Columbia County New Building Permit Application

Clerk # 44776

For Office Use Only Application # 94675 Date Received 3/5 By JW Permit # 39492

Zoning Official LW/GH Date 3-12-20 Flood Zone X Land Use Ag Zoning PRD

FEMA Map # _____ Elevation _____ MFE _____ River _____ Plans Examiner T.C. Date 3-17-20

Comments _____

☐ NOC ☑ EH ☑ Deed or PA ☑ Site Plan ☐ State Road Info ☑ Well letter ☑ 911 Sheet ☐ Parent Parcel # _____

☐ Dev Permit # _____ ☐ In Floodway ☑ Letter of Auth. from Contractor ☐ F W Comp. letter

☐ Owner Builder Disclosure Statement ☐ Land Owner Affidavit ☐ Ellisville Water ☑ App Fee Paid ☑ Sub VF Form

Septic Permit No. 20-0178 OR City Water ☐ Fax _____

Applicant (Who will sign/pickup the permit) LORA DAVID Phone 365-5671

Address 601 SW ROSEMARY DR, LAKE CITY, FL 32024

Owners Name MATTHEW KARLTON Phone 386-623-0267

911 Address 171 NW Ansley Place, Lake City, FL 32055

Contractors Name AARON SIMQUE HOMES, INC. Phone 867-5395

Address 601 SW ROSEMARY DR, LAKE CITY, FL 32024

Contractor Email AARON@AARONSIMQUE.COM, Lora david@yahoo.com ***Include to get updates on this job.

~~Fee Simple Owner~~ Name & Address _____

~~Bonding Co.~~ Name & Address MARK DISOSWAY PE, 163 SW Midtown Pl #103 LCFL 32024

Architect/Engineer Name & Address _____

Mortgage Lenders Name & Address Campus 1658 W. US Hwy 90 Lake City, FL 32055

Circle the correct power company ☒ FL Power & Light ☐ Clay Elec. ☐ Suwannee Valley Elec. ☐ Duke Energy

Property ID Number 20-3S-16-02202-128 Estimated Construction Cost 250K

Subdivision Name HIGH POINTE Lot 28 Block ____ Unit ____ Phase ____

Driving Directions from a Major Road HWY 90 W, THEN LEFT ON BROWN RD, THEN RIGHT ONTO HIGH POINT DR, THEN RIGHT ONTO ANSLEY PL, PROPERTY 2ND ON RIGHT

Construction of SFR _____ Commercial OR X Residential

Proposed Use/Occupancy SFR Number of Existing Dwellings on Property ____

Is the Building Fire Sprinkled? NO If Yes, blueprints included ____ Or Explain _____

Circle Proposed ☒ Culvert Permit or ☐ Culvert Waiver or ☐ D.O.T. Permit or ☐ Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 50 Side 72.2 Side 70.5 Rear 142

Number of Stories 1 Heated Floor Area 2286 Total Floor Area 3983 Acreage 1.59

Zoning Applications applied for (Site & Development Plan, Special Exception, etc.) _____

94675 44776

**Columbia County Building Permit Application**

**CODE: Florida Building Code 2014 and the 2011 National Electrical Code.**

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

**TIME LIMITATIONS OF APPLICATION :** An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless pursued in good faith or a permit has been issued.

**TIME LIMITATIONS OF PERMITS:** Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

**FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment:** According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

**NOTICE OF RESPONSIBILITY TO CONTRACTOR AND AGENT:** ***YOU ARE HEREBY NOTIFIED*** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

**WARNING TO OWNER:** YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

**OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.**

**NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them.** These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and or fines.

M-Kent — **Print Owners Name**

MATT KARLTON M-Kent — **Owners Signature**

****Property owners must sign here before any permit will be issued.**

****If this is an Owner Builder Permit Application then, ONLY the owner can sign the building permit when it is issued.**

**CONTRACTORS AFFIDAVIT:** By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

**Contractor's Signature** AARon Simque

**Contractor's License Number** RR2828811879
**Columbia County Competency Card Number** 000713

**Affirmed under penalty of perjury to by the Contractor and subscribed before me this 21 day of** Feb **2020.**

**Personally known**____ **or Produced Identification** FL DL

Linda R. Craft

**State of Florida Notary Signature (For the Contractor)**

LINDA RUTH CRAFT
MY COMMISSION # GG017780
EXPIRES August 28, 2020

## SUBCONTRACTOR VERIFICATION

APPLICATION/PERMIT # 44675 JOB NAME ICARLTON

**THIS FORM MUST BE SUBMITTED BEFORE A PERMIT WILL BE ISSUED**

Columbia County issues combination permits. One permit will cover all trades doing work at the permitted site. It is REQUIRED that we have records of the subcontractors who actually did the trade specific work under the general contractors permit.

***NOTE:** It shall be the responsibility of the general contractor to make sure that all of the subcontractors are licensed with the Columbia County Building Department.*
**Use website to confirm licenses:** http://www.columbiacountyfla.com/PermitSearch/ContractorSearch.aspx

***NOTE:** If this should change prior to completion of the project, it is your responsibility to have a corrected form submitted to our office, before that work has begun.*

Violations will result in stop work orders and/or fines.

| | | | Need |
|---|---|---|---|
| **ELECTRICAL** ☑ CC# 811 | Print Name RYAN BEVILLE | Signature | Lic / Liab / W/C / EX / DE — Rec'd |
| | Company Name: RBI ELECTRICAL Contracting | | |
| | License #: EC13004236 | Phone #: 386 339 0360 | |
| **MECHANICAL/ A/C** ☐ CC# 1517 | Print Name [redacted] | Signature [redacted] | Lic / Liab / W/C / EX / DE |
| | Company Name: [redacted] | | |
| | License #: [redacted] | Phone #: [redacted] | |
| **PLUMBING/ GAS** ☑ CC# 1023 | Print Name MARK GANSKOP | Signature | Lic / Liab / W/C / EX / DE |
| | Company Name: Express Plumbing | | |
| | License #: CFC1428040 | Phone #: 386-867-0269 | |
| **ROOFING** ☑ CC# 1129 | Print Name DANA Johnson | Signature | Lic / Liab / W/C / EX / DE |
| | Company Name: MAC Johnson Roofing | | |
| | License #: CCC1325497 | Phone #: | |
| **SHEET METAL** ☐ CC# | Print Name | Signature | Lic / Liab / W/C / EX / DE |
| | Company Name: | | |
| | License #: | Phone #: | |
| **FIRE SYSTEM/ SPRINKLER** ☐ CC# | Print Name | Signature | Lic / Liab / W/C / EX / DE |
| | Company Name: | | |
| | License#: | Phone #: | |
| **SOLAR** ☐ CC# | Print Name | Signature | Lic / Liab / W/C / EX / DE |
| | Company Name: | | |
| | License #: | Phone #: | |
| **STATE SPECIALTY** ☐ CC# | Print Name | Signature | Lic / Liab / W/C / EX / DE |
| | Company Name: | | |
| | License #: | Phone #: | |

Ref: F.S. 440.103; ORD. 2016-30

## SUBCONTRACTOR VERIFICATION

APPLICATION/PERMIT # 24675 JOB NAME KARLTON

**THIS FORM MUST BE SUBMITTED BEFORE A PERMIT WILL BE ISSUED**

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**Use website to confirm licenses:** http://www.columbiacountyfla.com/PermitSearch/ContractorSearch.aspx

***NOTE:** If this should change prior to completion of the project, it is your responsibility to have a corrected form submitted to our office, before that work has begun.*

Violations will result in stop work orders and/or fines.

| | | | Need |
|---|---|---|---|
| **ELECTRICAL** ☐ CC#_____ | Print Name_____ Signature_____ | Company Name:_____ | Lic / Liab / W/C / EX / DE |
| **MECHANICAL/ A/C** ☑ CC# 1762 | Print Name Steve Brisbois Signature | Company Name: Arctic A/C Services Heating & Air | Lic / Liab ✓ / W/C / EX / DE — Need |
| | License #: CAC 1815182 Phone #: 386-688-4107 | | |
| **PLUMBING/ GAS** ☐ CC#_____ | Print Name_____ Signature_____ | Company Name:_____ | Lic / Liab / W/C / EX / DE |
| | License #:_____ Phone #:_____ | | |
| **ROOFING** ☐ CC#_____ | Print Name_____ Signature_____ | Company Name:_____ | Lic / Liab / W/C / EX / DE |
| | License #:_____ Phone #:_____ | | |
| **SHEET METAL** ☐ CC#_____ | Print Name_____ Signature_____ | Company Name:_____ | Lic / Liab / W/C / EX / DE |
| | License #:_____ Phone #:_____ | | |
| **FIRE SYSTEM/ SPRINKLER** ☐ CC#_____ | Print Name_____ Signature_____ | Company Name:_____ | Lic / Liab / W/C / EX / DE |
| | License#:_____ Phone #:_____ | | |
| **SOLAR** ☐ CC#_____ | Print Name_____ Signature_____ | Company Name:_____ | Lic / Liab / W/C / EX / DE |
| | License #:_____ Phone #:_____ | | |
| **STATE SPECIALTY** ☐ CC#_____ | Print Name_____ Signature_____ | Company Name:_____ | Lic / Liab / W/C / EX / DE |
| | License #:_____ Phone #:_____ | | |

Ref: F.S. 440.103; ORD. 2016-30

44675

S900650026 42

STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. 20-0178
DATE PAID: 3/5/20
FEE PAID: 725.00
RECEIPT #: 1271729

APPLICATION FOR:
[X] New System [ ] Existing System [ ] Holding Tank [ ] Innovative
[ ] Repair [ ] Abandonment [ ] Temporary [ ] _____

APPLICANT: Matthew Kidston

AGENT: Lora David TELEPHONE: 3655671

MAILING ADDRESS: PO Box 327, Lake City, FL 32056

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3)(m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: 28 BLOCK: ____ SUBDIVISION: High Pointe PLATTED: ____

PROPERTY ID #: 20-3S-16-02202-128 ZONING: ____ I/M OR EQUIVALENT: [ Y / (N) ]

PROPERTY SIZE: 1.59 (ACRES) WATER SUPPLY: [X] PRIVATE PUBLIC [ ]<=2000GPD [ ]>2000GPD

IS SEWER AVAILABLE AS PER 381.0065, FS? [ Y / (N) ] DISTANCE TO SEWER: ____ FT

PROPERTY ADDRESS: 171 NW Ansley Place

DIRECTIONS TO PROPERTY: Hwy 90 W, then left on Brown Rd then Right onto High Pointe. Then Right onto Ansley PL. Property 2nd on Right

BUILDING INFORMATION [X] RESIDENTIAL [ ] COMMERCIAL

| Unit No | Type of Establishment | No. of Bedrooms | Building Area Sqft | Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC |
|---|---|---|---|---|
| 1 | SFR | 3 | 2286 | |
| 2 | | | | |
| 3 | | | | |
| 4 | | | | |

[ ] Floor/Equipment Drains [ ] Other (Specify) _____

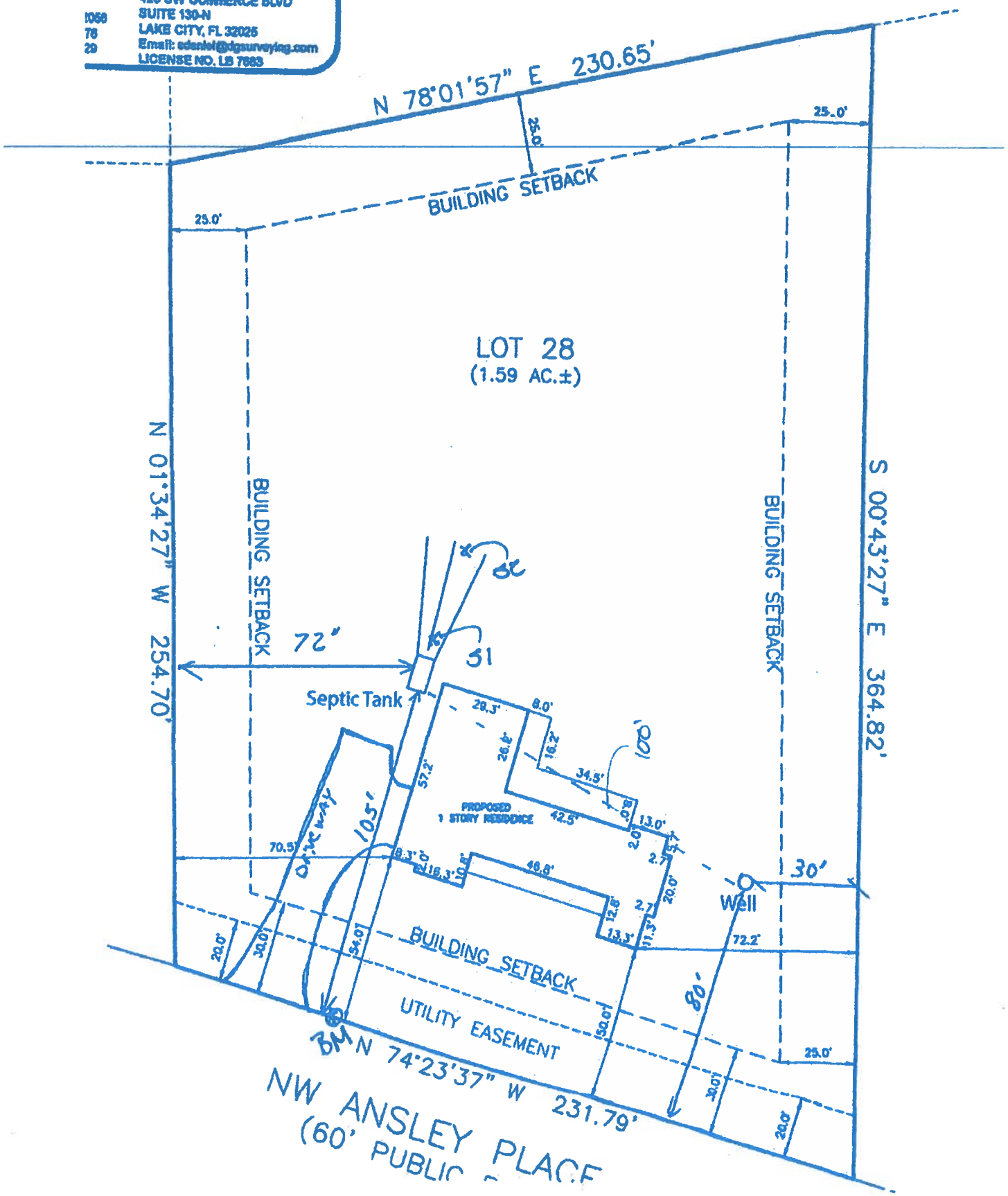
RECEIVED
MAR 05 2020
By EH

SIGNATURE: Lora David DATE: 2/17/20

DH 4015, 08/09 (Obsoletes previous editions which may not be used)
Incorporated 64E-6.001, FAC

L & GORE, LLC
Surveying and Mapping
426 SW COMMERCE BLVD
SUITE 130-N
LAKE CITY, FL 32025
Email: cdaniel@dgsurveying.com
LICENSE NO. LB 7663

20-0178



KARLTON RESIDENCE

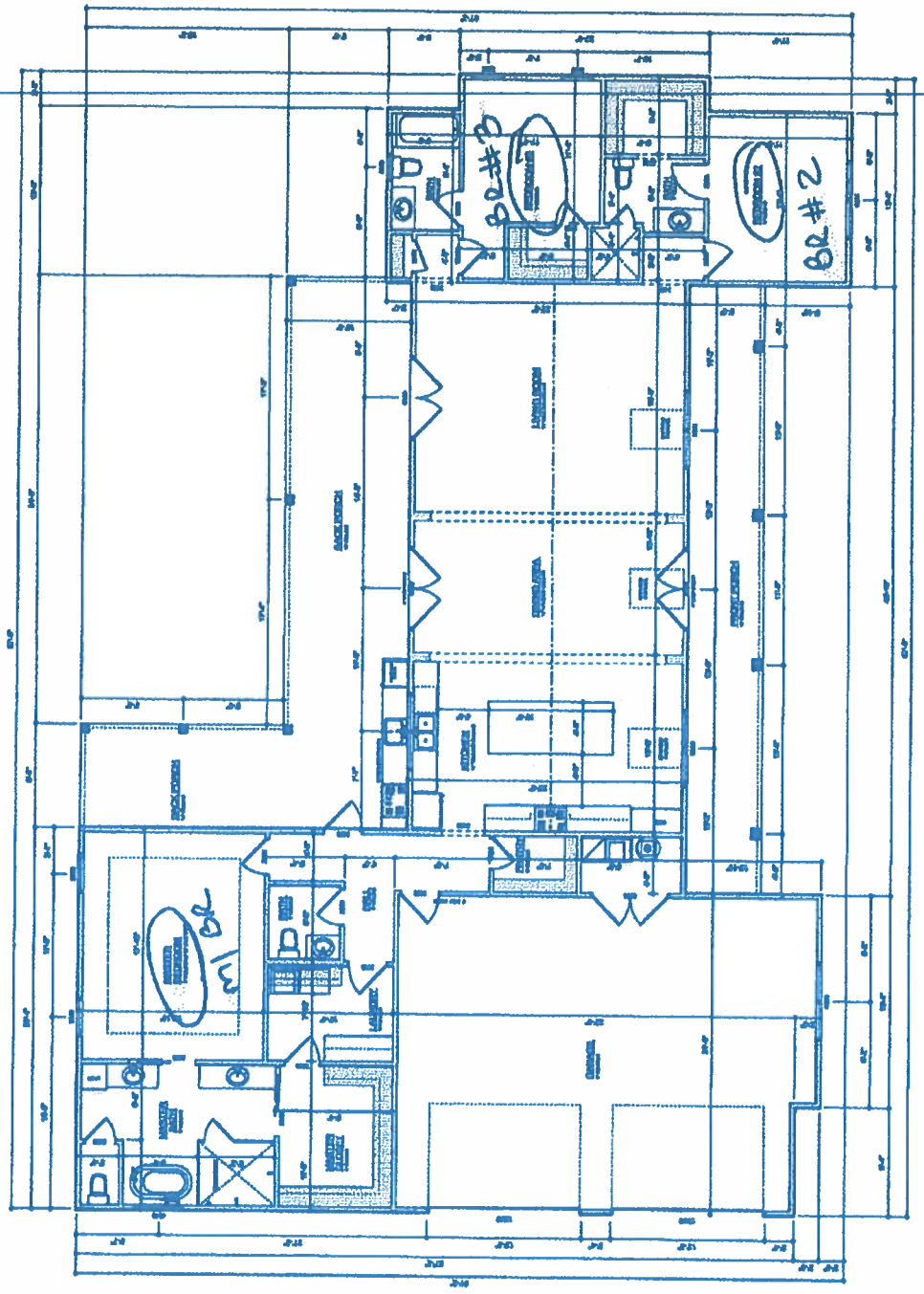
RIDGEPOINT DESIGN

A3

20-0128

AREA SUMMARY

| | |
|---|---|
| LIVING | 2,268 S.F. |
| FRONT PORCH | 351 S.F. |
| BACK PORCH | 264 S.F. |
| GARAGE | 802 S.F. |
| TOTAL AREA | 3,983 S.F. |




DIMENSIONED FLOOR PLAN

Legend

2018Aerials

SRWMD Wetlands

2018 Flood Zones

- 0.2 PCT ANNUAL CHANCE
- A
- AE
- AH

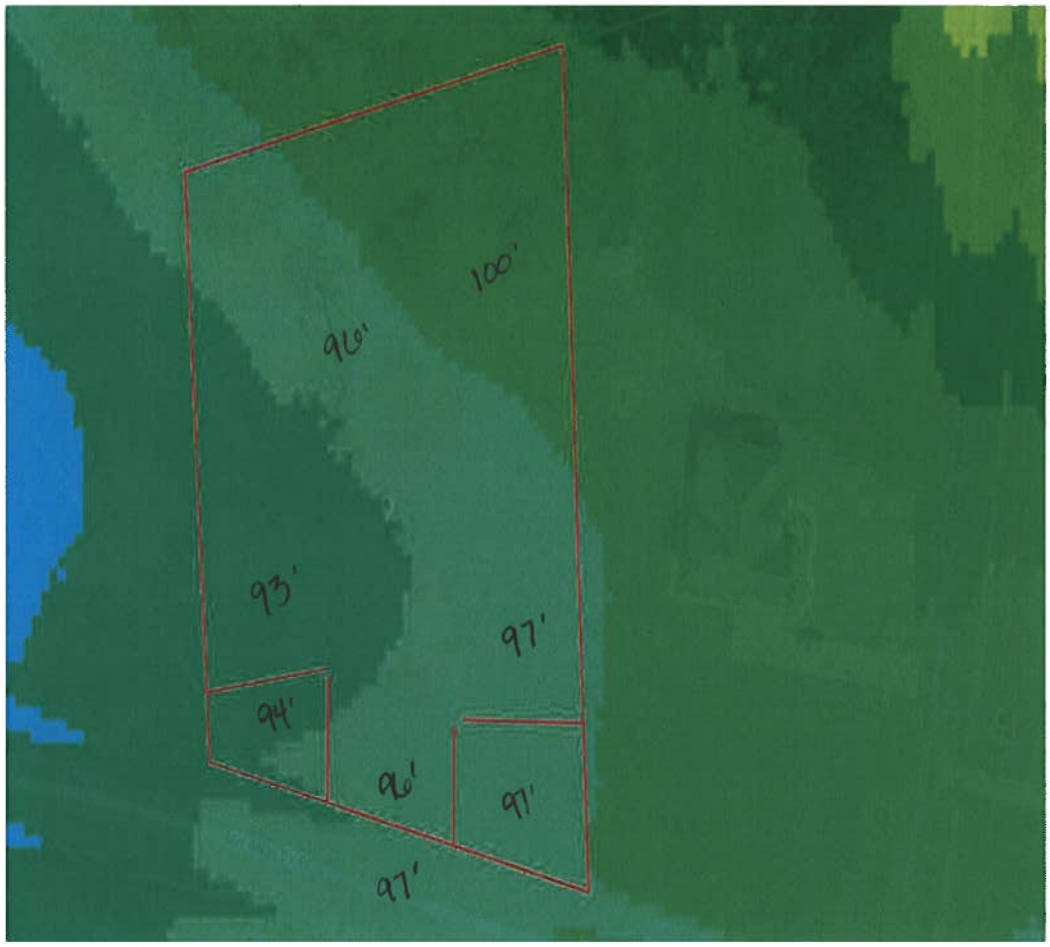
Roads

- Roads
- others
- Dirt
- Interstate
- Main
- Other
- Paved
- Private

LidarElevations

# Columbia County, FLA - Building & Zoning Property Map

Printed: Thu Mar 12 2020 16:39:15 GMT-0400 (Eastern Daylight Time)




## Parcel Information

Parcel No: 20-3S-16-02202-128

Owner: KARLTON MATTHEW B & LYNDI M

Subdivision: HIGH POINTE

Lot: 28

Acres: 1.59474277

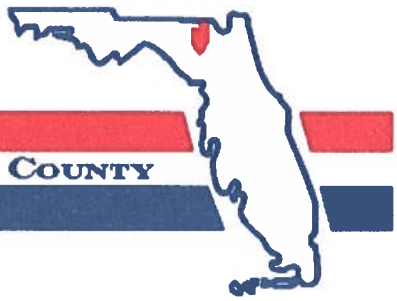
Deed Acres: 1.59 Ac

District: District 3 Bucky Nash

Future Land Uses: Agriculture - 3

Flood Zones:

Official Zoning Atlas: PRD



BOARD OF COUNTY COMMISSIONERS • COLUMBIA COUNTY

## Address Assignment and Maintenance Document

To maintain the county wide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for addressing and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Services Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County

| | |
|---|---|
| Date/Time Issued: | **2/14/2020 8:47:21 PM** |
| Address: | **171 NW ANSLEY Pl** |
| City: | **LAKE CITY** |
| State: | **FL** |
| Zip Code | **32055** |
| Parcel ID | **02202-128** |

REMARKS: Address for proposed structure on parcel.

***NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION AND ACCESS INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION AND/OR ACCESS INFORMATION BE FOUND TO BE IN ERROR OR CHANGED, THIS ADDRESS IS SUBJECT TO CHANGE.***

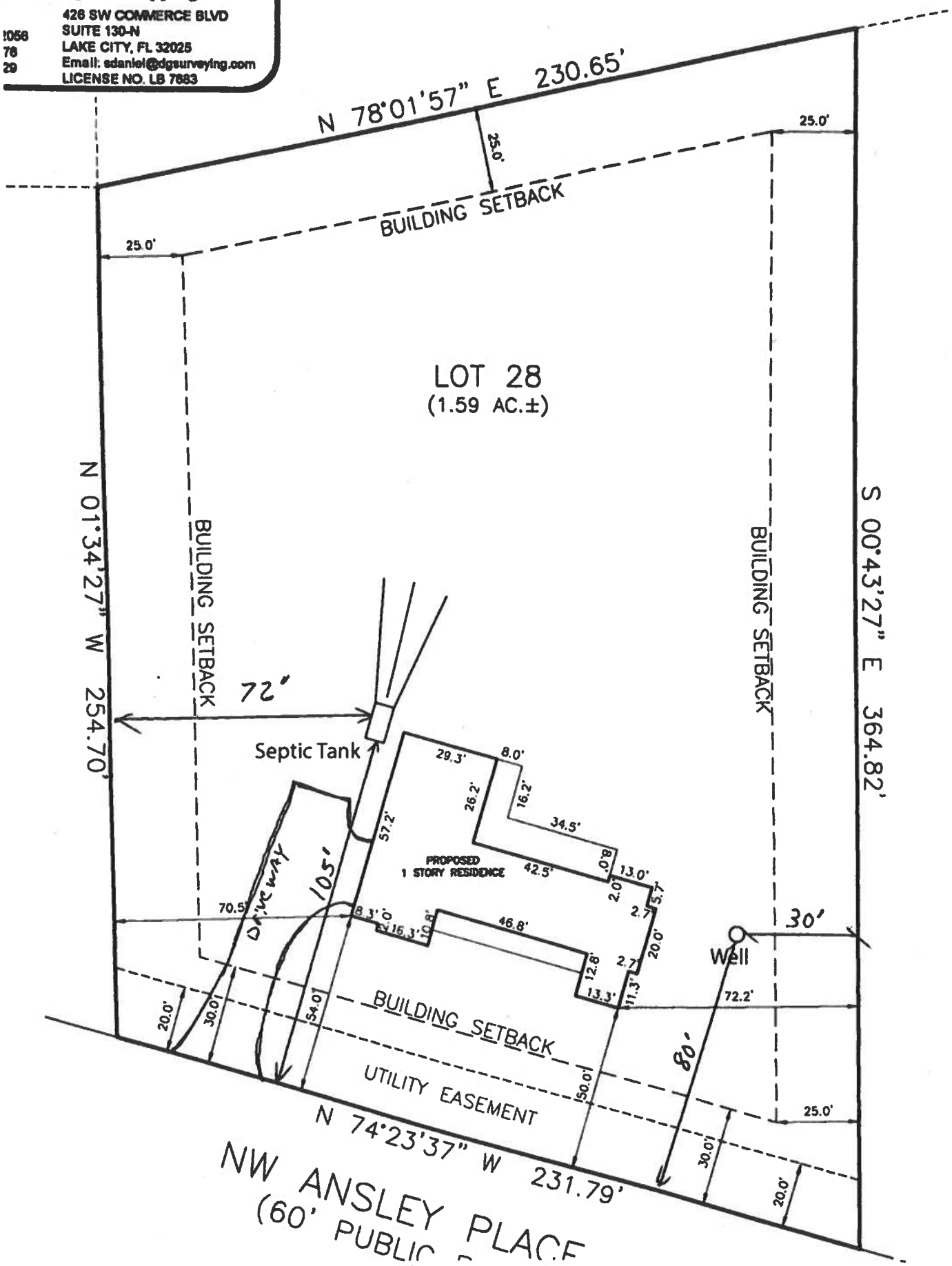
Address Issued By: **Signed:/ Matt Crews**

**Columbia County GIS/911 Addressing Coordinator**

**COLUMBIA COUNTY**
**911 ADDRESSING / GIS DEPARTMENT**

**263 NW Lake City Ave., Lake City, FL 32055 Telephone: (386) 758-1125**
**Email: gis@columbiacountyfla.com**

L & GORE, LLC
Surveying and Mapping
426 SW COMMERCE BLVD
SUITE 130-N
LAKE CITY, FL 32025
Email: sdaniel@dgsurveying.com
LICENSE NO. LB 7683



THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

File No. 09-74
Property Appraiser's
Parcel Identification No.
20-3S-16-02202-128

Recording Fee $18.50
Revenue Stamps $300.30
Consideration $42,900.00

Inst:200912005279 Date:3/31/2009 Time:2:29 PM
Doc Stamp-Deed:300.30
DC,P.DeWitt Cason,Columbia County Page 1 of 2 B:1170 P:733

## WARRANTY DEED

**This Warranty Deed**, made this 31st day of March 2009, BETWEEN HIGHPOINT FARMS, LLC, A Florida Limited Liability Company, whose post office address is 4158 US Highway 90 West, Lake City, FL 32055, of the County of Columbia, State of Florida, grantor*, and MATTHEW B. KARLTON and LYNDI M. KARLTON, Husband and Wife, whose post office address is Post Office Box 3127, Lake City, FL 32056, of the County of Columbia, State of Florida, grantee*.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

**Witnesseth:** that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

Lot 28, HIGH POINTE, a subdivision according to the plat thereof as recorded in PRRD Book 1, Pages 28-31 of the public records of Columbia County, Florida.

**Together** with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

**To Have and to Hold,** the same in fee simple forever.

**And** subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

**In Witness Whereof,** grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered
in our presence:

DeEtte F. Brown
(Signature of First Witness)
DeEtte F. Brown
(Typed Name of First Witness)

Myrtle Ann McElroy
(Signature of Second Witness)
Myrtle Ann McElroy
(Typed Name of Second Witness)

HIGHPOINT FARMS, LLC

By: _____ (SEAL)
O.P. Daughtry, III, Managing Member

STATE OF Florida
COUNTY OF Columbia

The foregoing instrument was acknowledged before me this 31st day of March, 2009, by O.P. DAUGHTRY, III, as Managing Member of HIGHPOINT FARMS, LLC, a Florida Limited Liability Company, on behalf of said company. He is personally known to me and did not take an oath.

De Ette F. Brown
Notary Public
My Commission Expires:_____

DE ETTE F. BROWN
MY COMMISSION # DD 676151
EXPIRES: October 22, 2010
Bonded Thru Notary Public Underwriters

# WaterBoy Well Repair & Drilling

19288 127th Dr.
O Brien, Fl. 32071
(386)330-6099
Waterboywellrepair@yahoo.com

January 9,2020

To Whom It May Concern,

We plan to install a 4" well located at 567 NW High Point Dr Lake City, Fl. 32055 . Well to include: 1hp submersible Pump, 1 ¼" drop pipe, 81 gallon bladder tank and backflow prevention. SRWMD permit and completion report once available.

Sincerely,

William Shuler-License #5002

RECEIVED
MAR 20 2020
By

COLUMBIA COUNTY BUILDING DEPARTMENT

# RESIDENTIAL CHECK LIST

MINIMUM PLAN REQUIREMENTS: FLORIDA BUILDING CODE RESIDENTIAL 2014 EFFECTIVE 1 JULY 2015 AND THE NATIONAL ELECTRICAL CODE 2011 EFFECTIVE 1 JULY 2015

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

**ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT 2014 FLORIDA BUILDING CODES RESIDENTIAL, EFFECTIVE 1 JULY 2015. NATIONAL ELECTRICAL CODE 2011 EFFECTIVE 1 JULY 2015. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS.**

**FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FLORIDA BUILDING CODE FIGURE 1609-A THROUGH 1609-C ULTIMATE DESIGN WIND SPEEDS FOR RISK CATEGORY AND BUILDINGS AND OTHER STRUCTURES**
**Revised 12/2016**

| | GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL | | | Items to Include- Each Box shall be Marked as Applicable |
|---|---|---|---|---|
| | | | | Select From the Dropbox |
| 1 | Two (2) complete sets of plans containing the following: | | | YES |
| 2 | All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void | | | YES |
| 3 | Condition space (Sq. Ft.) 2286 | Total (Sq. Ft.) under roof 3983 | | YES / NO / N/A |

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL R101.2.1

## Site Plan information including:

| | | |
|---|---|---|
| 4 | Dimensions of lot or parcel of land | YES |
| 5 | Dimensions of all building set backs | YES |
| 6 | Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements. | YES |
| 7 | Provide a full legal description of property. | YES |

## Wind-load Engineering Summary, calculations and any details are required.

| | GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL | Items to Include- Each Box shall be Marked as Applicable |
|---|---|---|
| 8 | **Plans or specifications must show compliance with FBCR Chapter 3** | YES / NO / N/A |

| | | Select From the Dropbox |
|---|---|---|
| 9 | Basic wind speed (3-second gust), miles per hour | YES |
| 10 | (Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated) | YES |
| 11 | Wind importance factor and nature of occupancy | YES |
| 12 | The applicable internal pressure coefficient, Components and Cladding | YES |
| 13 | The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component, cladding materials not specifally designed by the registered design professional. | YES |

## Elevations Drawing including:

| | | |
|---|---|---|
| 14 | All side views of the structure | YES |
| 15 | Roof pitch | YES |
| 16 | Overhang dimensions and detail with attic ventilation | YES |
| 17 | Location, size and height above roof of chimneys | YES |
| 18 | Location and size of skylights with Florida Product Approval | YES |
| 18 | Number of stories | YES |
| 20A | Building height from the established grade to the roofs highest peak | YES |

## Floor Plan including:

| | | |
|---|---|---|
| 20 | Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies | YES |
| 21 | Raised floor surfaces located more than 30 inches above the floor or grade | YES |
| 22 | All exterior and interior shear walls indicated | YES |
| 23 | Shear wall opening shown (Windows, Doors and Garage doors) | YES |
| 24 | Show compliance with Section FBCR 310 Emergency escape and rescue opening shown in each bedroom (net clear opening shown) and Show compliance with Section FBC 1405.13.2 where the opening of an operable window is located more than 72 inches above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches above the finished floor of the room in which the window is located. Glazing between the floor and 24 inches shall be fixed or have openings through which a 4-inch-diameter sphere cannot pass. | YES |
| 25 | Safety glazing of glass where needed | YES |
| 26 | Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 and chapter 24 of FBCR) | YES |
| 27 | Show stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails | YES |
| 28 | Identify accessibility of bathroom (see FBCR SECTION 320) | YES |

**All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)**

| GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL | Items to Include- Each Box shall be Marked as Applicable |
|---|---|

YES / NO / N/A

## FBCR 403: Foundation Plans

Select From the Dropbox

| | | |
|---|---|---|
| 29 | Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing. | YES |
| 30 | All posts and/or column footing including size and reinforcing | YES |
| 31 | Any special support required by soil analysis such as piling. | YES |
| 32 | Assumed load-bearing valve of soil_____ Pound Per Square Foot | YES |
| 33 | Location of horizontal and vertical steel, for foundation or walls (include # size and type) For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an grounding electrode system. Per the National Electrical Code article 250.52.3 | YES |

## FBCR 506: CONCRETE SLAB ON GRADE

| | | |
|---|---|---|
| 34 | Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed) | YES |
| 35 | Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports | YES |

## FBCR 318: PROTECTION AGAINST TERMITES

| | | |
|---|---|---|
| 36 | Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or Submit other approved termite protection methods. **Protection shall be provided by registered termiticides** | YES |

## FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

| | | |
|---|---|---|
| 37 | Show all materials making up walls, wall height, and Block size, mortar type | YES |
| 38 | Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement | YES |

**Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect**

## Floor Framing System: First and/or second story

| | | |
|---|---|---|
| 39 | Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer | YES |

| | | |
|---|---|---|
| 40 | Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers | YES |
| 41 | Girder type, size and spacing to load bearing walls, stem wall and/or priers | YES |
| 42 | Attachment of joist to girder | YES |
| 43 | Wind load requirements where applicable | YES |
| 44 | Show required under-floor crawl space | YES |
| 45 | Show required amount of ventilation opening for under-floor spaces | YES |
| 46 | Show required covering of ventilation opening | YES |
| 47 | Show the required access opening to access to under-floor spaces | YES |
| 48 | Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing | YES |
| 49 | Show Draftstopping, Fire caulking and Fire blocking | YES |
| 50 | Show fireproofing requirements for garages attached to living spaces, per FBCR section 302.6 | YES |
| 51 | Provide live and dead load rating of floor framing systems (psf). | YES |

YES / NO / N/A

## FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

| GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL | | Items to Include- Each Box shall be Marked as Applicable |
|---|---|---|
| | | Select From the Dropbox |
| 52 | Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls | YES |
| 53 | Fastener schedule for structural members per table IRC 602.3 are to be shown | YES |
| 54 | Show Wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing | YES |
| 55 | Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems | YES |
| 56 | Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per IRC Table 502.5 (1) | YES |
| 57 | Indicate where pressure treated wood will be placed | YES |
| 58 | Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas | YES |
| 59 | A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail | YES |

## FBCR :ROOF SYSTEMS:

| | | |
|---|---|---|
| 60 | Truss design drawing shall meet section FBCR 802.1.6.1 Wood trusses | YES |
| 61 | Include a layout and truss details, signed and sealed by Florida Professional Engineer | YES |
| 62 | Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters | YES |
| 63 | Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details | YES |
| 64 | Provide dead load rating of trusses | YES |

## FBCR 802:Conventional Roof Framing Layout

| | | |
|---|---|---|
| 65 | Rafter and ridge beams sizes, span, species and spacing | YES |
| 66 | Connectors to wall assemblies' include assemblies' resistance to uplift rating | YES |
| 67 | Valley framing and support details | YES |
| 68 | Provide dead load rating of rafter system | YES |

## FBCR 803 ROOF SHEATHING

| | | |
|---|---|---|
| 69 | Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness | YES |
| 70 | Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas | YES |

## ROOF ASSEMBLIES FRC Chapter 9

| | | |
|---|---|---|
| 71 | Include all materials which will make up the roof assembles covering | YES |
| 72 | Submit Florida Product Approval numbers for each component of the roof assembles covering | YES |

## FBCR Chapter 11 Energy Efficiency Code for residential building

Residential construction shall comply with this code by using the following compliance methods in the FBCR chapter11 Residential buildings compliance methods. **Two of the required forms are to be submitted,** *N1100.1.1.1 As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form 600A, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.*

YES / NO / N/A

| | GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL | Items to Include- Each Box shall be Marked as Applicable |
|---|---|---|
| | | **Select From the Dropbox** |
| 73 | Show the insulation R value for the following areas of the structure | YES |
| 74 | Attic space | YES |
| 75 | Exterior wall cavity | YES |
| 76 | Crawl space | YES |

### HVAC information

| | | |
|---|---|---|
| 77 | Submit two copies of a Manual J sizing equipment or equivalent computation study | YES |
| 78 | Exhaust fans shown in bathrooms **Mechanical exhaust capacity of 50 cfm intermittent or 20 cfm continuous required** | YES |
| 79 | Show clothes dryer route and total run of exhaust duct | YES |

### Plumbing Fixture layout shown

| | | |
|---|---|---|
| 80 | All fixtures waste water lines shall be shown on the foundation plan | YES |
| 81 | Show the location of water heater | YES |

### Private Potable Water

| | | |
|---|---|---|
| 82 | Pump motor horse power | YES |
| 83 | Reservoir pressure tank gallon capacity | YES |
| 84 | Rating of cycle stop valve if used | YES |

### Electrical layout shown including

| | | |
|---|---|---|
| 85 | Show Switches, receptacles outlets, lighting fixtures and Ceiling fans | YES |
| 86 | Show all 120-volt, single phase, 15- and 20-ampere branch circuits outlets required to be protected by **Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A** | YES |
| 87 | Show the location of smoke detectors & Carbon monoxide detectors | YES |
| 88 | Show service panel, sub-panel, location(s) and total ampere ratings | YES |
| 89 | On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.<br><br>**For structures** with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an Grounding electrode system. Per the National Electrical Code article 250.52.3 | YES |
| 90 | Appliances and HVAC equipment and disconnects | YES |
| 91 | Show all 120-volt, single phase, 15- and 20-ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sunrooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed **Combination arc-fault circuit interrupter,** Protection device. | N/A |

| GENERAL REQUIREMENTS:<br>APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL | Items to Include- Each Box shall be Circled as Applicable |
|---|---|

## THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

| | | YES | NO | N/A |
|---|---|---|---|---|
| 92 | **Building Permit Application** A current Building Permit Application is to be completed, by following the Checklist all supporting documents must be submitted. There is a **$15.00** application fee. The completed application with attached documents and application fee can be mailed. | YES | | |
| 93 | **Parcel Number** The parcel number (Tax ID number) from the Property Appraisers Office (386) 758-1083 is required. A copy of property deed is also required. www.columbiacountyfla.com | YES | | |
| 94 | **Town of Fort White** (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White, an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit. | NO | | |
| *** | **BELOW ITEMS ONLY NEEDED AFTER ZONING APPROVAL HAS GIVEN.** | **** | *** | *** |
| 95 | **Environmental Health Permit or Sewer Tap Approval** A copy of a approved Columbia County Environmental Health (386) 758-1058 | YES | | |
| 96 | **City of Lake City** A City Water and/or Sewer letter. Call 386-752-2031 | YES | | |
| 97 | **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations | NO | | |
| 98 | **CERTIFIED FINISHED FLOOR ELEVATIONS** will be required on any project where the approved FIRM Flood Maps show the property is in a AE, Floodway, and AH flood zones. Additionally One Foot Rise letters are required for AE and AH zones. In the Floodway Flood zones a Zero Rise letter is required. | | | |
| 99 | A Flood development permit is also required for AE, Floodway & AH. Development permit cost is **$50.00** | | | |
| 100 | **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (**$25.00**) must be made. County Public Works Dept. determines the size and length of every culvert before instillation and completes a final inspection before permanent power is granted. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (**$50.00**) Separate Check when issued. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required. | YES | | |
| 101 | **911 Address:** An application for a 911address must be applied for and **received** through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125. | YES | | |

**TOILET FACILITIES SHALL BE PROVIDED FOR ALL CONSTRUCTION SITES.** NO

**Disclosure Statement for Owner Builders** *If you as the applicant will be acting as an owner/builder under section 489.103(7) of the Florida Statutes, submit the required owner builder disclosure statement form.*

**Notice Of Commencement**

A notice of commencement form **recorded** in the Columbia County Clerk Office is required to be filed with the building department Before Any Inspections can be preformed.

