17-09898-003 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES 1.00

		CRC1326967		
Culvert Permit No.	Culvert Waiver	Contractor's License Number		Applicant/Owner/Contractor
EXISTING	07-638	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: FLOOR ONE FOOT ABOVE THE ROAD. LEGAL NON-CONFORMING LOT THROUGH TH ESTATE SUCCESSIONS OF LINEAL DESCENDANTS.

Check # or Cash 5477

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

RECORDED 0775 PG 1028

OFFICIAL RECORDS

RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Grantee #1 S.S. No. 261-44-5793

Property Appraiser's
Parcel Identification No.
05751709898003

DOCUMENTARY STAMP 1.70
INTANGIBLE TAX 0
P. DeWITT CASON, CLERK OF
COURTS, COLUMBIA COUNTY
BY Mary Lou D.C.

WARRANTY DEED

THIS INDENTURE, made this 12 day of May, 1993, BETWEEN **ELLA MAE RANDOLPH, FLORENCE McCLOUD, JAMES ZIMMERMAN, TOMMIE LEE ZIMMERMAN, RUDOLPH ZIMMERMAN, VERLENE GRIFFIN, LOUISE JAMES and ROBERT LEE ZIMMERMAN,** whose post office address is Route 2, Box 150, High Springs, Florida 32643, of the County of Alachua, State of Florida, grantor*, and **ROBERT LEE ZIMMERMAN,** whose post office address is 519 Hull Street, Lakeland, Florida 33805, of the County of Polk, State of Florida, grantee*.

WITNESSETH: that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever,

BK 0775 PG 1033

OFFICIAL RECORDS

A PART OF THE NE ¼ OF SECTION 5, TOWNSHIP 7 SOUTH, RANGE 17 EAST, MORE PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCE AT THE SE CORNER OF SAID NE ¼ AND RUN S 88°16'07" W, ALONG THE SOUTH LINE THEREOF, 852.25 FEET FOR A POINT OF BEGINNING; THENCE CONTINUE S 88°16'07" W, 411.63 FEET TO THE EAST RIGHT-OF-WAY LINE OF SCRUB TOWN ROAD; THENCE N 06°33'24" W, ALONG SAID EAST RIGHT-OF-WAY LINE 207.39 FEET; THENCE CONTINUE ALONG SAID EAST LINE N 00°26'54" W, 671.81 FEET; THENCE CONTINUE ALONG SAID EAST RIGHT-OF-WAY LINE N 37°51'23" E, 54.72 FEET TO THE SOUTH RIGHT-OF-WAY LINE OF BELLAMY ROAD; THENCE S 82°00'16" E, ALONG SAID SOUTH RIGHT-OF-WAY LINE 374.86 FEET; THENCE S 02°22'45" E, 857.19 FEET TO THE POINT OF BEGINNING. COLUMBIA COUNTY, FLORIDA.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0708-49 Date Received 8/21/07 By G Permit # 26241

Application Approved by - Zoning Official BLK Date 24.08.07 Plans Examiner OKJTH Date 8-22-07

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments Legal Non-conforming Lot, through testate succession of lineal descendant

Need: EH, NOC, ℄

Applicants Name Hugo Escalante Phone 3862888666

Address P.O. BOX 280, Fort White, FL 32038

Owners Name Verlene Griffin Phone 386-497-2956

911 Address 141 SW Scrubtown Road, Fort White, FL 32038

Contractors Name Hugo Escalante (EWP INC) Phone 3862888666

Address P.O. BOX 280, Fort White, FL 32038

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Daniel Shoheen, Lake City, FL

Mortgage Lenders Name & Address First Federal Bank, US 90 Lake City, FL

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 05-7S-17-09898-003 Estimated Cost of Construction @ 140,000

Subdivision Name N/A Lot ____ Block ____ Unit ____ Phase ____

Driving Directions 441 South, T/R @ CR 18, T/L @ Tustenuggee Ave, T/L @ Old Bellany RD, T/R @ Scrubtown RD, Corner Lot on Left

Type of Construction SFD Number of Existing Dwellings on Property ____

Total Acreage 1.0 Lot Size 1 Acre Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Per Owner

Actual Distance of Structure from Property Lines - Front 60' Side 30' Side 30' Rear 200'

Total Building Height 18'-6" Number of Stories 1 Heated Floor Area 1580 Sq FT Roof Pitch 6-12

TOTAL 2,116

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Hugo Escalante

Owner Builder or Agent (Including Contractor)

MARSHA B. WARD
MY COMMISSION # DD 279841
EXPIRES: March 9, 2008
Bonded Thru Notary Public Underwriters

Hugo Escalante

Contractor Signature

Contractors License Number CRC1326967

Competency Card Number ____

NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 21 day of August 2007.

Personally known ✓ or Produced Identification ____

Marsha B Ward

Notary Signature

Griffin

COLUMBIA COUNTY BUILDING DEPARTMENT

Revised 10-01-05

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ——— 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ———110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** Plans or specifications must state compliance with FBC Section 1609. *The following information must be shown as per section 1603.1.4 FBC* a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.
☑	☐	**Elevations including:** a) All sides
☐	☐	b) Roof pitch
☐	☐	c) Overhang dimensions and detail with attic ventilation

☑ ☐ d) Location, size and height above roof of chimneys.
☑ ☐ e) Location and size of skylights
☑ ☐ f) Building height
☑ ☐ e) Number of stories

Floor Plan including:

☑ ☐ a) Rooms labeled and dimensioned.
☑ ☐ b) Shear walls identified.
☑ ☐ c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
☑ ☐ d) Show safety glazing of glass, where required by code.
☐ ☐ e) Identify egress windows in bedrooms, and size.
☑ ☐ f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, (Please circle applicable type).
☑ ☐ g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
☑ ☐ h) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

☑ ☐ a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
☐ ☐ b) All posts and/or column footing including size and reinforcing
☐ ☐ c) Any special support required by soil analysis such as piling
☑ ☐ d) Location of any vertical steel.

Roof System:

☑ ☐ a) Truss package including:
1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

☐ ☐ b) Conventional Framing Layout including:
1. Rafter size, species and spacing
2. Attachment to wall and uplift
3. Ridge beam sized and valley framing and support details
4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

☐ N/A ☐ a) Masonry wall
1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. *All required connectors with uplift rating* and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termiticide or alternative method)
10. Slab on grade
    a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

☑ ☐ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ NA ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

☐ NA ☐ **Floor Framing System:**

☐ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ ☐ b) Floor joist size and spacing

☐ ☐ c) Girder size and spacing

☐ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☐ ☐ f) Appliances and HVAC equipment

☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

☑ ☐ h) Exhaust fans in bathroom

HVAC information

☑ ☐ a) Energy Calculations (dimensions shall match plans)

☑ ☐ b) Manual J sizing equipment or equivalent computation

☐ ☐ c)Gas System Type (LP or Natural) Location and BTU demand of equipment

☑ ☐ **Disclosure Statement for Owner Builders**

☑ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done

☑ ☐ **Private Potable Water**

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)

4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
A development permit will also be required. Development permit cost is $50.00

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit ($25.00) must be made. If the applicant feels that a culvert is not needed, *they may apply for a culvert waiver ($50.00). All culvert waivers are sent to the Columbia County Public* Works Department for approval or denial. **If the project is to be located on a F.D.O.T. maintained road, than an F.D.O.T. access permit is required.**

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING			
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG			
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING			
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES			
B. NON-STRUCT METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			
A.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

APPLICANT SIGNATURE

DATE

Columbia County 9-1-1 Addressing / GIS Department
P.O. Box 1787, Lake City, FL 32056
Telephone: (386) 758-1125 * Fax: (386) 758-1365 * E-mail: ron_croft@columbiacountyfla.com

9-1-1 Address Request Form

NOTE: ADDRESS ASSIGNMENT MAY REQUIRE UP TO 10 WORKING DAYS. IF THE ADDRESSING DEPARTMENT NEEDS TO CONDUCT ON SITE GPS LOCATION IDENTIFICATION, ADDITIONAL TIME MAY BE REQUIRED.

Date of Request: _____

Requester Last Name: _____

First Name: _____

Contact Telephone Number: _____

(Cell Phone Number if Provided): _____

Requested for Self: _____ or Requested for Company: _____
(check one)
If Address is Requested by a Company, Provide Name of Requesting Company:

_____

Parcel Identification Number: ____ - ____ - ____ - _____

If in Subdivision, Provide Name Of Subdivision:

_____

Phase or Unit Number (if any): _____ Block Number (if any): _____

Lot Number: _____

Attach Site Plan or you may use back of Request Form for Site Plan:

Requirements for Site Plan Are Listed on Back of Request From: (NOTE: Site Plan Does NOT have to be a survey or to scale; FURTHER a Environmental Health Dept. Site Plan showing only a 210 by 210 cutout of a property will NOT suffice for Addressing Requirements.)

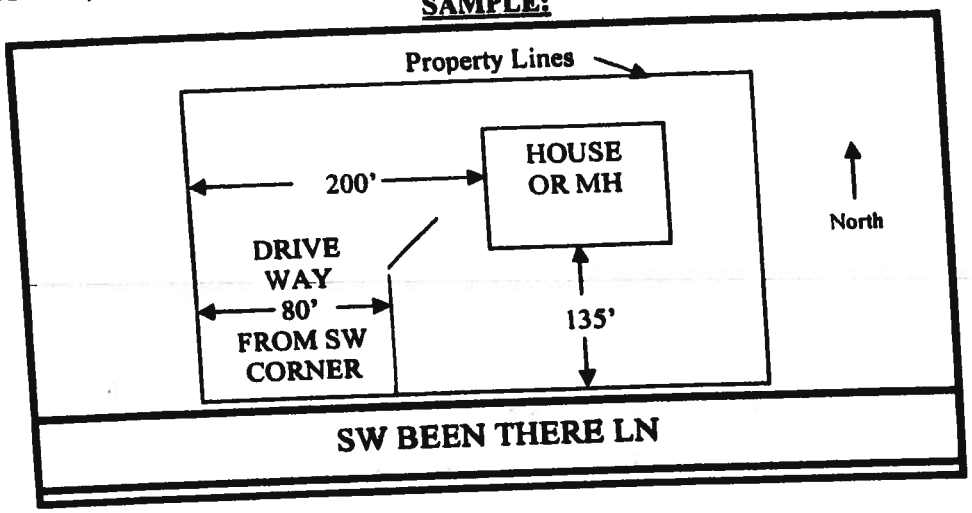
Addressing / GIS Department Use Only:

Date Received: _____ Date Assigned: _____

ID Number: _____

1. A PLAT, PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
2. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM AT LEAST TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
3. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
4. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



SITE PLAN BOX:

Columbia County Property Appraiser

DB Last Updated: 8/2/2007

2007 Proposed Values

Tax Record | Property Card | Interactive GIS Map

New Super Homestead Taxable Value Calculator

Print

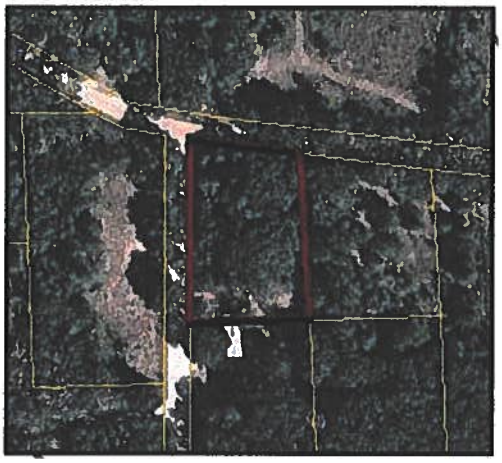
Parcel: 05-7S-17-09898-003

Search Result: 68 of 70 << Prev Next >>

Owner & Property Info

Owner's Name	GRIFFIN VERLENE Z		
Site Address			
Mailing Address	P O BOX 238 FT WHITE, FL 32038		
Use Desc. (code)	NO AG ACRE (009900)		
Neighborhood	5717.00	**Tax District**	3
UD Codes	MKTA02	**Market Area**	02
Total Land Area	1.100 ACRES		
Description	COMM SE COR OF NE1/4, RUN W 852.25 FT FOR POB, CONT W 411.63 FT TO E R/W SCRUB TOWN RD, N ALONG R/W 933.92 FT TO S R/W BELLAMY RD, SE ALONG R/W 374.86 FT, S 857.19 FT TO POB, EX THE S 645 FT & EX THE E 200 FT. ORB 742-1785, 742-1786, 775-1028, QC 1115-281.(CORR QC 1117-679)		

GIS Aerial



Property & Assessment Values

Mkt Land Value	cnt: (1)	$14,520.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$14,520.00

Just Value	$14,520.00
Class Value	$0.00
Assessed Value	$14,520.00
Exempt Value	$0.00
Total Taxable Value	$14,520.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
3/27/2007	1117/679	QC	V	U	01	$0.00

3/5/1991	742/1785	CT	V	U	11	$1,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
009900	AC NON-AG (MKT)	1.100 AC	1.00/1.00/1.00/1.00	$13,200.00	$14,520.00

Columbia County Property Appraiser DB Last Updated: 8/2/2007

[<< Prev] 68 of 70 [Next >>]

Disclaimer

This information was derived from data which was compiled by the Columbia County Property Appraiser's Office solely for the government purpose of property assessment. The information shown is a **work in progress** and should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's Office. The assessed values are **NOT CERTIFIED** values and therefore are subject to change before finalized for ad-valorem assessment purposes.

Notice:
Under Florida Law, e-mail addresses are public record. If you do not want your e-mail address released in response to a public-records request, do not send electronic mail to this entity. Instead contact this office by phone or in writing.

Scroll to Top

A & B Construction Inc.

P. O. Box 39
Ft. White, FL, 32038
386-497-2311

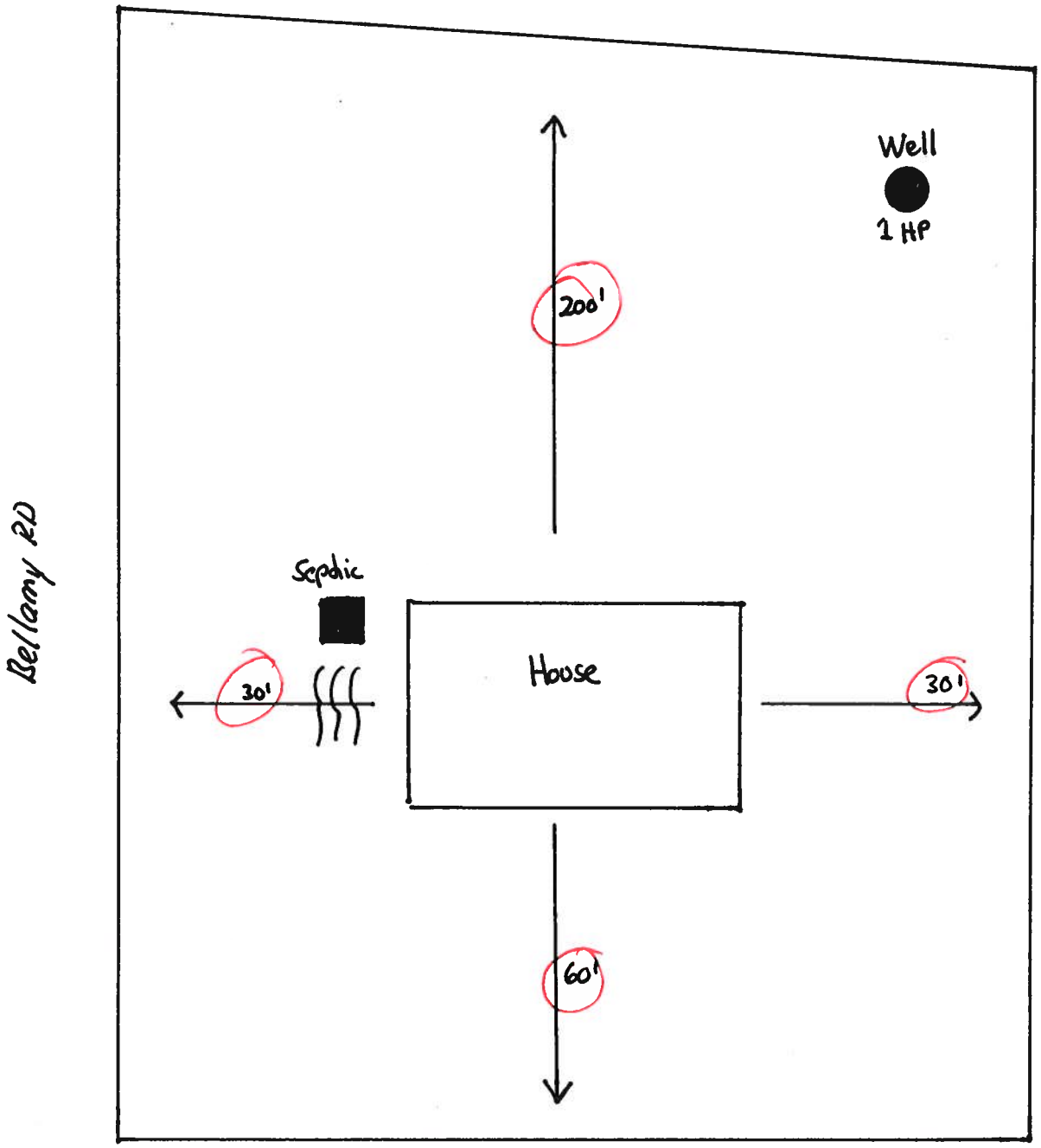
TO: Columbia County Building Department

Description of well to be installed for Customer: GRIFFEN
Located at Address: 141 Scrubtown Road

1 hp – 1 ¼" drop over 86 gallon tank, 250 gallon equivalent captive with back flow preventer. 35-gallon draw down with check valve pass requirements.

William Bias
William Bias

Griffin Residence
Parcel # 05-75-17-09898.003



141 Scrub Town Rd

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 8/6/2007 **DATE ISSUED:** 8/7/2007

ENHANCED 9-1-1 ADDRESS:

141 SW SCRUBTOWN RD

FORT WHITE FL 32038

PROPERTY APPRAISER PARCEL NUMBER:

05-7S-17-09898-003

Remarks:

Address Issued By: _____

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

Approved Address

AUG 07 2007

911Addressing/GIS Dept

Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: 5365E Bayne Dr
City Lake City, FL Phone 386-752-1703

Site Location: Subdivision _____
Lot # _____ Block# _____ Permit # 26241
Address 141 Scrubtown Rd FT. White

Product used	Active Ingredient	% Concentration
☑ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☐ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☐ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Main Body + Front	Porch 2116	223	200

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

_____ Date _____ Time _____ Print Technician's Name

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink
10/05 ©

GEO-TECH, INC.
ENGINEERING CONSULTANTS IN GEOTECHNICAL • ENVIRONMENTAL • CONSTRUCTION MATERIALS TESTING

August 8, 2007

Hugo Escalante
P. O. Box 280
Fort White, Florida 32038

Reference: Proposed Residence
Parcel 05-7S-17-09898-003
S. W. Old Bellamy Road
Columbia County, Florida

Dear Mr. Escalante,

Geo-Tech, Inc. has completed an investigation and evaluation of a lot identified by the referenced parcel number. The purposes of our work were to evaluate the potential for flooding of a home to be constructed on the lot and to provide recommendations for selecting the finished floor elevation.

Based upon the U.S.G.S. quadrangle map and elevations determined on site, the centerline of Old Bellamy Road adjacent the lot has an elevation of approximately 112.0 feet. Using this elevation as reference, the center of the building site has a ground surface elevation of approximately 107.0 feet. Thus the building site is approximately 5.0 feet lower than the centerline of the adjacent roadway. With placement of the building pad and floor, the finished floor elevation is to be approximately 109.0 feet or about 3 feet lower than the centerline of the adjacent roadway.

Columbia County regulations require the finished floor elevation of a new residence to be at least 12 inches above the elevation of the adjacent roadway unless it can be shown that such an elevation is not required to substantially reduce the likelihood of flooding.

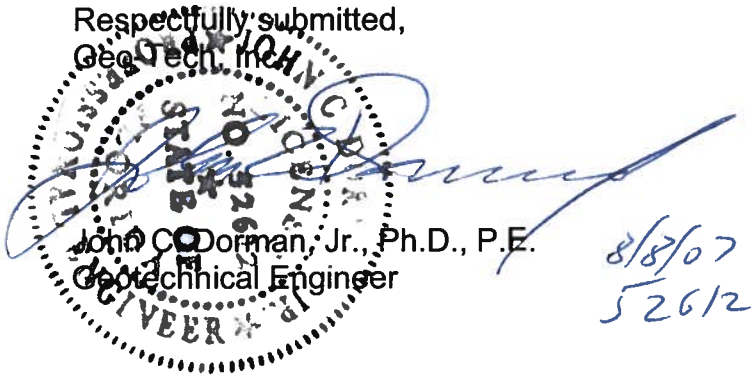
Based upon the FEMA flood map, there are roughly 20 identified flood areas within approximately 2 miles of this home site. Each of these flood areas is a topographically isolated low area, and flood elevations within these areas are estimated to range from about 75 feet to 135 feet. Due to topography, however, flooding within any one of these identified flood areas will not affect this home site. If fact, the area topography and site elevation are such that flooding is not possible short of sea level rising to the elevation of the home site. If this site floods, it will be seawater.

Based upon our evaluation, we believe there is no risk for flooding at the proposed home site, and raising the finished floor elevation to 1 foot above the adjacent roadway will not be required. We recommend us use a finished floor elevation of approximately 109.0 feet or higher but no less than about 1.5 above the existing ground

surface elevation at the center of the building site. Providing this elevation will prevent any local surface water from entering the home.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be of further assistance.

Respectfully submitted,
Geo-Tech, Inc.



John C. Dorman, Jr., Ph.D., P.E.
Geotechnical Engineer

8/8/07
5261 2

FORM 600A-2004 EnergyGauge® 4.0

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**NICOLAS 1580 - Griffin Res**	Builder:	**EWPL Inc.**
Address:	**Scrubtown Rd**	Permitting Office:	
City, State:	**Fort White, FL 32038-**	Permit Number:	
Owner:	**Verlene Griffin**	Jurisdiction Number:	
Climate Zone:	**North**		

1. New construction or existing	New		
2. Single family or multi-family	Single family		
3. Number of units, if multi-family	1		
4. Number of Bedrooms	3		
5. Is this a worst case?	Yes		
6. Conditioned floor area (ft²)	1580 ft²		
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)			
a. U-factor: (or Single or Double DEFAULT)	7a.	Description Area (Dble Default) 158.7 ft²	
b. SHGC: (or Clear or Tint DEFAULT)	7b.	(Clear) 158.7 ft²	
8. Floor types			
a. Slab-On-Grade Edge Insulation	R=0.0, 181.0(p) ft		
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior	R=13.0, 1396.0 ft²		
b. Frame, Wood, Adjacent	R=13.0, 200.0 ft²		
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic	R=30.0, 1580.0 ft²		
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 120.0 ft		
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 30.0 kBtu/hr	
	SEER: 13.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 30.0 kBtu/hr	
	HSPF: 7.20	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 40.0 gallons	
	EF: 0.92	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	CF,	

Glass/Floor Area: 0.12

Total as-built points: 22438
Total base points: 25790

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 8-1-07

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA
IN GOD WE TRUST

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Scrubtown Rd, Fort White, FL, 32038-** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	1580.0	20.04	5699.4	Double, Clear	W	1.5	8.0	36.0	38.52	0.96	1328.7
				Double, Clear	W	9.0	10.0	13.3	38.52	0.55	284.7
				Double, Clear	W	9.0	10.0	6.0	38.52	0.55	128.1
				Double, Clear	W	1.5	6.0	17.5	38.52	0.91	615.8
				Double, Clear	N	1.5	6.0	30.0	19.20	0.94	540.7
				Double, Clear	E	1.5	6.0	17.5	42.06	0.91	671.9
				Double, Clear	E	1.5	7.5	20.0	42.06	0.95	798.0
				Double, Clear	E	1.5	6.0	30.0	42.06	0.91	1151.8
				Double, Clear	S	1.0	7.0	20.0	35.87	0.97	693.7
				As-Built Total:				**190.3**			**6213.3**

WALL TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Adjacent	200.0	0.70	140.0	Frame, Wood, Exterior	13.0	1396.0	1.50	2094.0
Exterior	1396.0	1.70	2373.2	Frame, Wood, Adjacent	13.0	200.0	0.60	120.0
Base Total:	**1596.0**		**2513.2**	**As-Built Total:**		**1596.0**		**2214.0**

DOOR TYPES	Area X	BSPM	= Points	Type	Area X	SPM	= Points
Adjacent	18.0	2.40	43.2	Exterior Wood	20.0	6.10	122.0
Exterior	60.0	6.10	366.0	Adjacent Wood	18.0	2.40	43.2
				Exterior Wood	40.0	6.10	244.0
Base Total:	**78.0**		**409.2**	**As-Built Total:**	**78.0**		**409.2**

CEILING TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM X SCM =	Points
Under Attic	1580.0	1.73	2733.4	Under Attic	30.0	1580.0	1.73 X 1.00	2733.4
Base Total:	**1580.0**		**2733.4**	**As-Built Total:**		**1580.0**		**2733.4**

FLOOR TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Slab	181.0(p)	-37.0	-6697.0	Slab-On-Grade Edge Insulation	0.0	181.0(p	-41.20	-7457.2
Raised	0.0	0.00	0.0					
Base Total:			**-6697.0**	**As-Built Total:**		**181.0**		**-7457.2**

INFILTRATION	Area X	BSPM	= Points		Area X	SPM	= Points
	1580.0	10.21	16131.8		1580.0	10.21	16131.8

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Scrubtown Rd, Fort White, FL, 32038-** PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 20790.0			**Summer As-Built Points:**					**20244.5**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 30000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)					
20790.0	0.4266	8869.0	20245 **20244.5**	1.00 **1.00**	(1.09 x 1.147 x 0.91) **1.138**	0.263 **0.263**	0.950 **0.950**	5744.5 **5744.5**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Scrubtown Rd, Fort White, FL, 32038-** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points
.18	1580.0	12.74	3623.3	Double, Clear	W	1.5	8.0	36.0	20.73	1.01	754.5
				Double, Clear	W	9.0	10.0	13.3	20.73	1.16	319.5
				Double, Clear	W	9.0	10.0	6.0	20.73	1.16	143.8
				Double, Clear	W	1.5	6.0	17.5	20.73	1.02	371.3
				Double, Clear	N	1.5	6.0	30.0	24.58	1.00	739.1
				Double, Clear	E	1.5	6.0	17.5	18.79	1.04	340.5
				Double, Clear	E	1.5	7.5	20.0	18.79	1.02	384.5
				Double, Clear	E	1.5	6.0	30.0	18.79	1.04	583.8
				Double, Clear	S	1.0	7.0	20.0	13.30	1.01	268.1
				As-Built Total:				**190.3**			**3905.0**
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Adjacent	200.0	3.60	720.0	Frame, Wood, Exterior			13.0	1396.0	3.40		4746.4
Exterior	1396.0	3.70	5165.2	Frame, Wood, Adjacent			13.0	200.0	3.30		660.0
Base Total:	**1596.0**		**5885.2**	**As-Built Total:**				**1596.0**			**5406.4**
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points
Adjacent	18.0	11.50	207.0	Exterior Wood				20.0	12.30		246.0
Exterior	60.0	12.30	738.0	Adjacent Wood				18.0	11.50		207.0
				Exterior Wood				40.0	12.30		492.0
Base Total:	**78.0**		**945.0**	**As-Built Total:**				**78.0**			**945.0**
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X WCM =		Points
Under Attic	1580.0	2.05	3239.0	Under Attic			30.0	1580.0	2.05 X 1.00		3239.0
Base Total:	**1580.0**		**3239.0**	**As-Built Total:**				**1580.0**			**3239.0**
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Slab	181.0(p)	8.9	1610.9	Slab-On-Grade Edge Insulation			0.0	181.0(p	18.80		3402.8
Raised	0.0	0.00	0.0								
Base Total:			**1610.9**	**As-Built Total:**				**181.0**			**3402.8**
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points
	1580.0	-0.59	-932.2					1580.0	-0.59		-932.2

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Scrubtown Rd, Fort White, FL, 32038-** PERMIT #:

BASE			AS-BUILT						
Winter Base Points:		**14371.2**	**Winter As-Built Points:**						**15966.0**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
			(sys 1: Electric Heat Pump 30000 btuh ,EFF(7.2) Ducts:Unc(S),Unc(R),Int(AH),R6.0						
14371.2	0.6274	9016.5	15966.0	1.00	1.162 (1.069 x 1.169 x 0.93)	0.474	1.000	8788.1	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Scrubtown Rd, Fort White, FL, 32038-** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2635.00	7905.0	40.0	0.92	3	1.00	2635.00	1.00	7905.0	
				As-Built Total:						**7905.0**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
8869	**9016**	**7905**	**25790**	**5745**	**8788**	**7905**	**22438**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Scrubtown Rd, Fort White, FL, 32038-** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.4

The higher the score, the more efficient the home.

Verlene Griffin, Scrubtown Rd, Fort White, FL, 32038-

1.	New construction or existing	New		12.	Cooling systems	
2.	Single family or multi-family	Single family			a. Central Unit	Cap: 30.0 kBtu/hr
3.	Number of units, if multi-family	1				SEER: 13.00
4.	Number of Bedrooms	3			b. N/A	
5.	Is this a worst case?	Yes				
6.	Conditioned floor area (ft²)	1580 ft²			c. N/A	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)					
	a. U-factor: (or Single or Double DEFAULT)	Description Area 7a. (Dble Default) 158.7 ft²		13.	Heating systems	
					a. Electric Heat Pump	Cap: 30.0 kBtu/hr
	b. SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 158.7 ft²				HSPF: 7.20
8.	Floor types				b. N/A	
	a. Slab-On-Grade Edge Insulation	R=0.0, 181.0(p) ft			c. N/A	
	b. N/A					
	c. N/A			14.	Hot water systems	
9.	Wall types				a. Electric Resistance	Cap: 40.0 gallons
	a. Frame, Wood, Exterior	R=13.0, 1396.0 ft²				EF: 0.92
	b. Frame, Wood, Adjacent	R=13.0, 200.0 ft²			b. N/A	
	c. N/A					
	d. N/A				c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)	
	e. N/A					
10.	Ceiling types			15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	CF,
	a. Under Attic	R=30.0, 1580.0 ft²				
	b. N/A					
	c. N/A					
11.	Ducts					
	a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 120.0 ft				
	b. N/A					

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

26241

THIS INSTRUMENT PREPARED BY
AND RETURN TO:
TITLE OFFICES, LLC
343 NW COLE TERRACE
SUITE 101
LAKE CITY, FLORIDA 32055

Parcel I.D. #: 09898-003

Inst. 200812014656 Date 8/8/2008 Time 2:10 PM
DC,P DeWitt Cason,Columbia County Page 1 of 2 B:1156 P:135

— SPACE ABOVE THIS LINE FOR PROCESSING DATA — *— SPACE ABOVE THIS LINE FOR RECORDING DATA —*

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement. This Notice shall be void and of no force and effect if construction is not commenced within ninety (90) days after recordation.

1. Description of property: (Legal description of property, and street address if available)

TBD SCRUBTOWN ROAD, FORT WHITE, FLORIDA 32038
A PART OF THE NE ¼ OF SECTION 5, TOWNSHIP 7 SOUTH, RANGE 17 EAST, MORE PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCE AT THE SE CORNER OF SAID NE ¼ AND RUN S 88°16'07" W, ALONG THE SOUTH LINE THEREOF 852.25 FEET FOR A POINT OF BEGINNING; THENCE CONTINUE S 88°16'07" W, 431.63 FEET TO THE EAST RIGHT-OF-WAY LINE OF SCRUB TOWN ROAD; THENCE N 06°33'24" W, ALONG SAID EAST RIGHT-OF-WAY LINE 207.39 FEET; THENCE CONTINUE ALONG SAID EAST LINE N 00°26'54" W, 671.81 FEET; THENCE CONTINUE ALONG SAID EAST RIGHT-OF-WAY LINE N 37°51'23" E, 54.72 FEET TO THE SOUTH RIGHT-OF-WAY LINE OF BELLAMY ROAD; THENCE S 82°00'16" E, ALONG SAID SOUTH RIGHT-OF-WAY LINE 374.86 FEET; THENCE S 02°22'45" E, 857.19 FEET TO THE POINT OF BEGINNING. COLUMBIA COUNTY, FLORIDA.

LESS AND EXCEPT THE SOUTH 645 FEET AND LESS AND EXCEPT THE EAST 200 FEET THEREOF.

2. General description of improvement: **construction of single family dwelling**

3. Owner information:
   a. Name and address:
      VERLENE Z. GRIFFIN
      P.O. BOX 238, FORT WHITE, FLORIDA 32038
   b. Interest in property: **Fee Simple**
   c. Name and Address of Fee Simple Titleholder (if other than owner):

4. Contractor: (Name and Address)
   EWPI, INC.
   6210 SW CR 18, FORT WHITE, FLORIDA 32038
   Telephone Number: **(386) 288-8666**

5. Surety (if any):
   a. Name and Address:
      Telephone Number: _____
   b. Amount of Bond $ _____

6. Lender: (Name and Address)
   FIRST FEDERAL BANK OF FLORIDA
   4705 WEST U.S. HWY 90, P.O. BOX 2029, LAKE CITY, FL. 32056
   Telephone Number: **755-0600**

7. Persons within the State of Florida designated by Owner upon whom notice or other documents may be served as provided by Section 713.13(1)(a)(7), Florida Statutes: (Name and Address)
   N/A

8. In addition to himself, Owner designates the following person(s) to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes: (Name and Address) **PAULA HACKER**
   FIRST FEDERAL BANK OF FLORIDA
   4705 WEST U.S. HWY 90, P.O. BOX 2029, LAKE CITY, FL 32056
   Telephone Number: **755-0600**

9. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of recording unless a different date is specified) _____

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

Signature of Owner(s) or Owner's Authorized Officer/Director/Partner/Manager:

Verlene Z. Griffin ____(SEAL)
VERLENE Z. GRIFFIN

_____ _____(SEAL)

The foregoing instrument was acknowledged before me this 5th day of August, 2008, by VERLENE Z. GRIFFIN, who is personally known to me or who has produced Driver's License as identification.

Patricia H. Lang
Notary Public
My Commission Expires: 12-14-10

PATRICIA H. LANG
Commission DD 622516
Expires December 14, 2010

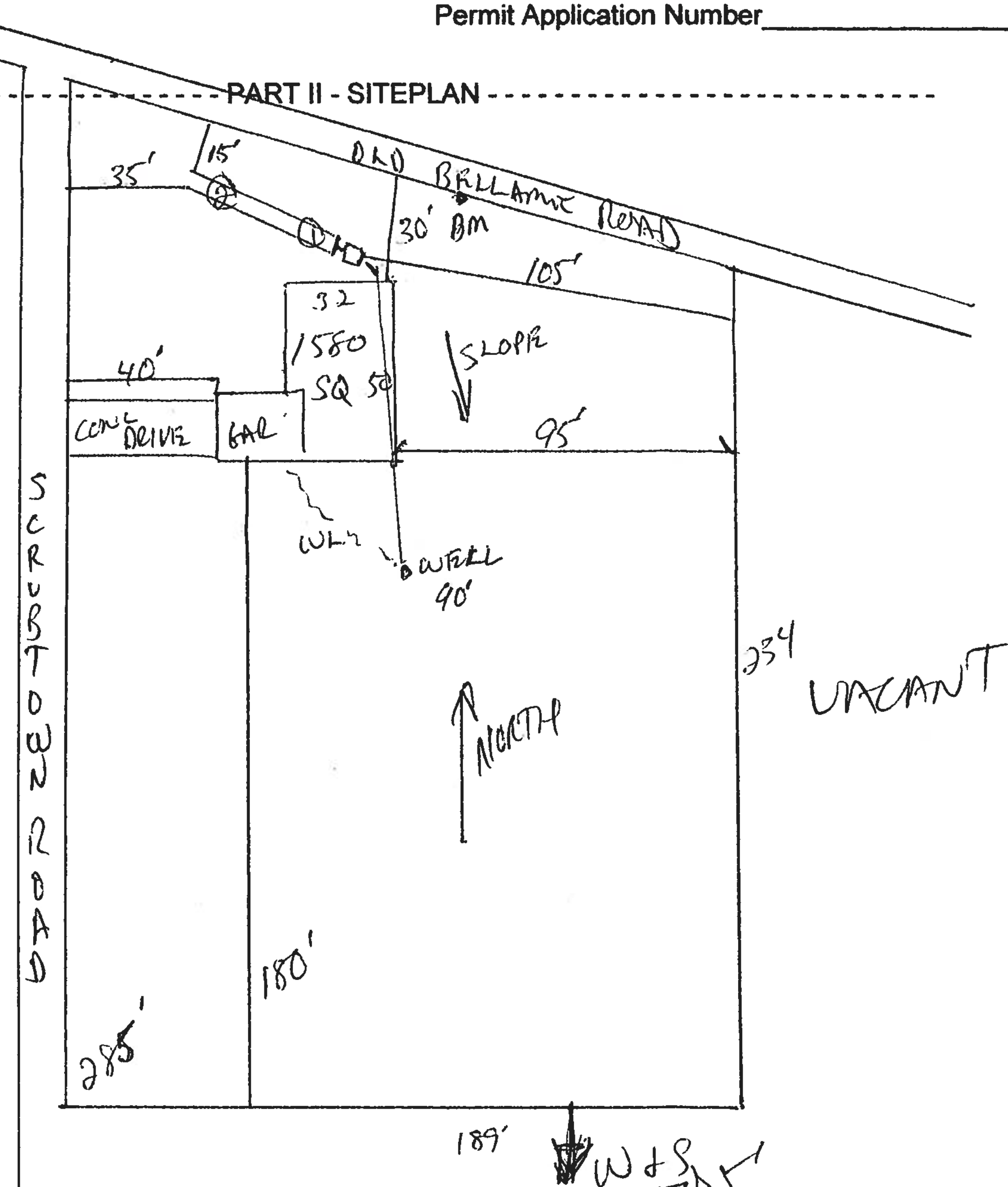
07-638

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number__________________

PART II - SITEPLAN

Scale: 1 inch = 50 feet.



Notes: ____

Site Plan submitted by: Rocky Ford **MASTER CONTRACTOR**

Plan Approved ✓ Not Approved_____ Date 8/13/07

By [signature] Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

CERTIFICATE OF

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 05-7S-17-09898-003 **Building permit No.** 000026241

Use Classification SFD/UTILITY **Fire:** 57.78

Permit Holder HUGO ESCALANTE **Waste:** 150.75

Owner of Building VERLENE GRIFFIN **Total:** 208.53

Location: 141 SW SCRUBTOWN ROAD, FT. WHITE, FL

Date: 01/16/2009

John Kerce

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001727

DATE 04/29/2009 PARCEL ID # 05-7S-17-09898-003

APPLICANT VERLENE GRIFFIN PHONE 386.497.2956

ADDRESS 141 SW SCRUBTOWN ROAD FT. WHITE FL 32038

OWNER VERLENE GRIFFIN PHONE 386.497.2956

ADDRESS 141 SW SCRUBTOWN ROAD FT. WHITE FL 32038

CONTRACTOR _____ PHONE _____

LOCATION OF PROPERTY 441-S TO C-18,TR TO C-131 AVE,TL TO OLD BELLAMY,TL TO SCRUBTOWN,TR

AND IT'S THE CORNER LOT ON L.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT _____

SIGNATURE Verlene Griffin

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

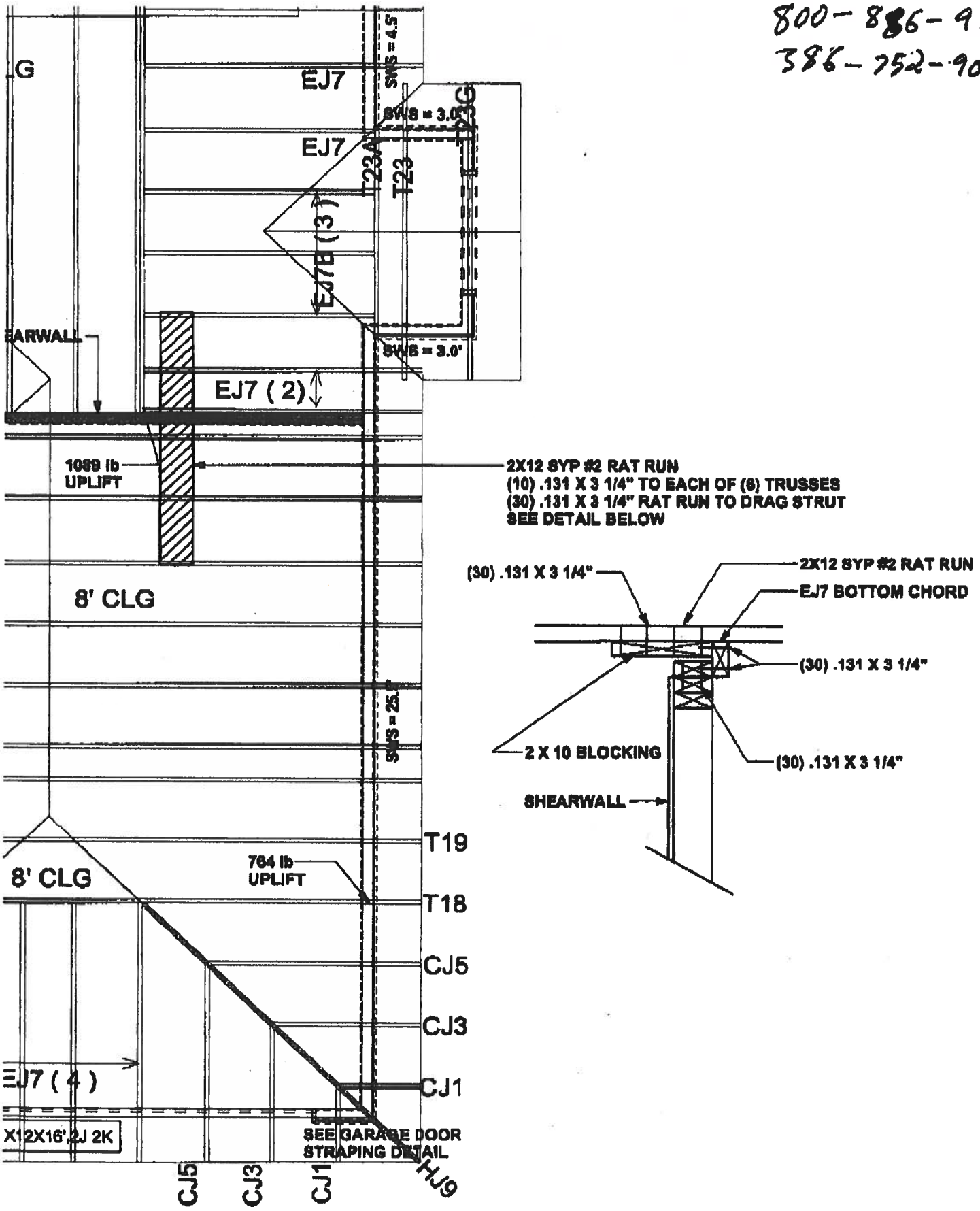
ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY FLORIDA

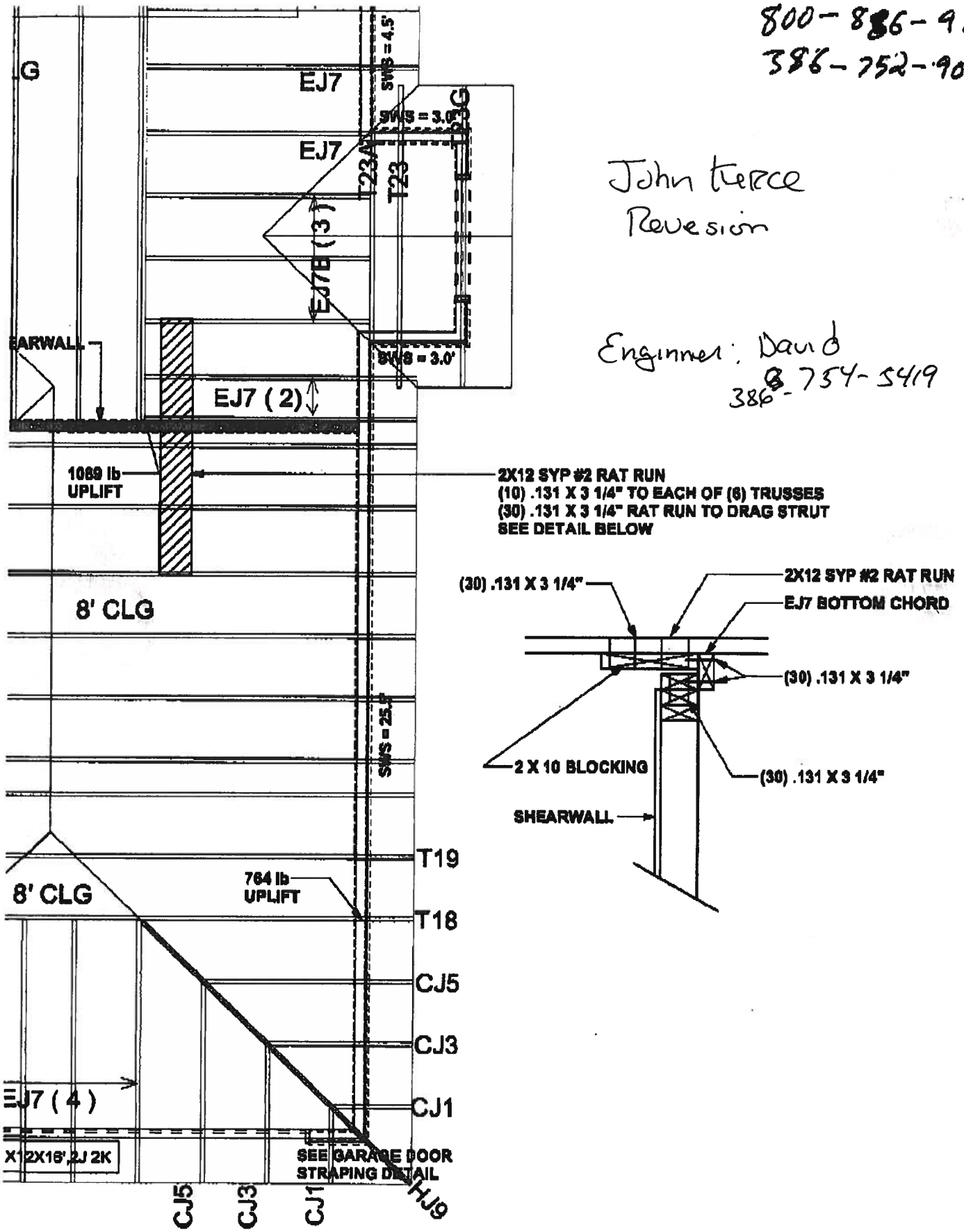
800-886-9563
386-752-9080



800-886-9563
386-752-9080

John Pierce
Revesion

Engineer: David
386-754-5419



Builders FirstSource

JULIUS LEE LICENSE No. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER
August 10, 2007

Project Information for: **L250193**

Builder: EWPL Inc.
Address: Scrubtown Road
.... Ft. White, FL
County: Columbia
Truss Count: 36
Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002

Truss Design Load Information:

Gravity: **Wind:**

Roof (psf): 42.0 Wind Standard: ASCE 7-02 Wind Exposure: B
Floor (psf): N/A Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:
Hugo Escalante Florida License No. CRC1326967
Address: P.O. Box 280 Fort White, Florida 32038

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

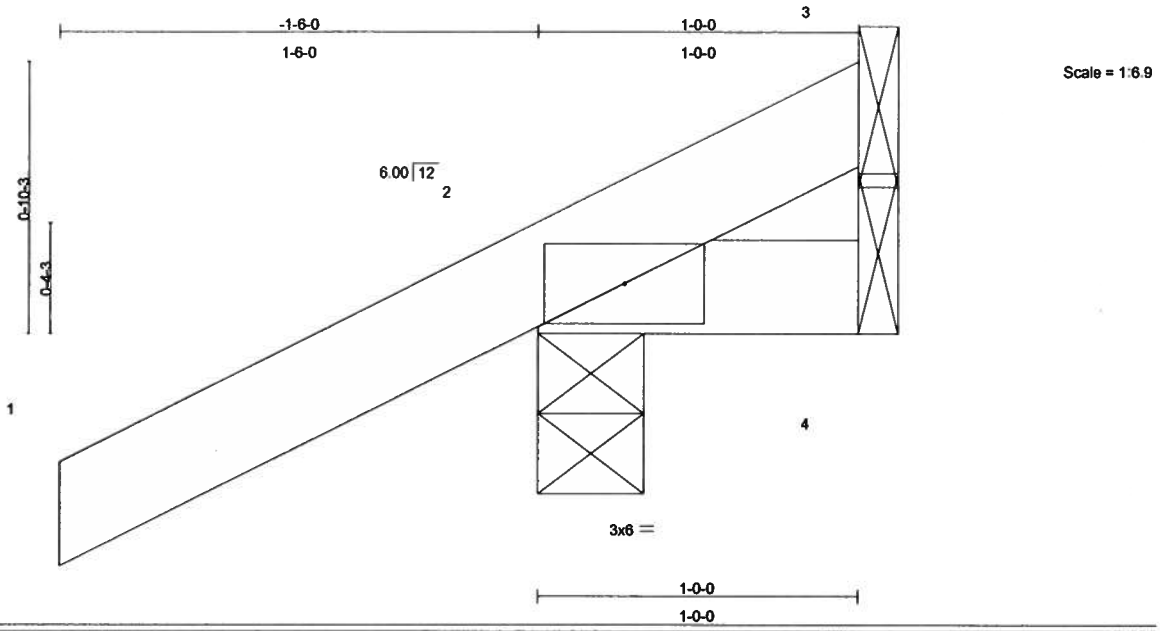
Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date	No.	Drwg. #	Truss ID	Date
1	J1876043	CJ1	8/10/07	29	J1876071	T21	8/10/07
2	J1876044	CJ3	8/10/07	30	J1876072	T22	8/10/07
3	J1876045	CJ5	8/10/07	31	J1876073	T22A	8/10/07
4	J1876046	EJ7	8/10/07	32	J1876074	T24	8/10/07
5	J1876047	EJ7A	8/10/07	33	J1876075	T24G	8/10/07
6	J1876048	EJ7B	8/10/07	34	J1876076	T25	8/10/07
7	J1876049	EJ7G	8/10/07	35	J1876077	T25A	8/10/07
8	J1876050	HJ9	8/10/07	36	J1876078	T25G	8/10/07
9	J1876051	T01	8/10/07				
10	J1876052	T02	8/10/07				
11	J1876053	T03	8/10/07				
12	J1876054	T04	8/10/07				
13	J1876055	T05	8/10/07				
14	J1876056	T06	8/10/07				
15	J1876057	T07	8/10/07				
16	J1876058	T08	8/10/07				
17	J1876059	T09	8/10/07				
18	J1876060	T10	8/10/07				
19	J1876061	T11	8/10/07				
20	J1876062	T12	8/10/07				
21	J1876063	T13	8/10/07				
22	J1876064	T14	8/10/07				
23	J1876065	T15	8/10/07				
24	J1876066	T16	8/10/07				
25	J1876067	T17	8/10/07				
26	J1876068	T18	8/10/07				
27	J1876069	T19	8/10/07				
28	J1876070	T20	8/10/07				

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	CJ1	JACK	6	1	J1876043
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 08 13:38:59 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=180/0-4-0, 4=5/Mechanical, 3=-41/Mechanical
Max Horz 2=70(load case 6)
Max Uplift 2=-181(load case 6), 3=-41(load case 1)
Max Grav 2=180(load case 1), 4=14(load case 2), 3=61(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/35
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 2 and 41 lb uplift at joint 3.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

August 10,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876043
L250193	CJ1	JACK	6	1	Job Reference (optional)

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Coastal Bay Blvd
Boynton Beach, FL 33436

August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	CJ3	JACK	6	1	J1876044
					Job Reference (optional)

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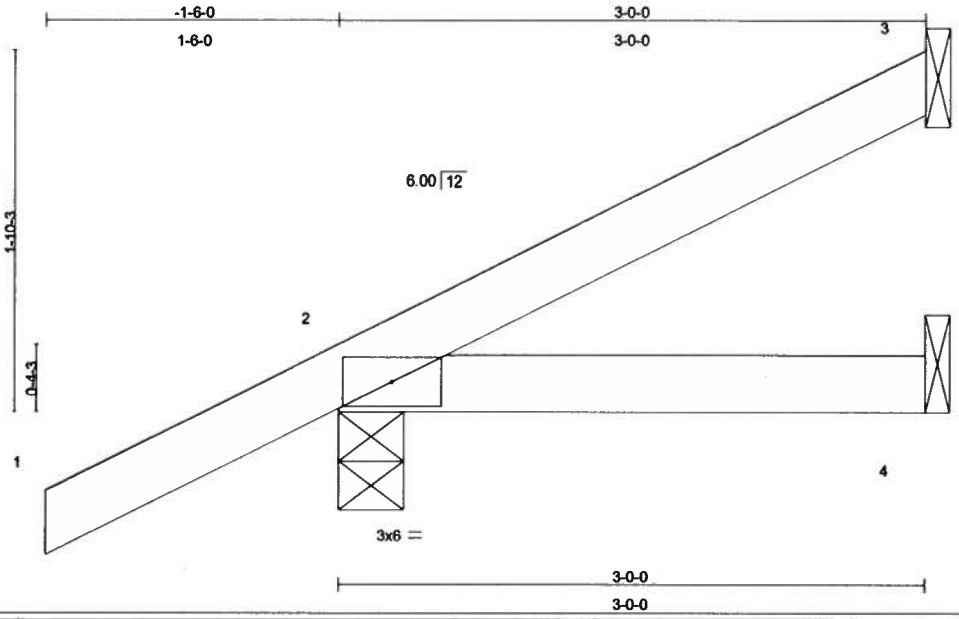


Scale = 1:11.4

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=48/Mechanical, 2=206/0-4-0, 4=14/Mechanical
Max Horz 2=115(load case 6)
Max Uplift 3=-37(load case 6), 2=-153(load case 6)
Max Grav 3=48(load case 1), 2=206(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-49/16
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 3 and 153 lb uplift at joint 2.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1102 Crestal Bay Blvd
Boynton Beach, FL 33435

August 10,2007

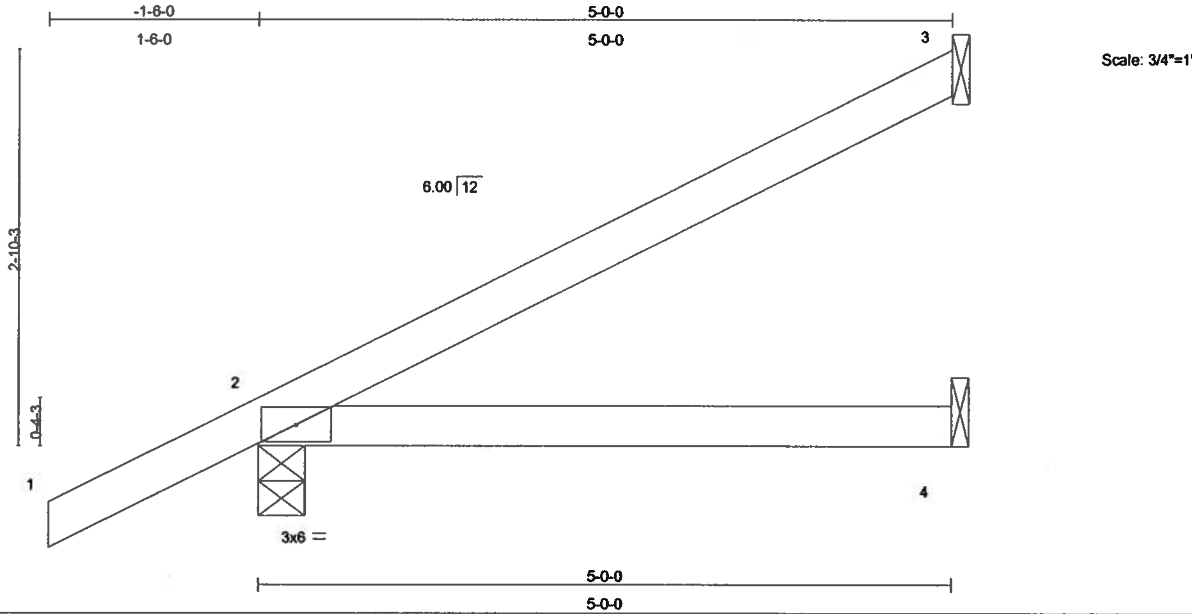
Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876044
L250193	CJ3	JACK	6	1	Job Reference (optional)

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1105 Coastal Bay Blvd
Boynton Beach, FL 33435

August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	CJ5	JACK	6	1	J1876045 Job Reference (optional)



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=113/Mechanical, 2=258/0-4-0, 4=24/Mechanical
Max Horz 2=162(load case 6)
Max Uplift 3=-101(load case 6), 2=-159(load case 6)
Max Grav 3=113(load case 1), 2=258(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-96/41
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.12

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 3 and 159 lb uplift at joint 2.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. [illegible]
1109 Coastal Bay Blvd
Boynton Beach, FL [illegible]

August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876045
L250193	CJ5	JACK	6	1	Job Reference (optional)

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No 34869
1106 Coastal Bay Drive
Boynton Beach, FL 33436

August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	EJ7	MONO TRUSS	23	1	J1876046
					Job Reference (optional)

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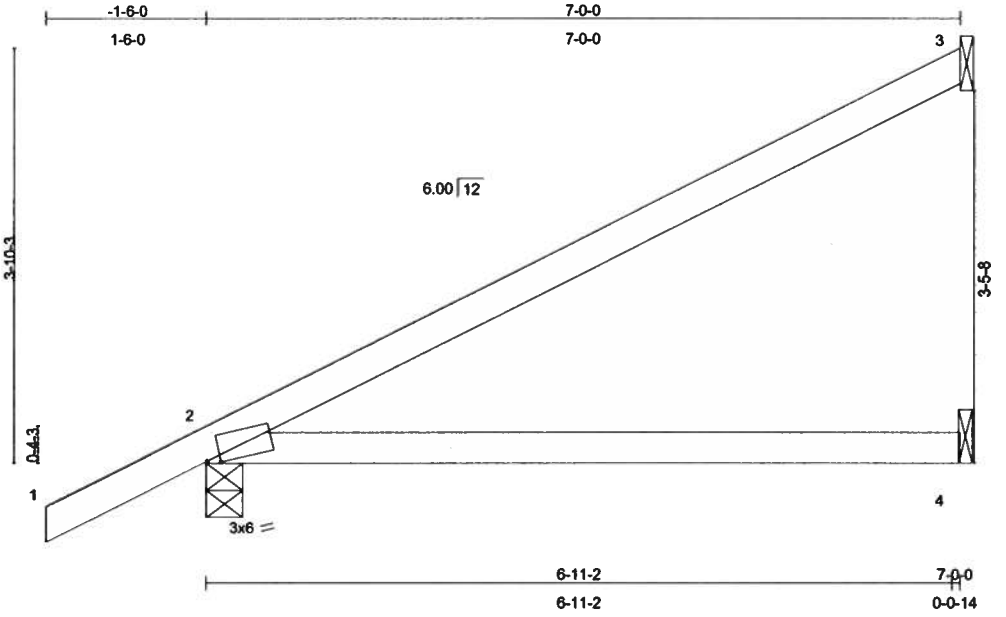


Plate Offsets (X,Y): [2:0-1-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.11	2-4	>756	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.17	2-4	>488	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=157/Mechanical, 2=318/0-4-0, 4=48/Mechanical
Max Horz 2=149(load case 6)
Max Uplift 3=-88(load case 6), 2=-112(load case 6)
Max Grav 3=157(load case 1), 2=318(load case 1), 4=94(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-122/56
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.88

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 88 lb uplift at joint 3 and 112 lb uplift at joint 2.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1466 Coastal Bay Blvd
Boynton Beach, FL 33435

August 10,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	EJ7	MONO TRUSS	23	1	J1876046 Job Reference (optional)

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1905 Crestal Bay Blvd.
Boynton Beach, FL 33436

August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	EJ7A	SPECIAL	6	1	J1876047
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 08 13:39:01 2007 Page 1

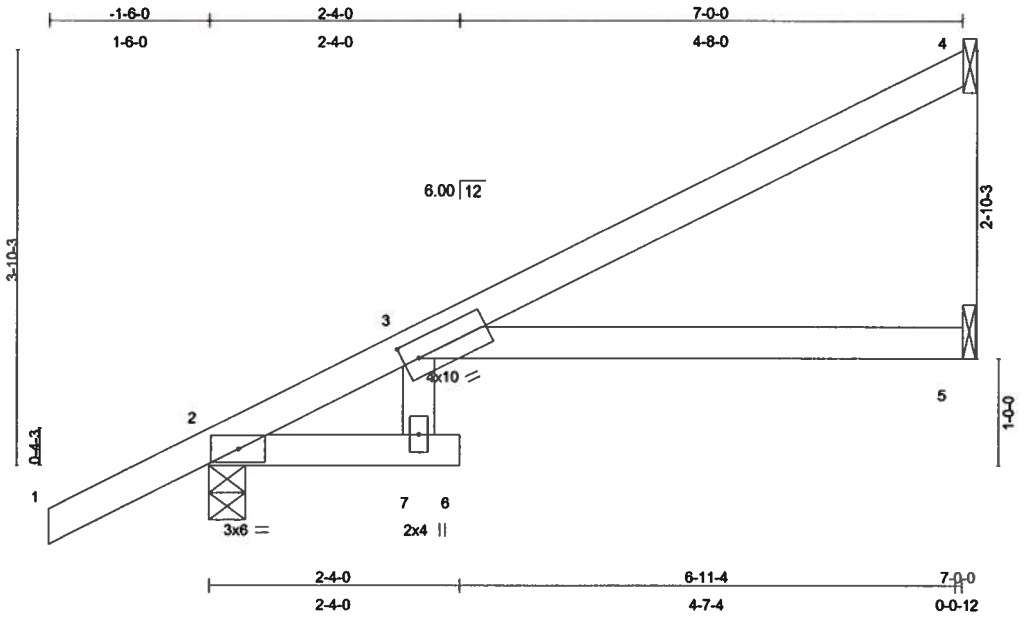


Plate Offsets (X,Y): [3:0-1-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.20	6	>397	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.20	3-5	>400	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.02	Horz(TL)	-0.10	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=142/Mechanical, 2=321/0-4-0, 5=64/Mechanical
Max Horz 2=149(load case 6)
Max Uplift 4=-74(load case 6), 2=-110(load case 6), 5=-4(load case 6)
Max Grav 4=142(load case 1), 2=321(load case 1), 5=92(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-129/0, 3-4=-97/52
BOT CHORD 2-7=-2/0, 6-7=0/0, 3-5=0/0
WEBS 3-7=0/61

JOINT STRESS INDEX
2 = 0.31, 3 = 0.91 and 7 = 0.04

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33436

August 10,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	EJ7A	SPECIAL	6	1	J1876047 Job Reference (optional)

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 74 lb uplift at joint 4, 110 lb uplift at joint 2 and 4 lb uplift at joint 5.