**Section R101.2.1 of the Florida Building Code Residential:**

**The provisions of Chapter 1, Florida Building Code shall govern the administration and enforcement of the Florida Building Code, Residential.**

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

| Category/Subcategory | Manufacturer | Product Description | Approval Number(s) |
|---|---|---|---|
| 1. EXTERIOR DOORS | | | |
| A. SWINGING | PGT IND. | EXTERIOR DOORS | FL253-R12 |
| B. SLIDING | | | |
| C. SECTIONAL/ROLL UP | OVERHEAD DOORS | GARAGE DOOR | FL742-R6 |
| D. OTHER | | | |
| | | | |
| 2. WINDOWS | | | |
| A. SINGLE/DOUBLE HUNG | PGT IND. | SH WINDOWS | FL239-R19 |
| B. HORIZONTAL SLIDER | | | |
| C. CASEMENT | | | |
| D. FIXED | | | |
| E. MULLION | | | |
| F. SKYLIGHTS | | | |
| G. OTHER | | | |
| | | | |
| 3. PANEL WALL | | | |
| A. SIDING | JAMES HARDI | HARDIBOARD SIDING | FLB192-R2 |
| B. SOFFITS | | | |
| C. STOREFRONTS | | | |
| D. GLASS BLOCK | | | |
| E. OTHER | | | |
| | | | |
| 4. ROOFING PRODUCTS | | | |
| A. ASPHALT SHINGLES | TAMKO | ASPHALT SHINGLES | FL1956-R8 |
| B. NON-STRUCTURAL METAL | | | |
| C. ROOFING TILES | | | |
| D. SINGLE PLY ROOF | | | |
| E. OTHER | | | |
| | | | |
| 5. STRUCTURAL COMPONENTS | | | |
| A. WOOD CONNECTORS | | | |
| B. WOOD ANCHORS | | | |
| C. TRUSS PLATES | | | |
| D. INSULATION FORMS | | | |
| E. LINTELS | | | |
| F. OTHERS | | | |
| | | | |
| 6. NEW EXTERIOR | | | |
| ENVELOPE PRODUCTS | | | |

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.
Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

Contractor OR Agent Signature Date

NOTES: _____

# Residential System Sizing Calculation

## Summary

Project Title:
Karlton Residence

Lake City, FL 32055

SCANNED

1/30/2020

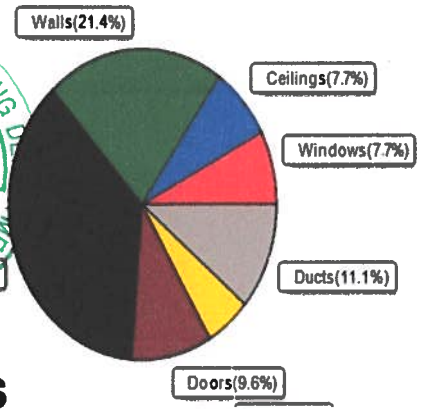
| Location for weather data: Gainesville, FL - Defaults: Latitude(29.7) Altitude(152 ft.) Temp Range(M) | | | | | |
|---|---|---|---|---|---|
| Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.) | | | | | |
| Winter design temperature(TMY3 99%) | 30 | F | Summer design temperature(TMY3 99%) | 94 | F |
| Winter setpoint | 70 | F | Summer setpoint | 75 | F |
| Winter temperature difference | 40 | F | Summer temperature difference | 19 | F |
| **Total heating load calculation** | **38014** | **Btuh** | **Total cooling load calculation** | **24866** | **Btuh** |
| Submitted heating capacity | % of calc | Btuh | Submitted cooling capacity | % of calc | Btuh |
| Total (Electric Heat Pump) | 94.7 | 36000 | Sensible (SHR = 0.85) | 139.9 | 30600 |
| Heat Pump + Auxiliary(0.0kW) | 94.7 | 36000 | Latent | 180.6 | 5400 |
| | | | Total (Electric Heat Pump) | 144.8 | 36000 |

## WINTER CALCULATIONS

Winter Heating Load (for 2286 sqft)

| Load component | | | Load | |
|---|---|---|---|---|
| Window total | 222 | sqft | 2930 | Btuh |
| Wall total | 2360 | sqft | 8138 | Btuh |
| Door total | 228 | sqft | 3648 | Btuh |
| Ceiling total | 2286 | sqft | 2912 | Btuh |
| Floor total | 2286 | sqft | 14113 | Btuh |
| Infiltration | 47 | cfm | 2042 | Btuh |
| Duct loss | | | 4230 | Btuh |
| **Subtotal** | | | **38014** | **Btuh** |
| Ventilation | 0 | cfm | 0 | Btuh |
| **TOTAL HEAT LOSS** | | | **38014** | **Btuh** |

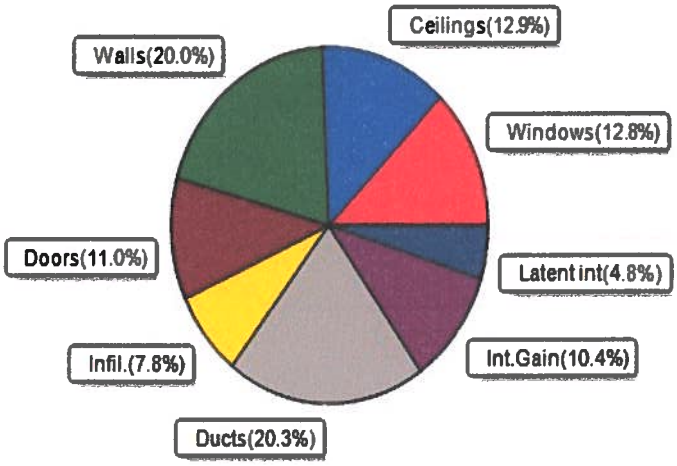





## SUMMER CALCULATIONS

Summer Cooling Load (for 2286 sqft)

| Load component | | | Load | |
|---|---|---|---|---|
| Window total | 222 | sqft | 3182 | Btuh |
| Wall total | 2360 | sqft | 4981 | Btuh |
| Door total | 228 | sqft | 2736 | Btuh |
| Ceiling total | 2286 | sqft | 3203 | Btuh |
| Floor total | | | 0 | Btuh |
| Infiltration | 35 | cfm | 727 | Btuh |
| Internal gain | | | 2580 | Btuh |
| Duct gain | | | 4466 | Btuh |
| Sens. Ventilation | 0 | cfm | 0 | Btuh |
| Blower Load | | | 0 | Btuh |
| **Total sensible gain** | | | **21876** | **Btuh** |
| Latent gain(ducts) | | | 583 | Btuh |
| Latent gain(infiltration) | | | 1207 | Btuh |
| Latent gain(ventilation) | | | 0 | Btuh |
| Latent gain(internal/occupants/other) | | | 1200 | Btuh |
| **Total latent gain** | | | **2990** | **Btuh** |
| **TOTAL HEAT GAIN** | | | **24866** | **Btuh** |




Approved by ACCA MANUAL J 8th Edition

EnergyGauge® System Sizing
PREPARED BY: _____
DATE: 1-30-20

# System Sizing Calculations - Summer

## Residential Load - Whole House Component Details

Project Title:
Karlton Residence

Lake City, FL 32055

1/30/2020

Reference City: Gainesville, FL  Temperature Difference: 19.0F(TMY3 99%)  Humidity difference: 51gr.

### Component Loads for Whole House

| Window | Type* Panes | SHGC | U | InSh | IS | Omt | Overhang Len | Hgt | Window Area(sqft) Gross | Shaded | Unshaded | HTM Shaded | Unshaded | Load |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 1 | 2 NFRC | 0.22, 0.33 | | No | No | N | 1.5ft | 1.3ft | 36.0 | 0.0 | 36.0 | 11 | 11 | 392 Btuh |
| 2 | 2 NFRC | 0.22, 0.33 | | No | No | N | 11.5f | 1.3ft | 24.0 | 0.0 | 24.0 | 11 | 11 | 262 Btuh |
| 3 | 2 NFRC | 0.22, 0.33 | | No | No | N | 1.5ft | 1.3ft | 8.0 | 0.0 | 8.0 | 11 | 11 | 87 Btuh |
| 4 | 2 NFRC | 0.22, 0.33 | | No | No | E | 1.5ft | 1.3ft | 30.0 | 0.0 | 30.0 | 11 | 27 | 824 Btuh |
| 5 | 2 NFRC | 0.22, 0.33 | | No | No | S | 1.5ft | 1.3ft | 36.0 | 36.0 | 0.0 | 11 | 13 | 392 Btuh |
| 6 | 2 NFRC | 0.22, 0.33 | | No | No | S | 11.5f | 1.3ft | 72.0 | 72.0 | 0.0 | 11 | 13 | 785 Btuh |
| 7 | 2 NFRC | 0.22, 0.33 | | No | No | W | 1.5ft | 1.3ft | 16.0 | 0.0 | 16.0 | 11 | 27 | 440 Btuh |
| | Window Total | | | | | | | | 222 (sqft) | | | | | 3182 Btuh |

| Walls | Type | U-Value | R-Value Cav/Sheath | Area(sqft) | HTM | Load |
|---|---|---|---|---|---|---|
| 1 | Frame - Wood - Ext | 0.09 | 13.0/0.6 | 228.0 | 2.2 | 501 Btuh |
| 2 | Frame - Wood - Ext | 0.09 | 13.0/0.6 | 211.5 | 2.2 | 465 Btuh |
| 3 | Frame - Wood - Ext | 0.09 | 13.0/0.6 | 301.7 | 2.2 | 663 Btuh |
| 4 | Frame - Wood - Ext | 0.09 | 13.0/0.6 | 18.0 | 2.2 | 40 Btuh |
| 5 | Frame - Wood - Ext | 0.09 | 13.0/0.6 | 112.0 | 2.2 | 246 Btuh |
| 6 | Frame - Wood - Ext | 0.09 | 13.0/0.6 | 51.0 | 2.2 | 112 Btuh |
| 7 | Frame - Wood - Ext | 0.09 | 13.0/0.6 | 24.0 | 2.2 | 53 Btuh |
| 8 | Frame - Wood - Ext | 0.09 | 13.0/0.6 | 150.0 | 2.2 | 330 Btuh |
| 9 | Frame - Wood - Ext | 0.09 | 13.0/0.6 | 24.0 | 2.2 | 53 Btuh |
| 10 | Frame - Wood - Ext | 0.09 | 13.0/0.6 | 102.0 | 2.2 | 224 Btuh |
| 11 | Frame - Wood - Ext | 0.09 | 13.0/0.6 | 84.0 | 2.2 | 185 Btuh |
| 12 | Frame - Wood - Ext | 0.09 | 13.0/0.6 | 115.5 | 2.2 | 254 Btuh |
| 13 | Frame - Wood - Ext | 0.09 | 13.0/0.6 | 348.3 | 2.2 | 766 Btuh |
| 14 | Frame - Wood - Ext | 0.09 | 13.0/0.6 | 219.5 | 2.2 | 483 Btuh |
| 15 | Frame - Wood - Adj | 0.09 | 13.0/0.6 | 370.5 | 1.6 | 607 Btuh |
| | Wall Total | | | 2360 (sqft) | | 4981 Btuh |

| Doors | Type | Area (sqft) | HTM | Load |
|---|---|---|---|---|
| 1 | Insulated - Exterior | 24.0 | 12.0 | 288 Btuh |
| 2 | Insulated - Exterior | 48.0 | 12.0 | 576 Btuh |
| 3 | Insulated - Exterior | 48.0 | 12.0 | 576 Btuh |
| 4 | Insulated - Exterior | 48.0 | 12.0 | 576 Btuh |
| 5 | Insulated - Garage | 20.0 | 12.0 | 240 Btuh |
| 6 | Insulated - Garage | 40.0 | 12.0 | 480 Btuh |
| | Door Total | 228 (sqft) | | 2736 Btuh |

| Ceilings | Type/Color/Surface | U-Value | R-Value | Area(sqft) | HTM | Load |
|---|---|---|---|---|---|---|
| 1 | Vented Attic/Light/Metal | 0.032 | 30.0/0.0 | 2286.0 | 1.40 | 3203 Btuh |
| | Ceiling Total | | | 2286 (sqft) | | 3203 Btuh |

| Floors | Type | R-Value | Size | HTM | Load |
|---|---|---|---|---|---|
| 1 | Slab On Grade | 0.0 | 2286 (ft-perimeter) | 0.0 | 0 Btuh |
| | Floor Total | | 2286.0 (sqft) | | 0 Btuh |

| | | Envelope Subtotal: | 14102 Btuh |
|---|---|---|---|

# Manual J Summer Calculations

## Residential Load - Component Details (continued)

Project Title: Karlton Residence

Climate:FL_GAINESVILLE_REGIONAL_A

Lake City, FL 32055

1/30/2020

| | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| **Infiltration** | Type<br>Natural | Average ACH<br>0.11 | Volume(cuft)<br>18288 | Wall Ratio<br>1 | | CFM=<br>35.0 | Load<br>727 | Btuh |
| **Internal gain** | | Occupants<br>6 | Btuh/occupant<br>X 230 | | + | Appliance<br>1200 | Load<br>2580 | Btuh |
| | | | | | Sensible Envelope Load: | | 17409 | Btuh |
| **Duct load** | Extremely sealed, Supply(R6.0-Attic), Return(R6.0-Attic) | | | | (DGM of 0.257) | | 4466 | Btuh |
| | | | | | **Sensible Load All Zones** | | **21876** | **Btuh** |

# Manual J Summer Calculations

## Residential Load - Component Details (continued)

Project Title: Karlton Residence
Climate:FL_GAINESVILLE_REGIONAL_A

Lake City, FL 32055

1/30/2020

### WHOLE HOUSE TOTALS

| | | |
|---|---|---|
| | **Sensible Envelope Load All Zones** | **17409 Btuh** |
| | Sensible Duct Load | 4466 Btuh |
| | **Total Sensible Zone Loads** | **21876 Btuh** |
| | Sensible ventilation | 0 Btuh |
| | Blower | 0 Btuh |
| **Whole House** | **Total sensible gain** | **21876 Btuh** |
| **Totals for Cooling** | Latent infiltration gain (for 51 gr. humidity difference) | 1207 Btuh |
| | Latent ventilation gain | 0 Btuh |
| | Latent duct gain | 583 Btuh |
| | Latent occupant gain (6.0 people @ 200 Btuh per person) | 1200 Btuh |
| | Latent other gain | 0 Btuh |
| | **Latent total gain** | **2990 Btuh** |
| | **TOTAL GAIN** | **24866 Btuh** |

### EQUIPMENT

| | | |
|---|---|---|
| 1. Central Unit | # | 36000 Btuh |

*Key: Window types (Panes - Number and type of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value)
(U - Window U-Factor)
(InSh - Interior shading device: none(No), Blinds(B), Draperies(D) or Roller Shades(R))
- For Blinds: Assume medium color, half closed
For Draperies: Assume medium weave, half closed
For Roller shades: Assume translucent, half closed
(IS - Insect screen: none(N), Full(F) or Half(½))
(Ornt - compass orientation)

Version 8

# System Sizing Calculations - Winter

## Residential Load - Whole House Component Details

Project Title:
Karlton Residence
Building Type: User

Lake City, FL 32055

1/30/2020

Reference City: Gainesville, FL (Defaults) Winter Temperature Difference: 40.0 F (TMY3 99%)

**Component Loads for Whole House**

| Window | Panes/Type | Frame | U | Orientation | Area(sqft) X | HTM= | Load |
|---|---|---|---|---|---|---|---|
| 1 | 2, NFRC 0.22 | Vinyl | 0.33 | N | 36.0 | 13.2 | 475 Btuh |
| 2 | 2, NFRC 0.22 | Vinyl | 0.33 | N | 24.0 | 13.2 | 317 Btuh |
| 3 | 2, NFRC 0.22 | Vinyl | 0.33 | N | 8.0 | 13.2 | 106 Btuh |
| 4 | 2, NFRC 0.22 | Vinyl | 0.33 | E | 30.0 | 13.2 | 396 Btuh |
| 5 | 2, NFRC 0.22 | Vinyl | 0.33 | S | 36.0 | 13.2 | 475 Btuh |
| 6 | 2, NFRC 0.22 | Vinyl | 0.33 | S | 72.0 | 13.2 | 950 Btuh |
| 7 | 2, NFRC 0.22 | Vinyl | 0.33 | W | 16.0 | 13.2 | 211 Btuh |
| | Window Total | | | | 222.0(sqft) | | 2930 Btuh |

| Walls | Type | Ornt. | Ueff. | R-Value (Cav/Sh) | Area X | HTM= | Load |
|---|---|---|---|---|---|---|---|
| 1 | Frame - Wood | - Ext | (0.086) | 13.0/0.6 | 228 | 3.45 | 786 Btuh |
| 2 | Frame - Wood | - Ext | (0.086) | 13.0/0.6 | 212 | 3.45 | 729 Btuh |
| 3 | Frame - Wood | - Ext | (0.086) | 13.0/0.6 | 302 | 3.45 | 1040 Btuh |
| 4 | Frame - Wood | - Ext | (0.086) | 13.0/0.6 | 18 | 3.45 | 62 Btuh |
| 5 | Frame - Wood | - Ext | (0.086) | 13.0/0.6 | 112 | 3.45 | 386 Btuh |
| 6 | Frame - Wood | - Ext | (0.086) | 13.0/0.6 | 51 | 3.45 | 176 Btuh |
| 7 | Frame - Wood | - Ext | (0.086) | 13.0/0.6 | 24 | 3.45 | 83 Btuh |
| 8 | Frame - Wood | - Ext | (0.086) | 13.0/0.6 | 150 | 3.45 | 517 Btuh |
| 9 | Frame - Wood | - Ext | (0.086) | 13.0/0.6 | 24 | 3.45 | 83 Btuh |
| 10 | Frame - Wood | - Ext | (0.086) | 13.0/0.6 | 102 | 3.45 | 352 Btuh |
| 11 | Frame - Wood | - Ext | (0.086) | 13.0/0.6 | 84 | 3.45 | 290 Btuh |
| 12 | Frame - Wood | - Ext | (0.086) | 13.0/0.6 | 116 | 3.45 | 398 Btuh |
| 13 | Frame - Wood | - Ext | (0.086) | 13.0/0.6 | 348 | 3.45 | 1201 Btuh |
| 14 | Frame - Wood | - Ext | (0.086) | 13.0/0.6 | 220 | 3.45 | 757 Btuh |
| 15 | Frame - Wood | - Adj | (0.086) | 13.0/0.6 | 371 | 3.45 | 1278 Btuh |
| | Wall Total | | | | 2360(sqft) | | 8138 Btuh |

| Doors | Type | Storm | Ueff. | Area X | HTM= | Load |
|---|---|---|---|---|---|---|
| 1 | Insulated - Exterior, | n | (0.400) | 24 | 16.0 | 384 Btuh |
| 2 | Insulated - Exterior, | n | (0.400) | 48 | 16.0 | 768 Btuh |
| 3 | Insulated - Exterior, | n | (0.400) | 48 | 16.0 | 768 Btuh |
| 4 | Insulated - Exterior, | n | (0.400) | 48 | 16.0 | 768 Btuh |
| 5 | Insulated - Garage, | n | (0.400) | 20 | 16.0 | 320 Btuh |
| 6 | Insulated - Garage, | n | (0.400) | 40 | 16.0 | 640 Btuh |
| | Door Total | | | 228(sqft) | | 3648Btuh |

| Ceilings | Type/Color/Surface | Ueff. | R-Value | Area X | HTM= | Load |
|---|---|---|---|---|---|---|
| 1 | Vented Attic/L/Metal | (0.032) | 30.0/0.0 | 2286 | 1.3 | 2912 Btuh |
| | Ceiling Total | | | 2286(sqft) | | 2912Btuh |

| Floors | Type | Ueff. | R-Value | Size X | HTM= | Load |
|---|---|---|---|---|---|---|
| 1 | Slab On Grade | (1.180) | 0.0 | 299.0 ft(perim.) | 47.2 | 14113 Btuh |
| | Floor Total | | | 2286 sqft | | 14113 Btuh |

# Manual J Winter Calculations

## Residential Load - Component Details (continued)

Project Title:
Karlton Residence
Building Type: User

Lake City, FL 32055

1/30/2020

| | | | | | | | |
|---|---|---|---|---|---|---|---|
| | | | | Envelope Subtotal: | | | 31742 Btuh |
| **Infiltration** | Type<br>Natural | Wholehouse | ACH<br>0.15 | Volume(cuft)<br>18288 | Wall Ratio<br>1.00 | CFM=<br>46.6 | 2042 Btuh |
| **Duct load** | Extremely sealed, R6.0, Supply(Att), Return(Att) | | | | (DLM of 0.125) | | 4230 Btuh |
| **All Zones** | | | | **Sensible Subtotal All Zones** | | | **38014 Btuh** |

## WHOLE HOUSE TOTALS

| | | |
|---|---|---|
| **Totals for Heating** | Subtotal Sensible Heat Loss<br>Ventilation Sensible Heat Loss<br>Total Heat Loss | 38014 Btuh<br>0 Btuh<br>38014 Btuh |

## EQUIPMENT

| | | |
|---|---|---|
| 1. Electric Heat Pump | # | 36000 Btuh |

Key: Window types - NFRC (Requires U-Factor and Shading coefficient(SHGC) of glass as numerical values)
or - Glass as 'Clear' or 'Tint' (Uses U-Factor and SHGC defaults)
U - (Window U-Factor)
HTM - (ManualJ Heat Transfer Multiplier)

Version 8

FORM R405-2017

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

## Florida Department of Business and Professional Regulation - Residential Performance Method

| | | | |
|---|---|---|---|
| Project Name: | Karlton Residence | Builder Name: | Aaron Simque Homes |
| Street: | | Permit Office: | |
| City, State, Zip: | Lake City , FL , 32055 | Permit Number: | |
| Owner: | | Jurisdiction: | |
| Design Location: | FL, Gainesville | County: | Columbia (Florida Climate Zone 2 ) |

SCANNED

| | |
|---|---|
| 1. New construction or existing | New (From Plans) |
| 2. Single family or multiple family | Single-family |
| 3. Number of units, if multiple family | 1 |
| 4. Number of Bedrooms | 3 |
| 5. Is this a worst case? | No |
| 6. Conditioned floor area above grade (ft²) | 2286 |
| Conditioned floor area below grade (ft²) | 0 |

| 7. Windows(222.0 sqft.) | Description | Area |
|---|---|---|
| a. U-Factor: | Dbl, U=0.33 | 222.00 ft² |
| SHGC: | SHGC=0.22 | |
| b. U-Factor: | N/A | ft² |
| SHGC: | | |
| c. U-Factor: | N/A | ft² |
| SHGC: | | |
| d. U-Factor: | N/A | ft² |
| SHGC: | | |
| Area Weighted Average Overhang Depth: | | 5.824 ft. |
| Area Weighted Average SHGC: | | 0.220 |

| 8. Floor Types (2286.0 sqft.) | Insulation | Area |
|---|---|---|
| a. Slab-On-Grade Edge Insulation | R=0.0 | 2286.00 ft² |
| b. N/A | R= | ft² |
| c. N/A | R= | ft² |

| 9. Wall Types (2810.0 sqft.) | Insulation | Area |
|---|---|---|
| a. Frame - Wood, Exterior | R=13.0 | 2379.50 ft² |
| b. Frame - Wood, Adjacent | R=13.0 | 430.50 ft² |
| c. N/A | R= | ft² |
| d. N/A | R= | ft² |

| 10. Ceiling Types (2286.0 sqft.) | Insulation | Area |
|---|---|---|
| a. Under Attic (Vented) | R=30.0 | 2286.00 ft² |
| b. N/A | R= | ft² |
| c. N/A | R= | ft² |

| 11. Ducts | R | ft² |
|---|---|---|
| a. Sup: Attic, Ret: Attic, AH: Main | 6 | 457.2 |

| 12. Cooling systems | kBtu/hr | Efficiency |
|---|---|---|
| a. Central Unit | 36.0 | SEER:14.00 |

| 13. Heating systems | kBtu/hr | Efficiency |
|---|---|---|
| a. Electric Heat Pump | 36.0 | HSPF:8.50 |

| 14. Hot water systems | |
|---|---|
| a. Electric | Cap: 40 gallons<br>EF: 0.920 |
| b. Conservation features<br>None | |
| 15. Credits | CF, Pstat |

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

Glass/Floor Area: 0.097

Total Proposed Modified Loads: 63.77

Total Baseline Loads: 64.74

**PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 1-30-20

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA * IN GOD WE TRUST *

BUILDING OFFICIAL: _____

DATE: _____

**- Compliance requires certification by the air handler unit manufacturer that the air handler enclosure qualifies as certified factory-sealed in accordance with R403.3.2.1.**

**- Compliance requires an Air Barrier and Insulation Inspection Checklist in accordance with R402.4.1.1 and this project requires an envelope leakage test report with envelope leakage no greater than 5.00 ACH50 (R402.4.1.2).**

**- Compliance with a proposed duct leakage Qn requires a Duct Leakage Test Report confirming duct leakage to outdoors, tested in accordance with ANSI/RESNET/ICC 380, is not greater than 0.030 Qn for whole house.**

# INPUT SUMMARY CHECKLIST REPORT

## PROJECT

| | | | | | |
|---|---|---|---|---|---|
| Title: | Karlton Residence | Bedrooms: | 3 | Address Type: | Lot Information |
| Building Type: | User | Conditioned Area: | 2150 | Lot # | |
| Owner Name: | | Total Stories: | 1 | Block/Subdivision: | High Pointe |
| # of Units: | 1 | Worst Case: | No | PlatBook: | |
| Builder Name: | Aaron Simque Homes | Rotate Angle: | 0 | Street: | |
| Permit Office: | | Cross Ventilation: | | County: | Columbia |
| Jurisdiction: | | Whole House Fan: | | City, State, Zip: | Lake City , FL , 32055 |
| Family Type: | Single-family | | | | |
| New/Existing: | New (From Plans) | | | | |
| Comment: | | | | | |

## CLIMATE

| ✓ | Design Location | TMY Site | Design Temp 97.5 % | Design Temp 2.5 % | Int Design Temp Winter | Int Design Temp Summer | Heating Degree Days | Design Moisture | Daily Temp Range |
|---|---|---|---|---|---|---|---|---|---|
| _____ | FL, Gainesville | FL_GAINESVILLE_REGI | 32 | 92 | 70 | 75 | 1305.5 | 51 | Medium |

## BLOCKS

| Number | Name | Area | Volume |
|---|---|---|---|
| 1 | Block1 | 2286 | 18288 |

## SPACES

| Number | Name | Area | Volume | Kitchen | Occupants | Bedrooms | Infil ID | Finished | Cooled | Heated |
|---|---|---|---|---|---|---|---|---|---|---|
| 1 | Main | 2286 | 18288 | Yes | 6 | 3 | 1 | Yes | Yes | Yes |

## FLOORS

| ✓ | # | Floor Type | Space | Perimeter | R-Value | Area | | Tile | Wood | Carpet |
|---|---|---|---|---|---|---|---|---|---|---|
| _____ | 1 | Slab-On-Grade Edge Insulatio | Main | 299 ft | 0 | 2286 ft² | ---- | 0.33 | 0.33 | 0.34 |

## ROOF

| ✓ | # | Type | Materials | Roof Area | Gable Area | Roof Color | Rad Barr | Solar Absor. | SA Tested | Emitt | Emitt Tested | Deck Insul. | Pitch (deg) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| _____ | 1 | Gable or shed | Metal | 2748 ft² | 762 ft² | Light | N | 0.6 | No | 0.9 | No | 0 | 33.7 |

## ATTIC

| ✓ | # | Type | Ventilation | Vent Ratio (1 in) | Area | RBS | IRCC |
|---|---|---|---|---|---|---|---|
| _____ | 1 | Full attic | Vented | 300 | 2286 ft² | N | N |

## CEILING

| ✓ | # | Ceiling Type | Space | R-Value | Ins Type | Area | Framing Frac | Truss Type |
|---|---|---|---|---|---|---|---|---|
| _____ | 1 | Under Attic (Vented) | Main | 30 | Blown | 2286 ft² | 0.11 | Wood |

# INPUT SUMMARY CHECKLIST REPORT

## WALLS

| ✓ | # | Ornt | Adjacent To | Wall Type | Space | Cavity R-Value | Width Ft | Width In | Height Ft | Height In | Area | Sheathing R-Value | Framing Fraction | Solar Absor. | Below Grade% |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | 1 | N | Exterior | Frame - Wood | Main | 13 | 29 | 4 | 9 | | 264.0 ft² | 0.625 | 0.23 | 0.75 | 0 |
| | 2 | E | Exterior | Frame - Wood | Main | 13 | 26 | 2 | 9 | | 235.5 ft² | 0.625 | 0.23 | 0.75 | 0 |
| | 3 | N | Exterior | Frame - Wood | Main | 13 | 42 | 2 | 10 | | 421.7 ft² | 0.625 | 0.23 | 0.75 | 0 |
| | 4 | W | Exterior | Frame - Wood | Main | 13 | 2 | | 9 | | 18.0 ft² | 0.625 | 0.23 | 0.75 | 0 |
| | 5 | N | Exterior | Frame - Wood | Main | 13 | 13 | 4 | 9 | | 120.0 ft² | 0.625 | 0.23 | 0.75 | 0 |
| | 6 | E | Exterior | Frame - Wood | Main | 13 | 5 | 8 | 9 | | 51.0 ft² | 0.625 | 0.23 | 0.75 | 0 |
| | 7 | N | Exterior | Frame - Wood | Main | 13 | 2 | 8 | 9 | | 24.0 ft² | 0.625 | 0.23 | 0.75 | 0 |
| | 8 | E | Exterior | Frame - Wood | Main | 13 | 20 | | 9 | | 180.0 ft² | 0.625 | 0.23 | 0.75 | 0 |
| | 9 | S | Exterior | Frame - Wood | Main | 13 | 2 | 8 | 9 | | 24.0 ft² | 0.625 | 0.23 | 0.75 | 0 |
| | 10 | E | Exterior | Frame - Wood | Main | 13 | 11 | 4 | 9 | | 102.0 ft² | 0.625 | 0.23 | 0.75 | 0 |
| | 11 | S | Exterior | Frame - Wood | Main | 13 | 13 | 4 | 9 | | 120.0 ft² | 0.625 | 0.23 | 0.75 | 0 |
| | 12 | W | Exterior | Frame - Wood | Main | 13 | 12 | 10 | 9 | | 115.5 ft² | 0.625 | 0.23 | 0.75 | 0 |
| | 13 | S | Exterior | Frame - Wood | Main | 13 | 46 | 10 | 10 | | 468.3 ft² | 0.625 | 0.23 | 0.75 | 0 |
| | 14 | W | Exterior | Frame - Wood | Main | 13 | 26 | 2 | 9 | | 235.5 ft² | 0.625 | 0.23 | 0.75 | 0 |
| | 15 | W | Garage | Frame - Wood | Main | 13 | 47 | 10 | 9 | | 430.5 ft² | 0.625 | 0.23 | 0.75 | 0 |

## DOORS

| ✓ | # | Ornt | Door Type | Space | Storms | U-Value | Width Ft | Width In | Height Ft | Height In | Area |
|---|---|---|---|---|---|---|---|---|---|---|---|
| | 1 | E | Insulated | Main | None | .4 | 3 | | 8 | | 24 ft² |
| | 2 | N | Insulated | Main | None | .4 | 6 | | 8 | | 48 ft² |
| | 3 | N | Insulated | Main | None | .4 | 6 | | 8 | | 48 ft² |
| | 4 | S | Insulated | Main | None | .4 | 6 | | 8 | | 48 ft² |
| | 5 | W | Insulated | Main | None | .4 | 3 | | 6 | 8 | 20 ft² |
| | 6 | W | Insulated | Main | None | .4 | 6 | | 6 | 8 | 40 ft² |

## WINDOWS

Orientation shown is the entered, Proposed orientation.

| ✓ | # | Ornt | Wall ID | Frame | Panes | NFRC | U-Factor | SHGC | Imp | Area | Overhang Depth | Overhang Separation | Int Shade | Screening |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | 1 | N | 1 | Vinyl | Low-E Double | Yes | 0.33 | 0.22 | N | 36.0 ft² | 1 ft 6 in | 1 ft 4 in | None | None |
| | 2 | N | 3 | Vinyl | Low-E Double | Yes | 0.33 | 0.22 | N | 24.0 ft² | 11 ft 6 in | 1 ft 4 in | None | None |
| | 3 | N | 5 | Vinyl | Low-E Double | Yes | 0.33 | 0.22 | N | 8.0 ft² | 1 ft 6 in | 1 ft 4 in | None | None |
| | 4 | E | 8 | Vinyl | Low-E Double | Yes | 0.33 | 0.22 | N | 30.0 ft² | 1 ft 6 in | 1 ft 4 in | None | None |
| | 5 | S | 11 | Vinyl | Low-E Double | Yes | 0.33 | 0.22 | N | 36.0 ft² | 1 ft 6 in | 1 ft 4 in | None | None |
| | 6 | S | 13 | Vinyl | Low-E Double | Yes | 0.33 | 0.22 | N | 72.0 ft² | 11 ft 6 in | 1 ft 4 in | None | None |
| | 7 | W | 14 | Vinyl | Low-E Double | Yes | 0.33 | 0.22 | N | 16.0 ft² | 1 ft 6 in | 1 ft 4 in | None | None |

FORM R405-2017

# INPUT SUMMARY CHECKLIST REPORT

## GARAGE

| √ | # | Floor Area | Ceiling Area | Exposed Wall Perimeter | Avg. Wall Height | Exposed Wall Insulation |
|---|---|---|---|---|---|---|
| _____ | 1 | 789.344 ft² | 789.344 ft² | 69.5 ft | 9 ft | 1 |

## INFILTRATION

| # | Scope | Method | SLA | CFM 50 | ELA | EqLA | ACH | ACH 50 |
|---|---|---|---|---|---|---|---|---|
| 1 | Wholehouse | Proposed ACH(50) | .000254 | 1524 | 83.67 | 157.35 | .0956 | 5 |

## HEATING SYSTEM

| √ | # | System Type | Subtype | Speed | Efficiency | Capacity | Block | Ducts |
|---|---|---|---|---|---|---|---|---|
| _____ | 1 | Electric Heat Pump/ | None | Singl | HSPF:8.5 | 36 kBtu/hr | 1 | sys#1 |

## COOLING SYSTEM

| √ | # | System Type | Subtype | Subtype | Efficiency | Capacity | Air Flow | SHR | Block | Ducts |
|---|---|---|---|---|---|---|---|---|---|---|
| _____ | 1 | Central Unit/ | None | Singl | SEER: 14 | 36 kBtu/hr | 1080 cfm | 0.85 | 1 | sys#1 |

## HOT WATER SYSTEM

| √ | # | System Type | SubType | Location | EF | Cap | Use | SetPnt | Conservation |
|---|---|---|---|---|---|---|---|---|---|
| _____ | 1 | Electric | None | Main | 0.92 | 40 gal | 60 gal | 120 deg | None |

## SOLAR HOT WATER SYSTEM

| √ | FSEC Cert # | Company Name | System Model # | Collector Model # | Collector Area | Storage Volume | FEF |
|---|---|---|---|---|---|---|---|
| _____ | None | None | | | ft² | | |

## DUCTS

| √ | # | ---- Supply ---- Location | R-Value | Area | ---- Return ---- Location | Area | Leakage Type | Air Handler | CFM 25 TOT | CFM25 OUT | QN | RLF | HVAC # Heat | HVAC # Cool |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| _____ | 1 | Attic | 6 | 457.2 ft | Attic | 114.3 ft | Prop. Leak Free | Main | --- cfm | 68.6 cfm | 0.03 | 0.50 | 1 | 1 |

# INPUT SUMMARY CHECKLIST REPORT

## TEMPERATURES

Programable Thermostat: Y

Ceiling Fans:

| | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| Cooling | [ ] | [ ] | [ ] | [ ] | [ ] | [X] | [X] | [X] | [X] | [ ] | [ ] | [ ] |
| Heating | [X] | [X] | [X] | [ ] | [ ] | [ ] | [ ] | [ ] | [ ] | [ ] | [X] | [X] |
| Venting | [ ] | [ ] | [X] | [X] | [ ] | [ ] | [ ] | [ ] | [ ] | [X] | [X] | [ ] |

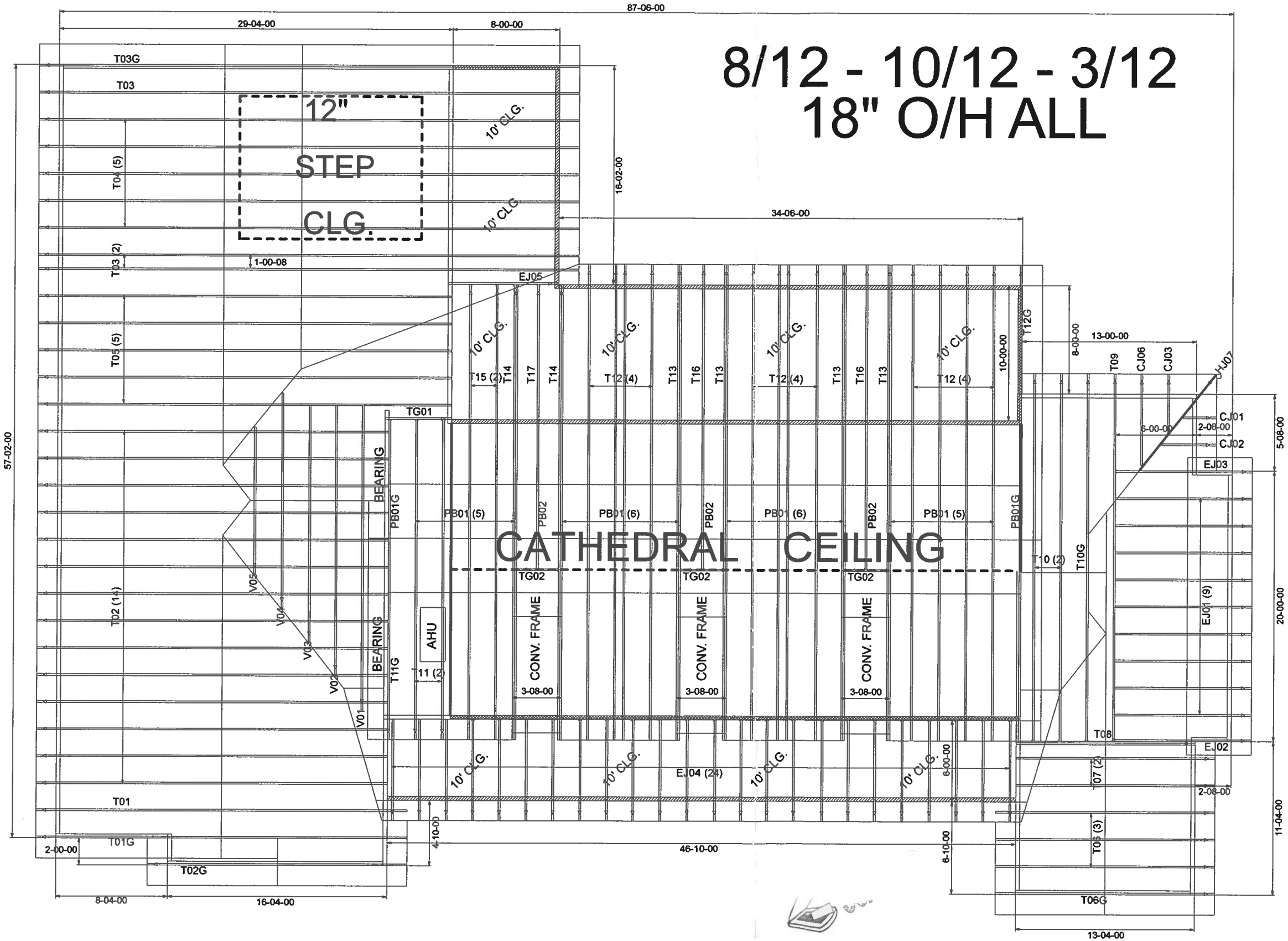
Thermostat Schedule: HERS 2006 Reference

| Schedule Type | | Hours 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| Cooling (WD) | AM | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 80 | 80 | 80 | 80 |
| | PM | 80 | 80 | 80 | 80 | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 78 |
| Cooling (WEH) | AM | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 80 | 80 | 80 | 80 |
| | PM | 80 | 80 | 80 | 80 | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 78 |
| Heating (WD) | AM | 65 | 65 | 65 | 65 | 65 | 65 | 65 | 68 | 68 | 68 | 68 | 68 |
| | PM | 68 | 68 | 68 | 68 | 68 | 68 | 68 | 68 | 68 | 68 | 68 | 68 |
| Heating (WEH) | AM | 65 | 65 | 65 | 65 | 65 | 65 | 65 | 68 | 68 | 68 | 68 | 68 |
| | PM | 68 | 68 | 68 | 68 | 68 | 68 | 68 | 68 | 68 | 68 | 68 | 68 |

## MASS

| Mass Type | Area | Thickness | Furniture Fraction | Space |
|---|---|---|---|---|
| Default(8 lbs/sq.ft. | 0 ft² | 0 ft | 0.3 | Main |

# 8/12 - 10/12 - 3/12
# 18" O/H ALL




## BEARING HEIGHT SCHEDULE

- 9' 1-1/8"
- 10' 1-1/8"
- 11' 1-1/8"

## NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.
3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.


Builders FirstSource

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1473

Tampa
PHONE: 813-621-9831 FAX: 813-628-8956

Lake City
PHONE: 386-755-6894 FAX: 386-755-7473


BUILDER: AARON SIMQUE HOMES
LEGAL ADDRESS: KARTLON RES.
MODEL: | Revision: | Rev. By:
DATE: 1-29-2020 | DRAWN BY: KLH | Original Reference #: 2233370
Additional Job #: | 2nd Additional Job #: | Real Job #: 2233370

**FL Approval Codes - Mitek Plates #'s 2197.2 - 2197.4, Versa-Lam #1644-R4 & BCI Joists #1392-R4**

Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: 2233370 - AARON SIMQUE - KARLTON RES.

**MiTek USA, Inc.**
6904 Parke East Blvd.
Tampa, FL 33610-4115

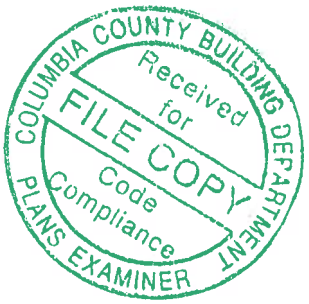
**Site Information:**
Customer Info: Aaron Simque Homes Project Name: Karlton Res. Model: Custom
Lot/Block: 29 Subdivision: High Pointe Farms
Address: TBD SW Ansley Place, N/A
City: Columbia Cty State: FL

**Name Address and License # of Structural Engineer of Record, If there is one, for the building.**
Name: License #:
Address:
City: State:

**General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):**
Design Code: FBC2017/TPI2014 Design Program: MiTek 20/20 8.2
Wind Code: ASCE 7-10 Wind Speed: 130 mph
Roof Load: 37.0 psf Floor Load: N/A psf

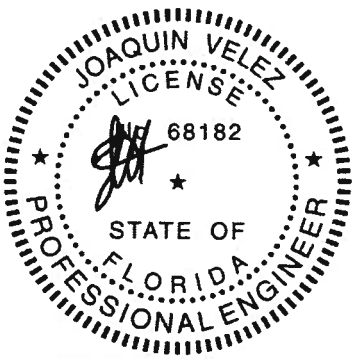
This package includes 44 individual, Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

| No. | Seal# | Truss Name | Date | No. | Seal# | Truss Name | Date |
|---|---|---|---|---|---|---|---|
| 1 | T19289605 | CJ01 | 1/31/20 | 23 | T19289627 | T06G | 1/31/20 |
| 2 | T19289606 | CJ02 | 1/31/20 | 24 | T19289628 | T07 | 1/31/20 |
| 3 | T19289607 | CJ03 | 1/31/20 | 25 | T19289629 | T08 | 1/31/20 |
| 4 | T19289608 | CJ06 | 1/31/20 | 26 | T19289630 | T09 | 1/31/20 |
| 5 | T19289609 | EJ01 | 1/31/20 | 27 | T19289631 | T10 | 1/31/20 |
| 6 | T19289610 | EJ02 | 1/31/20 | 28 | T19289632 | T10G | 1/31/20 |
| 7 | T19289611 | EJ03 | 1/31/20 | 29 | T19289633 | T11 | 1/31/20 |
| 8 | T19289612 | EJ04 | 1/31/20 | 30 | T19289634 | T11G | 1/31/20 |
| 9 | T19289613 | EJ05 | 1/31/20 | 31 | T19289635 | T12 | 1/31/20 |
| 10 | T19289614 | HJ07 | 1/31/20 | 32 | T19289636 | T12G | 1/31/20 |
| 11 | T19289615 | PB01 | 1/31/20 | 33 | T19289637 | T13 | 1/31/20 |
| 12 | T19289616 | PB01G | 1/31/20 | 34 | T19289638 | T14 | 1/31/20 |
| 13 | T19289617 | PB02 | 1/31/20 | 35 | T19289639 | T15 | 1/31/20 |
| 14 | T19289618 | T01 | 1/31/20 | 36 | T19289640 | T16 | 1/31/20 |
| 15 | T19289619 | T01G | 1/31/20 | 37 | T19289641 | T17 | 1/31/20 |
| 16 | T19289620 | T02 | 1/31/20 | 38 | T19289642 | TG01 | 1/31/20 |
| 17 | T19289621 | T02G | 1/31/20 | 39 | T19289643 | TG02 | 1/31/20 |
| 18 | T19289622 | T03 | 1/31/20 | 40 | T19289644 | V01 | 1/31/20 |
| 19 | T19289623 | T03G | 1/31/20 | 41 | T19289645 | V02 | 1/31/20 |
| 20 | T19289624 | T04 | 1/31/20 | 42 | T19289646 | V03 | 1/31/20 |
| 21 | T19289625 | T05 | 1/31/20 | 43 | T19289647 | V04 | 1/31/20 |
| 22 | T19289626 | T06 | 1/31/20 | 44 | T19289648 | V05 | 1/31/20 |




The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Velez, Joaquin
My license renewal date for the state of Florida is February 28, 2021.




**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

SCANNED

Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

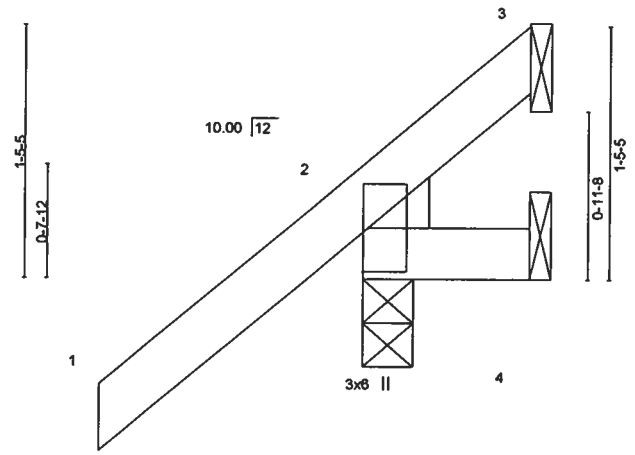
January 31,2020

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | CJ01 | Jack-Open | 1 | 1 | T19289605 |
| | | | | | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:01 2020 Page 1
ID:Tz9oYMPearyZ7mQ7t5lg8hzmID-Tw4FjXmR4OA3zxKSnuqGcxPqU3a9itwMNO2ju?zq6ua

-1-6-0 1-6-0 | 0-11-7 0-11-7

Scale = 1:12.7




0-11-7 0-11-7

Plate Offsets (X,Y)-- [2:0-0-2,0-0-3], [2:0-0-5,0-3-2]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.22 | Vert(LL) | 0.00 | 7 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.04 | Vert(CT) | 0.00 | 7 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.00 | Horz(CT) | 0.00 | 3 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MP | | | | | | | Weight: 7 lb | FT = 20% |

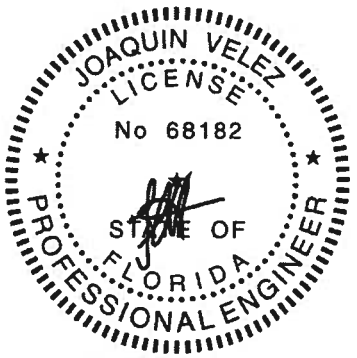
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEDGE
Left: 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 0-11-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 3=-8/Mechanical, 2=180/0-3-8, 4=-22/Mechanical
Max Horz 2=90(LC 12)
Max Uplift 3=-8(LC 1), 2=-95(LC 12), 4=-22(LC 1)
Max Grav 3=10(LC 8), 2=180(LC 1), 4=25(LC 16)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.

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Date:
January 31,2020



MiTek
6904 Parke East Blvd.
Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | CJ02 | Jack-Open | 1 | 1 | T19289606 |
| | | | | | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:02 2020 Page 1
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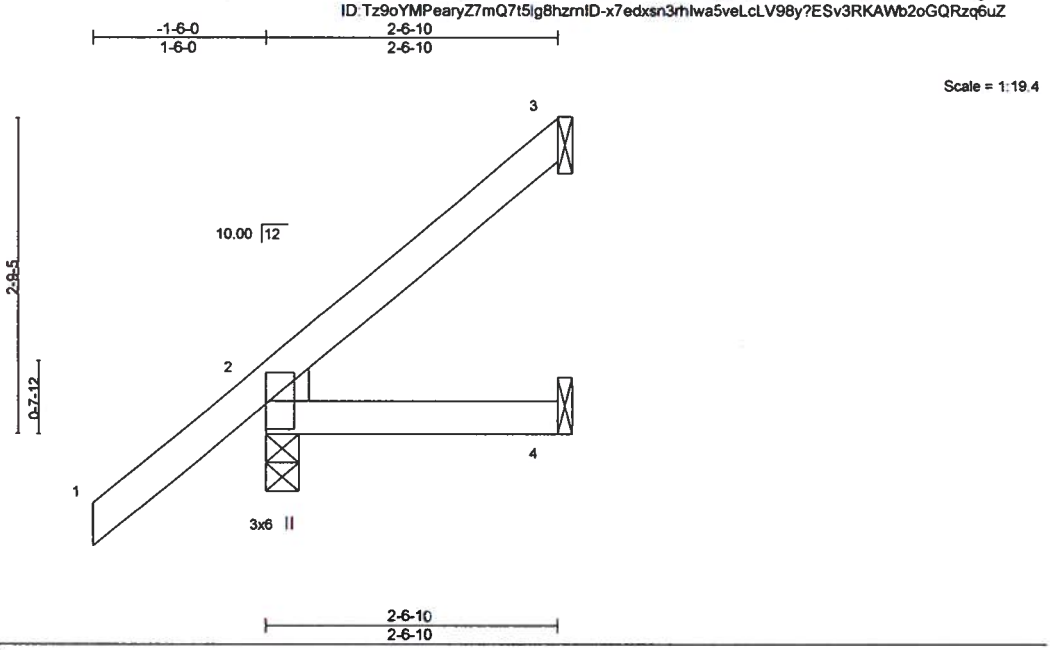



Plate Offsets (X,Y)— [2:0-0-2,0-0-3], [2:0-0-5,0-3-2]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.22 | Vert(LL) | -0.01 | 4-7 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.13 | Vert(CT) | -0.01 | 4-7 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.00 | Horz(CT) | 0.00 | 3 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MP | | | | | | | Weight: 13 lb | FT = 20% |

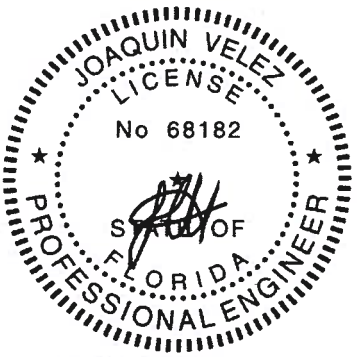
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEDGE
Left: 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 2-6-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 3=47/Mechanical, 2=198/0-3-8, 4=20/Mechanical
Max Horz 2=153(LC 12)
Max Uplift 3=-69(LC 12), 2=-59(LC 12), 4=-7(LC 12)
Max Grav 3=61(LC 19), 2=198(LC 1), 4=42(LC 3)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.



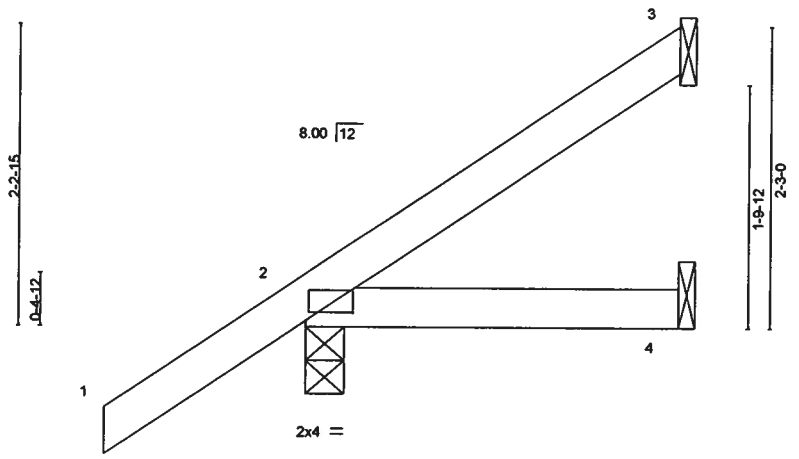

Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | CJ03 | Jack-Open | 1 | 1 | T19289607 |
| | | | | | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:03 2020 Page 1
ID:Tz9oYMPearyZ7mQ7t5Ig8hzmID-PJC?8Cnic?QnCFTquJskhMVBasFYAnQfqiXqyuzq6uY

-1-6-0 / 1-6-0 — 2-9-5 / 2-9-5

Scale = 1:16.5




2-9-5 / 2-9-5

Plate Offsets (X,Y)– [2:0-4-4,0-0-11]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.18 | Vert(LL) | -0.01 | 4-7 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.11 | Vert(CT) | -0.01 | 4-7 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.00 | Horz(CT) | 0.00 | 3 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MP | | | | | | | Weight: 12 lb | FT = 20% |

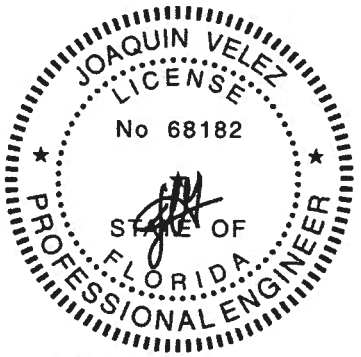
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 2-9-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 3=54/Mechanical, 2=204/0-3-8, 4=24/Mechanical
Max Horz 2=130(LC 12)
Max Uplift 3=-59(LC 12), 2=-85(LC 12)
Max Grav 3=64(LC 19), 2=204(LC 1), 4=46(LC 3)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



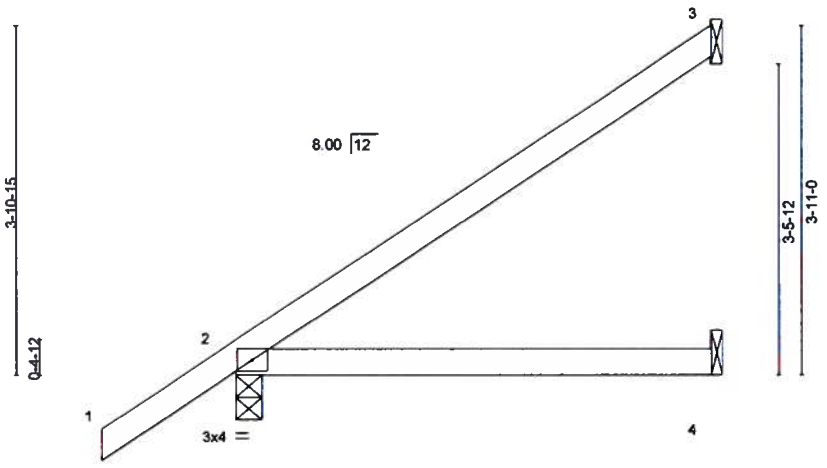

Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. T19289608 |
|---|---|---|---|---|---|
| 2233370 | CJ06 | Jack-Open | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:04 2020 Page 1
ID:Tz9oYMPearyZ7mQ7t5lg8hzmID-uVINMYoKNJYdqP21S0OzEZ1IxGWRvEgo3MHNVKzq6uX

-1-6-0 / 1-6-0 | 5-3-5 / 5-3-5

Scale = 1:24.7




5-3-5 / 5-3-5

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.40 | Vert(LL) | 0.05 | 4-7 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.32 | Vert(CT) | -0.08 | 4-7 | >739 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.00 | Horz(CT) | 0.01 | 3 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MP | | | | | | | Weight: 20 lb | FT = 20% |

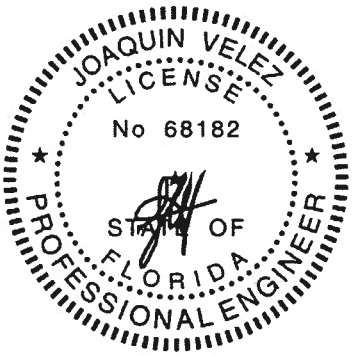
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 5-3-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 3=120/Mechanical, 2=286/0-3-8, 4=61/Mechanical
Max Horz 2=211(LC 12)
Max Uplift 3=-129(LC 12), 2=-91(LC 12), 4=-8(LC 12)
Max Grav 3=140(LC 19), 2=286(LC 1), 4=94(LC 3)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4 except (jt=lb) 3=129.




Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.**
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ***ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information*** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek
6904 Parke East Blvd.
Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | EJ01 | Jack-Partial | 9 | 1 | T19289609 |
| | | | | | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:04 2020 Page 1
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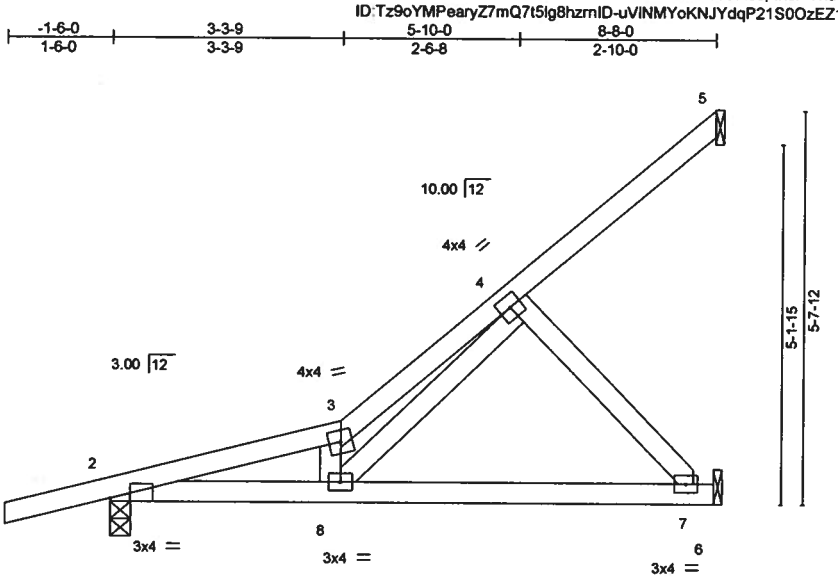


Plate Offsets (X,Y)-- [2:0-3-6,Edge]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.14 | Vert(LL) | -0.05 | 7-8 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.34 | Vert(CT) | -0.11 | 7-8 | >947 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.34 | Horz(CT) | -0.04 | 5 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 43 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 5-8-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 5=59/Mechanical, 2=406/0-3-8, 6=252/Mechanical
Max Horz 2=269(LC 12)
Max Uplift 5=-86(LC 12), 2=-158(LC 8), 6=-135(LC 12)
Max Grav 5=74(LC 19), 2=406(LC 1), 6=252(LC 1)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-750/79, 3-4=-1042/222
BOT CHORD 2-8=-316/708
WEBS 3-8=-553/221, 4-8=-282/895, 4-7=-337/238

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 2=158, 6=135.


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Date:
January 31,2020



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Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. T19289610 |
|---|---|---|---|---|---|
| 2233370 | EJ02 | Jack-Partial Structural Gable | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:05 2020 Page 1
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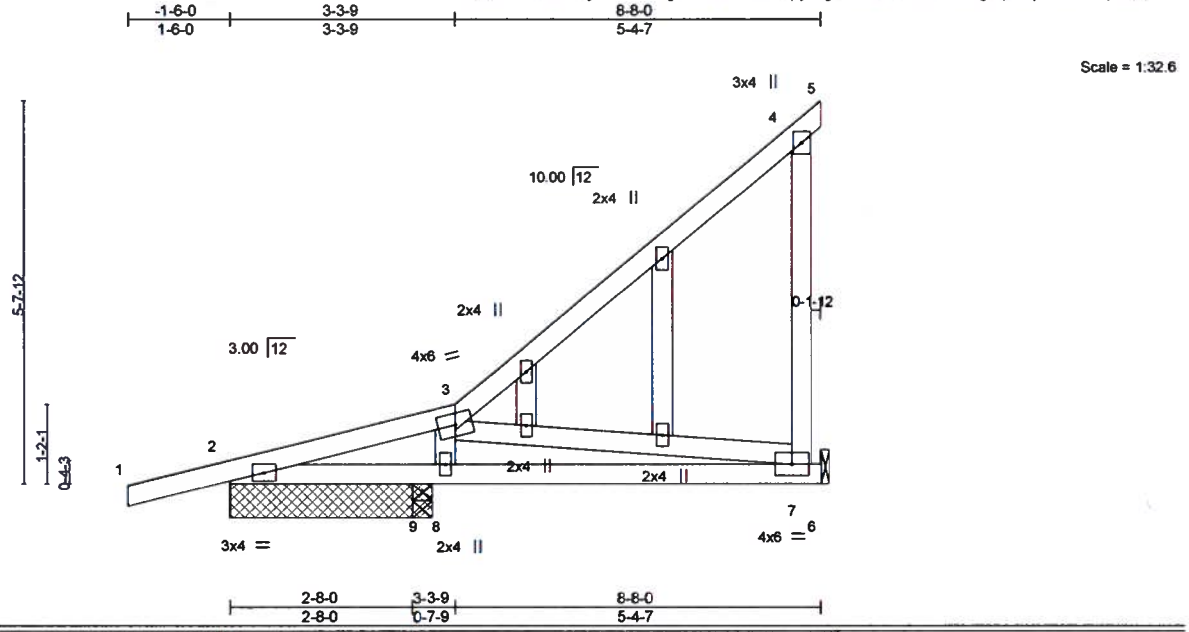



| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.36 | Vert(LL) | -0.02 | 7-8 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.25 | Vert(CT) | -0.05 | 7-8 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.05 | Horz(CT) | 0.00 | 7 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 52 lb | FT = 20% |

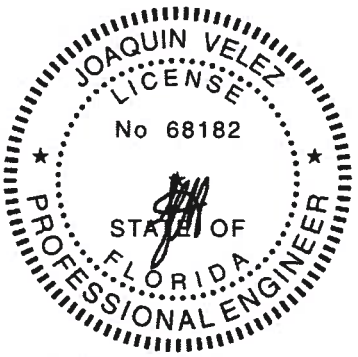
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 7=204/Mechanical, 2=182/2-11-8, 9=325/0-3-8
Max Horz 2=270(LC 12)
Max Uplift 7=-190(LC 12), 2=-162(LC 8), 9=-113(LC 12)
Max Grav 7=237(LC 19), 2=182(LC 1), 9=325(LC 1)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) Gable studs spaced at 2-0-0 oc.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=190, 2=162, 9=113.




Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.**
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see *ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information* available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. | |
|---|---|---|---|---|---|---|
| 2233370 | EJ03 | Half Hip Girder | 1 | 1 | Job Reference (optional) | T19289611 |

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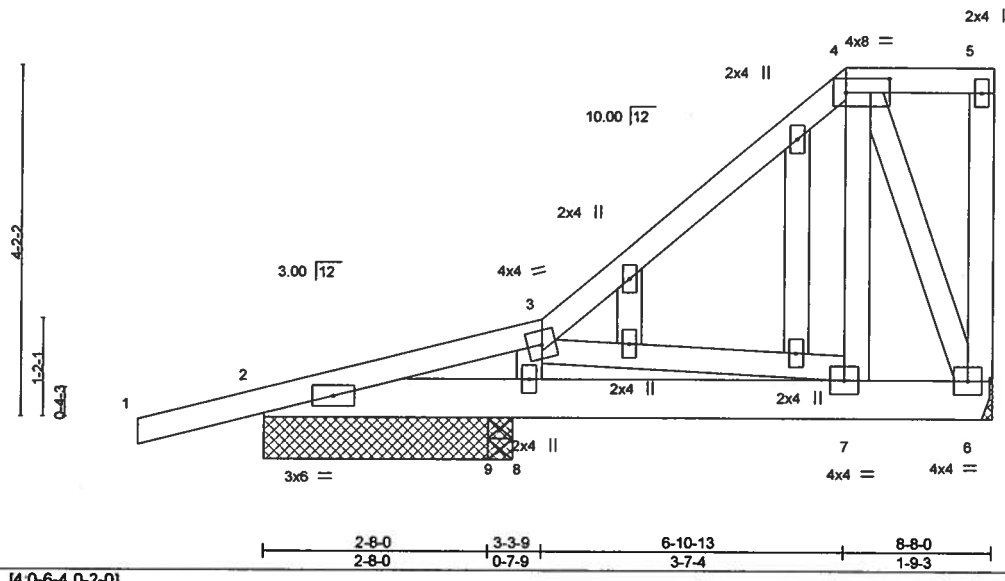



Plate Offsets (X,Y)— [4:0-6-4,0-2-0]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.18 | Vert(LL) | 0.00 | 7 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.10 | Vert(CT) | -0.01 | 7-8 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.12 | Horz(CT) | -0.00 | 6 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 65 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

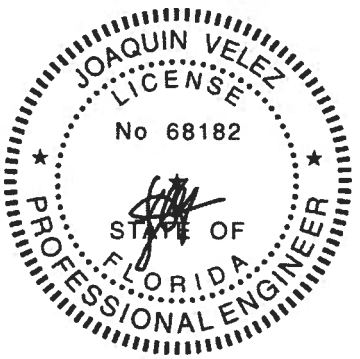
**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 2=173/2-11-8, 6=363/Mechanical, 9=406/0-3-8
Max Horz 2=202(LC 8)
Max Uplift 2=-167(LC 23), 6=-288(LC 8), 9=-203(LC 8)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 3-8=-268/196, 4-6=-376/307

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Provide adequate drainage to prevent water ponding.
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
9) Refer to girder(s) for truss to truss connections.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=167, 6=288, 9=203.
11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 152 lb down and 183 lb up at 6-10-13 on top chord, and 184 lb down and 129 lb up at 6-10-13 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-54, 4-5=-54, 2-6=-20
Concentrated Loads (lb)
Vert: 7=-144(F) 4=-86(F)

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Date:
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| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | EJ04 | Monopitch | 24 | 1 | T19289612 |

Job Reference (optional)

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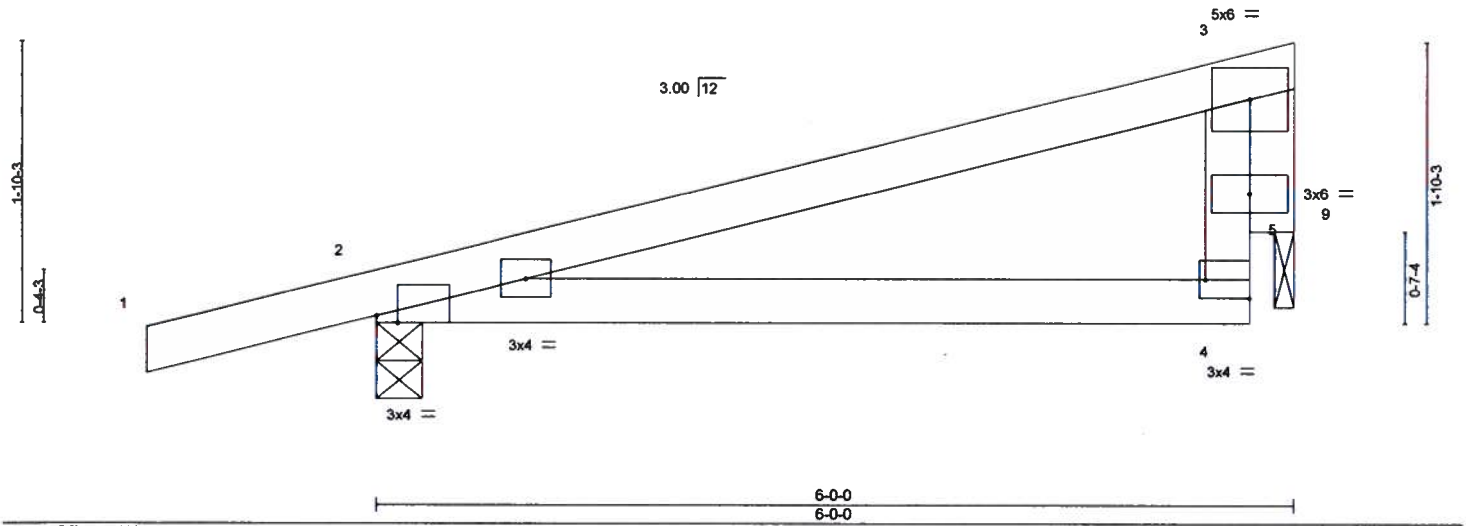
-1-6-0 1-6-0 | 6-0-0 6-0-0

Scale = 1:14.5




6-0-0
6-0-0

Plate Offsets (X,Y)-- [2:0-1-10,Edge], [4:Edge,0-1-8]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.49 | Vert(LL) | 0.06 | 4-8 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.29 | Vert(CT) | 0.05 | 4-8 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.34 | Horz(CT) | -0.00 | 2 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MR | | | | | | | Weight: 23 lb | FT = 20% |

**LUMBER-**

| | |
|---|---|
| TOP CHORD | 2x4 SP No.2 |
| BOT CHORD | 2x4 SP No.2 |
| WEBS | 2x4 SP No.3 |
| OTHERS | 2x4 SP No.3 |

**BRACING-**

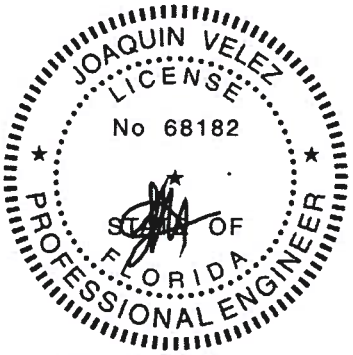
| | |
|---|---|
| TOP CHORD | Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. |
| BOT CHORD | Rigid ceiling directly applied or 9-1-14 oc bracing. |

**REACTIONS.** (lb/size) 2=310/0-3-8, 9=183/0-1-8
Max Horz 2=92(LC 8)
Max Uplift 2=-257(LC 8), 9=-147(LC 8)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-226/321
BOT CHORD 2-4=-363/203

**NOTES-**

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 9.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=257, 9=147.




Joaquin Velez PE No.68182
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Date:
January 31,2020

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. | T19289613 |
|---|---|---|---|---|---|---|
| 2233370 | EJ05 | Monopitch Girder | 1 | 1 | Job Reference (optional) | |

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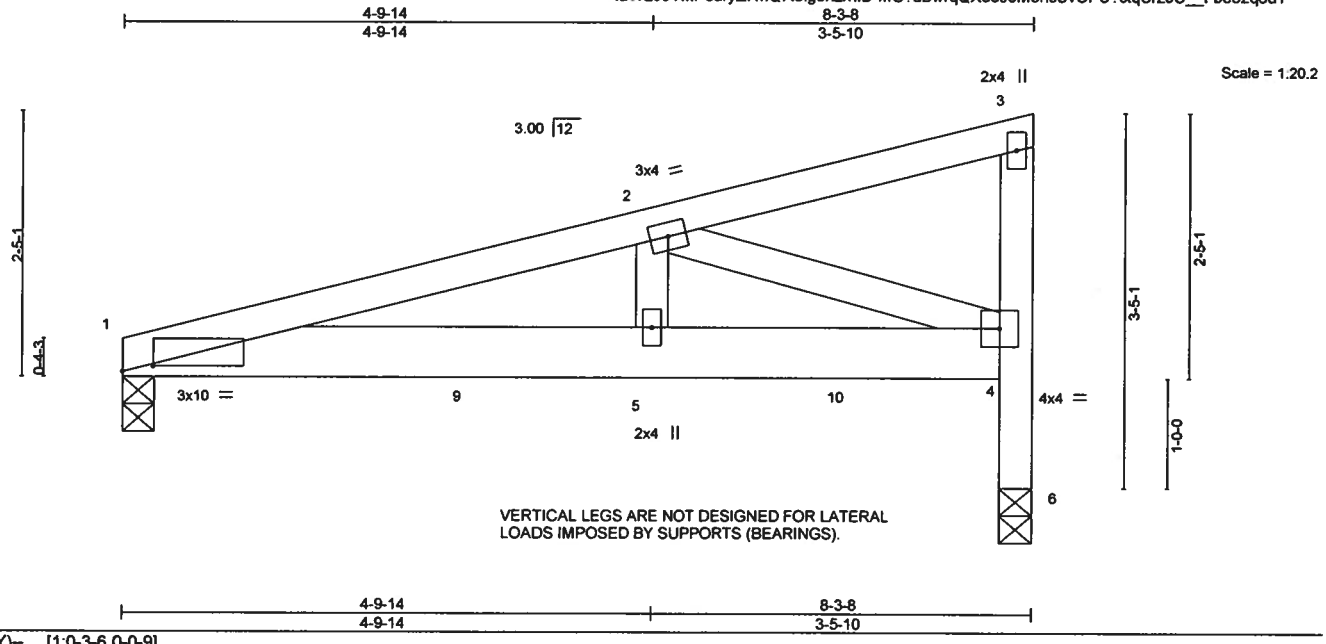


Plate Offsets (X,Y)-- [1:0-3-6,0-0-9]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.36 | Vert(LL) | 0.07 | 5-8 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.59 | Vert(CT) | 0.07 | 5-8 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.36 | Horz(CT) | -0.03 | 6 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 42 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 5-8-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS.** (lb/size) 1=570/0-3-8, 6=475/0-3-8
Max Horz 1=100(LC 4)
Max Uplift 1=-509(LC 4), 6=-555(LC 4)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-977/1146, 4-6=-475/555
BOT CHORD 1-5=-1177/942, 4-5=-1177/942
WEBS 2-5=-510/386, 2-4=-969/1215

**NOTES-**
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=509, 6=555.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 345 lb down and 124 lb up at 1-6-1, 196 lb down and 418 lb up at 3-2-8, and 126 lb down and 178 lb up at 4-7-4, and 126 lb down and 178 lb up at 6-7-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 1-4=-20
Concentrated Loads (lb)
Vert: 5=-106(B) 8=-345(B) 9=115 10=-106(B)


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Date:
January 31,2020



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6904 Parke East Blvd.
Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. T19289614 |
|---|---|---|---|---|---|
| 2233370 | HJ07 | Diagonal Hip Girder | 1 | 1 | Job Reference (optional) |

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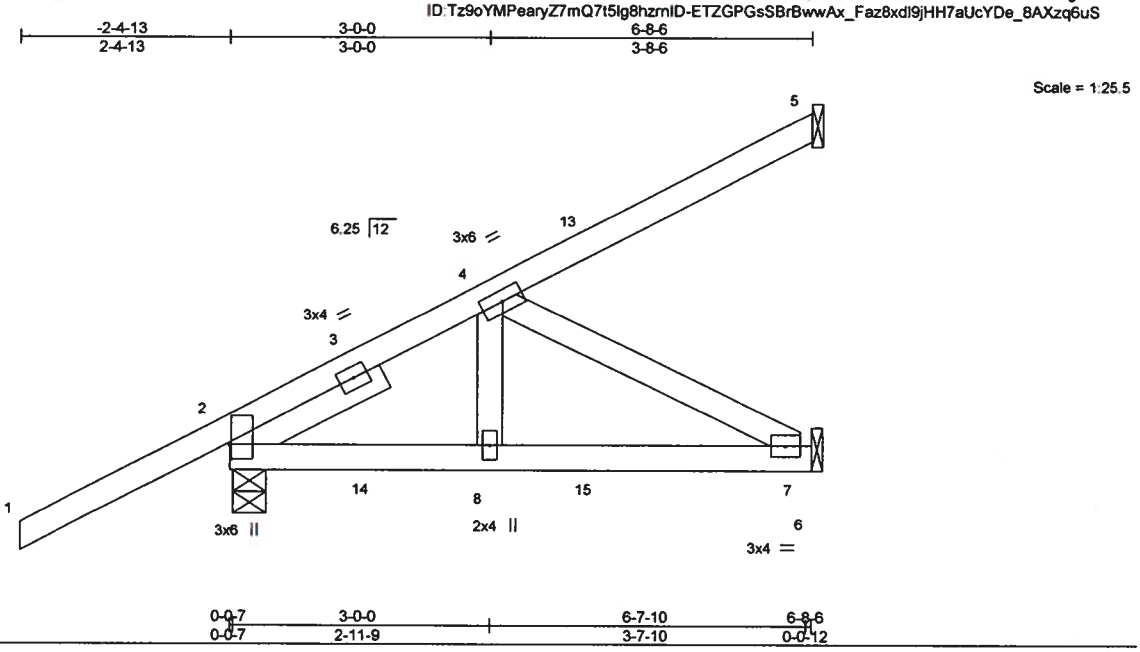



Plate Offsets (X,Y)— [2:0-2-0,0-0-4]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.40 | Vert(LL) | -0.01 | 7-8 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.16 | Vert(CT) | -0.02 | 7-8 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.08 | Horz(CT) | -0.00 | 5 | n/a | n/a | | |
| BCDL | 10.0 | Code | FBC2017/TPI2014 | Matrix-MP | | | | | | | Weight: 36 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 1-11-8

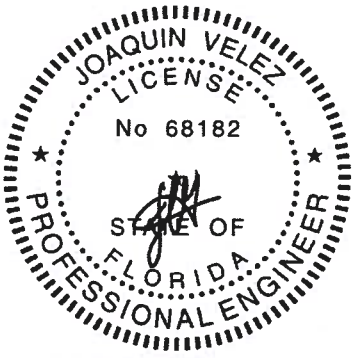
**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 5=155/Mechanical, 2=364/0-4-11, 7=169/Mechanical
Max Horz 2=220(LC 8)
Max Uplift 5=-176(LC 8), 2=-187(LC 8), 7=-103(LC 8)
Max Grav 5=163(LC 32), 2=364(LC 1), 7=203(LC 32)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-4=-282/174

**NOTES-**
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=176, 2=187, 7=103.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 72 lb down and 77 lb up at 1-7-4, 113 lb down and 75 lb up at 3-1-12, and 102 lb down and 83 lb up at 4-2-0, and 132 lb down and 147 lb up at 6-7-10 on top chord, and 17 lb down and 47 lb up at 1-7-4, 24 lb down at 3-1-12, and 27 lb down and 13 lb up at 4-2-0, and 57 lb down and 24 lb up at 6-4-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 6-9=-20
Concentrated Loads (lb)
Vert: 5=-69(F) 8=-1(F) 7=-42(F) 3=19(B) 14=26(B) 15=0(B)




Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
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Date:
January 31,2020

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | PB01 | Piggyback | 22 | 1 | T19289615 |
| | | | | | Job Reference (optional) |

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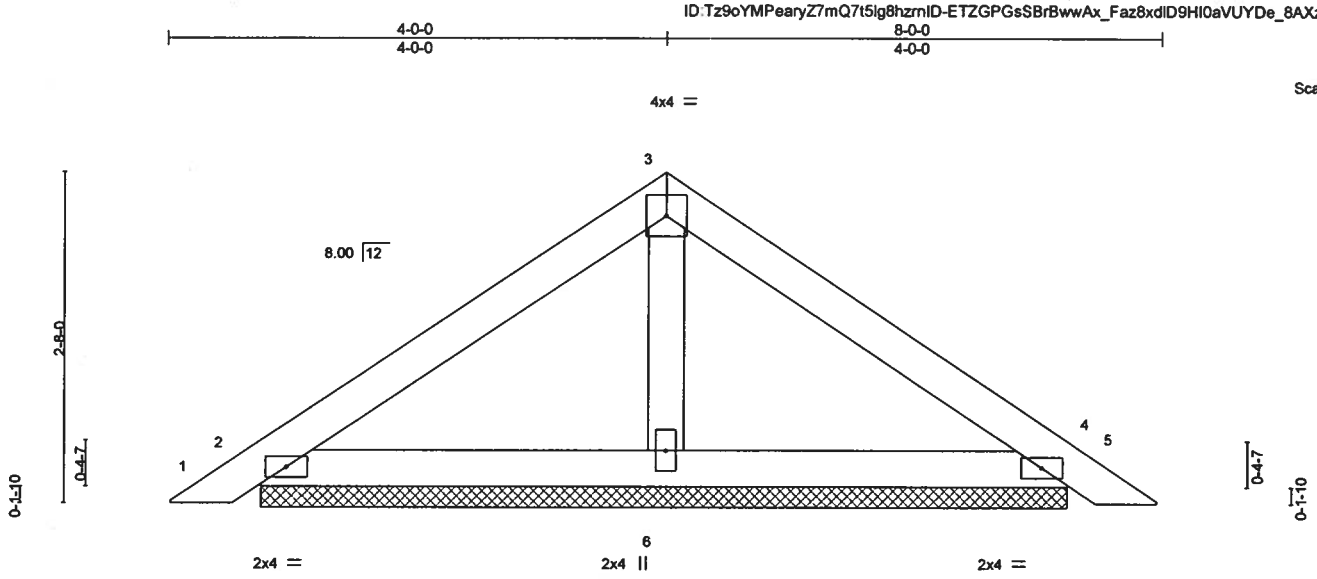



| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.18 | Vert(LL) | 0.01 | 5 | n/r | 120 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.11 | Vert(CT) | 0.01 | 5 | n/r | 120 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.03 | Horz(CT) | 0.00 | 4 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-P | | | | | | | Weight: 27 lb | FT = 20% |

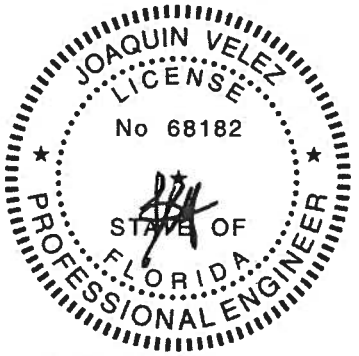
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 2=156/6-5-12, 4=156/6-5-12, 6=219/6-5-12
Max Horz 2=-77(LC 10)
Max Uplift 2=-82(LC 12), 4=-92(LC 13), 6=-42(LC 12)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.




Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | PB01G | GABLE | 2 | 1 | T19289616 |
| | | | | | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:10 2020 Page 1
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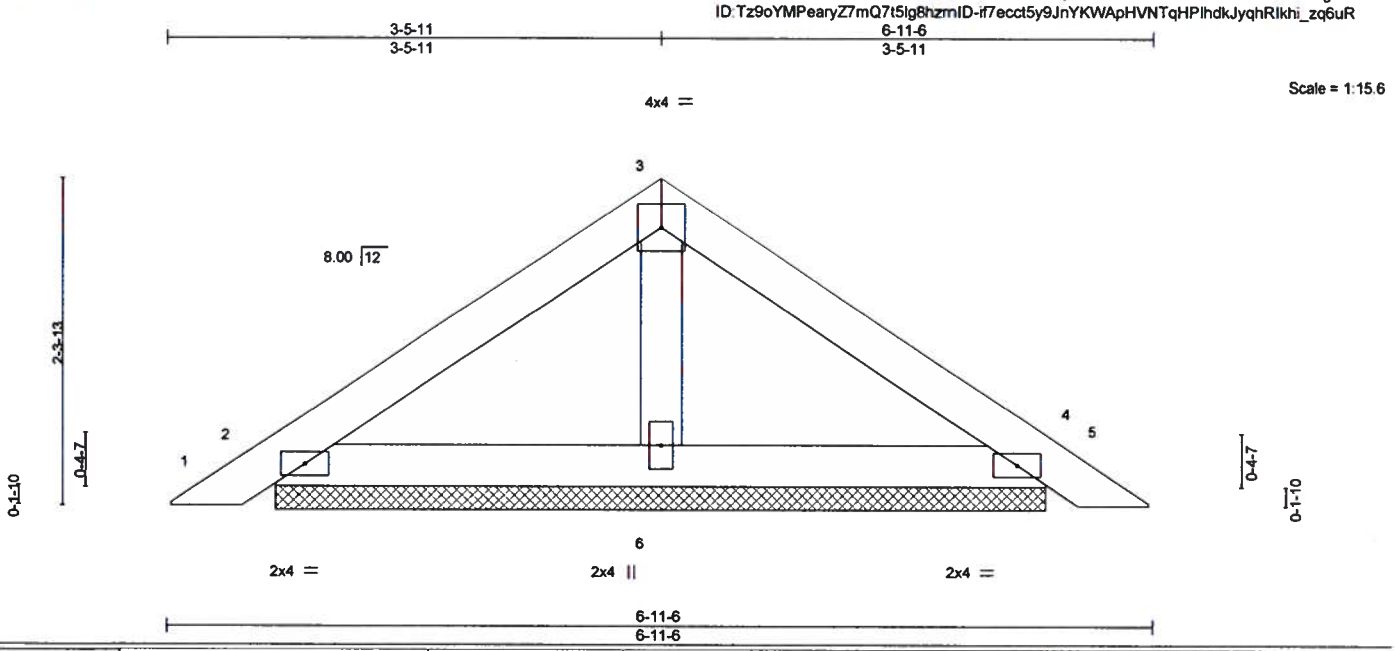


| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.12 | Vert(LL) | 0.00 | 5 | n/r | 120 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.07 | Vert(CT) | 0.00 | 5 | n/r | 120 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.02 | Horz(CT) | 0.00 | 4 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-P | | | | | | | Weight: 23 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 2=136/5-5-2, 4=136/5-5-2, 6=182/5-5-2
Max Horz 2=-66(LC 10)
Max Uplift 2=-72(LC 12), 4=-80(LC 13), 6=-34(LC 12)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.