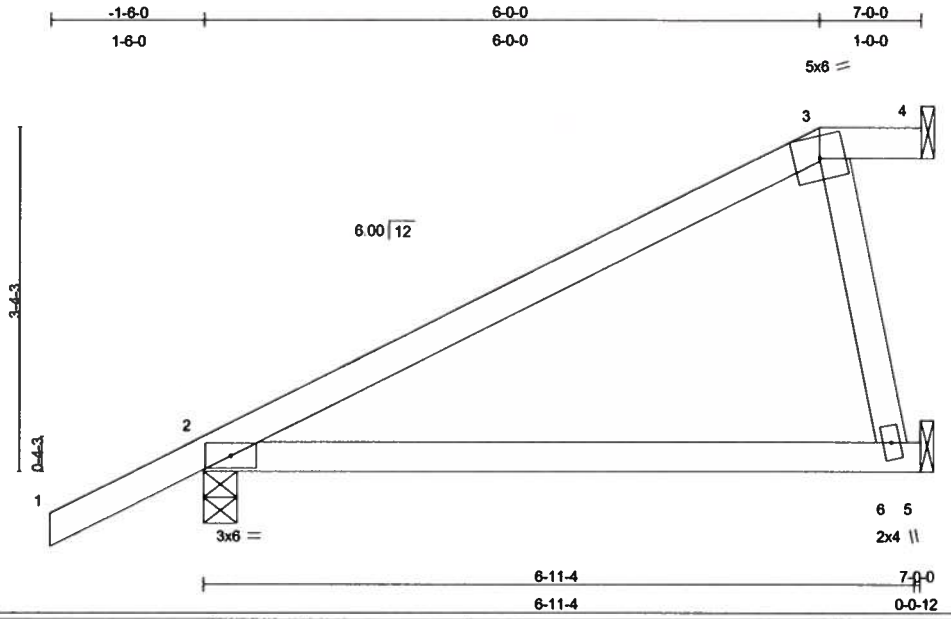
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33436

August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	EJ7B	MONO HIP	2	1	J1876048
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 08 13:39:01 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.07	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.12	2-6	>644	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.10	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=-145/Mechanical, 2=307/0-4-0, 6=362/Mechanical
Max Horz 2=134(load case 6)
Max Uplift 4=-145(load case 1), 2=-114(load case 6), 6=-138(load case 6)
Max Grav 4=74(load case 6), 2=307(load case 1), 6=362(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-156/8, 3-4=-2/2
BOT CHORD 2-6=-97/79, 5-6=0/0
WEBS 3-6=-334/412

JOINT STRESS INDEX
2 = 0.53, 3 = 0.63 and 6 = 0.22

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	EJ7B	MONO HIP	2	1	J1876048 Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 145 lb uplift at joint 4, 114 lb uplift at joint 2 and 138 lb uplift at joint 6.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE J1876049
L250193	EJ7G	GABLE	2	1	Job Reference (optional)

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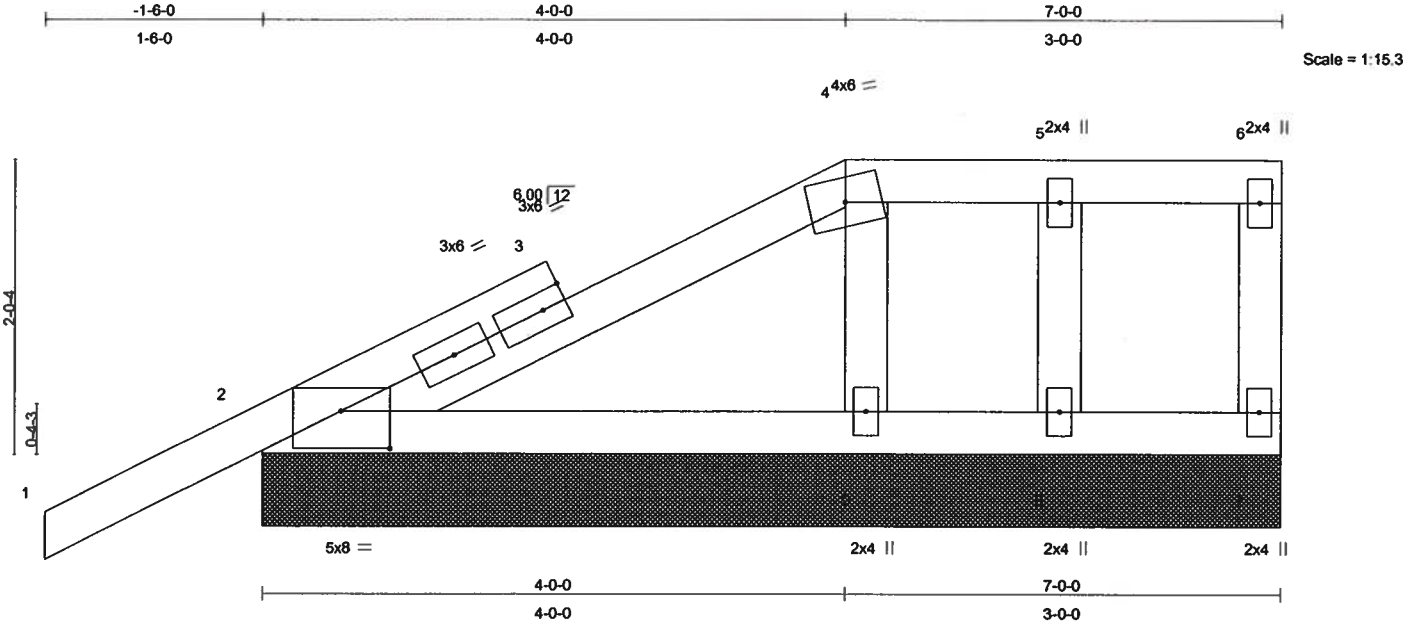

Plate Offsets (X,Y): [2:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.00	1	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.07	Horz(TL)	-0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=334/7-0-0, 7=66/7-0-0, 9=328/7-0-0, 8=67/7-0-0
Max Horz 2=126(load case 6)
Max Uplift 2=-181(load case 6), 7=-25(load case 5), 9=-125(load case 6), 8=-71(load case 4)
Max Grav 2=334(load case 1), 7=66(load case 1), 9=328(load case 1), 8=104(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-12/56, 2-3=-87/0, 3-4=-53/50, 4-5=-1/0, 5-6=0/0
BOT CHORD 2-9=-24/23, 8-9=0/0, 7-8=0/0
WEBS 4-9=-276/279, 6-7=-57/57, 5-8=-106/75

JOINT STRESS INDEX
2 = 0.83, 3 = 0.00, 3 = 0.32, 3 = 0.33, 4 = 0.40, 5 = 0.04, 6 = 0.03, 7 = 0.03, 8 = 0.04 and 9 = 0.16

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

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August 10,2007

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	EJ7G	GABLE	2	1	J1876049
					Job Reference (optional)

NOTES

6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 2, 25 lb uplift at joint 7, 125 lb uplift at joint 9 and 71 lb uplift at joint 8.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-87(F=-33), 4-6=-87(F=-33), 2-7=-10

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	HJ9	MONO TRUSS	3	1	J1876050
					Job Reference (optional)

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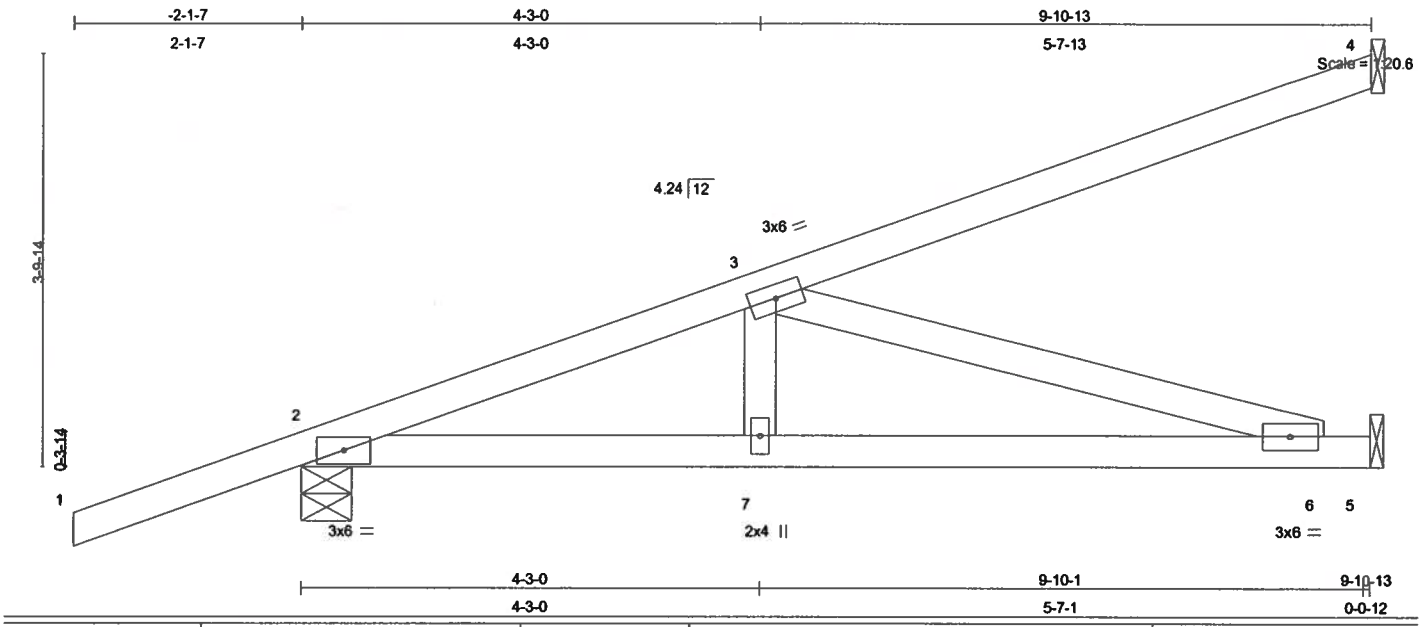

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	0.05	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.12	6-7	>989	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.37	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-7 oc bracing.

REACTIONS (lb/size) 4=266/Mechanical, 2=406/0-5-10, 5=231/Mechanical
Max Horz 2=253(load case 3)
Max Uplift 4=-230(load case 3), 2=-228(load case 3), 5=-77(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-699/180, 3-4=-104/64
BOT CHORD 2-7=-370/652, 6-7=-370/652, 5-6=0/0
WEBS 3-7=0/195, 3-6=-680/386

JOINT STRESS INDEX
2 = 0.65, 3 = 0.17, 6 = 0.19 and 7 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 4, 228 lb uplift at joint 2 and 77 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	HJ9	MONO TRUSS	3	1	J1876050 Job Reference (optional)

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-5=-25(F=-7, B=-7)

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T01	HIP	1	1	J1876051
					Job Reference (optional)

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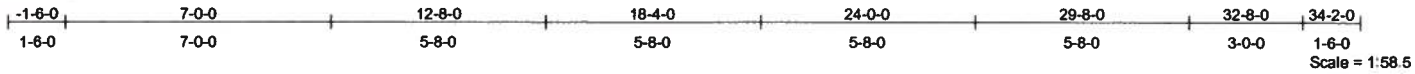

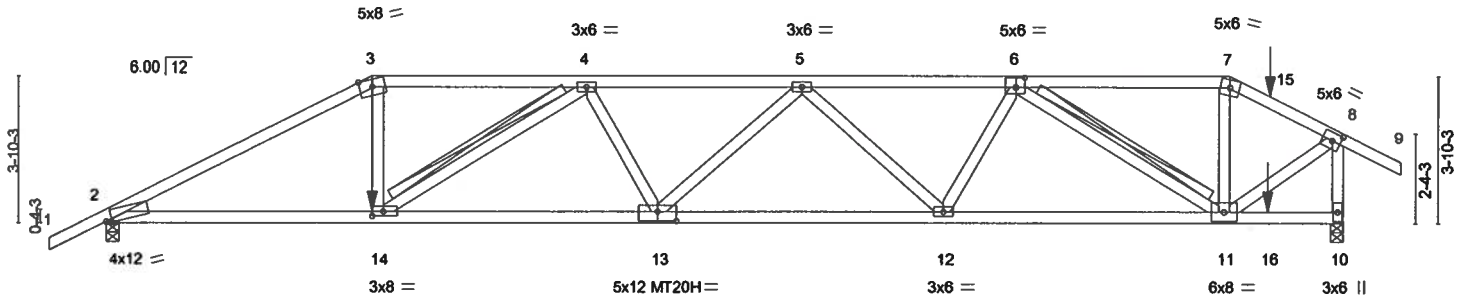

Plate Offsets (X,Y): [2:0-1-13,Edge], [6:0-2-12,0-3-0], [8:0-2-11,0-2-8], [13:0-6-0,0-3-0], [14:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.36	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.72	12-13	>537	240	MT20H	187/143
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.77	Horz(TL)	0.20	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 166 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-3-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-11-13 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-14, 6-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=2213/0-4-0, 10=2572/0-4-0
Max Horz 2=85(load case 4)
Max Uplift 2=-674(load case 4), 10=-790(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-4284/1396, 3-4=-3794/1290, 4-5=-5244/1763, 5-6=-4616/1538, 6-7=-1941/664, 7-15=-2075/704, 8-15=-2171/709, 8-9=0/40, 8-10=-2426/738
BOT CHORD 2-14=-1264/3736, 13-14=-1772/5138, 12-13=-1820/5279, 11-12=-1438/4203, 11-16=-15/41, 10-16=-15/41
WEBS 3-14=-432/1441, 4-14=-1697/641, 4-13=0/294, 5-13=-78/114, 5-12=-915/401, 6-12=-193/884, 6-11=-2699/975, 7-11=-95/492, 8-11=-732/2306

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JOINT STRESS INDEX
2 = 0.82, 3 = 0.80, 4 = 0.50, 5 = 0.36, 6 = 0.81, 7 = 0.72, 8 = 0.76, 10 = 0.43, 11 = 0.91, 12 = 0.71, 13 = 0.86 and 14 = 0.93

NOTES
1) Unbalanced roof live loads have been considered for this design.

August 10,2007

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T01	HIP	1	1	J1876051
					Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 674 lb uplift at joint 2 and 790 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-7=-117(F=-63), 7-8=-54, 8-9=-54, 2-14=-10, 11-14=-22(F=-12), 10-11=-10
   Concentrated Loads (lb)
   Vert: 14=-411(F) 15=-74(F) 16=-362(F)

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T02	HIP	1	1	J1876052
					Job Reference (optional)

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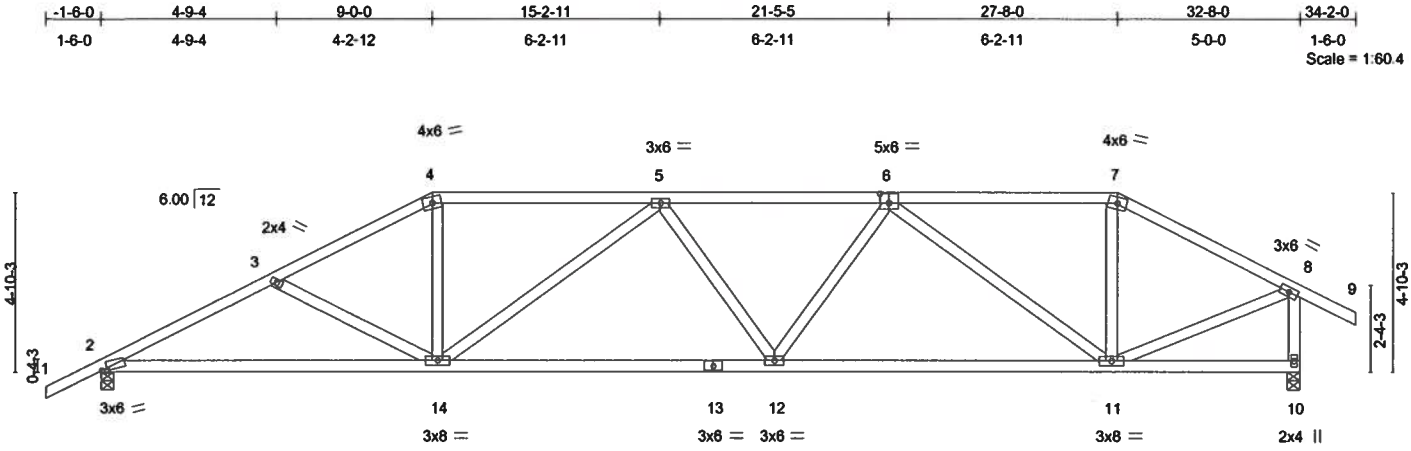

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.15	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.31	11-12	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.08	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 173 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=1125/0-4-0, 10=1124/0-4-0
Max Horz 2=98(load case 5)
Max Uplift 2=-255(load case 6), 10=-228(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1936/1033, 3-4=-1709/928, 4-5=-1502/889, 5-6=-1789/1025, 6-7=-984/629, 7-8=-1148/642, 8-9=0/40, 8-10=-1105/677
BOT CHORD 2-14=-801/1664, 13-14=-819/1845, 12-13=-819/1845, 11-12=-731/1669, 10-11=-1/43
WEBS 3-14=-200/211, 4-14=-188/454, 5-14=-517/249, 5-12=-112/112, 6-12=-44/280, 6-11=-891/465, 7-11=-62/268, 8-11=-435/1041

JOINT STRESS INDEX
2 = 0.86, 3 = 0.33, 4 = 0.62, 5 = 0.40, 6 = 0.44, 7 = 0.58, 8 = 0.58, 10 = 0.39, 11 = 0.94, 12 = 0.40, 13 = 0.59 and 14 = 0.56

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T02	HIP	1	1	J1876052 Job Reference (optional)

NOTES

3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 255 lb uplift at joint 2 and 228 lb uplift at joint 10.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE J1876053
L250193	T03	HIP	1	1	Job Reference (optional)

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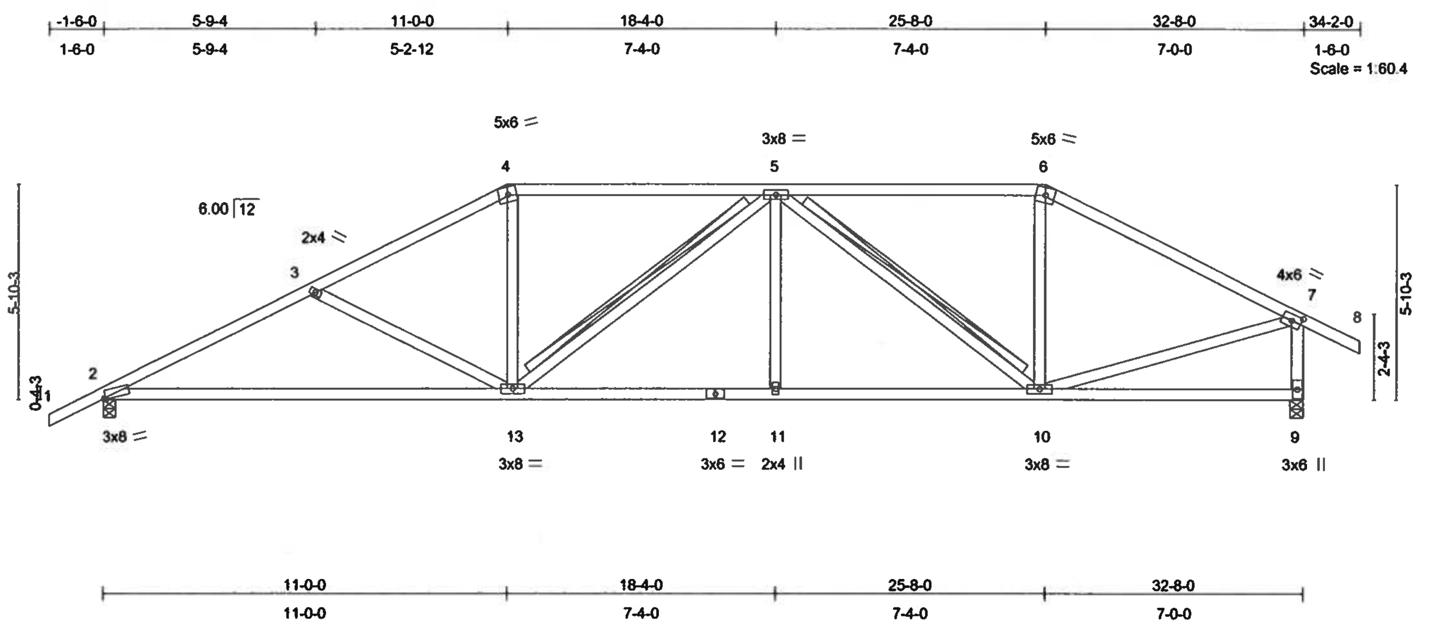

Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-2-15,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.30	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.57	2-13	>677	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.07	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 177 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-0-9 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-13, 5-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1125/0-4-0, 9=1124/0-4-0
Max Horz 2=111(load case 5)
Max Uplift 2=-269(load case 6), 9=-231(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1905/1042, 3-4=-1600/895, 4-5=-1386/867, 5-6=-1054/706, 6-7=-1254/704, 7-8=0/40, 7-9=-1087/693
BOT CHORD 2-13=-799/1635, 12-13=-624/1521, 11-12=-624/1521, 10-11=-624/1521, 9-10=0/80
WEBS 3-13=-294/288, 4-13=-131/407, 5-13=-300/135, 5-11=0/184, 5-10=-658/301, 6-10=-18/267, 7-10=-404/1013

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JOINT STRESS INDEX
2 = 0.87, 3 = 0.33, 4 = 0.57, 5 = 0.56, 6 = 0.65, 7 = 0.72, 9 = 0.30, 10 = 0.90, 11 = 0.33, 12 = 0.50 and 13 = 0.56

Continued on page 2

August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T03	HIP	1	1	J1876053
					Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 269 lb uplift at joint 2 and 231 lb uplift at joint 9.

LOAD CASE(S) Standard

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T04	HIP	1	1	J1876054
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 08 13:39:06 2007 Page 1

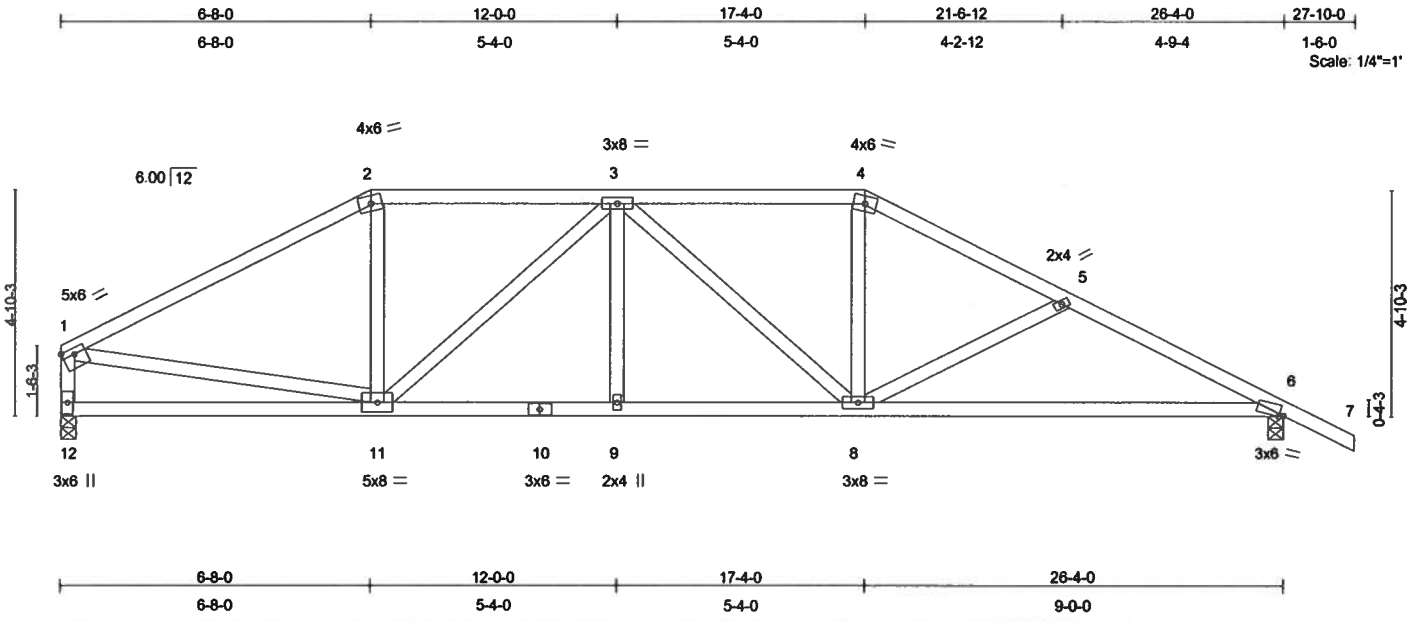

Plate Offsets (X,Y): [1:0-3-0,0-1-8], [6:0-1-1,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.14	6-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.27	6-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.35	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 141 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-2-11 oc bracing.

REACTIONS (lb/size) 12=830/0-4-0, 6=926/0-4-0
Max Horz 12=-116(load case 7)
Max Uplift 12=-141(load case 5), 6=-231(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1126/606, 2-3=-940/613, 3-4=-1099/684, 4-5=-1268/703, 5-6=-1511/816, 6-7=0/35, 1-12=-790/465
BOT CHORD 11-12=-79/193, 10-11=-452/1199, 9-10=-452/1199, 8-9=-452/1199, 6-8=-589/1290
WEBS 2-11=-16/241, 3-11=-424/172, 3-9=0/114, 3-8=-233/103, 4-8=-101/323, 5-8=-231/223, 1-11=-302/755

JOINT STRESS INDEX
1 = 0.74, 2 = 0.78, 3 = 0.56, 4 = 0.53, 5 = 0.33, 6 = 0.87, 8 = 0.56, 9 = 0.33, 10 = 0.42, 11 = 0.34 and 12 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

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August 10,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876054
L250193	T04	HIP	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 12 and 231 lb uplift at joint 6.

LOAD CASE(S) Standard

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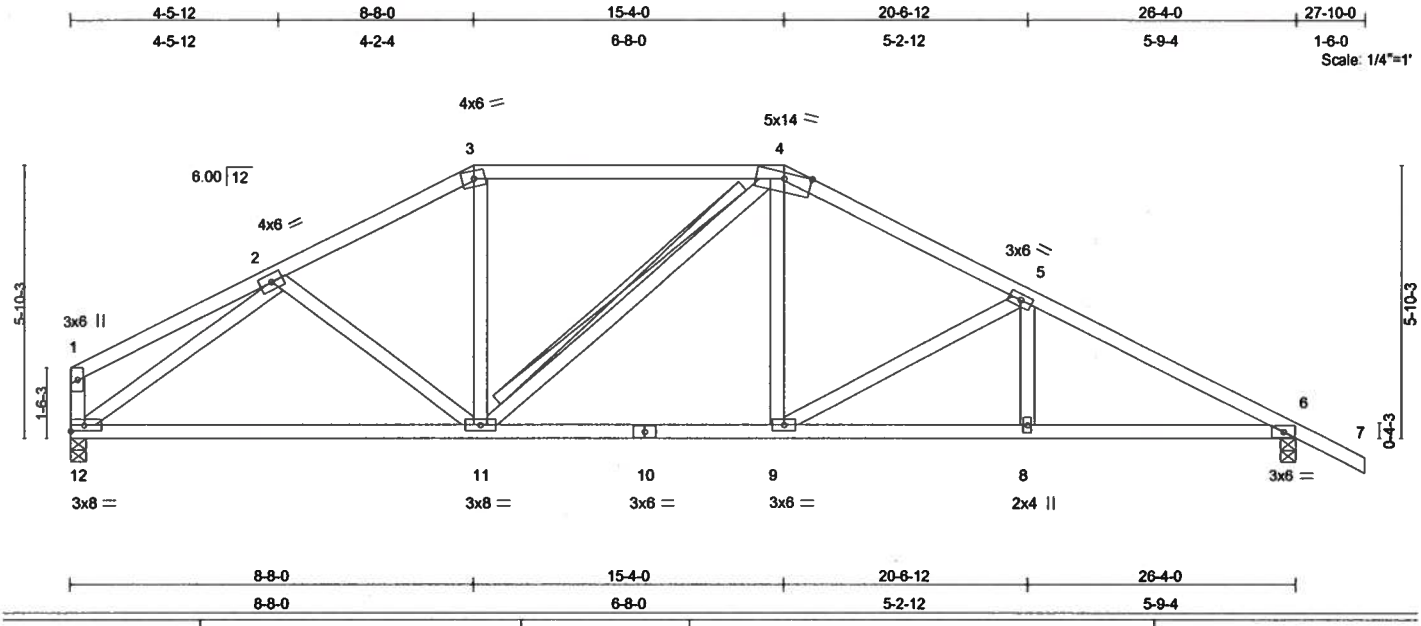
August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T05	HIP	1	1	J1876055
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 08 13:39:07 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.10	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.18	11-12	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 142 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-5-7 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 12=830/0-4-0, 6=926/0-4-0
Max Horz 12=-127(load case 7)
Max Uplift 12=-154(load case 6), 6=-243(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-220/112, 2-3=-1039/627, 3-4=-896/615, 4-5=-1145/678, 5-6=-1511/794, 6-7=0/35, 1-12=-194/137
BOT CHORD 11-12=-347/868, 10-11=-327/977, 9-10=-327/977, 8-9=-561/1279, 6-8=-561/1279
WEBS 2-11=-32/147, 3-11=-50/226, 4-11=-199/70, 4-9=-99/285, 5-9=-351/268, 5-8=0/175, 2-12=-939/552

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JOINT STRESS INDEX
1 = 0.30, 2 = 0.29, 3 = 0.67, 4 = 0.87, 5 = 0.39, 6 = 0.65, 8 = 0.33, 9 = 0.34, 10 = 0.35, 11 = 0.56 and 12 = 0.57

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T05	HIP	1	1	J1876055 Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 154 lb uplift at joint 12 and 243 lb uplift at joint 6.

LOAD CASE(S) Standard

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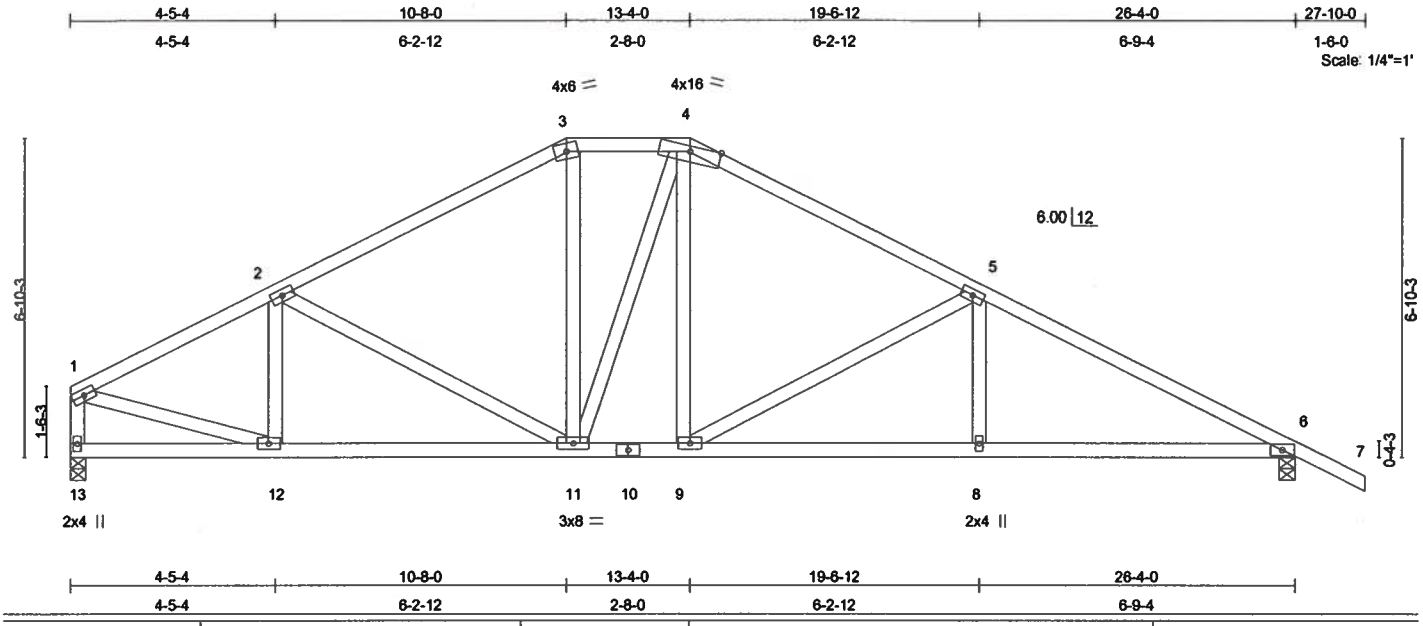
August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T06	HIP	1	1	J1876056
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 08 13:39:08 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	0.07	8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.12	8-9	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 151 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-4-12 oc bracing.

REACTIONS (lb/size) 6=926/0-4-0, 13=830/0-4-0
Max Horz 13=-139(load case 7)
Max Uplift 6=-253(load case 7), 13=-167(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 4-5=-1016/633, 5-6=-1491/799, 6-7=0/35, 3-4=-819/620, 1-2=-1065/605, 2-3=-989/617, 1-13=-804/469
BOT CHORD 12-13=-47/126, 11-12=-367/915, 10-11=-240/840, 9-10=-240/840, 8-9=-555/1256, 6-8=-555/1256
WEBS 5-8=0/219, 5-9=-480/361, 4-9=-144/272, 4-11=-194/111, 3-11=-82/217, 2-11=-162/159, 2-12=-206/177, 1-12=-462/883

JOINT STRESS INDEX
1 = 0.59, 2 = 0.39, 3 = 0.65, 4 = 0.75, 5 = 0.39, 6 = 0.64, 8 = 0.33, 9 = 0.34, 10 = 0.30, 11 = 0.64, 12 = 0.48 and 13 = 0.41

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

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August 10,2007

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876056
L250193	T06	HIP	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 253 lb uplift at joint 6 and 167 lb uplift at joint 13.

LOAD CASE(S) Standard

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T07	SPECIAL	1	1	J1876057
					Job Reference (optional)

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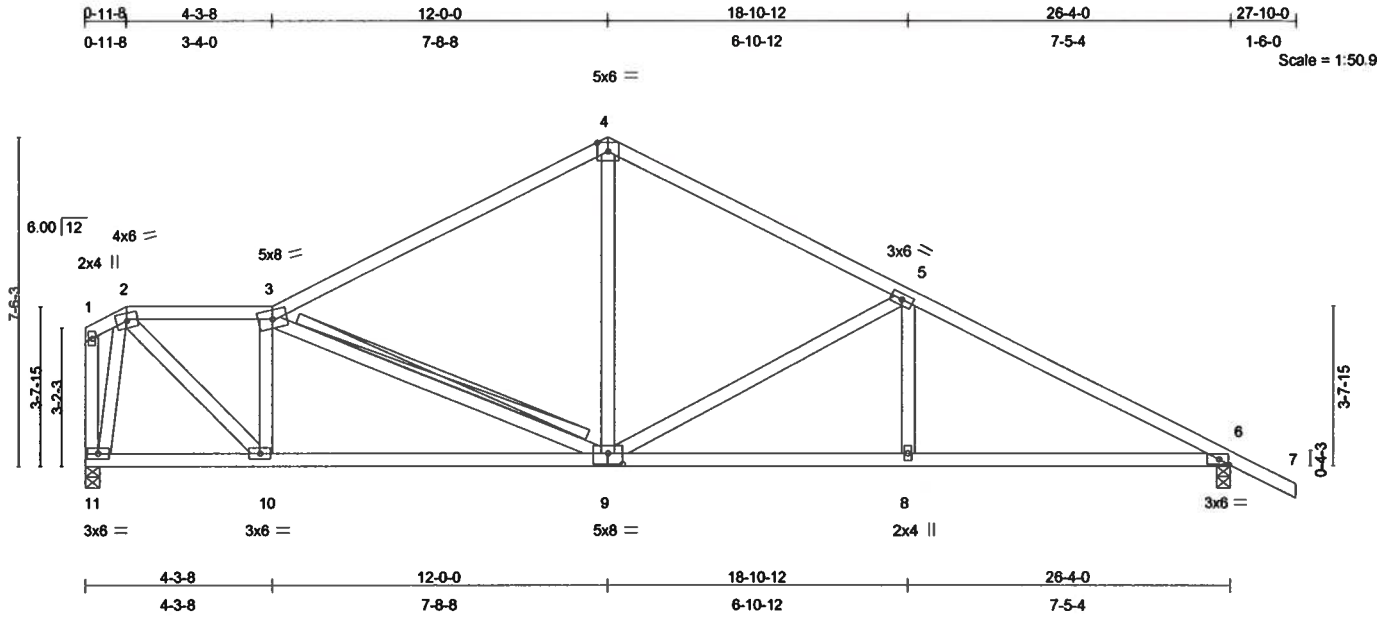

Plate Offsets (X,Y): [6:0-2-12,0-1-8], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.09	9-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.17	9-10	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 144 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-6-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 11=830/0-4-0, 6=926/0-4-0
Max Horz 11=-181(load case 7)
Max Uplift 11=-178(load case 6), 6=-256(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-60/57, 2-3=-900/544, 3-4=-965/589, 4-5=-957/599, 5-6=-1461/775, 6-7=0/35, 1-11=-105/99
BOT CHORD 10-11=-15/188, 9-10=-336/930, 8-9=-523/1222, 6-8=-523/1222
WEBS 2-10=-615/1034, 3-10=-690/493, 3-9=-249/160, 4-9=-199/436, 5-9=-532/380, 5-8=0/224, 2-11=-733/380

JOINT STRESS INDEX
1 = 0.33, 2 = 0.41, 3 = 0.70, 4 = 0.71, 5 = 0.39, 6 = 0.72, 8 = 0.33, 9 = 0.41, 10 = 0.64 and 11 = 0.

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August 10,2007

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876057
L250193	T07	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 178 lb uplift at joint 11 and 256 lb uplift at joint 6.

LOAD CASE(S) Standard

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T08	SPECIAL	1	1	J1876058
					Job Reference (optional)

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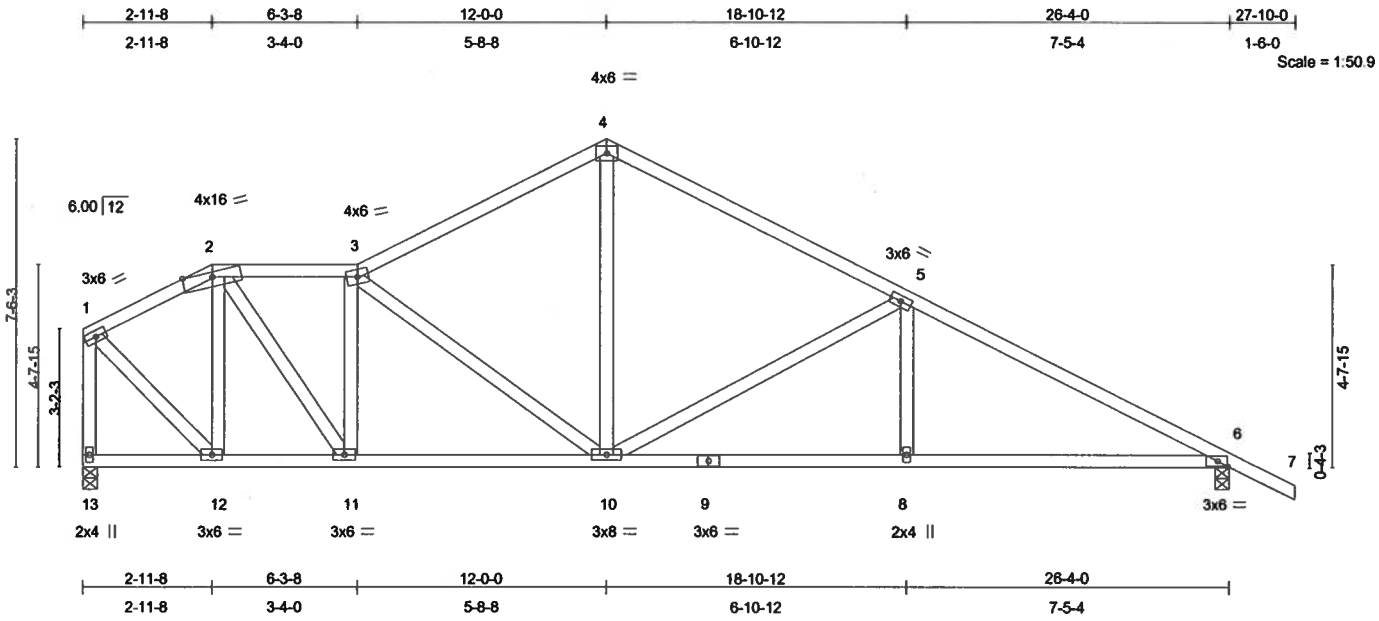

Plate Offsets (X,Y): [6:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.08	6-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.16	6-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 151 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-6-2 oc bracing.

REACTIONS (lb/size) 13=830/0-4-0, 6=926/0-4-0
Max Horz 13=-181(load case 7)
Max Uplift 13=-178(load case 6), 6=-256(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-595/350, 2-3=-898/573, 3-4=-923/597, 4-5=-950/593, 5-6=-1465/775, 6-7=0/35, 1-13=-809/472
BOT CHORD 12-13=-7/195, 11-12=-74/487, 10-11=-305/915, 9-10=-525/1227, 8-9=-525/1227, 6-8=-525/1227
WEBS 2-12=-461/277, 2-11=-405/696, 3-11=-519/356, 3-10=-249/158, 4-10=-244/442, 5-10=-537/394, 5-8=0/232, 1-12=-365/684

JOINT STRESS INDEX
1 = 0.51, 2 = 0.50, 3 = 0.63, 4 = 0.76, 5 = 0.39, 6 = 0.70, 8 = 0.33, 9 = 0.42, 10 = 0.56, 11 = 0.51, 12 = 0.43 and 13 = 0.33

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

Continued on page 2

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T08	SPECIAL	1	1	J1876058 Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 178 lb uplift at joint 13 and 256 lb uplift at joint 6.

LOAD CASE(S) Standard

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T09	SPECIAL	1	1	J1876059
					Job Reference (optional)

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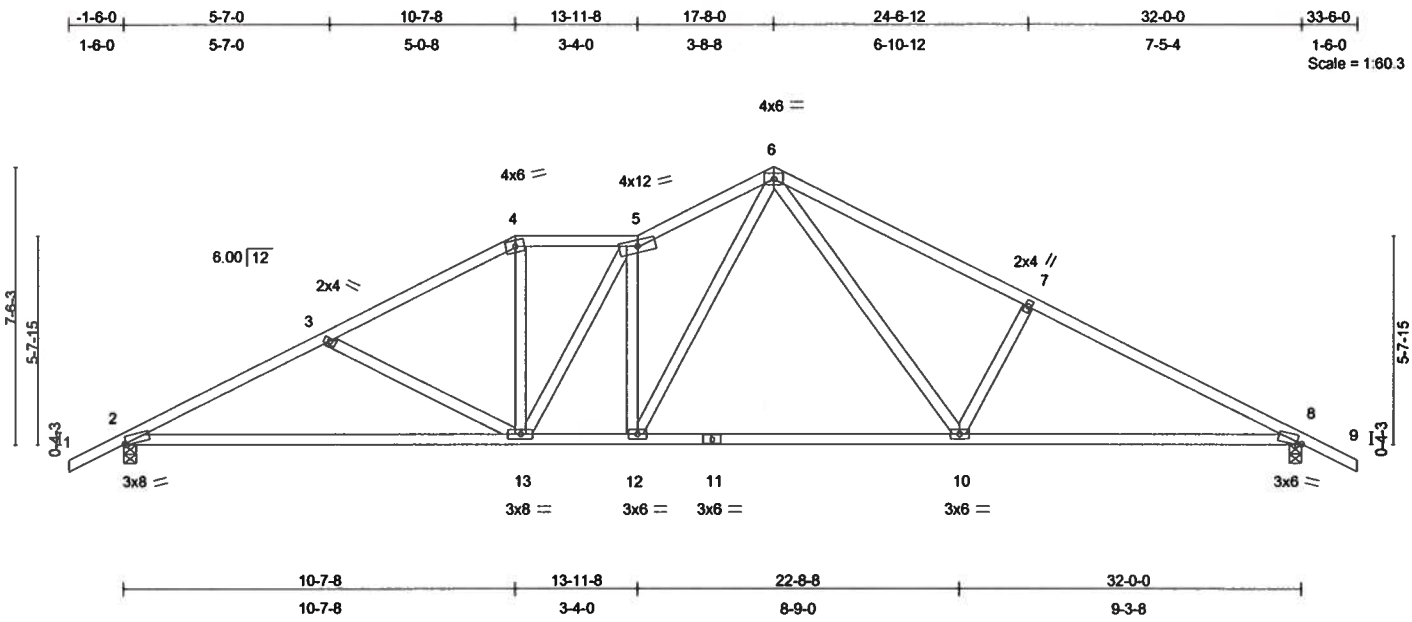

Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-1-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.28	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.52	2-13	>725	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 168 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-1-3 oc bracing.

REACTIONS (lb/size) 2=1103/0-4-0, 8=1103/0-4-0
Max Horz 2=-111(load case 7)
Max Uplift 2=-305(load case 6), 8=-286(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1870/1049, 3-4=-1564/900, 4-5=-1351/867, 5-6=-1665/1089, 6-7=-1657/1033, 7-8=-1838/1009, 8-9=0/35
BOT CHORD 2-13=-788/1606, 12-13=-600/1460, 11-12=-389/1109, 10-11=-389/1109, 8-10=-734/1561
WEBS 3-13=-310/289, 4-13=-189/418, 5-13=-215/125, 5-12=-660/442, 6-12=-472/791, 6-10=-319/524, 7-10=-357/352

JOINT STRESS INDEX
2 = 0.83, 3 = 0.33, 4 = 0.57, 5 = 0.45, 6 = 0.73, 7 = 0.33, 8 = 0.78, 10 = 0.44, 11 = 0.42, 12 = 0.63 and 13 = 0.59

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

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August 10,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T09	SPECIAL	1	1	J1876059
					Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 305 lb uplift at joint 2 and 286 lb uplift at joint 8.

LOAD CASE(S) Standard

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Boynton Beach, FL 33436

August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T10	SPECIAL	1	1	J1876060
					Job Reference (optional)

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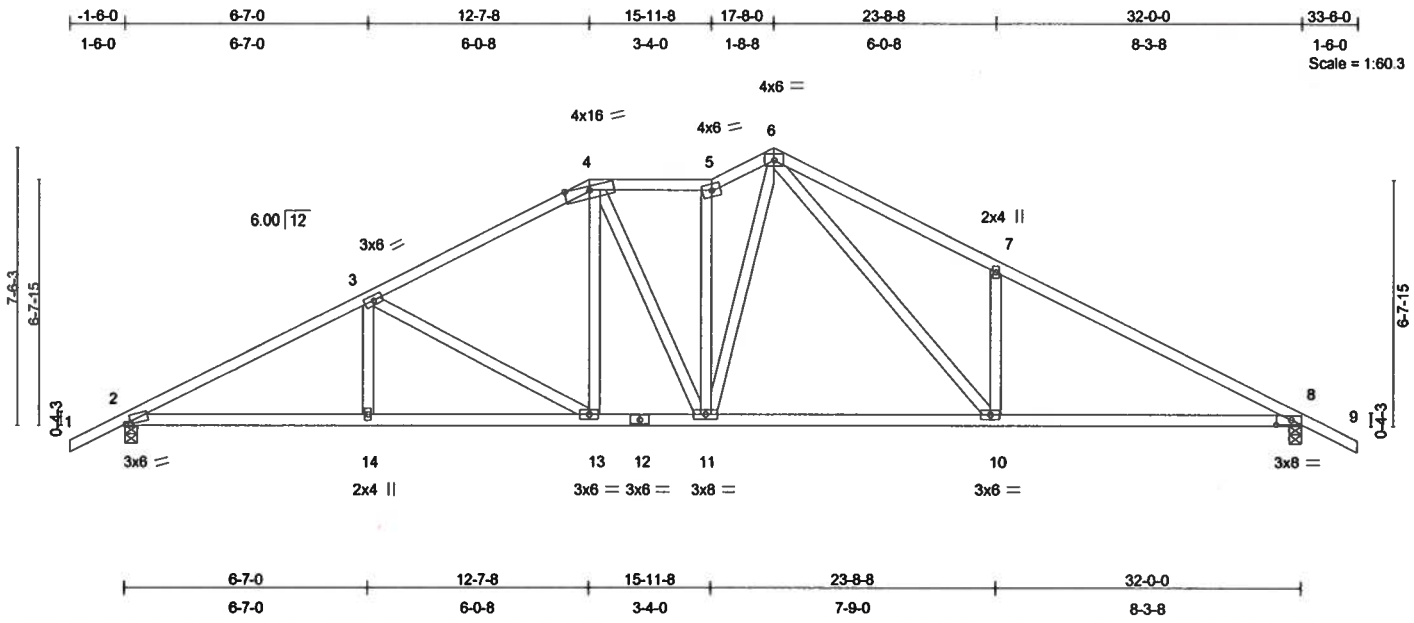

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [8:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	0.15	8-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.26	8-10	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 178 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-4 oc bracing.

REACTIONS (lb/size) 2=1103/0-4-0, 8=1103/0-4-0
Max Horz 2=-111(load case 7)
Max Uplift 2=-305(load case 6), 8=-286(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1878/1010, 3-4=-1431/860, 4-5=-1244/836, 5-6=-1381/928, 6-7=-1793/1182, 7-8=-1815/975, 8-9=0/35
BOT CHORD 2-14=-744/1600, 13-14=-744/1600, 12-13=-454/1217, 11-12=-454/1217, 10-11=-376/1105, 8-10=-690/1529
WEBS 3-14=0/212, 3-13=-450/333, 4-13=-139/263, 4-11=-67/161, 5-11=-603/381, 6-11=-350/647, 6-10=-487/674, 7-10=-405/406

JOINT STRESS INDEX
2 = 0.77, 3 = 0.39, 4 = 0.87, 5 = 0.56, 6 = 0.58, 7 = 0.33, 8 = 0.70, 10 = 0.45, 11 = 0.82, 12 = 0.40, 13 = 0.34 and 14 = 0.33

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

Continued on page 2

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876060
L250193	T10	SPECIAL	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 305 lb uplift at joint 2 and 286 lb uplift at joint 8.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T11	HIP	1	1	J1876061
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 08 13:39:12 2007 Page 1

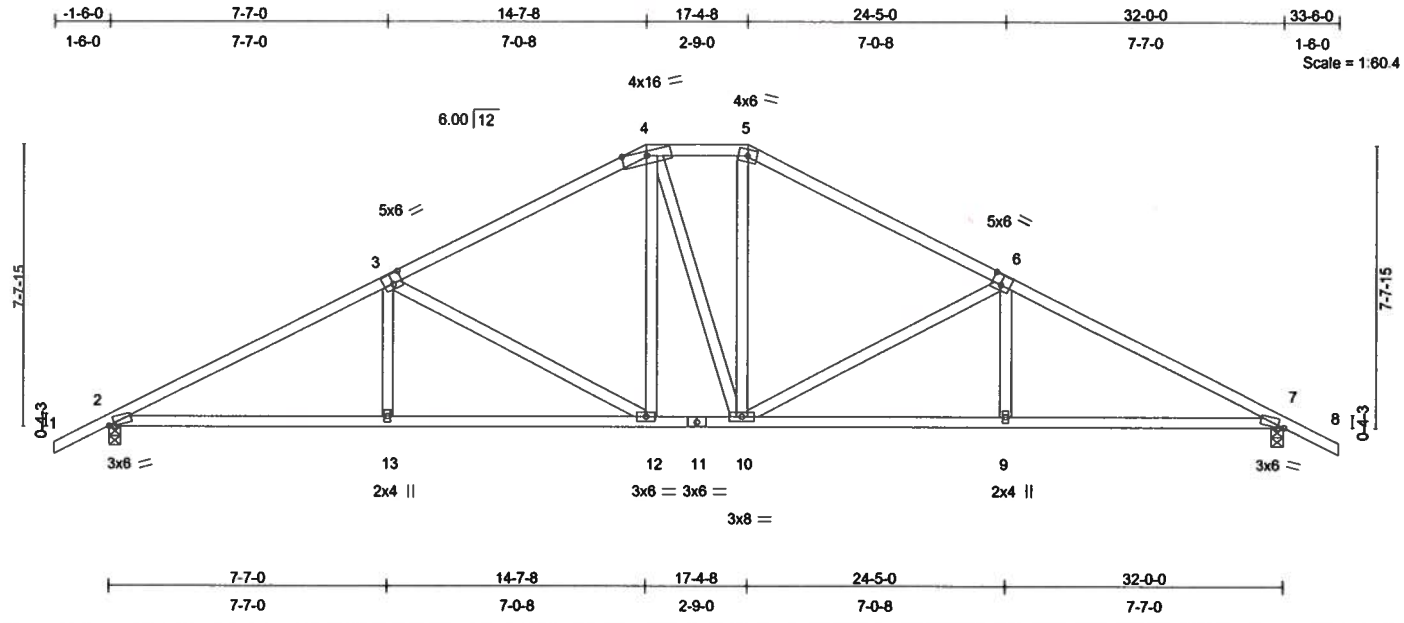

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [3:0-3-0,0-3-4], [6:0-3-0,0-3-4], [7:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.12	12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.20	12-13	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 172 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-3 oc bracing.