Joaquin Velez PE No.68182
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Date:
January 31,2020

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.**
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ***ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information*** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | PB02 | Piggyback | 3 | 1 | T19289617 |
| | | | | | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:11 2020 Page 1
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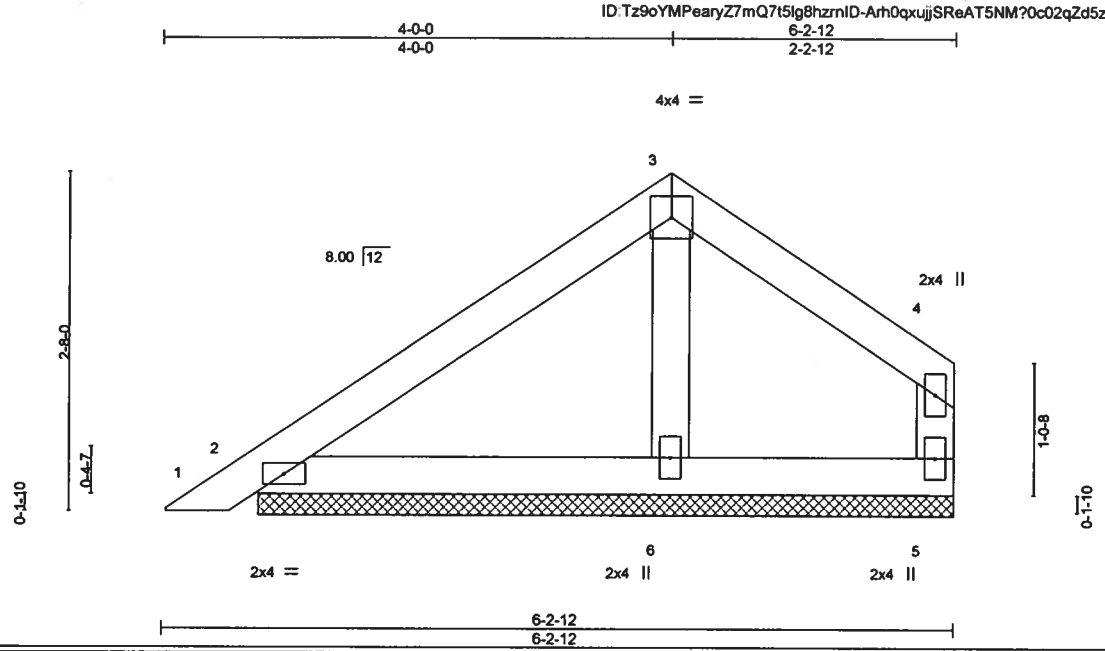



| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.18 | Vert(LL) | 0.00 | 1 | n/r | 120 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.08 | Vert(CT) | 0.00 | 1 | n/r | 120 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.03 | Horz(CT) | 0.00 | | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-P | | | | | | | Weight: 23 lb | FT = 20% |

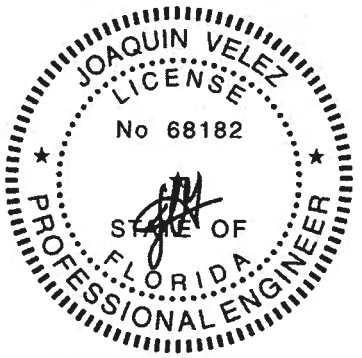
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 5=67/5-5-10, 2=142/5-5-10, 6=211/5-5-10
Max Horz 2=80(LC 12)
Max Uplift 5=-61(LC 13), 2=-65(LC 12), 6=-61(LC 12)
Max Grav 5=77(LC 20), 2=142(LC 1), 6=219(LC 19)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2, 6.
8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.




Joaquin Velez PE No.68182
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6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020



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6904 Parke East Blvd.
Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. T19289618 |
|---|---|---|---|---|---|
| 2233370 | T01 | Common | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:12 2020 Page 1
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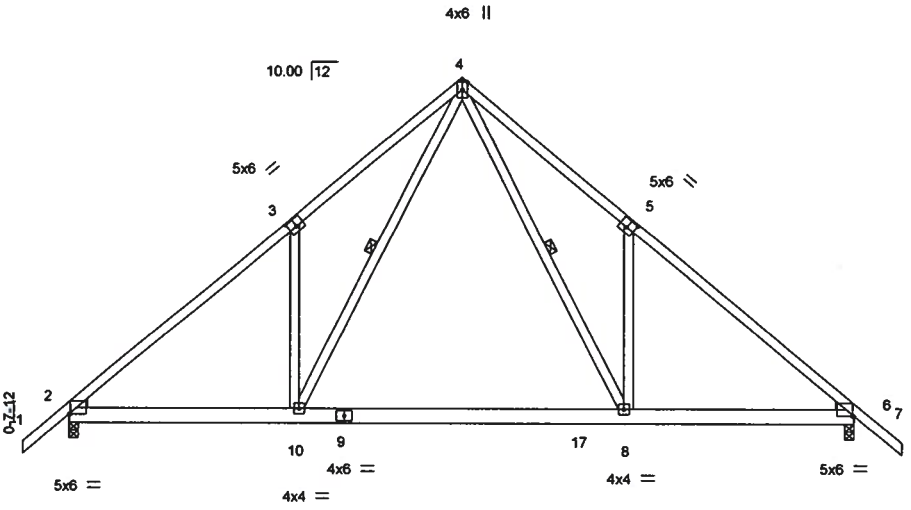



Plate Offsets (X,Y)— [2:0-0-8,0-0-2], [3:0-3-0,0-3-0], [5:0-3-0,0-3-0], [6:0-0-8,0-0-2]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.61 | Vert(LL) | -0.22 | 8-10 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.39 | Vert(CT) | -0.40 | 8-10 | >741 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.47 | Horz(CT) | 0.02 | 6 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 164 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP M 26
WEBS 2x4 SP No.3
WEDGE
Left: 2x4 SP No.3, Right: 2x4 SP No.3

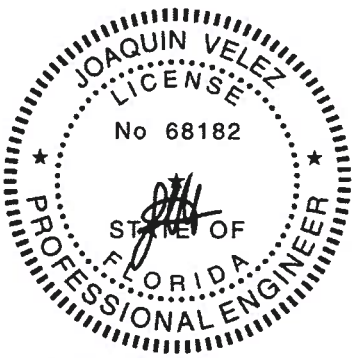
**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 3-10-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-8, 4-10

**REACTIONS.** (lb/size) 2=1309/0-3-8, 6=1309/0-3-8
Max Horz 2=-359(LC 10)
Max Uplift 2=-508(LC 12), 6=-508(LC 13)
Max Grav 2=1345(LC 19), 6=1345(LC 20)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1806/715, 3-4=-1881/1024, 4-5=-1881/1024, 5-6=-1806/715
BOT CHORD 2-10=-497/1506, 8-10=-183/942, 6-8=-361/1355
WEBS 4-8=-697/1258, 5-8=-492/477, 4-10=-697/1257, 3-10=-492/477

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=508, 6=508.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 10-11=-20, 8-10=-80(F=-60), 8-14=-20




Joaquin Velez PE No.68182
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Date:
January 31,2020

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Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. | T19289619 |
|---|---|---|---|---|---|---|
| 2233370 | T01G | GABLE | 1 | 1 | Job Reference (optional) | |

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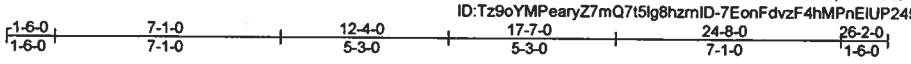


4x6 ||

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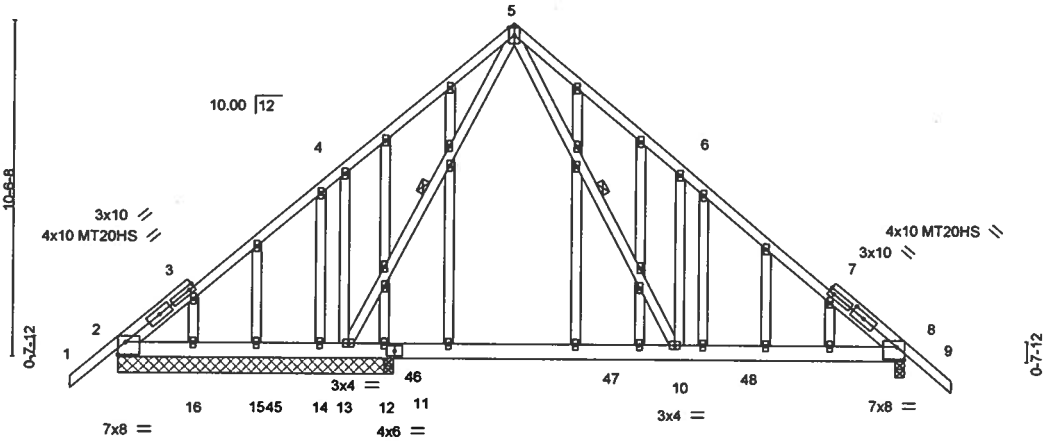


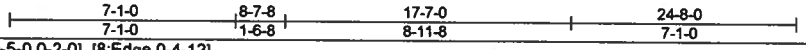




Plate Offsets (X,Y)-- [2:Edge,0-4-12], [7:0-5-0,0-2-0], [8:Edge,0-4-12]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.53 | Vert(LL) | -0.07 | 10-12 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.38 | Vert(CT) | -0.10 | 10-12 | >999 | 180 | MT20HS | 187/143 |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.39 | Horz(CT) | 0.02 | 8 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 240 lb | FT = 20% |

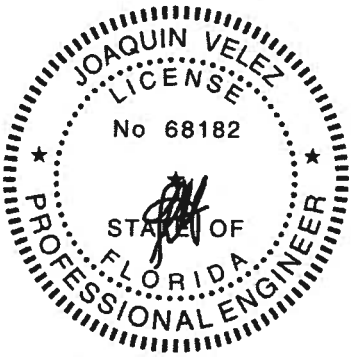
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 5-11-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-10, 5-13

**REACTIONS.** All bearings 8-7-8 except (jt=length) 8=0-3-8.
(lb) - Max Horz 2=-348(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 2, 14, 15 except 8=-251(LC 13), 13=-631(LC 12), 16=-274(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 2, 15, 2 except 8=731(LC 20), 13=605(LC 1), 12=678(LC 18), 12=262(LC 1), 14=330(LC 18), 16=303(LC 19)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-4=-335/339, 5-6=-849/519, 6-8=-727/212
BOT CHORD 2-16=-353/415, 15-16=-353/415, 14-15=-353/415, 13-14=-353/415, 12-13=-115/324, 10-12=-115/324, 8-10=0/512
WEBS 5-10=-527/879, 6-10=-509/468, 5-13=-614/277, 4-13=-470/443

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 15, 2 except (jt=lb) 8=251, 13=631, 16=274.

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MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020



MiTek
6904 Parke East Blvd.
Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. | T19289620 |
|---|---|---|---|---|---|---|
| 2233370 | T02 | Common | 14 | 1 | Job Reference (optional) | |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:14 2020 Page 1
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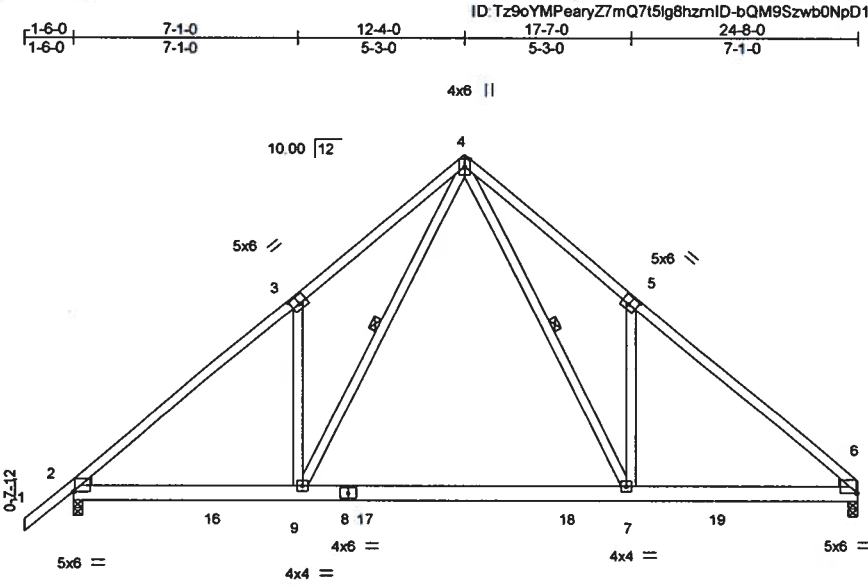



Plate Offsets (X,Y)-- [2:0-0-8,0-0-2], [3:0-3-0,0-3-0], [5:0-3-0,0-3-0], [6:0-0-8,0-0-2]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.61 | Vert(LL) | -0.20 | 7-9 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.37 | Vert(CT) | -0.39 | 7-9 | >768 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.48 | Horz(CT) | 0.02 | 6 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 161 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP M 26
WEBS 2x4 SP No.3
WEDGE
Left: 2x4 SP No.3, Right: 2x4 SP No.3

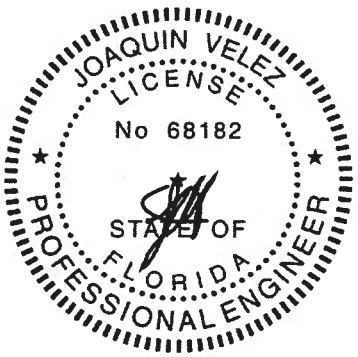
**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 3-10-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-7, 4-9

**REACTIONS.** (lb/size) 2=1311/0-3-8, 6=1225/0-3-8
Max Horz 2=346(LC 9)
Max Uplift 2=-509(LC 12), 6=-456(LC 13)
Max Grav 2=1411(LC 19), 6=1329(LC 20)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1884/720, 3-4=-1924/1029, 4-5=-1936/1037, 5-6=-1885/727
BOT CHORD 2-9=-524/1545, 7-9=-210/957, 6-7=-403/1397
WEBS 4-7=-710/1325, 5-7=-491/479, 4-9=-696/1310, 3-9=-492/477

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=509, 6=456.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 9-10=-20, 7-9=-80(F=-60), 7-13=-20




Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
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| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | T02G | Common Supported Gable | 1 | 1 | T19289621 |
| | | | | | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:16 2020 Page 1
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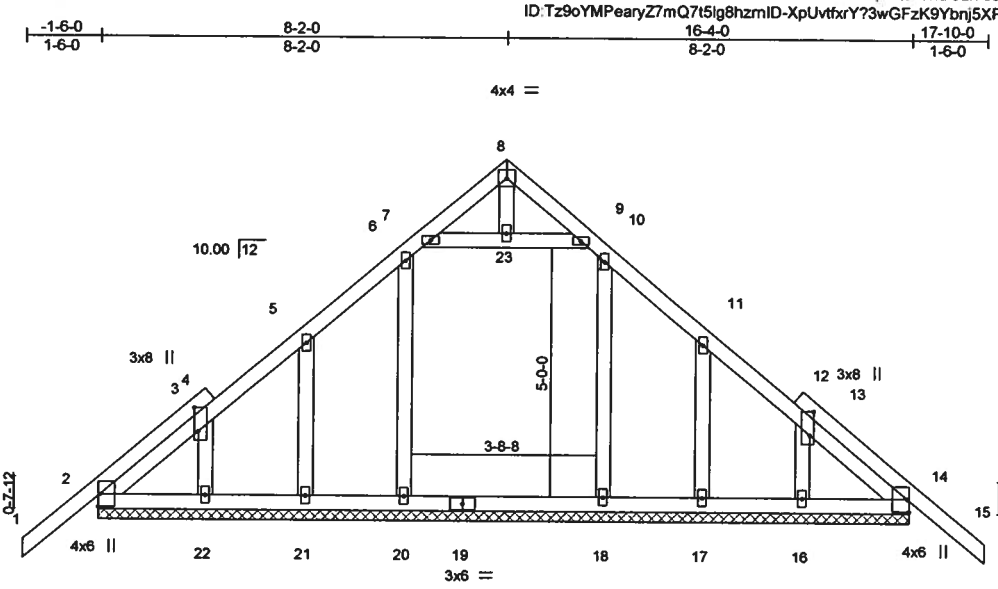


Plate Offsets (X,Y)-- [3:0-5-11,0-1-0], [13:0-5-11,0-1-0]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.16 | Vert(LL) | -0.01 | 15 | n/r | 120 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.14 | Vert(CT) | -0.01 | 15 | n/r | 120 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.09 | Horz(CT) | 0.00 | 14 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-S | | | | | | | Weight: 106 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** All bearings 16-4-0.
(lb) - Max Horz 2=-239(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 2, 14, 20, 18 except 21=-139(LC 12), 22=-126(LC 12), 17=-144(LC 13), 16=-124(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 2, 14, 21, 22, 17, 16 except 20=327(LC 19), 18=282(LC 20)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 20, 18 except (jt=lb) 21=139, 22=126, 17=144, 16=124.


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6904 Parke East Blvd. Tampa FL 33610
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Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. | T19289622 |
|---|---|---|---|---|---|---|
| 2233370 | T03 | Roof Special | 3 | 1 | Job Reference (optional) | |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:17 2020 Page 1
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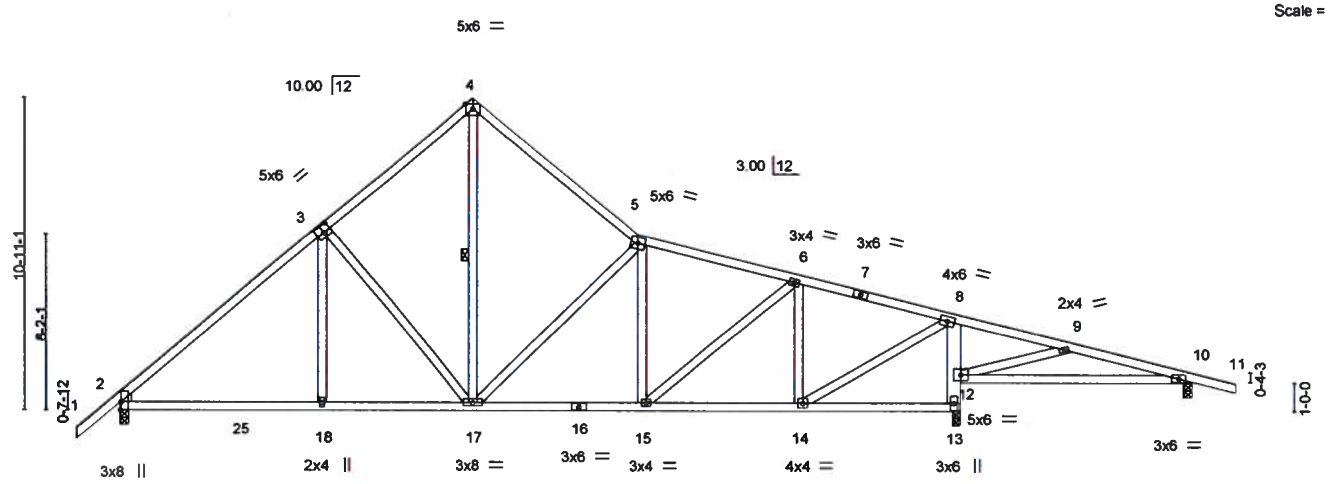
| 1-6-0 | 7-1-0 | 12-4-0 | 18-0-7 | 23-7-6 | 29-3-8 | 32-9-14 | 37-4-0 | 38-10-0 |
|---|---|---|---|---|---|---|---|---|
| 1-6-0 | 7-1-0 | 5-3-0 | 5-8-7 | 5-6-15 | 5-8-2 | 3-6-6 | 4-6-2 | 1-6-0 |

Scale = 1:77.3




| 7-1-0 | 12-4-0 | 18-0-7 | 23-7-6 | 29-3-8 | 37-4-0 |
|---|---|---|---|---|---|
| 7-1-0 | 5-3-0 | 5-8-7 | 5-6-15 | 5-8-2 | 8-0-8 |

Plate Offsets (X,Y)— [2:0-0-2,0-0-3], [2:0-0-5,0-3-2], [2:0-3-8,Edge], [3:0-3-0,0-3-0]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.53 | Vert(LL) | 0.14 | 12-24 | >708 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.41 | Vert(CT) | -0.17 | 12-24 | >589 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.87 | Horz(CT) | 0.03 | 13 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 220 lb | FT = 20% |

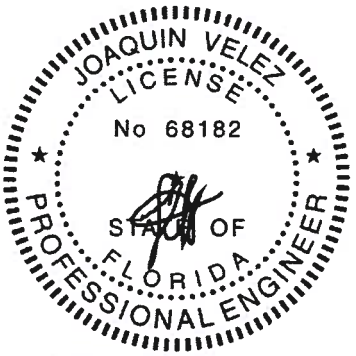
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
8-13: 2x6 SP No.2
WEBS 2x4 SP No.3
WEDGE
Left: 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 4-7-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-1 oc bracing.
WEBS 1 Row at midpt 4-17

**REACTIONS.** (lb/size) 2=1123/0-3-8, 13=1530/0-3-0, 10=271/0-3-8
Max Horz 2=-350(LC 10)
Max Uplift 2=-399(LC 12), 13=-605(LC 9), 10=-284(LC 9)
Max Grav 2=1123(LC 1), 13=1530(LC 1), 10=282(LC 24)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1311/589, 3-4=-1015/619, 4-5=-993/591, 5-6=-1234/659, 6-8=-981/509, 8-9=-235/447, 9-10=-147/271
BOT CHORD 2-18=-394/1076, 17-18=-394/1077, 15-17=-386/1162, 14-15=-310/917, 13-14=-271/245, 12-13=-1481/986, 8-12=-1264/693
WEBS 3-17=-523/380, 4-17=-563/975, 5-17=-679/483, 6-15=-118/348, 6-14=-565/351, 8-14=-640/1368, 9-12=-529/579

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=399, 13=605, 10=284.

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6904 Parke East Blvd. Tampa FL 33610
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| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. | |
|---|---|---|---|---|---|---|
| 2233370 | T03G | GABLE | 1 | 1 | Job Reference (optional) | T19289623 |

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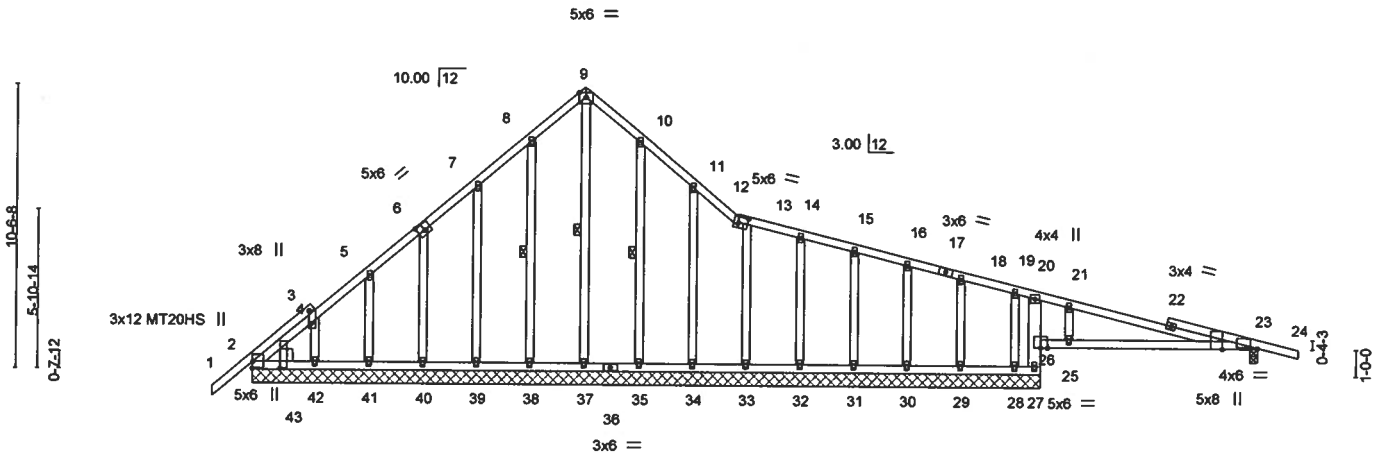
| 1-6-0 | 12-4-0 | 18-0-7 | 29-3-8 | 37-4-0 | 38-10-0 |
|---|---|---|---|---|---|
| 1-6-0 | 12-4-0 | 5-8-7 | 11-3-1 | 8-0-8 | 1-6-0 |

Scale = 1:82.4




| 29-3-8 | 37-4-0 |
|---|---|
| 29-3-8 | 8-0-8 |

Plate Offsets (X,Y)— [2:0-3-0,1-0-4], [3:0-5-11,0-1-0], [6:0-3-0,0-3-0], [12:0-0-5,0-1-4], [12:0-3-15,0-2-8], [13:0-1-13,0-0-0], [23:0-3-4,0-0-5], [23:0-0-9,Edge]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.60 | Vert(LL) | 0.17 | 23-25 | >563 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.43 | Vert(CT) | -0.17 | 23-25 | >561 | 180 | MT20HS | 187/143 |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.17 | Horz(CT) | 0.02 | 23 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-S | | | | | | | Weight: 254 lb | FT = 20% |

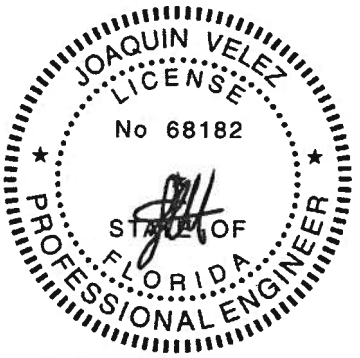
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
20-27: 2x6 SP No.2
OTHERS 2x4 SP No.3
WEDGE
Left: 2x6 SP No.2

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
10-0-0 oc bracing: 26-27,25-26,23-25.
WEBS 1 Row at midpt 9-37, 8-38, 10-35

**REACTIONS.** All bearings 29-3-8 except (jt=length) 23=0-3-8.
(lb) - Max Horz 2=-338(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 33, 32, 31, 30, 29, 43 except 26=-447(LC 9), 23=-278(LC 9), 38=-132(LC 12), 39=-143(LC 12), 40=-137(LC 12), 41=-124(LC 12), 42=-117(LC 12), 35=-127(LC 13), 34=-144(LC 13), 28=-177(LC 1), 2=-102(LC 8)
Max Grav All reactions 250 lb or less at joint(s) 27, 38, 39, 40, 41, 42, 35, 34, 33, 32, 31, 30, 29, 28, 2, 43 except 26=568(LC 1), 23=343(LC 1), 37=360(LC 22)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-339/304, 3-5=-256/292, 5-6=-158/269, 8-9=-145/272, 9-10=-146/253
BOT CHORD 2-43=-130/272, 42-43=-126/269, 41-42=-129/272, 40-41=-129/272, 39-40=-131/273, 38-39=-131/273, 37-38=-131/273, 35-37=-131/273, 34-35=-131/273, 33-34=-131/273, 32-33=-131/273, 31-32=-131/273, 30-31=-131/273, 29-30=-131/273, 28-29=-131/273, 27-28=-131/273, 20-26=-387/468
WEBS 9-37=-320/71

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) N/A
5) All plates are MT20 plates unless otherwise indicated.
6) All plates are 2x4 MT20 unless otherwise indicated.
7) Gable studs spaced at 2-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.




Joaquin Velez PE No.68182
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6904 Parke East Blvd. Tampa FL 33610
Date:
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MiTek
6904 Parke East Blvd.
Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | T03G | GABLE | 1 | 1 | T19289623 |
| | | | | | Job Reference (optional) |



**NOTES-**

11) Bearing at joint(s) 26 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 33, 32, 31, 30, 29, 43 except (jt=lb) 26=447, 23=278, 38=132, 39=143, 40=137, 41=124, 42=117, 35=127, 34=144, 28=177, 2=102.

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. | T19289624 |
|---|---|---|---|---|---|---|
| 2233370 | T04 | Roof Special | 5 | 1 | Job Reference (optional) | |

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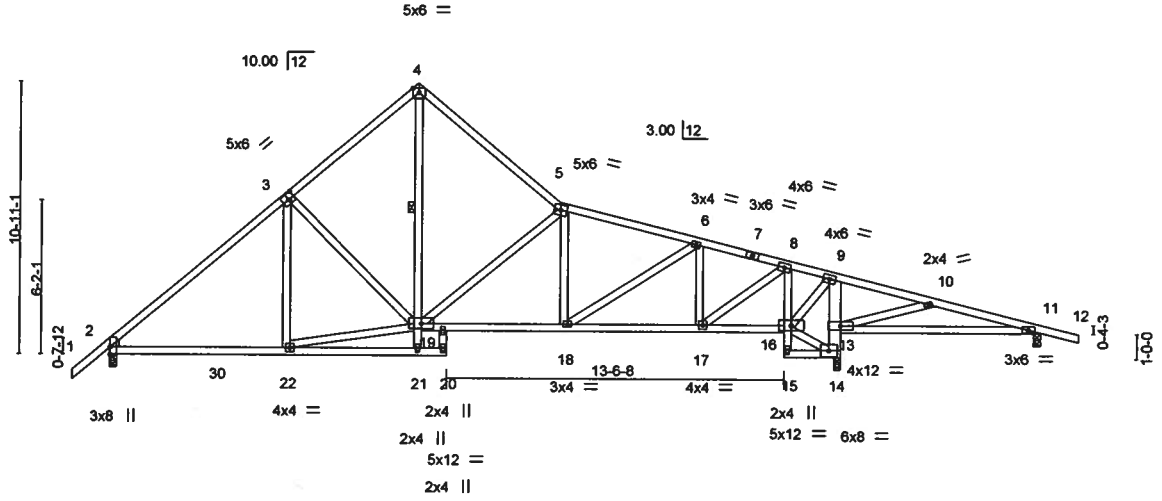
1-6-0 | 7-1-0 | 12-4-0 | 18-0-7 | 23-7-6 | 27-0-8 | 29-3-8 | 32-9-14 | 37-4-0 | 38-10-0
1-6-0 | 7-1-0 | 5-3-0 | 5-8-7 | 5-6-15 | 3-5-2 | 2-3-0 | 3-6-6 | 4-6-2 | 1-6-0

Scale = 1:88.9




7-1-0 | 12-5-12 | 13-6-0 | 18-0-7 | 23-7-6 | 27-0-8 | 29-3-8 | 37-4-0
7-1-0 | 5-4-12 | 1-0-4 | 4-6-7 | 5-6-15 | 3-5-2 | 2-3-0 | 8-0-8

Plate Offsets (X,Y)— [2:0-0-2,0-0-3], [2:0-0-5,0-3-2], [2:0-3-8,Edge], [3:0-3-0,0-3-0]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.52 | Vert(LL) | 0.09 | 22-26 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.49 | Vert(CT) | -0.16 | 13-29 | >605 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.86 | Horz(CT) | 0.06 | 14 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 232 lb | FT = 20% |

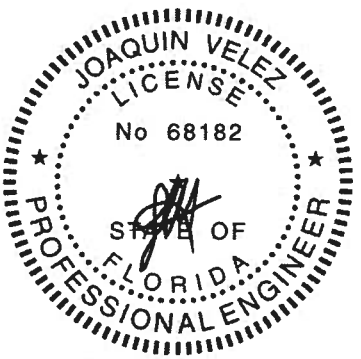
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
4-21,8-15: 2x4 SP No.3, 9-14: 2x6 SP No.2
WEBS 2x4 SP No.3
WEDGE
Left: 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 4-5-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-5 oc bracing. Except:
1 Row at midpt 4-19
10-0-0 oc bracing: 19-21

**REACTIONS.** (lb/size) 2=1120/0-3-8, 11=214/0-3-8, 14=1613/0-3-0
Max Horz 2=-350(LC 10)
Max Uplift 2=-392(LC 12), 11=-196(LC 9), 14=-613(LC 13)
Max Grav 2=1120(LC 1), 11=233(LC 24), 14=1613(LC 1)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1307/587, 3-4=-1082/623, 4-5=-1078/597, 5-6=-1444/741, 6-8=-1125/615, 9-10=-255/678
BOT CHORD 2-22=-385/1071, 4-19=-570/1064, 18-19=-468/1366, 17-18=-409/1076, 8-16=-890/425, 13-14=-1358/657, 9-13=-1134/578
WEBS 19-22=-386/1072, 3-19=-402/325, 5-19=-803/543, 6-18=-119/390, 6-17=-542/300, 8-17=-459/1067, 14-16=-535/285, 9-16=-395/958, 10-13=-550/363

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=392, 11=196, 14=613.




Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. T19289625 |
|---|---|---|---|---|---|
| 2233370 | T05 | Roof Special | 5 | 1 | Job Reference (optional) |

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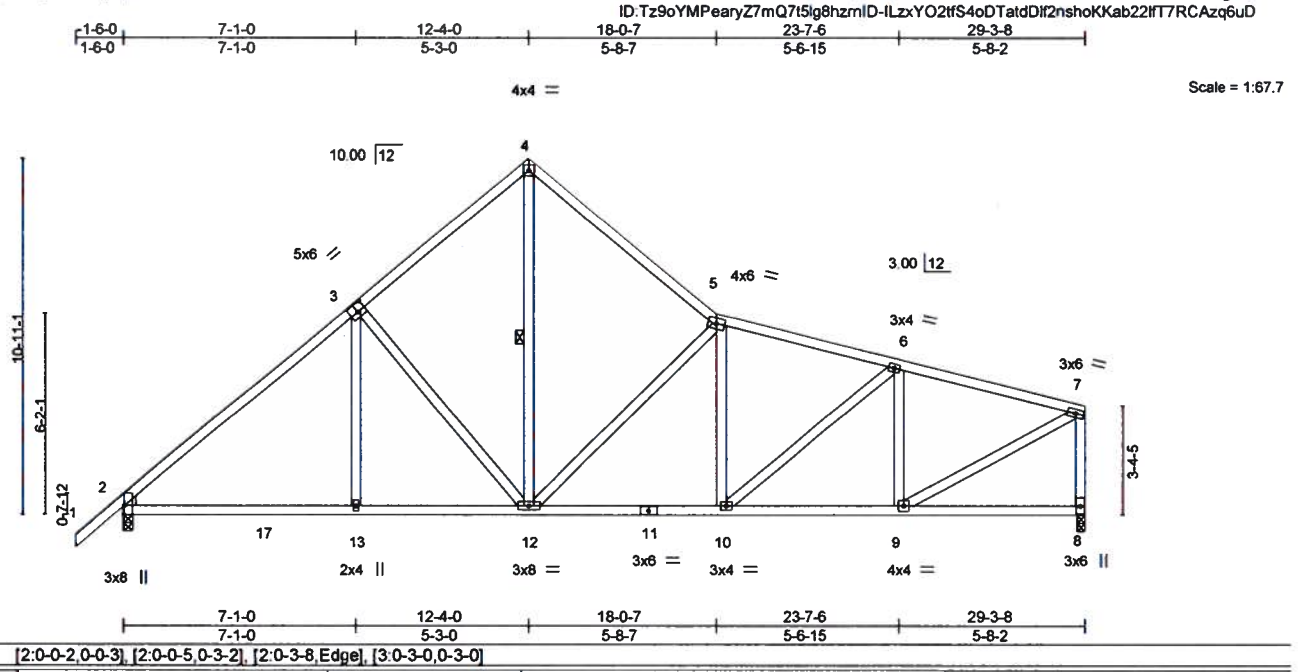



Plate Offsets (X,Y)— [2:0-0-2,0-0-3], [2:0-0-5,0-3-2], [2:0-3-8,Edge], [3:0-3-0,0-3-0]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.54 | Vert(LL) | 0.09 | 13-16 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.42 | Vert(CT) | -0.14 | 10-12 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 1.00 | Horz(CT) | 0.04 | 8 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 185 lb | FT = 20% |

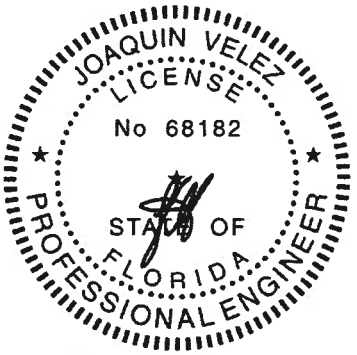
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
WEDGE
Left: 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 4-6-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-10-0 oc bracing.
WEBS 1 Row at midpt 4-12

**REACTIONS.** (lb/size) 2=1161/0-3-8, 8=1076/0-3-0
Max Horz 2=345(LC 12)
Max Uplift 2=-404(LC 12), 8=-411(LC 13)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1368/612, 3-4=-1057/643, 4-5=-1050/615, 5-6=-1357/711, 6-7=-1189/595, 7-8=-1024/544
BOT CHORD 2-13=-463/1075, 12-13=-463/1076, 10-12=-572/1278, 9-10=-539/1123
WEBS 3-12=-520/380, 4-12=-593/1024, 5-12=-782/526, 6-10=-103/263, 6-9=-502/332, 7-9=-600/1257

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=404, 8=411.




Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | T06 | Common | 3 | 1 | T19289626 |
| | | | | | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:25 2020 Page 1
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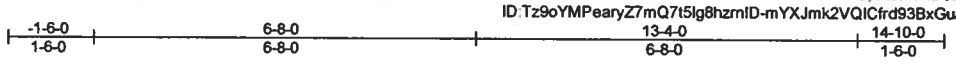



4x6 =

Scale = 1:38.7

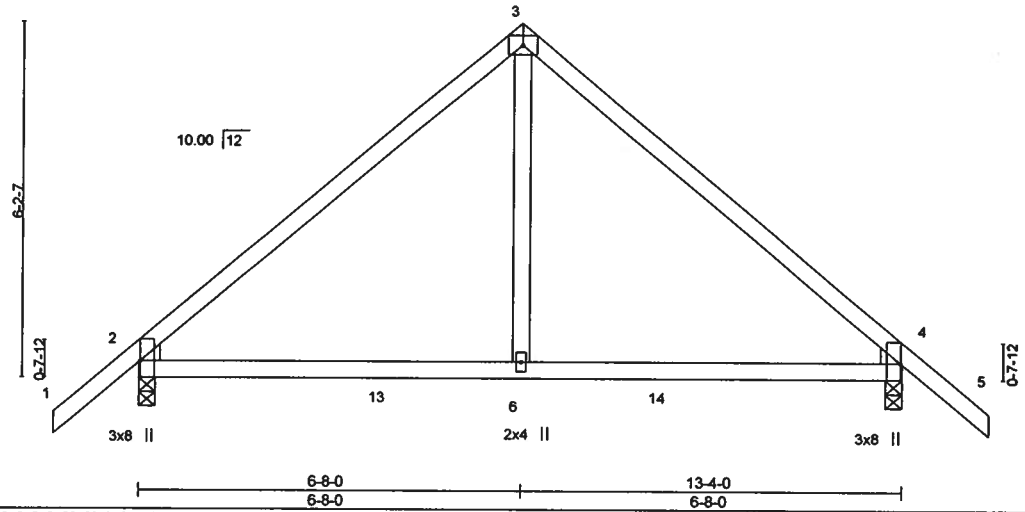



Plate Offsets (X,Y)-- [2:0-0-2,0-0-3], [2:0-0-5,0-3-2], [2:0-3-8,Edge], [4:0-0-2,0-0-3], [4:0-0-5,0-3-2], [4:0-3-8,Edge]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.52 | Vert(LL) | 0.10 | 6-9 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.44 | Vert(CT) | -0.11 | 6-9 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.12 | Horz(CT) | 0.02 | 4 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 61 lb | FT = 20% |

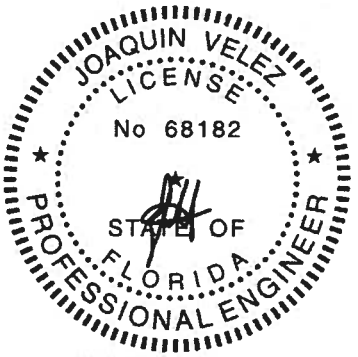
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
WEDGE
Left: 2x4 SP No.3, Right: 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 2=574/0-3-8, 4=574/0-3-8
Max Horz 2=-212(LC 10)
Max Uplift 2=-219(LC 12), 4=-219(LC 13)
Max Grav 2=599(LC 19), 4=599(LC 20)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-589/237, 3-4=-589/237
BOT CHORD 2-6=-46/436, 4-6=-46/436
WEBS 3-6=-38/323

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=219, 4=219.




Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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January 31,2020



MiTek
6904 Parke East Blvd.
Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | T06G | Common Supported Gable | 1 | 1 | T19289627 |
| | | | | | Job Reference (optional) |

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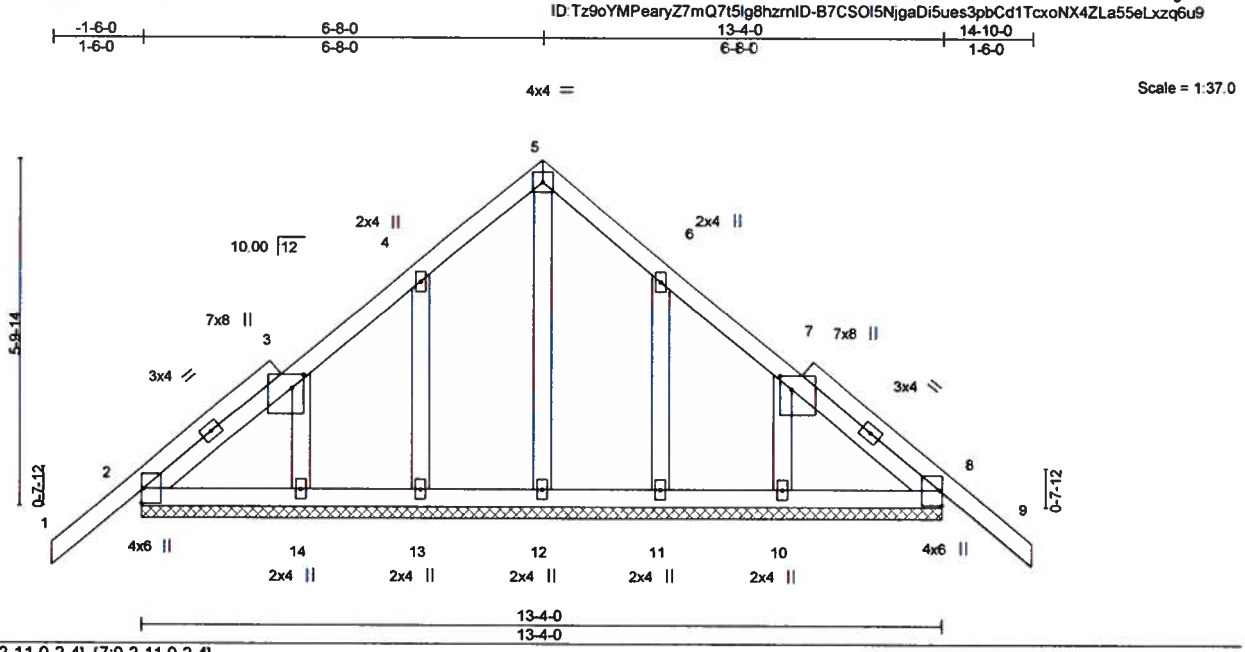



Plate Offsets (X,Y)-- [3:0-2-11,0-2-4], [7:0-2-11,0-2-4]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.16 | Vert(LL) | -0.01 | 9 | n/r | 120 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.05 | Vert(CT) | -0.01 | 9 | n/r | 120 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.07 | Horz(CT) | 0.00 | 8 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-S | | | | | | | Weight: 85 lb | FT = 20% |

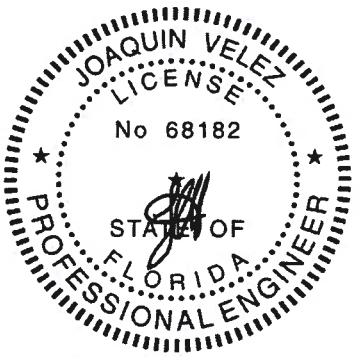
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** All bearings 13-4-0.
(lb) - Max Horz 2=200(LC 11)
Max Uplift All uplift 100 lb or less at joint(s) 2, 8 except 13=-137(LC 12), 14=-149(LC 12), 11=-138(LC 13), 10=-145(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8 except (jt=lb) 13=137, 14=149, 11=138, 10=145.
10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 8.




Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | T07 | Common | 2 | 1 | T19289628 |
| | | | | | Job Reference (optional) |

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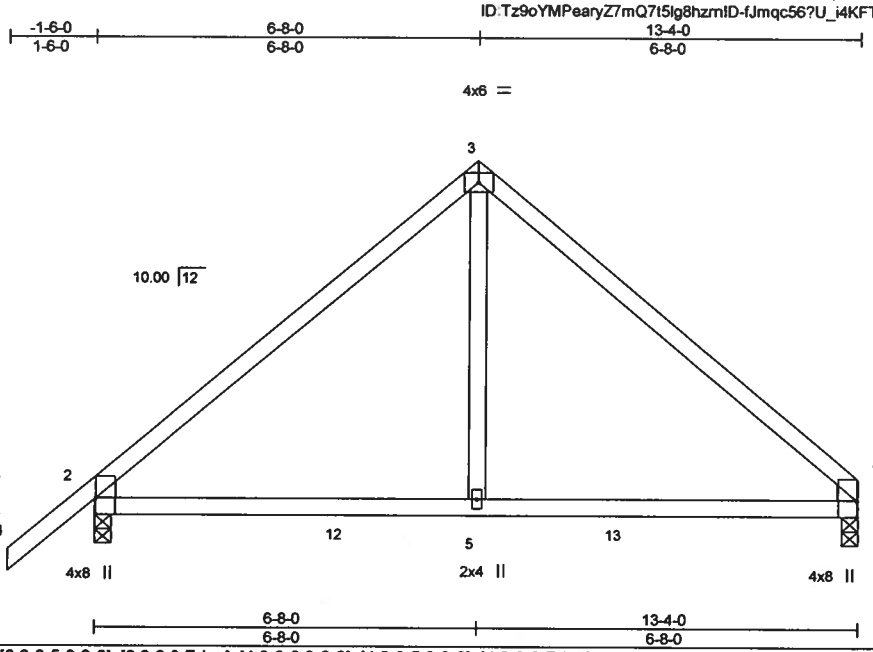


Plate Offsets (X,Y)-- [2:0-0-2,0-0-3], [2:0-0-5,0-3-2], [2:0-3-8,Edge], [4:0-0-2,0-0-3], [4:0-0-5,0-3-2], [4:0-3-8,Edge]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.51 | Vert(LL) | 0.11 | 5-11 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.43 | Vert(CT) | -0.11 | 5-11 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.12 | Horz(CT) | 0.03 | 4 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 59 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
WEDGE
Left: 2x4 SP No.3, Right: 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 2=579/0-3-8, 4=489/0-3-8
Max Horz 2=199(LC 9)
Max Uplift 2=-220(LC 12), 4=-165(LC 13)
Max Grav 2=601(LC 19), 4=516(LC 20)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-598/250, 3-4=-591/248
BOT CHORD 2-5=-74/420, 4-5=-74/420
WEBS 3-5=-51/325

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=220, 4=165.


Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. T19289629 |
|---|---|---|---|---|---|
| 2233370 | T08 | Common Girder | 1 | 2 | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:30 2020 Page 1
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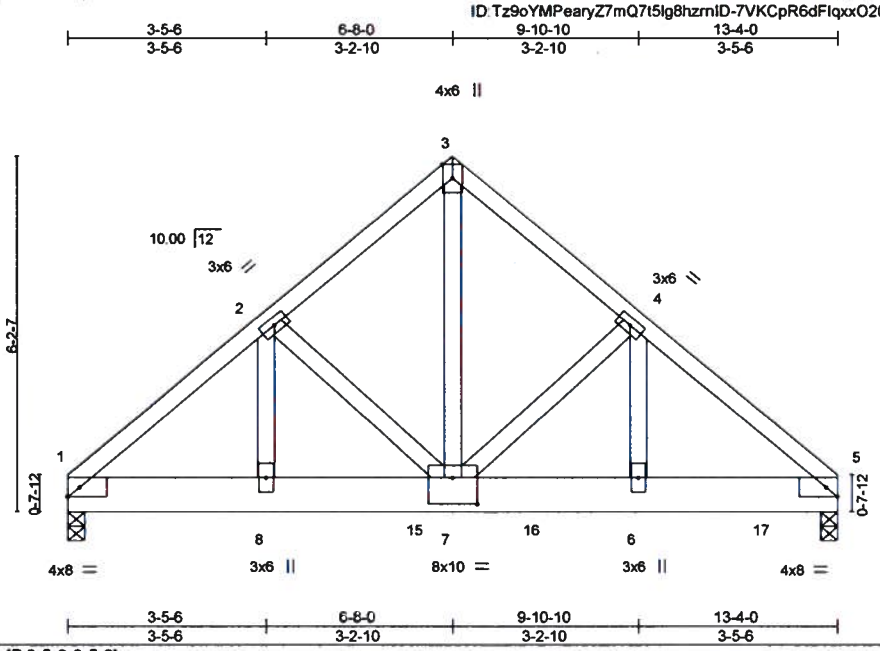



Plate Offsets (X,Y)— [5:0-2-7,Edge], [7:0-5-0,0-5-8]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.23 | Vert(LL) | 0.05 | 7-8 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.15 | Vert(CT) | -0.06 | 7 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.77 | Horz(CT) | 0.01 | 5 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 190 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x8 SP 2400F 2.0E
WEBS 2x4 SP No.3

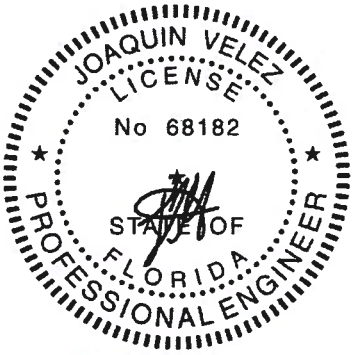
**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 1=2558/0-3-8, 5=3723/0-3-8
Max Horz 1=-173(LC 25)
Max Uplift 1=-1369(LC 8), 5=-1725(LC 9)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-3576/1955, 2-3=-3338/1875, 3-4=-3339/1876, 4-5=-4382/2114
BOT CHORD 1-8=-1525/2693, 7-8=-1525/2693, 6-7=-1570/3320, 5-6=-1570/3320
WEBS 3-7=-2268/4021, 4-7=-1082/452, 4-6=-353/1252, 2-7=-240/276

**NOTES-**
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-4-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=1369, 5=1725.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2513 lb down and 1746 lb up at 6-0-12, 927 lb down and 358 lb up at 8-0-12, and 927 lb down and 355 lb up at 10-0-12, and 927 lb down and 355 lb up at 12-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

**LOAD CASE(S)** Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 9-12=-20
Concentrated Loads (lb)
Vert: 6=-927(F) 15=-2513(F) 16=-927(F) 17=-927(F)




Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. | T19289630 |
|---|---|---|---|---|---|---|
| 2233370 | T09 | Half Hip Girder | 1 | 1 | Job Reference (optional) | |

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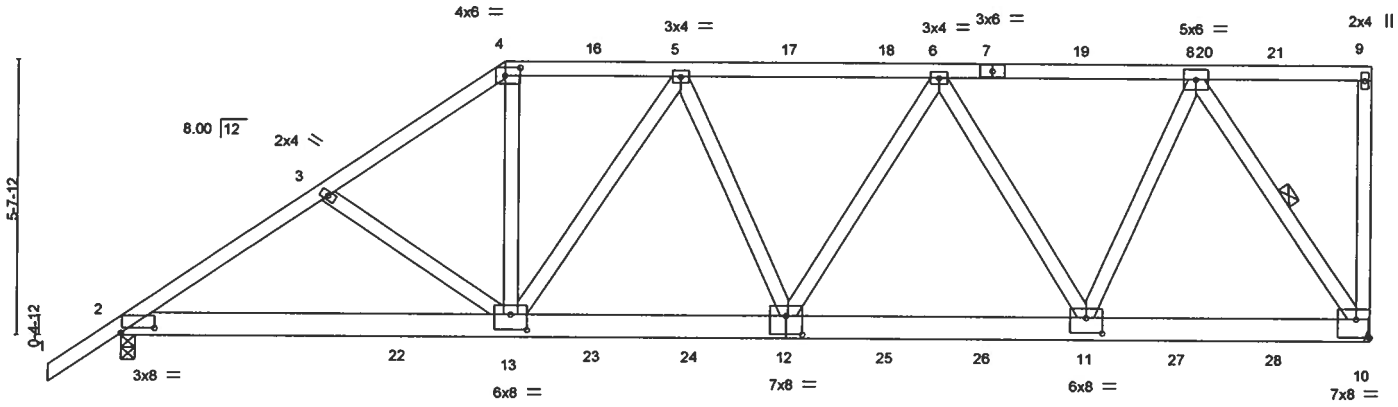
| -1-6-0 | 4-2-15 | 7-10-8 | 11-5-14 | 16-9-5 | 22-0-10 | 25-8-0 |
|---|---|---|---|---|---|---|
| 1-6-0 | 4-2-15 | 3-7-9 | 3-7-6 | 5-3-7 | 5-3-6 | 3-7-6 |

Scale = 1:45.7




| 7-10-8 | 13-8-8 | 19-10-1 | 25-8-0 |
|---|---|---|---|
| 7-10-8 | 5-10-0 | 6-1-10 | 5-9-15 |

Plate Offsets (X,Y)-- [2:0-8-4,0-1-3], [4:0-3-12,0-2-0], [10:Edge,0-4-8], [11:0-4-0,0-3-12], [12:0-4-0,0-4-8], [13:0-4-0,0-3-12]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.61 | Vert(LL) | 0.18 | 13-15 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.90 | Vert(CT) | -0.22 | 13-15 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.73 | Horz(CT) | 0.06 | 10 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 175 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 2-11-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-0-10 oc bracing.
WEBS 1 Row at midpt 8-10

**REACTIONS.** (lb/size) 10=2533/Mechanical, 2=2110/0-3-8
Max Horz 2=299(LC 8)
Max Uplift 10=-1726(LC 5), 2=-1270(LC 8)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-3280/2015, 3-4=-3103/1958, 4-5=-2564/1687, 5-6=-2937/1916, 6-8=-2096/1360
BOT CHORD 2-13=-1860/2709, 12-13=-1897/2858, 11-12=-1738/2606, 10-11=-909/1351
WEBS 3-13=-263/242, 4-13=-923/1517, 5-13=-546/470, 6-12=-386/653, 6-11=-1015/751, 8-11=-1165/1923, 8-10=-2434/1640

**NOTES-**
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
2) Provide adequate drainage to prevent water ponding.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=1726, 2=1270.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 81 lb down and 90 lb up at 7-10-8, 90 lb down and 81 lb up at 9-8-12, 90 lb down and 81 lb up at 11-8-12, 90 lb down and 81 lb up at 13-8-12, 90 lb down and 81 lb up at 15-8-12, 90 lb down and 81 lb up at 17-8-12, 90 lb down and 81 lb up at 19-8-12, and 90 lb down and 81 lb up at 21-8-12, and 90 lb down and 81 lb up at 23-8-12 on top chord, and 343 lb down and 308 lb up at 5-8-12, 232 lb down and 155 lb up at 7-8-12, 232 lb down and 155 lb up at 9-8-12, 232 lb down and 155 lb up at 11-8-12, 232 lb down and 155 lb up at 13-8-12, 232 lb down and 155 lb up at 15-8-12, 232 lb down and 155 lb up at 17-8-12, 232 lb down and 155 lb up at 19-8-12, 232 lb down and 155 lb up at 21-8-12, and 232 lb down and 155 lb up at 23-8-12, and 208 lb down and 199 lb up at 25-6-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-9=-54, 2-10=-20

Continued on page 2

JOAQUIN VELEZ LICENSE No 68182 STATE OF FLORIDA PROFESSIONAL ENGINEER

Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020



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Tampa, FL 36610

**LOAD CASE(S)** Standard

Concentrated Loads (lb)

Vert: 4=-5(B) 7=-5(B) 10=-194(B) 13=-232(B) 5=-5(B) 12=-232(B) 11=-232(B) 16=-5(B) 17=-5(B) 18=-5(B) 19=-5(B) 20=-5(B) 21=-5(B) 22=-343(B) 23=-232(B) 24=-232(B) 25=-232(B) 26=-232(B) 27=-232(B) 28=-232(B)

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. | T19289631 |
|---|---|---|---|---|---|---|
| 2233370 | T10 | Common | 2 | 1 | Job Reference (optional) | |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:33 2020 Page 1
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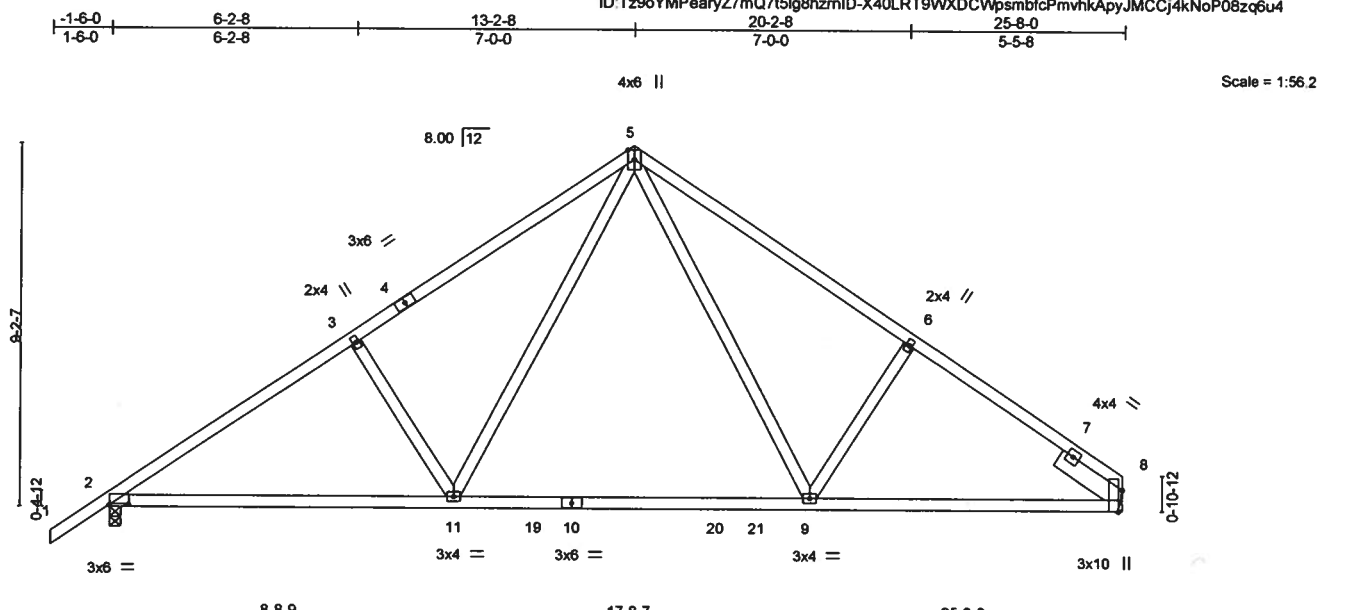



Plate Offsets (X,Y)-- [2:0-6-0,0-0-3], [8:0-6-9,Edge]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.71 | Vert(LL) | -0.33 | 9-11 | >942 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.83 | Vert(CT) | -0.47 | 9-11 | >657 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.58 | Horz(CT) | 0.06 | 8 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 134 lb | FT = 20% |

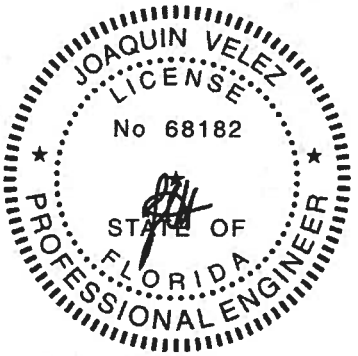
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Right 2x6 SP No.2 1-11-8

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 3-3-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-2-4 oc bracing.

**REACTIONS.** (lb/size) 8=947/Mechanical, 2=1033/0-3-8
Max Horz 2=293(LC 9)
Max Uplift 8=-335(LC 13), 2=-396(LC 12)
Max Grav 8=951(LC 20), 2=1036(LC 19)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1400/613, 3-5=-1301/657, 5-6=-1212/626, 6-8=-1281/584
BOT CHORD 2-11=-509/1295, 9-11=-173/782, 8-9=-392/1000
WEBS 3-11=-433/375, 5-11=-305/687, 5-9=-265/555, 6-9=-346/345

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=335, 2=396.

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6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020




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Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. T19289632 |
|---|---|---|---|---|---|
| 2233370 | T10G | GABLE | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:34 2020 Page 1
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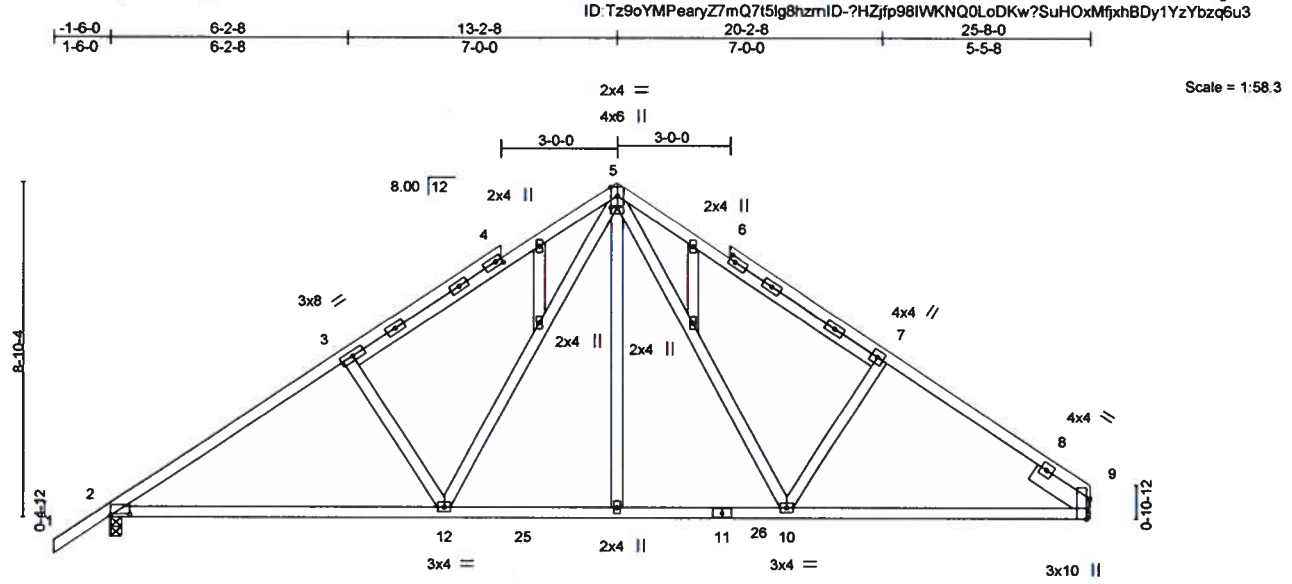



Plate Offsets (X,Y)-- [2:0-6-0,0-0-8], [4:0-2-0,0-1-8], [5:0-2-0,0-0-0], [6:0-2-0,0-1-8], [9:0-6-9,Edge]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.56 | Vert(LL) | -0.32 | 10-12 | >973 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.82 | Vert(CT) | -0.46 | 10-12 | >675 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.51 | Horz(CT) | 0.06 | 9 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 166 lb | FT = 20% |

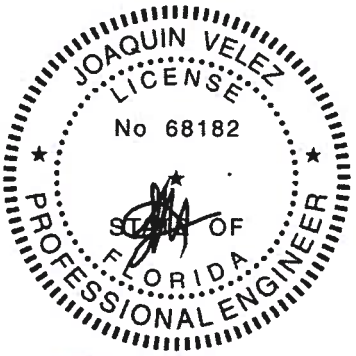
**LUMBER-**
TOP CHORD 2x4 SP M 31 *Except*
1-4,6-9: 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3
SLIDER Right 2x6 SP No.2 1-9-3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 4-2-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-2-15 oc bracing.

**REACTIONS.** (lb/size) 2=1033/0-3-8, 9=947/Mechanical
Max Horz 2=282(LC 9)
Max Uplift 2=-398(LC 12), 9=-338(LC 13)
Max Grav 2=1037(LC 19), 9=951(LC 20)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1399/610, 3-5=-1290/639, 5-7=-1186/603, 7-9=-1270/574
BOT CHORD 2-12=-499/1285, 10-12=-185/806, 9-10=-379/984
WEBS 3-12=-389/348, 5-12=-282/645, 5-10=-236/499, 7-10=-281/309

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
9) Refer to girder(s) for truss to truss connections.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=398, 9=338.

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January 31,2020

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.**
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ***ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information*** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. | T19289633 |
|---|---|---|---|---|---|---|
| 2233370 | T11 | Piggyback Base | 2 | 1 | Job Reference (optional) | |

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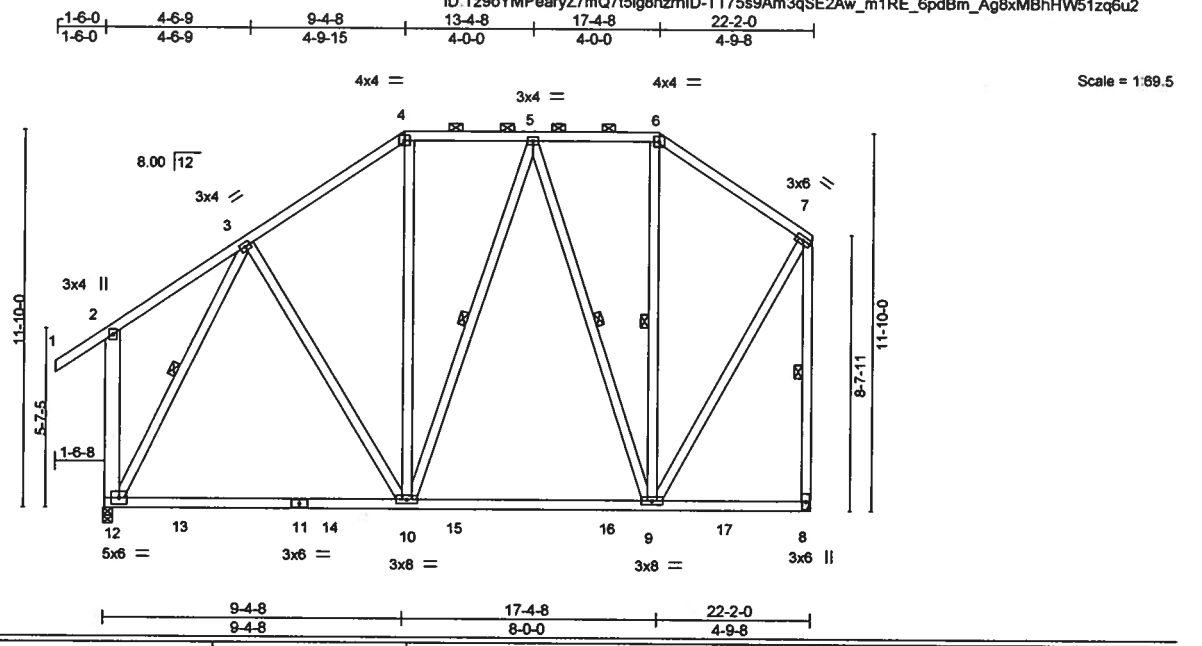



| LOADING (psf) | SPACING- 2-0-0 | CSI. | DEFL. in (loc) l/defl L/d | PLATES GRIP |
|---|---|---|---|---|
| TCLL 20.0 | Plate Grip DOL 1.25 | TC 0.27 | Vert(LL) -0.25 10-12 >999 240 | MT20 244/190 |
| TCDL 7.0 | Lumber DOL 1.25 | BC 0.87 | Vert(CT) -0.42 10-12 >615 180 | |
| BCLL 0.0 * | Rep Stress Incr YES | WB 0.47 | Horz(CT) 0.01 8 n/a n/a | |
| BCDL 10.0 | Code FBC2017/TPI2014 | Matrix-MS | | Weight: 207 lb FT = 20% |

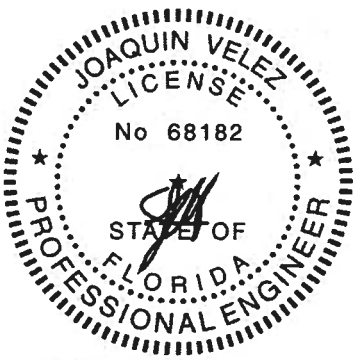
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except*
2-12: 2x6 SP No.2

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-10, 5-9, 6-9, 3-12, 7-8

**REACTIONS.** (lb/size) 12=904/0-3-8, 8=801/Mechanical
Max Horz 12=-346(LC 10)
Max Uplift 12=-281(LC 12), 8=-257(LC 12)
Max Grav 12=950(LC 2), 8=881(LC 2)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-265/273, 3-4=-579/383, 4-5=-495/378, 5-6=-348/278, 6-7=-408/257, 2-12=-325/276, 7-8=-802/450
BOT CHORD 10-12=-334/458, 9-10=-212/391
WEBS 5-10=-114/262, 5-9=-332/263, 3-12=-690/252, 7-9=-265/584

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=281, 8=257.
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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January 31,2020



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| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. T19289634 |
|---|---|---|---|---|---|
| 2233370 | T11G | GABLE | 1 | 1 | Job Reference (optional) |

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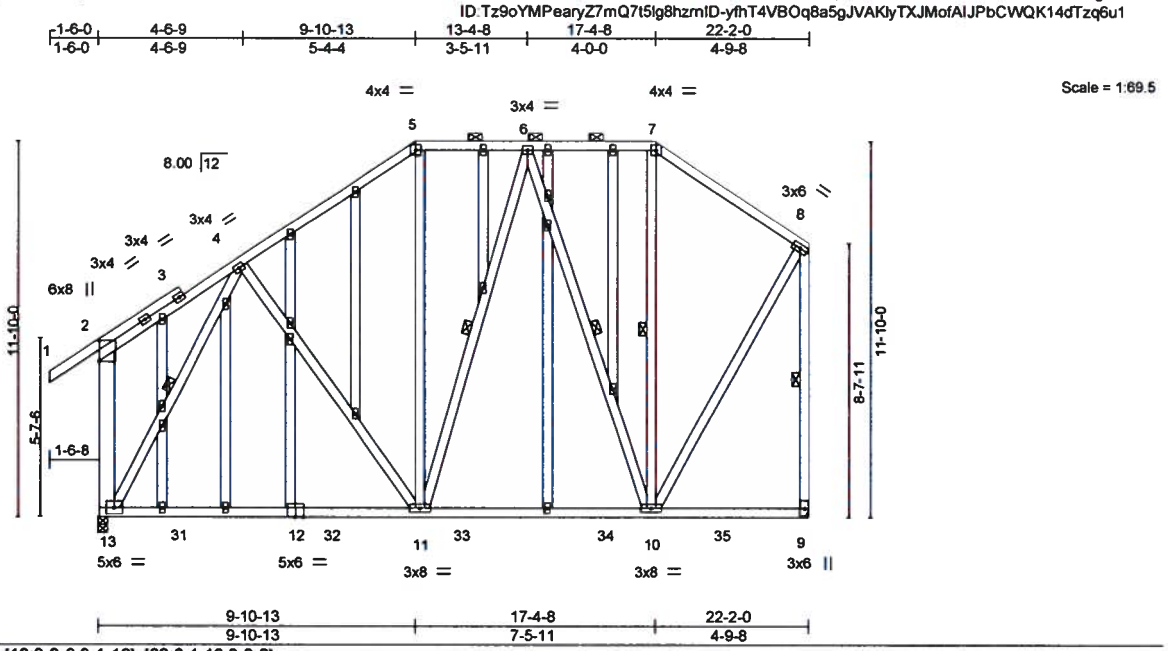


Plate Offsets (X,Y)— [2:0-7-9,0-0-0], [12:0-0-0,0-1-12], [23:0-1-12,0-0-0]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.29 | Vert(LL) | -0.31 | 11-13 | >831 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.94 | Vert(CT) | -0.54 | 11-13 | >483 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.47 | Horz(CT) | 0.01 | 9 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 289 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except*
2-13: 2x6 SP No.2
OTHERS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-7.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 6-11, 6-10, 7-10, 4-13, 8-9

**REACTIONS.** (lb/size) 13=904/0-3-8, 9=801/Mechanical
Max Horz 13=-342(LC 10)
Max Uplift 13=-286(LC 12), 9=-263(LC 12)
Max Grav 13=946(LC 2), 9=877(LC 2)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-4=-253/250, 4-5=-590/371, 5-6=-499/375, 6-7=-349/278, 7-8=-403/256, 2-13=-314/257, 8-9=-791/448
BOT CHORD 11-13=-348/477, 10-11=-209/391
WEBS 6-11=-129/295, 6-10=-346/252, 4-13=-678/276, 8-10=-264/575

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Refer to girder(s) for truss to truss connections.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 13=286, 9=263.
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.


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January 31,2020

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. | T19289635 |
|---|---|---|---|---|---|---|
| 2233370 | T12 | Piggyback Base | 12 | 1 | Job Reference (optional) | |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:38 2020 Page 1
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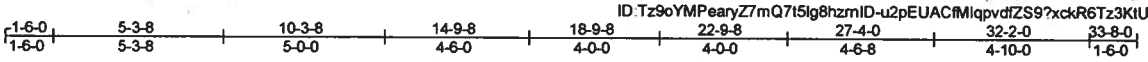


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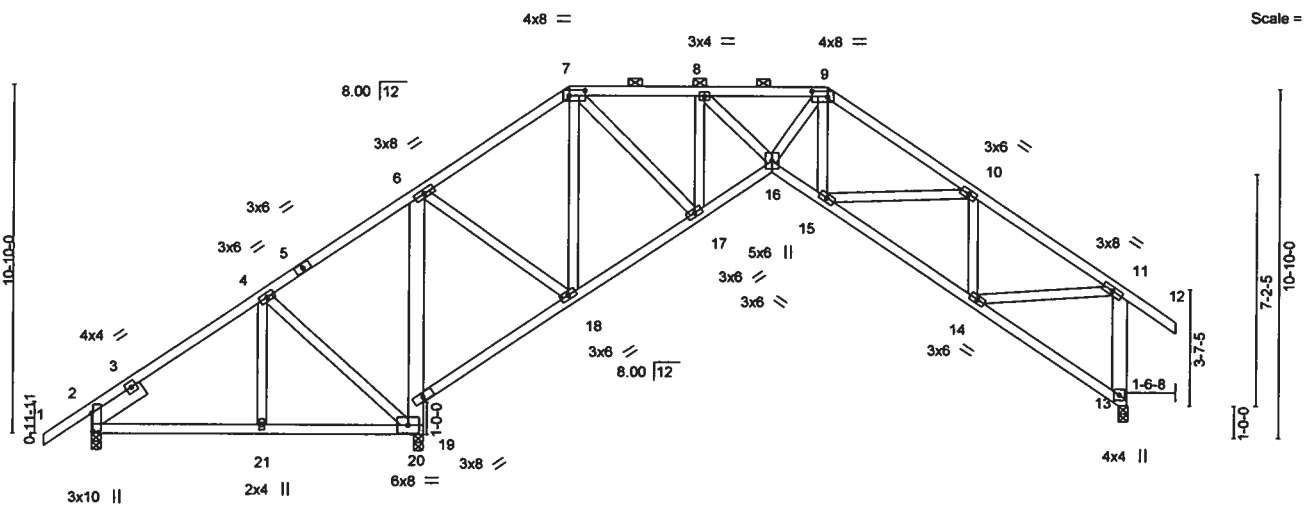


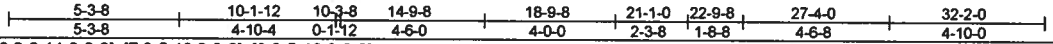




Plate Offsets (X,Y)– [2:0-6-11,0-0-8], [7:0-5-12,0-2-0], [9:0-5-12,0-2-0]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.39 | Vert(LL) | -0.07 | 16 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.58 | Vert(CT) | -0.13 | 16 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.55 | Horz(CT) | 0.16 | 13 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 220 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
6-20: 2x6 SP No.2
WEBS 2x4 SP No.3 *Except*
11-13: 2x6 SP No.2
SLIDER Left 2x6 SP No.2 1-11-8

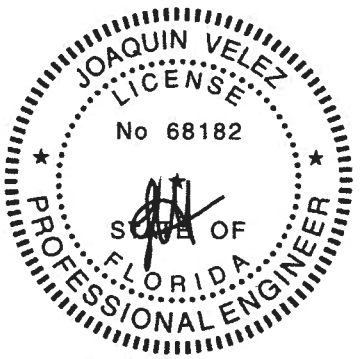
**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 4-10-9 oc purlins, except end verticals, and 2-0-0 oc purlins (4-9-6 max.): 7-9.
BOT CHORD Rigid ceiling directly applied or 5-8-5 oc bracing.

**REACTIONS.** (lb/size) 13=875/0-3-8, 2=153/0-3-8, 20=1727/0-3-8
Max Horz 2=391(LC 11)
Max Uplift 13=-368(LC 13), 2=-160(LC 12), 20=-696(LC 9)
Max Grav 13=875(LC 1), 2=205(LC 23), 20=1727(LC 1)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-4=-167/342, 4-6=-295/565, 6-7=-423/376, 7-8=-784/571, 8-9=-1449/802, 9-10=-1341/726, 10-11=-1033/589, 11-13=-820/547
BOT CHORD 19-20=-1452/674, 6-19=-1075/541, 18-19=-581/247, 17-18=-230/306, 16-17=-529/945, 15-16=-481/1250, 14-15=-490/1060
WEBS 4-21=-263/251, 4-20=-447/493, 6-18=-280/753, 7-18=-694/299, 7-17=-362/859, 9-16=-420/676, 10-15=-238/287, 11-14=-377/795, 8-17=-900/403, 8-16=-329/946, 10-14=-446/258

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Bearing at joint(s) 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 13=368, 2=160, 20=696.
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard

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Date:
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| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. T19289635 |
|---|---|---|---|---|---|
| 2233370 | T12 | Piggyback Base | 12 | 1 | Job Reference (optional) |



**LOAD CASE(S)** Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-7=-54, 7-9=-54, 9-11=-54, 11-12=-54, 20-22=-20, 16-19=-30(F=-10), 13-16=-30(F=-10)

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | T12G | GABLE I I | 1 | 1 | T19289636 |
| | | | | | Job Reference (optional) |

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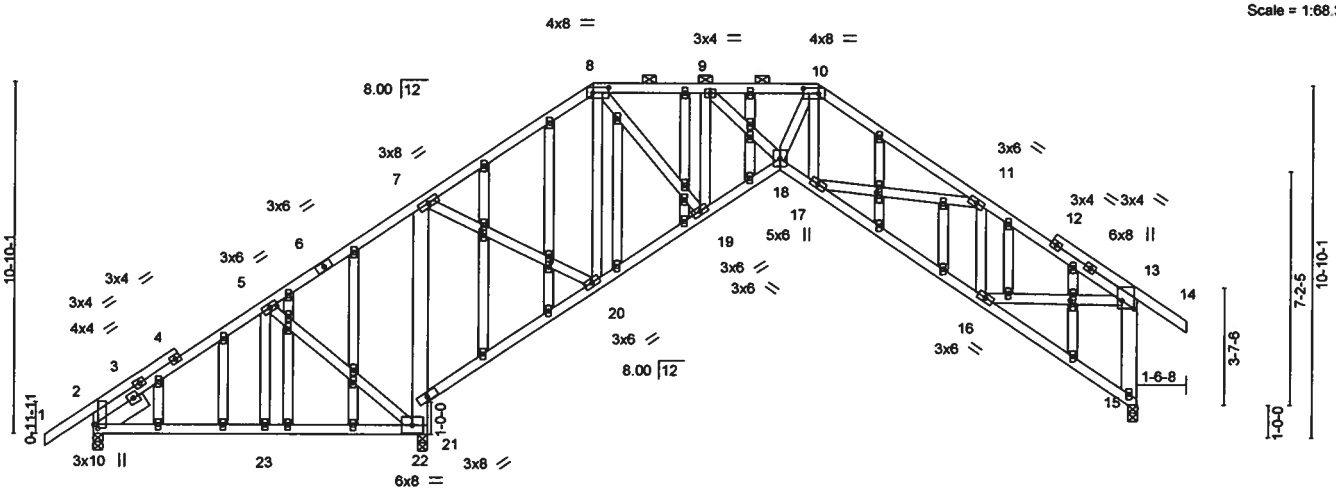
| 1-6-0 | 5-3-8 | 10-3-8 | 15-3-13 | 18-9-8 | 22-3-3 | 27-4-0 | 32-2-0 | 33-8-0 |
|---|---|---|---|---|---|---|---|---|
| 1-6-0 | 5-3-8 | 5-0-0 | 5-0-5 | 3-5-11 | 3-5-11 | 5-0-13 | 4-10-0 | 1-6-0 |

Scale = 1:68.3




| 5-3-8 | 10-1-12 | 10-3-8 | 15-3-13 | 18-9-8 | 21-1-0 | 22-3-3 | 27-4-0 | 32-2-0 |
|---|---|---|---|---|---|---|---|---|
| 5-3-8 | 4-10-4 | 0-1-12 | 5-0-5 | 3-5-11 | 2-3-8 | 1-2-3 | 5-0-13 | 4-10-0 |

Plate Offsets (X,Y)– [2:0-1-8,0-1-7], [8:0-5-12,0-2-0], [10:0-5-12,0-2-0], [13:0-3-0,Edge], [28:0-1-8,0-1-0], [35:0-1-9,0-1-0], [52:0-1-14,0-1-0], [55:0-1-14,0-1-0]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.41 | Vert(LL) | -0.06 | 18 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.53 | Vert(CT) | -0.11 | 18 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.45 | Horz(CT) | 0.13 | 15 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 294 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
7-22: 2x6 SP No.2
WEBS 2x4 SP No.3 *Except*
13-15: 2x6 SP No.2
OTHERS 2x4 SP No.3
SLIDER Left 2x6 SP No.2 1-8-15

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 4-11-12 oc purlins, except end verticals, and 2-0-0 oc purlins (5-4-10 max.): 8-10.
BOT CHORD Rigid ceiling directly applied or 5-9-1 oc bracing.