REACTIONS (lb/size) 2=1103/0-4-0, 7=1103/0-4-0
Max Horz 2=-113(load case 7)
Max Uplift 2=-287(load case 6), 7=-287(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1847/998, 3-4=-1309/806, 4-5=-1094/794, 5-6=-1310/807, 6-7=-1847/998, 7-8=0/35
BOT CHORD 2-13=-721/1566, 12-13=-722/1564, 11-12=-361/1093, 10-11=-361/1093, 9-10=-722/1563, 7-9=-721/1565
WEBS 3-13=0/247, 3-12=-546/414, 4-12=-168/313, 4-10=-176/183, 5-10=-168/313, 6-10=-544/413, 6-9=0/246

JOINT STRESS INDEX
2 = 0.79, 3 = 0.72, 4 = 0.93, 5 = 0.71, 6 = 0.72, 7 = 0.79, 9 = 0.33, 10 = 0.65, 11 = 0.38, 12 = 0.34 and 13 = 0.33

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

Continued on page 2

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August 10,2007

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876061
L250193	T11	HIP	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 2 and 287 lb uplift at joint 7.

LOAD CASE(S) Standard

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T12	COMMON	3	1	J1876062 Job Reference (optional)

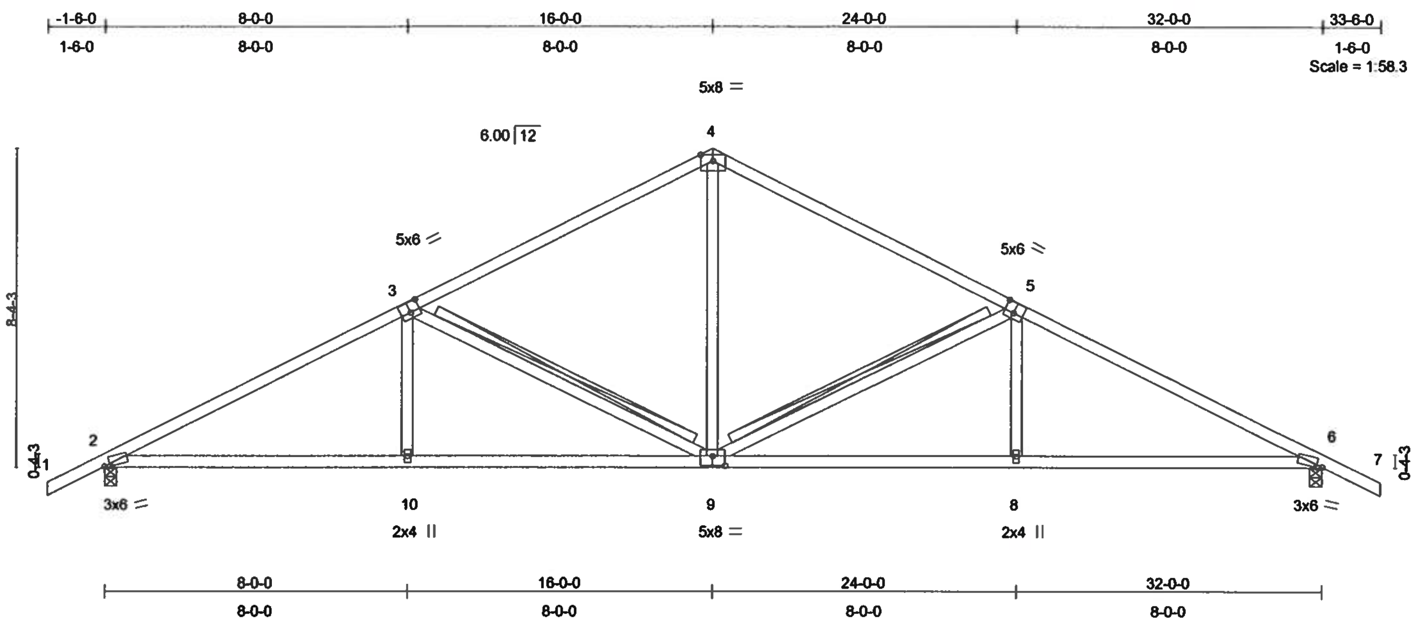

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [3:0-3-0,0-3-4], [5:0-3-0,0-3-4], [6:0-1-13,0-0-7], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.13	2-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.22	2-10	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.08	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 155 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-2-9 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-9, 5-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1103/0-4-0, 6=1103/0-4-0
Max Horz 2=121(load case 6)
Max Uplift 2=-294(load case 6), 6=-294(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1835/1003, 3-4=-1248/785, 4-5=-1248/785, 5-6=-1835/1003, 6-7=0/35
BOT CHORD 2-10=-721/1552, 9-10=-722/1551, 8-9=-722/1551, 6-8=-721/1552
WEBS 3-10=0/257, 3-9=-614/447, 4-9=-351/604, 5-9=-614/447, 5-8=0/257

JOINT STRESS INDEX
2 = 0.83, 3 = 0.79, 4 = 0.82, 5 = 0.79, 6 = 0.83, 8 = 0.33, 9 = 0.45 and 10 = 0.33

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August 10,2007

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876062
L250193	T12	COMMON	3	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 294 lb uplift at joint 2 and 294 lb uplift at joint 6.

LOAD CASE(S) Standard

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T13	SPECIAL	1	1	J1876063
					Job Reference (optional)

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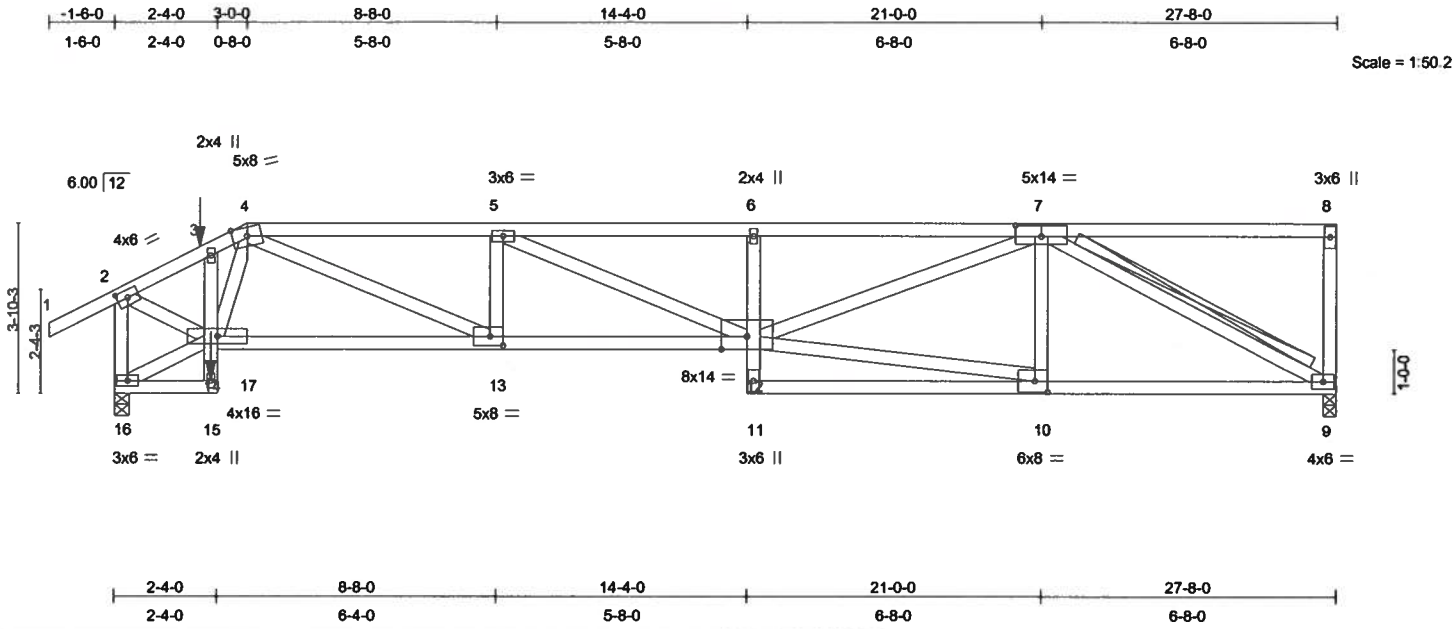

2-4-0	8-8-0	14-4-0	21-0-0	27-8-0
2-4-0	6-4-0	5-8-0	6-8-0	6-8-0

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [7:0-7-0,0-3-0], [10:0-3-8,0-3-0], [13:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.35	12-13	>932	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.68	12-13	>481	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.86	Horz(TL)	0.23	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.1D *Except*
1-4 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
3-15 2 X 4 SYP No.3, 12-14 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
8-9 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-5-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-2-15 oc bracing.
WEBS T-Brace: 2 X 6 SYP No.1D - 7-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 9=1926/0-3-8, 16=2198/0-4-0
Max Horz 16=137(load case 5)
Max Uplift 9=-665(load case 4), 16=-670(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-2165/729, 3-4=-2007/653, 4-5=-4650/1614, 5-6=-5287/1837, 6-7=-5181/1812, 7-8=-120/41, 8-9=-358/183, 2-16=-2175/690
BOT CHORD 15-16=-53/11, 14-15=-103/358, 3-14=-181/264, 14-17=-794/2213, 13-17=-794/2213, 12-13=-1614/4650, 11-12=0/140, 6-12=-710/368, 10-11=-133/472, 9-10=-979/2834
WEBS 4-14=-906/466, 4-13=-915/2671, 5-13=-936/452, 5-12=-260/696, 10-12=-856/2390, 7-12=-894/2521, 7-10=-258/240, 7-9=-3092/1069, 2-14=-673/2134, 14-16=-51/75

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JOINT STRESS INDEX

2 = 0.86, 3 = 0.47, 4 = 0.71, 5 = 0.40, 6 = 0.54, 7 = 0.65, 8 = 0.84, 9 = 0.82, 10 = 0.54, 11 = 0.61, 12 = 0.79, 13 = 0.60, 14 = 0.86, 15 = 0.34 and 16 = 0.71

August 10,2007

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876063
L250193	T13	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 665 lb uplift at joint 9 and 670 lb uplift at joint 16.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54, 2-4=-54, 4-8=-118(F=-64), 15-16=-10, 14-17=-10, 12-17=-22(F=-12), 9-11=-22(F=-12)
   Concentrated Loads (lb)
   Vert: 15=-362(F) 3=-74(F)

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T14	SPECIAL	1	1	J1876064
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 08 13:39:15 2007 Page 1

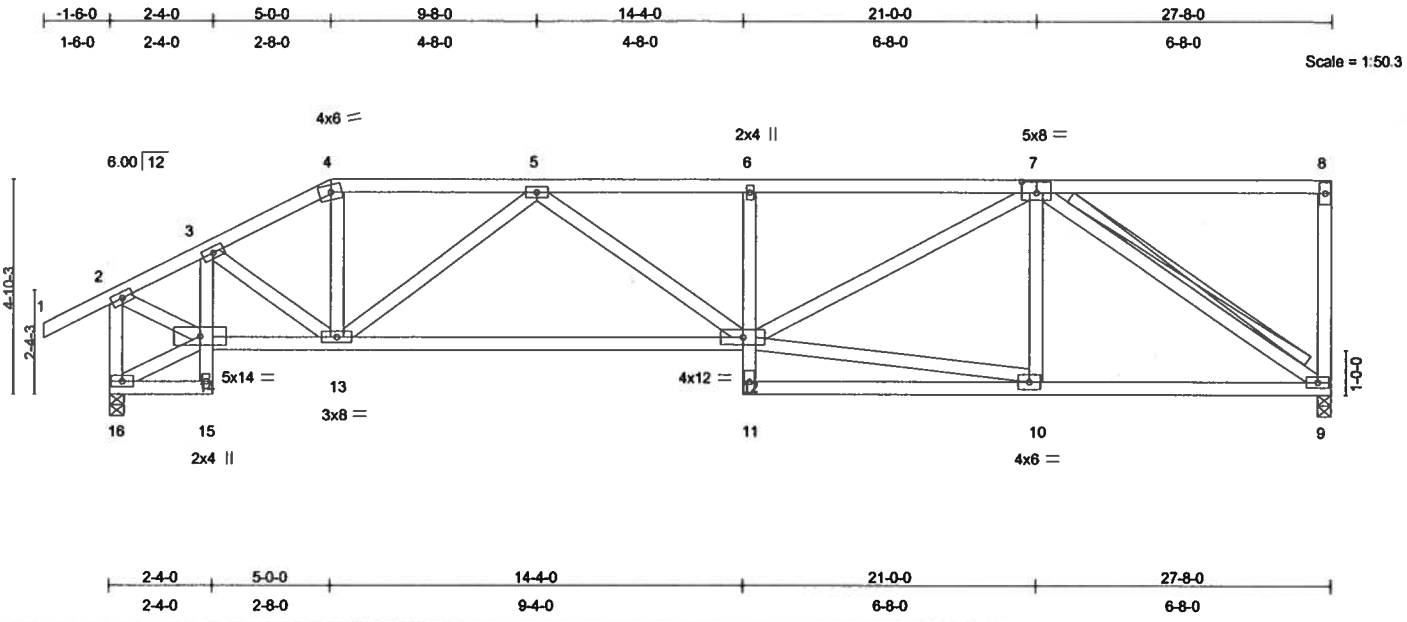

Plate Offsets (X,Y): [7:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.16	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.32	12-13	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.08	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 175 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
3-15 2 X 4 SYP No.3, 6-11 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-7-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 7-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 9=873/0-3-8, 16=968/0-4-0
Max Horz 16=169(load case 6)
Max Uplift 9=-247(load case 5), 16=-207(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-872/597, 3-4=-1154/679, 4-5=-1030/652, 5-6=-1719/945, 6-7=-1704/948, 7-8=-38/20, 8-9=-161/114, 2-16=-932/676
BOT CHORD 15-16=-39/59, 14-15=-12/32, 3-14=-395/96, 13-14=-631/756, 12-13=-910/1525, 11-12=0/95, 6-12=-303/217, 10-11=-72/82, 9-10=-545/1003
WEBS 3-13=-74/322, 4-13=-135/303, 5-13=-630/329, 5-12=-43/256, 10-12=-479/932, 7-12=-459/799, 7-10=-88/126, 7-9=-1178/641, 2-14=-457/839, 14-16=-184/0

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JOINT STRESS INDEX
2 = 0.63, 3 = 0.39, 4 = 0.38, 5 = 0.34, 6 = 0.33, 7 = 0.44, 8 = 0.33, 9 = 0.37, 10 = 0.38, 11 = 0.33, 12 = 0.70, 13 = 0.56, 14 = 0.53, 15 = 0.33 and 16 = 0.40

August 10,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T14	SPECIAL	1	1	J1876064
					Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 247 lb uplift at joint 9 and 207 lb uplift at joint 16.

LOAD CASE(S) Standard

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T15	SPECIAL	1	1	J1876065
					Job Reference (optional)

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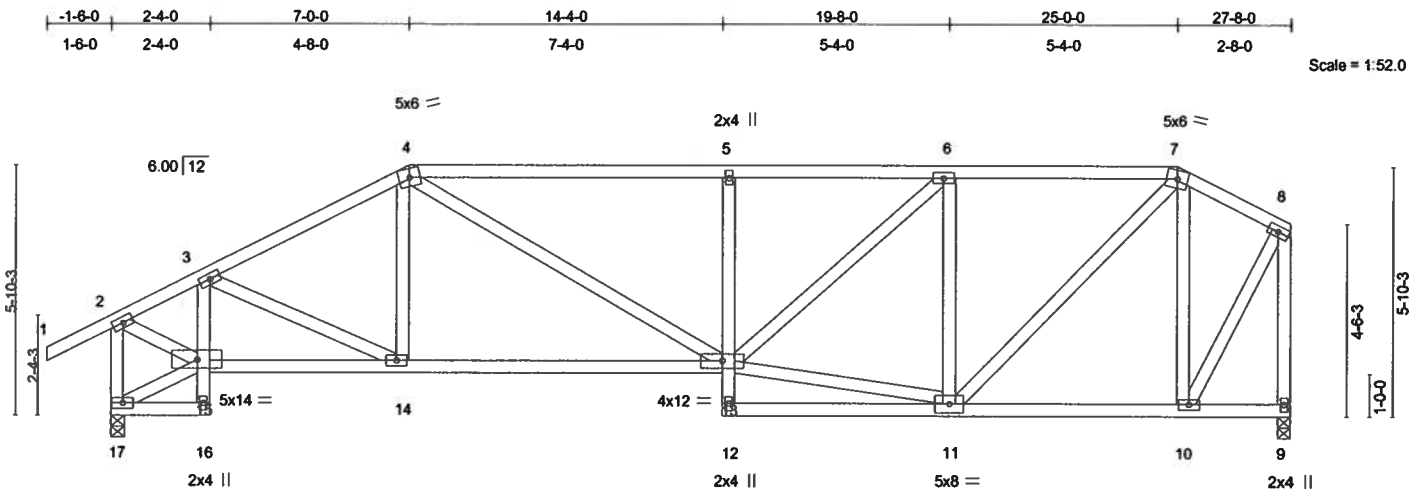

2-4-0	7-0-0	14-4-0	19-8-0	25-0-0	27-8-0
2-4-0	4-8-0	7-4-0	5-4-0	5-4-0	2-8-0

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	0.09	13-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.18	13-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 190 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
3-16 2 X 4 SYP No.3, 5-12 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
7-11-4 oc bracing: 14-15
8-0-6 oc bracing: 13-14.

REACTIONS (lb/size) 9=873/0-3-8, 17=968/0-4-0
Max Horz 17=175(load case 6)
Max Uplift 9=-201(load case 4), 17=-208(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-881/598, 3-4=-1195/695, 4-5=-1362/813, 5-6=-1329/797, 6-7=-904/551, 7-8=-423/243, 2-17=-935/669, 8-9=-863/476
BOT CHORD 16-17=-54/37, 15-16=-14/29, 3-15=-375/134, 14-15=-638/787, 13-14=-620/1032, 12-13=0/75, 5-13=-360/250, 11-12=-74/86, 10-11=-201/380, 9-10=-5/6
WEBS 3-14=-99/330, 4-14=-71/126, 4-13=-150/461, 11-13=-420/832, 6-13=-328/568, 6-11=-657/415, 7-11=-411/764, 7-10=-605/367, 2-15=-470/855, 8-10=-384/739, 15-17=-147/0

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JOINT STRESS INDEX
2 = 0.69, 3 = 0.48, 4 = 0.57, 5 = 0.85, 6 = 0.35, 7 = 0.29, 8 = 0.60, 9 = 0.33, 10 = 0.60, 11 = 0.38, 12 = 0.73, 13 = 0.65, 14 = 0.34, 15 = 0.31, 16 = 0.33 and 17 = 0.41

NOTES
1) Unbalanced roof live loads have been considered for this design.

August 10,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T15	SPECIAL	1	1	J1876065 Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 201 lb uplift at joint 9 and 208 lb uplift at joint 17.

LOAD CASE(S) Standard

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Truss Design Engineer
Florida PE No. 34869
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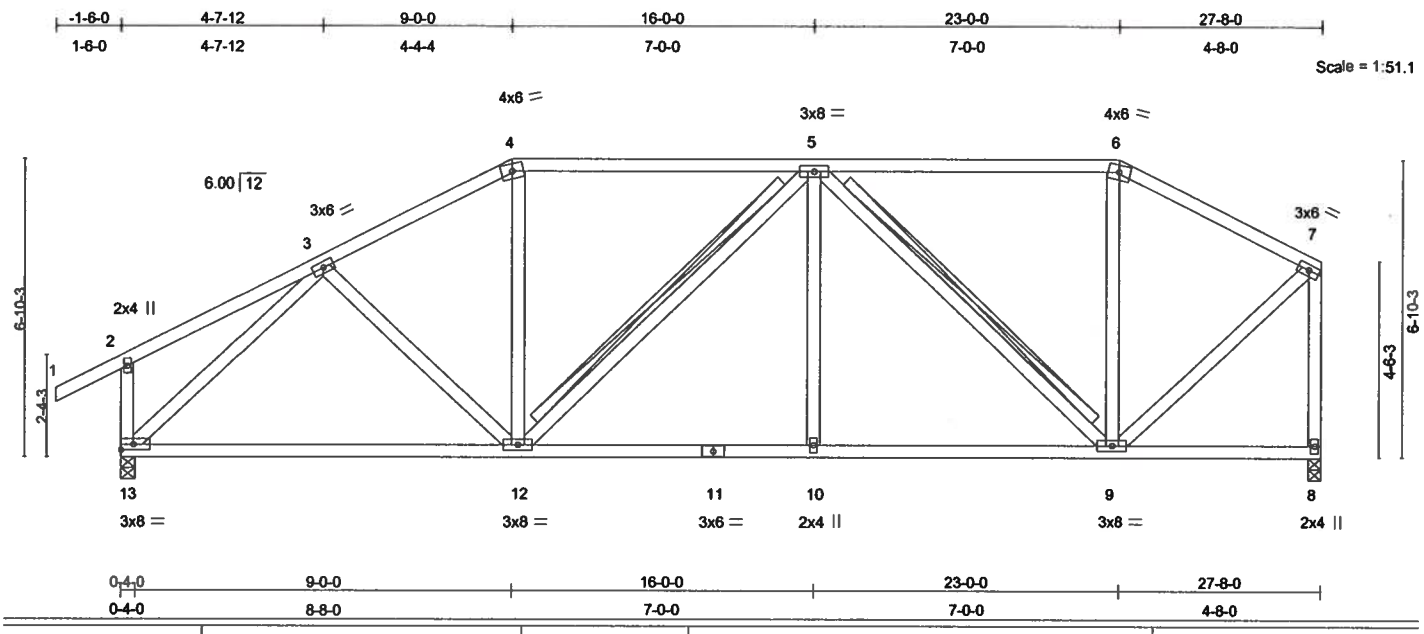
Boynton Beach, FL 33435

August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T16	HIP	1	1	J1876066
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 08 13:39:17 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.11	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.21	12-13	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.03	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 178 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-12, 5-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 13=968/0-4-0, 8=873/0-3-8
Max Horz 13=187(load case 6)
Max Uplift 13=-222(load case 6), 8=-170(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-139/171, 3-4=-968/593, 4-5=-830/585, 5-6=-543/396, 6-7=-650/379, 2-13=-253/297, 7-8=-852/492
BOT CHORD 12-13=-516/744, 11-12=-501/929, 10-11=-501/929, 9-10=-501/929, 8-9=-12/13
WEBS 3-13=-959/448, 3-12=-47/202, 4-12=-28/204, 5-12=-223/94, 5-10=0/196, 5-9=-564/305, 6-9=-76/102, 7-9=-366/720

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JOINT STRESS INDEX
2 = 0.49, 3 = 0.39, 4 = 0.65, 5 = 0.56, 6 = 0.67, 7 = 0.56, 8 = 0.33, 9 = 0.69, 10 = 0.33, 11 = 0.32, 12 = 0.56 and 13 = 0.56

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T16	HIP	1	1	J1876066 Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 13 and 170 lb uplift at joint 8.

LOAD CASE(S) Standard

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T17	SPECIAL	1	1	J1876067
					Job Reference (optional)

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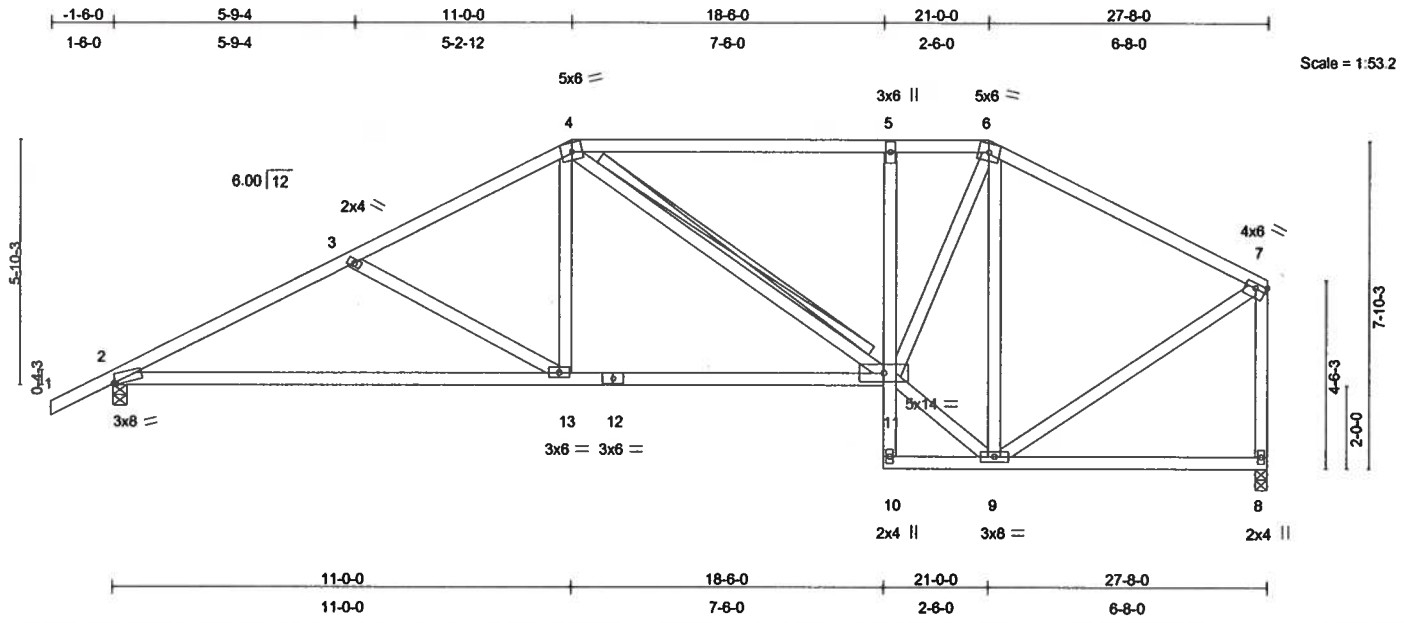

Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.69	Vert(LL)	-0.26	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.49	2-13	>676	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 170 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 5-10 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=968/0-4-0, 8=873/0-3-8
Max Horz 2=148(load case 6)
Max Uplift 2=-247(load case 6), 8=-144(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1567/852, 3-4=-1271/713, 4-5=-984/647, 5-6=-954/629, 6-7=-738/426, 7-8=-835/489
BOT CHORD 2-13=-785/1334, 12-13=-534/1079, 11-12=-534/1079, 10-11=-14/12, 5-11=-318/145, 9-10=-37/32, 8-9=-36/46
WEBS 3-13=-286/282, 4-13=-74/365, 4-11=-166/66, 9-11=-333/735, 6-11=-421/906, 6-9=-785/435, 7-9=-305/656

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JOINT STRESS INDEX
2 = 0.78, 3 = 0.33, 4 = 0.60, 5 = 0.33, 6 = 0.56, 7 = 0.78, 8 = 0.75, 9 = 0.70, 10 = 0.33, 11 = 0.81, 12 = 0.52 and 13 = 0.34

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876067
L250193	T17	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 247 lb uplift at joint 2 and 144 lb uplift at joint 8.

LOAD CASE(S) Standard

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE J1876068
L250193	T18	SPECIAL	1	1	Job Reference (optional)

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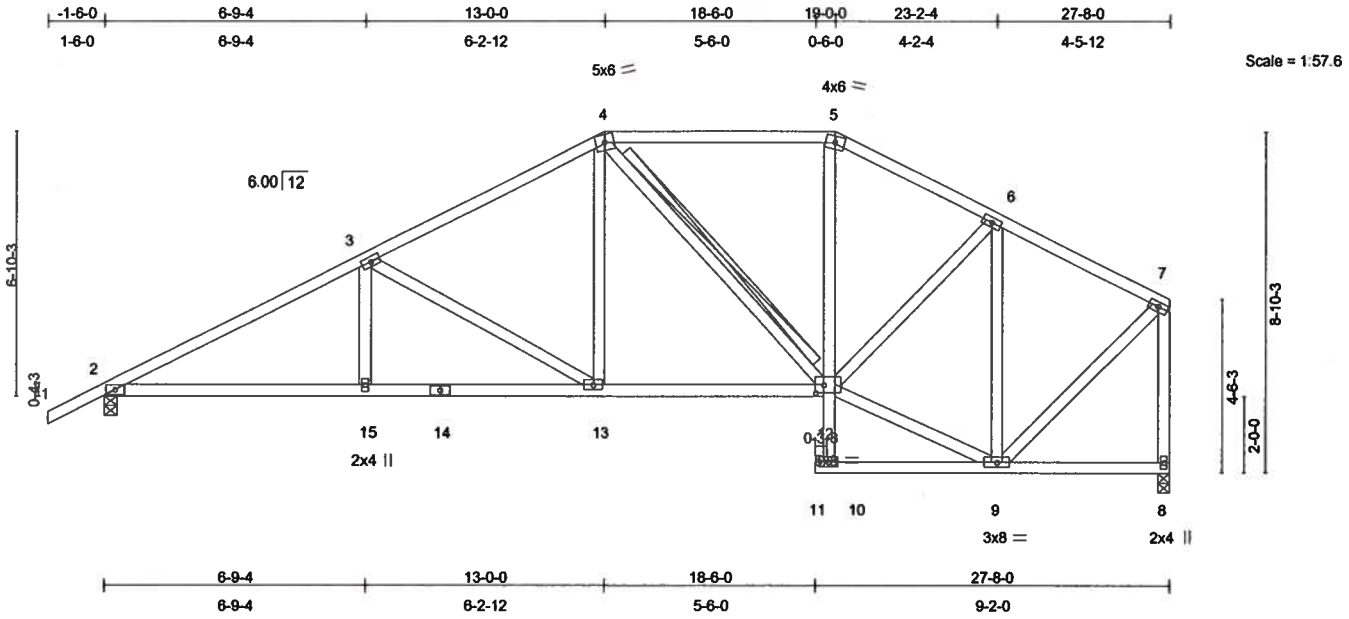

Plate Offsets (X,Y): [12:0-2-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.07	15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.13	2-15	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 177 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-2-6 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=969/0-4-0, 8=874/0-3-8
Max Horz 2=159(load case 6)
Max Uplift 2=-258(load case 6), 8=-158(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1579/831, 3-4=-1131/676, 4-5=-809/593, 5-6=-943/607, 6-7=-618/363, 7-8=-849/491
BOT CHORD 2-15=-756/1333, 14-15=-756/1333, 13-14=-756/1333, 12-13=-450/936, 10-11=0/0, 9-10=-2/7, 8-9=-15/20
WEBS 3-15=0/201, 3-13=-449/348, 4-13=-130/349, 4-12=-270/113, 10-12=0/63, 5-12=-74/200, 6-9=-662/404, 7-9=-353/685, 9-12=-289/549, 6-12=-150/415

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JOINT STRESS INDEX
2 = 0.68, 3 = 0.39, 4 = 0.50, 5 = 0.53, 6 = 0.40, 7 = 0.63, 8 = 0.38, 9 = 0.66, 10 = 0.26, 12 = 0.36, 13 = 0.34, 14 = 0.43 and 15 = 0.33

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876068
L250193	T18	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 258 lb uplift at joint 2 and 158 lb uplift at joint 8.

LOAD CASE(S) Standard

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T19	SPECIAL	1	1	J1876069
					Job Reference (optional)

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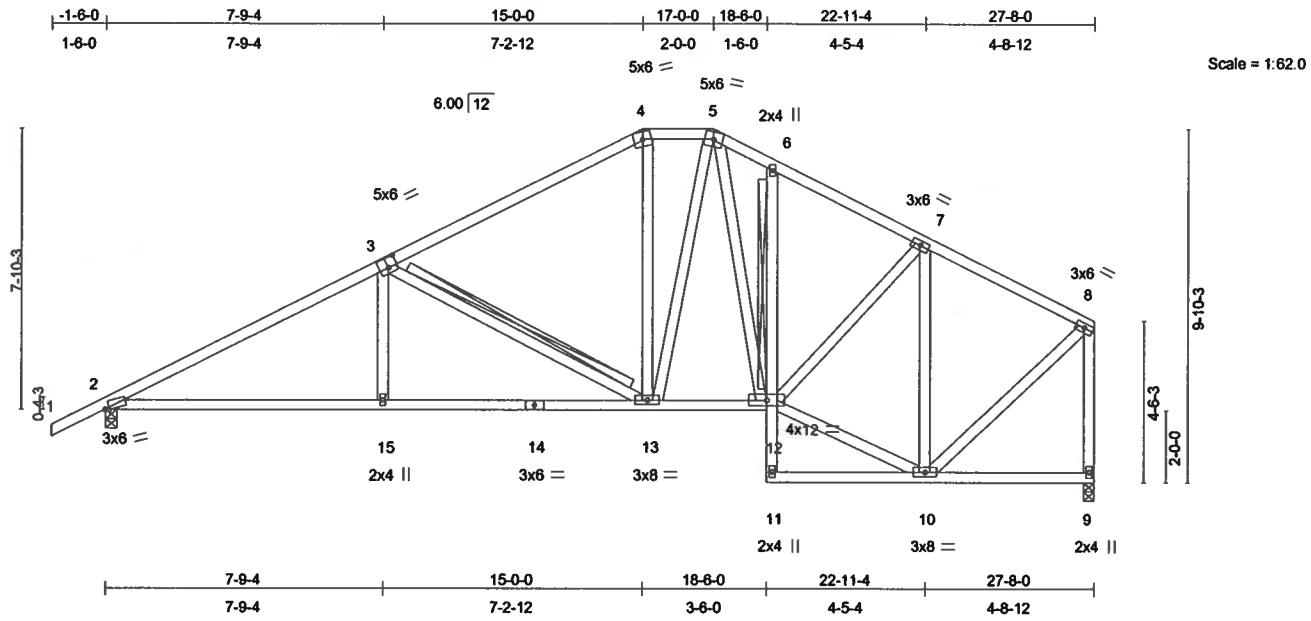

Plate Offsets (X,Y): [2:0-1-5,0-0-7], [3:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.10	2-15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.18	2-15	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 192 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 6-11 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-1-12 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 6-12
WEBS T-Brace: 2 X 4 SYP No.3 - 3-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=968/0-4-0, 9=873/0-3-8
Max Horz 2=171(load case 6)
Max Uplift 2=-267(load case 6), 9=-185(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1546/825, 3-4=-992/628, 4-5=-805/635, 5-6=-909/710, 6-7=-932/622, 7-8=-636/382, 8-9=-845/502
BOT CHORD 2-15=-739/1296, 14-15=-739/1294, 13-14=-739/1294, 12-13=-326/744, 11-12=0/60, 6-12=-183/188, 10-11=-5/28, 9-10=-16/21
WEBS 3-15=0/250, 3-13=-564/424, 4-13=-12/200, 5-13=-170/332, 5-12=-208/206, 10-12=-303/554, 7-12=-135/379, 7-10=-666/414, 8-10=-362/686

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August 10,2007

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T19	SPECIAL	1	1	J1876069
					Job Reference (optional)

JOINT STRESS INDEX

2 = 0.83, 3 = 0.73, 4 = 0.58, 5 = 0.32, 6 = 0.33, 7 = 0.40, 8 = 0.68, 9 = 0.40, 10 = 0.66, 11 = 0.42, 12 = 0.85, 13 = 0.59, 14 = 0.45 and 15 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 267 lb uplift at joint 2 and 185 lb uplift at joint 9.

LOAD CASE(S) Standard

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T20	HIP	1	1	J1876070
					Job Reference (optional)

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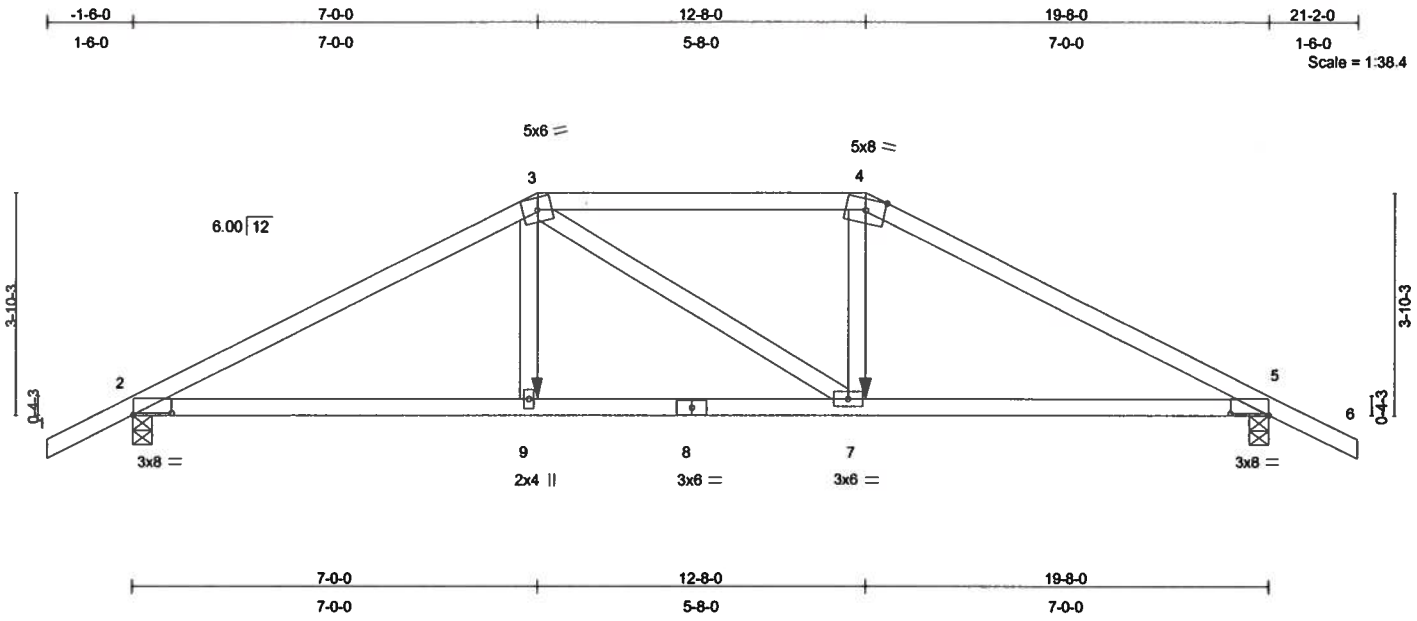

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [5:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.08	7-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.18	7-9	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.18	Horz(TL)	0.07	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 85 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-1-1 oc bracing.

REACTIONS (lb/size) 2=1339/0-4-0, 5=1326/0-4-0
Max Horz 2=68(load case 5)
Max Uplift 2=-446(load case 5), 5=-442(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2391/713, 3-4=-2050/669, 4-5=-2360/705, 5-6=0/35
BOT CHORD 2-9=-611/2056, 8-9=-606/2035, 7-8=-606/2035, 5-7=-567/2029
WEBS 3-9=-111/521, 3-7=-114/149, 4-7=-127/566

JOINT STRESS INDEX
2 = 0.73, 3 = 0.68, 4 = 0.78, 5 = 0.72, 7 = 0.36, 8 = 0.73 and 9 = 0.37

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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August 10,2007

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Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876070
L250193	T20	HIP	1	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 446 lb uplift at joint 2 and 442 lb uplift at joint 5.
7) Girder carries hip end with 7-0-0 end setback.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-9=-10, 7-9=-22(F=-12), 5-7=-10
   Concentrated Loads (lb)
   Vert: 9=-411(F) 7=-411(F)

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August 10,2007

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T21	HIP	1	1	J1876071
					Job Reference (optional)

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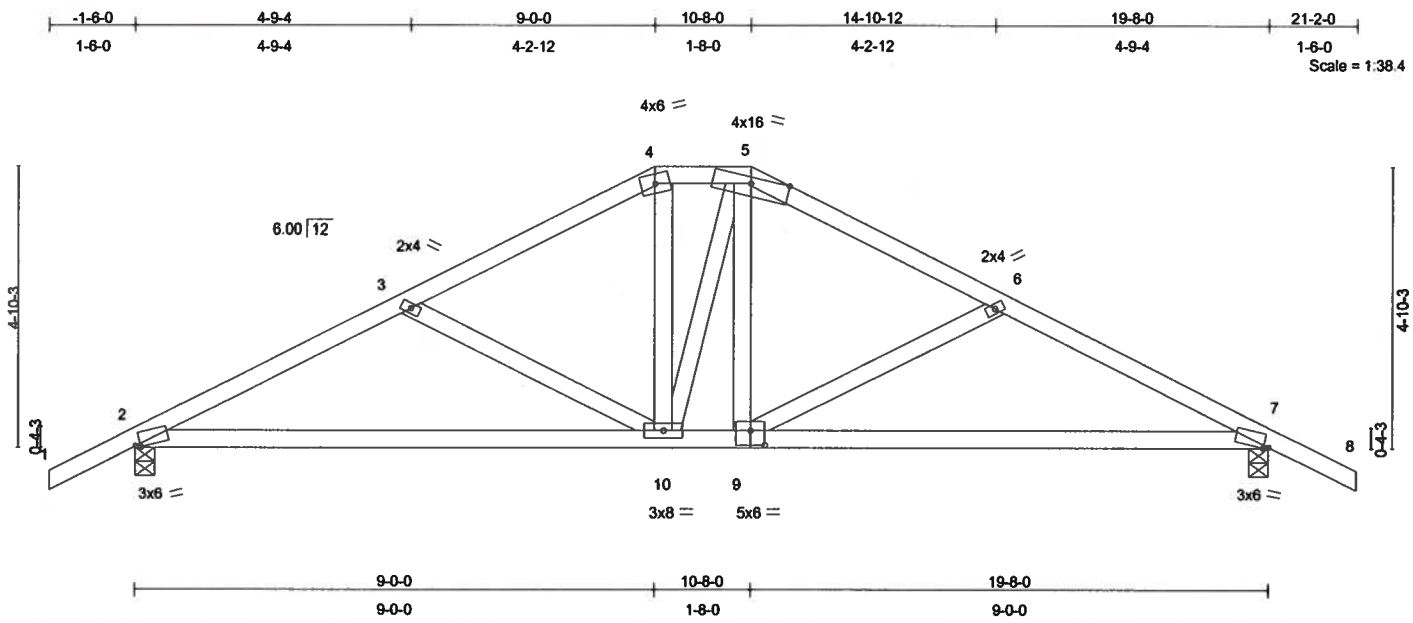

Plate Offsets (X,Y): [2:0-1-1,0-0-7], [7:0-1-1,0-0-7], [9:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.13	7-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.24	7-9	>962	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.12	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 101 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=709/0-4-0, 7=709/0-4-0
Max Horz 2=-80(load case 7)
Max Uplift 2=-204(load case 6), 7=-204(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1055/584, 3-4=-792/458, 4-5=-663/460, 5-6=-791/458, 6-7=-1055/584, 7-8=0/35
BOT CHORD 2-10=-385/889, 9-10=-170/662, 7-9=-385/889
WEBS 3-10=-265/244, 4-10=-77/206, 5-10=-108/116, 5-9=-77/207, 6-9=-266/245

JOINT STRESS INDEX
2 = 0.86, 3 = 0.33, 4 = 0.35, 5 = 0.47, 6 = 0.33, 7 = 0.87, 9 = 0.73 and 10 = 0.66

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2

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August 10,2007

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876071
L250193	T21	HIP	1	1	Job Reference (optional)

NOTES

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 204 lb uplift at joint 2 and 204 lb uplift at joint 7.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T22	COMMON	4	1	J1876072
					Job Reference (optional)

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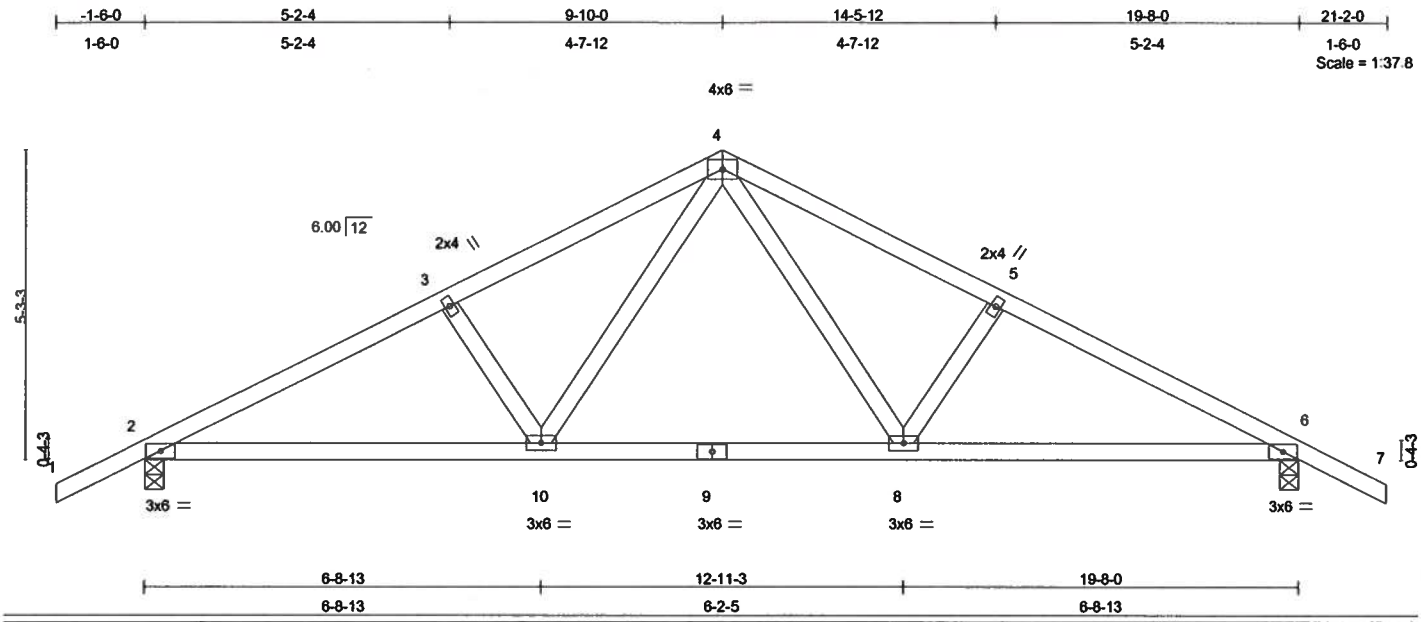

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.14	8-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.22	8-10	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.17	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 92 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-5-5 oc bracing.

REACTIONS (lb/size) 2=864/0-4-0, 6=864/0-4-0
Max Horz 2=85(load case 6)
Max Uplift 2=-251(load case 6), 6=-251(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1416/774, 3-4=-1274/773, 4-5=-1274/773, 5-6=-1416/774, 6-7=0/35
BOT CHORD 2-10=-548/1198, 9-10=-287/832, 8-9=-287/832, 6-8=-548/1198
WEBS 3-10=-217/211, 4-10=-280/503, 4-8=-280/503, 5-8=-217/211

JOINT STRESS INDEX
2 = 0.61, 3 = 0.33, 4 = 0.46, 5 = 0.33, 6 = 0.61, 8 = 0.41, 9 = 0.61 and 10 = 0.41

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 2 and 251 lb uplift at joint 6.

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Continued on page 2

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T22	COMMON	4	1	J1876072 Job Reference (optional)

NOTES

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-60(F=-50), 6-8=-10

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T22A	COMMON	2	1	J1876073
					Job Reference (optional)

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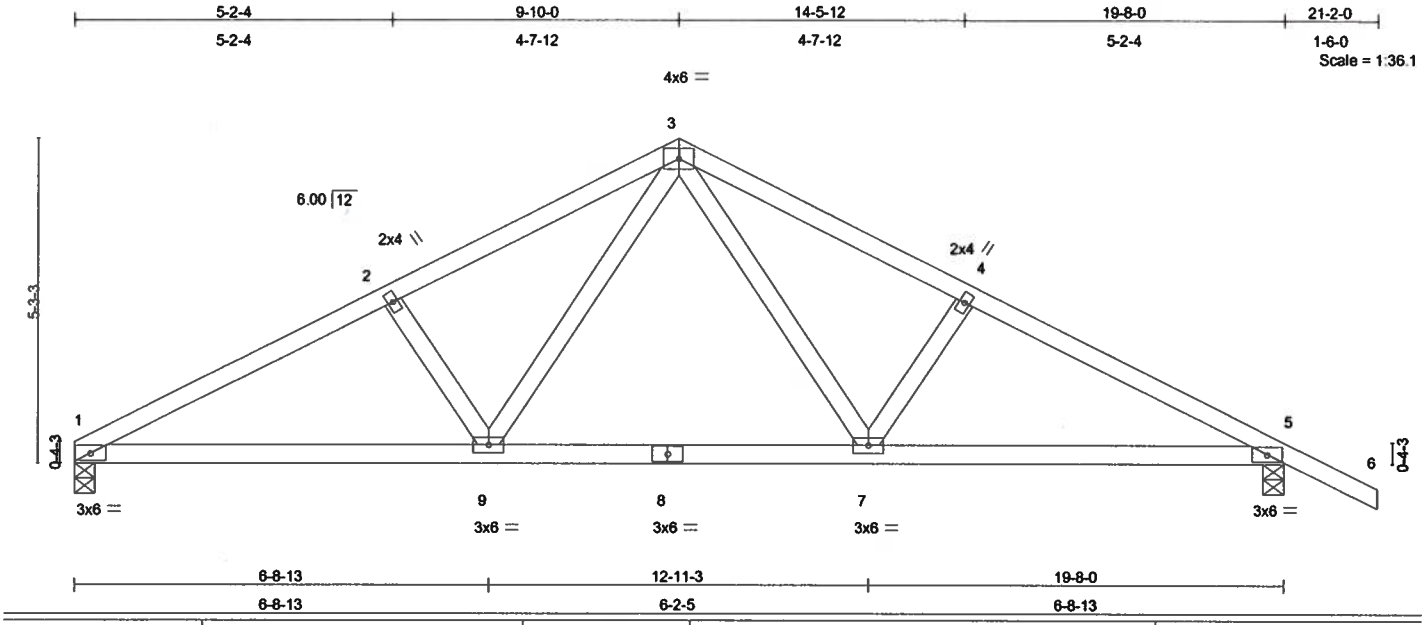

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.14	7-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.21	7-9	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.19	Horz(TL)	0.04	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 90 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-0-12 oc bracing.

REACTIONS (lb/size) 1=770/0-4-0, 5=867/0-4-0
Max Horz 1=-94(load case 7)
Max Uplift 1=-177(load case 6), 5=-252(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1438/811, 2-3=-1296/809, 3-4=-1281/785, 4-5=-1423/786, 5-6=0/35
BOT CHORD 1-9=-588/1222, 8-9=-300/839, 7-8=-300/839, 5-7=-559/1204
WEBS 2-9=-228/229, 3-9=-314/524, 3-7=-276/502, 4-7=-217/212

JOINT STRESS INDEX
1 = 0.70, 2 = 0.33, 3 = 0.45, 4 = 0.33, 5 = 0.70, 7 = 0.41, 8 = 0.60 and 9 = 0.41

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 177 lb uplift at joint 1 and 252 lb uplift at joint 5.

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Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T22A	COMMON	2	1	J1876073 Job Reference (optional)

NOTES

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-6=-54, 1-9=-10, 7-9=-60(F=-50), 5-7=-10

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T24	COMMON	4	1	J1876074
					Job Reference (optional)

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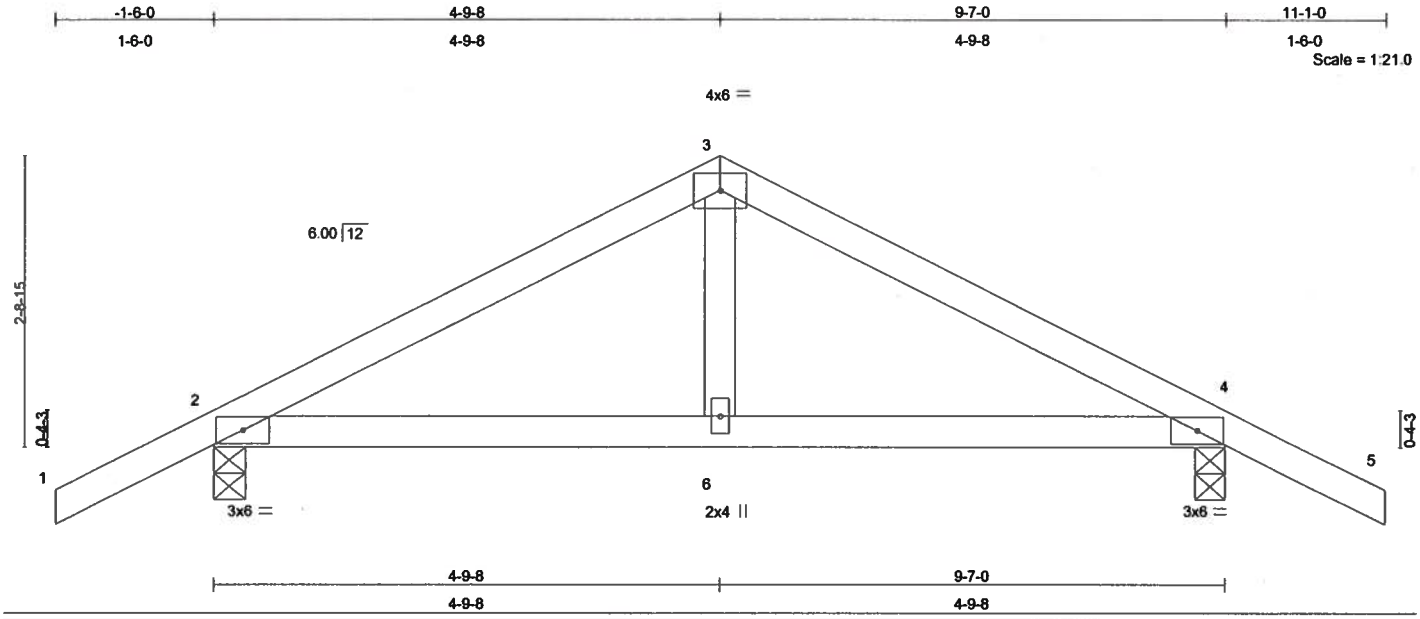

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	0.04	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.02	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.05	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 39 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-9 oc bracing.

REACTIONS (lb/size) 2=386/0-3-8, 4=386/0-3-8
Max Horz 2=-55(load case 7)
Max Uplift 2=-257(load case 6), 4=-257(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-400/601, 3-4=-400/601, 4-5=0/35
BOT CHORD 2-6=-399/305, 4-6=-399/305
WEBS 3-6=-284/157

JOINT STRESS INDEX
2 = 0.34, 3 = 0.57, 4 = 0.34 and 6 = 0.11

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876074
L250193	T24	COMMON	4	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 257 lb uplift at joint 2 and 257 lb uplift at joint 4.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T24G	GABLE	1	1	J1876075
					Job Reference (optional)

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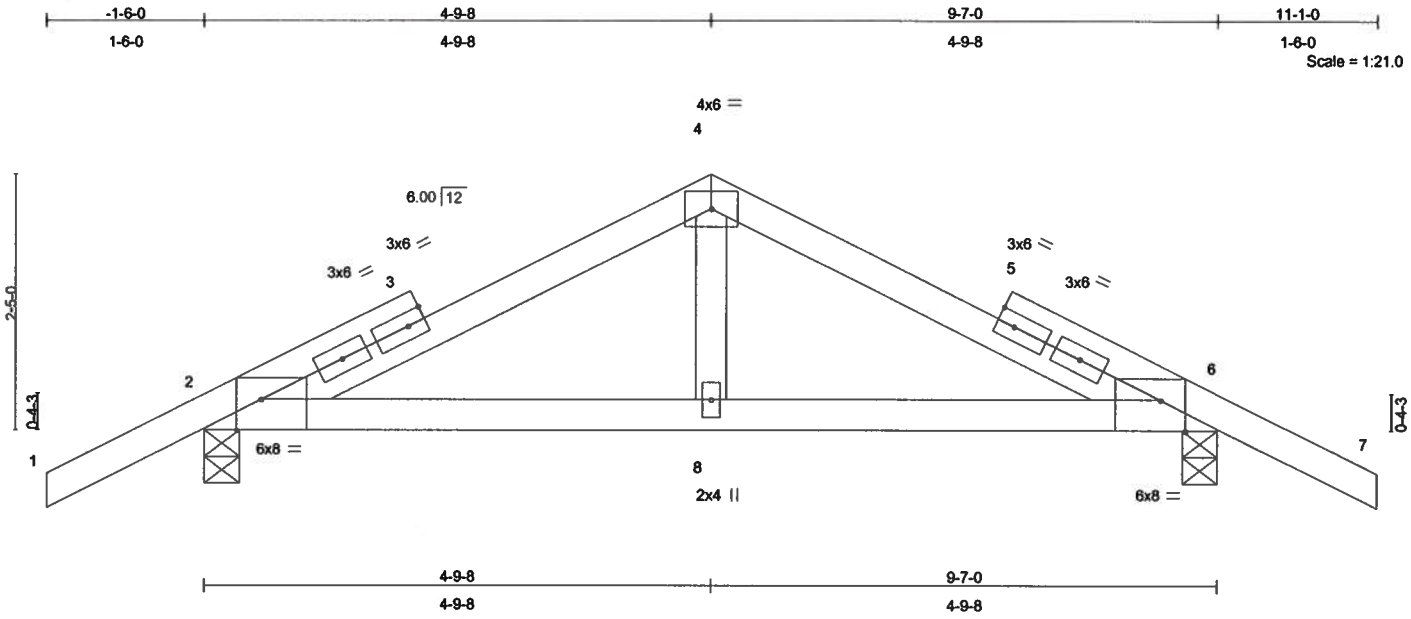

Plate Offsets (X,Y): [2:0-2-13,Edge], [6:0-2-13,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.05	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.03	2-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.05	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-10-14 oc bracing.

REACTIONS (lb/size) 2=594/0-4-0, 6=594/0-4-0
Max Horz 2=-57(load case 7)
Max Uplift 2=-447(load case 6), 6=-447(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-10/57, 2-3=-654/807, 3-4=-593/798, 4-5=-593/798, 5-6=-654/807, 6-7=-10/57
BOT CHORD 2-8=-615/530, 6-8=-615/530
WEBS 4-8=-286/156

JOINT STRESS INDEX
2 = 0.54, 3 = 0.00, 3 = 0.35, 3 = 0.35, 4 = 0.80, 5 = 0.00, 5 = 0.35, 5 = 0.35, 6 = 0.54 and 8 = 0.11

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T24G	GABLE	1	1	J1876075 Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 447 lb uplift at joint 2 and 447 lb uplift at joint 6.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) Gable truss supports 1' 4" max. rake gable overhang.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-87(F=-33), 4-7=-87(F=-33), 2-6=-10

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T25	COMMON	1	1	J1876076
					Job Reference (optional)

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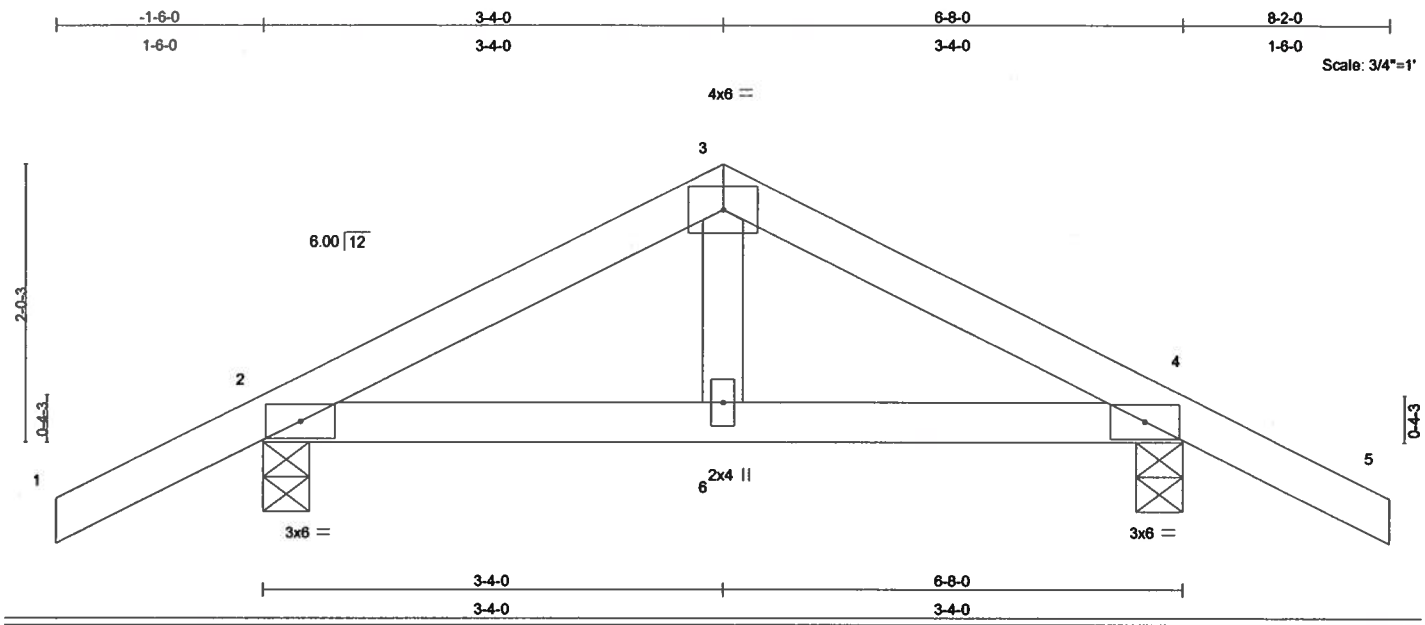

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.00	4-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	4-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 28 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=293/0-4-0, 4=293/0-4-0
Max Horz 2=46(load case 6)
Max Uplift 2=-124(load case 6), 4=-124(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-221/110, 3-4=-221/110, 4-5=0/35
BOT CHORD 2-6=0/149, 4-6=0/149
WEBS 3-6=0/116

JOINT STRESS INDEX
2 = 0.15, 3 = 0.05, 4 = 0.15 and 6 = 0.08

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 2 and 124 lb uplift at joint 4.

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Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876076
L250193	T25	COMMON	1	1	Job Reference (optional)

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T25A	COMMON	1	1	J1876077
					Job Reference (optional)

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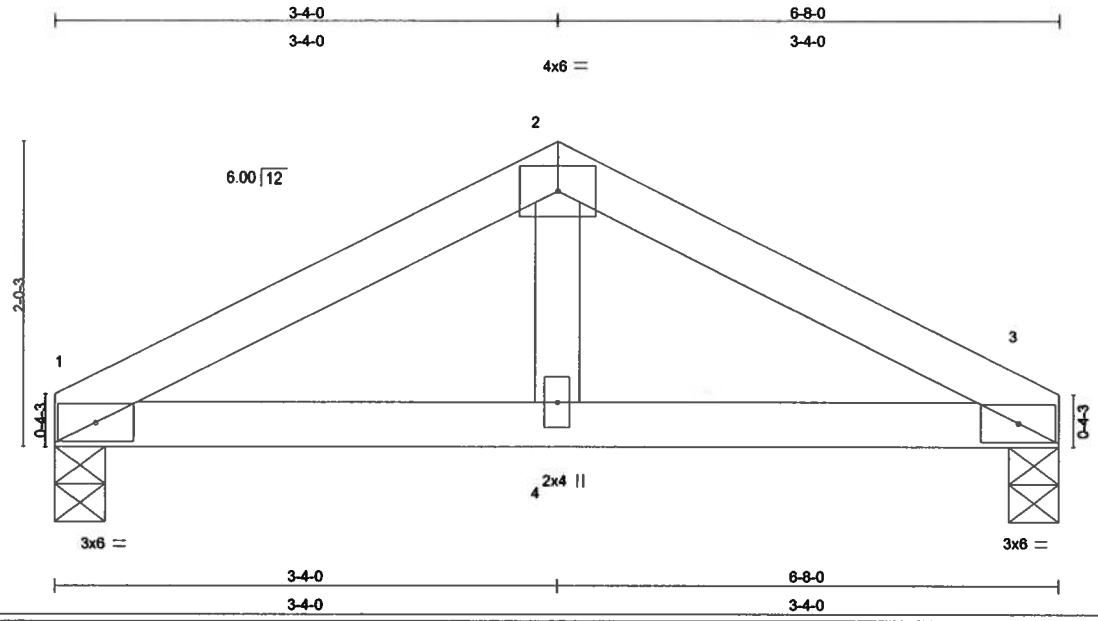

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.01	3-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.02	3-4	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.10	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 24 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=439/0-4-0, 3=439/0-4-0
Max Horz 1=-22(load case 3)
Max Uplift 1=-109(load case 5), 3=-109(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-556/129, 2-3=-556/129
BOT CHORD 1-4=-92/461, 3-4=-92/461
WEBS 2-4=-57/326

JOINT STRESS INDEX
1 = 0.24, 2 = 0.15, 3 = 0.24 and 4 = 0.23

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 109 lb uplift at joint 1 and 109 lb uplift at joint 3.
6) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 6-8-0

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876077
L250193	T25A	COMMON	1	1	Job Reference (optional)

NOTES

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54, 2-3=-54, 1-3=-85(B=-75)

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T25G	GABLE	1	1	J1876078
					Job Reference (optional)

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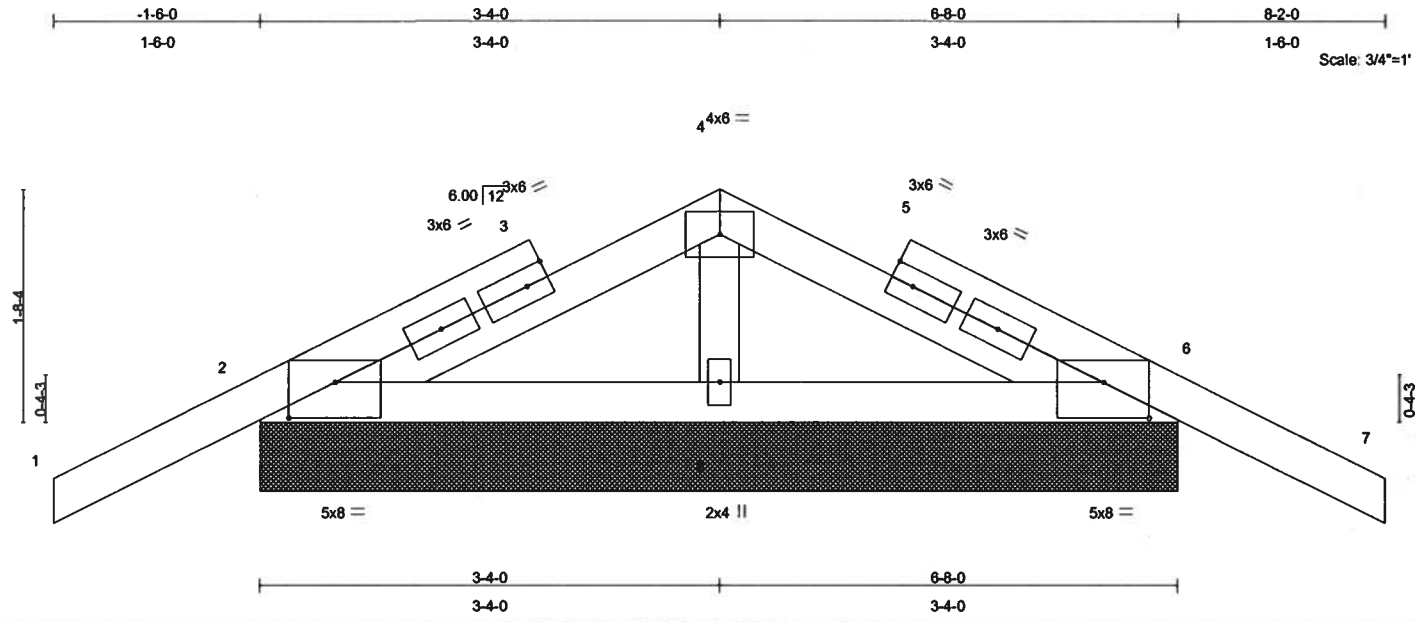


Plate Offsets (X,Y): [2:0-4-0,0-3-1], [6:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.01	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	7	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-8-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=284/6-8-0, 6=285/6-8-0, 8=339/6-8-0
Max Horz 2=47(load case 6)
Max Uplift 2=-183(load case 6), 6=-191(load case 7), 8=-92(load case 6)
Max Grav 2=292(load case 10), 6=292(load case 11), 8=339(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-12/56, 2-3=-35/85, 3-4=-42/125, 4-5=-43/125, 5-6=-35/85, 6-7=-12/56
BOT CHORD 2-8=-79/141, 6-8=-79/141
WEBS 4-8=-297/233

JOINT STRESS INDEX
2 = 0.68, 3 = 0.00, 3 = 0.30, 3 = 0.30, 4 = 0.09, 5 = 0.00, 5 = 0.30, 5 = 0.30, 6 = 0.68 and 8 = 0.13

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34985
1100 Coastal Bay Blvd
Boynton Beach, FL 33436

August 10,2007

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
					J1876078
L250193	T25G	GABLE	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 183 lb uplift at joint 2, 191 lb uplift at joint 6 and 92 lb uplift at joint 8.
9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 6.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

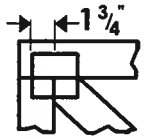
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-87(F=-33), 4-7=-87(F=-33), 2-6=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33436

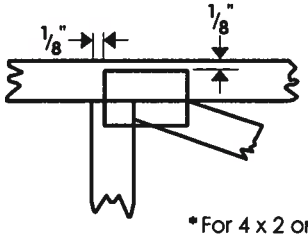
August 10,2007

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

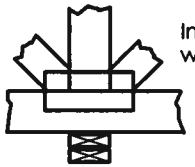
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

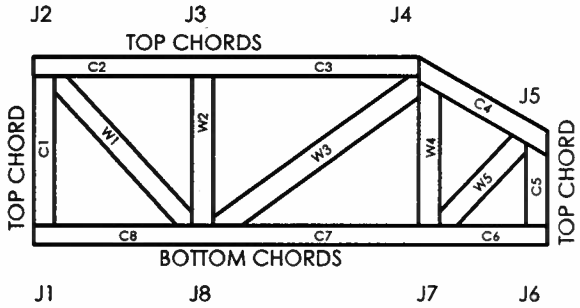
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

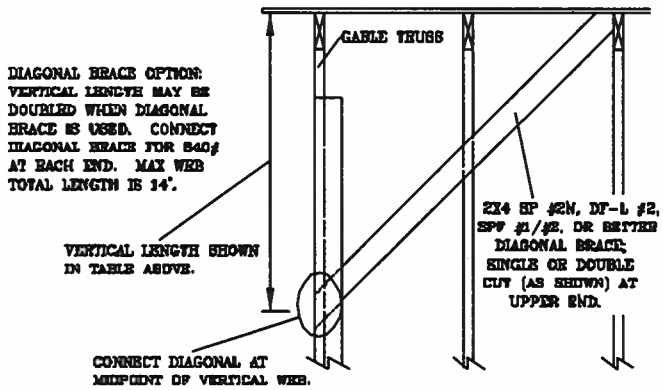
Failure to Follow Could Cause Property Damage or Personal Injury

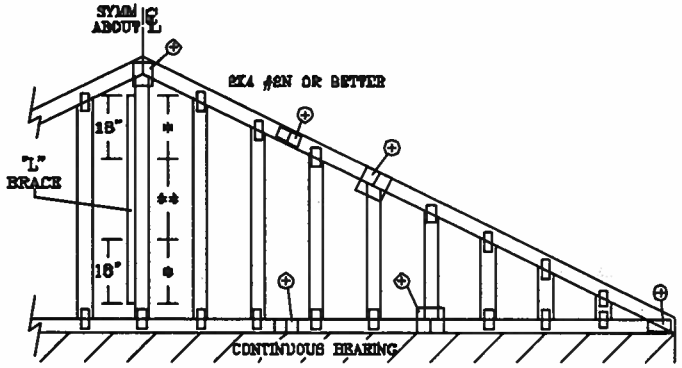
1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	5' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 5"	11' 5"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	5' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 6"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	6' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	8' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 5"	7' 4"	7' 7"	8' 9"	8' 11"	10' 6"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	6' 9"	6' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	15' 6"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 11"	5' 11"	7' 10"	7' 10"	10' 5"	10' 5"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR, SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-02-GAB13015

DATE 11/26/03

DRWG MITEK STD GABLE 15 E HT

-ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	5' 6"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	5' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 6"	5' 8"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 5"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 8"
		STUD	3' 3"	4' 6"	4' 8"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 8"
		STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 6"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 6"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 6"	14' 0"	14' 0"
		STUD	3' 8"	5' 6"	5' 8"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 8"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 8"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 5"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 6"	5' 8"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

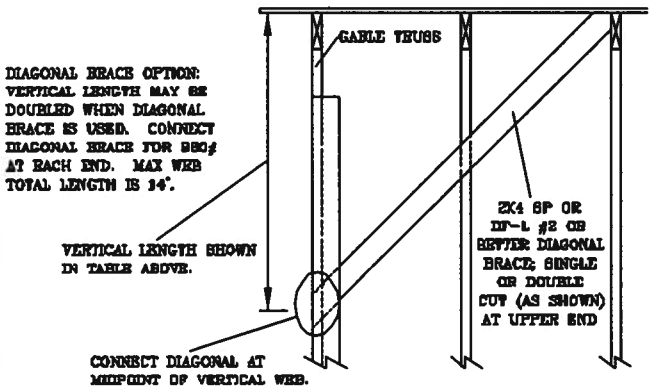
SOUTHERN PINE
#3
STUD
STANDARD

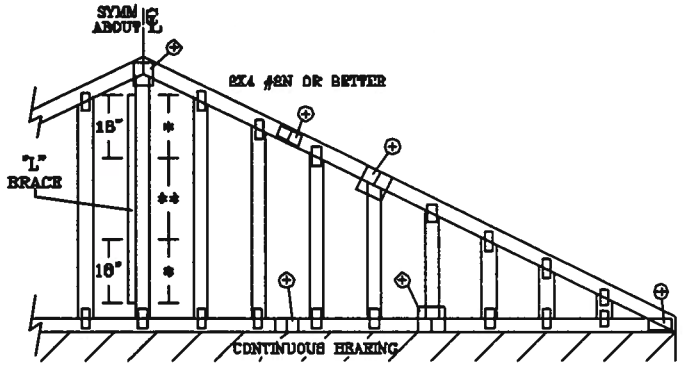
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#2
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	MD SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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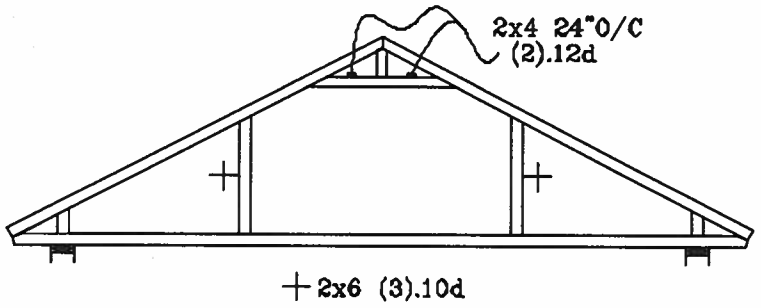
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

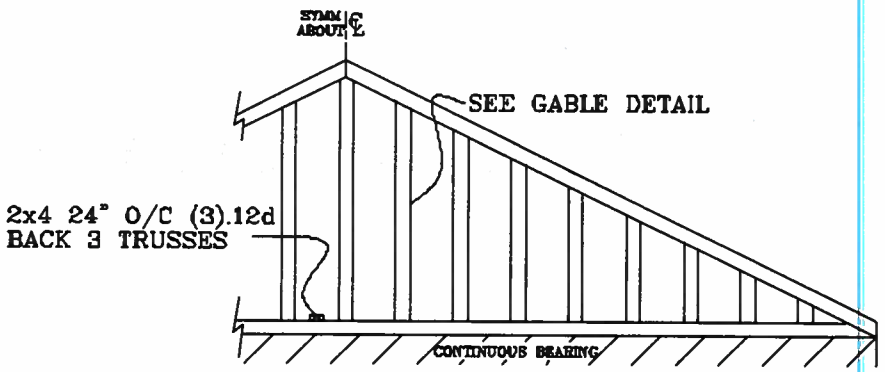
MAX. SPACING 24.0"

REF ASCE7-02-GAB13090
DATE 11/26/03
DWG MTEK STD GABLE 30' E W7
-ENG

TYPICAL ATTIC TRUSS BRACING

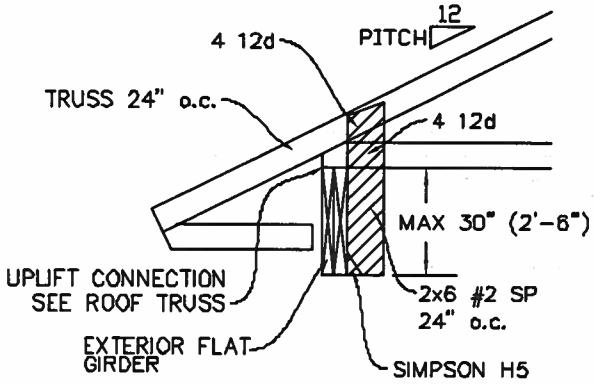


GABLE END TRUSS DETAIL

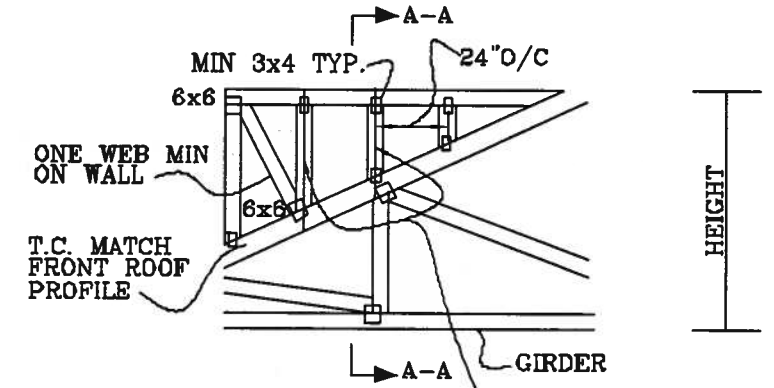


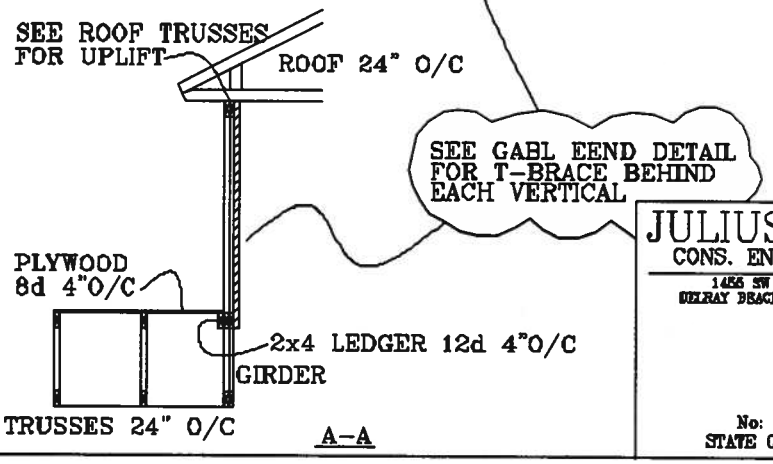
MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL





JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

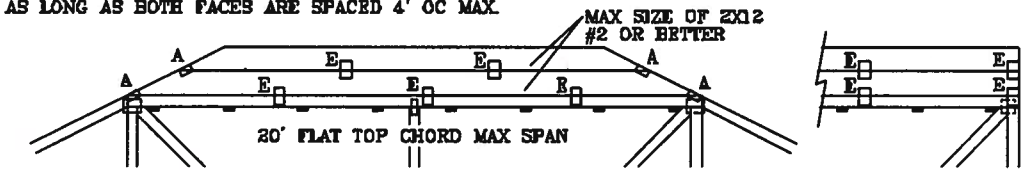
ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

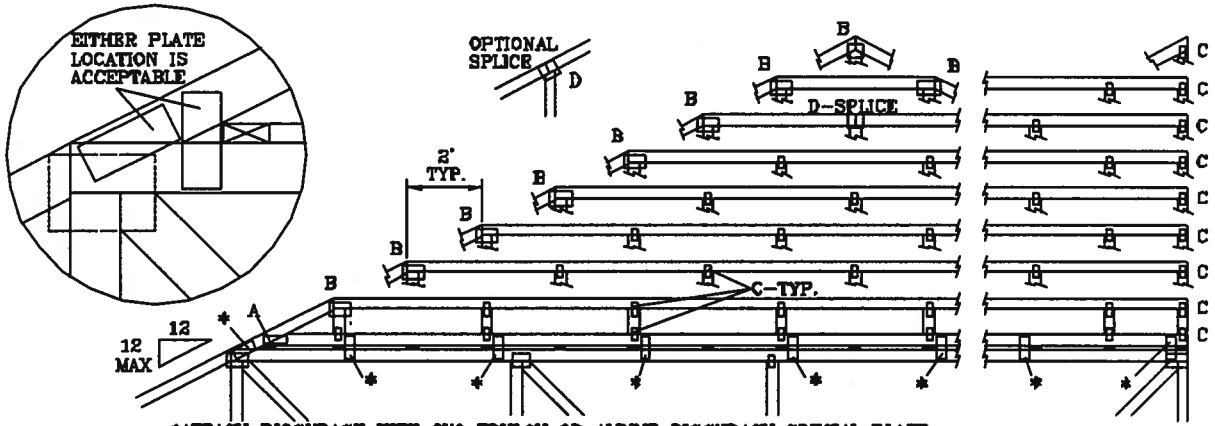
REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

- 110 MPH WIND, 30' MEAN HGT, ASCE 7-93, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF
- 110 MPH WIND, 30' MEAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF
- 130 MPH WIND, 30' MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.





*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

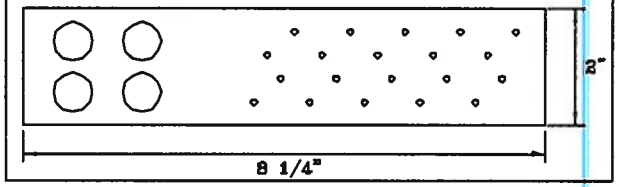
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X8	5X8	5X8	5X8
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X8 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 11/26/03
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR SBC 110 MPH, ASCE 7-93 110 MPH WIND OR (3) 16d FOR ASCE 7-98 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

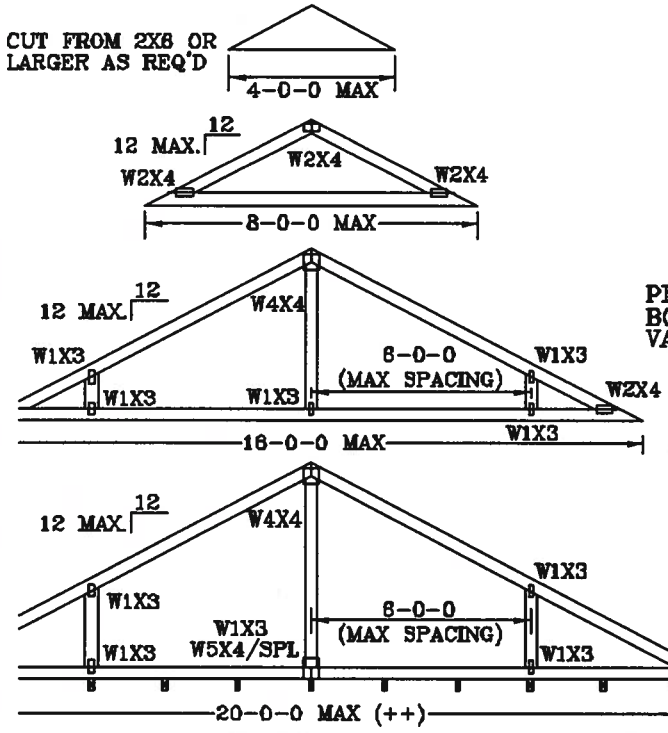
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

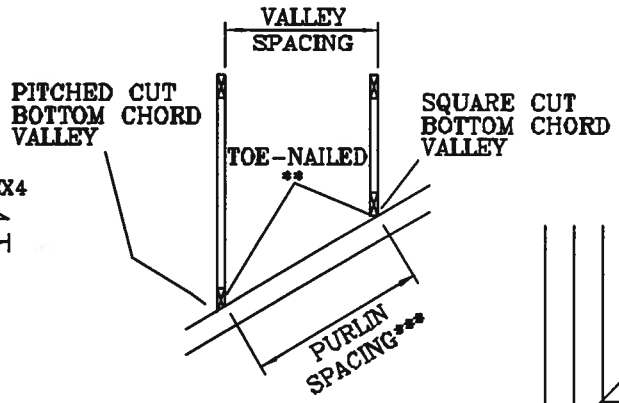
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

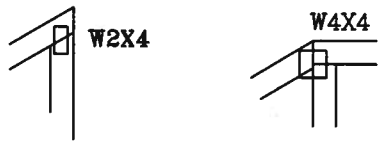
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

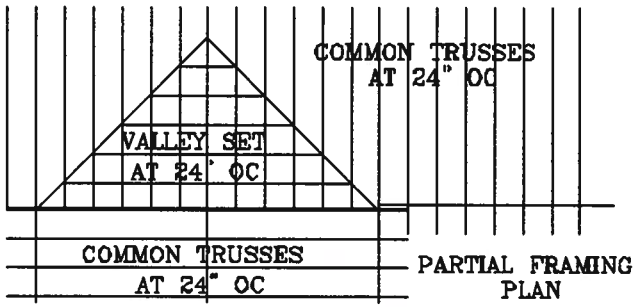
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.







THIS DRAWING REPLACES DRAWING A105

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR.; SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DURFAC.	1.25	1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-1997 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

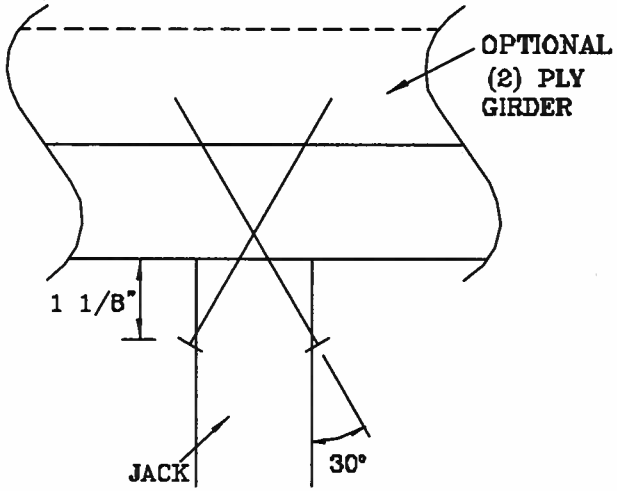
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

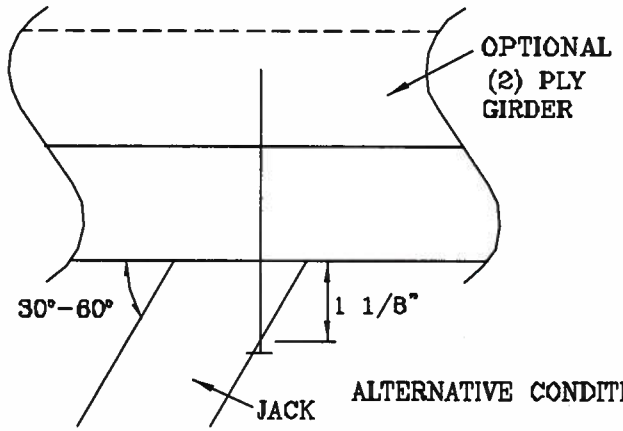
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM LATERAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1450 SW 4th AVENUE
DELRAY BEACH, FL 33444-2181

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

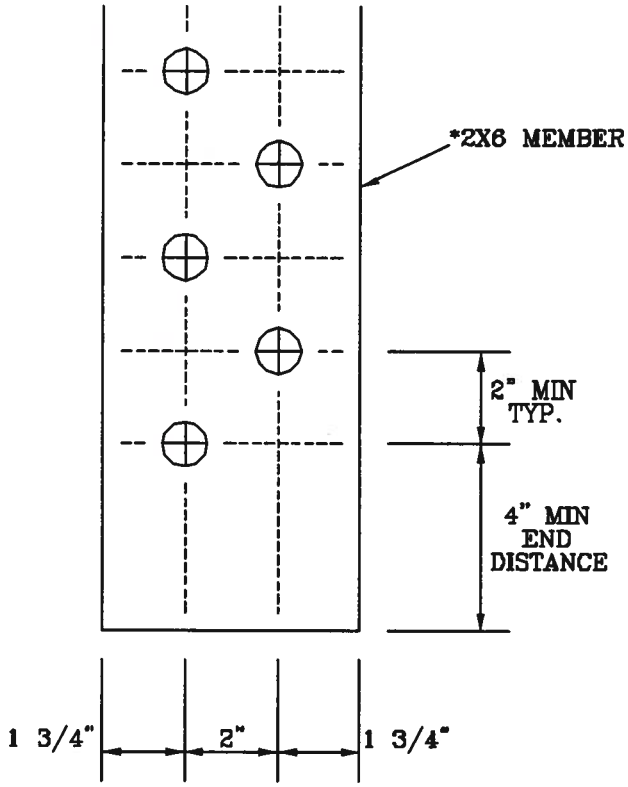
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

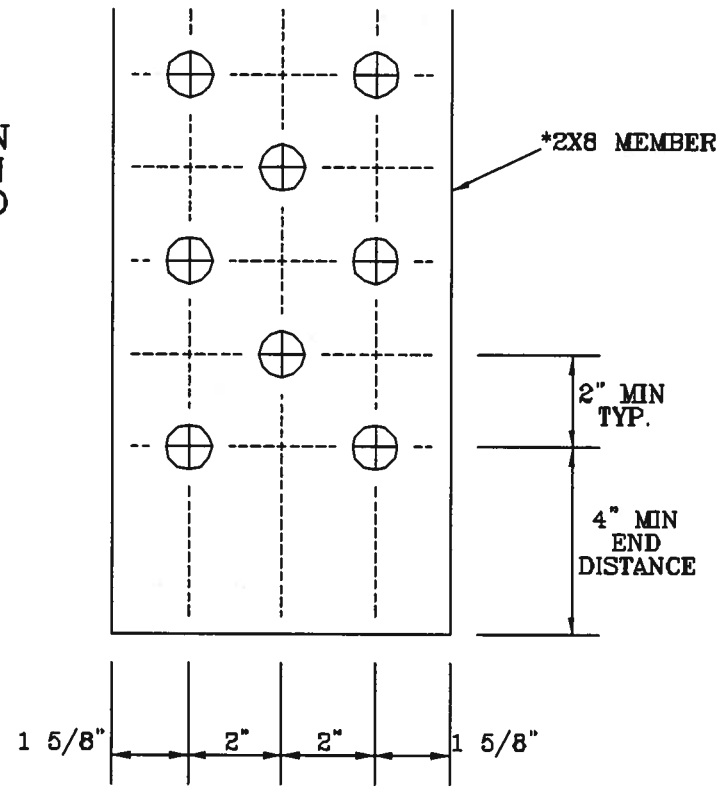
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
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1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSP1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

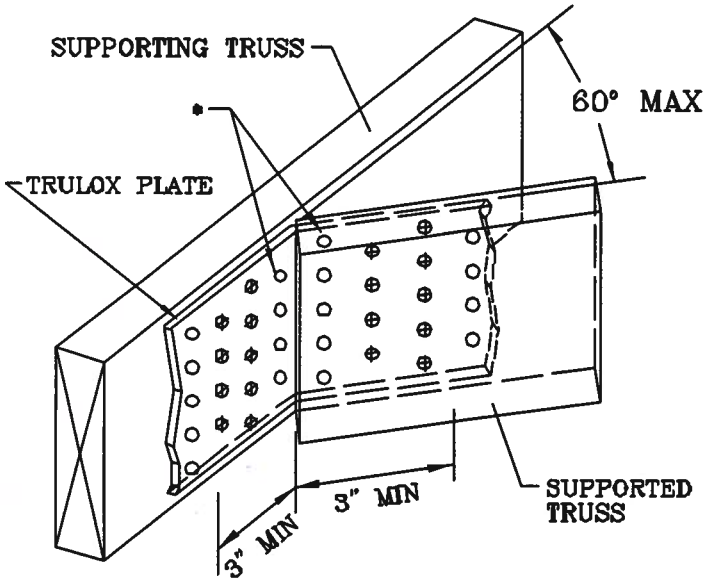
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

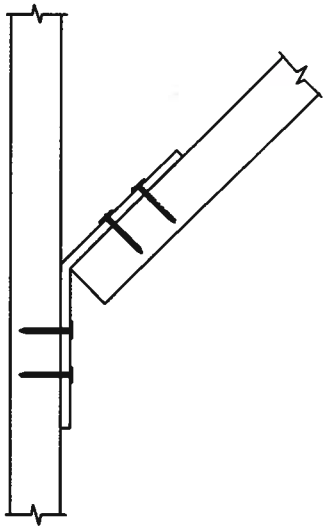
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

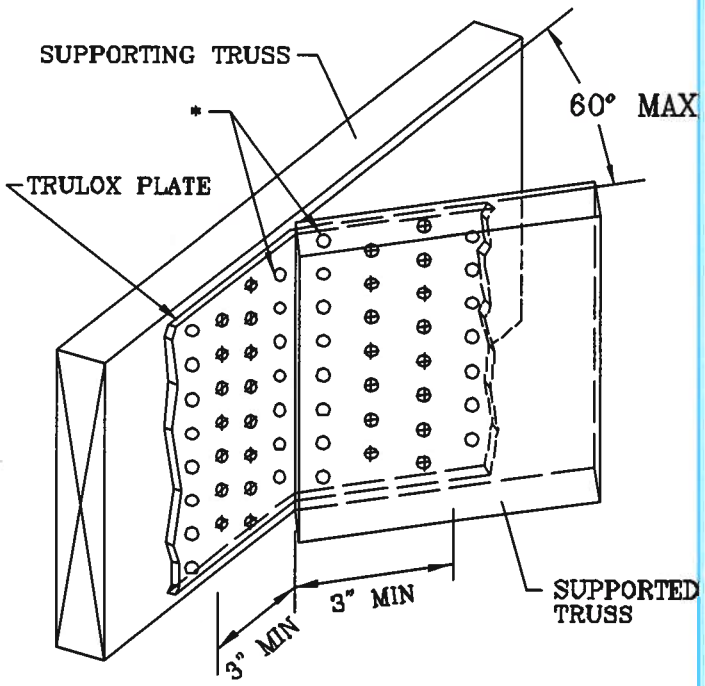
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

==WARNING== TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI) (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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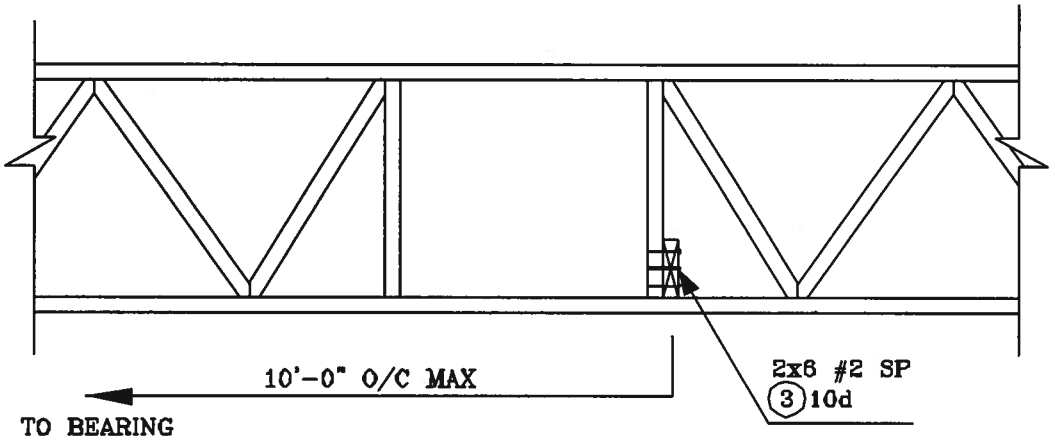
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

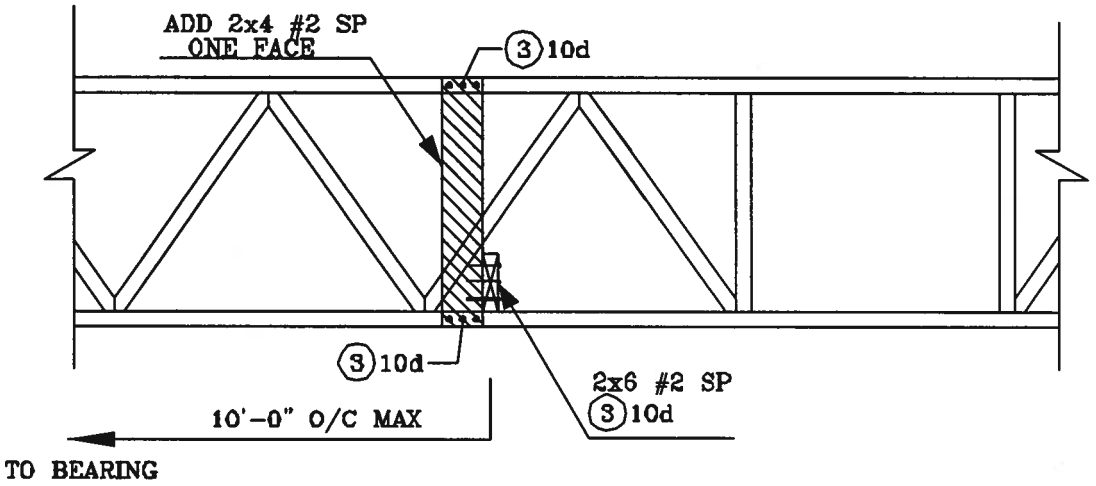
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



JULIUS LEE'S
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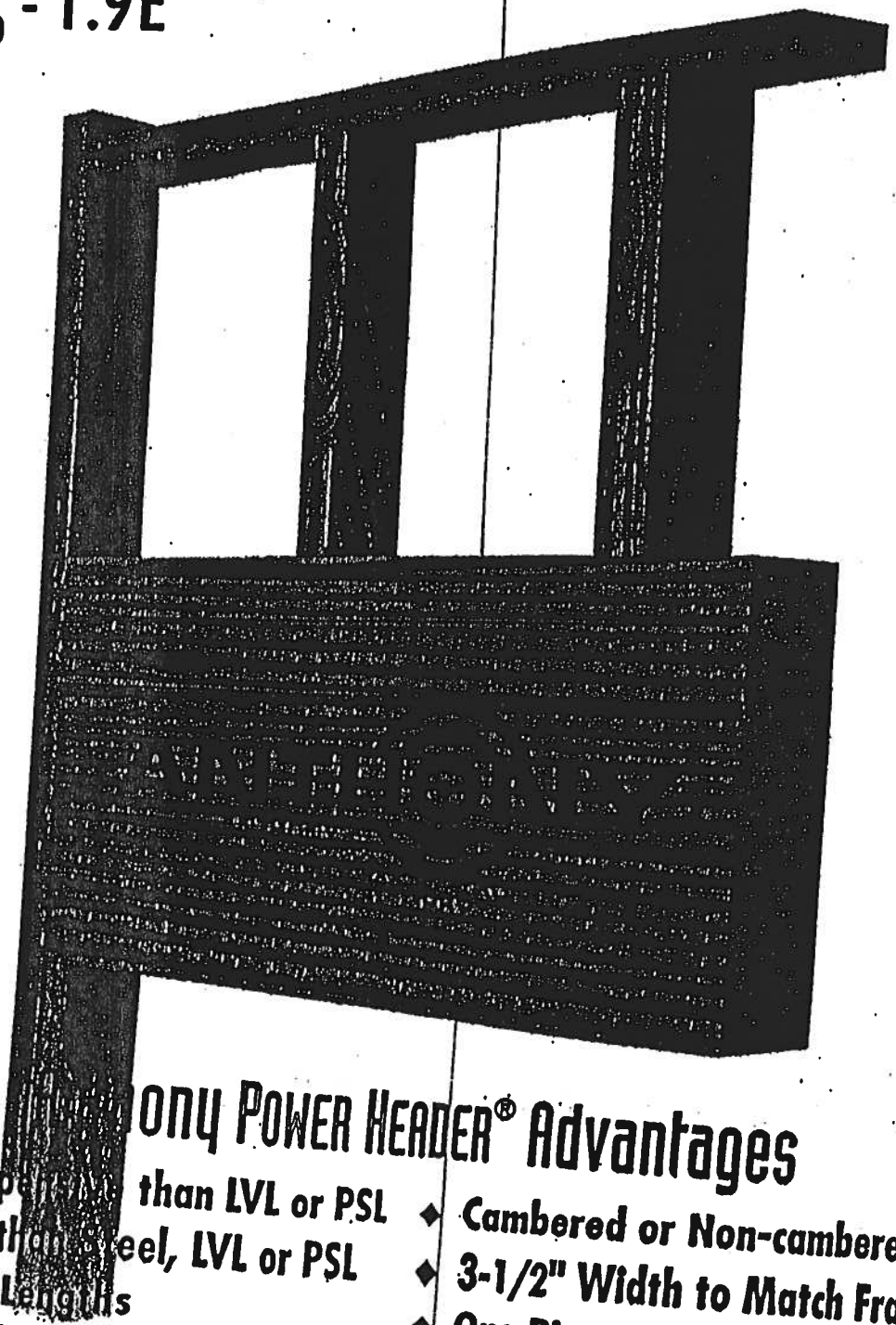
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

Anthony POWER HEADER®

2600F_b - 1.9E



ony POWER HEADER® Advantages

- Less Exp[illegible] than LVL or PSL
- Lighter th[illegible]eel, LVL or PSL
- Pre-Cut Lengths
- Renewable Resource
- Cambered or Non-cambered
- 3-1/2" Width to Match Framing
- One Piece - No Nail Laminating
- Lifetime Warranty

Garage Header Sizing Tables

ANTHONY
ANTHONY FOREST PRODUCTS CO.

Anthony Power Header®

$26F_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER APPLICATION - SINGLE STORY

HEADER SUPPORTING: 1/2 ROOF SPAN

9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	16-3/4
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8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	16-3/4	9-3/4	15-3/8		9-3/4		
8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4			9-3/4		
8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4		
8-3/8	14	15-3/8	8-3/8	15-3/8		8-3/8	15-3/8		9-3/4			9-3/4			11-1/4		
8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4			11-1/4		

9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14
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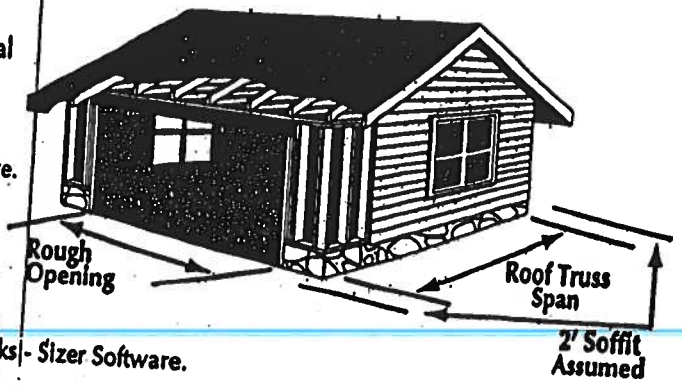
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	

NOTES:

1. Table assumes a simple span header supporting a uniform load transferred from 1/2 the roof span plus a 2' soffit.
2. Roof live and dead loads shown are applied vertically to the horizontal projection. No reductions in roof live loads or snow loads were considered. The header weight is accounted for in the table.
3. Deflection is limited to L/240 for live load and L/180 for total load.
4. Headers are assumed to have continuous lateral support along top edge.
5. Bearing length based on full width bearing is indicated as follows:
   Non-shaded sizes require two trimmers (3" bearing).
   Shaded sizes require three trimmers (4.5" bearing).
   Shaded & outlined sizes require four trimmers (6" bearing).
6. ** Applications where load carrying capacity of 16-3/4" depth has been exceeded. See AFP $30F_b$ POWER-BEAM® literature or AFP's WoodWorks - Sizer Software.



Anthony Power Header®

$26F_b$ - 1.9E

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS (F_b) =	2600
COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) =	740
HORIZONTAL SHEAR (F_v) =	225
MODULUS OF ELASTICITY (MOE) =	1.9×10^6

	7.7	9.0	10.4	11.7	12.9	14.2	15.5
	326	514	789	1115	1521	2014	2604
	8865	12015	15996	20145	24772	29877	35460
	3908	4550	5250	5892	6533	7175	7817

NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b, shall be modified by the Volume Factor, C_v, as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1997.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for Power Header®.

GARAGE HEADER COMPARISONS

810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-5/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
640 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-3/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"*
765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"*
750 / 480	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"*
900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	-----

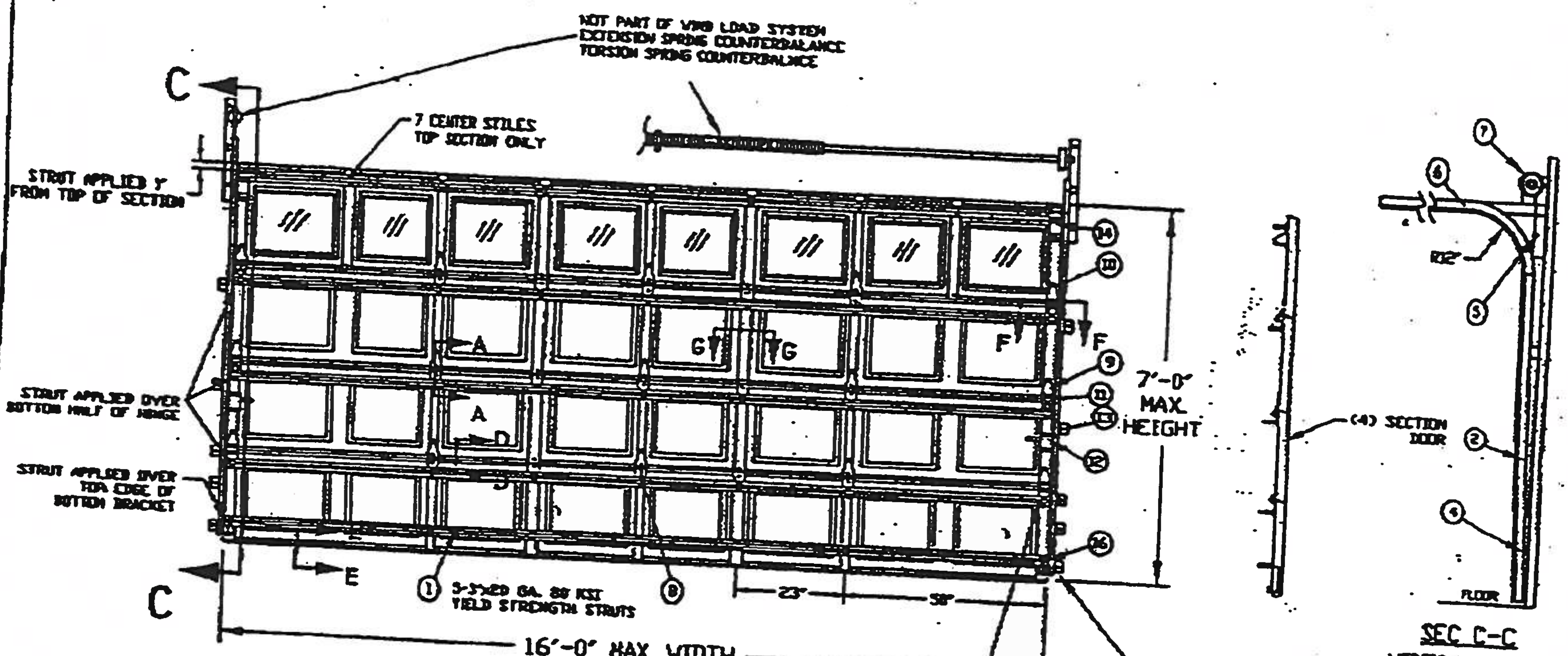
For more information on Power Header®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

Distributed by:

NL

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT= 21"
3. SECTION HEIGHTS OF 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION (AS TESTED WITH 1/8" DSB GLASS OR EQUIVALENT) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MINIMUM LENGTH OF ROLLER STEM IS 5½" (7" AS TESTED)
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOWN.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE 0,1, OR 2 AS TESTED.
△ 9. DROP IN TYPE OF INSULATION IS OPTIONAL.

REV.	DATE	BY	DESCRIPTION
A–A	11-10-00	DW	SEE ECN. 831



INSIDE ELEVATION

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.

NORTH CAROLINA PROFESSIONAL ENGINEER SEAL PE No. 024280

LISTED SBCCI PST&ESI REPORT No. 2202

TEST REPORTS ON FILE: VIDEO 10/19/00 400293

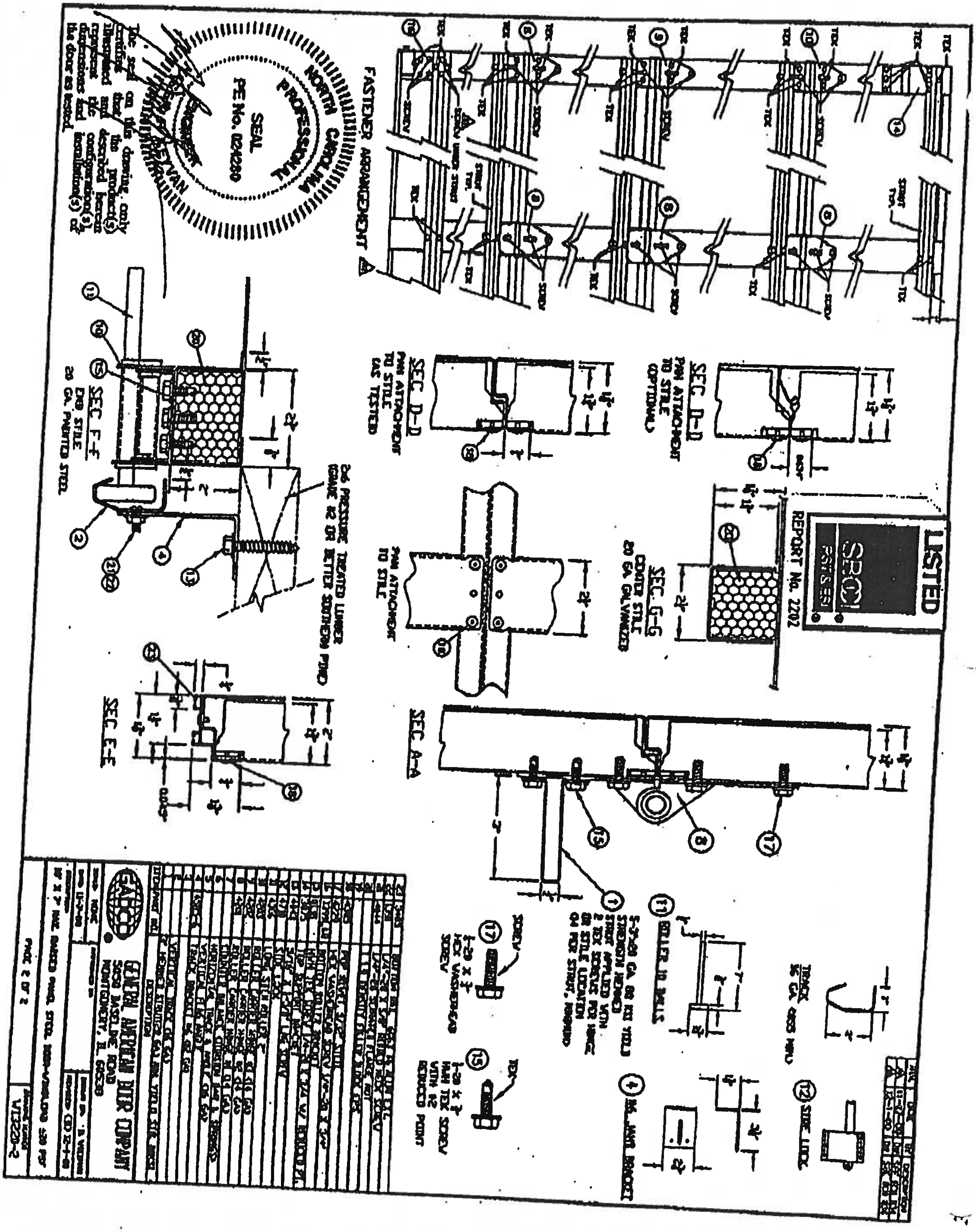
GADCO DOORS
SERIES 7400, EXTERIOR STEEL =.017 MIN (AS TESTED)
SERIES 7825, EXTERIOR STEEL =.019" MIN △
SERIES 7324, EXTERIOR STEEL =.024" MIN △
(TESTED WITH WINDOWS)

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CTR. STILE SPACING	STRUTS 80 KSI SIZE	STRUTS 80 KSI QTY.	VERTICAL TRACK
16'	7'	23"	3"	5	2 IN.

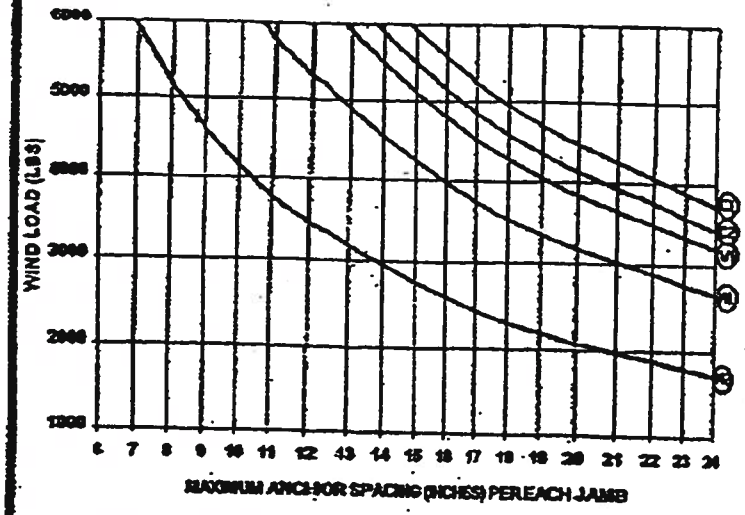
DESIGN LOAD +20.0 PSF & -20.0 PSF
TEST LOAD +30.0 PSF & -30.0 PSF

GADCO
GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

DATE 10-20-00 | DRAWN BY: D. WIEDMAN | REVISED (A) 11-10-00
DESCRIPTION: 16' X 7' MAX. RAISED PANEL STEEL DOOR -W/WINDLOAD ±20 PSF
PAGE 1 OF 2
DRAWING NUMBER V13220-1


FASTENER ARRANGEMENT
NORTH CAROLINA
PROFESSIONAL
SEAL
PE No. 024280
LISTED
REPORT No. 2202
SEC D-D
SEC G-G
SEC F-F
SEC E-E
SEC A-A
2x6 PRESSURE TREATED LUMBER
(GRADE #2 OR BETTER SOUTHERN PINE)
20 GA. PAINTED STEEL
20 GA. GALVANIZED
ROLLER IN SHELLS
SIDE LOCK
GENERAL AMERICAN DOOR COMPANY
MONTGOMERY, IL
VT3220-2

WIND LOAD vs ANCHOR SPACING



(I) CONCRETE MSOG: HILTI KWIK BOLT II EXPANSION ANCHOR 3/8" DIA. 1-5/8" EMBEDMENT

(II) CONCRETE MSOG: RAWL LOK/BOLT SLEEVE ANCHOR 3/8" DIA. 1-5/8" EMBEDMENT

(III) MASONRY/CMU MSOG: RAWL LOK/BOLT SLEEVE ANCHOR 3/8" DIA. 1-3/8" EMBEDMENT

(IV) MASONRY/CMU MSOG: ITW BUILDEX TAPCON MASONRY ANCHOR 1/4" DIA. 1-3/4" EMBEDMENT

(V) WOOD STUD MSOG: LAG SCREWS 5/16" DIA. 1-1/2" EMBEDMENT

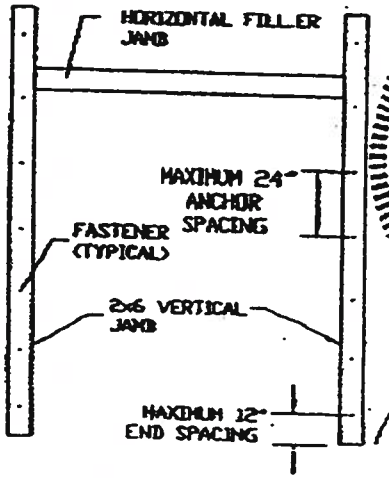
DESIGN LOAD (LBS/FT²) X GARAGE DOOR AREA(WIDTH-FT X HEIGHT-FT) = WIND LOAD(LBS)

EXAMPLE

30 LBS/FT² X 16 FT WIDE X 8 FT HIGH = 3840 LBS

(I) USE 22" SPACING (III) USE 16" SPACING
(II) USE 21" SPACING (IV) USE 10" SPACING
(V) USE 19" SPACING

SEE NOTE 10 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS



NORTH CAROLINA PROFESSIONAL SEAL PE No. 024280 ENGINEER NASER R KEYVAN

NASER R KEYVAN NC 24774 STATE OF FLORIDA PROFESSIONAL ENGINEER

3/26/2002

2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION" SSTD 10," CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE #2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.