**REACTIONS.** (lb/size) 15=760/0-3-8, 2=137/0-3-8, 22=1635/0-3-8
Max Horz 2=375(LC 11)
Max Uplift 15=-322(LC 13), 2=-178(LC 8), 22=-591(LC 9)
Max Grav 15=760(LC 1), 2=206(LC 23), 22=1635(LC 1)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-5=-257/406, 5-7=-337/639, 7-8=-349/310, 8-9=-601/382, 9-10=-1171/504, 10-11=-1236/495, 11-13=-959/453, 13-15=-731/443
BOT CHORD 2-23=-271/76, 22-23=-266/70, 21-22=-1387/579, 7-21=-997/465, 20-21=-625/235, 18-19=-327/725, 17-18=-303/1123, 16-17=-384/982
WEBS 5-22=-401/456, 7-20=-218/712, 8-20=-636/253, 8-19=-210/692, 9-19=-759/260, 9-18=-174/811, 10-18=-264/476, 11-17=-236/269, 11-16=-425/235, 13-16=-282/741

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Bearing at joint(s) 15 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 15=322, 2=178, 22=591.
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.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| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. | T19289637 |
|---|---|---|---|---|---|---|
| 2233370 | T13 | Piggyback Base Girder | 4 | 2 | Job Reference (optional) | |

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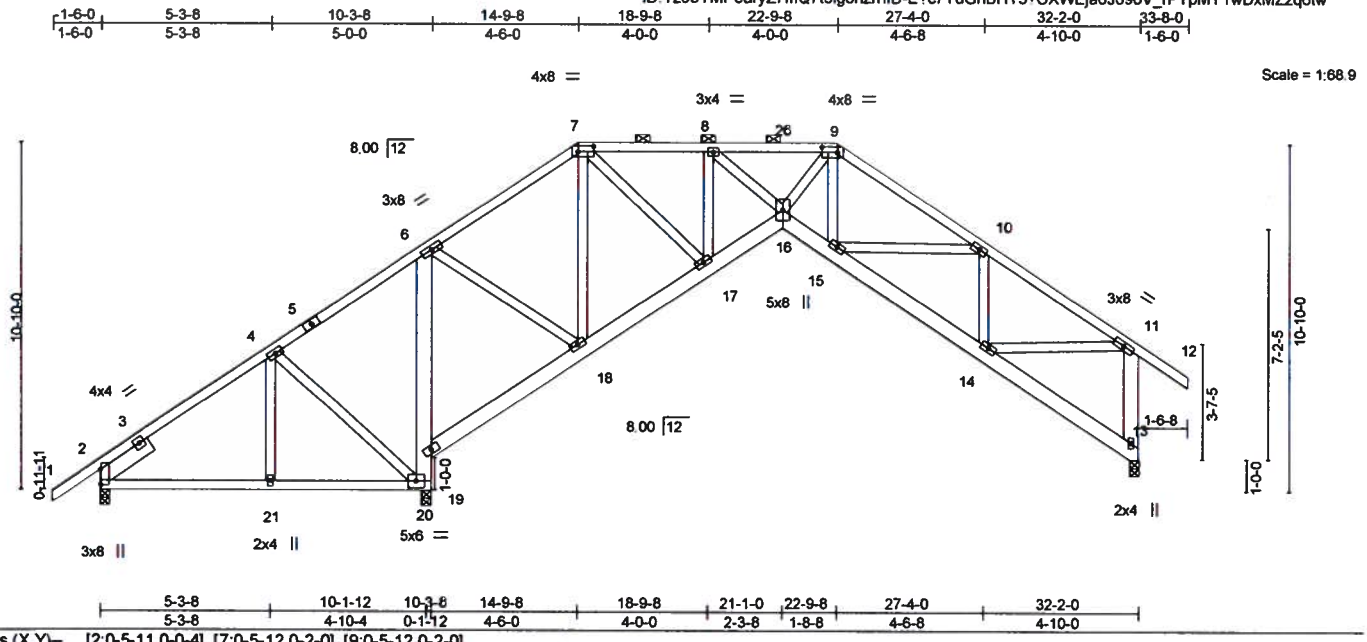



Plate Offsets (X,Y)— [2:0-5-11,0-0-4], [7:0-5-12,0-2-0], [9:0-5-12,0-2-0]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.25 | Vert(LL) | -0.05 | 16 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.27 | Vert(CT) | -0.09 | 16 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.32 | Horz(CT) | 0.11 | 13 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 482 lb | FT = 20% |

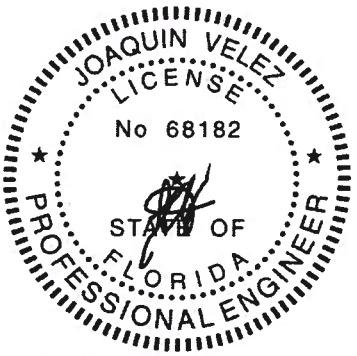
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2 *Except*
2-20: 2x4 SP No.2
WEBS 2x4 SP No.3 *Except*
11-13: 2x6 SP No.2
SLIDER Left 2x6 SP No.2 1-11-8

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 7-9.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS.** (lb/size) 13=1256/0-3-8, 2=-1/0-3-8, 20=2198/0-3-8
Max Horz 2=503(LC 7)
Max Uplift 13=-563(LC 9), 2=-254(LC 27), 20=-851(LC 5)
Max Grav 13=1256(LC 1), 2=150(LC 16), 20=2198(LC 1)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-4=-247/535, 4-6=-263/814, 6-7=-302/316, 7-8=-1135/548, 8-9=-2337/940, 9-10=-2114/753, 10-11=-1589/684, 11-13=-1166/558
BOT CHORD 2-21=-419/329, 20-21=-419/329, 19-20=-1908/647, 6-19=-1339/504, 18-19=-874/457, 17-18=-337/328, 16-17=-537/1338, 15-16=-619/1955, 14-15=-506/1560
WEBS 4-21=-133/264, 4-20=-428/277, 6-18=-329/1009, 7-18=-1021/370, 7-17=-513/1343, 8-17=-1391/563, 8-16=-545/1672, 9-16=-520/1095, 9-15=-258/410, 10-15=-377/450, 10-14=-661/253, 11-14=-430/1224

**NOTES-**
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; end vertical right exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
5) Provide adequate drainage to prevent water ponding.
6) All plates are 3x6 MT20 unless otherwise indicated.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Bearing at joint(s) 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

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**NOTES-**

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 13=563, 2=254, 20=851.
12) Girder carries tie-in span(s): 3-0-0 from 21-1-0 to 32-2-0; 3-0-0 from 21-1-0 to 32-2-0
13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
14) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 299 lb down and 208 lb up at 21-1-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

**LOAD CASE(S)** Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-7=-54, 7-26=-54, 9-26=-72(F=-18), 9-11=-73(F=-19), 11-12=-54, 20-22=-20, 16-19=-30(F=-10), 13-16=-49(F=-29)
   Concentrated Loads (lb)
   Vert: 16=-299(F)




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| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES | T19289638 |
|---|---|---|---|---|---|---|
| 2233370 | T14 | Piggyback Base Girder | 2 | 2 | Job Reference (optional) | |

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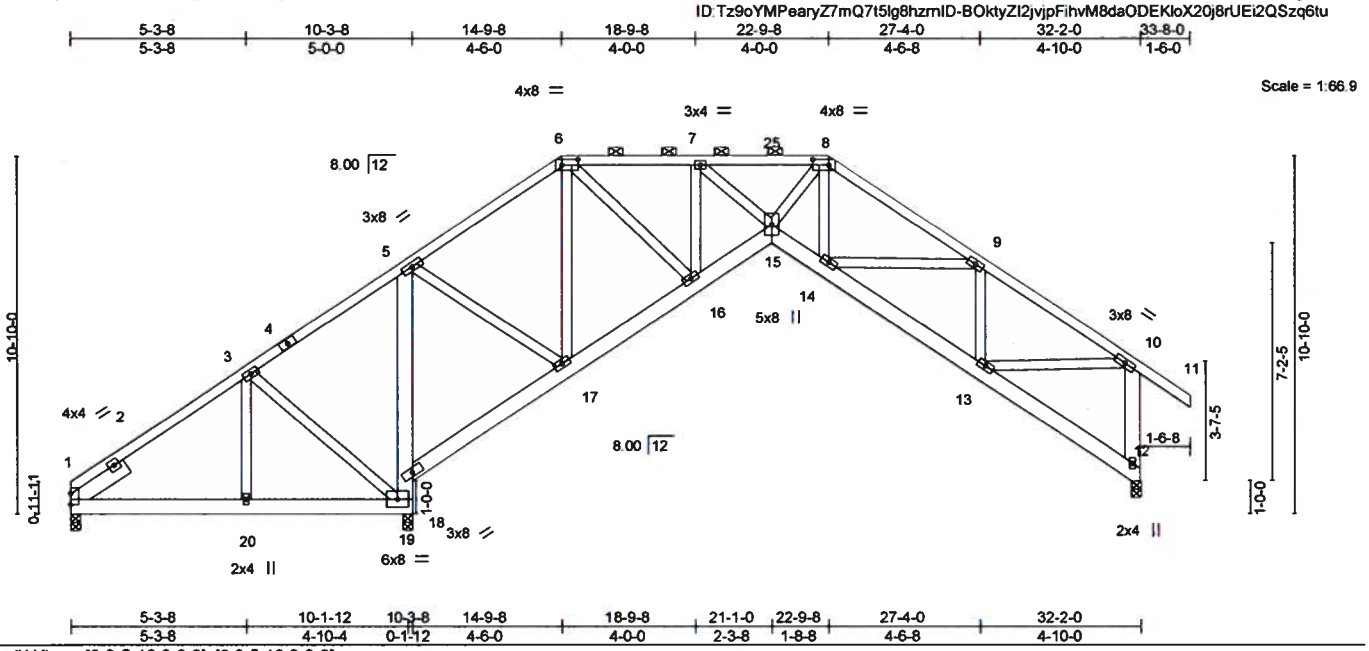






Plate Offsets (X,Y)-- [6:0-5-12,0-2-0], [8:0-5-12,0-2-0]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.26 | Vert(LL) | -0.05 | 15 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.26 | Vert(CT) | -0.09 | 15 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.30 | Horz(CT) | 0.10 | 12 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 492 lb | FT = 20% |

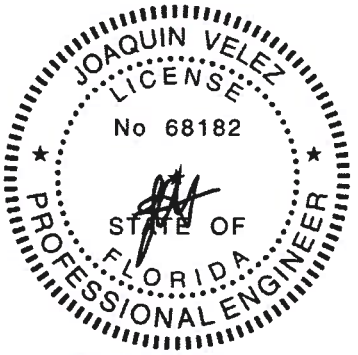
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3 *Except*
10-12: 2x6 SP No.2
SLIDER Left 2x6 SP No.2 1-11-8

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-8.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS.** (lb/size) 12=1198/0-3-8, 1=-214/0-3-8, 19=2389/0-3-8
Max Horz 1=482(LC 7)
Max Uplift 12=-545(LC 9), 1=-395(LC 20), 19=-937(LC 5)
Max Grav 12=1198(LC 1), 1=157(LC 7), 19=2389(LC 1)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-3=-250/709, 3-5=-375/1060, 5-6=-196/282, 6-7=-931/453, 7-8=-2061/810, 8-9=-1921/662, 9-10=-1494/654, 10-12=-1108/539
BOT CHORD 1-20=-563/351, 19-20=-563/351, 18-19=-2108/740, 5-18=-1409/535, 17-18=-1112/497, 16-17=-360/355, 15-16=-424/1098, 14-15=-533/1771, 13-14=-476/1465
WEBS 3-20=-180/331, 3-19=-498/299, 5-17=-353/1064, 6-17=-1076/396, 6-16=-482/1276, 7-16=-1323/531, 7-15=-497/1572, 8-15=-438/903, 8-14=-264/408, 9-14=-373/378, 9-13=-602/235, 10-13=-405/1146

**NOTES-**
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; end vertical right exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
5) Provide adequate drainage to prevent water ponding.
6) All plates are 3x6 MT20 unless otherwise indicated.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=545, 1=395, 19=937.

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| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | T14 | Piggyback Base Girder | 2 | 2 | T19289638 |
| | | | | | Job Reference (optional) |

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**NOTES-**

12) Girder carries tie-in span(s): 3-0-0 from 21-1-0 to 32-2-0; 3-0-0 from 21-1-0 to 32-2-0
13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
14) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 299 lb down and 208 lb up at 21-1-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

**LOAD CASE(S)** Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-6=-54, 6-25=-54, 8-25=-72(F=-18), 8-10=-72(F=-18), 10-11=-54, 19-21=-20, 15-18=-30(F=-10), 12-15=-49(F=-29)
   Concentrated Loads (lb)
   Vert: 15=-299(F)



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| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. | T19289639 |
|---|---|---|---|---|---|---|
| 2233370 | T15 | Piggyback Base | 2 | 1 | Job Reference (optional) | |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:47 2020 Page 1
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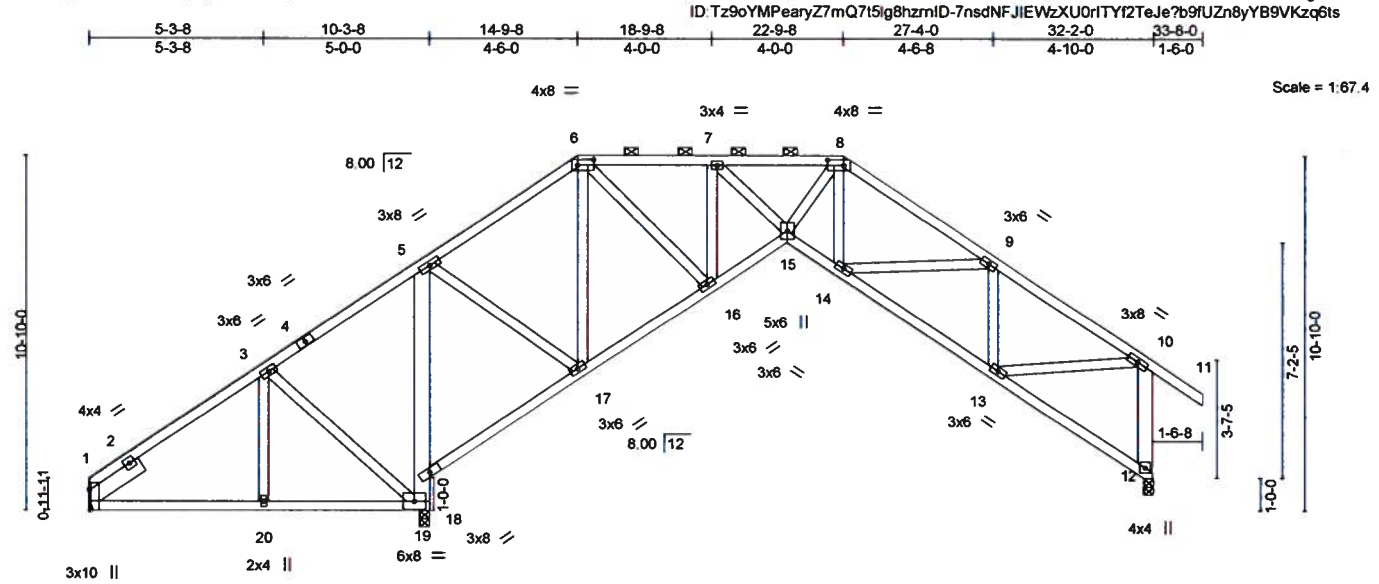



Plate Offsets (X,Y)— [1:0-7-7,Edge], [6:0-5-12,0-2-0], [8:0-5-12,0-2-0]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.41 | Vert(LL) | -0.07 | 15 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.57 | Vert(CT) | -0.13 | 15 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.55 | Horz(CT) | 0.16 | 12 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 218 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
5-19: 2x6 SP No.2
WEBS 2x4 SP No.3 *Except*
10-12: 2x6 SP No.2
SLIDER Left 2x6 SP No.2 1-11-8

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 4-10-3 oc purlins, except end verticals, and 2-0-0 oc purlins (4-8-15 max.): 6-8.
BOT CHORD Rigid ceiling directly applied or 5-8-15 oc bracing.

**REACTIONS.** (lb/size) 12=880/0-3-8, 1=76/Mechanical, 19=1718/0-3-8
Max Horz 1=370(LC 11)
Max Uplift 12=-369(LC 13), 1=-122(LC 8), 19=-694(LC 9)
Max Grav 12=880(LC 1), 1=128(LC 23), 19=1718(LC 1)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-3=-163/365, 3-5=-292/548, 5-6=-419/391, 6-7=-799/578, 7-8=-1470/812, 8-9=-1356/733, 9-10=-1040/592, 10-12=-825/549
BOT CHORD 18-19=-1436/666, 5-18=-1069/538, 17-18=-563/246, 16-17=-233/320, 15-16=-532/964, 14-15=-484/1264, 13-14=-493/1065
WEBS 3-20=-265/252, 3-19=-457/500, 5-17=-279/749, 6-17=-689/298, 6-16=-365/864, 7-16=-905/406, 7-15=-332/954, 8-15=-423/691, 9-14=-239/291, 9-13=-451/260, 10-13=-380/801

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Refer to girder(s) for truss to truss connections.
8) Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=369, 1=122, 19=694.
10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard

Continued on page 2

JOAQUIN VELEZ LICENSE No 68182 STATE OF FLORIDA PROFESSIONAL ENGINEER

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6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020



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Tampa, FL 36610

**LOAD CASE(S)** Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-6=-54, 6-8=-54, 8-10=-54, 10-11=-54, 19-21=-20, 15-18=-30(F=-10), 12-15=-30(F=-10)

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. T19289640 |
|---|---|---|---|---|---|
| 2233370 | T16 | Piggyback Base | 2 | 1 | Job Reference (optional) |

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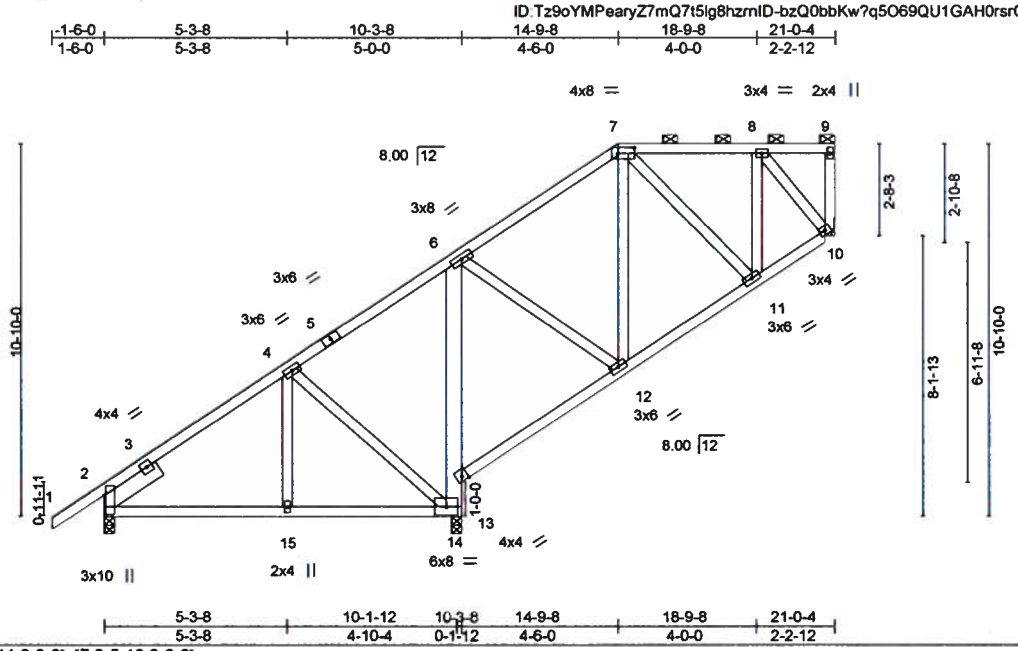



Plate Offsets (X,Y)— [2:0-6-11,0-0-8], [7:0-5-12,0-2-0]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.30 | Vert(LL) | 0.04 | 15-18 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.26 | Vert(CT) | -0.04 | 12-13 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.30 | Horz(CT) | -0.02 | 10 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 147 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
6-14: 2x6 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x6 SP No.2 1-11-8

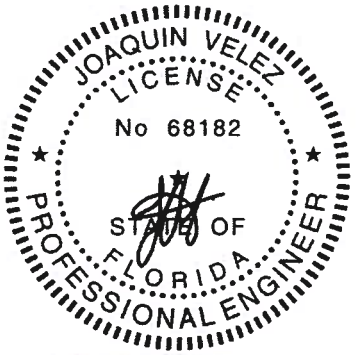
**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 7-9.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS.** (lb/size) 2=438/0-3-8, 14=861/0-3-8, 10=435/Mechanical
Max Horz 2=523(LC 12)
Max Uplift 2=-121(LC 9), 14=-376(LC 12), 10=-271(LC 9)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-4=-274/215, 6-7=-306/191
BOT CHORD 2-15=-549/422, 14-15=-549/422, 13-14=-612/278, 6-13=-538/367, 11-12=-260/326, 10-11=-223/265
WEBS 4-15=-302/214, 4-14=-404/533, 8-10=-320/278

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=121, 14=376, 10=271.
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-7=-54, 7-9=-54, 14-16=-20, 10-13=-30(F=-10)

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Date:
January 31,2020



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Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | T17 | Piggyback Base | 1 | 1 | T19289641 |
| Builders FirstSource, Jacksonville, FL - 32244, | | | | | Job Reference (optional) |

8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:50 2020 Page 1
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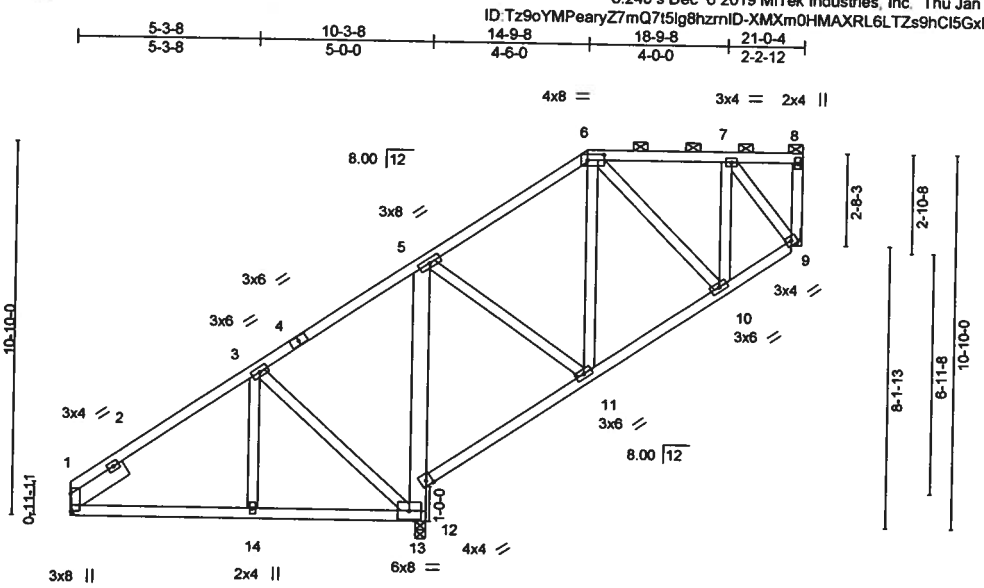


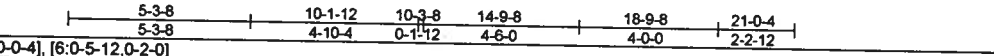




Plate Offsets (X,Y)— [1:0-5-15,0-0-4], [6:0-5-12,0-2-0]

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.30 | Vert(LL) | 0.04 | 14-17 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.26 | Vert(CT) | -0.04 | 11-12 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.30 | Horz(CT) | -0.03 | 9 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MS | | | | | | | Weight: 144 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
5-13: 2x6 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x6 SP No.2 1-11-8

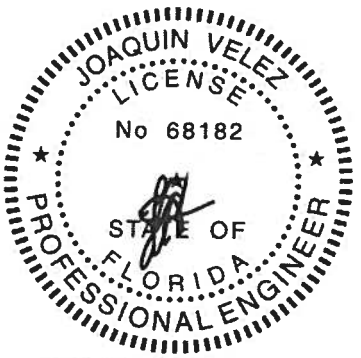
**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-8.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS.** (lb/size) 1=365/Mechanical, 13=841/0-3-8, 9=447/Mechanical
Max Horz 1=481(LC 12)
Max Uplift 1=-104(LC 9), 13=-366(LC 12), 9=-273(LC 9)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-3=-304/228, 5-6=-320/197
BOT CHORD 1-14=-561/416, 13-14=-561/416, 12-13=-585/252, 5-12=-525/355, 10-11=-266/323, 9-10=-226/273
WEBS 3-14=-304/216, 3-13=-401/539, 7-9=-330/283

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=104, 13=366, 9=273.
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-54, 6-8=-54, 13-15=-20, 9-12=-30(F=-10)

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Date:
January 31,2020

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.**
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ***ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information*** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. | T19289642 |
|---|---|---|---|---|---|---|
| 2233370 | TG01 | FLAT GIRDER | 1 | 1 | Job Reference (optional) | |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:50 2020 Page 1
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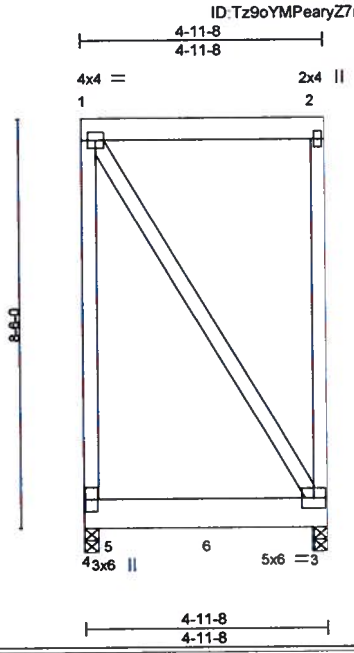


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| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.16 | Vert(LL) | -0.03 | 3-4 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.33 | Vert(CT) | -0.05 | 3-4 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.00 | Horz(CT) | -0.00 | 3 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MP | | | | | | | Weight: 62 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x6 SP No.2
BOT CHORD 2x8 SP 2400F 2.0E
WEBS 2x4 SP No.3

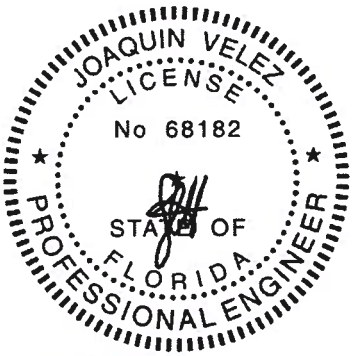
**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 4-11-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 4=1254/0-3-8, 3=1450/0-3-8
Max Uplift 4=-442(LC 4), 3=-513(LC 4)
Max Grav 4=1371(LC 2), 3=1565(LC 2)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
2) Provide adequate drainage to prevent water ponding.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=442, 3=513.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 848 lb down and 273 lb up at 0-7-4, and 851 lb down and 278 lb up at 2-7-4, and 865 lb down and 277 lb up at 4-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 3-4=-20
Concentrated Loads (lb)
Vert: 3=-788(B) 5=-788(B) 6=-783(B)

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6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | TG02 | FLAT GIRDER | 3 | 1 | T19289643 |
| | | | | | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244,

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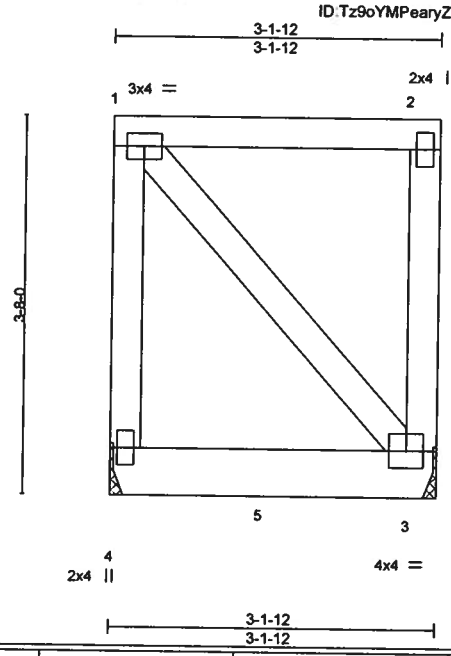



Scale = 1:21.5

| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.14 | Vert(LL) | 0.01 | 3-4 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.34 | Vert(CT) | -0.01 | 3-4 | >999 | 180 | | |
| BCLL | 0.0 * | Rep Stress Incr | NO | WB | 0.00 | Horz(CT) | -0.00 | 3 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-MP | | | | | | | Weight: 26 lb | FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

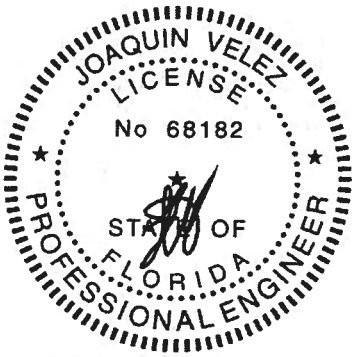
**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 3-1-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 4=319/Mechanical, 3=319/Mechanical
Max Uplift 4=-188(LC 4), 3=-189(LC 4)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
2) Provide adequate drainage to prevent water ponding.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=188, 3=189.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 427 lb down and 293 lb up at 1-6-14 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 3-4=-20
Concentrated Loads (lb)
Vert: 5=-427(B)




Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020



MiTek
6904 Parke East Blvd.
Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. | T19289644 |
|---|---|---|---|---|---|---|
| 2233370 | V01 | Valley | 1 | 1 | Job Reference (optional) | |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:52 2020 Page 1
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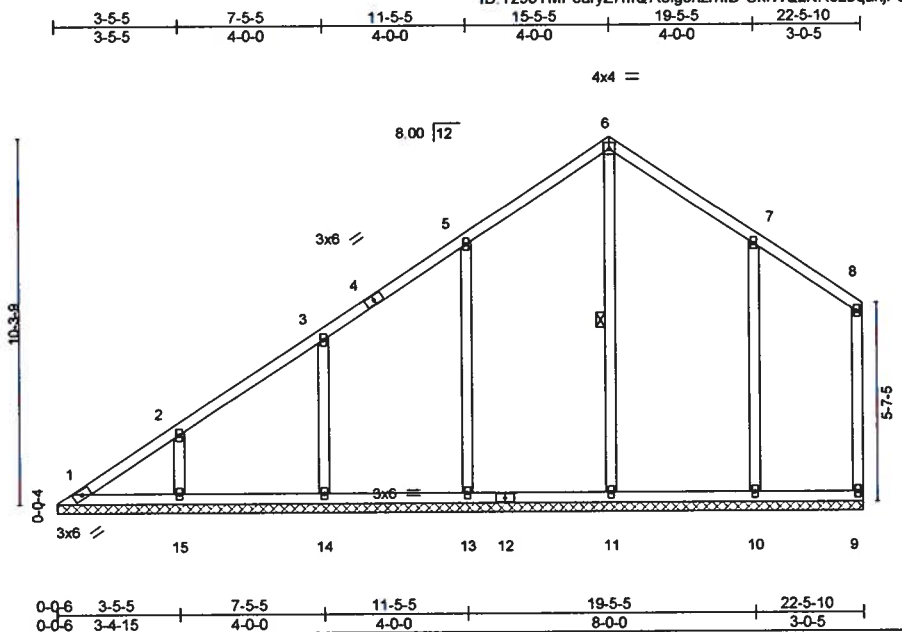



| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.17 | Vert(LL) | n/a | - | n/a | 999 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.17 | Vert(CT) | n/a | - | n/a | 999 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.29 | Horz(CT) | 0.00 | 9 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-S | | | | | | | Weight: 125 lb | FT = 20% |

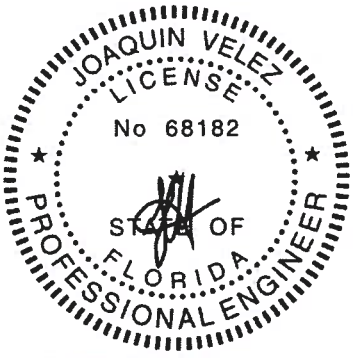
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-11

**REACTIONS.** All bearings 22-5-4.
(lb) - Max Horz 1=359(LC 12)
Max Uplift All uplift 100 lb or less at joint(s) 9, 1, 11 except 13=-241(LC 12), 14=-222(LC 12), 15=-217(LC 12), 10=-230(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 9, 1 except 11=428(LC 19), 13=478(LC 19), 14=391(LC 19), 15=313(LC 19), 10=447(LC 20)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-391/283, 2-3=-250/230
WEBS 5-13=-290/264, 3-14=-277/247, 2-15=-263/234, 7-10=-276/252

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are 2x4 MT20 unless otherwise indicated.
4) Gable requires continuous bottom chord bearing.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 1, 11 except (jt=lb) 13=241, 14=222, 15=217, 10=230.




Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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January 31,2020

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES | T19289645 |
|---|---|---|---|---|---|---|
| 2233370 | V02 | Valley | 1 | 1 | Job Reference (optional) | |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:53 2020 Page 1
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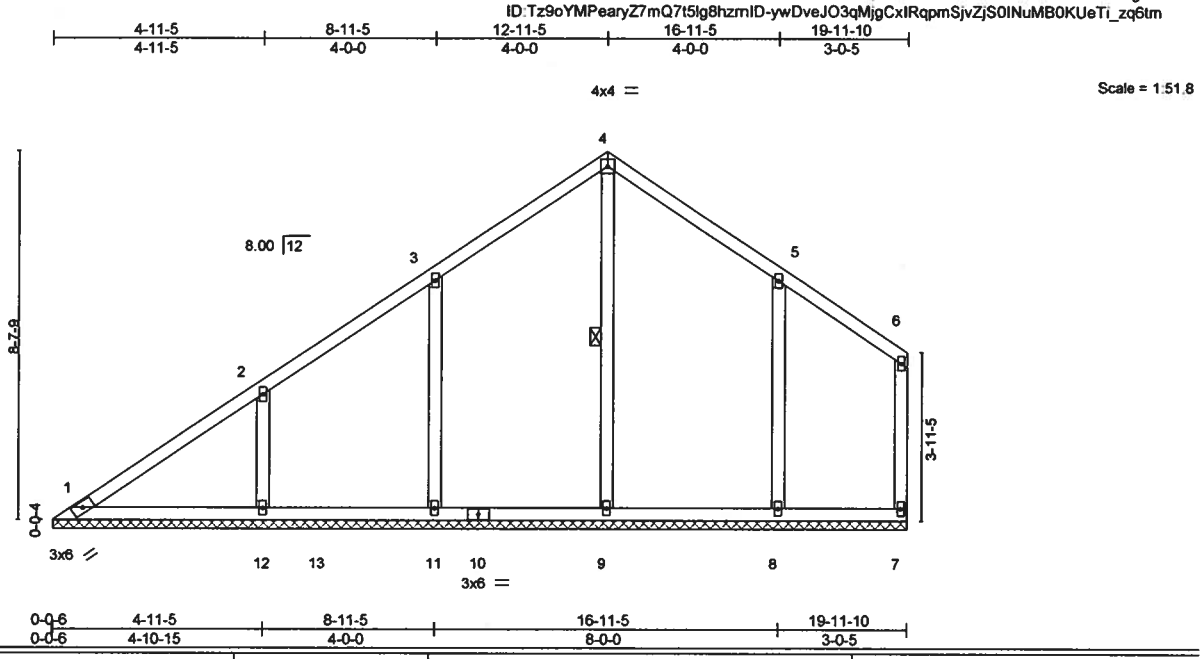



| LOADING (psf) | SPACING- 2-0-0 | CSI. | DEFL. in (loc) l/defl L/d | PLATES GRIP |
|---|---|---|---|---|
| TCLL 20.0 | Plate Grip DOL 1.25 | TC 0.22 | Vert(LL) n/a - n/a 999 | MT20 244/190 |
| TCDL 7.0 | Lumber DOL 1.25 | BC 0.17 | Vert(CT) n/a - n/a 999 | |
| BCLL 0.0 * | Rep Stress Incr YES | WB 0.17 | Horz(CT) 0.00 7 n/a n/a | |
| BCDL 10.0 | Code FBC2017/TPI2014 | Matrix-S | | Weight: 102 lb FT = 20% |

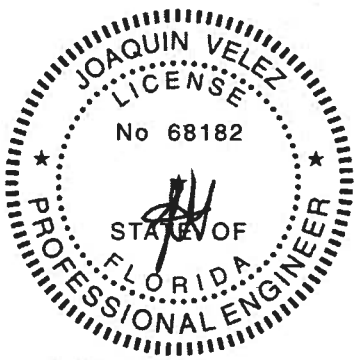
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-9

**REACTIONS.** All bearings 19-11-4.
(lb) - Max Horz 1=277(LC 12)
Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 9 except 11=-227(LC 12), 12=-269(LC 12), 8=-231(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 9=432(LC 19), 11=452(LC 19), 12=423(LC 19), 8=445(LC 20)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-265/241
WEBS 3-11=-277/254, 2-12=-323/285, 5-8=-276/252

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are 2x4 MT20 unless otherwise indicated.
4) Gable requires continuous bottom chord bearing.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 9 except (jt=lb) 11=227, 12=269, 8=231.




Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | V03 | Valley | 1 | 1 | T19289646 |
| | | | | | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:54 2020 Page 1
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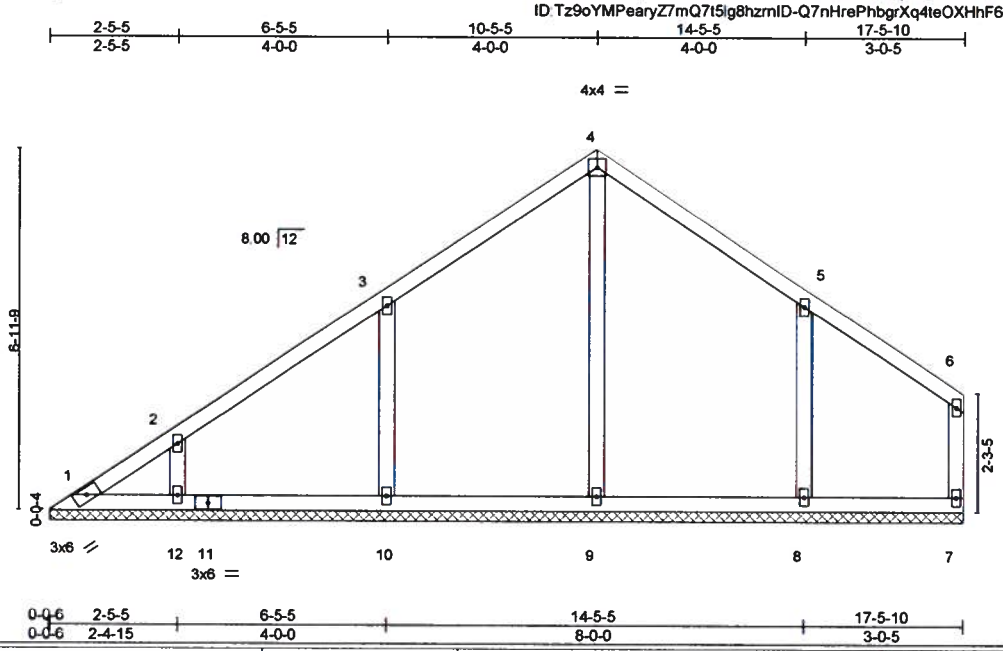



| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.18 | Vert(LL) | n/a | - | n/a | 999 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.19 | Vert(CT) | n/a | - | n/a | 999 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.18 | Horz(CT) | 0.00 | 7 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-S | | | | | | | Weight: 81 lb | FT = 20% |

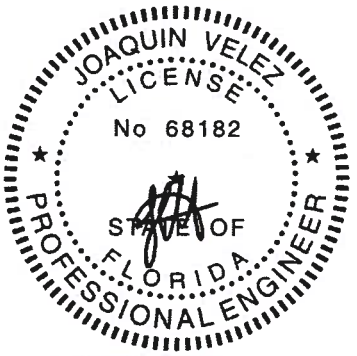
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** All bearings 17-5-4.
(lb) - Max Horz 1=200(LC 9)
Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 9 except 10=-249(LC 12), 12=-185(LC 12), 8=-234(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 9=436(LC 19), 10=421(LC 19), 12=267(LC 19), 8=395(LC 20)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 3-10=-300/273, 5-8=-277/254

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are 2x4 MT20 unless otherwise indicated.
4) Gable requires continuous bottom chord bearing.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 9 except (jt=lb) 10=249, 12=185, 8=234.




Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
January 31,2020



MiTek
6904 Parke East Blvd.
Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. |
|---|---|---|---|---|---|
| 2233370 | V04 | Valley | 1 | 1 | T19289647 |
| | | | | | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:55 2020 Page 1
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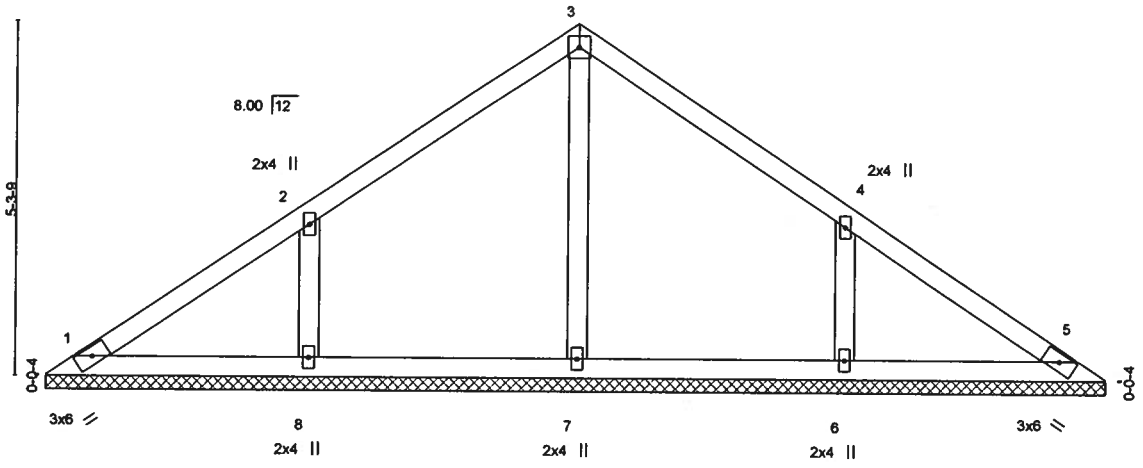
3-11-5 | 7-11-5 | 11-11-5 | 15-10-10
3-11-5 | 4-0-0 | 4-0-0 | 3-11-5

4x4 =

Scale = 1:33.1




0-0-6 | 3-11-5 | 11-11-5 | 15-10-10
0-0-6 | 3-10-15 | 8-0-0 | 3-11-5

| LOADING (psf) | SPACING- 2-0-0 | CSI. | DEFL. in (loc) l/defl L/d | PLATES GRIP |
|---|---|---|---|---|
| TCLL 20.0 | Plate Grip DOL 1.25 | TC 0.17 | Vert(LL) n/a - n/a 999 | MT20 244/190 |
| TCDL 7.0 | Lumber DOL 1.25 | BC 0.11 | Vert(CT) n/a - n/a 999 | |
| BCLL 0.0 * | Rep Stress Incr YES | WB 0.08 | Horz(CT) 0.00 5 n/a n/a | |
| BCDL 10.0 | Code FBC2017/TPI2014 | Matrix-S | | Weight: 63 lb FT = 20% |

**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** All bearings 15-9-14.
(lb) - Max Horz 1=-155(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-252(LC 12), 6=-251(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=366(LC 19), 6=366(LC 20)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-8=-300/269, 4-6=-300/269

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=252, 6=251.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January 31,2020



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Tampa, FL 36610

| Job | Truss | Truss Type | Qty | Ply | AARON SIMQUE - KARLTON RES. T19289648 |
|---|---|---|---|---|---|
| 2233370 | V05 | Valley | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Thu Jan 30 16:02:56 2020 Page 1
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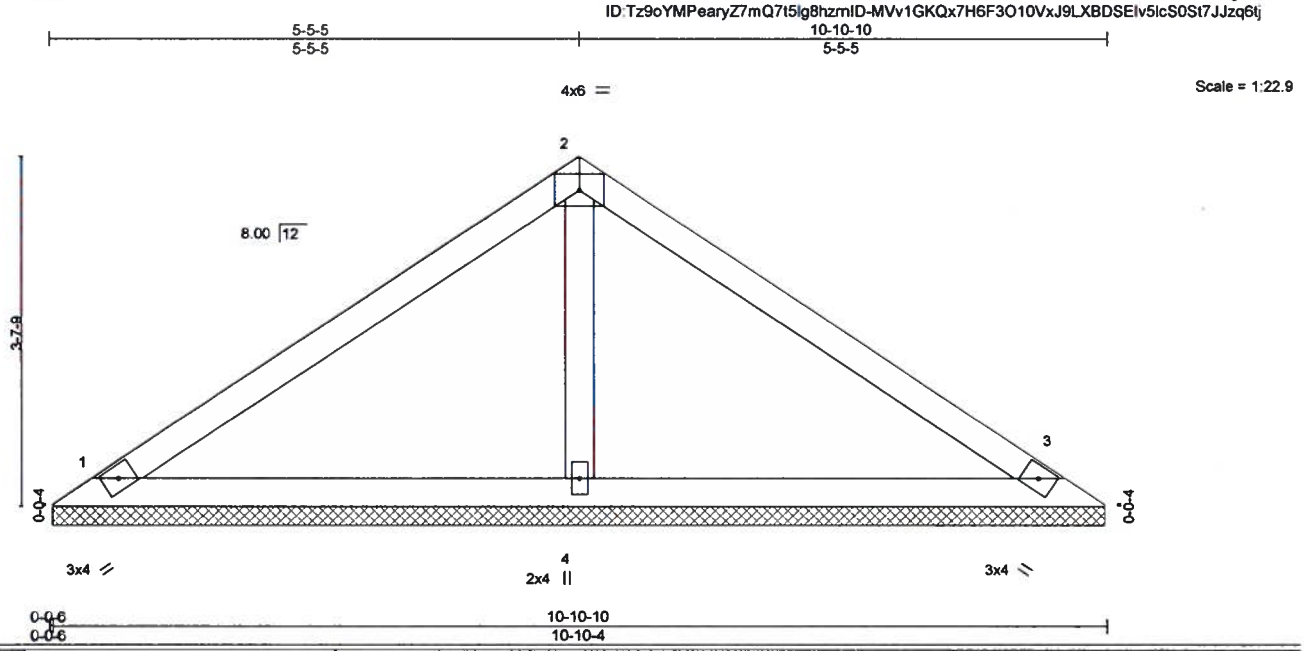



| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plate Grip DOL | 1.25 | TC | 0.30 | Vert(LL) | n/a | - | n/a | 999 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber DOL | 1.25 | BC | 0.24 | Vert(CT) | n/a | - | n/a | 999 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.06 | Horz(CT) | 0.00 | 3 | n/a | n/a | | |
| BCDL | 10.0 | Code FBC2017/TPI2014 | | Matrix-S | | | | | | | Weight: 38 lb | FT = 20% |

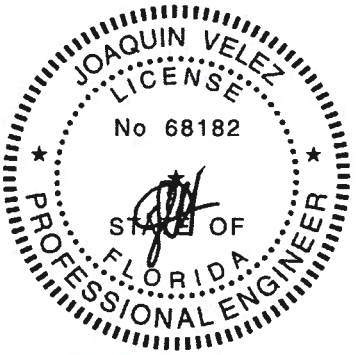
**LUMBER-**
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

**BRACING-**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 1=180/10-9-14, 3=180/10-9-14, 4=375/10-9-14
Max Horz 1=-103(LC 8)
Max Uplift 1=-81(LC 12), 3=-94(LC 13), 4=-103(LC 12)
Max Grav 1=180(LC 1), 3=181(LC 20), 4=375(LC 1)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3 except (jt=lb) 4=103.