5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2530 PSI; GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI; REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4"

8) LAG SCREWS SHALL BE CENTERED IN DIE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.

9) WASHERS ARE REQUIRED ON ALL FASTENERS.

10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.

11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

GENERAL AMERICAN DOOR COMPANY
5500 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: NONE	APPROVED BY:	DRAWN BY: DV
DATE: 9-30-99		ISSUED:

DESCRIPTION: JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS

DRAWING NUMBER: A10560

MI HOME PRODUCTS

MI Home Products, Inc.
650 West Market St.
P.O. Box 370
Gratz, PA 17030-0370

(717) 365-3300
(717) 362-7025 Fax

740/744 Single Hung (Fin & Flange)
165 Single Hung (Fin & Flange)
BB165/740/744 Fixed (Fin & Flange)

- Test Reports
  - 165 Single Hung
    - #CTLA-787W (Fin)
    - #CTLA-787W-1 (Flange)
  - 740/744 Single Hung
    - #01-40351.03 (Fin)
    - #01-40351.04 (Flange)
  - 165/740/744 Fixed
    - #NCTL-310-0005-2.1 (Fin)
    - # NCTL-310-0005-5.1 (Flange)
    - #01-40486.03 (2-Panel Fixed)
- Installation Instructions
- Sample 110/120/140 MPH Labels

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 740/744
TYPE: Aluminum Single Hung Window with Nail Fin

Title of Test	Results
Rating	H R45 52 x 72
Overall Design Pressure	45 psf
Operating Force	24 lb max.
Air Infiltration	0.10 cfm/ft^2
Water Resistance	6.75 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-40351.03 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:baw

Allen N. Reeves
15 February 2002

ALLEN N. REEVES
CERTIFICATE
NO. 19
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

THIS FENESTRATION PRODUCT COMPLIES* WITH THE

NEW FLORIDA BUILDING CODE

FOR RESIDENTIAL BUILDINGS WITH A MEAN ROOF HEIGHT OF 30 FT. OR LESS, *EXPOSURE "B"* (WHICH IS INLAND OF A LINE THAT IS 1500 FT. FROM THE COAST), AND *WALL ZONE "5"* (INSTALLED NEAR THE CORNER OF THE BUILDING).

PER *ASTM E1300*, THE CORRECT GLASS THICKNESS, BASED ON THE *NEGATIVE* DESIGN PRESSURE (DP) LISTED BELOW, HAS BEEN INSTALLED IN THIS UNIT. THE GLASS THICKNESS IS BASED ON ITS' WIDTH, HEIGHT, AND ASPECT RATIO.

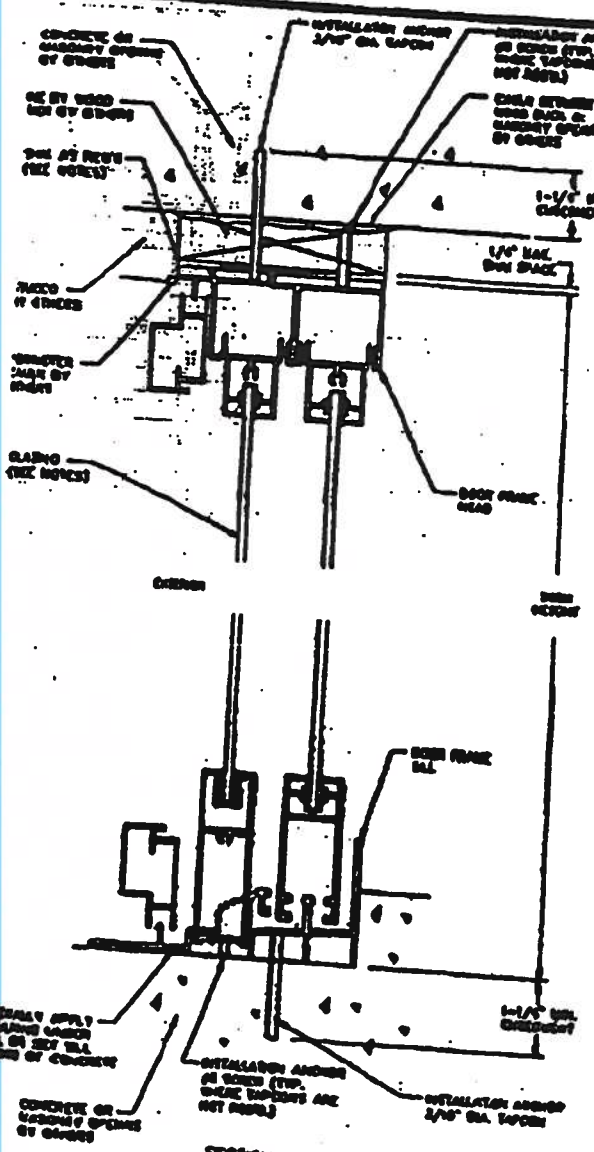
Series 470HP SLIDING GLASS DOOR – all 6'- 8" High Panels

- 2'- 6" WIDE DP +40.0 / -55.4
- 3'- 0" WIDE DP +40.0 / -48.5
- 4'- 0" WIDE DP +40.0 / -40.3

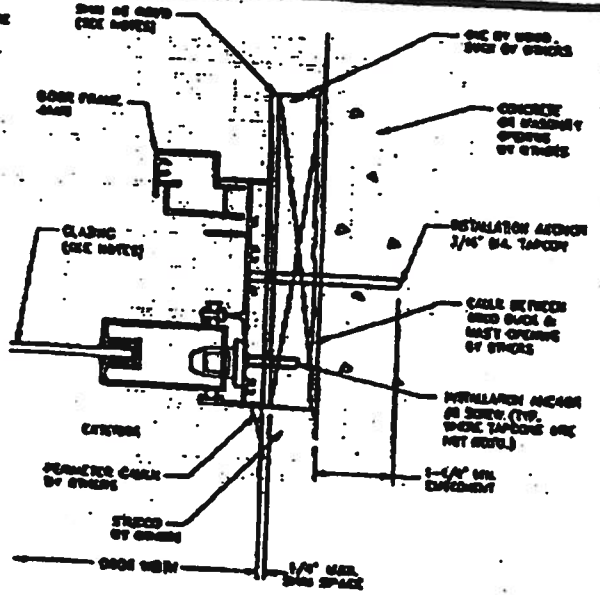
THIS PRODUCT MEETS THE REQUIREMENTS FOR STRUCTURAL LOADS, WATER AND AIR INFILTRATION PER ATTACHED *AAMA* PERFORMANCE LABEL. BE ADVISED THAT IF LOADS ARE PLACED UP TO OR EXCEEDING THE TESTED LEVELS, THIS PRODUCT MAY BE ALTERED IN SUCH A WAY THAT FUTURE PERFORMANCE WILL BE REDUCED.

* COMPLIANCE MUST INCLUDE INSTALLATION ACCORDING TO MANUFACTURER'S INSTRUCTIONS AND FLORIDA CODE REQUIREMENTS.

MIP-686

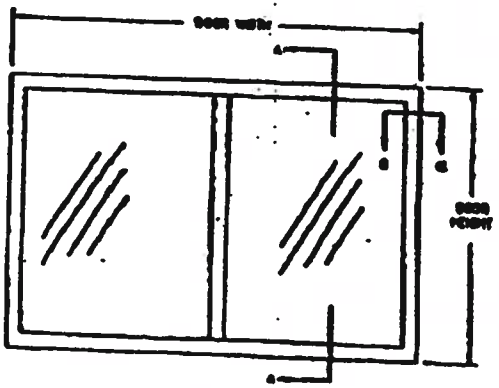


SECTION A-A



SECTION B-B

TAPCON INSTALLATION CHART					
CALL SIZE	DOOR SIZE				



EXTERIOR ELEVATION

MI HOME PRODUCTS
GRATZ, PA.

SERIES 470 SLIDING GLASS DOOR
INSTALLATION WITH TAPCONS

MANUAL MAINTENANCE		DR	1/28/02
CIVIL		N.T.S.	
47182			1 OF 1

TOTAL P.04

DOCUMENT CONTROL ADDENDUM #01-40351.00

Current Issue Date: 02/15/02

Report No.: 01-40351.01

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 744 aluminum single hung window with flange.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Report No.: 01-40351.02

Requested by: William Emley, MI Home Products, Inc.
Purpose: Change of glass type.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories.

Report No.: 01-40351.03

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 740/744 aluminum single hung window with nail fin.
Issued Date: 02/15/02
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

ALLEN N. REEVES
CERTIFICATE
NO. 13354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
15 FEBRUARY 2002

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meting rail) (Loads were held for 52 seconds)		
	@ 45.0 psf (positive)	0.91"*	0.29" max.
	@ 45.0 psf (negative)	0.97"*	0.29" max.
4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads held for 10 seconds)		
	@ 67.5 psf (positive)	0.14"	0.20" max.
	@ 67.5 psf (negative)	0.19"	0.20" max.
4.4.2	@ 70.8 psf (negative)	0.20"	0.20" max.

** Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:baw
01-40351.03

Allen N. Reeves, P.E.
Director - Engineering Services
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into the #2 2 x 8 Spruce-Pine-Fir wood buck with 1" galvanized roofing nails through the nail fin every 8" on center. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	24 lbs	30 lbs max.
2.1.2	Air Infiltration (ASTM E 283) @ 1.57 psf (25 mph)	0.10 cfm/ft^2	0.30 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-96) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.86"*	0.29" max.
	@ 15.0 psf (negative)	0.81"*	0.29" max.

*Note: * Exceeds L/175 for deflection, but meets all other test requirements.*

2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.20" max.
	@ 22.5 psf (negative)	<0.01"	0.20" max.
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction at 70 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.03"/6%	0.50"/100%
	Right stile	0.03"/6%	0.50"/100%

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES CERTIFICATE NO. 19354 STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.330" high by 0.187" backed polypile with center fin	1 Row	Fixed meeting rail interlock
0.170" high by 0.187" backed polypile with center fin	1 Row	Fixed lite, stiles and top rail
3/8" diameter hollow bulb gasket	1 Row	Bottom rail
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash stiles
0.150" high by 0.187" wide polypile	1 Row	Active sash stiles

Frame Construction: All frame members were constructed of extruded aluminum with coped, butted and sealed corners fastened with two screws each. Fixed meeting rail was secured utilizing one screw in each end directly through exterior face into jamb. Silicone was utilized around exterior meeting rail/jamb joinery.

Sash Construction: All sash members were constructed of extruded aluminum with coped and butted corners fastened with one screw each.

Screen Construction: The screen frame was constructed from roll-formed aluminum members with plastic keyed corners. The screening consisted of a fiberglass mesh and was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Plastic tilt latch	2	One each end of the interior Meeting rail
Metal sweep lock	2	13" from meeting rail ends
Balance assembly	2	One per jamb
Screen tension spring	2	One per end of screen stile
Tilt pin	2	One each end of bottom rail

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
NO. 18356
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-40351.03
Test Dates: 10/22/01
And: 10/23/01
Report Date: 02/15/02
Expiration Date: 10/23/05

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness performance testing on a Series/Model 740/744, aluminum single hung window at MI Home Products, Inc.'s test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R45 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 740/744

Type: Aluminum Single Hung Window With Nail Fin

Overall Size: 4' 4-1/8" wide by 5' 11-5/8" high

Active Sash Size: 4' 2-3/4" wide by 2' 11-5/8" high

Fixed Daylight Opening Size: 4' 1-1/8" wide by 2' 9" high

Screen Size: 4' 1-7/8" wide by 2' 11-5/16" high

Finish: All aluminum was polished.

Glazing Details: The active sash and fixed lite were glazed with one sheet of 1/8" thick clear tempered glass. Each sash was channel glazed using a flexible vinyl gasket.

ALLEN M. REEVES
CERTIFICATE
NO. 19354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen M. Reeves

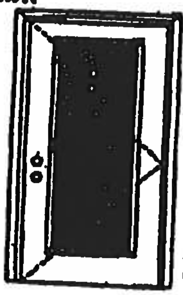
130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.testati.com

X
Glazed Inswing Unit

COP-WL-EN4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+50.5/-60.5
[illegible] special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

Test Data Review Certificate [illegible] and COP/Test Report Validation Matrix [illegible] provides additional information - available from the ITS/WHI website [illegible], the Masonite website [illegible] or the Masonite technical center.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:
1/4 GLASS:

100 Series | 123, 135 Series | 125 Series | 580 Series | 522 Series

1/2 GLASS:

105 Series* | 108, 109 Series* | 130 Series* | 300 Series* | 12 R.L., 26 R.L., 34 R.L. Series* | 107 Series* | 108 Series | 204 Series

*This glass kit may also be used in the following door styles: 6-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 6-panel with scroll.

Entergy™
Entry Systems
June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOOR Entry Systems Premium Quality Doors
Exclusively from Masonite®
Masonite International Corporation

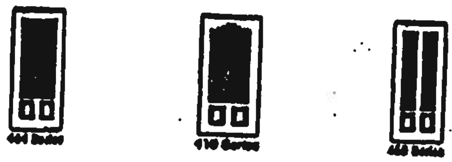
X
Glazed Inswing Unit

COP WL FH4141-02

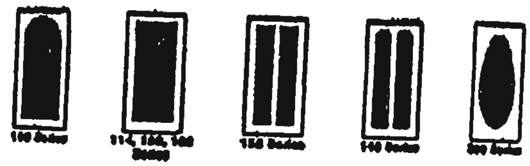
WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:



FULL GLASS:



CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 25-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Entergy
Entry Systems

June 17, 2002

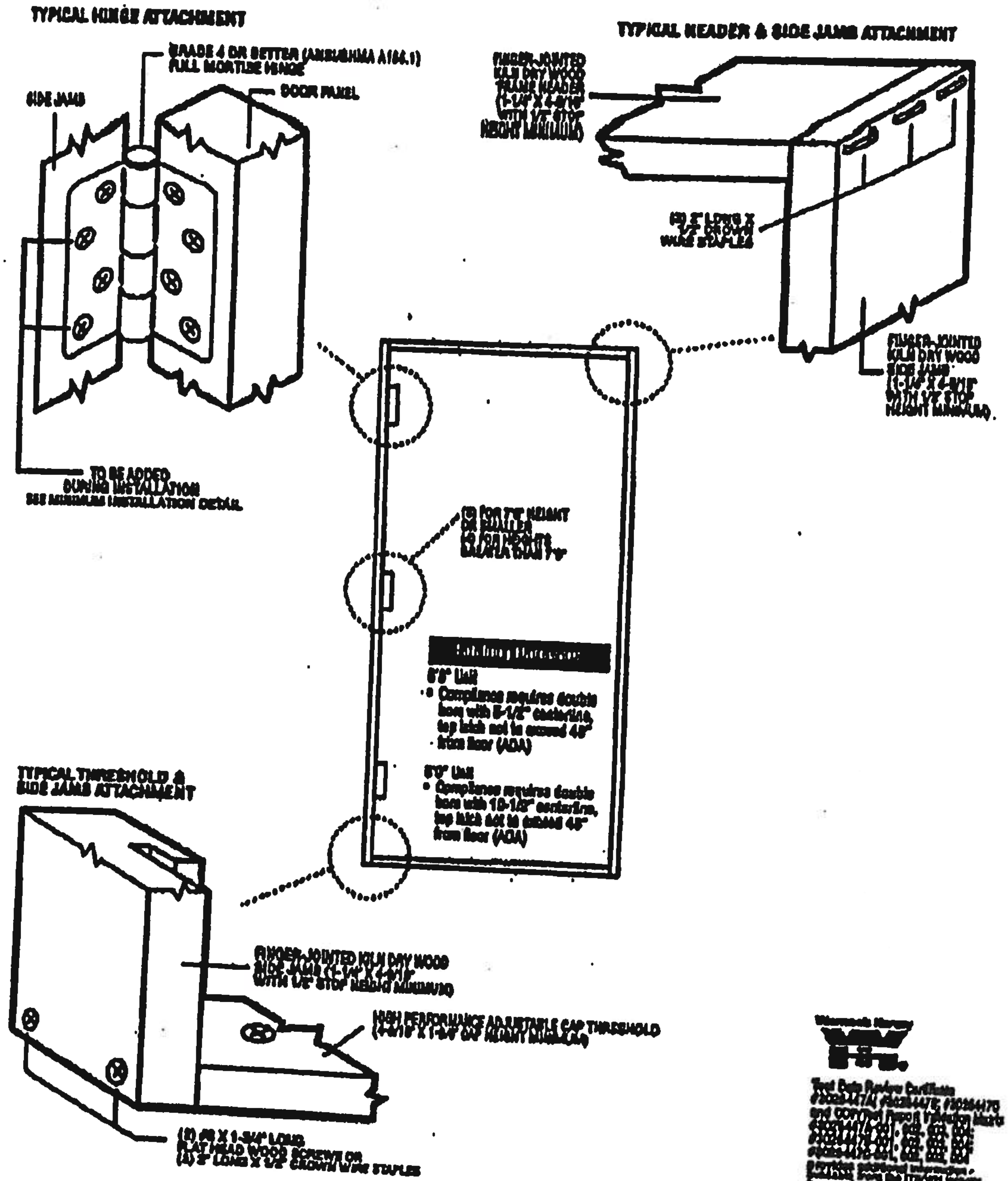
PREMDOOR Premium Quality Doors

Masonite®
Masonite International Corporation

X
Unit

MAD-WI-MA0001-02

INSWING UNIT WITH SINGLE DOOR

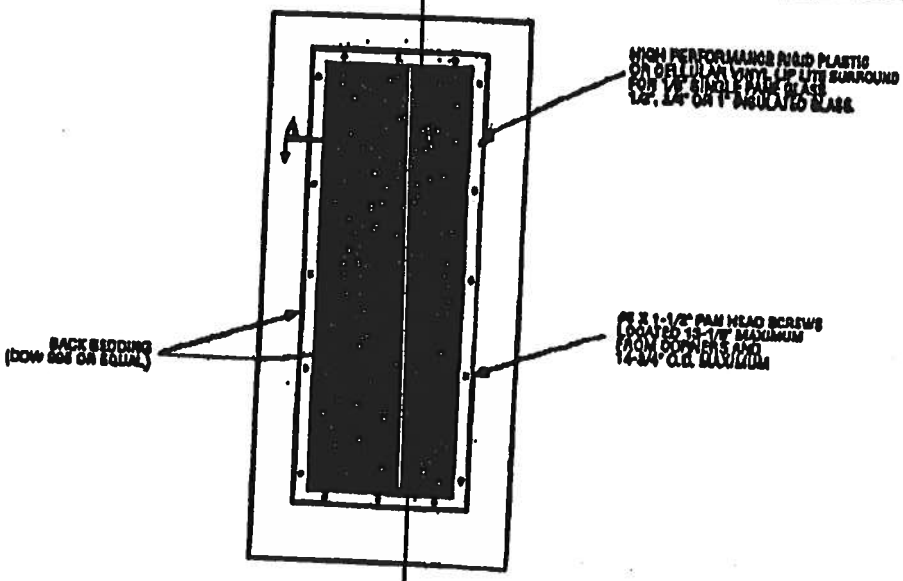


October 14, 2003

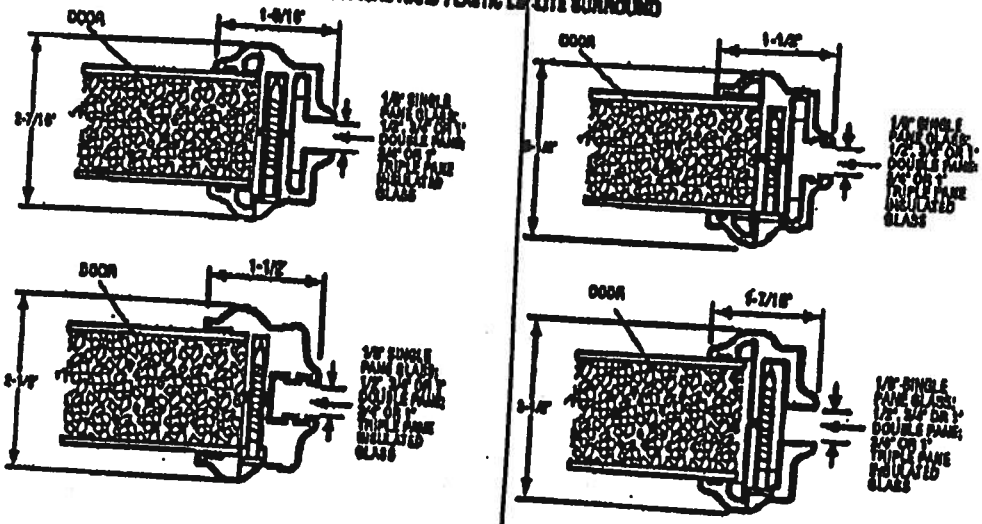
Masonite

MAD-WI-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND

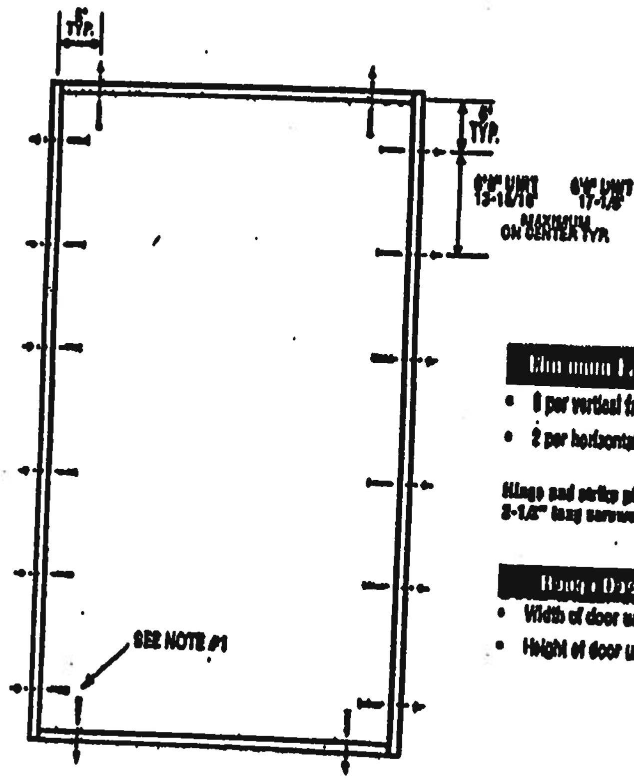


*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

X Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 3-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate #3026447A, #3026447B, #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004 / #3026447B-001, 002, 003, 004 / #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 5245*, 5256*, 5241*, 5246, 5261* or 5255 Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade County approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

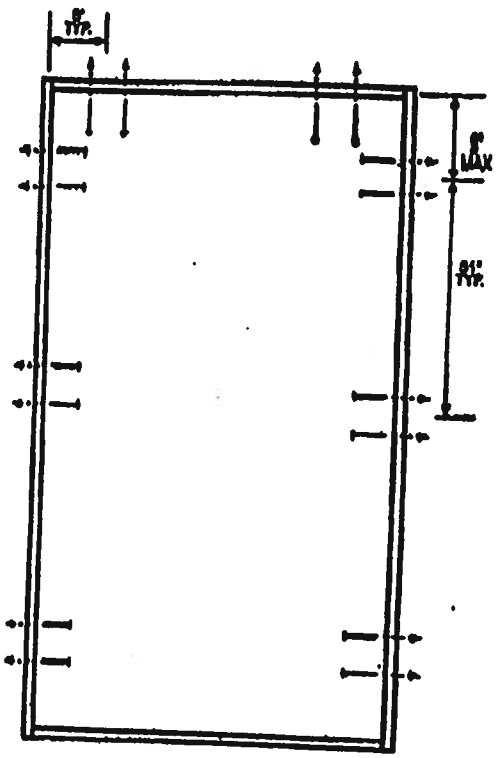
March 10, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

Masonite

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0" height and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 4 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate [illegible] and COP/Test Report Validation Matrix [illegible] additional information - available from the [illegible] website [illegible] www.masonite.com or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 0248*, 8288*, 3241*, 3248, 3281* or 3288
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

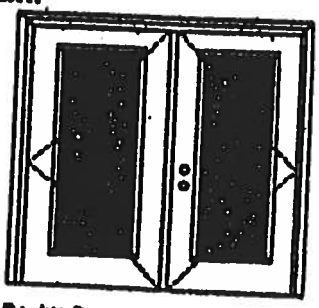
1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

XX
Glazed Outswing Unit

COP-WI-FM4162.02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+50.6/-50.6
Limited water inflow special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-edition, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

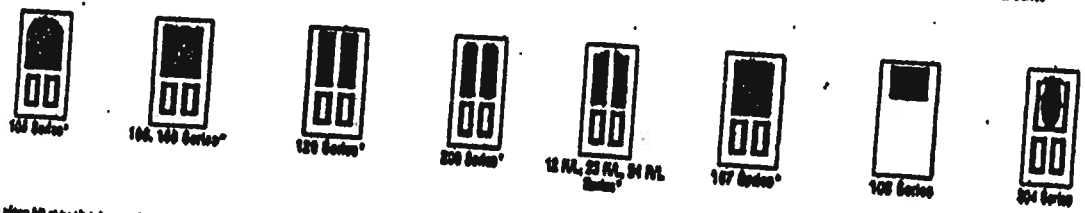
Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:
1/4 GLASS:

1/2 GLASS:



*This glass lite style may also be used in the following door styles: 6-panel 6-panel with raised fielves 6-panel fielves 6-panel fielves with raised.

Entergy
Entry Systems
June 17, 2008
Our continuing program of product improvement makes specifications, design and product data subject to change without notice.

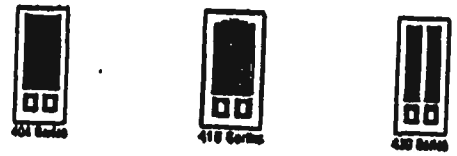
Exclusively from Masonite
Masonite International Corporation

XX
Glazed Outswing Unit

COP-WI-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:
3/4 GLASS:



FULL GLASS:

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.032" steel. Bottom and rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202
COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Entergy
Entry Systems

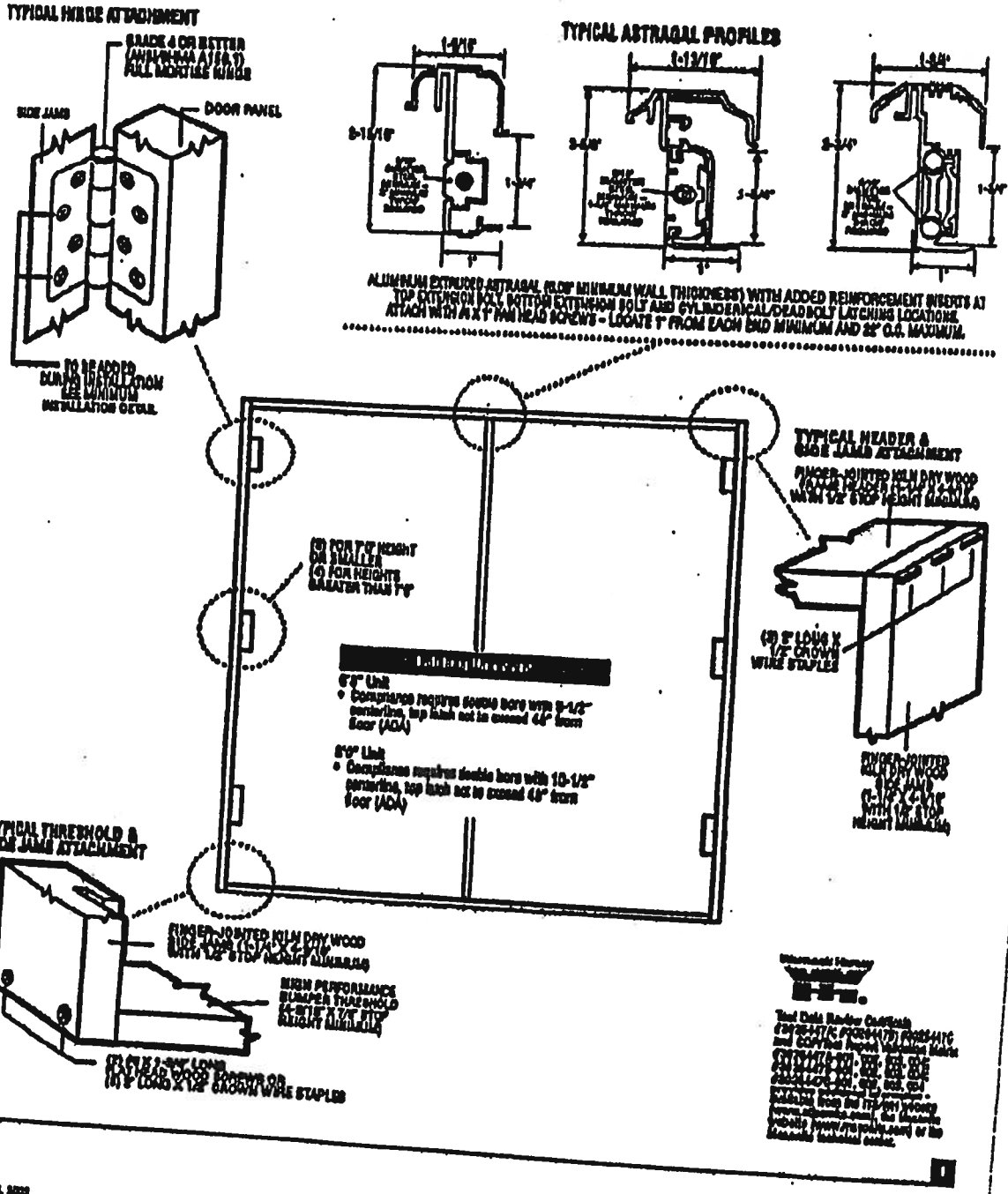
June 17, 2004

Masonite International Corporation

XX
Unit

MAD WL-MA0012-02

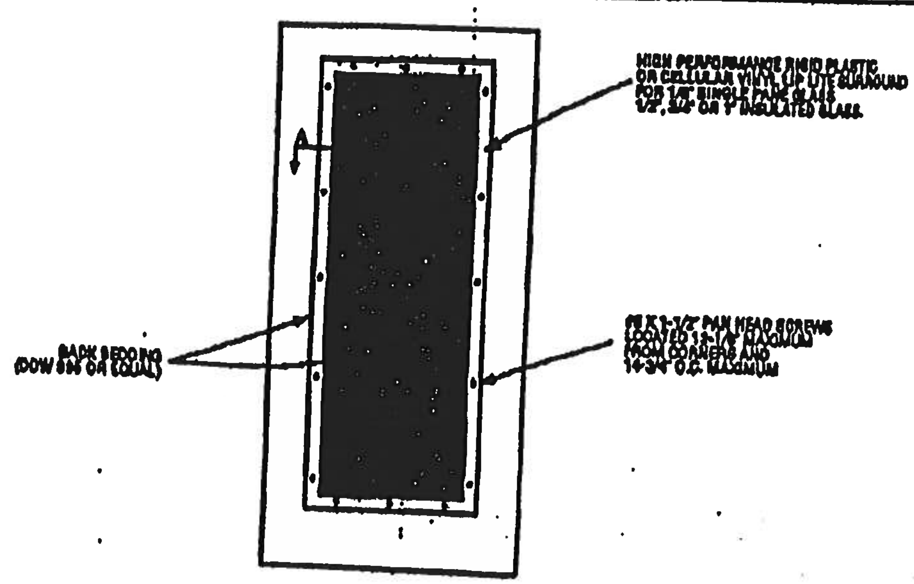
OUTSWING UNITS WITH DOUBLE DOOR



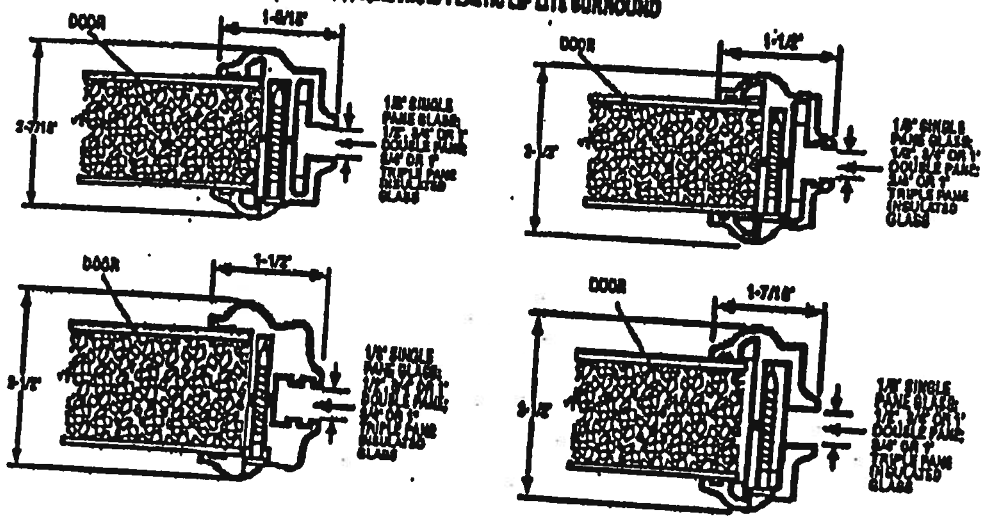
Masonite

MAD-WL-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND

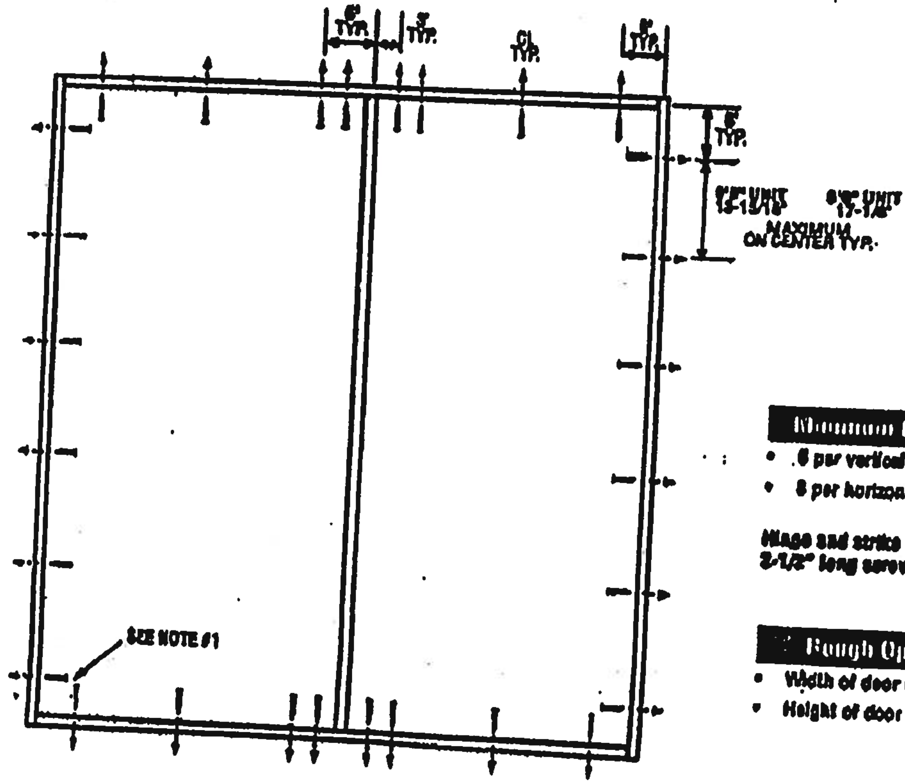


*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4".

[illegible] Test Data Review Certificate [illegible] additional information - available from the ITW/WH website [illegible], the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 8247*, 8257*, 8242*, 8247, 8232* or 8257
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

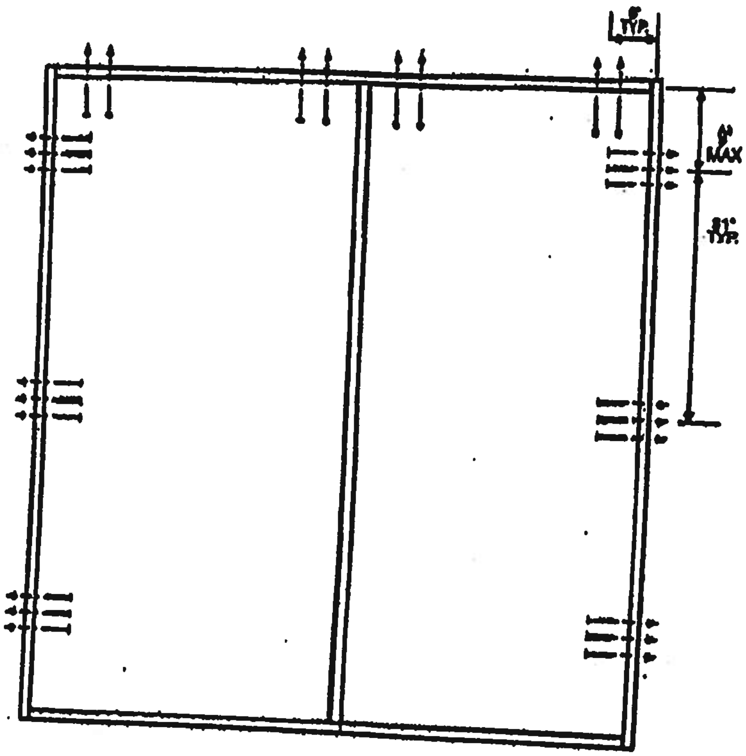
Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0" heights and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 6 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate [illegible] and COP/Test Report Validation Matrix [illegible] provides additional information - available from the [illegible] website [illegible], the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 0247*, 0267*, 3242*, 3247, 3262* or 3267
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 wood screws and 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw and common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Residential System Sizing Calculation

Summary

Verlene Griffin
Scrubtown Rd
Fort White, FL 32038-

Project Title:
NICOLAS 1580 - Griffin Res

Code Only
Professional Version
Climate: North

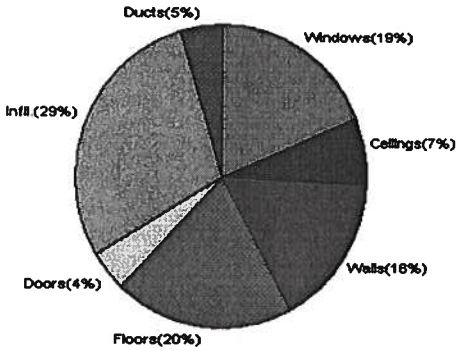
8/1/2007

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**28560**	**Btuh**	**Total cooling load calculation**	**28995**	**Btuh**
Submitted heating capacity	30000	Btuh	Submitted cooling capacity	30000	Btuh
Submitted as % of calculated	105.0	%	Submitted as % of calculated	103.5	%

WINTER CALCULATIONS

Winter Heating Load (for 1580 sqft)

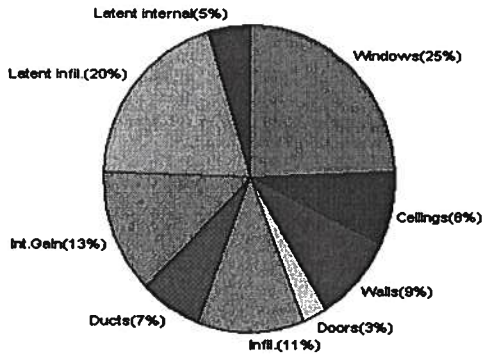
Load component			Load	
Window total	190	sqft	5386	Btuh
Wall total	1596	sqft	4648	Btuh
Door total	78	sqft	1242	Btuh
Ceiling total	1580	sqft	2054	Btuh
Floor total	181	ft	5720	Btuh
Infiltration	190	cfm	8150	Btuh
Subtotal			**27200**	**Btuh**
Duct loss			1360	Btuh
TOTAL HEAT LOSS			**28560**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1580 sqft)

Load component			Load	
Window total	190	sqft	7113	Btuh
Wall total	1596	sqft	2637	Btuh
Door total	78	sqft	778	Btuh
Ceiling total	1580	sqft	2244	Btuh
Floor total			0	Btuh
Infiltration	166	cfm	3291	Btuh
Internal gain			3800	Btuh
Subtotal(sensible)			**19864**	**Btuh**
Duct gain			1986	Btuh
Total sensible gain			**21850**	**Btuh**
Latent gain(infiltration)			5765	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**7145**	**Btuh**
TOTAL HEAT GAIN			**28995**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY:
DATE: 8-1-07

System Sizing Calculations - Winter

Residential Load - Component Details

Verlene Griffin
Scrubtown Rd
Fort White, FL 32038-

Project Title:
NICOLAS 1580 - Griffin Res

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

8/1/2007

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	E	36.0	28.3	1019 Btuh
2	2, Clear, Metal, DEF	E	13.3	28.3	377 Btuh
3	2, Clear, Metal, DEF	E	6.0	28.3	170 Btuh
4	2, Clear, Metal, DEF	E	17.5	28.3	495 Btuh
5	2, Clear, Metal, DEF	S	30.0	28.3	849 Btuh
6	2, Clear, Metal, DEF	W	17.5	28.3	495 Btuh
7	2, Clear, Metal, DEF	W	20.0	28.3	566 Btuh
8	2, Clear, Metal, DEF	W	30.0	28.3	849 Btuh
9	2, Clear, Metal, DEF	N	20.0	28.3	566 Btuh
	Window Total		190		5386 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Exterior	13.0	1396	3.1	4328 Btuh
2	Frame - Adjacent	13.0	200	1.6	320 Btuh
	Wall Total		1596		4648 Btuh
Doors	Type		Area X	HTM=	Load
1	Wood - Exter		20	17.9	359 Btuh
2	Wood - Adjac		18	9.2	166 Btuh
3	Wood - Exter		40	17.9	718 Btuh
	Door Total		78		1242Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1580	1.3	2054 Btuh
	Ceiling Total		1580		2054Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	181.0 ft(p)	31.6	5720 Btuh
	Floor Total		181		5720 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.80	14220(sqft)	190	8150 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			190	8150 Btuh

Totals for Heating	**Subtotal**	**27200 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1360 Btuh**
	Total Btuh Loss	**28560 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Summer

Residential Load - Component Details

Verlene Griffin
Scrubtown Rd
Fort White, FL 32038-

Project Title:
NICOLAS 1580 - Griffin Res

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F 8/1/2007

Window	Type Panes/SHGC/U/InSh/ExSh	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, DEF, B, N	E	1.5	8	36.0	0.0	36.0	15	46	1656 Btuh
2	2, Clear, DEF, B, N	E	9	10	13.3	4.1	9.2	15	46	485 Btuh
3	2, Clear, DEF, B, N	E	9	10	6.0	0.0	6.0	15	46	276 Btuh
4	2, Clear, DEF, B, N	E	1.5	6	17.5	0.9	16.6	15	46	778 Btuh
5	2, Clear, DEF, B, N	S	1.5	6	30.0	15.0	15.0	15	24	585 Btuh
6	2, Clear, DEF, B, N	W	1.5	6	17.5	0.9	16.6	15	46	778 Btuh
7	2, Clear, DEF, B, N	W	1.5	7.5	20.0	0.0	20.0	15	46	920 Btuh
8	2, Clear, DEF, B, N	W	1.5	6	30.0	1.5	28.5	15	46	1334 Btuh
9	2, Clear, DEF, B, N	N	1	7	20.0	0.0	20.0	15	15	300 Btuh
	Window Total				190					7113 Btuh

Walls	Type	R-Value	Area	HTM	Load
1	Frame - Exterior	13.0	1396.0	1.7	2429 Btuh
2	Frame - Adjacent	13.0	200.0	1.0	208 Btuh
	Wall Total		1596.0		2637 Btuh

Doors	Type	Area	HTM	Load
1	Wood - Exter	20.0	10.0	200 Btuh
2	Wood - Adjac	18.0	10.0	180 Btuh
3	Wood - Exter	40.0	10.0	399 Btuh
	Door Total	78.0		778 Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load
1	Under Attic/Dark	30.0	1580.0	1.4	2244 Btuh
	Ceiling Total		1580.0		2244 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab-On-Grade Edge Insulation	0.0	181.0 ft(p)	0.0	0 Btuh
	Floor Total		181.0		0 Btuh

Infiltration	Type	ACH	Volume	CFM=	Load
	Natural	0.70	14220	166.2	3291 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			166	3291 Btuh

Internal gain		Occupants		Btuh/occupant		Appliance	Load
		6	X	300	+	2000	3800 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Verlene Griffin
Scrubtown Rd
Fort White, FL 32038-

Project Title:
NICOLAS 1580 - Griffin Res

Code Only
Professional Version
Climate: North

8/1/2007

Totals for Cooling	**Subtotal**	**19864**	**Btuh**
	Duct gain(using duct multiplier of 0.10)	**1986**	**Btuh**
	Total sensible gain	**21850**	**Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**5765**	**Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380**	**Btuh**
	Latent other gain	**0**	**Btuh**
	TOTAL GAIN	**28995**	**Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

ROBINSON, KENNON & KENDRON, P.A.
ATTORNEYS AT LAW

BRUCE W. ROBINSON *†
KRIS B. ROBINSON
FREDERICK L. KOBERLEIN, JR.

582 WEST DUVAL STREET
POST OFFICE BOX 1178
LAKE CITY, FLORIDA 32056-1178
Telephone (386) 755-1334
Facsimile (386) 755-1336
www.rkkattorneys.com

THOMAS J. KENNON,III ††
JOHN J. KENDRON

May 20, 2008

Brian, under these circumstances in my opinion Ms. Griffin should be allowed to continue building under her existing permit.
5/29/08 Marlin Feagle

Columbia County Building Department
Post Office Box 1529
Lake City, FL 32056-1529

RE: Parcel No.: 05-7S-17-09898-003
Estate of Robert L. Zimmerman
Our File No.: 10996.1 Permit: 26241

To Whom It May Concern:

Our firm represents Verlene Griffin in the Estate of Robert L. Zimmerman. Mrs. Griffin asked that I write you to explain the events concerning her property and the building permit she had obtained approximately one year ago.

The title to the property on which Verlene Griffin's home was to be built was still in the name of Robert L. Zimmerman, her deceased brother, at the time of the issuance of the building permit. Therefore, the contractors could not commence with construction of the home until the title had been cleared. As her brother resided in Polk County, probate through the Polk County court was initiated and has been completed with the title to the property transferring to Mr. Zimmerman's spouse. Ms. Zimmerman has subsequently signed a *Special Warranty Deed* transferring title to the property to Ms. Griffin. The *Warranty Deed*, as well as the probate documents from Polk County, are now in the process of being recorded in Columbia County.

Due to the above-related facts, construction on the property purchased by Verlene Griffin could not commence until title to the property had been transferred into her name. At the time the building permits were issued, Ms. Griffin was unaware that she did not have clear title to the property and was also unaware that she would have to probate her brother's estate to transfer title into her name. The probate process has now been completed and title to the property is now in Ms. Griffin's name and she may commence with the construction on the property.

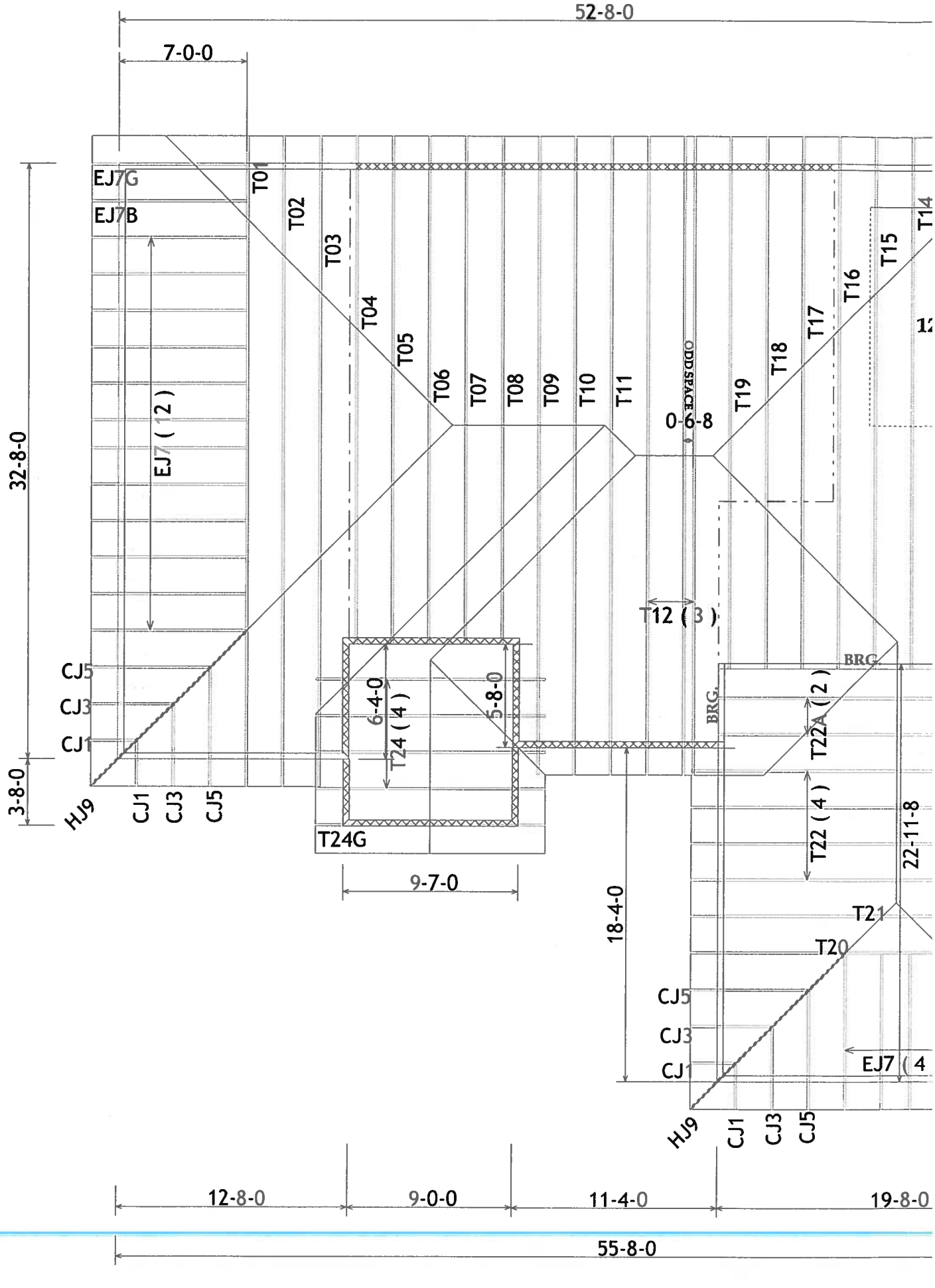
If you have any questions, or need documentation of the above facts, please feel free to contact my office.

Sincerely,

John J. Kendron
For the Firm

*BOARD CERTIFIED CIVIL TRIAL ATTORNEY
†FLORIDA SUPREME COURT CERTIFIED CIRCUIT CIVIL MEDIATOR

††FLORIDA SUPREME COURT CERTIFIED FAMILY LAW MEDIATOR


52-8-0
7-0-0
EJ7G
EJ7B
T01
T02
T03
T04
T05
T06
T07
T08
T09
T10
T11
T12 (3)
T14
T15
T16
T17
T18
T19
0-6-8
32-8-0
EJ7 (12)
CJ5
CJ3
CJ1
3-8-0
HJ9
CJ1
CJ3
CJ5
6-4-0
T24 (4)
5-8-0
T24G
9-7-0
BRG.
BRG.
T22A (2)
T22 (4)
22-11-8
18-4-0
T21
T20
CJ5
CJ3
CJ1
EJ7 (4
HJ9
CJ1
CJ3
CJ5
12-8-0
9-0-0
11-4-0
19-8-0
55-8-0

UPDATED

Builders FirstSource

HLAKMF

REVIEWED
By julius lee at 5:46 pm, Sep 04, 2008

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

Project Information:

Builder: EWPL, Inc.
Model: CUSTOM
Builders FirstSource Job #: L250193
Street: SCRUBTOWN RD.
City: Ft. White
County: Columbia
Building Code: FBC2004/TPI2002
Computer Program Used: MiTek 6.3

Builders FirstSource
2525 E. Duval St.
Lake City, FL 32055

Truss Design Information:

Gravity Loads
Roof: 32 psf Total
Floor: 55 psf Total

Wind
Wind Standard: ASCE 7-02
Wind Speed: 110 mph
Mean Roof Ht: 20 ft
Exposure: B

JULIUS LEE'S CONSULT. ENGR INC
1455 SW 4TH AVE , DELRAY BEACH
FLORIDA. 33444

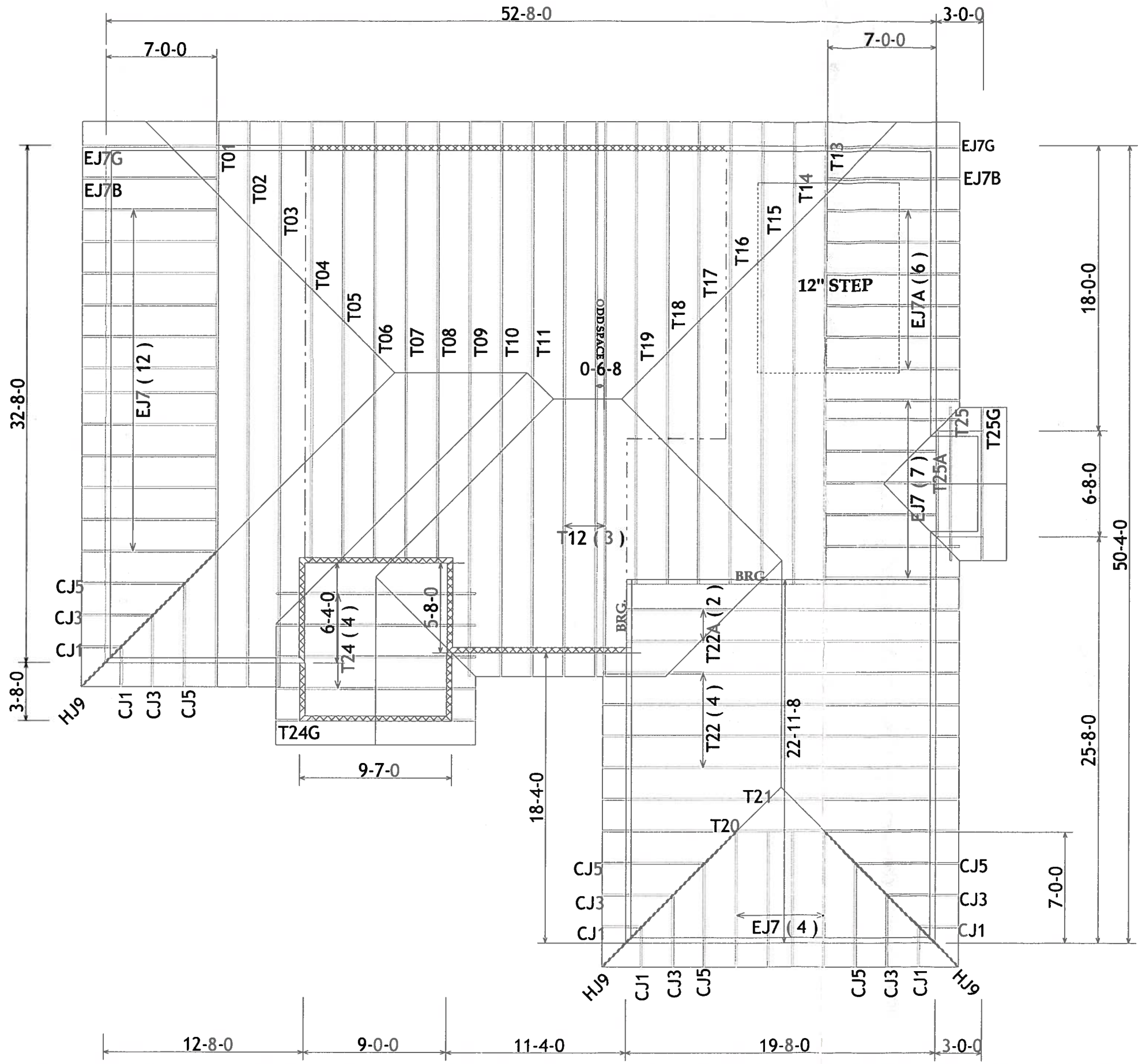
Note: Refer to individual truss design drawings for special loading conditions, design criteria, truss geometry, lumber, and plate information.