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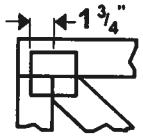
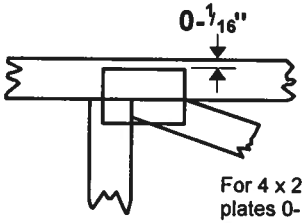
# Symbols

## PLATE LOCATION AND ORIENTATION




Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.




For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*** Plate location details available in MiTek 20/20 software or upon request.**

## PLATE SIZE

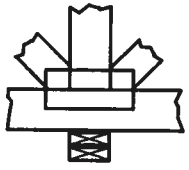
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

## BEARING

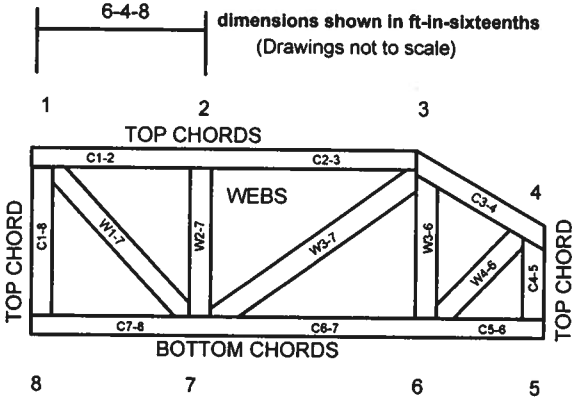


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

**Industry Standards:**

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

# Numbering System




**JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.**

**CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.**

## PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.



MiTek

MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

# General Safety Notes

## Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

| AUGUST 1, 2016 | T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY | MII-T-BRACE 2 |
|---|---|---|



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Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

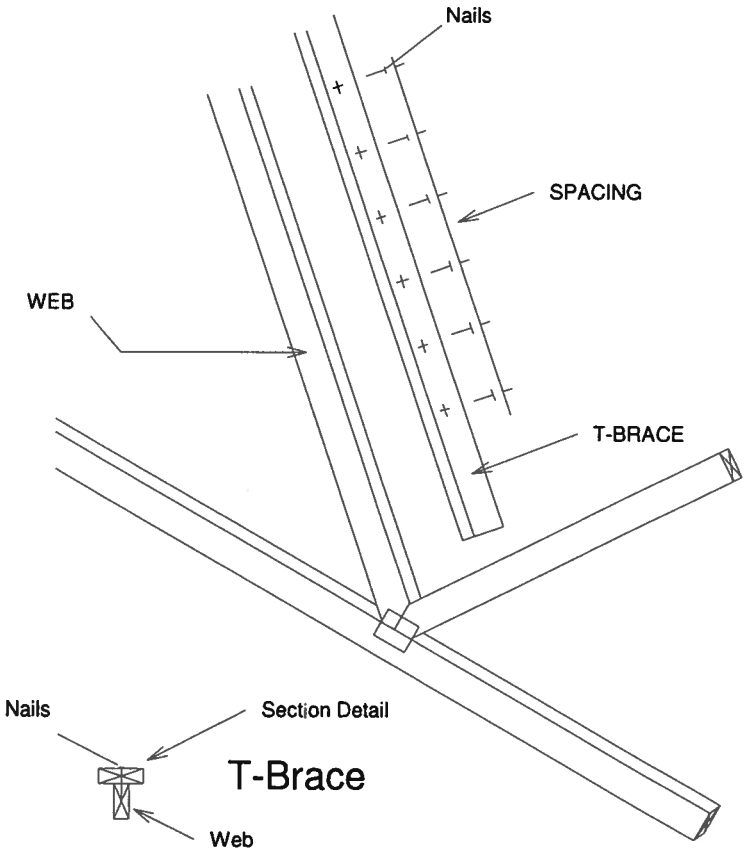
Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

| Nailing Pattern | | |
|---|---|---|
| T-Brace size | Nail Size | Nail Spacing |
| 2x4 or 2x6 or 2x8 | 10d (0.131" X 3") | 6" o.c. |
| Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies) | | |

| | Brace Size for One-Ply Truss | |
|---|---|---|
| | Specified Continuous Rows of Lateral Bracing | |
| Web Size | 1 | 2 |
| 2x3 or 2x4 | 2x4 T-Brace | 2x4 I-Brace |
| 2x6 | 2x6 T-Brace | 2x6 I-Brace |
| 2x8 | 2x8 T-Brace | 2x8 I-Brace |

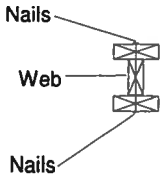
| | Brace Size for Two-Ply Truss | |
|---|---|---|
| | Specified Continuous Rows of Lateral Bracing | |
| Web Size | 1 | 2 |
| 2x3 or 2x4 | 2x4 T-Brace | 2x4 I-Brace |
| 2x6 | 2x6 T-Brace | 2x6 I-Brace |
| 2x8 | 2x8 T-Brace | 2x8 I-Brace |

T-Brace / I-Brace must be same species and grade (or better) as web member.

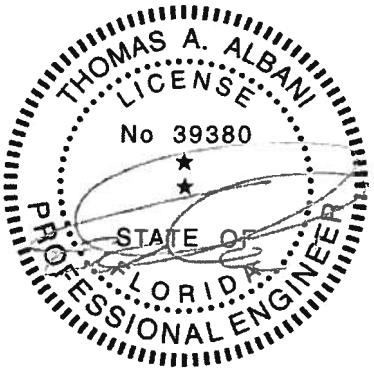









I-Brace

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Date:
**February 12, 2018**

| AUGUST 1, 2016 | SCAB-BRACE DETAIL | MII-SCAB-BRACE |
|---|---|---|

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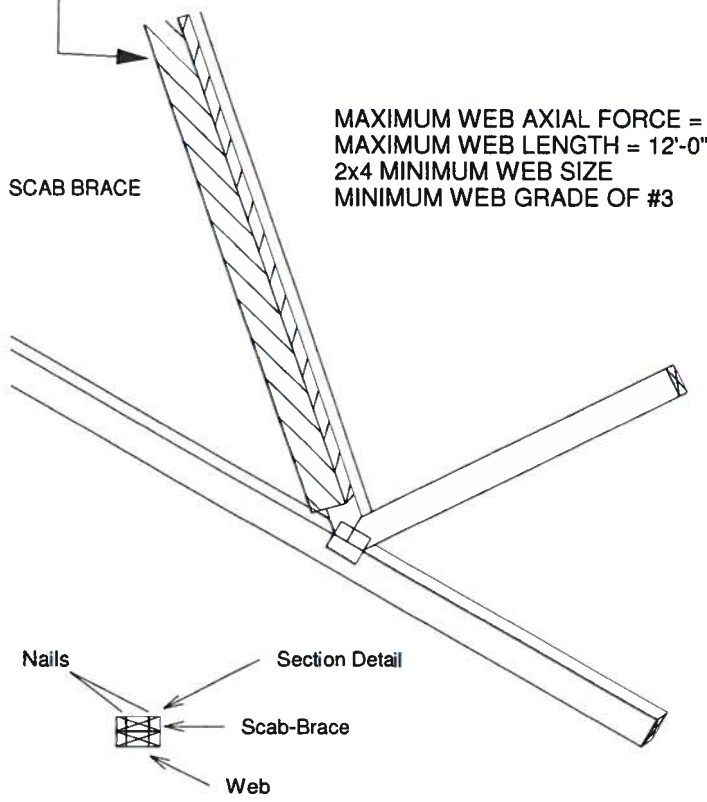
Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical.
Scab must cover full length of web +/- 6".

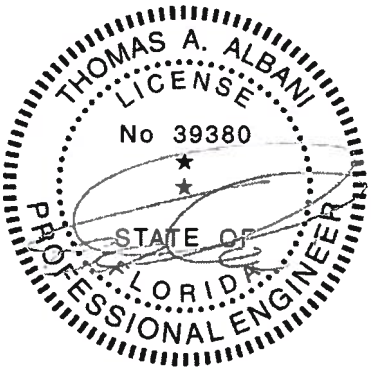
*** THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x___ SCAB TO ONE FACE OF WEB WITH
2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
SCAB MUST BE THE SAME GRADE, SIZE AND
SPECIES (OR BETTER) AS THE WEB.




MAXIMUM WEB AXIAL FORCE = 2500 lbs
MAXIMUM WEB LENGTH = 12'-0"
2x4 MINIMUM WEB SIZE
MINIMUM WEB GRADE OF #3

Scab-Brace must be same species grade (or better) as web member.

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**February 12, 2018**

| AUGUST 1, 2016 | STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL) | MII-REP05 |
|---|---|---|



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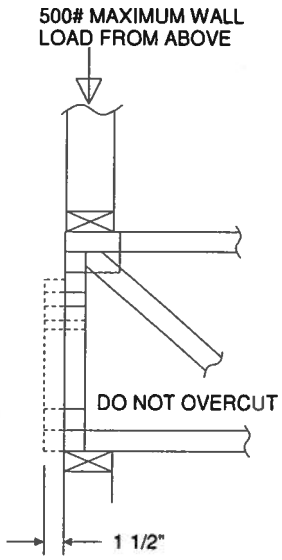
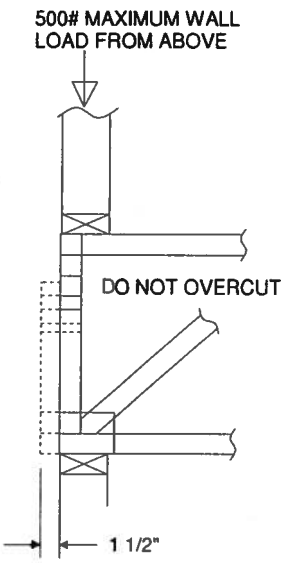
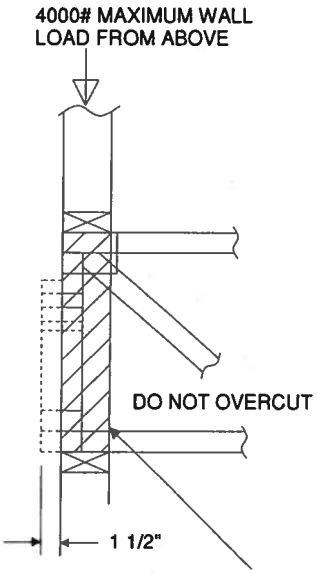
1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.
5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X_ ORIENTATION ONLY.
6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.



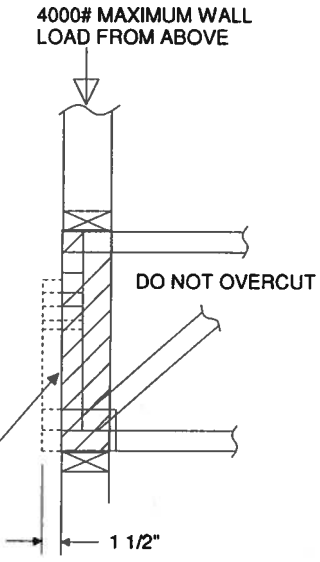

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES




TRUSSES BUILT WITH 4x2 MEMBERS

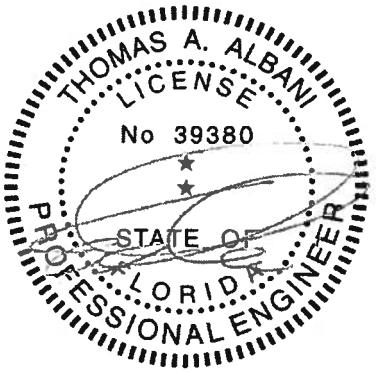



REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES




TRUSSES BUILT WITH 4x2 MEMBERS

ATTACH 2x4 SQUASH BLOCK (CUT TO FIT TIGHTLY) TO BOTH SIDES OF THE TRUSS AS SHOWN WITH 10d (0.131" X 3") NAILS SPACED 3" O.C.

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**February 12, 2018**

| AUGUST 1, 2016 | Standard Gable End Detail | MII-GE130-D-SP |
|---|---|---|

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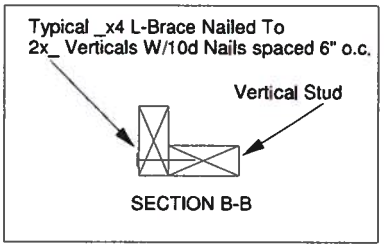
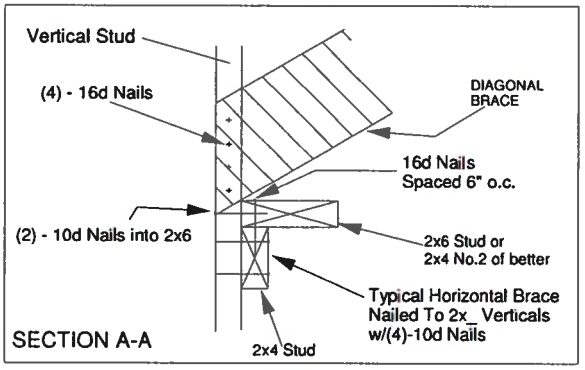
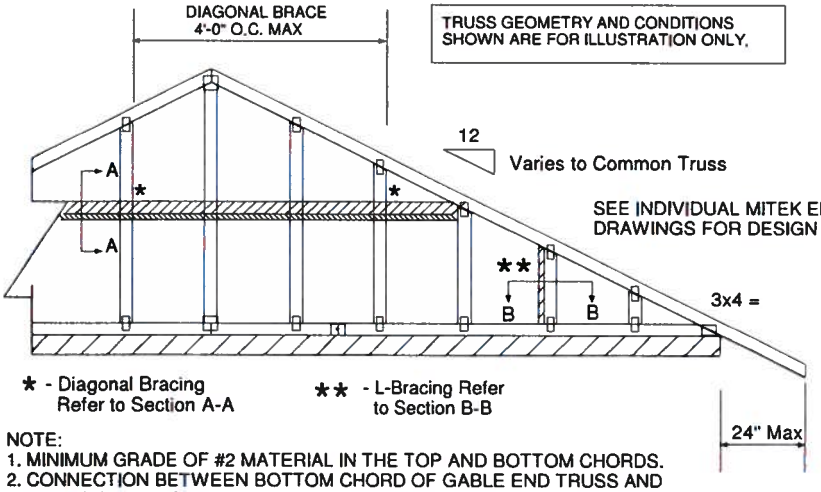


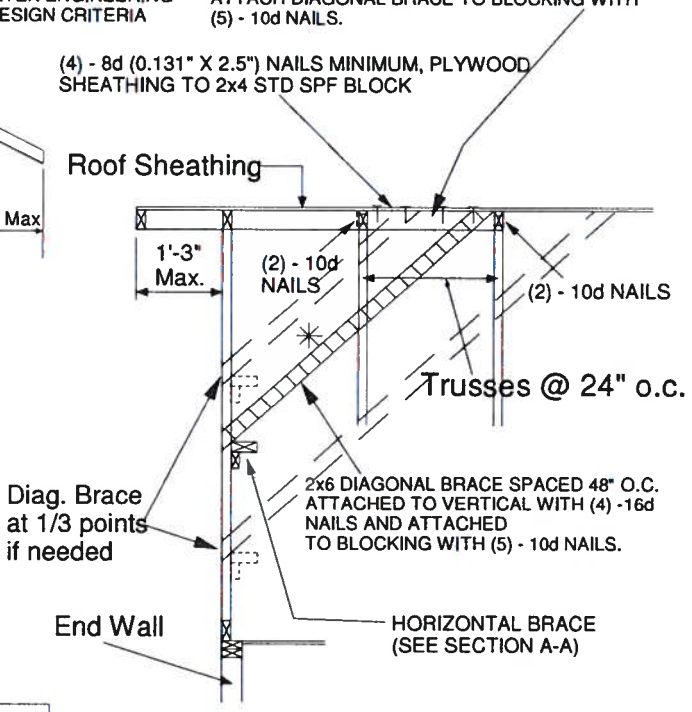




* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.



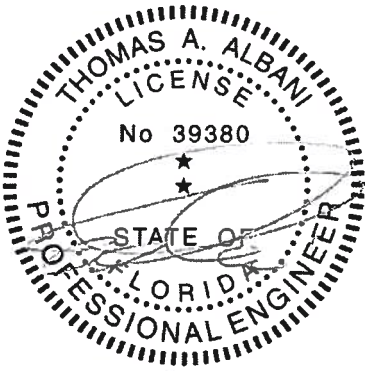

NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

| Minimum Stud Size Species and Grade | Stud Spacing | Without Brace | 1x4 L-Brace | 2x4 L-Brace | DIAGONAL BRACE | 2 DIAGONAL BRACES AT 1/3 POINTS |
|---|---|---|---|---|---|---|
| | | Maximum Stud Length | | | | |
| 2x4 SP No. 3 / Stud | 12" O.C. | 3-9-13 | 4-1-1 | 5-9-6 | 7-1-3 | 11-5-7 |
| 2x4 SP No. 3 / Stud | 16" O.C. | 3-5-4 | 3-6-8 | 5-0-2 | 6-10-8 | 10-3-13 |
| 2x4 SP No. 3 / Stud | 24" O.C. | 2-9-11 | 2-10-11 | 4-1-1 | 5-7-6 | 8-5-1 |

✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE D
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

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Date:
**February 12, 2018**

| AUGUST 1, 2016 | Standard Gable End Detail | MII-GE130-SP |
|---|---|---|

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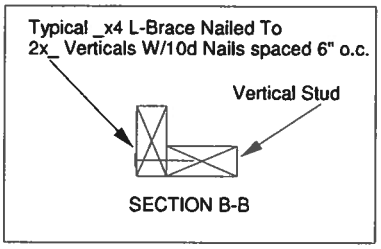
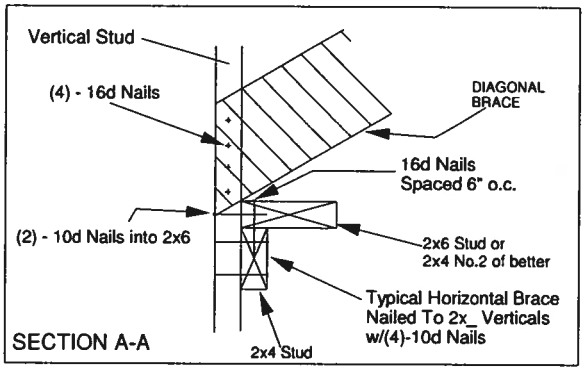
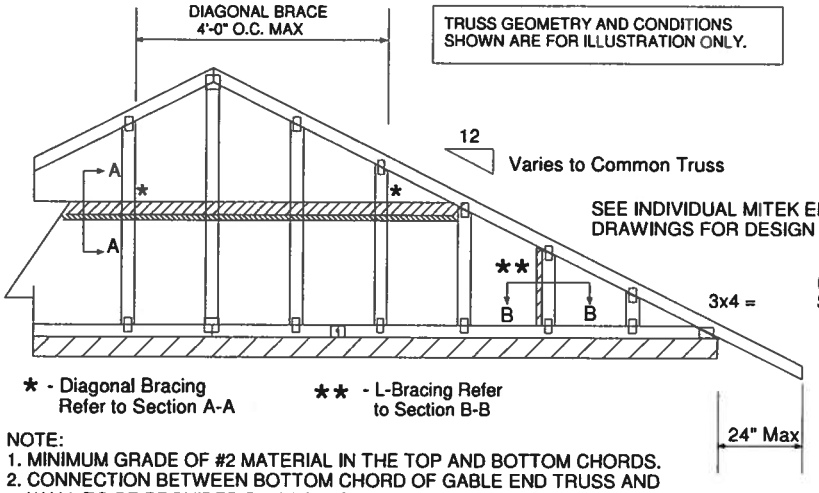


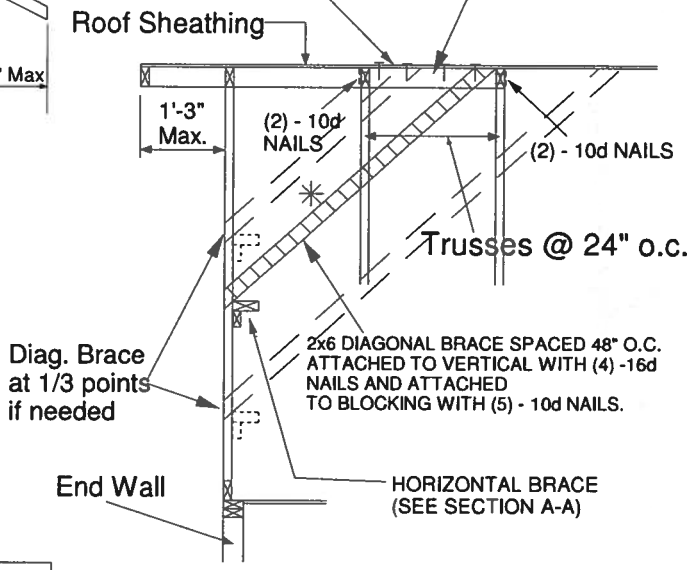




TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK




* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B

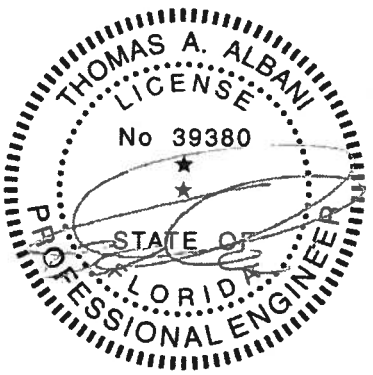
NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

| Minimum Stud Size Species and Grade | Stud Spacing | Without Brace | 1x4 L-Brace | 2x4 L-Brace | DIAGONAL BRACE | 2 DIAGONAL BRACES AT 1/3 POINTS |
|---|---|---|---|---|---|---|
| | | Maximum Stud Length | | | | |
| 2x4 SP No. 3 / Stud | 12" O.C. | 4-0-7 | 4-5-6 | 6-3-8 | 8-0-15 | 12-1-6 |
| 2x4 SP No. 3 / Stud | 16" O.C. | 3-8-0 | 3-10-4 | 5-5-6 | 7-4-1 | 11-0-1 |
| 2x4 SP No. 3 / Stud | 24" O.C. | 3-0-10 | 3-1-12 | 4-5-6 | 6-1-5 | 9-1-15 |

✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

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**February 12, 2018**

| JANUARY 6, 2017 | Standard Gable End Detail | MII-GE140-001 |
|---|---|---|

MiTek USA, Inc. Page 1 of 2

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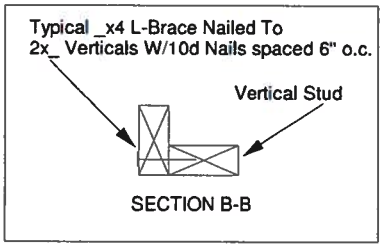
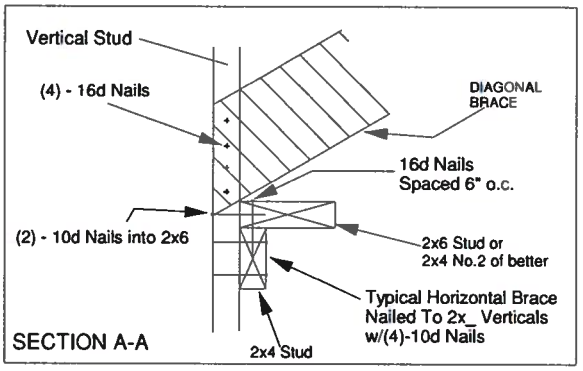
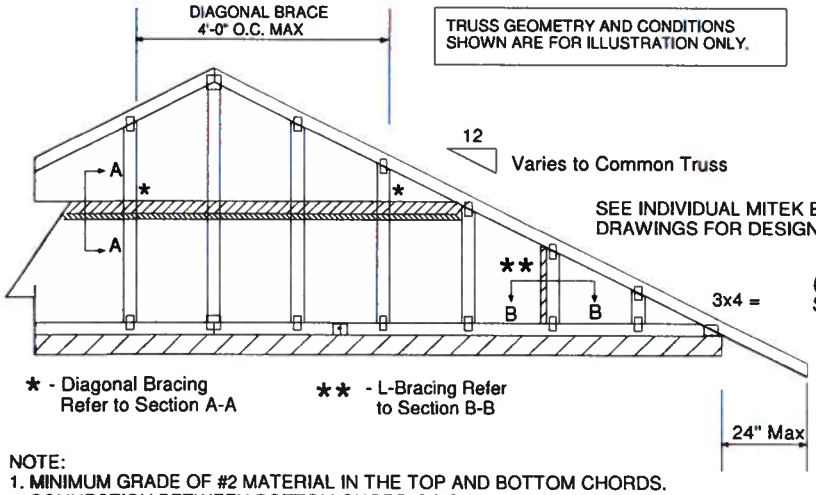



TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

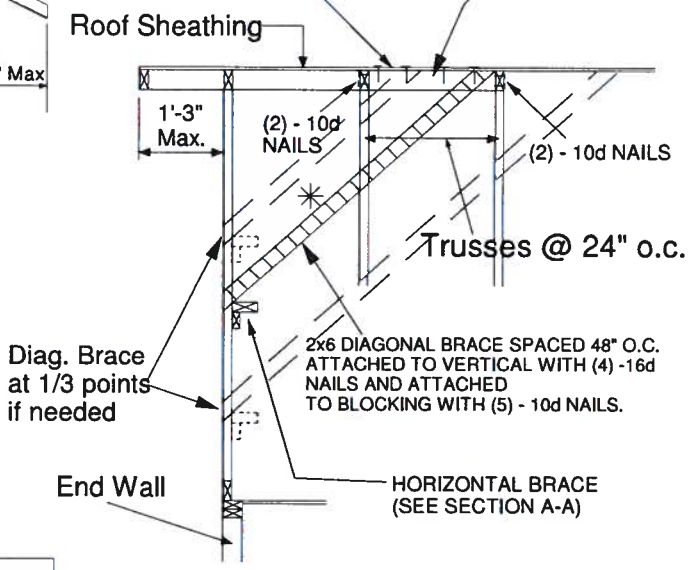
SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD DF/SPF BLOCK



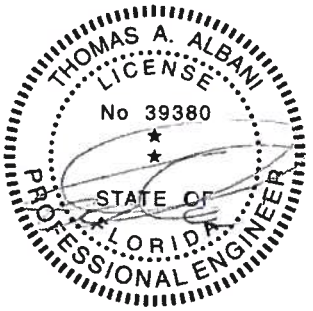

NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

| Minimum Stud Size Species and Grade | Stud Spacing | Without Brace | 1x4 L-Brace | 2x4 L-Brace | DIAGONAL BRACE | 2 DIAGONAL BRACES AT 1/3 POINTS |
|---|---|---|---|---|---|---|
| | | Maximum Stud Length | | | | |
| 2x4 DF/SPF Std/Stud | 12" O.C. | 3-10-1 | 3-11-7 | 5-7-2 | 7-8-2 | 11-6-4 |
| 2x4 DF/SPF Std/Stud | 16" O.C. | 3-3-14 | 3-5-1 | 4-10-2 | 6-7-13 | 9-11-11 |
| 2x4 DF/SPF Std/Stud | 24" O.C. | 2-8-9 | 2-9-8 | 3-11-7 | 5-5-2 | 8-1-12 |

✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 140 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

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**January 19, 2018**

| AUGUST 1, 2016 | Standard Gable End Detail | MII-GE170-D-SP |
|---|---|---|

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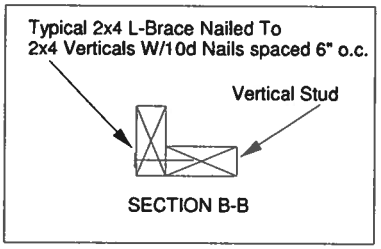
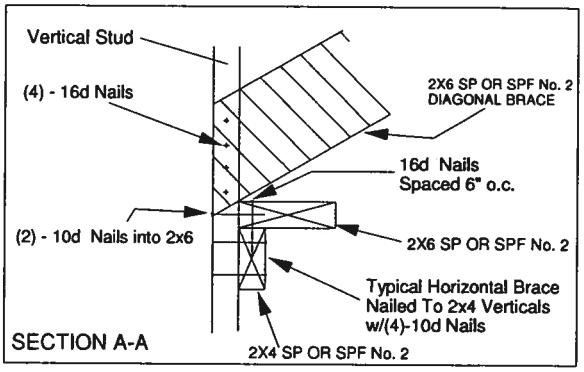
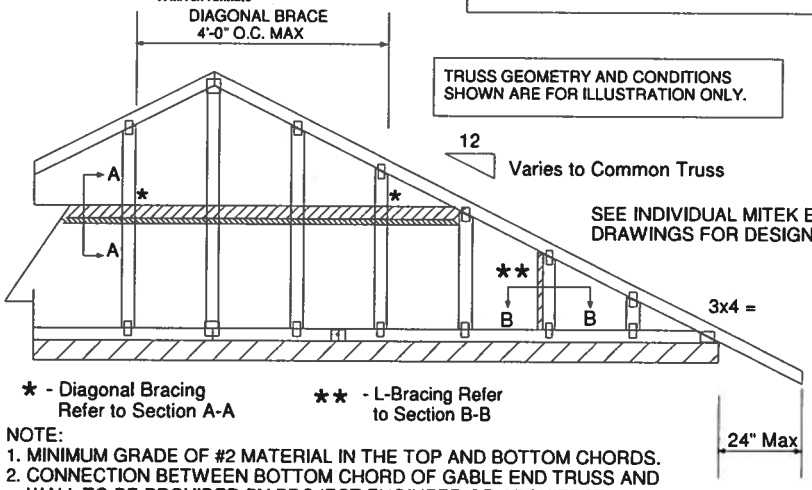



TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B

NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

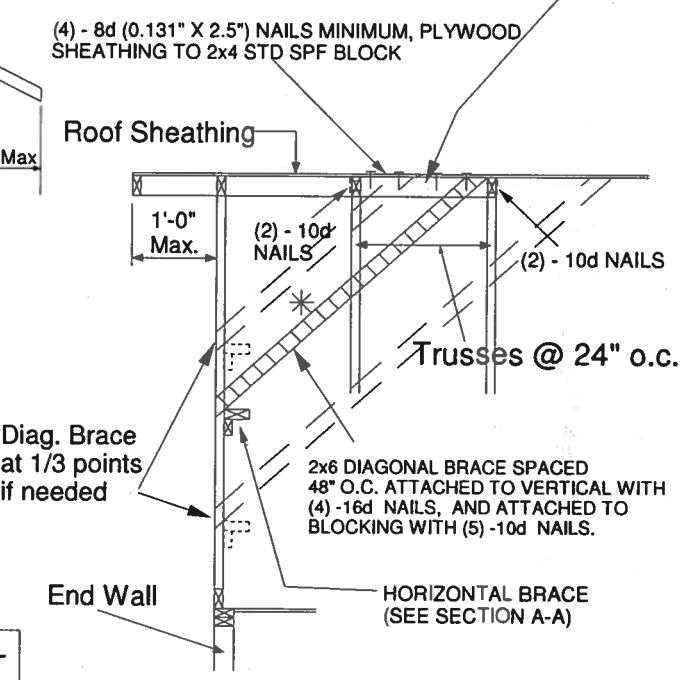
| Minimum Stud Size Species and Grade | Stud Spacing | Without Brace | 2x4 L-Brace | DIAGONAL BRACE | 2 DIAGONAL BRACES AT 1/3 POINTS |
|---|---|---|---|---|---|
| | | Maximum Stud Length | | | |
| 2x4 SP No. 3 / Stud | 12" O.C. | 3-9-7 | 5-8-8 | 6-11-1 | 11-4-4 |
| 2x4 SP No. 3 / Stud | 16" O.C. | 3-4-12 | 4-11-15 | 6-9-8 | 10-2-3 |
| 2x4 SP No. 3 / Stud | 24" O.C. | 2-9-4 | 4-0-7 | 5-6-8 | 8-3-13 |
| 2x4 SP No. 2 | 12" O.C. | 3-11-13 | 5-8-8 | 6-11-1 | 11-11-7 |
| 2x4 SP No. 2 | 16" O.C. | 3-7-7 | 4-11-5 | 6-11-1 | 10-10-5 |
| 2x4 SP No. 2 | 24" O.C. | 3-1-15 | 4-0-7 | 6-3-14 | 9-5-14 |

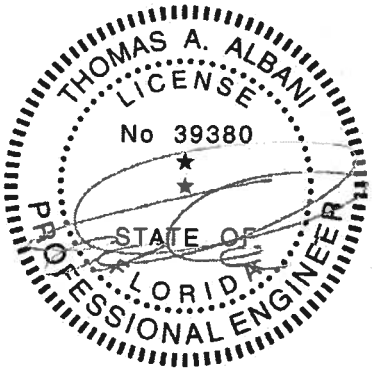
✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET
EXPOSURE D
ASCE 7-10 170 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

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**February 12, 2018**

| AUGUST 1, 2016 | Standard Gable End Detail | MII-GE180-D-SP |
|---|---|---|



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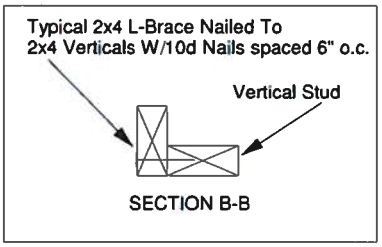
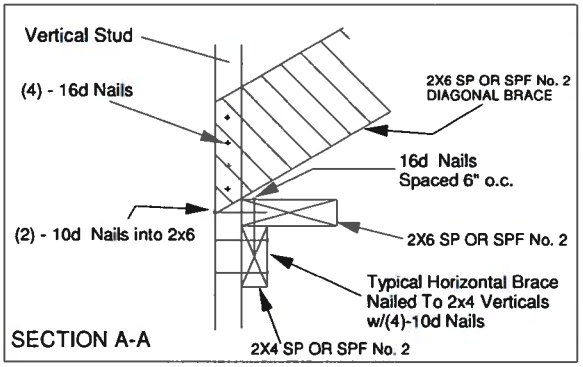
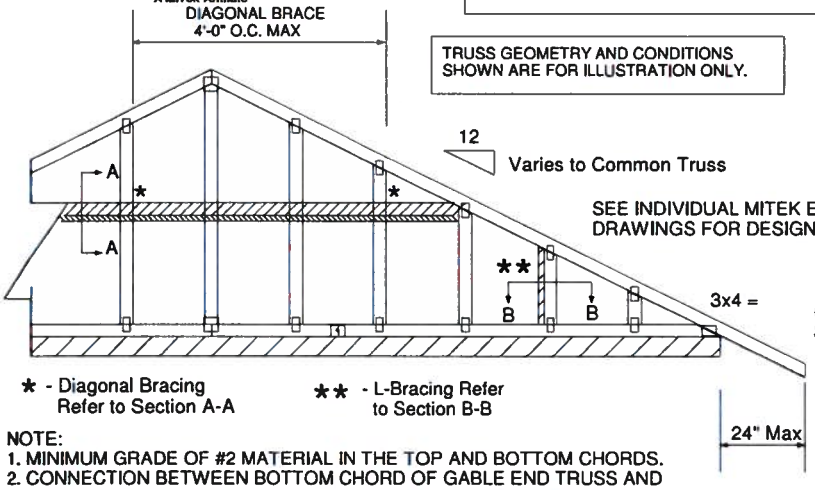



TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

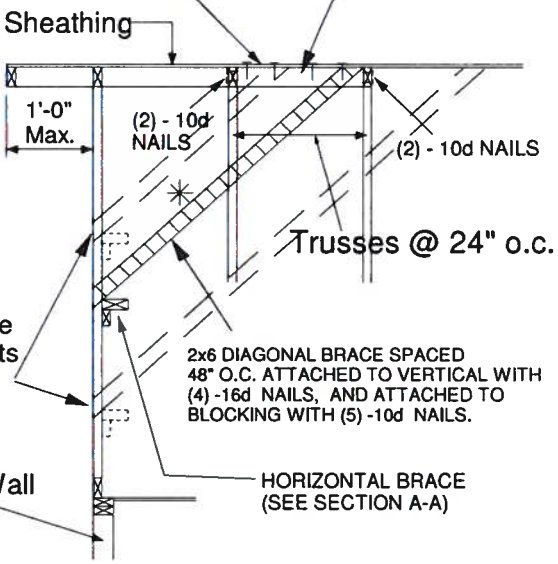
SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK



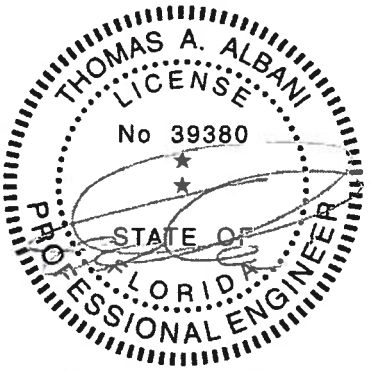

NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

| Minimum Stud Size Species and Grade | Stud Spacing | Without Brace | 2x4 L-Brace | DIAGONAL BRACE | 2 DIAGONAL BRACES AT 1/3 POINTS |
|---|---|---|---|---|---|
| | | Maximum Stud Length | | | |
| 2x4 SP No. 3 / Stud | 12" O.C. | 3-7-12 | 5-4-11 | 6-2-1 | 10-11-3 |
| 2x4 SP No. 3 / Stud | 16" O.C. | 3-2-8 | 4-8-1 | 6-2-1 | 9-7-7 |
| 2x4 SP No. 3 / Stud | 24" O.C. | 2-7-7 | 3-9-12 | 5-2-13 | 7-10-4 |
| 2x4 SP No. 2 | 12" O.C. | 3-10-0 | 5-4-11 | 6-2-1 | 11-6-1 |
| 2x4 SP No. 2 | 16" O.C. | 3-5-13 | 4-8-1 | 6-2-1 | 10-5-7 |
| 2x4 SP No. 2 | 24" O.C. | 3-0-8 | 3-9-12 | 6-1-1 | 9-1-9 |

✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET
EXPOSURE D
ASCE 7-10 180 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

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| AUGUST 1, 2016 | STANDARD PIGGYBACK TRUSS CONNECTION DETAIL | MII-PIGGY-7-10 |
|---|---|---|



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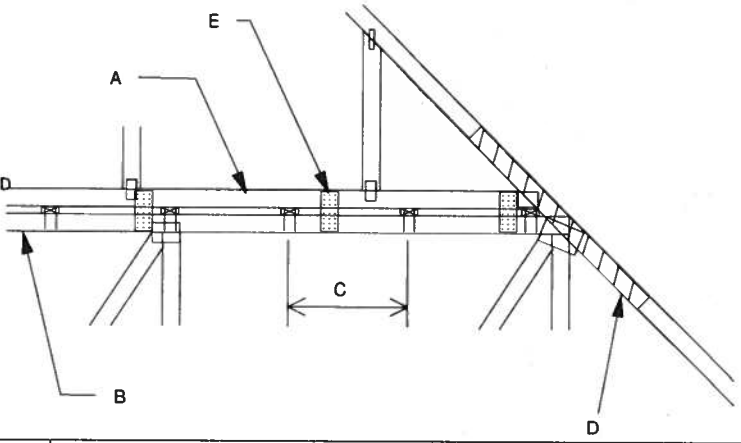
| |
|---|
| MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E<br>MAX MEAN ROOF HEIGHT = 30 FEET<br>MAX TRUSS SPACING = 24 " O.C.<br>CATEGORY II BUILDING<br>EXPOSURE B or C<br>ASCE 7-10<br>DURATION OF LOAD INCREASE : 1.60 |
| DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED. |

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) (0.131" X 3.5") TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
D - 2 X __ X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

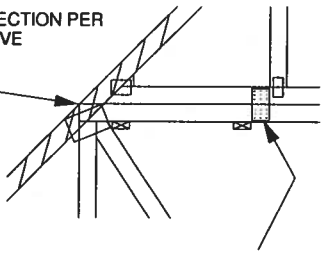
E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



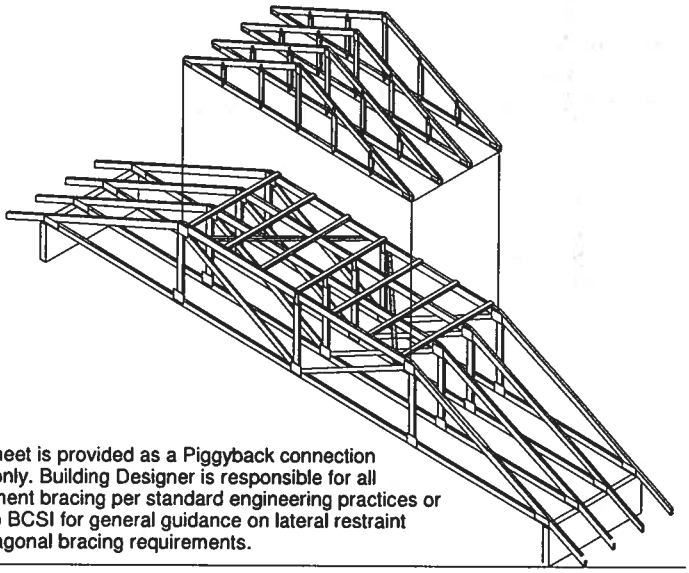

WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER NOTE D ABOVE

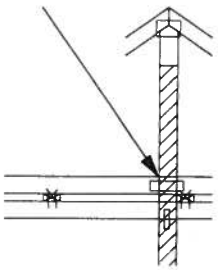


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



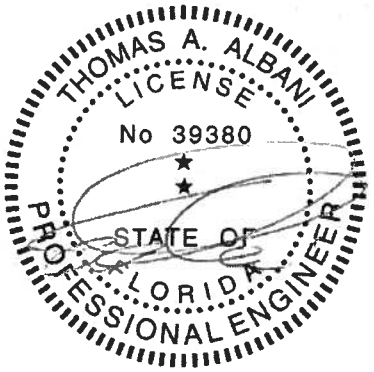
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
2) ATTACH 2 x ___ x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.