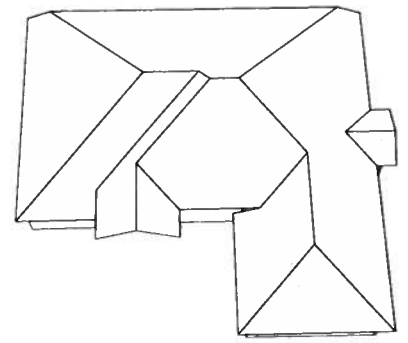
Design Professional Information:

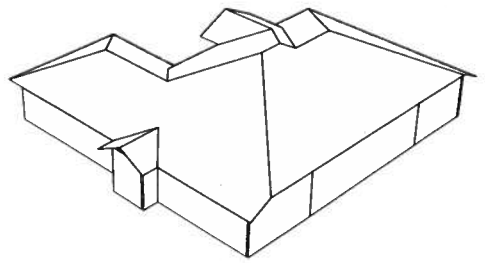
Design Professional Of Record: Hugo Escalante — License # : CRC1326967
Delegated Truss Engineer: Julius Lee — License # : 34869

This truss specification package consists of this index sheet and 36 truss design drawings. This signed and sealed index sheet indicates acceptance of my professional engineering responsibility solely for listed truss design drawings. The suitability and use of each truss component for any particular building is the responsibility of the building designer per TPI.

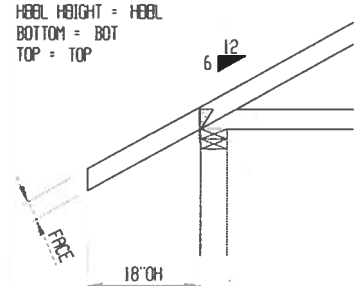
Truss #	Truss Label	Drawing #	Seal Date	Truss #	Truss Label	Drawing #	Seal Date	Truss #	Truss Label	Drawing #	Seal Date
1	CJ1	L250193001	9/4/2008	31	T22A	L250193031	9/4/2008				
2	CJ3	L250193002	9/4/2008	32	T24	L250193032	9/4/2008				
3	CJ5	L250193003	9/4/2008	33	T24G	L250193033	9/4/2008				
4	EJ7	L250193004	9/4/2008	34	T25	L250193034	9/4/2008				
5	EJ7A	L250193005	9/4/2008	35	T25A	L250193035	9/4/2008				
6	EJ7B	L250193006	9/4/2008	36	T25G	L250193036	9/4/2008				
7	EJ7G	L250193007	9/4/2008								
8	HJ9	L250193008	9/4/2008								
9	T01	L250193009	9/4/2008								
10	T02	L250193010	9/4/2008								
11	T03	L250193011	9/4/2008								
12	T04	L250193012	9/4/2008								
13	T05	L250193013	9/4/2008								
14	T06	L250193014	9/4/2008								
15	T07	L250193015	9/4/2008								
16	T08	L250193016	9/4/2008								
17	T09	L250193017	9/4/2008								
18	T10	L250193018	9/4/2008								
19	T11	L250193019	9/4/2008								
20	T12	L250193020	9/4/2008								
21	T13	L250193021	9/4/2008								
22	T14	L250193022	9/4/2008								
23	T15	L250193023	9/4/2008								
24	T16	L250193024	9/4/2008								
25	T17	L250193025	9/4/2008								
26	T18	L250193026	9/4/2008								
27	T19	L250193027	9/4/2008								
28	T20	L250193028	9/4/2008								
29	T21	L250193029	9/4/2008								
30	T22	L250193030	9/4/2008								







APPROVED TRUSS ANCHOR BY BUILDER
PLUMB CUT OVERHANG
HEEL HEIGHT = HEEL
BOTTOM = BOT
TOP = TOP



TRUSS END DETAIL

BEARING HEIGHT SCHEDULE

8'-0"

10'-0"

OVERHANG

1'-6"

ROOF PITCH(S)

6/12

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V10S FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904 437 3349 FAX: 904 437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER:
HUGO ESCALANTE

LEGAL ADDRESS:
COLUMBIA, FL

MODEL:		REVISION:
CUSTOM		SCALE: NTS
DATE: **08/07/07**	DRAWN BY: **A MONDRAGON**	JOB #: **L250193**

DATE 09/17/2007

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000026241

APPLICANT HUGO ESCALANTE PHONE 386.288.8666

ADDRESS POB 280 FT. WHITE FL 32038

OWNER VERLENE GRIFFIN PHONE 386.497.2956

ADDRESS 141 SCRUBTOWN ROAD FT. WHITE FL 32038

CONTRACTOR HUGO ESCALANTE PHONE 386.288.8666

LOCATION OF PROPERTY 441-S TO C-18,TR TO TUSTENUGGEE AVE,TL TO OLD BELLAMY,TL TO SCRUBTOWN,TR AND IT'S THE CORNER LOT ON L.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 79000.00

HEATED FLOOR AREA 1580.00 TOTAL AREA 2116.00 HEIGHT 18.60 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 05-7S-17-09898-003 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES 1.00

		CRC1326967		
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
EXISTING	07-638	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: FLOOR ONE FOOT ABOVE THE ROAD. LEGAL NON-CONFORMING LOT THROUGH TH ESTATE SUCCESSIONS OF LINEAL DESCENDANTS.

Check # or Cash 5477

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by

Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by

Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by

Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by

Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by

M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE $ 395.00 CERTIFICATION FEE $ 10.58 SURCHARGE FEE $ 10.58

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 491.16

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

BK 0775 PG 1028

OFFICIAL RECORDS

RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Grantee #1 S.S. No. 261-44-5793

Property Appraiser's
Parcel Identification No.
05751709898003

DOCUMENTARY STAMP .70
INTANGIBLE TAX .0
P. DeWITT CASON, CLERK OF
COURTS, COLUMBIA COUNTY
BY Mary Lou D.C.

WARRANTY DEED

THIS INDENTURE, made this 12 day of May, 1993, BETWEEN **ELLA MAE RANDOLPH, FLORENCE McCLOUD, JAMES ZIMMERMAN, TOMMIE LEE ZIMMERMAN, RUDOLPH ZIMMERMAN, VERLENE GRIFFIN, LOUISE JAMES and ROBERT LEE ZIMMERMAN,** whose post office address is Route 2, Box 150, High Springs, Florida 32643, of the County of Alachua, State of Florida, grantor*, and **ROBERT LEE ZIMMERMAN,** whose post office address is 519 Hull Street, Lakeland, Florida 33805, of the County of Polk, State of Florida, grantee*.

WITNESSETH: that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

> The parcel described in Schedule A attached, less and except the South 645 feet and less and except the East 200 feet.
>
> SUBJECT TO an easement over the South 30 feet.
>
> SUBJECT TO: Restrictions, easements and outstanding mineral rights of record, if any, and taxes for the current year.

and said grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

*"Grantor" and "grantee" are used for singular or plural, as context requires.

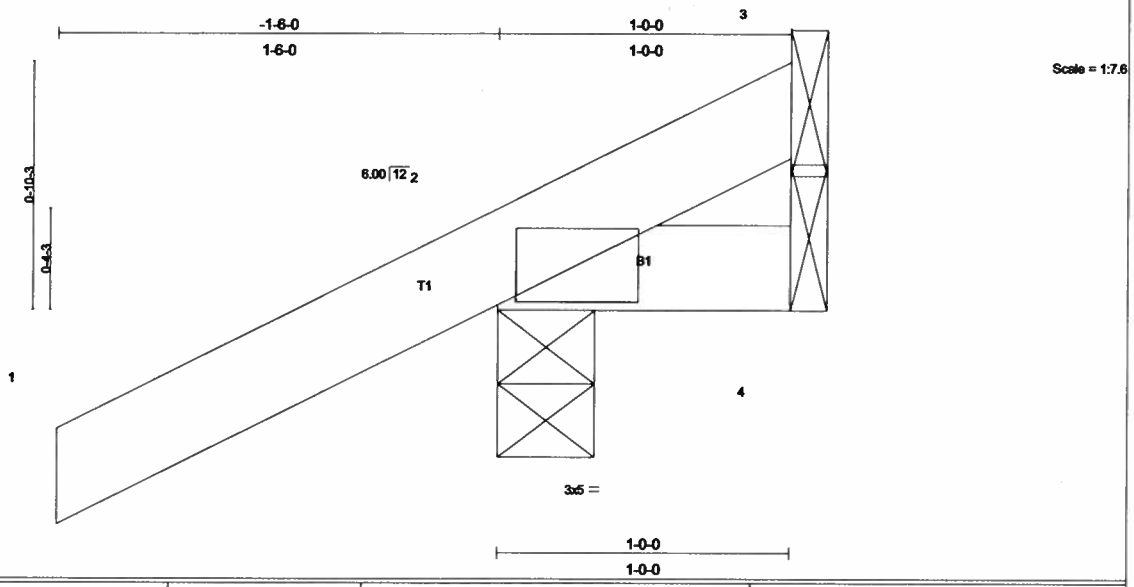
IN WITNESS WHEREOF, grantor has hereunto set grantor's hand

A PART OF THE NE ¼ OF SECTION 5, TOWNSHIP 7 SOUTH, RANGE 17 EAST, MORE PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCE AT THE SE CORNER OF SAID NE ¼ AND RUN S 88°16'07" W, ALONG THE SOUTH LINE THEREOF, 852.25 FEET FOR A POINT OF BEGINNING; THENCE CONTINUE S 88°16'07" W, 411.63 FEET TO THE EAST RIGHT-OF-WAY LINE OF SCRUB TOWN ROAD; THENCE N 06°33'24" W, ALONG SAID EAST RIGHT-OF-WAY LINE 207.39 FEET; THENCE CONTINUE ALONG SAID EAST LINE N 00°26'54" W, 671.81 FEET; THENCE CONTINUE ALONG SAID EAST RIGHT-OF-WAY LINE N 37°51'23" E, 54.72 FEET TO THE SOUTH RIGHT-OF-WAY LINE OF BELLAMY ROAD; THENCE S 82°00'16" E, ALONG SAID SOUTH RIGHT-OF-WAY LINE 374.86 FEET; THENCE S 02°22'45" E, 857.19 FEET TO THE POINT OF BEGINNING. COLUMBIA COUNTY, FLORIDA.

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	CJ1	JACK	6	1	L250193001

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 04 16:58:24 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=180/0-4-0, 4=5/Mechanical, 3=-41/Mechanical
Max Horz 2=70(load case 6)
Max Uplift2=-181(load case 6), 3=-41(load case 1)
Max Grav 2=180(load case 1), 4=14(load case 2), 3=61(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/35
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.12

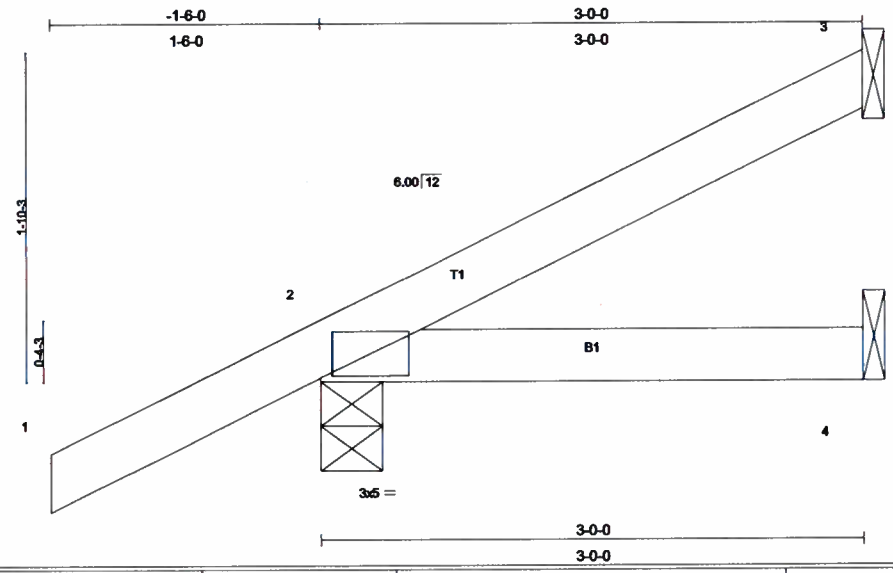
NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 2 and 41 lb uplift at joint 3.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	CJ3	JACK	6	1	L250193002

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 04 16:58:25 2008 Page 1



LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.17	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=48/Mechanical, 2=206/0-4-0, 4=14/Mechanical
Max Horz 2=115(load case 6)
Max Uplift3=-37(load case 6), 2=-153(load case 6)
Max Grav 3=48(load case 1), 2=206(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-49/16
BOT CHORD 2-4=0/0

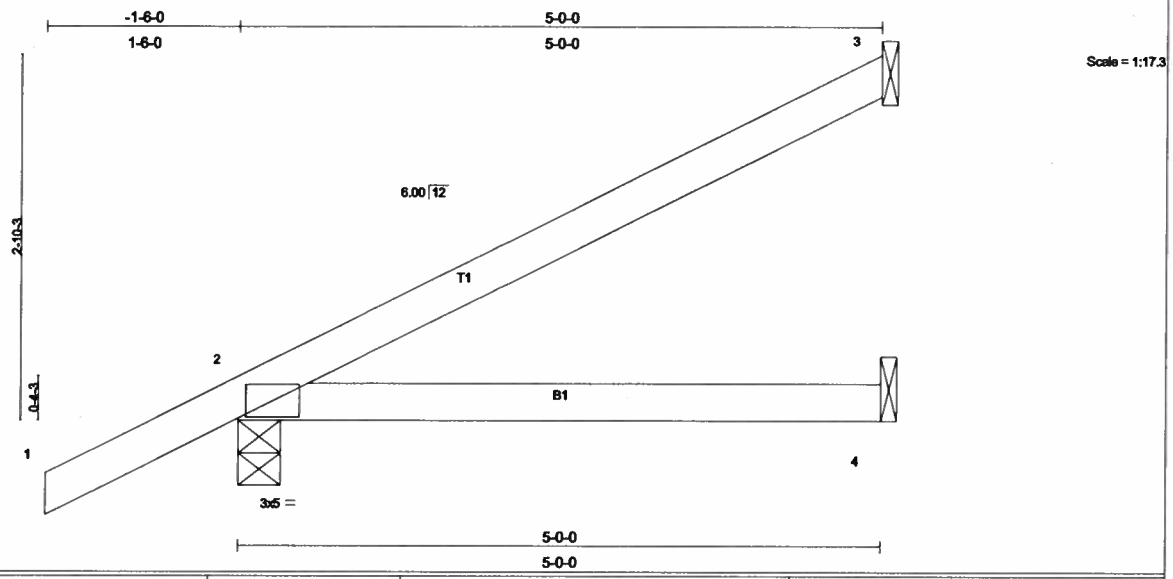
JOINT STRESS INDEX
2 = 0.13

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 3 and 153 lb uplift at joint 2.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	CJ5	JACK	6	1	L250193003
					Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2008 MiTek Industries, Inc. Thu Sep 04 16:58:26 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=113/Mechanical, 2=258/0-4-0, 4=24/Mechanical
Max Horz 2=162(load case 6)
Max Uplift3=-101(load case 6), 2=-159(load case 6)
Max Grav 3=113(load case 1), 2=258(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-96/41
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 3 and 159 lb uplift at joint 2.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	EJ7	MONO TRUSS	23	1	L250193004
					Job Reference (optional)

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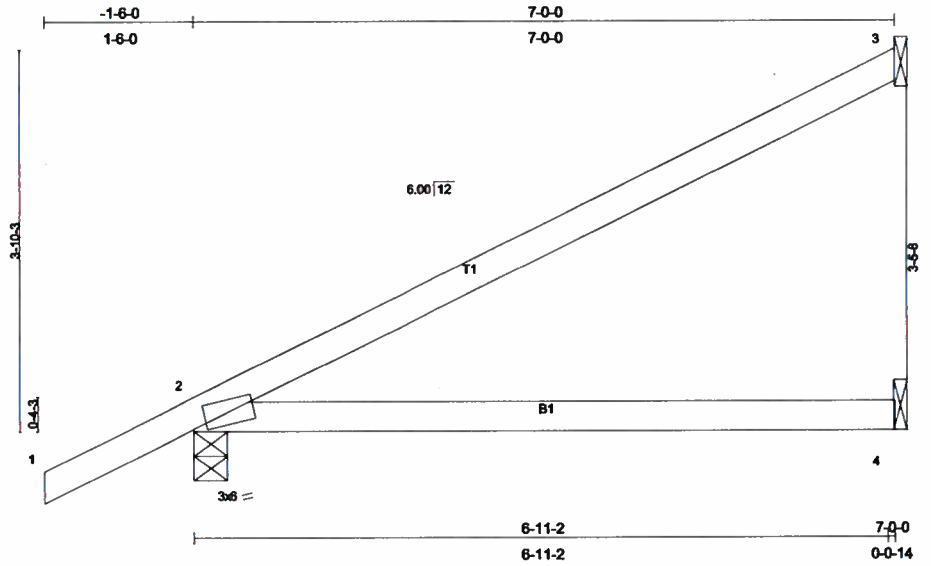

Plate Offsets (X,Y): [2:0-1-9,0-0-7]

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.43	Vert(LL)	0.11	2-4	>756	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.17	2-4	>488	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=157/Mechanical, 2=318/0-4-0, 4=48/Mechanical
Max Horz 2=149(load case 6)
Max Uplift3=-88(load case 6), 2=-112(load case 6)
Max Grav 3=157(load case 1), 2=318(load case 1), 4=94(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-122/56
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.91

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 88 lb uplift at joint 3 and 112 lb uplift at joint 2.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	EJ7A	SPECIAL	6	1	L250193005
					Job Reference (optional)

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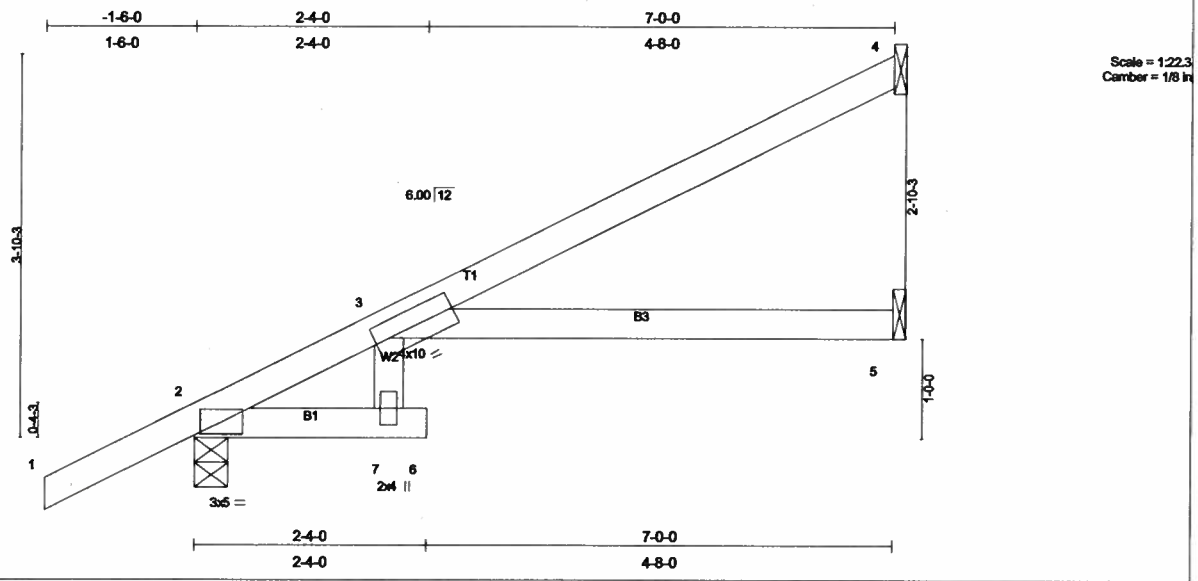

Plate Offsets (X,Y): [3:0-1-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.20	6	>397	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.20	3-5	>400	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.02	Horz(TL)	-0.10	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=142/Mechanical, 2=321/0-4-0, 5=64/Mechanical
Max Horz 2=149(load case 6)
Max Uplift4=-74(load case 6), 2=-110(load case 6), 5=-4(load case 6)
Max Grav 4=142(load case 1), 2=321(load case 1), 5=92(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-129/0, 3-4=-97/52
BOT CHORD 2-7=-2/0, 6-7=0/0, 3-5=0/0
WEBS 3-7=0/61

JOINT STRESS INDEX
2 = 0.41, 3 = 0.94 and 7 = 0.04

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 74 lb uplift at joint 4, 110 lb uplift at joint 2 and 4 lb uplift at joint 5.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	EJ7B	MONO HIP	2	1	L250193006

Job Reference (optional)

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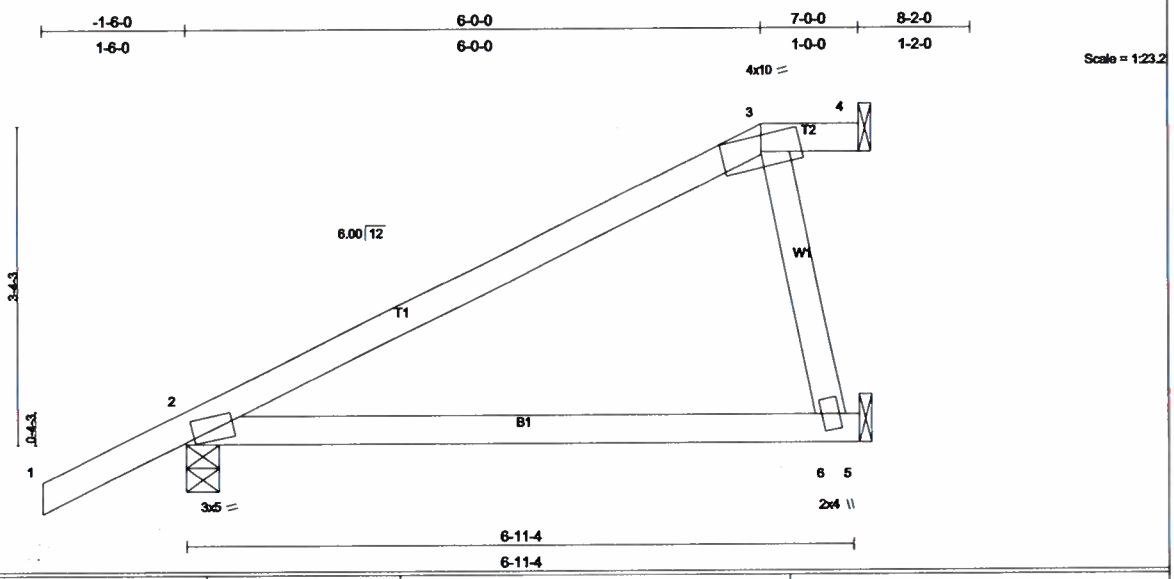

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.07	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.12	2-6	>644	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.10	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=-145/Mechanical, 2=307/0-4-0, 6=362/Mechanical
Max Horz 2=134(load case 6)
Max Uplift4=-145(load case 1), 2=-114(load case 6), 6=-138(load case 6)
Max Grav 4=74(load case 6), 2=307(load case 1), 6=362(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-156/8, 3-4=-2/2
BOT CHORD 2-6=-97/79, 5-6=0/0
WEBS 3-6=-334/412

JOINT STRESS INDEX
2 = 0.68, 3 = 0.75 and 6 = 0.23

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 145 lb uplift at joint 4, 114 lb uplift at joint 2 and 138 lb uplift at joint 6.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE	J1876049
L250193	EJ7G	GABLE	2	1	L250193007	
					Job Reference (optional)	

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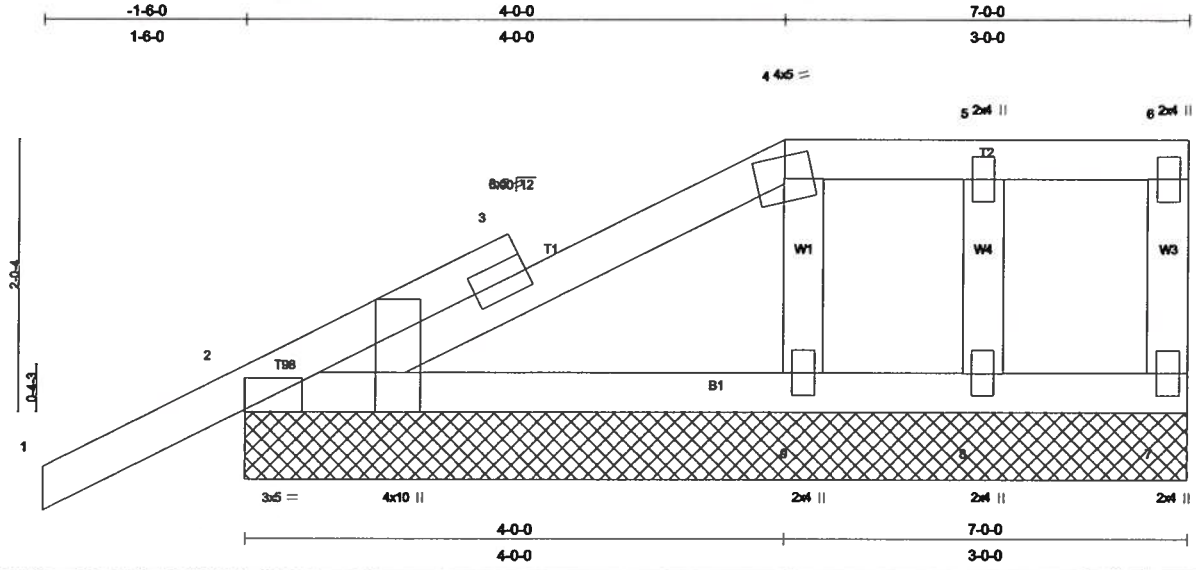

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-1-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.00	1	n/r	90		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.07	Horz(TL)	-0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=334/7-0-0, 7=66/7-0-0, 9=328/7-0-0, 8=67/7-0-0
Max Horz 2=126(load case 6)
Max Uplift2=-181(load case 6), 7=-25(load case 5), 9=-125(load case 6), 8=-71(load case 4)
Max Grav 2=334(load case 1), 7=66(load case 1), 9=328(load case 1), 8=104(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-12/56, 2-3=-87/0, 3-4=-53/50, 4-5=-1/0, 5-6=0/0
BOT CHORD 2-9=-24/23, 8-9=0/0, 7-8=0/0
WEBS 4-9=-276/279, 6-7=-57/57, 5-8=-106/75

JOINT STRESS INDEX
2 = 0.49, 2 = 0.00, 3 = 0.00, 3 = 0.00, 3 = 0.39, 4 = 0.52, 5 = 0.04, 6 = 0.03, 7 = 0.03, 8 = 0.04 and 9 = 0.16

NOTES (11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 2, 25 lb uplift at joint 7, 125 lb uplift at joint 9 and 71 lb uplift at joint 8.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-87(F=-33), 4-6=-87(F=-33), 2-7=-10

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	HJ9	MONO TRUSS	3	1	L250193008
					Job Reference (optional)

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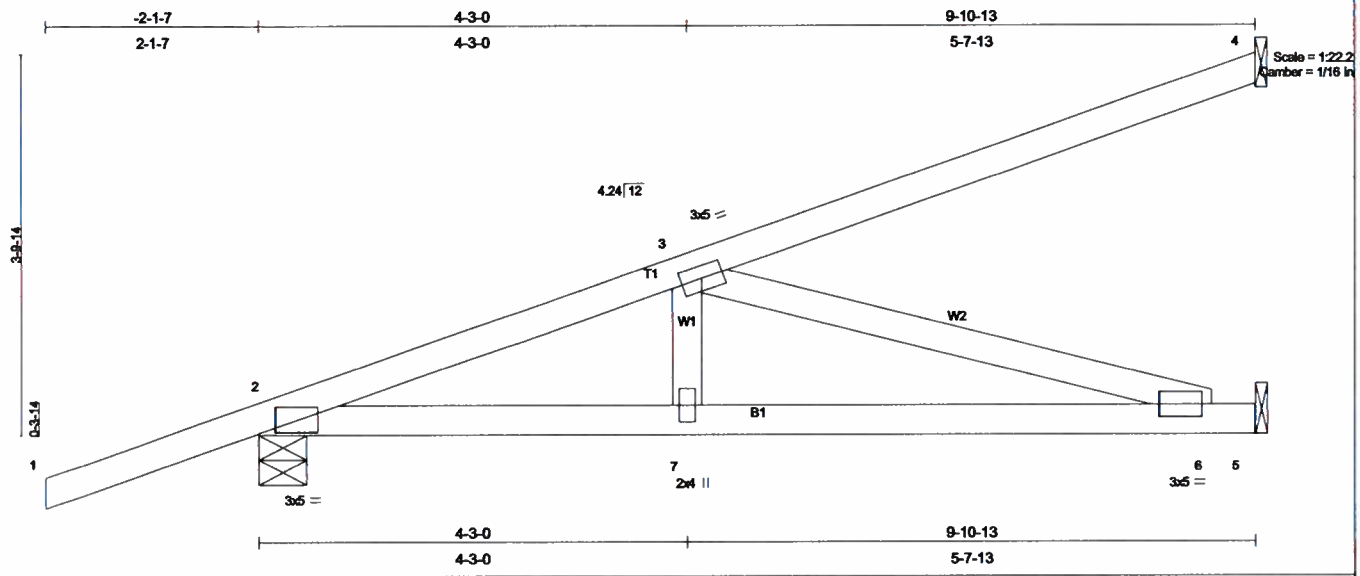

Plate Offsets (X,Y): [2:0-2-4,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	0.05	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.12	6-7	>989	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.37	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-7 oc bracing.

REACTIONS (lb/size) 4=266/Mechanical, 2=406/0-5-10, 5=231/Mechanical
Max Horz 2=253(load case 3)
Max Uplift4=-230(load case 3), 2=-228(load case 3), 5=-77(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-699/180, 3-4=-104/64
BOT CHORD 2-7=-370/652, 6-7=-370/652, 5-6=0/0
WEBS 3-7=0/195, 3-6=-680/386

JOINT STRESS INDEX
2 = 0.74, 3 = 0.20, 6 = 0.23 and 7 = 0.14

NOTES (6)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 4, 228 lb uplift at joint 2 and 77 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE	J1876051
L250193	T01	HIP	1	1	L250193009	

Job Reference (optional)

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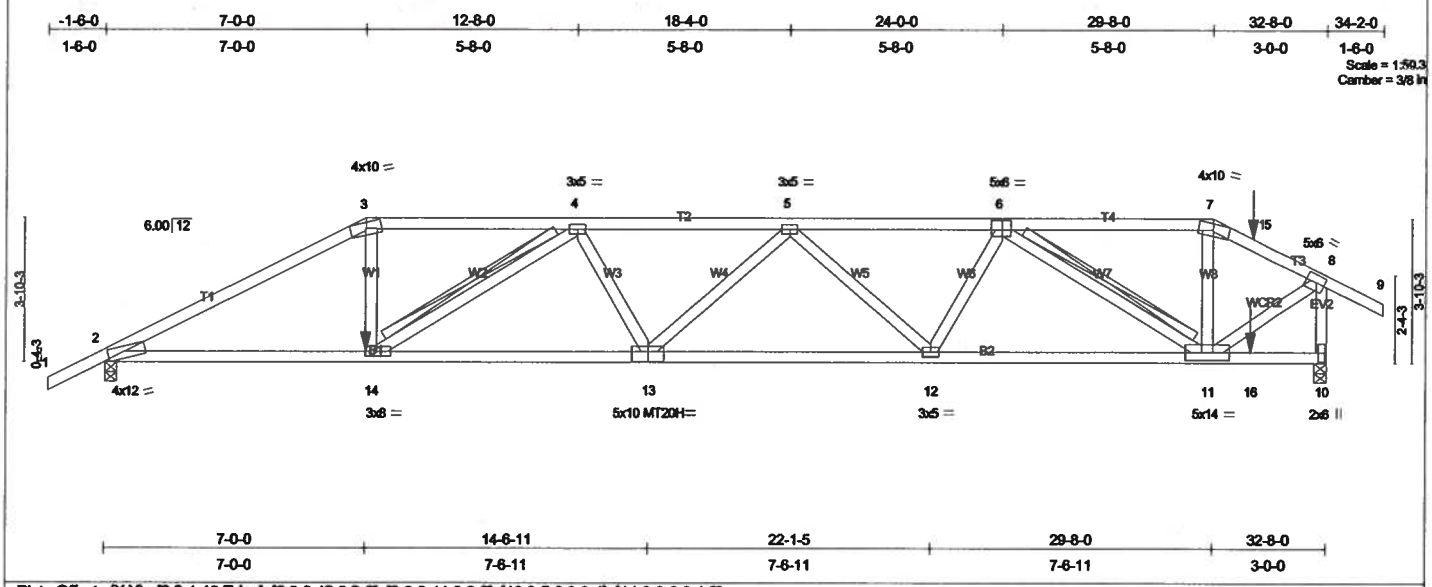

Plate Offsets (X,Y): [2:0-1-13,Edge], [6:0-2-12,0-3-0], [8:0-2-11,0-2-8], [13:0-5-0,0-3-4], [14:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.36	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.72	12-13	>537	240	MT20H	187/143
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.77	Horz(TL)	0.20	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 166 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-3-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-11-13 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-14, 6-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=2213/0-4-0, 10=2572/0-4-0
Max Horz 2=85(load case 4)
Max Uplift2=-674(load case 4), 10=-790(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-4284/1396, 3-4=-3794/1290, 4-5=-5244/1763, 5-6=-4616/1538, 6-7=-1941/664, 7-15=-2075/704, 8-15=-2171/709, 8-9=0/40, 8-10=-2426/738
BOT CHORD 2-14=-1264/3736, 13-14=-1772/5138, 12-13=-1820/5279, 11-12=-1438/4203, 11-16=-15/41, 10-16=-15/41
WEBS 3-14=-432/1441, 4-14=-1697/641, 4-13=0/294, 5-13=-78/114, 5-12=-915/401, 6-12=-193/884, 6-11=-2699/975, 7-11=-95/492, 8-11=-732/2306

JOINT STRESS INDEX
2 = 0.82, 3 = 0.86, 4 = 0.57, 5 = 0.40, 6 = 0.81, 7 = 0.73, 8 = 0.76, 10 = 0.71, 11 = 0.65, 12 = 0.71, 13 = 0.91 and 14 = 0.93

NOTES (11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 674 lb uplift at joint 2 and 790 lb uplift at joint 10.
8) Girder carries hip end with 3-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 74 lb down and 20 lb up at 30-8-0 on top chord, and 362 lb down and 100 lb up at 30-8-0, and 411 lb down and 165 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-117(F=-63), 7-8=-54, 8-9=-54, 2-14=-10, 11-14=-22(F=-12), 10-11=-10
Concentrated Loads (lb)
Vert: 14=-411(F) 15=-74(F) 16=-362(F)

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T02	HIP	1	1	L250193010

Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 04 16:58:33 2008 Page 1

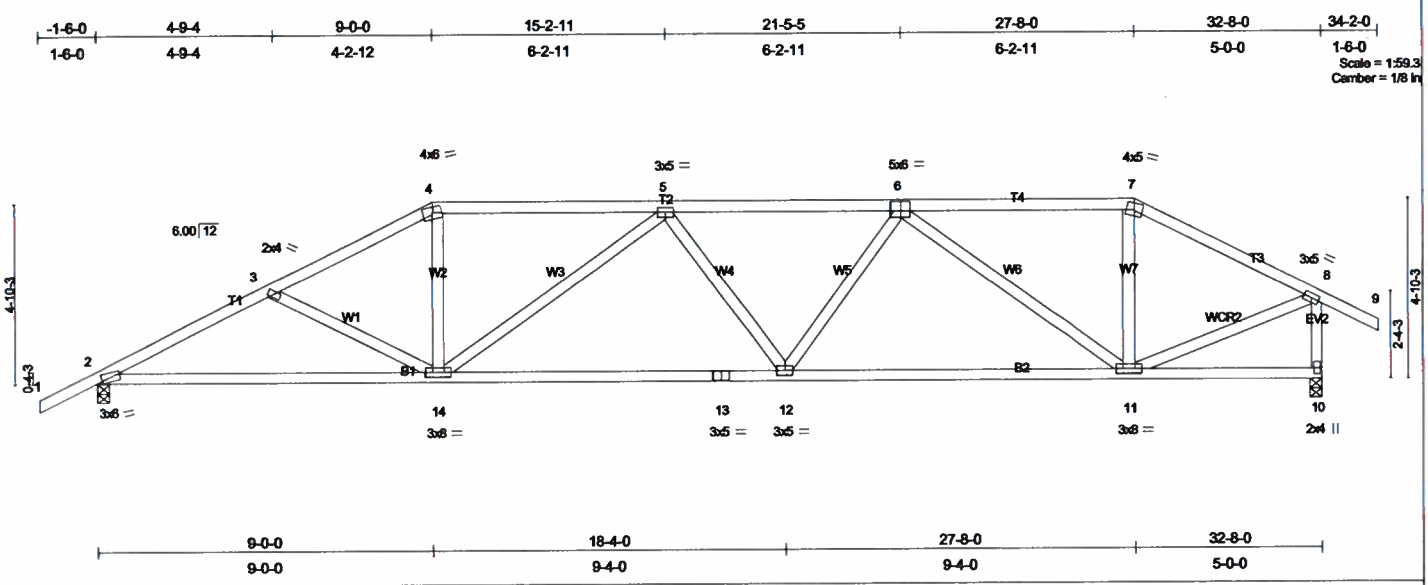

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.15	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.31	11-12	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.08	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 173 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=1125/0-4-0, 10=1124/0-4-0
Max Horz 2=98(load case 5)
Max Uplift2=-255(load case 6), 10=-228(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1936/1033, 3-4=-1709/928, 4-5=-1502/889, 5-6=-1789/1025, 6-7=-984/629, 7-8=-1148/642, 8-9=0/40, 8-10=-1105/677
BOT CHORD 2-14=-801/1664, 13-14=-819/1845, 12-13=-819/1845, 11-12=-731/1669, 10-11=-1/43
WEBS 3-14=-200/211, 4-14=-188/454, 5-14=-517/249, 5-12=-112/112, 6-12=-44/280, 6-11=-891/465, 7-11=-62/268, 8-11=-435/1041

JOINT STRESS INDEX
2 = 0.88, 3 = 0.34, 4 = 0.64, 5 = 0.42, 6 = 0.46, 7 = 0.80, 8 = 0.62, 10 = 0.40, 11 = 0.97, 12 = 0.42, 13 = 0.61 and 14 = 0.57

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 255 lb uplift at joint 2 and 228 lb uplift at joint 10.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T03	HIP	1	1	L250193011

Job Reference (optional)

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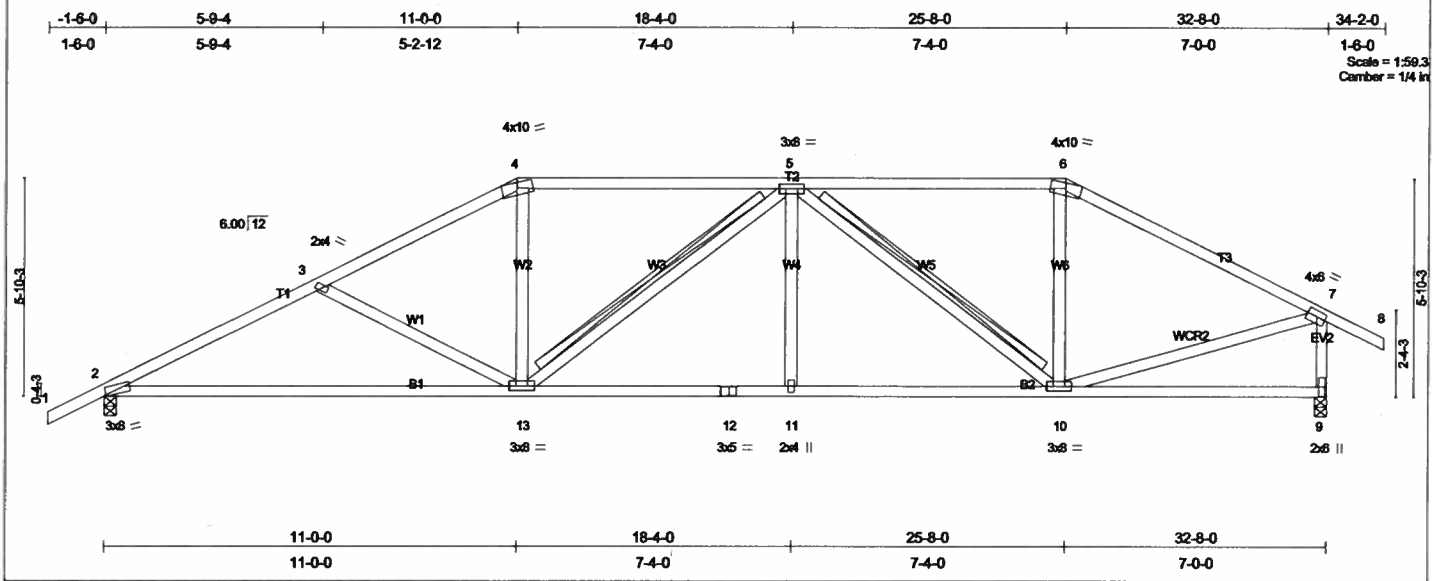

Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-2-15,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.30	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.57	2-13	>677	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.07	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 177 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-0-9 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-13, 5-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1125/0-4-0, 9=1124/0-4-0
Max Horz 2=111(load case 5)
Max Uplift2=-269(load case 6), 9=-231(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1905/1042, 3-4=-1600/895, 4-5=-1386/867, 5-6=-1054/706, 6-7=-1254/704, 7-8=0/40, 7-9=-1087/693
BOT CHORD 2-13=-799/1635, 12-13=-624/1521, 11-12=-624/1521, 10-11=-624/1521, 9-10=0/80
WEBS 3-13=-294/288, 4-13=-131/407, 5-13=-300/135, 5-11=0/184, 5-10=-658/301, 6-10=-18/267, 7-10=-404/1013

JOINT STRESS INDEX
2 = 0.90, 3 = 0.34, 4 = 0.61, 5 = 0.57, 6 = 0.74, 7 = 0.74, 9 = 0.59, 10 = 0.93, 11 = 0.34, 12 = 0.51 and 13 = 0.57

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 269 lb uplift at joint 2 and 231 lb uplift at joint 9.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T04	HIP	1	1	L250193012

Job Reference (optional)

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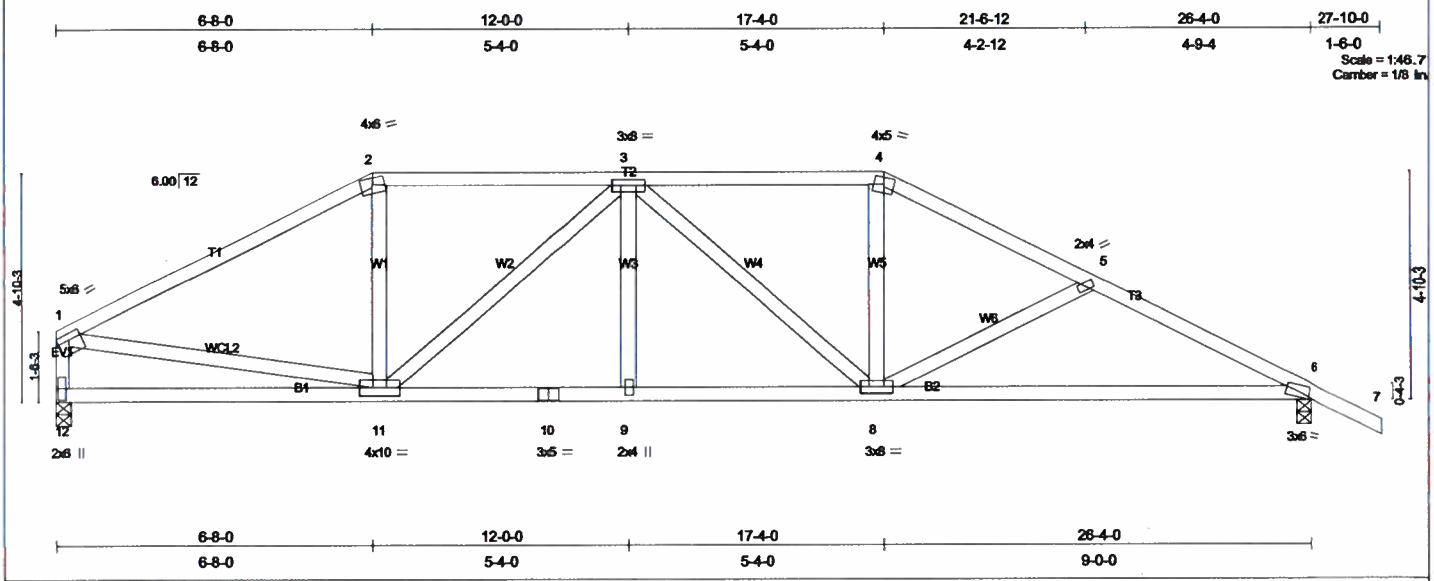

Plate Offsets (X,Y): [1:0-3-0,0-1-8], [6:0-1-1,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.14	6-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.27	6-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.35	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 141 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-2-11 oc bracing.

REACTIONS (lb/size) 12=830/0-4-0, 6=926/0-4-0
Max Horz 12=-116(load case 7)
Max Uplift12=-141(load case 5), 6=-231(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1126/606, 2-3=-940/613, 3-4=-1099/684, 4-5=-1268/703, 5-6=-1511/816, 6-7=0/35, 1-12=-790/465
BOT CHORD 11-12=-79/193, 10-11=-452/1199, 9-10=-452/1199, 8-9=-452/1199, 6-8=-589/1290
WEBS 2-11=-16/241, 3-11=-424/172, 3-9=0/114, 3-8=-233/103, 4-8=-101/323, 5-8=-231/223, 1-11=-302/755

JOINT STRESS INDEX
1 = 0.77, 2 = 0.80, 3 = 0.57, 4 = 0.73, 5 = 0.34, 6 = 0.89, 8 = 0.57, 9 = 0.34, 10 = 0.43, 11 = 0.40 and 12 = 0.66

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 12 and 231 lb uplift at joint 6.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T05	HIP	1	1	L250193013
					Job Reference (optional)

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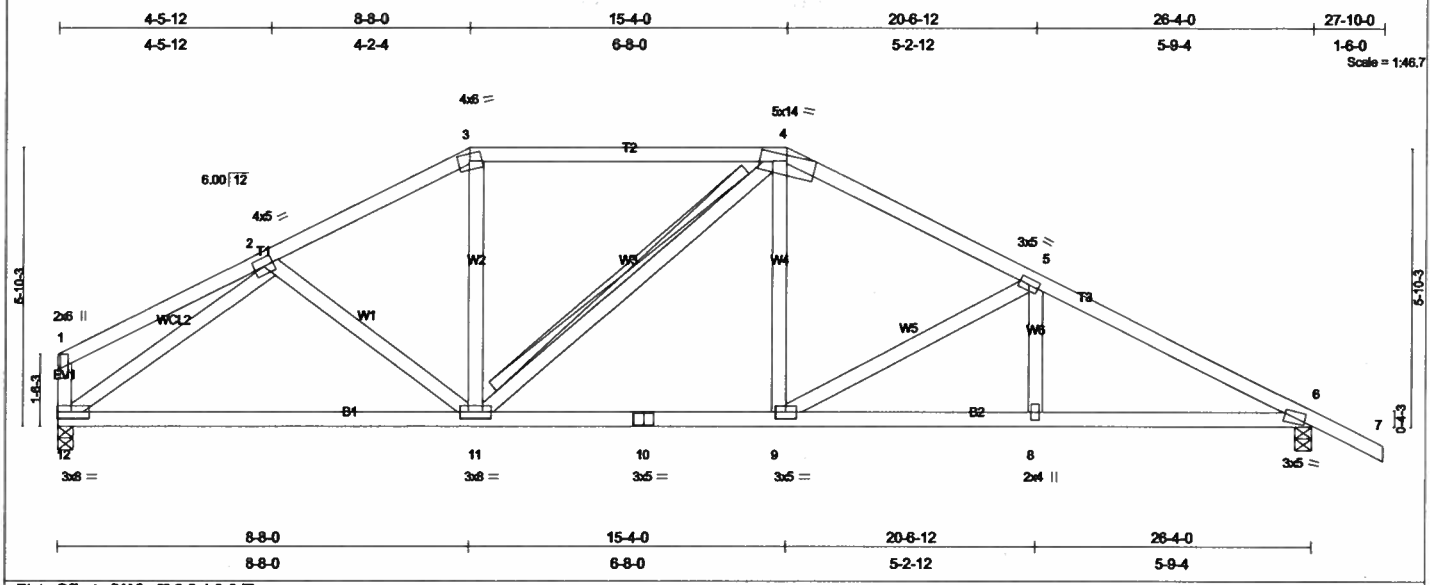

Plate Offsets (X,Y): [6:0-2-1,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.10	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.18	11-12	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 142 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-5-7 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 12=830/0-4-0, 6=926/0-4-0
Max Horz 12=-127(load case 7)
Max Uplift12=-154(load case 6), 6=-243(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-220/112, 2-3=-1039/627, 3-4=-896/615, 4-5=-1145/678, 5-6=-1511/794, 6-7=0/35, 1-12=-194/137
BOT CHORD 11-12=-347/868, 10-11=-327/977, 9-10=-327/977, 8-9=-561/1279, 6-8=-561/1279
WEBS 2-11=-32/147, 3-11=-50/226, 4-11=-199/70, 4-9=-99/285, 5-9=-351/268, 5-8=0/175, 2-12=-939/552

JOINT STRESS INDEX
1 = 0.60, 2 = 0.36, 3 = 0.69, 4 = 0.87, 5 = 0.48, 6 = 0.80, 8 = 0.34, 9 = 0.40, 10 = 0.36, 11 = 0.57 and 12 = 0.59

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 154 lb uplift at joint 12 and 243 lb uplift at joint 6.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T06	HIP	1	1	L250193014
					Job Reference (optional)

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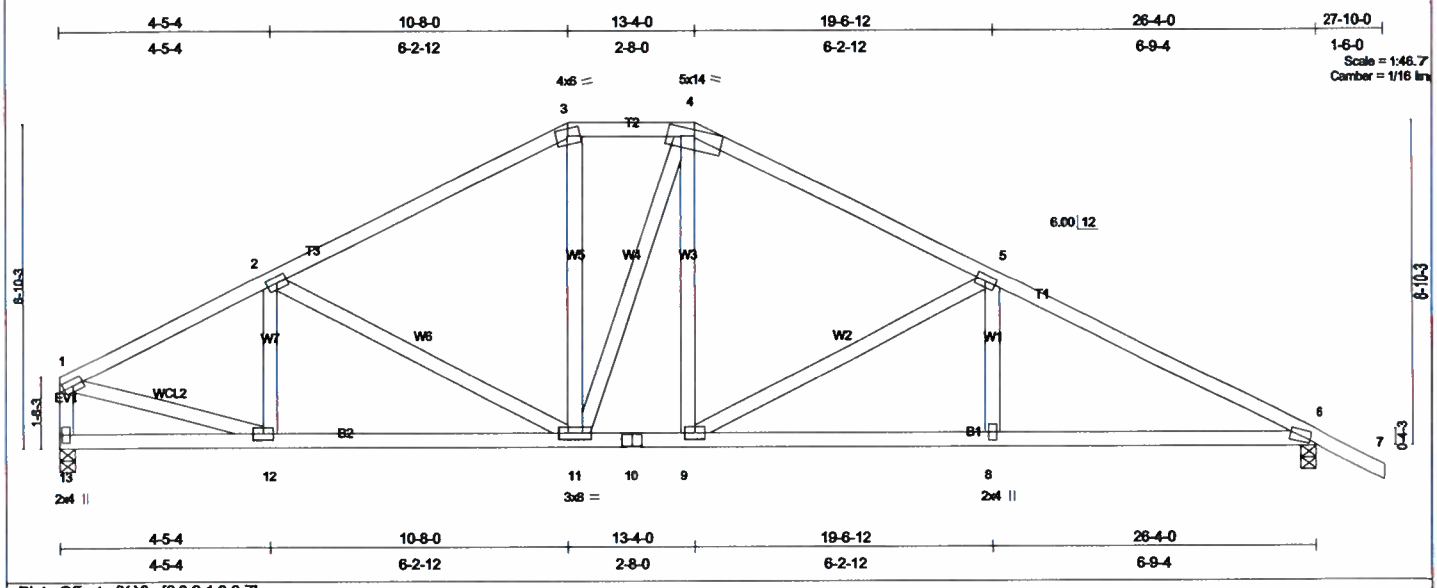

Plate Offsets (X,Y): [6:0-2-1,0-0-7]

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.32	Vert(LL)	0.07	8	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.12	8-9	>999	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 151 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-4-12 oc bracing.

REACTIONS (lb/size) 6=926/0-4-0, 13=830/0-4-0
Max Horz 13=-139(load case 7)
Max Uplift6=-253(load case 7), 13=-167(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 4-5=-1016/633, 5-6=-1491/799, 6-7=0/35, 3-4=-819/620, 1-2=-1065/605, 2-3=-989/617, 1-13=-804/469
BOT CHORD 12-13=-47/126, 11-12=-367/915, 10-11=-240/840, 9-10=-240/840, 8-9=-555/1256, 6-8=-555/1256
WEBS 5-8=0/219, 5-9=-480/361, 4-9=-144/272, 4-11=-194/111, 3-11=-82/217, 2-11=-162/159, 2-12=-206/177, 1-12=-462/883

JOINT STRESS INDEX
1 = 0.78, 2 = 0.48, 3 = 0.67, 4 = 0.58, 5 = 0.48, 6 = 0.79, 8 = 0.34, 9 = 0.40, 10 = 0.32, 11 = 0.66, 12 = 0.57 and 13 = 0.42

NOTES (8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x5 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 253 lb uplift at joint 6 and 167 lb uplift at joint 13.
8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T07	SPECIAL	1	1	L250193015
					Job Reference (optional)

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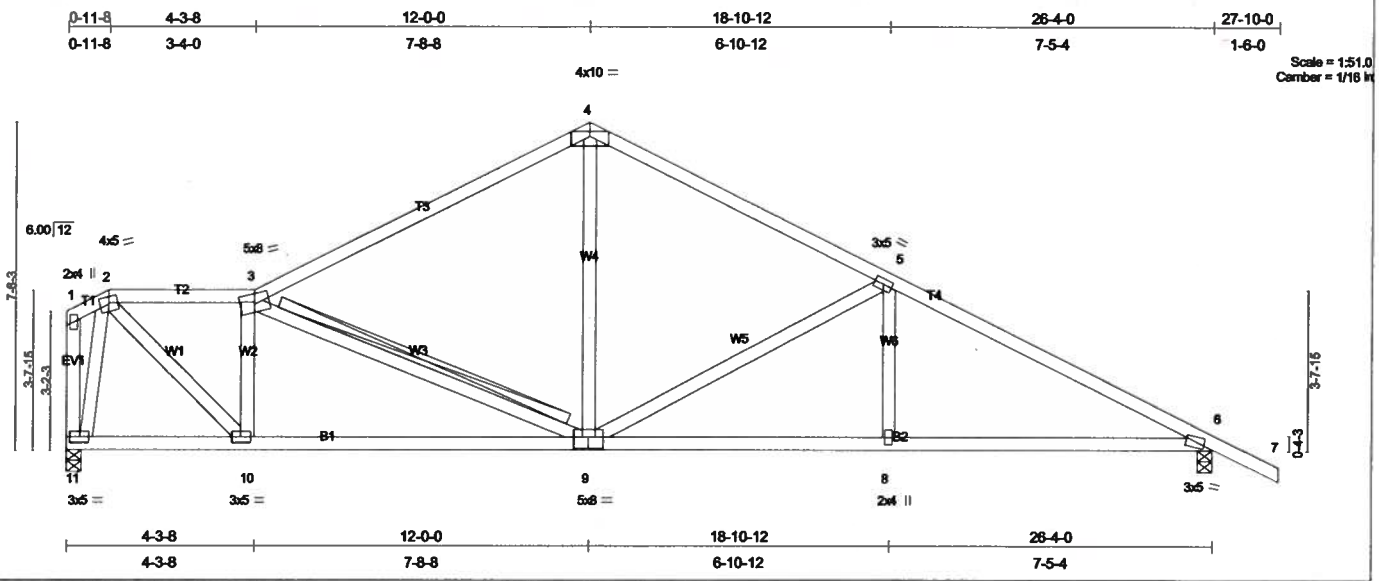

Plate Offsets (X,Y): [6:0-2-9,0-0-7], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.09	9-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.17	9-10	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 144 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-6-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 11=830/0-4-0, 6=926/0-4-0
Max Horz 11=-181(load case 7)
Max Uplift11=-178(load case 6), 6=-256(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-60/57, 2-3=-900/544, 3-4=-965/589, 4-5=-957/599, 5-6=-1461/775, 6-7=0/35, 1-11=-105/99
BOT CHORD 10-11=-15/188, 9-10=-336/930, 8-9=-523/1222, 6-8=-523/1222
WEBS 2-10=-615/1034, 3-10=-690/493, 3-9=-249/160, 4-9=-199/436, 5-9=-532/380, 5-8=0/224, 2-11=-733/380

JOINT STRESS INDEX
1 = 0.34, 2 = 0.44, 3 = 0.72, 4 = 0.99, 5 = 0.48, 6 = 0.83, 8 = 0.34, 9 = 0.42, 10 = 0.72 and 11 = 0.42

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 178 lb uplift at joint 11 and 256 lb uplift at joint 6.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T08	SPECIAL	1	1	L250193016
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 04 16:58:40 2008 Page 1

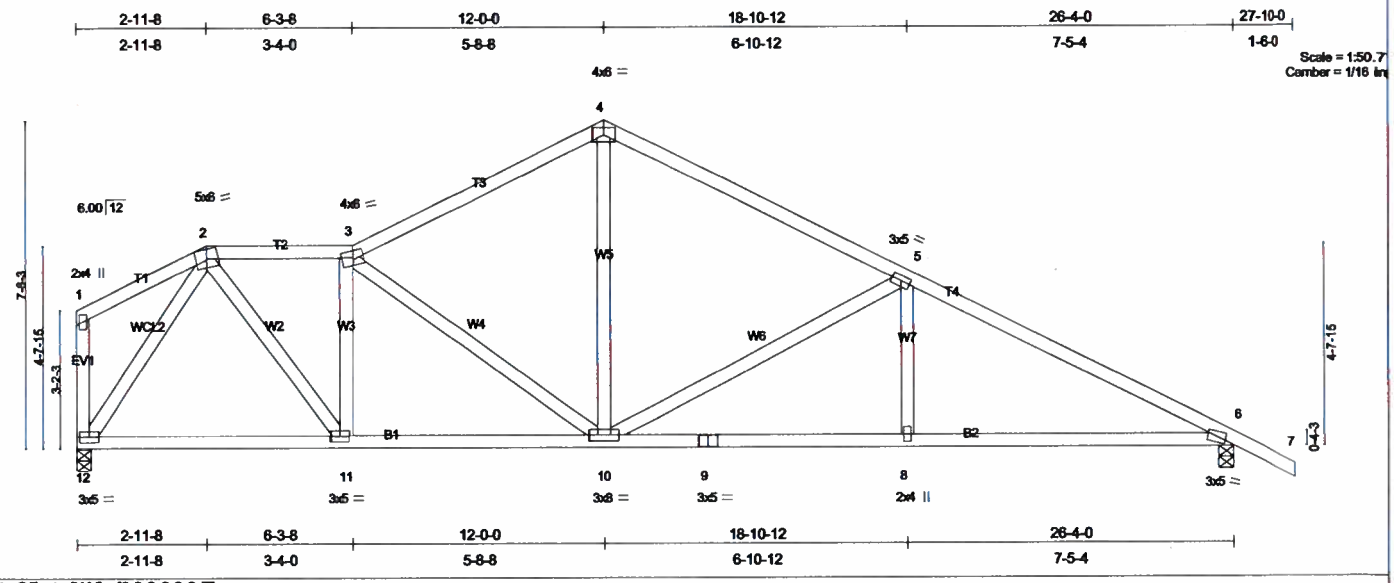

Plate Offsets (X,Y): [6:0-2-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.08	6-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.16	6-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 147 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-6-2 oc bracing.

REACTIONS (lb/size) 12=830/0-4-0, 6=926/0-4-0
Max Horz 12=-181(load case 7)
Max Uplift12=-178(load case 6), 6=-256(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-62/54, 2-3=-900/571, 3-4=-923/597, 4-5=-949/594, 5-6=-1465/775, 6-7=0/35, 1-12=-92/81
BOT CHORD 11-12=-78/481, 10-11=-304/917, 9-10=-525/1228, 8-9=-525/1228, 6-8=-525/1228
WEBS 2-11=-394/710, 3-11=-518/358, 3-10=-252/156, 4-10=-244/441, 5-10=-538/393, 5-8=0/234, 2-12=-842/478

JOINT STRESS INDEX
1 = 0.37, 2 = 0.31, 3 = 0.65, 4 = 0.78, 5 = 0.48, 6 = 0.81, 8 = 0.34, 9 = 0.43, 10 = 0.57, 11 = 0.52 and 12 = 0.44

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 178 lb uplift at joint 12 and 256 lb uplift at joint 6.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T09	SPECIAL	1	1	L250193017

Job Reference (optional)

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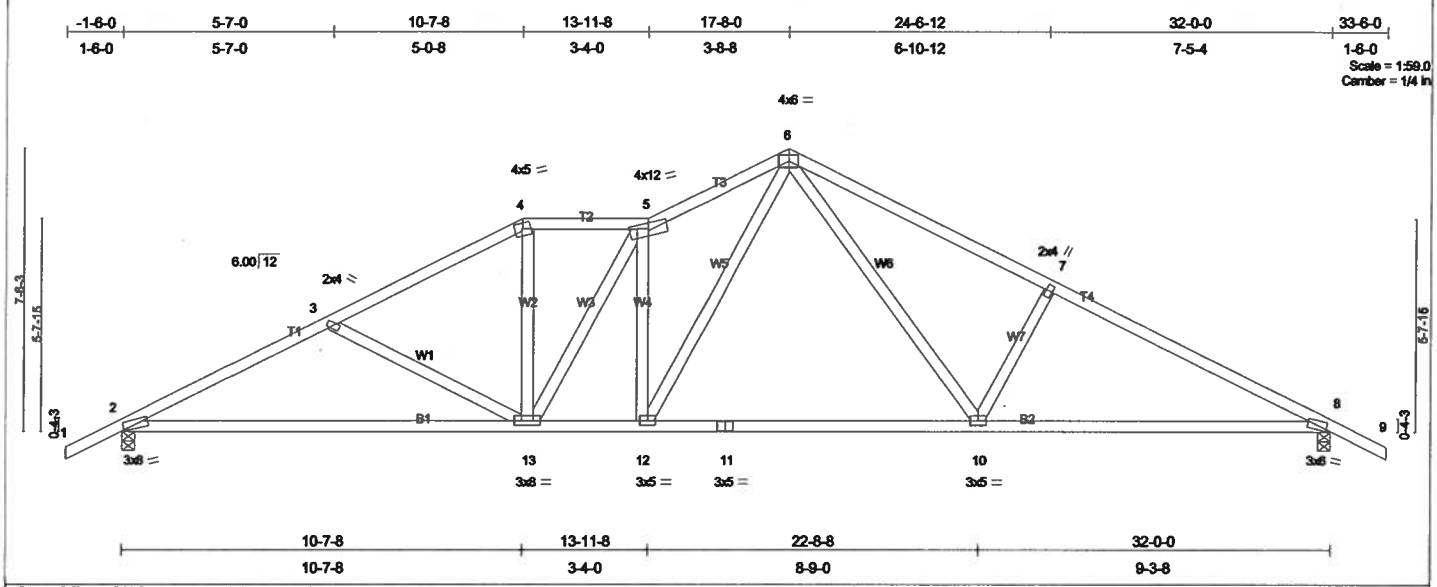

Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-1-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.28	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.52	2-13	>725	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 168 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-1-3 oc bracing.

REACTIONS (lb/size) 2=1103/0-4-0, 8=1103/0-4-0
Max Horz 2=-111(load case 7)
Max Uplift2=-305(load case 6), 8=-286(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1870/1049, 3-4=-1564/900, 4-5=-1351/867, 5-6=-1665/1089, 6-7=-1657/1033, 7-8=-1838/1009, 8-9=0/35
BOT CHORD 2-13=-788/1606, 12-13=-600/1460, 11-12=-389/1109, 10-11=-389/1109, 8-10=-734/1561
WEBS 3-13=-310/289, 4-13=-189/418, 5-13=-215/125, 5-12=-660/442, 6-12=-472/791, 6-10=-319/524, 7-10=-357/352

JOINT STRESS INDEX
2 = 0.85, 3 = 0.34, 4 = 0.78, 5 = 0.46, 6 = 0.75, 7 = 0.34, 8 = 0.81, 10 = 0.45, 11 = 0.49, 12 = 0.65 and 13 = 0.61

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 305 lb uplift at joint 2 and 286 lb uplift at joint 8.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T10	SPECIAL	1	1	L250193018

Job Reference (optional)

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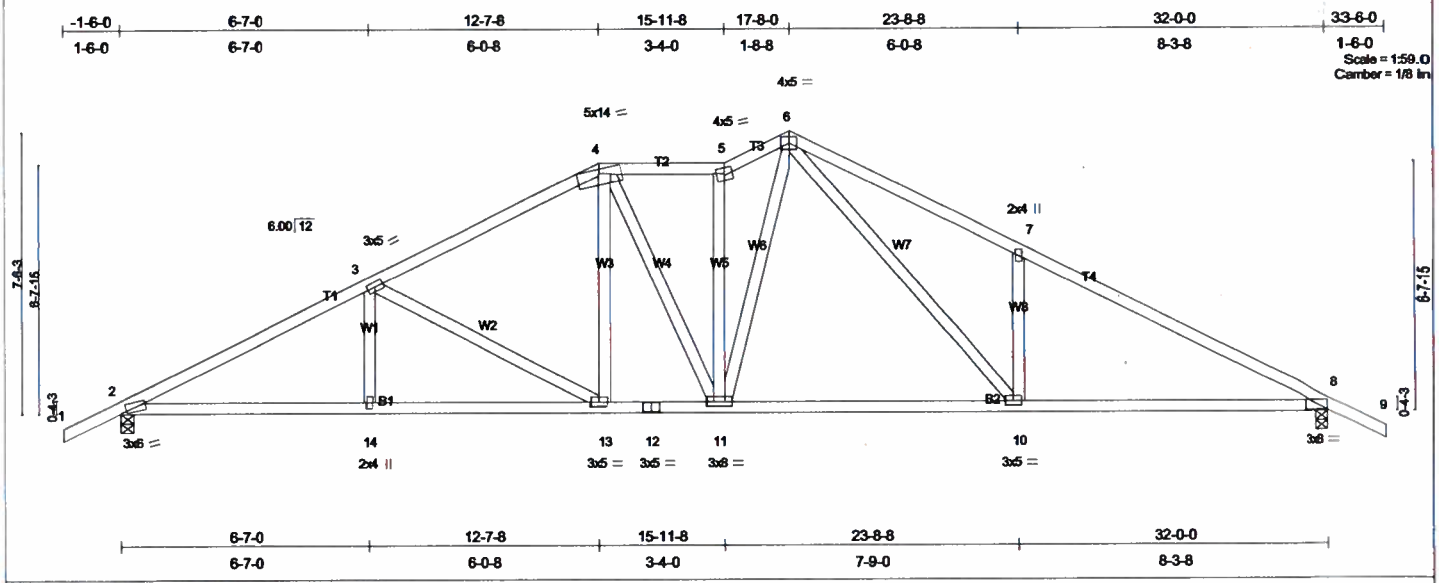

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [8:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	0.15	8-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.26	8-10	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 178 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-4 oc bracing.

REACTIONS (lb/size) 2=1103/0-4-0, 8=1103/0-4-0
Max Horz 2=-111(load case 7)
Max Uplift2=-305(load case 6), 8=-286(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1878/1010, 3-4=-1431/860, 4-5=-1244/836, 5-6=-1381/928, 6-7=-1793/1182, 7-8=-1815/975, 8-9=0/35
BOT CHORD 2-14=-744/1600, 13-14=-744/1600, 12-13=-454/1217, 11-12=-454/1217, 10-11=-376/1105, 8-10=-690/1529
WEBS 3-14=0/212, 3-13=-450/333, 4-13=-139/263, 4-11=-67/161, 5-11=-603/381, 6-11=-350/647, 6-10=-487/674, 7-10=-405/406

JOINT STRESS INDEX
2 = 0.80, 3 = 0.48, 4 = 0.67, 5 = 0.66, 6 = 0.71, 7 = 0.34, 8 = 0.73, 10 = 0.48, 11 = 0.84, 12 = 0.41, 13 = 0.40 and 14 = 0.34

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 305 lb uplift at joint 2 and 286 lb uplift at joint 8.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T11	HIP	1	1	L250193019
					Job Reference (optional)

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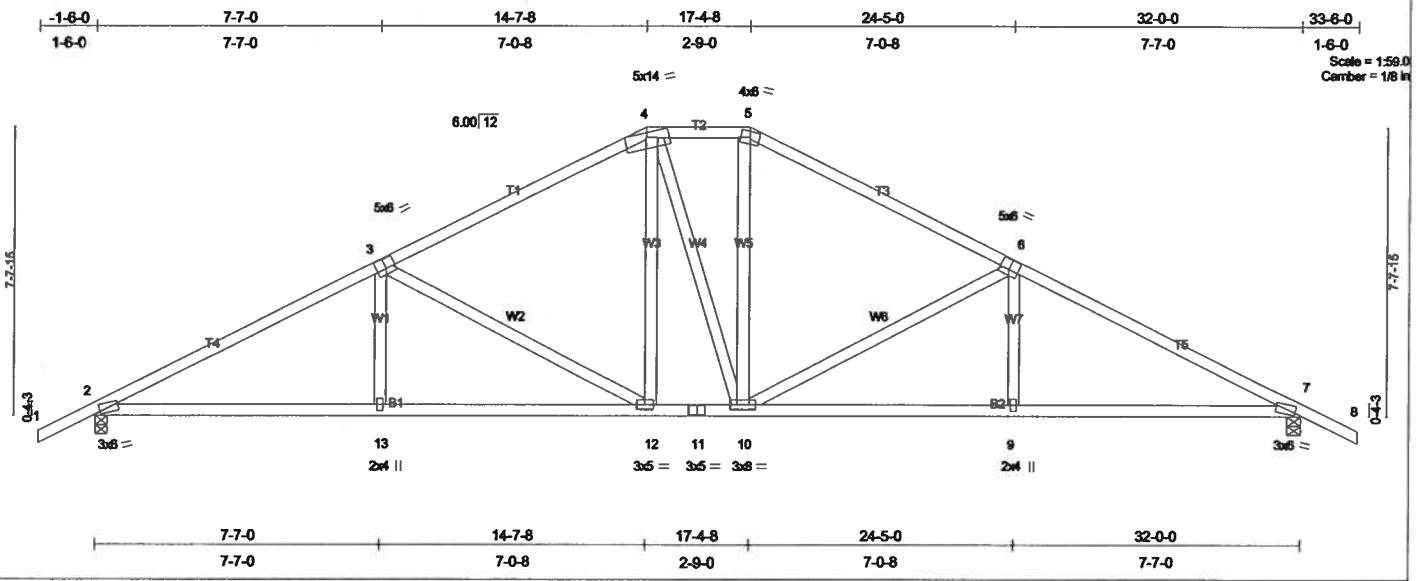

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [3:0-3-0,0-3-4], [6:0-3-0,0-3-4], [7:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.12	12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.20	12-13	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 172 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-3 oc bracing.

REACTIONS (lb/size) 2=1103/0-4-0, 7=1103/0-4-0
Max Horz 2=-113(load case 7)
Max Uplift2=-287(load case 6), 7=-287(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1847/998, 3-4=-1309/806, 4-5=-1094/794, 5-6=-1310/807, 6-7=-1847/998, 7-8=0/35
BOT CHORD 2-13=-721/1566, 12-13=-722/1564, 11-12=-361/1093, 10-11=-361/1093, 9-10=-722/1563, 7-9=-721/1565
WEBS 3-13=0/247, 3-12=-546/414, 4-12=-168/313, 4-10=-176/183, 5-10=-168/313, 6-10=-544/413, 6-9=0/246

JOINT STRESS INDEX
2 = 0.81, 3 = 0.75, 4 = 0.72, 5 = 0.73, 6 = 0.74, 7 = 0.81, 9 = 0.34, 10 = 0.67, 11 = 0.41, 12 = 0.40 and 13 = 0.34

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 2 and 287 lb uplift at joint 7.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T12	COMMON	3	1	L250193020
					Job Reference (optional)

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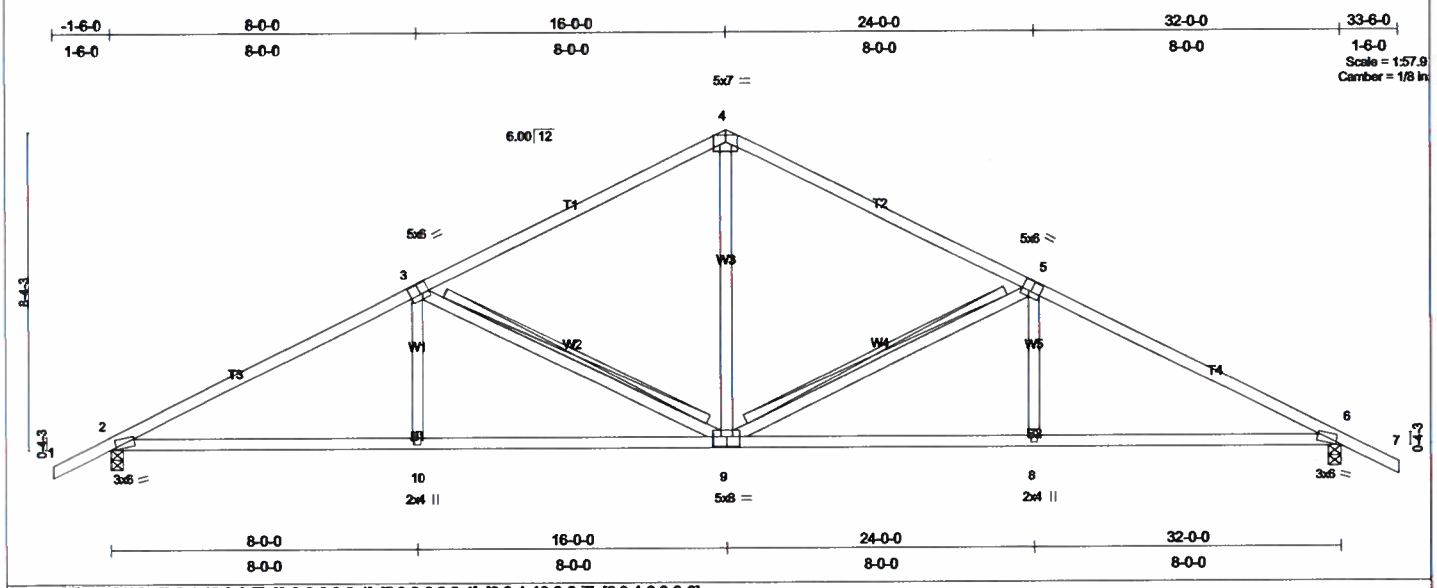

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [3:0-3-0,0-3-4], [5:0-3-0,0-3-4], [6:0-1-13,0-0-7], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.13	2-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.22	2-10	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.08	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 155 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-2-9 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-9, 5-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1103/0-4-0, 6=1103/0-4-0
Max Horz 2=121(load case 6)
Max Uplift2=-294(load case 6), 6=-294(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1835/1003, 3-4=-1248/785, 4-5=-1248/785, 5-6=-1835/1003, 6-7=0/35
BOT CHORD 2-10=-721/1552, 9-10=-722/1551, 8-9=-722/1551, 6-8=-721/1552
WEBS 3-10=0/257, 3-9=-614/447, 4-9=-351/604, 5-9=-614/447, 5-8=0/257

JOINT STRESS INDEX
2 = 0.85, 3 = 0.82, 4 = 0.71, 5 = 0.82, 6 = 0.85, 8 = 0.34, 9 = 0.47 and 10 = 0.34

NOTES (6)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 294 lb uplift at joint 2 and 294 lb uplift at joint 6.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE	
L250193	T13	SPECIAL	1	1	L250193021	J1876063
					Job Reference (optional)	

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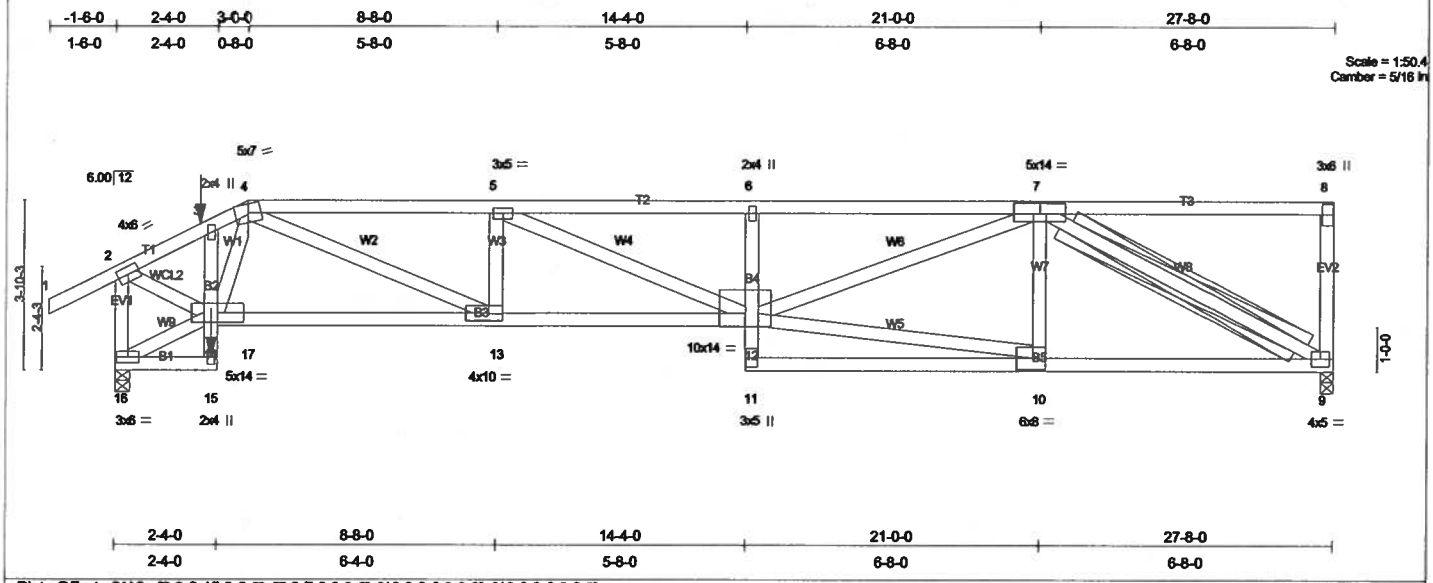


Plate Offsets (X,Y): [2:0-2-15,0-2-0], [7:0-7-0,0-3-0], [10:0-3-8,0-3-0], [13:0-3-8,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.35	12-13	>932	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.68	12-13	>481	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.89	Horz(TL)	0.23	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except*
T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B3 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
EV2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-2-15 oc bracing.
WEBS I-Brace: 2 X 4 SYP No.3 - 7-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 9=1926/0-3-8, 16=2198/0-4-0
Max Horz 16=137(load case 5)
Max Uplift9=-665(load case 4), 16=-670(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-2165/729, 3-4=-2007/653, 4-5=-4650/1614, 5-6=-5287/1837, 6-7=-5181/1812, 7-8=-120/41, 8-9=-358/183, 2-16=-2175/690
BOT CHORD 15-16=-53/11, 14-15=-103/358, 3-14=-181/264, 14-17=-794/2213, 13-17=-794/2213, 12-13=-1614/4650, 11-12=0/140, 6-12=-710/368, 10-11=-133/472, 9-10=-979/2834
WEBS 4-14=-906/466, 4-13=-915/2671, 5-13=-936/452, 5-12=-260/696, 10-12=-856/2390, 7-12=-894/2521, 7-10=-258/240, 7-9=-3092/1069, 2-14=-673/2134, 14-16=-51/75

JOINT STRESS INDEX
2 = 0.86, 3 = 0.47, 4 = 0.82, 5 = 0.46, 6 = 0.54, 7 = 0.65, 8 = 0.84, 9 = 0.99, 10 = 0.54, 11 = 0.78, 12 = 0.78, 13 = 0.57, 14 = 0.67, 15 = 0.34 and 16 = 0.71

NOTES (9)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 665 lb uplift at joint 9 and 670 lb uplift at joint 16.
6) Girder carries hip end with 0-0-0 right side setback, 3-0-0 left side setback, and 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 74 lb down and 20 lb up at 2-0-0 on top chord, and 362 lb down and 100 lb up at 2-2-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-4=-54, 4-8=-118(F=-64), 15-16=-10, 14-17=-10, 12-17=-22(F=-12), 9-11=-22(F=-12)
Concentrated Loads (lb)
Vert: 15=-362(F) 3=-74(F)

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T14	SPECIAL	1	1	L250193022

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 04 16:58:47 2008 Page 1

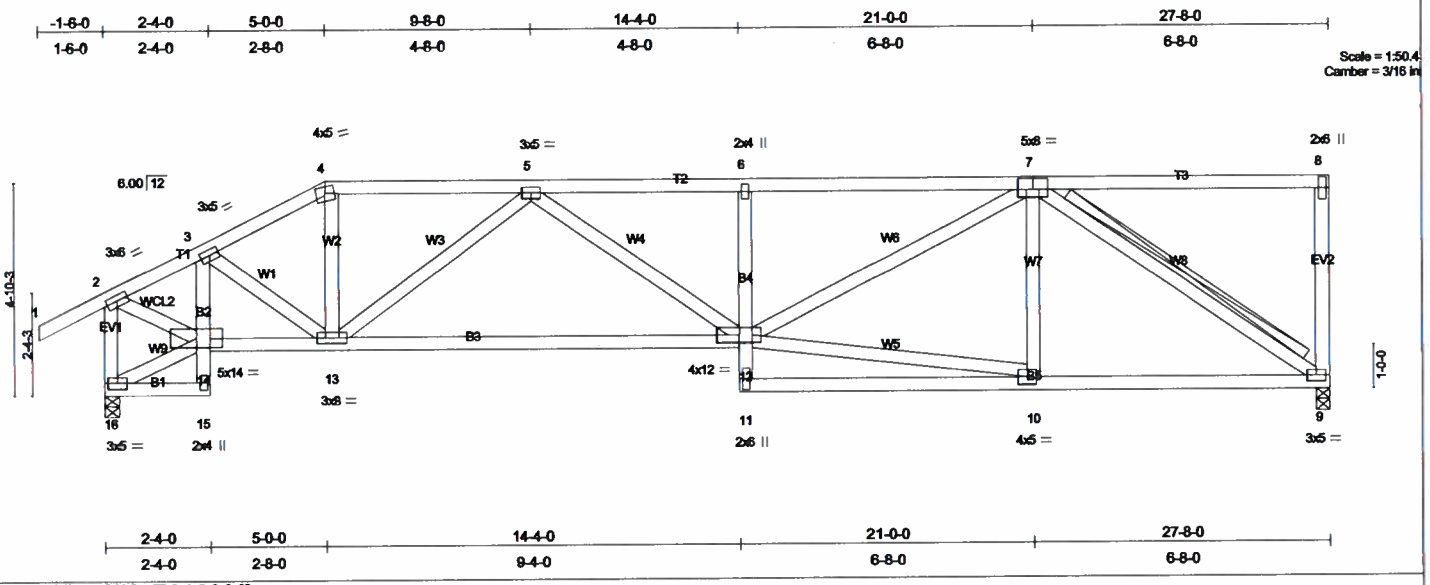


Plate Offsets (X,Y): [7:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.16	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.32	12-13	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.08	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 175 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-7-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 7-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 9=873/0-3-8, 16=968/0-4-0
Max Horz 16=169(load case 6)
Max Uplift9=-247(load case 5), 16=-207(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-872/597, 3-4=-1154/679, 4-5=-1030/652, 5-6=-1719/945, 6-7=-1704/948, 7-8=-38/20, 8-9=-161/114, 2-16=-932/676
BOT CHORD 15-16=-39/59, 14-15=-12/32, 3-14=-395/96, 13-14=-631/756, 12-13=-910/1525, 11-12=0/95, 6-12=-303/217, 10-11=-72/82, 9-10=-545/1003
WEBS 3-13=-74/322, 4-13=-135/303, 5-13=-630/329, 5-12=-43/256, 10-12=-479/932, 7-12=-459/799, 7-10=-88/126, 7-9=-1178/641, 2-14=-457/839, 14-16=-184/0

JOINT STRESS INDEX
2 = 0.65, 3 = 0.48, 4 = 0.50, 5 = 0.40, 6 = 0.34, 7 = 0.45, 8 = 0.67, 9 = 0.48, 10 = 0.45, 11 = 0.62, 12 = 0.72, 13 = 0.57, 14 = 0.53, 15 = 0.34 and 16 = 0.49

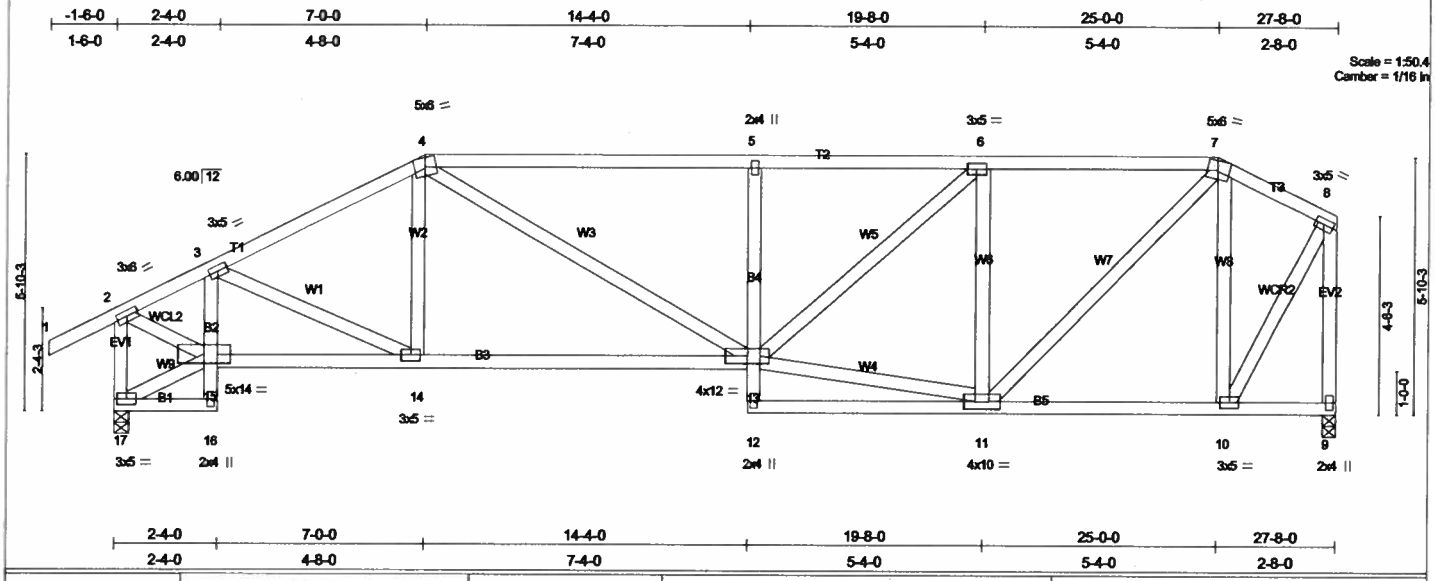
NOTES (6)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 247 lb uplift at joint 9 and 207 lb uplift at joint 16.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T15	SPECIAL	1	1	L250193023

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 04 16:58:48 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	0.09	13-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.18	13-14	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 190 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
7-11-4 oc bracing: 14-15
8-0-6 oc bracing: 12-14.

REACTIONS (lb/size) 9=873/0-3-8, 17=968/0-4-0
Max Horz 17=175(load case 6)
Max Uplift9=-201(load case 4), 17=-208(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-881/598, 3-4=-1195/695, 4-5=-1362/813, 5-6=-1329/797, 6-7=-904/551, 7-8=-423/243, 2-17=-935/669, 8-9=-863/476
BOT CHORD 16-17=-54/37, 15-16=-14/29, 3-15=-375/134, 14-15=-638/787, 13-14=-620/1032, 12-13=0/75, 5-13=-360/250, 11-12=-74/86, 10-11=-201/380, 9-10=-5/6
WEBS 3-14=-99/330, 4-14=-71/126, 4-13=-150/461, 11-13=-420/832, 6-13=-328/568, 6-11=-657/415, 7-11=-411/764, 7-10=-605/367, 2-15=-470/855, 8-10=-384/739, 15-17=-147/0

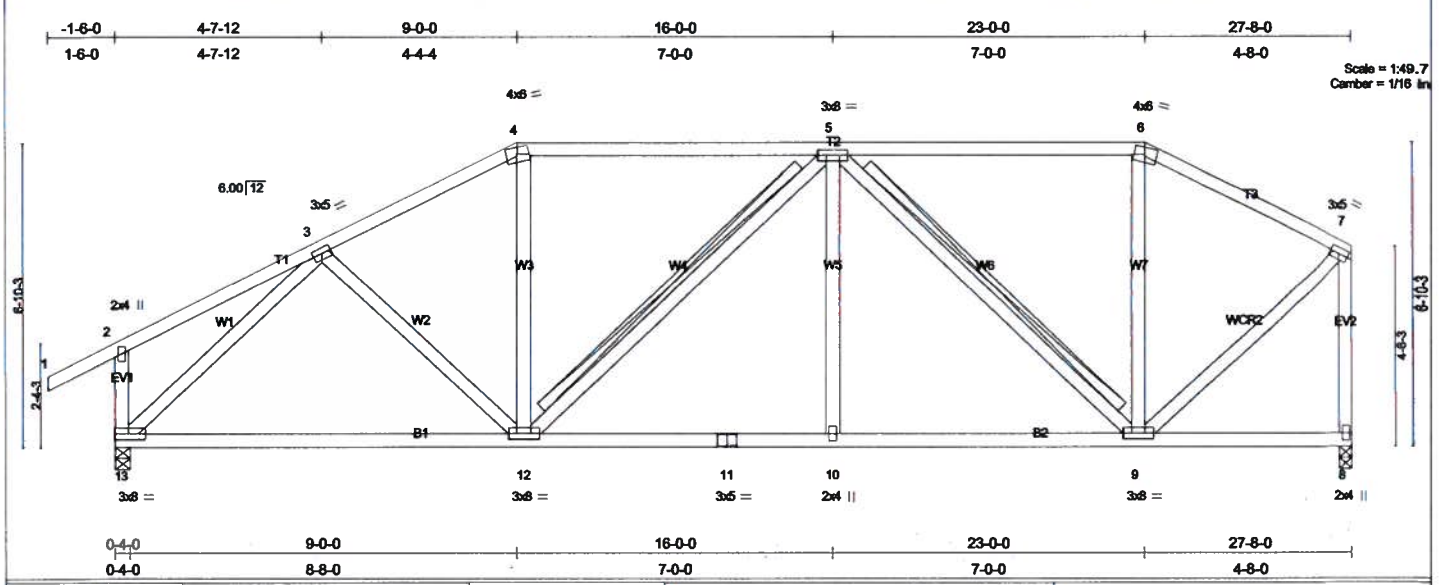
JOINT STRESS INDEX
2 = 0.71, 3 = 0.69, 4 = 0.58, 5 = 0.87, 6 = 0.40, 7 = 0.30, 8 = 0.62, 9 = 0.34, 10 = 0.62, 11 = 0.44, 12 = 0.75, 13 = 0.67, 14 = 0.40, 15 = 0.32, 16 = 0.34 and 17 = 0.50

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 201 lb uplift at joint 9 and 208 lb uplift at joint 17.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T16	HIP	1	1	L250193024
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 04 16:58:49 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.11	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.21	12-13	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.03	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 178 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-12, 5-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 13=968/0-4-0, 8=873/0-3-8
Max Horz 13=187(load case 6)
Max Uplift13=-222(load case 6), 8=-170(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-139/171, 3-4=-968/593, 4-5=-830/585, 5-6=-543/396, 6-7=-650/379, 2-13=-253/297, 7-8=-852/492
BOT CHORD 12-13=-516/744, 11-12=-501/929, 10-11=-501/929, 9-10=-501/929, 8-9=-12/13
WEBS 3-13=-959/448, 3-12=-47/202, 4-12=-28/204, 5-12=-223/94, 5-10=0/196, 5-9=-564/305, 6-9=-76/102, 7-9=-366/720

JOINT STRESS INDEX
2 = 0.51, 3 = 0.48, 4 = 0.67, 5 = 0.57, 6 = 0.69, 7 = 0.73, 8 = 0.34, 9 = 0.71, 10 = 0.34, 11 = 0.33, 12 = 0.57 and 13 = 0.58

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 13 and 170 lb uplift at joint 8.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T17	SPECIAL	1	1	L250193025
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 04 16:58:50 2008 Page 1

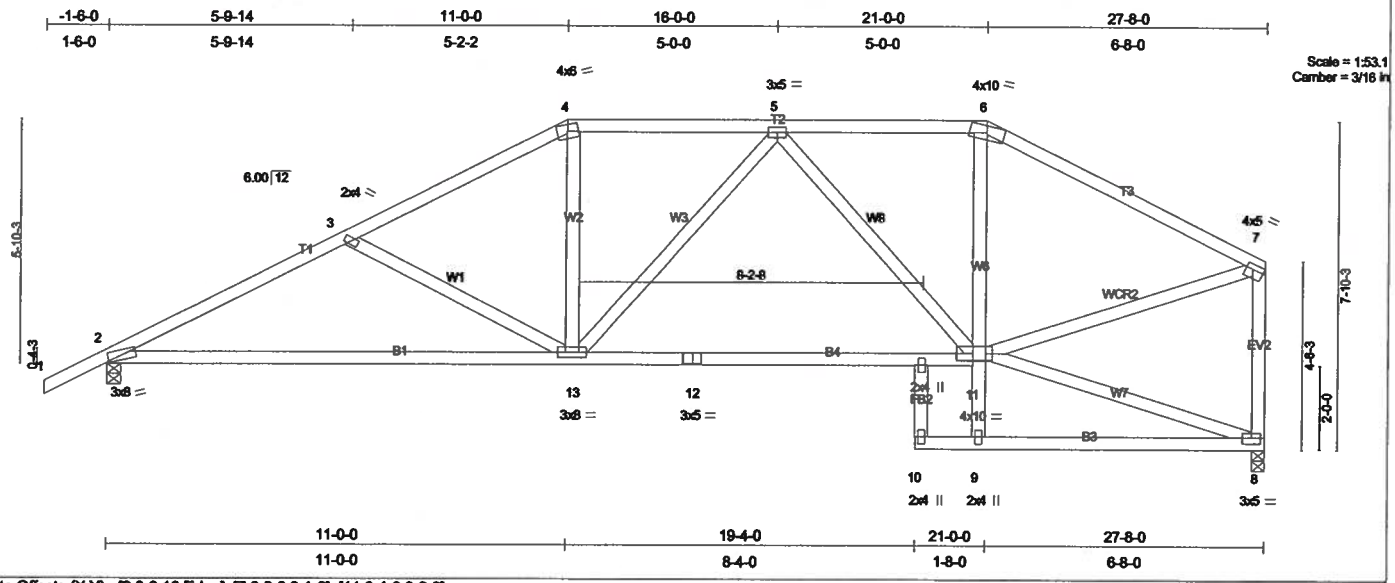


Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-2-0,0-1-8], [11:0-4-8,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.24	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.45	2-13	>723	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-1-5 oc bracing.

REACTIONS (lb/size) 2=972/0-4-0, 8=883/0-3-8
Max Horz 2=148(load case 6)
Max Uplift2=-245(load case 6), 8=-138(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1582/850, 3-4=-1262/693, 4-5=-1076/681, 5-6=-843/569, 6-7=-1012/553, 7-8=-853/499
BOT CHORD 2-13=-783/1349, 12-13=-523/1061, 11-12=-523/1061, 9-10=0/0, 8-9=0/13
WEBS 3-13=-319/306, 4-13=-81/334, 5-13=-107/109, 9-11=0/136, 6-11=-7/213, 7-11=-391/835, 8-11=-34/40, 5-11=-413/185

JOINT STRESS INDEX
2 = 0.77, 3 = 0.34, 4 = 0.63, 5 = 0.41, 6 = 0.58, 7 = 0.74, 8 = 0.75, 9 = 0.34, 10 = 0.34, 11 = 0.64, 12 = 0.38, 13 = 0.57 and 14 = 0.34

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 245 lb uplift at joint 2 and 138 lb uplift at joint 8.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T18	SPECIAL	1	1	L250193026
					Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 04 16:58:51 2008 Page 1

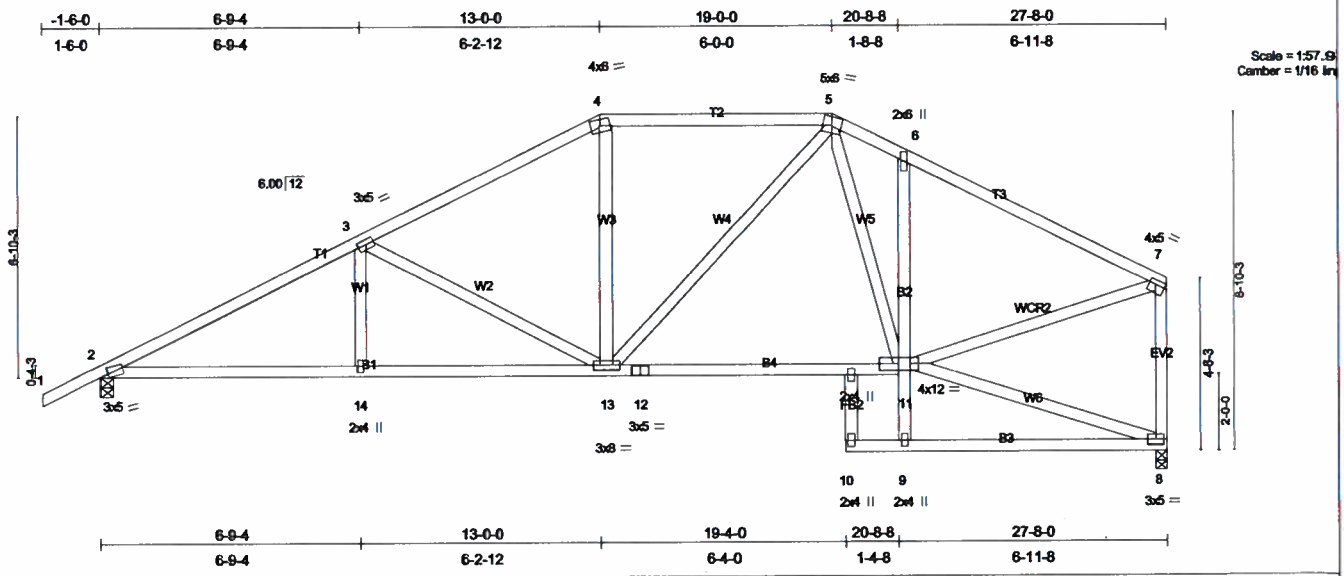


Plate Offsets (X,Y): [2:0-2-5,0-0-7], [7:0-2-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.08	11-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.18	11-13	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.42	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 177 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=972/0-4-0, 8=884/0-3-8
Max Horz 2=159(load case 6)
Max Uplift2=-256(load case 6), 8=-152(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1585/826, 3-4=-1129/664, 4-5=-944/661, 5-6=-948/673, 6-7=-1013/560, 7-8=-862/507
BOT CHORD 2-14=-752/1339, 13-14=-752/1339, 12-13=-363/806, 11-12=-363/806, 9-11=0/147, 6-11=-238/271, 9-10=0/0, 8-9=-14/12
WEBS 3-14=0/200, 3-13=-455/350, 4-13=-34/251, 5-13=-121/275, 5-11=-113/147, 7-11=-394/833, 8-11=-50/64

JOINT STRESS INDEX
2 = 0.81, 3 = 0.48, 4 = 0.80, 5 = 0.33, 6 = 0.43, 7 = 0.77, 8 = 0.65, 9 = 0.76, 10 = 0.34, 11 = 0.58, 12 = 0.32, 13 = 0.57, 14 = 0.34 and 15 = 0.34

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 256 lb uplift at joint 2 and 152 lb uplift at joint 8.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T19	SPECIAL	1	1	L250193027

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 04 16:58:53 2008 Page 1

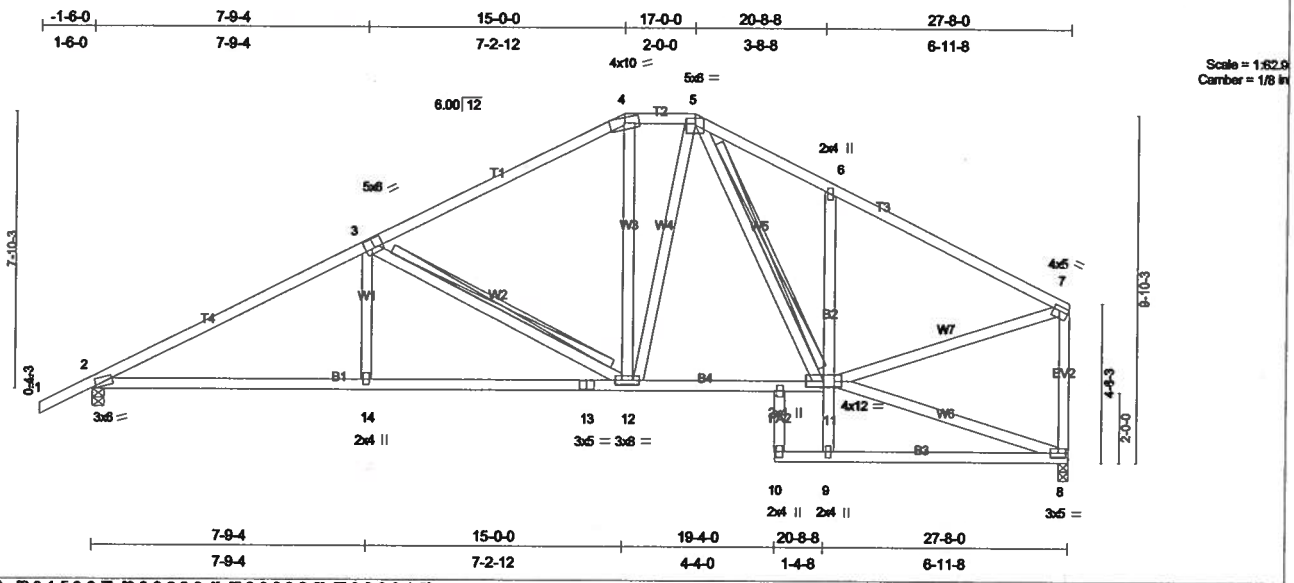


Plate Offsets (X,Y): [2:0-1-5,0-0-7], [3:0-3-0,0-3-4], [5:0-3-0,0-2-0], [7:0-2-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	0.10	2-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.18	2-14	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.35	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 182 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-2-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-12, 5-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=972/0-4-0, 8=884/0-3-8
Max Horz 2=171(load case 6)
Max Uplift2=-264(load case 6), 8=-178(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1557/823, 3-4=-1004/622, 4-5=-815/630, 5-6=-985/724, 6-7=-1009/574, 7-8=-858/516
BOT CHORD 2-14=-738/1307, 13-14=-738/1307, 12-13=-738/1307, 11-12=-325/756, 9-11=0/149, 6-11=-324/333, 9-10=0/0, 8-9=0/27
WEBS 3-14=0/246, 3-12=-565/425, 4-12=-13/215, 5-12=-159/330, 5-11=-183/214, 8-11=-52/47, 7-11=-409/833

JOINT STRESS INDEX
2 = 0.83, 3 = 0.75, 4 = 0.56, 5 = 0.35, 6 = 0.64, 7 = 0.75, 8 = 0.63, 9 = 0.85, 10 = 0.34, 11 = 0.58, 12 = 0.61, 13 = 0.44, 14 = 0.34 and 15 = 0.34

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 264 lb uplift at joint 2 and 178 lb uplift at joint 8.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T20	HIP	1	1	L250193028

Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 04 16:58:54 2008 Page 1

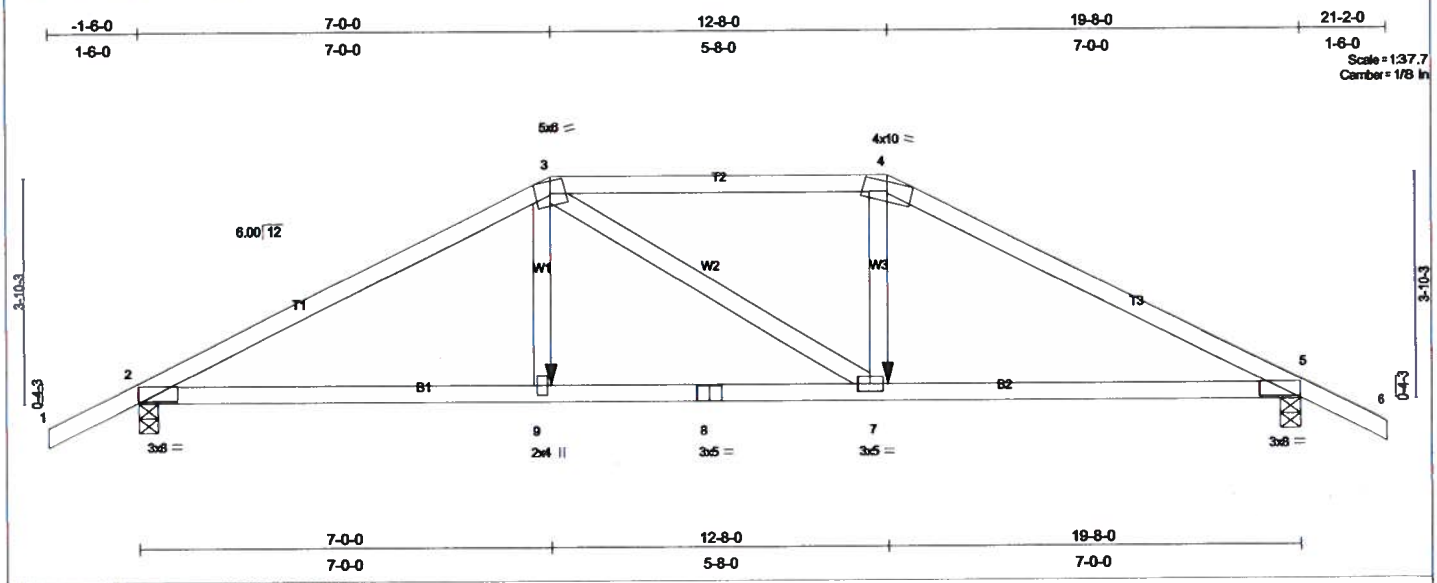


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [5:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.08	7-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.18	7-9	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.18	Horz(TL)	0.07	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 85 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-1-1 oc bracing.

REACTIONS (lb/size) 2=1339/0-4-0, 5=1326/0-4-0
Max Horz 2=68(load case 5)
Max Uplift2=-446(load case 5), 5=-442(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2391/713, 3-4=-2050/669, 4-5=-2360/705, 5-6=0/35
BOT CHORD 2-9=-611/2056, 8-9=-606/2035, 7-8=-606/2035, 5-7=-567/2029
WEBS 3-9=-111/521, 3-7=-114/149, 4-7=-127/566

JOINT STRESS INDEX
2 = 0.75, 3 = 0.70, 4 = 0.96, 5 = 0.74, 7 = 0.43, 8 = 0.75 and 9 = 0.38

NOTES (10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 446 lb uplift at joint 2 and 442 lb uplift at joint 5.
7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 411 lb down and 165 lb up at 12-8-0, and 411 lb down and 165 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-9=-10, 7-9=-22(F=-12), 5-7=-10
Concentrated Loads (lb)
Vert: 9=-411(F) 7=-411(F)

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T21	HIP	1	1	L250193029
					Job Reference (optional)

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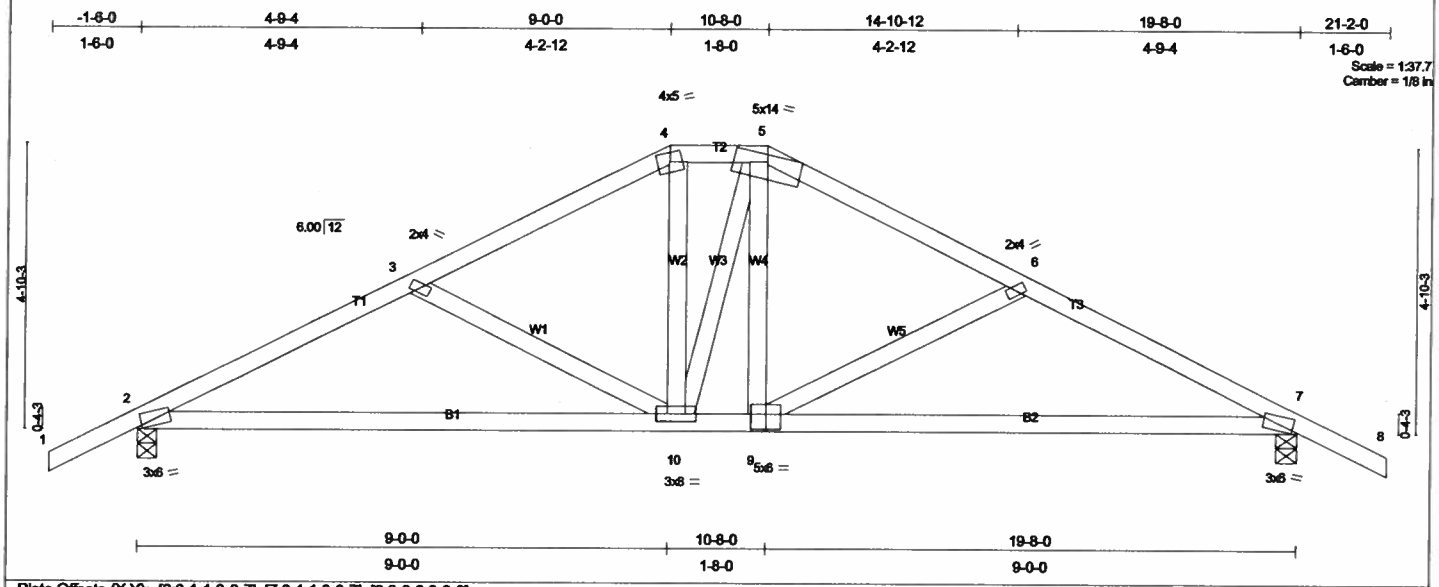

Plate Offsets (X,Y): [2:0-1-1,0-0-7], [7:0-1-1,0-0-7], [9:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.13	7-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.24	7-9	>962	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.12	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 101 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=709/0-4-0, 7=709/0-4-0
Max Horz 2=-80(load case 7)
Max Uplift2=-204(load case 6), 7=-204(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1055/584, 3-4=-792/458, 4-5=-663/460, 5-6=-791/458, 6-7=-1055/584, 7-8=0/35
BOT CHORD 2-10=-385/889, 9-10=-170/662, 7-9=-385/889
WEBS 3-10=-265/244, 4-10=-77/206, 5-10=-108/116, 5-9=-77/207, 6-9=-266/245

JOINT STRESS INDEX
2 = 0.89, 3 = 0.34, 4 = 0.47, 5 = 0.31, 6 = 0.34, 7 = 0.90, 9 = 0.75 and 10 = 0.68

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 204 lb uplift at joint 2 and 204 lb uplift at joint 7.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T22	COMMON	4	1	L250193030

Job Reference (optional)

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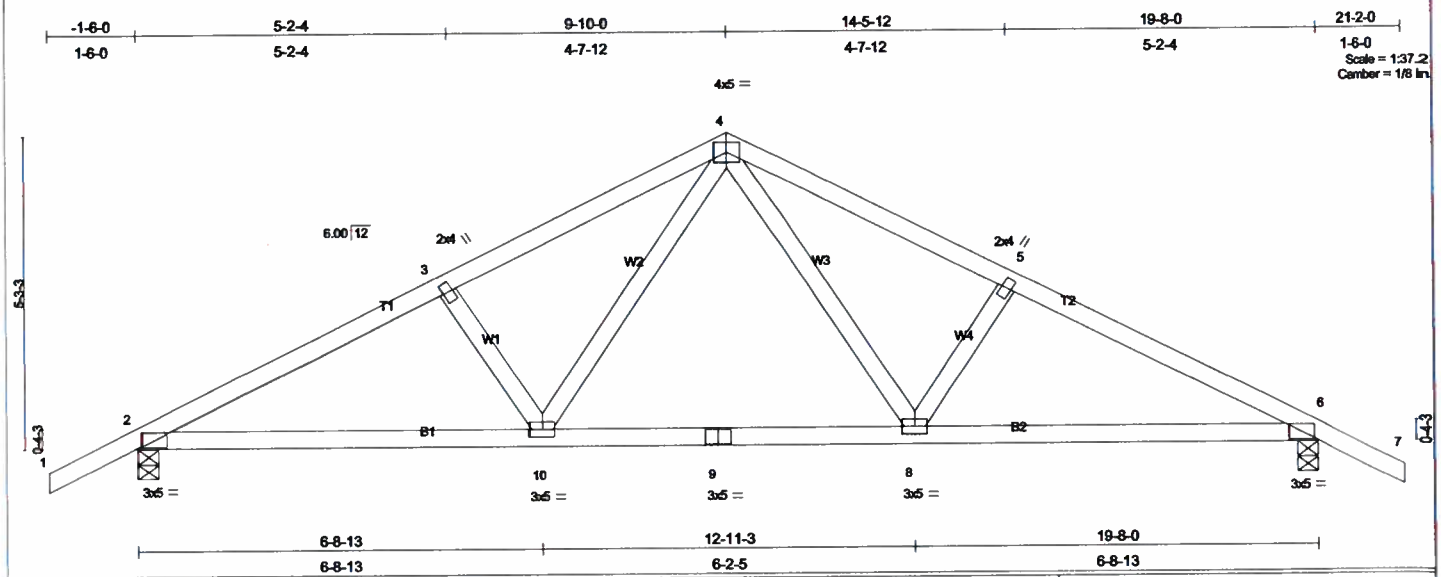

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.14	8-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.22	8-10	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.17	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 92 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-5-5 oc bracing.

REACTIONS (lb/size) 2=864/0-4-0, 6=864/0-4-0
Max Horz 2=85(load case 6)
Max Uplift2=-251(load case 6), 6=-251(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1416/774, 3-4=-1274/773, 4-5=-1274/773, 5-6=-1416/774, 6-7=0/35
BOT CHORD 2-10=-548/1198, 9-10=-287/832, 8-9=-287/832, 6-8=-548/1198
WEBS 3-10=-217/211, 4-10=-280/503, 4-8=-280/503, 5-8=-217/211

JOINT STRESS INDEX
2 = 0.76, 3 = 0.34, 4 = 0.64, 5 = 0.34, 6 = 0.76, 8 = 0.43, 9 = 0.77 and 10 = 0.43

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 2 and 251 lb uplift at joint 6.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-60(F=-50), 6-8=-10

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T22A	COMMON	2	1	L250193031

Job Reference (optional)

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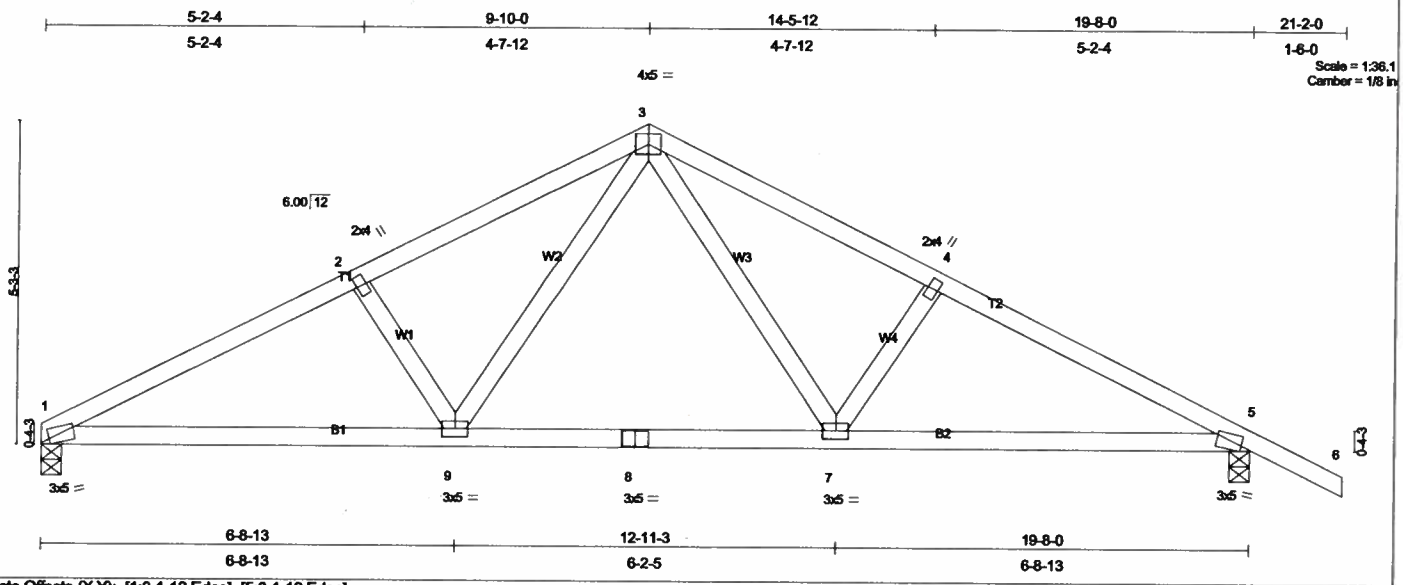

Plate Offsets (X,Y): [1:0-1-12,Edge], [5:0-1-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.14	7-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.21	7-9	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.19	Horz(TL)	0.04	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 90 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-0-12 oc bracing.