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**February 12, 2018**

| AUGUST 1, 2016 | STANDARD PIGGYBACK TRUSS CONNECTION DETAIL | MII-PIGGY-ALT 7-10 |
|---|---|---|

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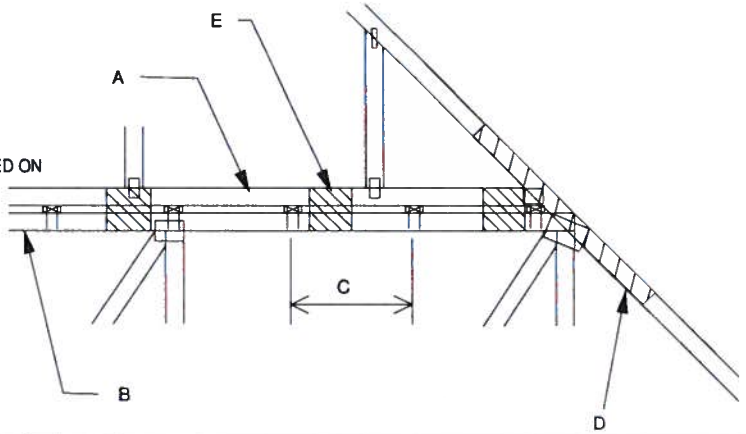
MiTek USA, Inc. Page 1 of 1

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24 " O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE : 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0(0.131" X 3.5") TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
D - 2 X __ X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

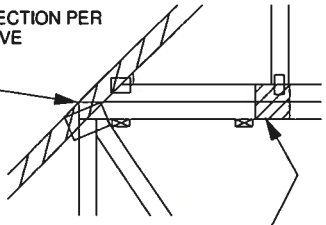
E - FOR WIND SPEED IN THE RANGE 126 MPH - 160 MPH ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



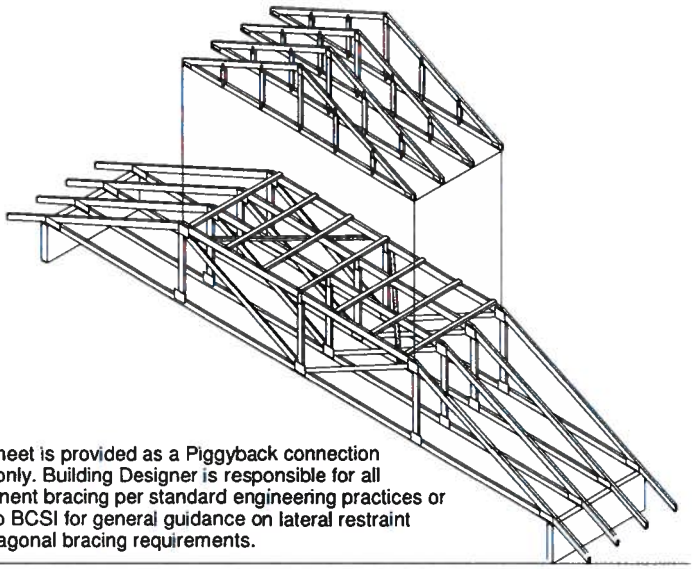

WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER NOTE D ABOVE

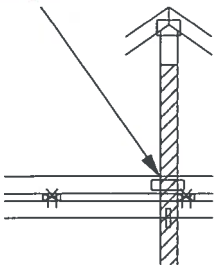


7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
2) ATTACH 2 x ___ x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.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Date:

**January 19, 2018**

| AUGUST 1, 2016 | STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES | MII-REP01A1 |
|---|---|---|



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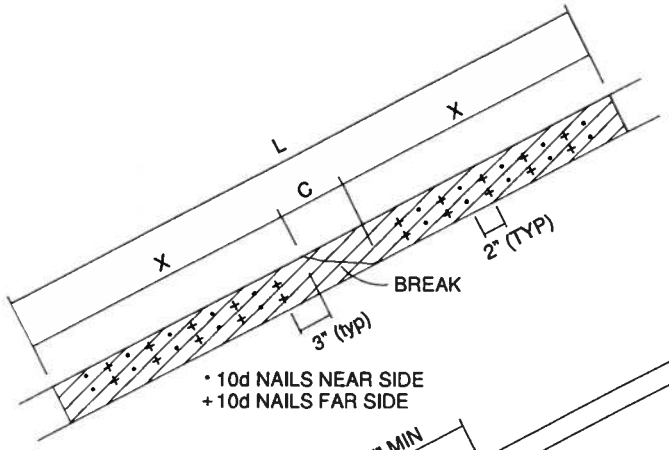
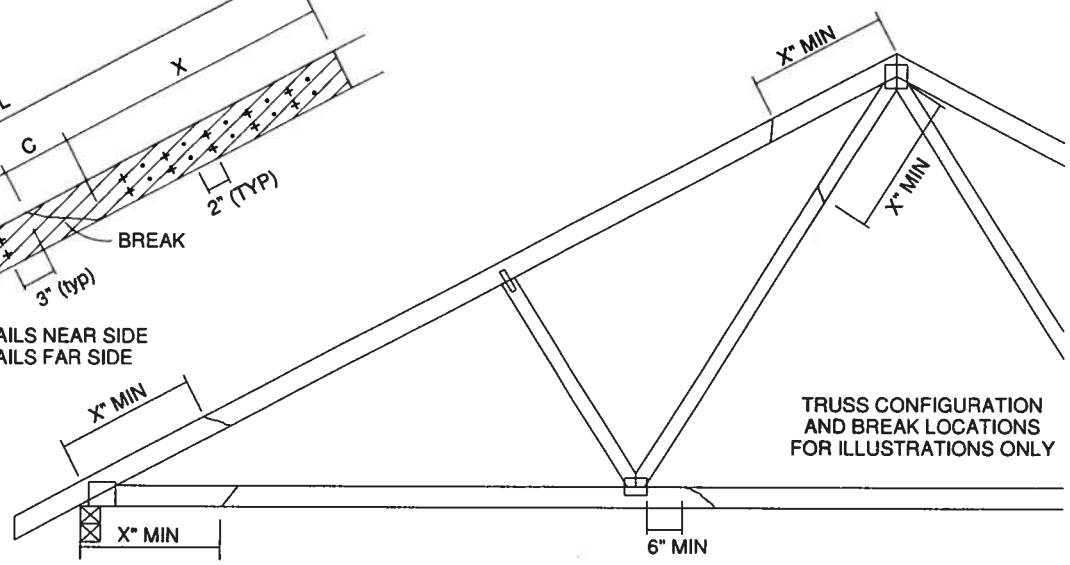
| TOTAL NUMBER OF NAILS EACH SIDE OF BREAK * | | X INCHES | MAXIMUM FORCE (lbs) 15% LOAD DURATION | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|
| | | | SP | | DF | | SPF | | HF | |
| 2x4 | 2x6 | | 2x4 | 2x6 | 2x4 | 2x6 | 2x4 | 2x6 | 2x4 | 2x6 |
| 20 | 30 | 24" | 1706 | 2559 | 1561 | 2342 | 1320 | 1980 | 1352 | 2028 |
| 26 | 39 | 30" | 2194 | 3291 | 2007 | 3011 | 1697 | 2546 | 1738 | 2608 |
| 32 | 48 | 36" | 2681 | 4022 | 2454 | 3681 | 2074 | 3111 | 2125 | 3187 |
| 38 | 57 | 42" | 3169 | 4754 | 2900 | 4350 | 2451 | 3677 | 2511 | 3767 |
| 44 | 66 | 48" | 3657 | 5485 | 3346 | 5019 | 2829 | 4243 | 2898 | 4347 |

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x_ SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS)
THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:
L = (2) X + C

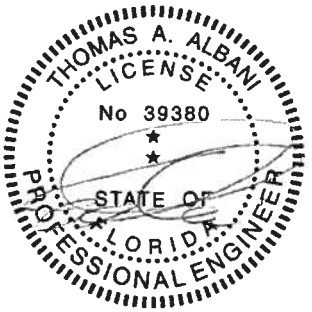



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

NOTES:
1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
4. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x_ ORIENTATION ONLY.
6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.

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**January 19, 2018**

| AUGUST 1, 2016 | LATERAL TOE-NAIL DETAIL | MII-TOENAIL_SP |
|---|---|---|



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NOTES:
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

| | DIAM. | SP | DF | HF | SPF | SPF-S |
|---|---|---|---|---|---|---|
| 3.5" LONG | .131 | 88.0 | 80.6 | 69.9 | 68.4 | 59.7 |
| | .135 | 93.5 | 85.6 | 74.2 | 72.6 | 63.4 |
| | .162 | 108.8 | 99.6 | 86.4 | 84.5 | 73.8 |
| 3.25" LONG | .128 | 74.2 | 67.9 | 58.9 | 57.6 | 50.3 |
| | .131 | 75.9 | 69.5 | 60.3 | 59.0 | 51.1 |
| | .148 | 81.4 | 74.5 | 64.6 | 63.2 | 52.5 |

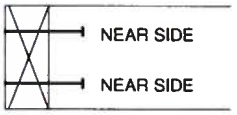
VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

EXAMPLE:
(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

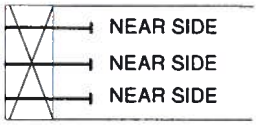
For load duration increase of 1.15:
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

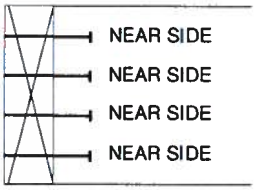
SIDE VIEW
(2x3)
2 NAILS

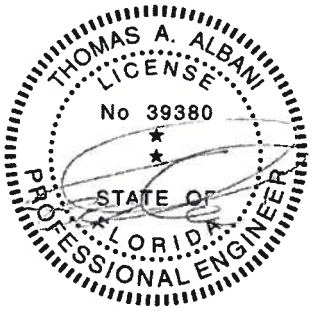



SIDE VIEW
(2x4)
3 NAILS




SIDE VIEW
(2x6)
4 NAILS

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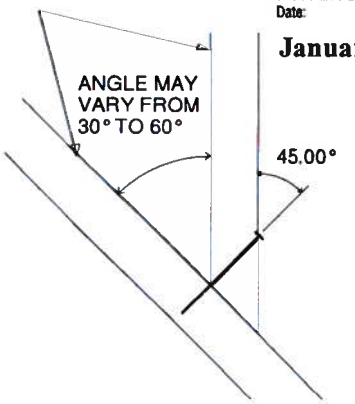


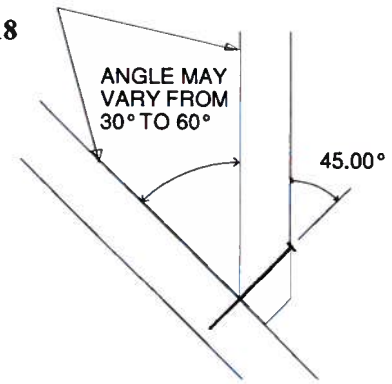


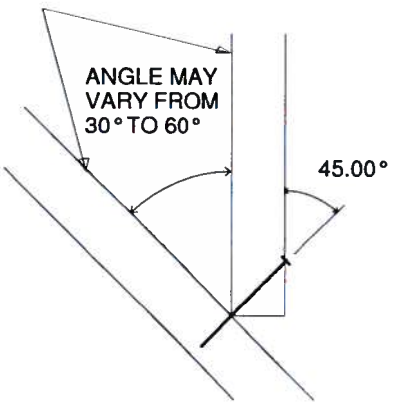

| AUGUST 1, 2016 | TRUSSED VALLEY SET DETAIL | MII-VALLEY HIGH WIND1 |
|---|---|---|



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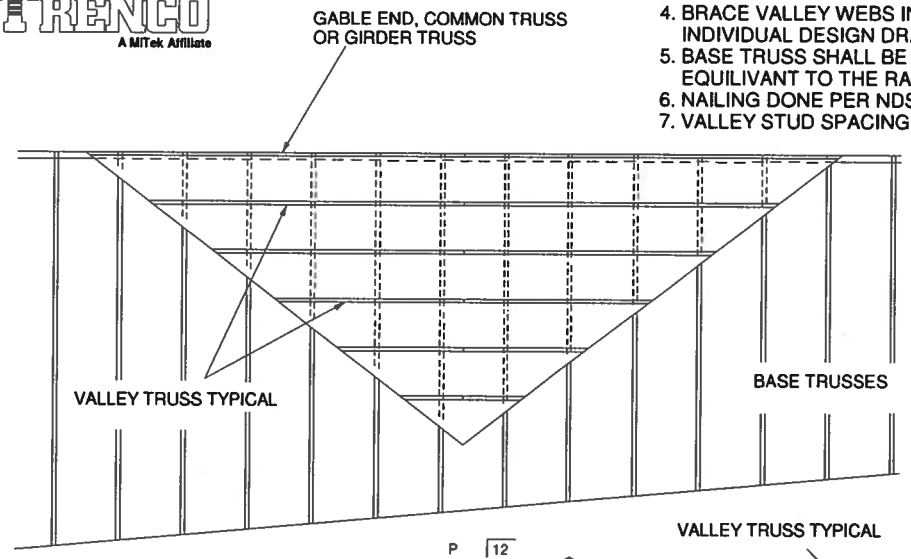
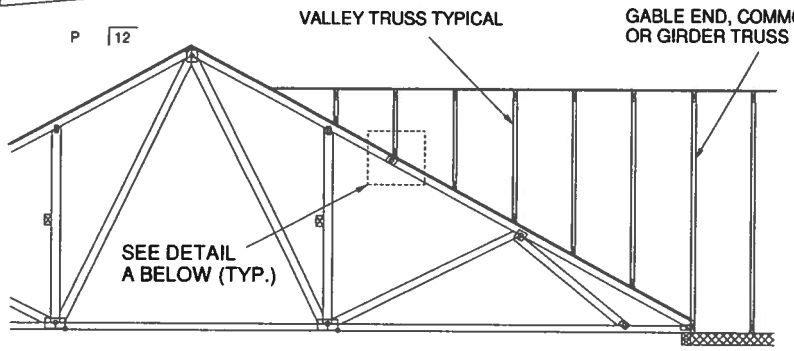
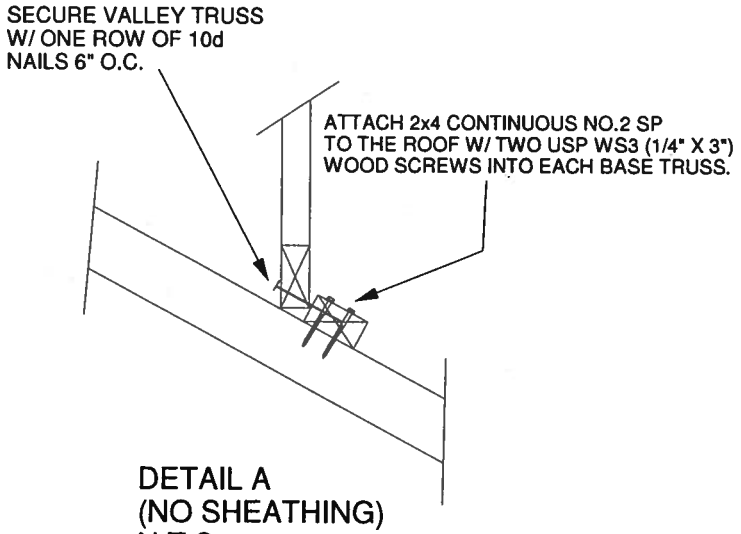
GENERAL SPECIFICATIONS

1. NAIL SIZE 10d (0.131" X 3")
2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT
   DO NOT USE DRYWALL OR DECKING TYPE SCREW
3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
6. NAILING DONE PER NDS - 01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.

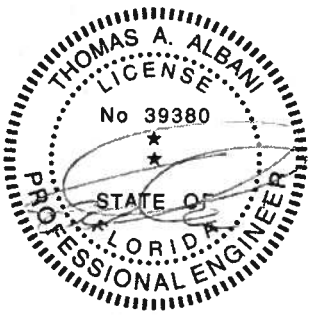



DETAIL A
(NO SHEATHING)
N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES

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**January 19, 2018**

| AUGUST 1, 2016 | TRUSSED VALLEY SET DETAIL | MII-VALLEY HIGH WIND2 |
|---|---|---|



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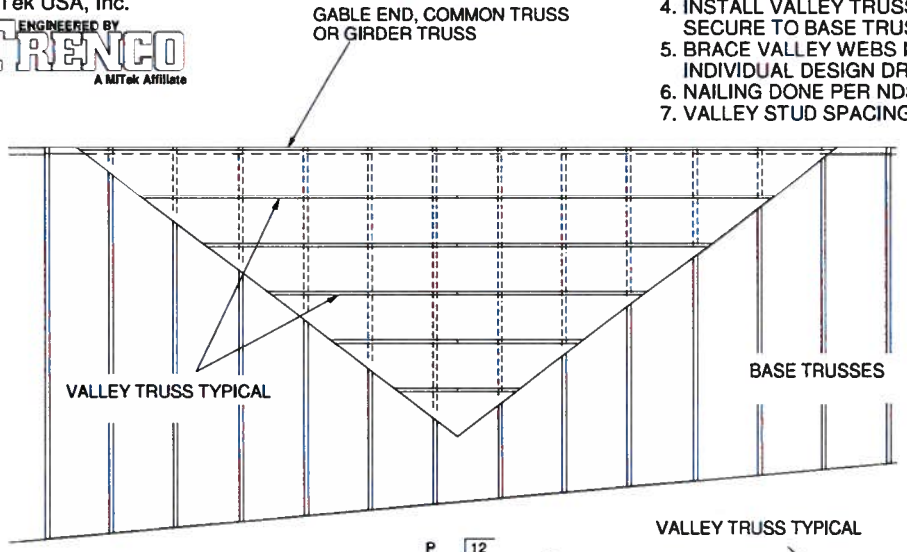
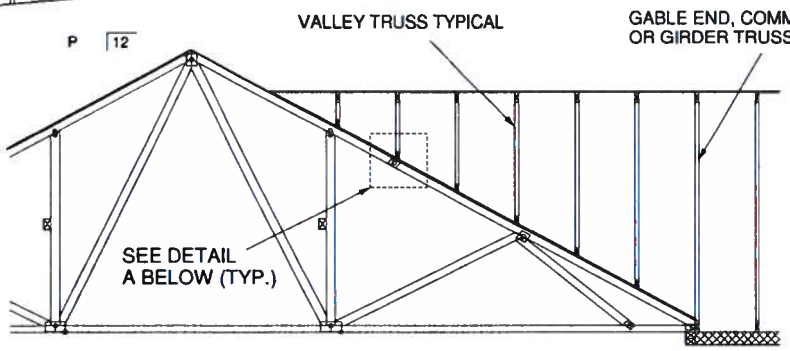
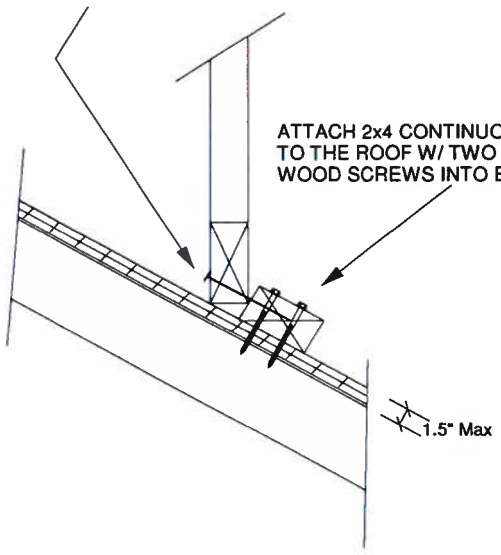
GENERAL SPECIFICATIONS

1. NAIL SIZE 10d (0.131" X 3")
2. WOOD SCREW = 4.5" WS45 USP OR EQUILIVANT
3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE TO BASE TRUSSES AS PER DETAIL A
5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
6. NAILING DONE PER NDS-01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.

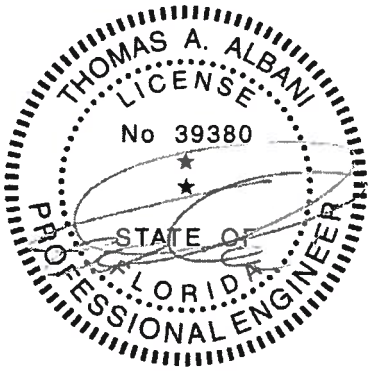



SECURE VALLEY TRUSS
W/ ONE ROW OF 10d
NAILS 6" O.C.




WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES

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**February 12, 2018**

| AUGUST 1, 2016 | TRUSSED VALLEY SET DETAIL | MII-VALLEY SP |
|---|---|---|

MiTek USA, Inc.



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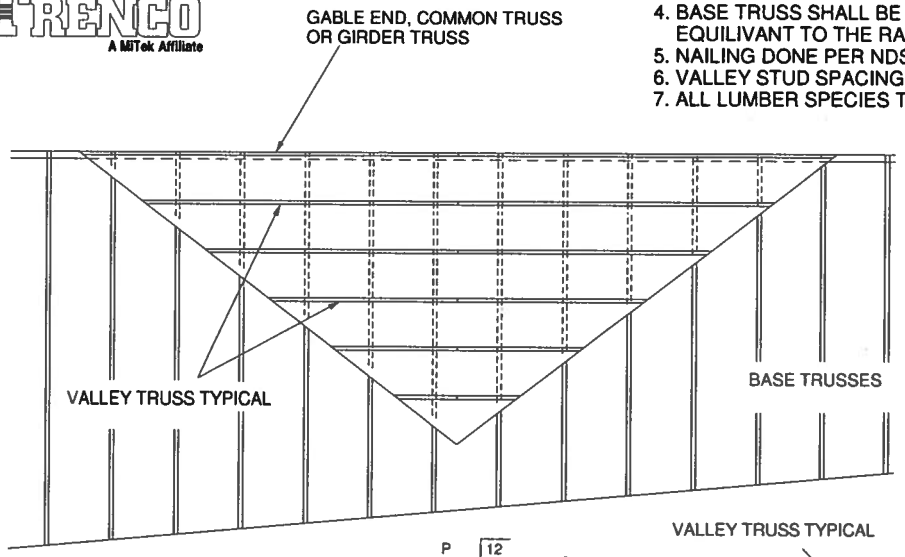
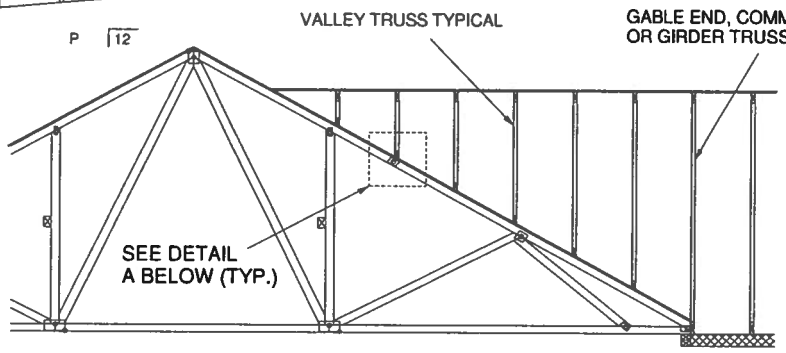
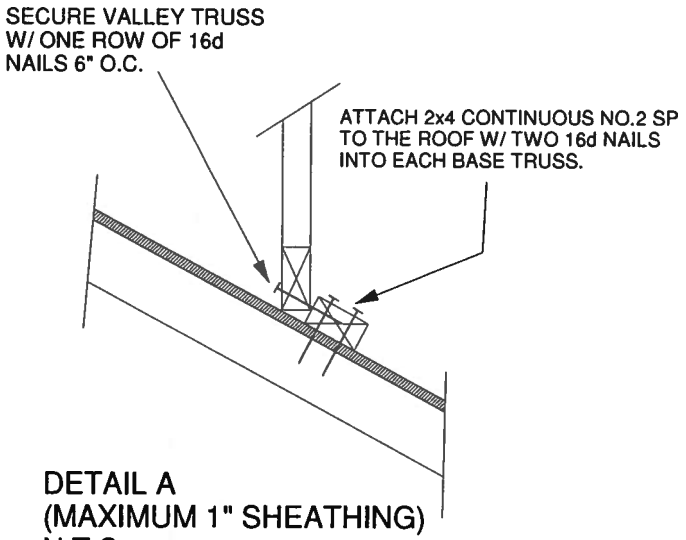
GENERAL SPECIFICATIONS

1. NAIL SIZE 16d (0.131" X 3.5")
2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
5. NAILING DONE PER NDS - 01
6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
7. ALL LUMBER SPECIES TO BE SP.

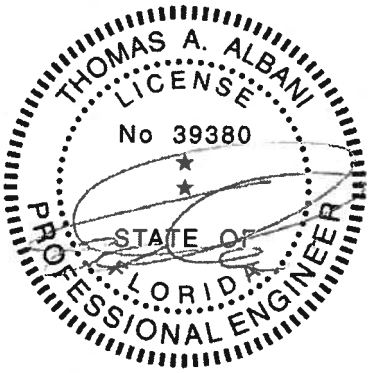



DETAIL A
(MAXIMUM 1" SHEATHING)
N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH
WIND DESIGN PER ASCE 7-10 150 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12
CATEGORY II BUILDING
EXPOSURE C OR B
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 60 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 4.2 PSF ON THE TRUSSES

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**February 12, 2018**

| AUGUST 1, 2016 | TRUSSED VALLEY SET DETAIL (HIGH WIND VELOCITY) | MII-VALLEY |
|---|---|---|

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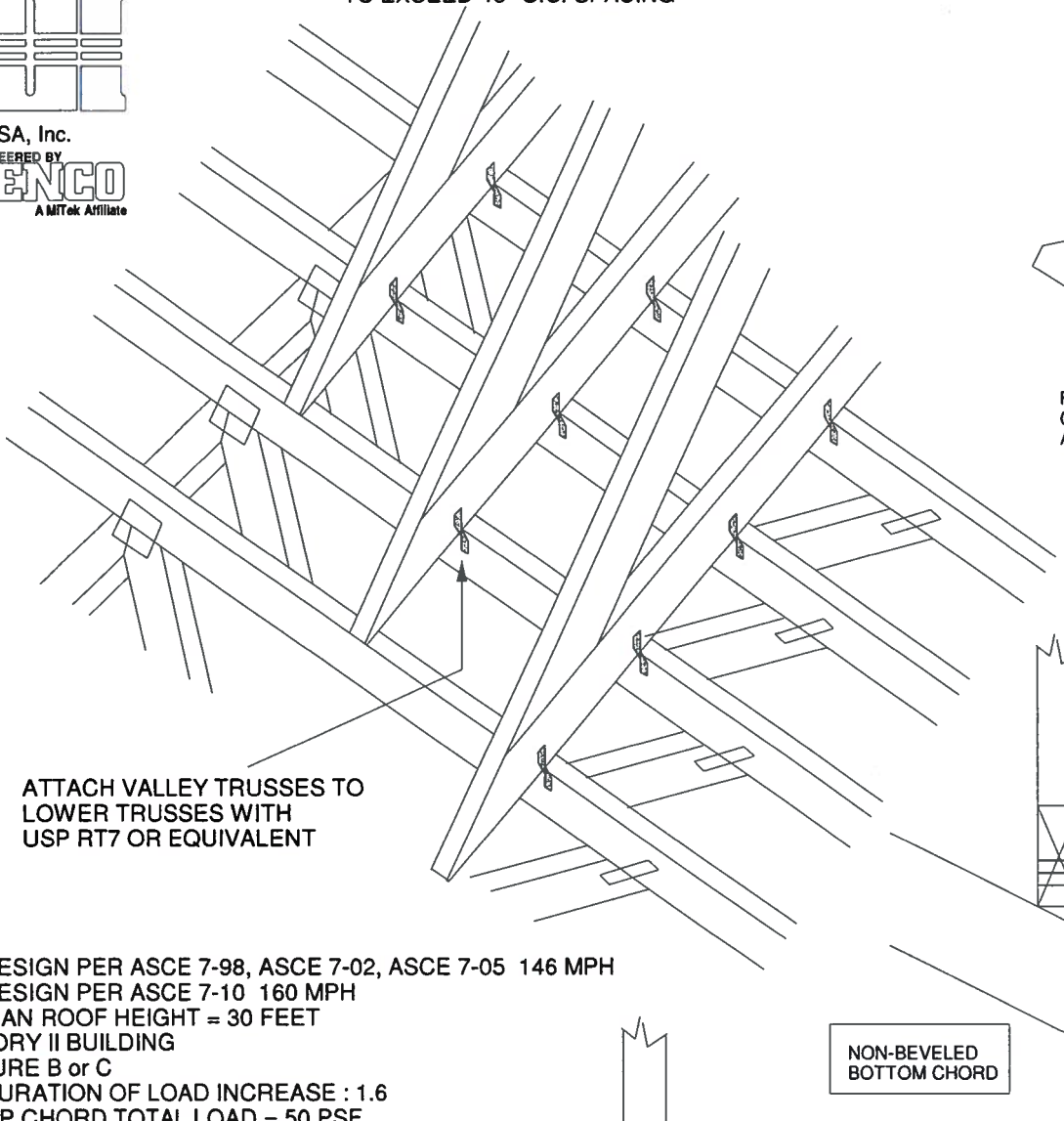
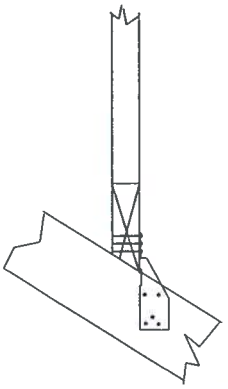
NOTE: VALLEY STUD SPACING NOT TO EXCEED 48" O.C. SPACING



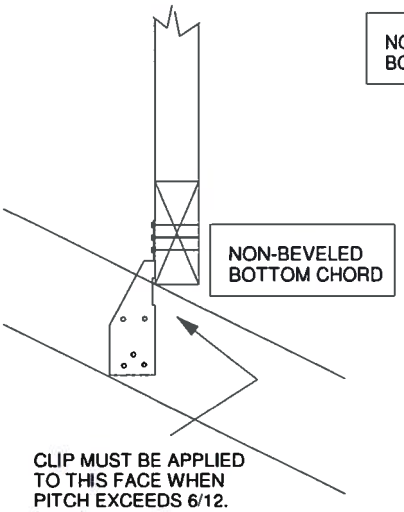



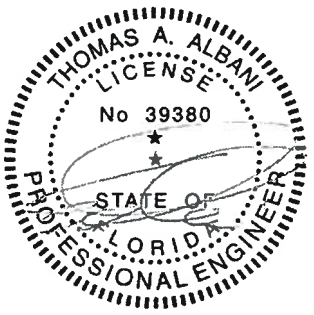
FOR BEVELED BOTTOM CHORD, CLIP MAY BE APPLIED TO EITHER FACE

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
WIND DURATION OF LOAD INCREASE : 1.6
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)

SUPPORTING TRUSSES DIRECTLY UNDER VALLEY TRUSSES MUST BE DESIGNED WITH A MAXIMUM UNBRACED LENGTH OF 2'-10" ON AFFECTED TOP CHORDS.

NOTES:
- SHEATHING APPLIED AFTER INSTALLATION OF VALLEY TRUSSES
- THIS DETAIL IS NOT APPLICABLE FOR SPF-S SPECIES LUMBER.

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**January 19, 2018**

| AUGUST 1, 2016 | Standard Gable End Detail | MII-GE146-001 |
|---|---|---|



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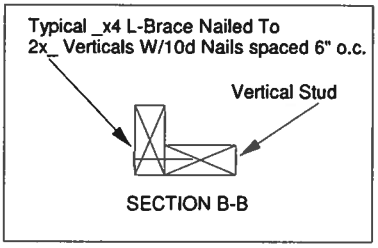
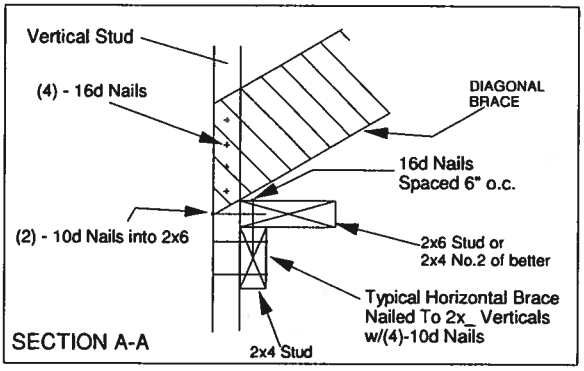
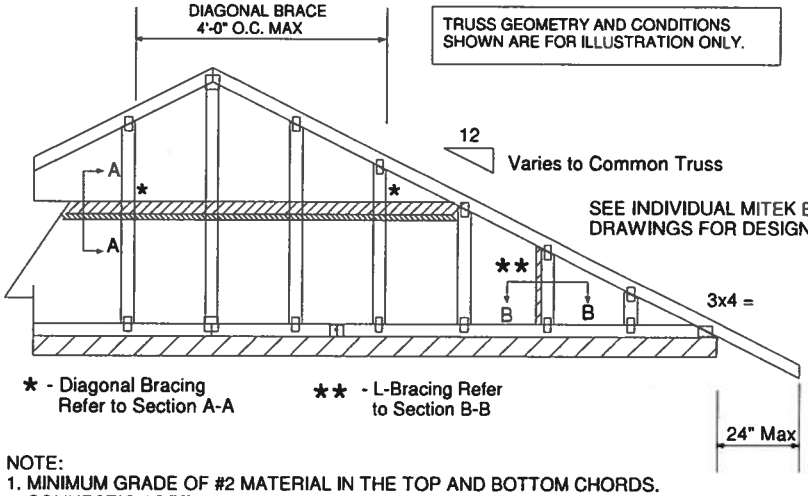


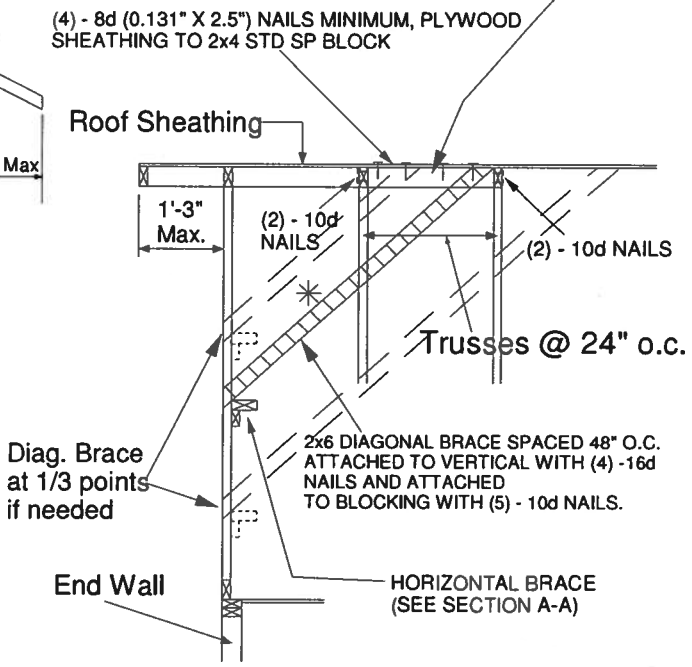




TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.



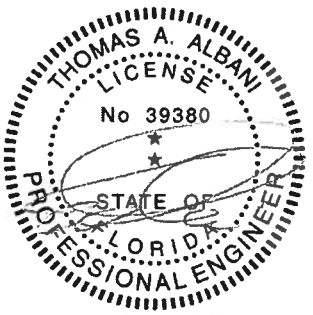

NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

| Minimum Stud Size Species and Grade | Stud Spacing | Without Brace | 2x4 L-Brace | DIAGONAL BRACE | 2 DIAGONAL BRACES AT 1/3 POINTS |
|---|---|---|---|---|---|
| | | Maximum Stud Length | | | |
| 2x4 SP No 3/Stud | 12" O.C. | 3-11-3 | 6-8-0 | 7-2-14 | 11-9-10 |
| 2x4 SP No 3/Stud | 16" O.C. | 3-6-14 | 5-9-5 | 7-1-13 | 10-8-11 |
| 2x4 SP No 3/Stud | 24" O.C. | 3-1-8 | 4-8-9 | 6-2-15 | 9-4-7 |

✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

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**January 19, 2018**

| OCTOBER 5, 2016 | REPLACE BROKEN OVERHANG | MII-REP13B |
|---|---|---|



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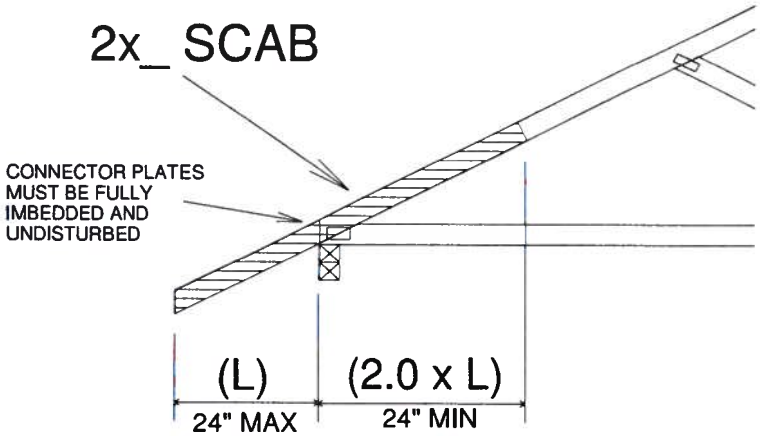
A MiTek Affiliate

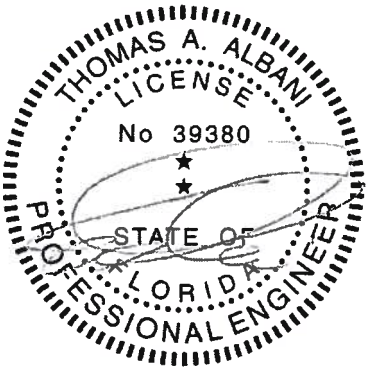
TRUSS CRITERIA:
LOADING: 40-10-0-10
DURATION FACTOR: 1.15
SPACING: 24" O.C.
TOP CHORD: 2x4 OR 2x6
PITCH: 4/12 - 12/12
HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL
END BEARING CONDITION

NOTES:
1. ATTACH 2x_ SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.




IMPORTANT
This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.
Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES

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February 12, 2018

| AUGUST 1, 2016 | LATERAL BRACING RECOMMENDATIONS | MII-STRGBCK |
|---|---|---|

MiTek USA, Inc. Page 1 of 1

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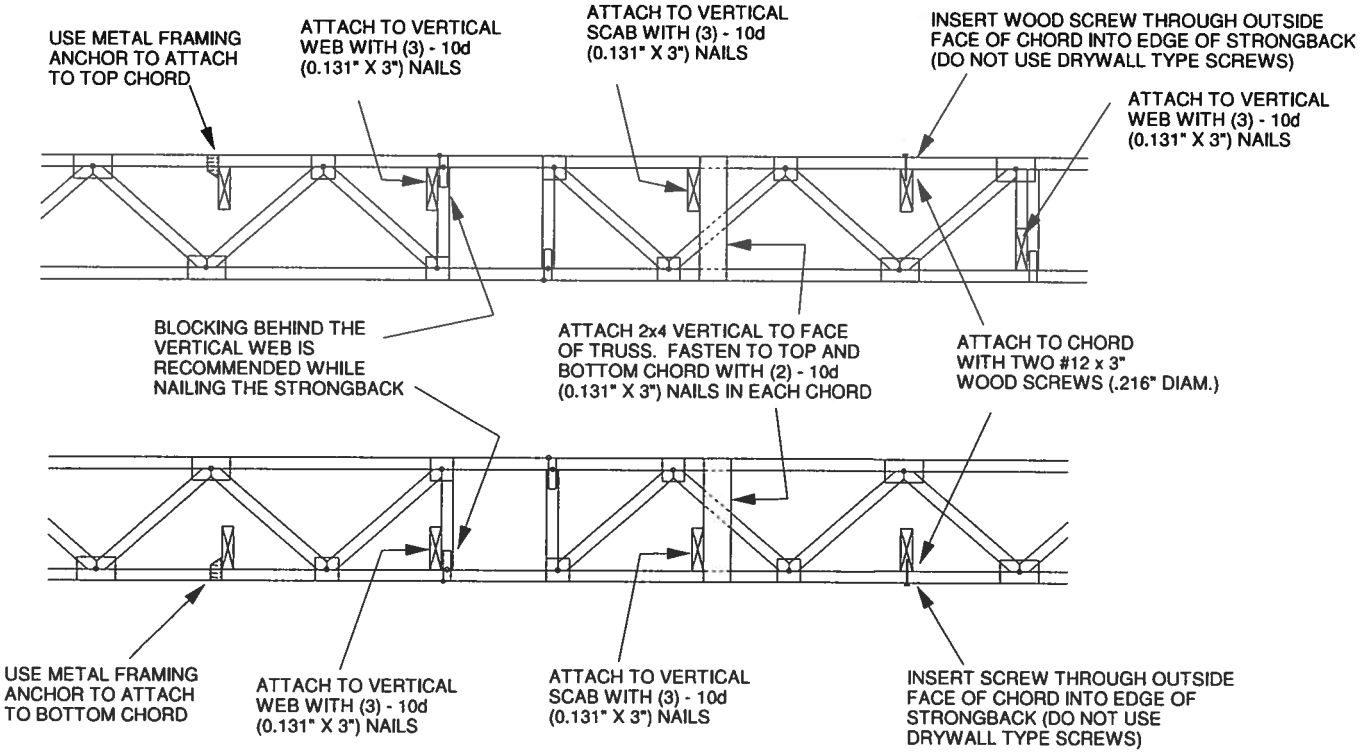
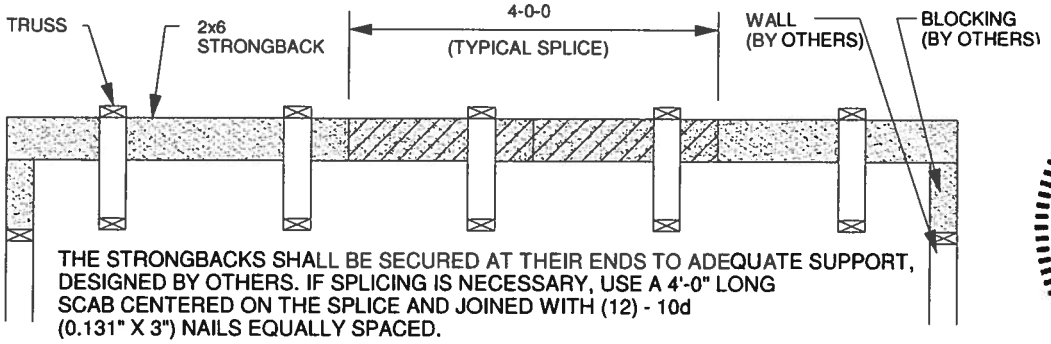
TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

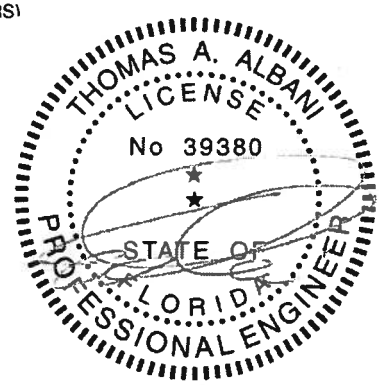
NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.




THE STRONGBACKS SHALL BE SECURED AT THEIR ENDS TO ADEQUATE SUPPORT, DESIGNED BY OTHERS. IF SPLICING IS NECESSARY, USE A 4'-0" LONG SCAB CENTERED ON THE SPLICE AND JOINED WITH (12) - 10d (0.131" X 3") NAILS EQUALLY SPACED.

ALTERNATE METHOD OF SPLICING:
OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d (0.131" X 3") NAILS STAGGERED AND EQUALLY SPACED.
(TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)

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