REACTIONS (lb/size) 1=770/0-4-0, 5=867/0-4-0
Max Horz 1=-94(load case 7)
Max Uplift1=-177(load case 6), 5=-252(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1438/811, 2-3=-1296/809, 3-4=-1281/785, 4-5=-1423/786, 5-6=0/35
BOT CHORD 1-9=-588/1222, 8-9=-300/839, 7-8=-300/839, 5-7=-559/1204
WEBS 2-9=-228/229, 3-9=-314/524, 3-7=-276/502, 4-7=-217/212

JOINT STRESS INDEX
1 = 0.77, 2 = 0.34, 3 = 0.62, 4 = 0.34, 5 = 0.77, 7 = 0.43, 8 = 0.76 and 9 = 0.43

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 177 lb uplift at joint 1 and 252 lb uplift at joint 5.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 1-9=-10, 7-9=-60(F=-50), 5-7=-10

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T24	COMMON	4	1	L250193032

Job Reference (optional)

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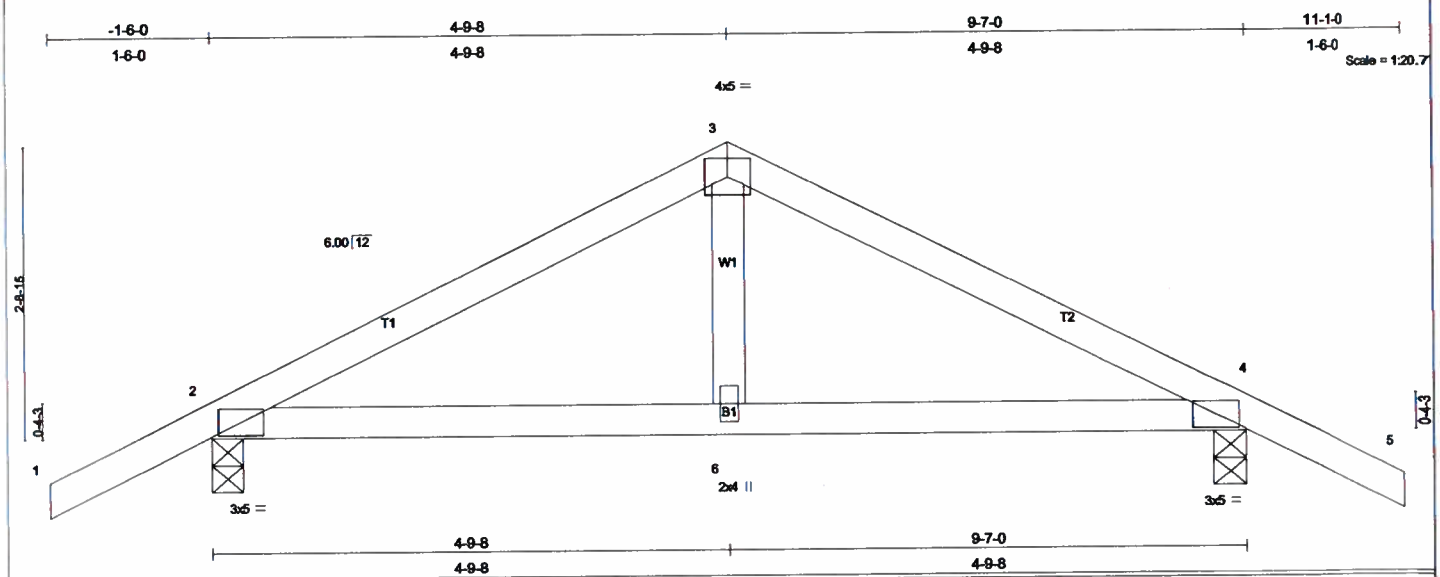

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	0.04	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.02	2-6	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.05	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 39 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-9 oc bracing.

REACTIONS (lb/size) 2=386/0-3-8, 4=386/0-3-8
Max Horz 2=-55(load case 7)
Max Uplift2=-257(load case 6), 4=-257(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-400/601, 3-4=-400/601, 4-5=0/35
BOT CHORD 2-6=-399/305, 4-6=-399/305
WEBS 3-6=-284/157

JOINT STRESS INDEX
2 = 0.45, 3 = 0.79, 4 = 0.45 and 6 = 0.11

NOTES (6)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 257 lb uplift at joint 2 and 257 lb uplift at joint 4.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T24G	GABLE	1	1	L250193033

Job Reference (optional)

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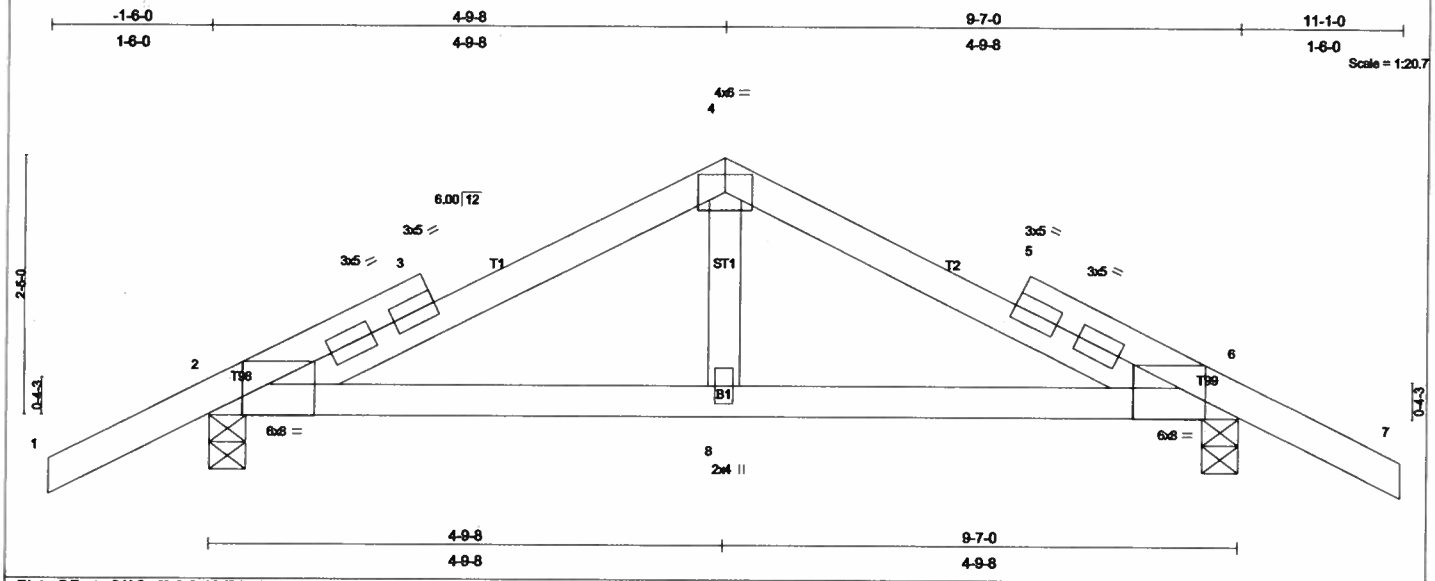

Plate Offsets (X,Y): [2:0-2-13,Edge], [6:0-2-13,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.05	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.03	2-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.05	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-10-14 oc bracing.

REACTIONS (lb/size) 2=594/0-4-0, 6=594/0-4-0
Max Horz 2=-57(load case 7)
Max Uplift2=-447(load case 6), 6=-447(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-10/57, 2-3=-654/807, 3-4=-593/798, 4-5=-593/798, 5-6=-654/807, 6-7=-10/57
BOT CHORD 2-8=-615/530, 6-8=-615/530
WEBS 4-8=-286/156

JOINT STRESS INDEX
2 = 0.55, 3 = 0.00, 3 = 0.43, 3 = 0.43, 4 = 0.83, 5 = 0.00, 5 = 0.43, 5 = 0.43, 6 = 0.55 and 8 = 0.11

NOTES (9)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 447 lb uplift at joint 2 and 447 lb uplift at joint 6.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-87(F=-33), 4-7=-87(F=-33), 2-6=-10

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T25	COMMON	1	1	L250193034
					Job Reference (optional)

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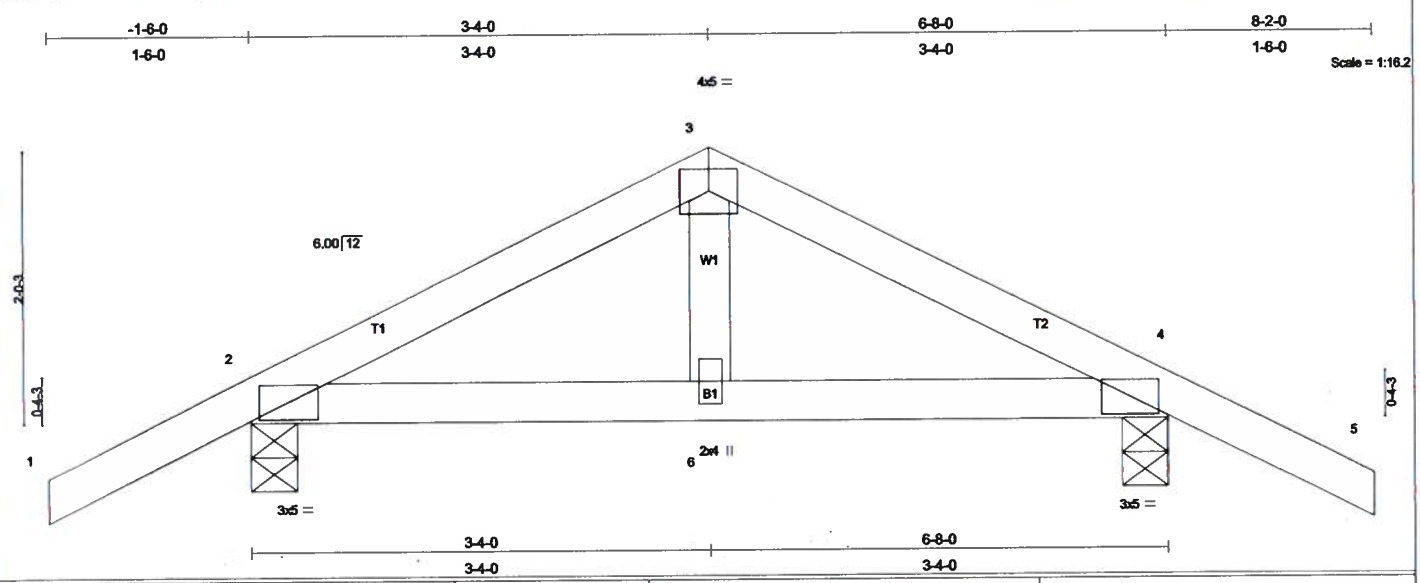

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.00	4-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	4-6	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 28 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=293/0-4-0, 4=293/0-4-0
Max Horz 2=46(load case 6)
Max Uplift2=-124(load case 6), 4=-124(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-221/110, 3-4=-221/110, 4-5=0/35
BOT CHORD 2-6=0/149, 4-6=0/149
WEBS 3-6=0/116

JOINT STRESS INDEX
2 = 0.18, 3 = 0.06, 4 = 0.18 and 6 = 0.08

NOTES (6)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 2 and 124 lb uplift at joint 4.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T25A	COMMON	1	1	L250193035

Job Reference (optional)

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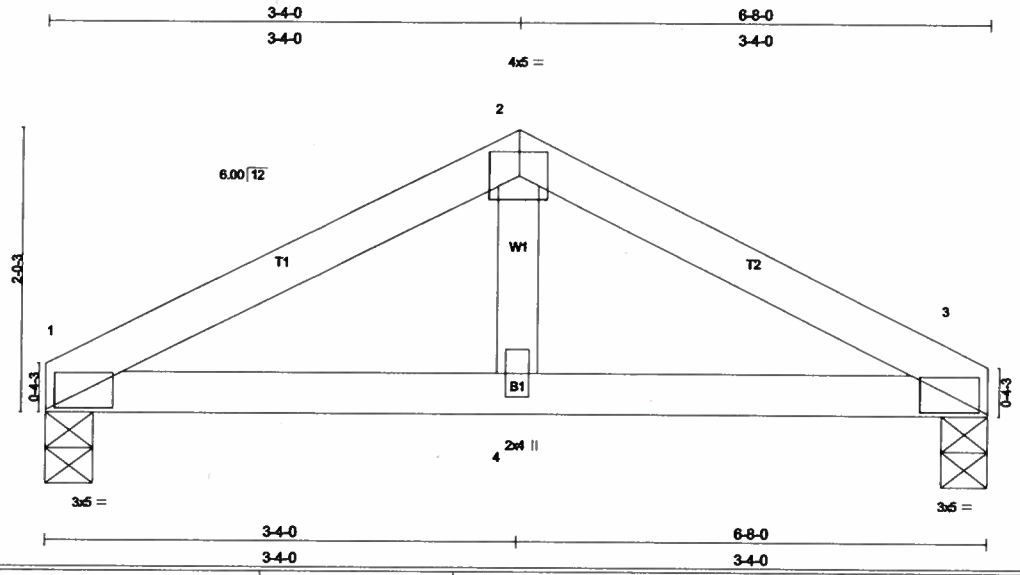


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.01	3-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.02	3-4	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.10	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 24 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=439/0-4-0, 3=439/0-4-0
Max Horz 1=-22(load case 3)
Max Uplift1=-109(load case 5), 3=-109(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-556/129, 2-3=-556/129
BOT CHORD 1-4=-92/461, 3-4=-92/461
WEBS 2-4=-57/326

JOINT STRESS INDEX
1 = 0.29, 2 = 0.16, 3 = 0.29 and 4 = 0.24

NOTES (8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 109 lb uplift at joint 1 and 109 lb uplift at joint 3.
6) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 6-8-0
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 1-3=-85(B=-75)

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE
L250193	T25G	GABLE	1	1	L250193036
					Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 04 16:59:01 2008 Page 1

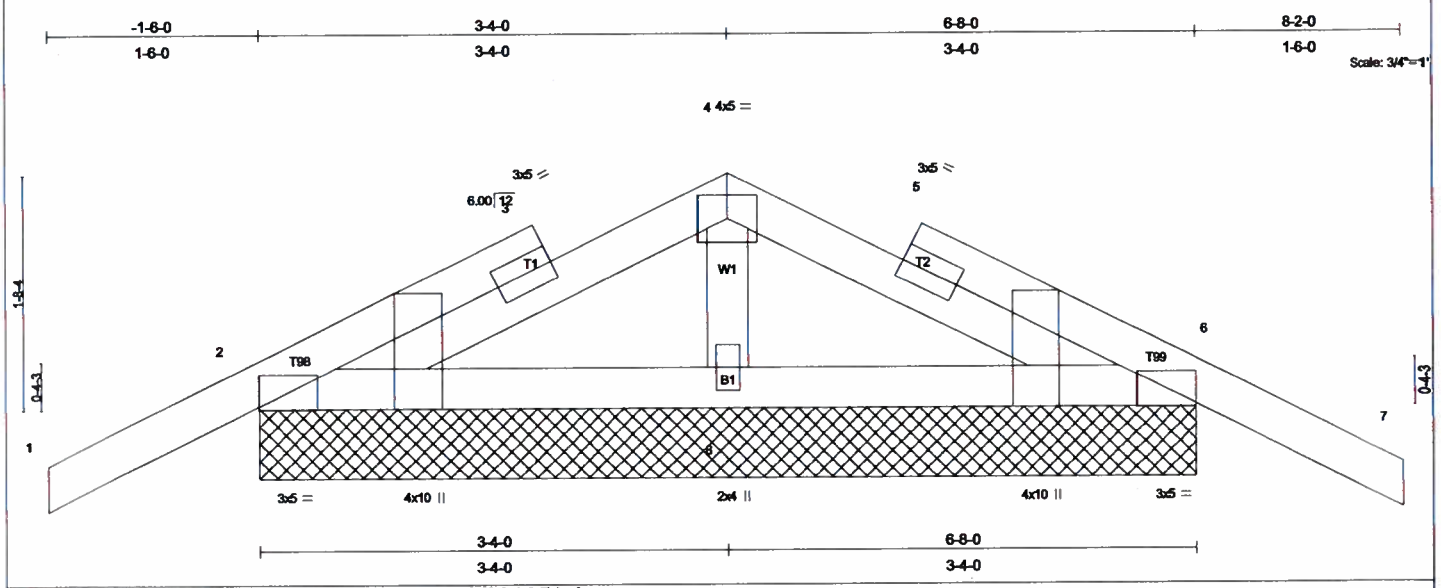


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-1-8,Edge], [6:0-3-8,Edge], [6:0-1-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.01	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	7	n/r	90		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-8-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=284/6-8-0, 6=285/6-8-0, 8=339/6-8-0
Max Horz 2=47(load case 6)
Max Uplift2=-183(load case 6), 6=-191(load case 7), 8=-92(load case 6)
Max Grav 2=292(load case 10), 6=292(load case 11), 8=339(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-12/56, 2-3=-35/85, 3-4=-42/125, 4-5=-43/125, 5-6=-35/85, 6-7=-12/56
BOT CHORD 2-8=-79/141, 6-8=-79/141
WEBS 4-8=-297/233

JOINT STRESS INDEX
2 = 0.46, 2 = 0.00, 3 = 0.00, 3 = 0.00, 3 = 0.37, 4 = 0.09, 5 = 0.00, 5 = 0.37, 5 = 0.00, 6 = 0.46, 6 = 0.00 and 8 = 0.13

NOTES (11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 183 lb uplift at joint 2, 191 lb uplift at joint 6 and 92 lb uplift at joint 8.
9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 6.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-87(F=-33), 4-7=-87(F=-33), 2-6=-10

STEPDOWN CORNER SET

TOP CHORD 2X4 SO. PINE #2 or Better
BOT CHORD 2X4 SO. PINE #2 or Better
WEBS 2X4 SO. PINE #3 or Better

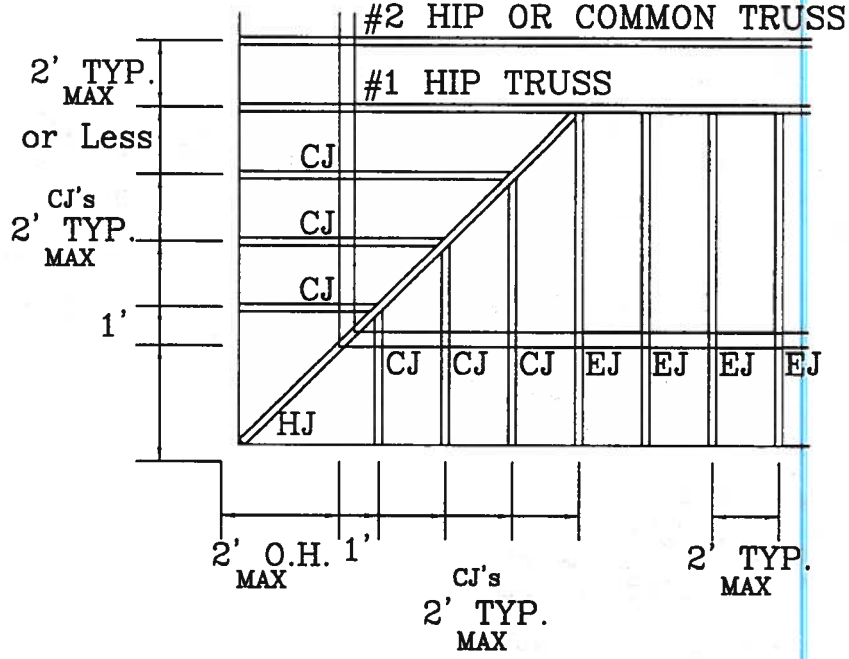
120 MPH MAX

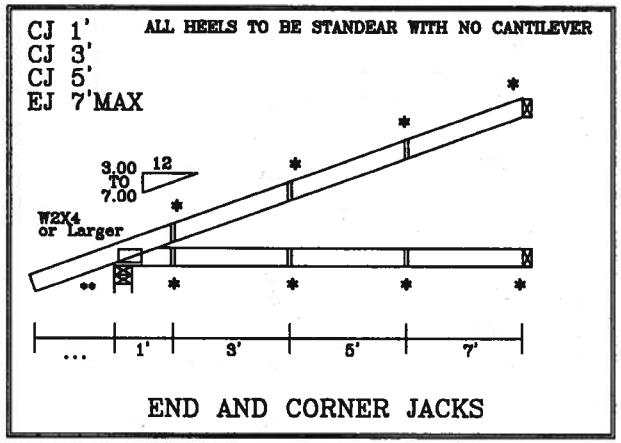
Setback 7' or Less

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT=28 FT. ENCLOSED. (ASCE 7-02)

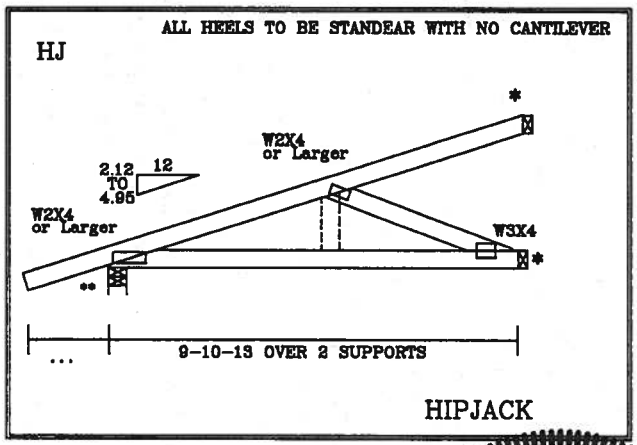
PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED. TILE
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)





END AND CORNER JACKS



HIPJACK

* (3) 16d TOENAILS

** SEE EOR FOR TIE DOWN

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
BC LIVE LOAD IS NON CONCURRENT 10*

SHINGLE	TC LL	20	MAX PSF
	TC DL	7	MAX PSF
	BC LL	10*	MAX PSF
	BC DL	5	MAX PSF
TILE	TC LL	20	MAX PSF
	TC DL	20	MAX PSF
	BC LL	10*	MAX PSF
	BC DL	5	MAX PSF
DUR. FAC.		1.25	
SPACING		2'	MAX

REF	7'MAX STBK CS
DATE	Jun./27/2008
DRWG	
-ENG	

REVIEWED
By julius lee at 10:52 am, Jun 27, 2008

CORNER SET SETBACK
7'0" MAX

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (V,H/S/O ASTM A653 GRADE 40/60 (V,K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

JULIUS LEE CONS. ENGINEERS, P.A.
7455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2181
NO. 34859
STATE OF FLORIDA
PROFESSIONAL ENGINEER

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 5"	11' 5"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 8"	12' 6"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	8' 8"	7' 2"	7' 11"	8' 5"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 3"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 5"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 6"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 8"	10' 6"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 6"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	6' 11"	6' 11"	7' 10"	7' 10"	10' 5"	10' 6"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	8' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 6"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 6"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 6"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

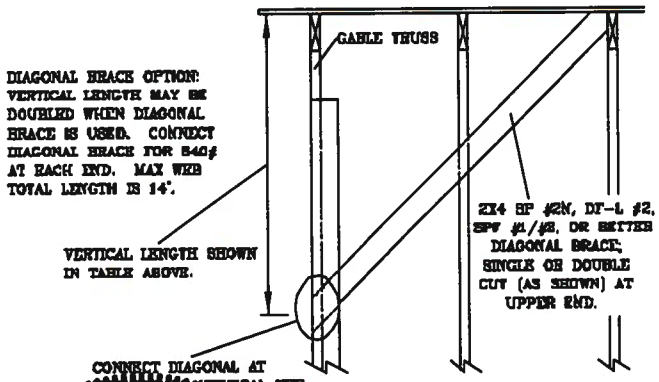
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

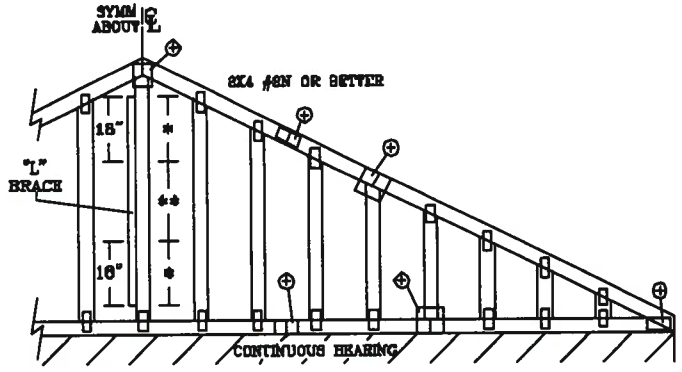
DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#2	#1
#3	#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 8' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 8" O.C. IN 16" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 8" O.C. IN 16" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-3161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

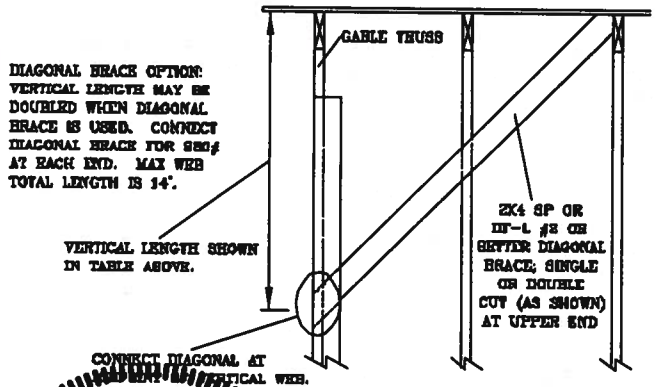
MAX. SPACING 24.0"

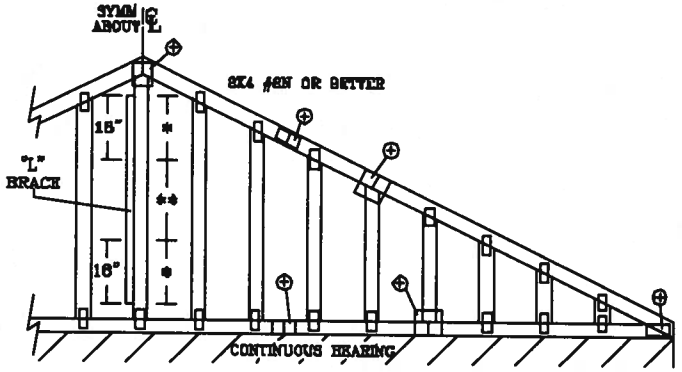
REF ASCE7-02-GAB13015
DATE 11/26/03
DRWG MITEK STD GABLE 16 E HT
-ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	6' 6"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	6' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 6"	4' 5"	5' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 6"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 6"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	13' 5"
		STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 5"
		STANDARD	3' 0"	3' 10"	3' 10"	6' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 6"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 6"	6' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	6' 2"	8' 3"	6' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	8' 7"	11' 9"	12' 6"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 6"	14' 0"	14' 0"
		STUD	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 8"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 8"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 6"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	MID SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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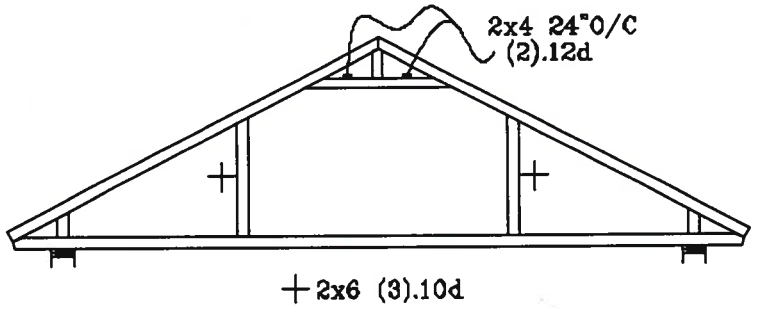
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

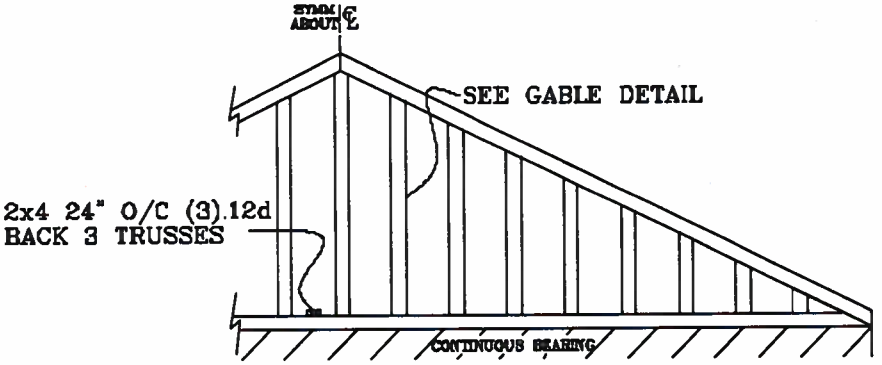
MAX. SPACING 24.0"

REF ASCE7-02-GAB13090
DATE 11/26/03
DWG MWFK STD GABLE 90' E W7
-ENG

TYPICAL ATTIC TRUSS BRACING

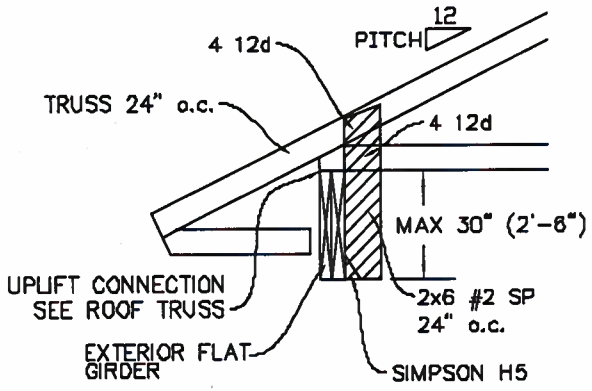


GABLE END TRUSS DETAIL



MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

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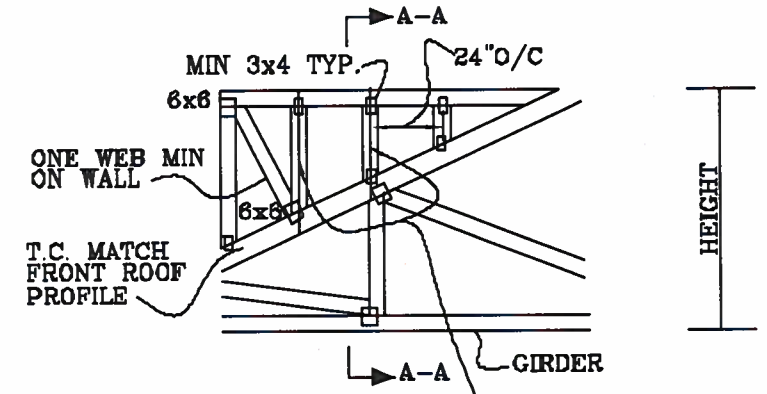
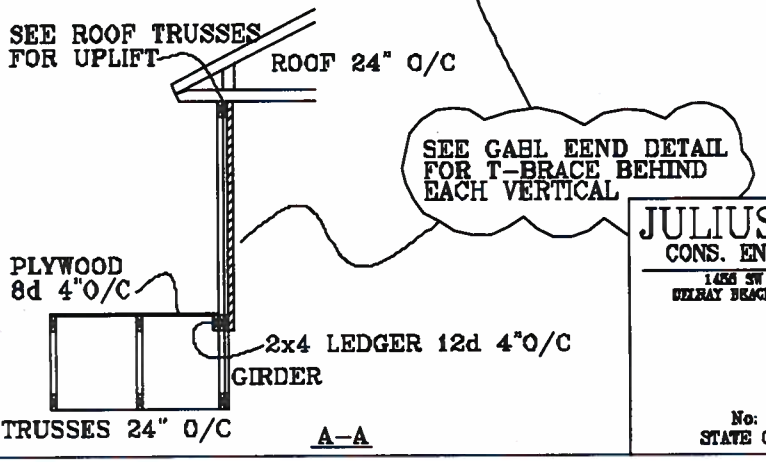
NO. 34869
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL





JULIUS LEE'S
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1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

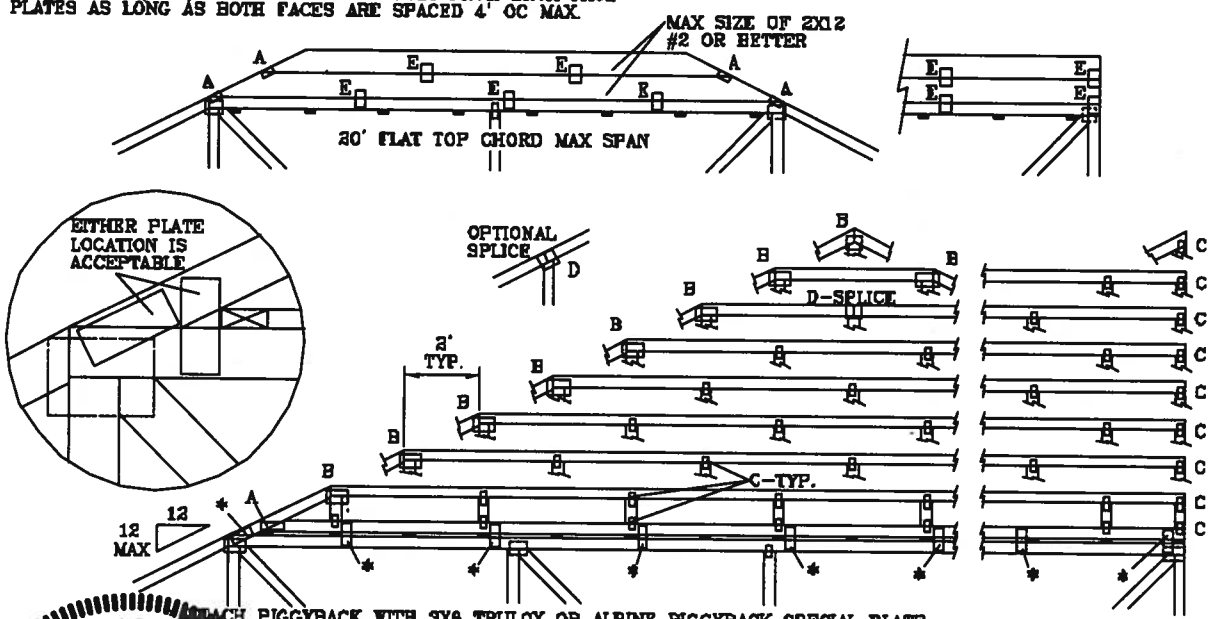
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

- 110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF
- 110 MPH WIND, 30' MEAN HGT, ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF
- 130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.6X4	2.6X4	3X6
B	4X6	6X6	6X8	5X6
C	1.5X3	1.6X4	1.6X4	1.5X4
D	5X4	6X6	6X8	5X6
E	4X8 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

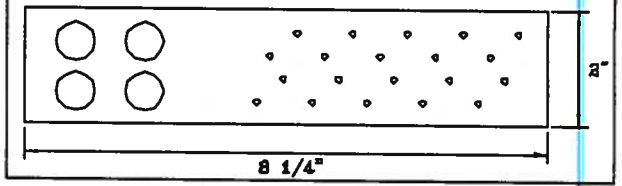


ATTACH PIGGYBACK WITH 3X6 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 6d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

*** PIGGYBACK SPECIAL PLATE**

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 647,045

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S CONS. ENGINEERS P.A.
1466 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

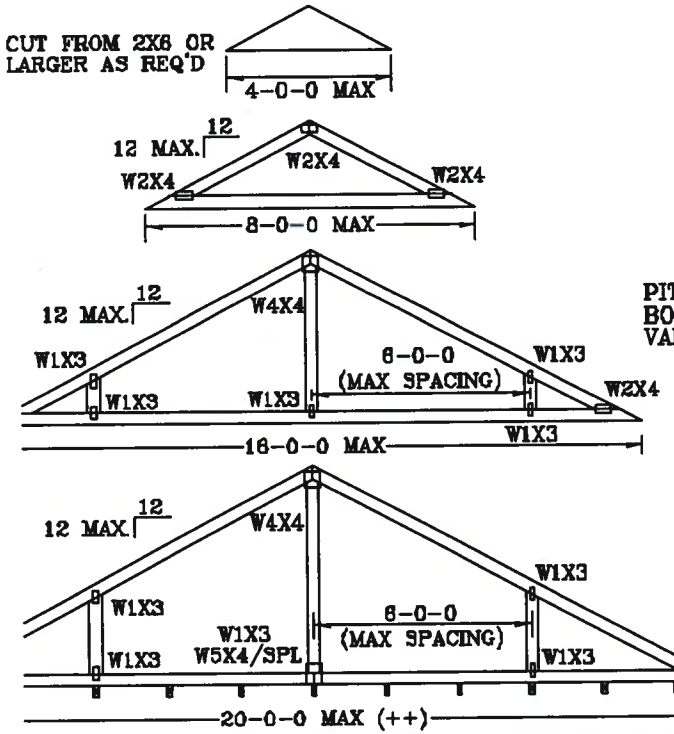
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

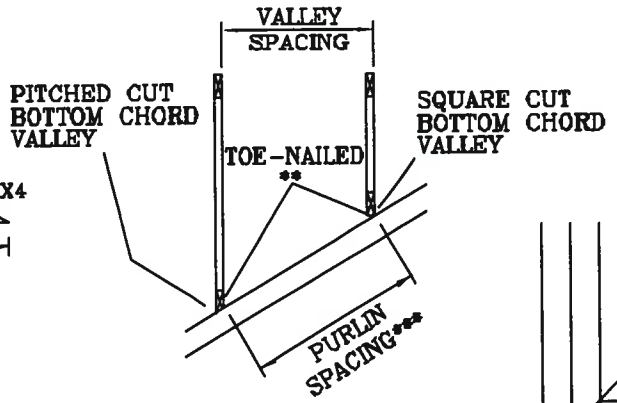
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

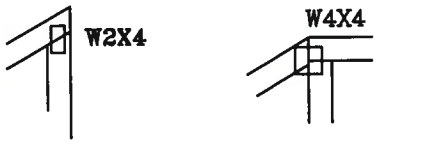
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

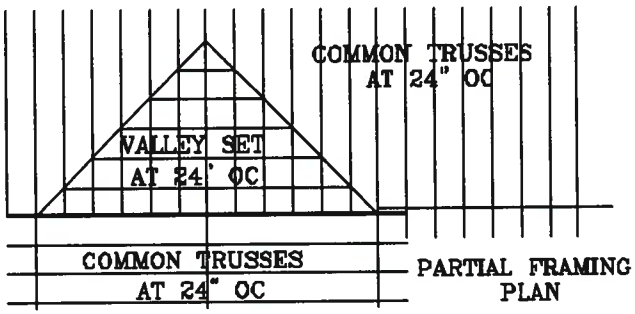
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.









SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.

THIS DRAWING REPLACES DRAWING A105

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

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REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC LL	0	0	PSF	-ENG	JL
TOT. LD.	32	40	PSF		
DURFAC. 1.25		1.25			
SPACING		24"			

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

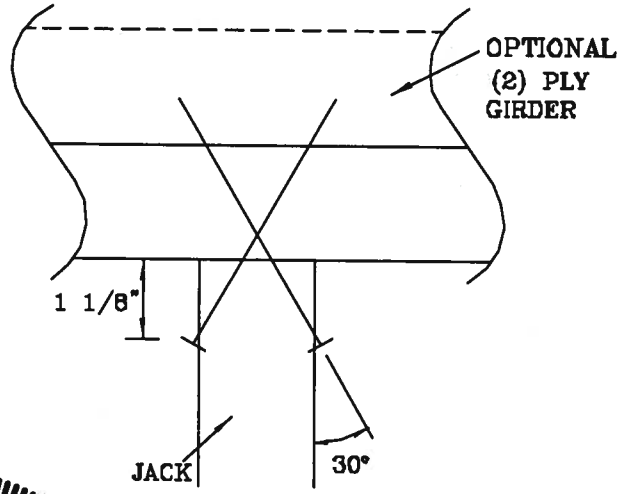
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

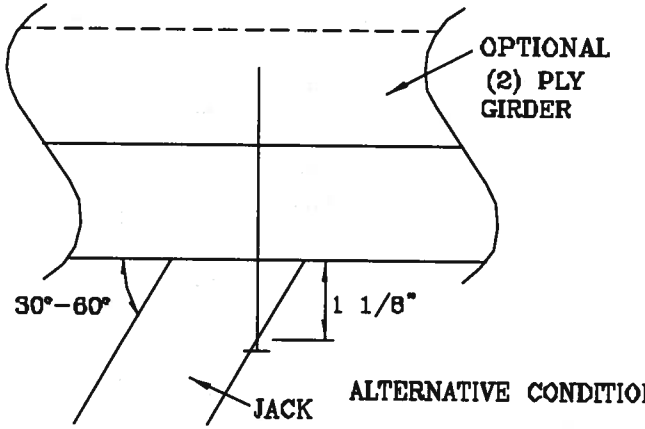
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

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By julius lee at 11:59 am, Jun 11, 2008

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CONS. ENGINEERS P.A.
1450 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

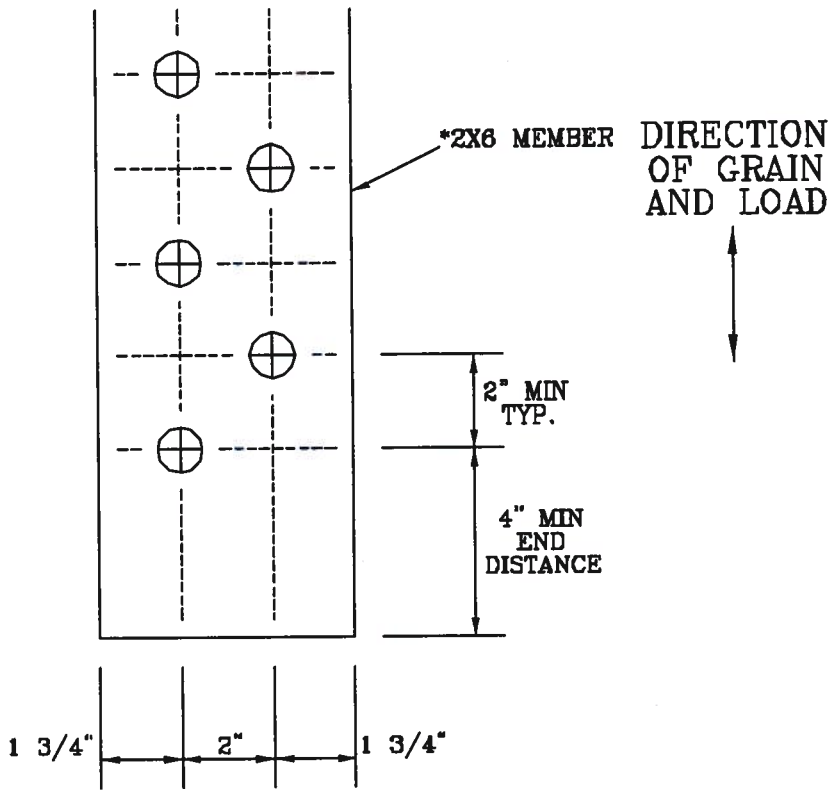
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

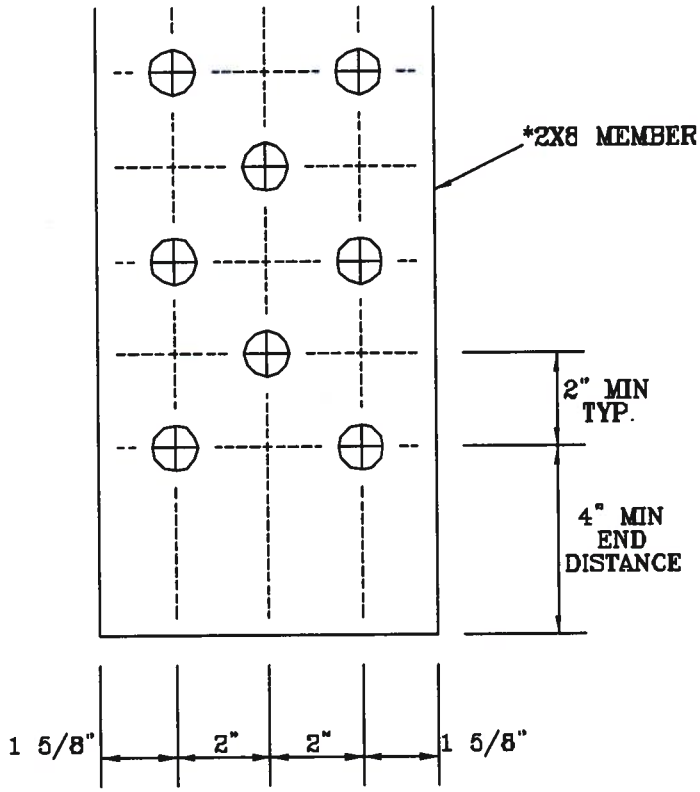
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A626.016

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

=WARNING= TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S CONS. ENGINEERS P.A.
1480 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSP1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

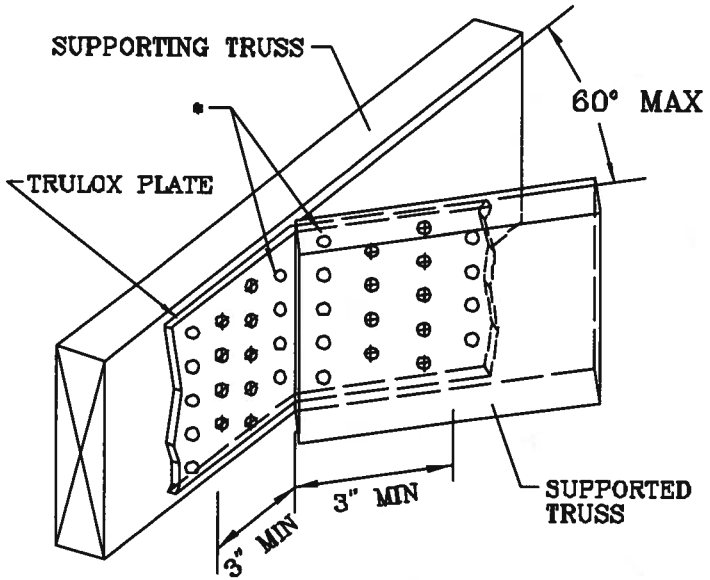
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

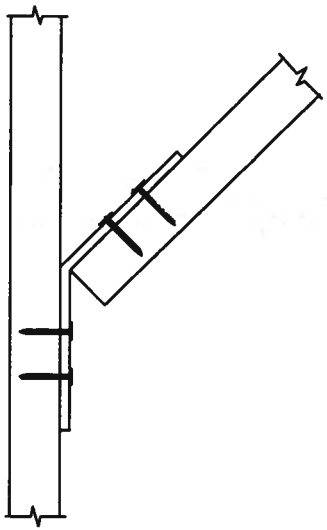
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

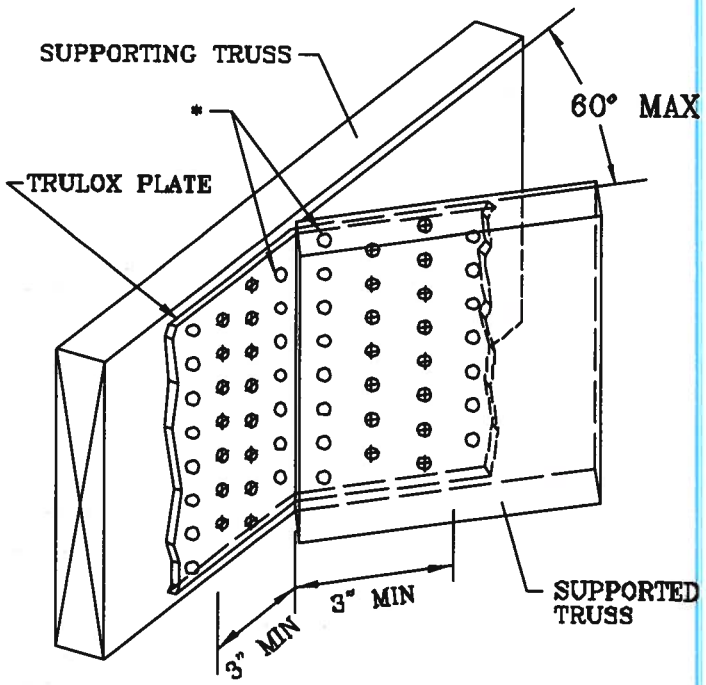
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

REVIEWED
By julius lee at 11:58 am, Jun 11, 2008

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

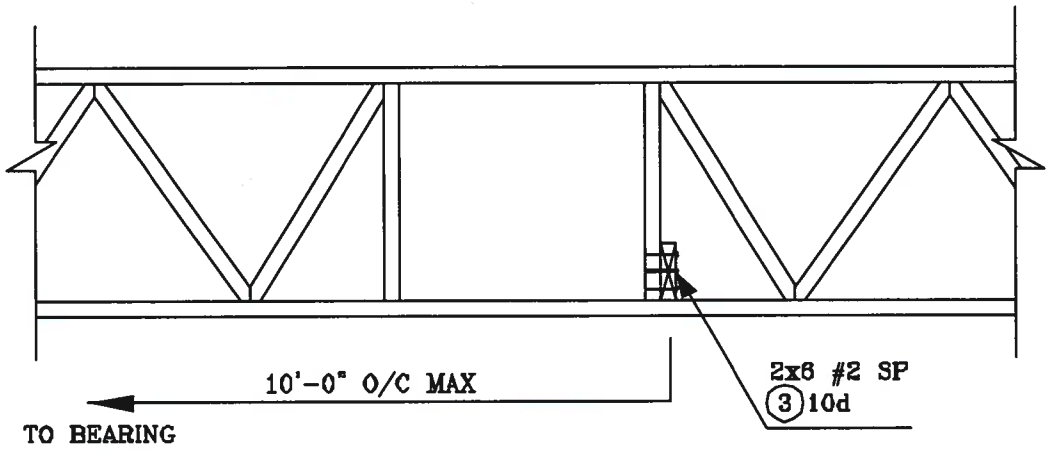
THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

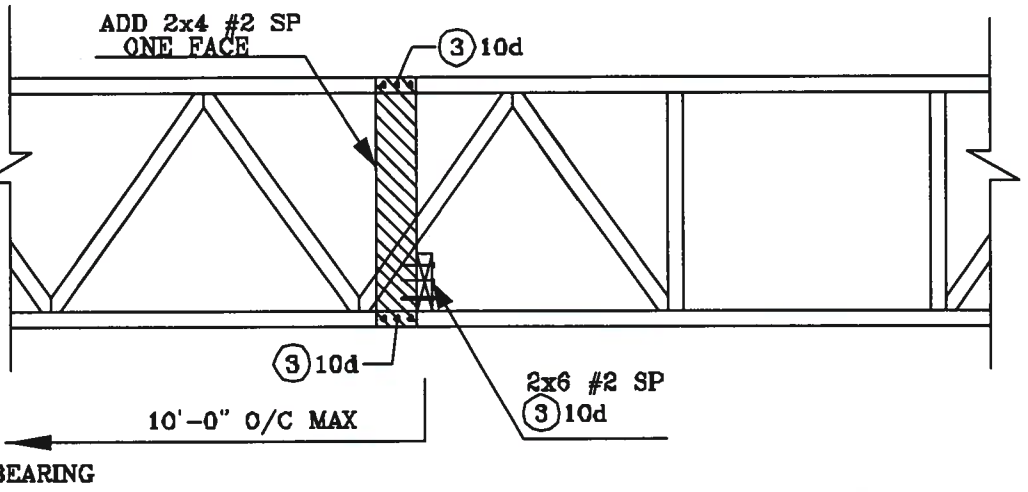
JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-6161
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM–42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



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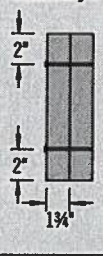
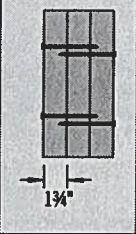
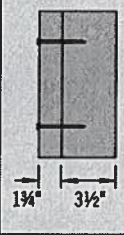
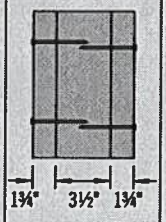
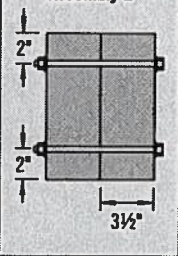
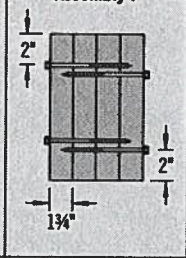
DELRAY BEACH, FL 33444-3161

No: 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A 	Assembly B 	Assembly C 	Assembly D 	Assembly E 	Assembly F 
			3½" 2-ply	5¼" 3-ply	5¼" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail[1]	2	12"	370	***280***	280	***245***		
	3	12"	555	***415***	415	***370***		
½" A307 Through Bolts[2][4]	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS ¼" x 3½"[4]	2	24"	680	***510***	510	***455***		
		19.2"	850	***640***	640	***565***		
		16"	1,020	***765***	765	***680***		
SDS ¼" x 6"[3][4]	2	24"				***455***	***465***	***455***
		19.2"				***565***	***580***	***565***
		16"				***680***	***695***	***680***
USP WS35 [4]	2	24"	480	***360***	360	***320***		
		19.2"	600	***450***	450	***400***		
		16"	715	***540***	540	***480***		
USP WS6 [3][4]	2	24"				***350***	***525***	***350***
		19.2"				***440***	***660***	***440***
		16"				***525***	***790***	***525***
3⅛" TrussLok[4]	2	24"	635	***475***	475	***425***		
		19.2"	795	***595***	595	***530***		
		16"	955	***715***	715	***635***		
5" TrussLok[4]	2	24"		***500***	500	***445***	***480***	***445***
		19.2"		***625***	625	***555***	***600***	***555***
		16"		***750***	750	***665***	***725***	***665***
6¼" TrussLok[4]	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 9/16" maximum.

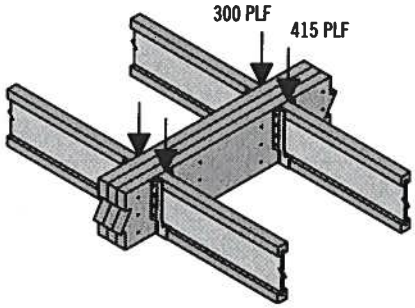
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- ***Bold Italic*** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by ½ the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1¾" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:
Two rows of ½" bolts or SDS ¼" x 3½" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern: Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
		3½" 2-ply	5¼" 3-ply	5¼" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	*835*	835	*740*		
	12	2,225	*1,670*	1,670	*1,485*		
	18	3,335	*2,505*	2,505	*2,225*		
	24	4,450	*3,335*	3,335	*2,965*		
SDS Screws ¼" x 3½" or WS35 ¼" x 6" or WS6(1)	4	1,915	1,435(4)	1,435	*1,275*	*1,860(2)*	*1,405(2)*
	6	2,870	2,150 (4)	2,150	*1,915*	*2,785(2)*	*2,110(2)*
	8	3,825	2,870 (4)	2,870	*2,550*	*3,715(2)*	*2,810(2)*
3⅜" or 5" TrussLok™	4	2,545	1,910 (4)	1,910	*1,695*	*1,925(3)*	*1,775(3)*
	6	3,815	2,860 (4)	2,860	*2,545*	*2,890(3)*	*2,665(3)*
	8	5,090	3,815 (4)	3,815	*3,390*	*3,855(3)*	*3,550(3)*

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

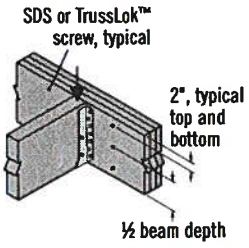
(2) 6" long screws required.

(3) 5" long screws required.

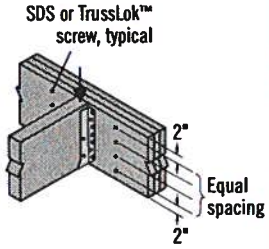
(4) 3½" and 3⅜" long screws must be installed on both sides.

Connections

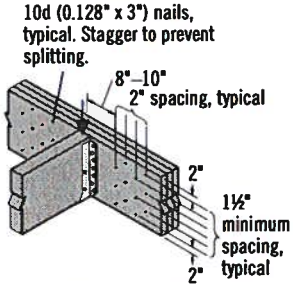
4 or 6 or Screw Connection



8 Screw Connection

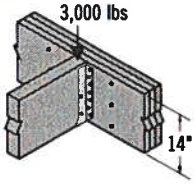


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1¾" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1¾" assembly, eight 3⅜" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

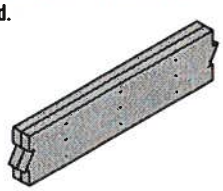
MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1¾" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3⅜" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3½" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of ½" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

L6