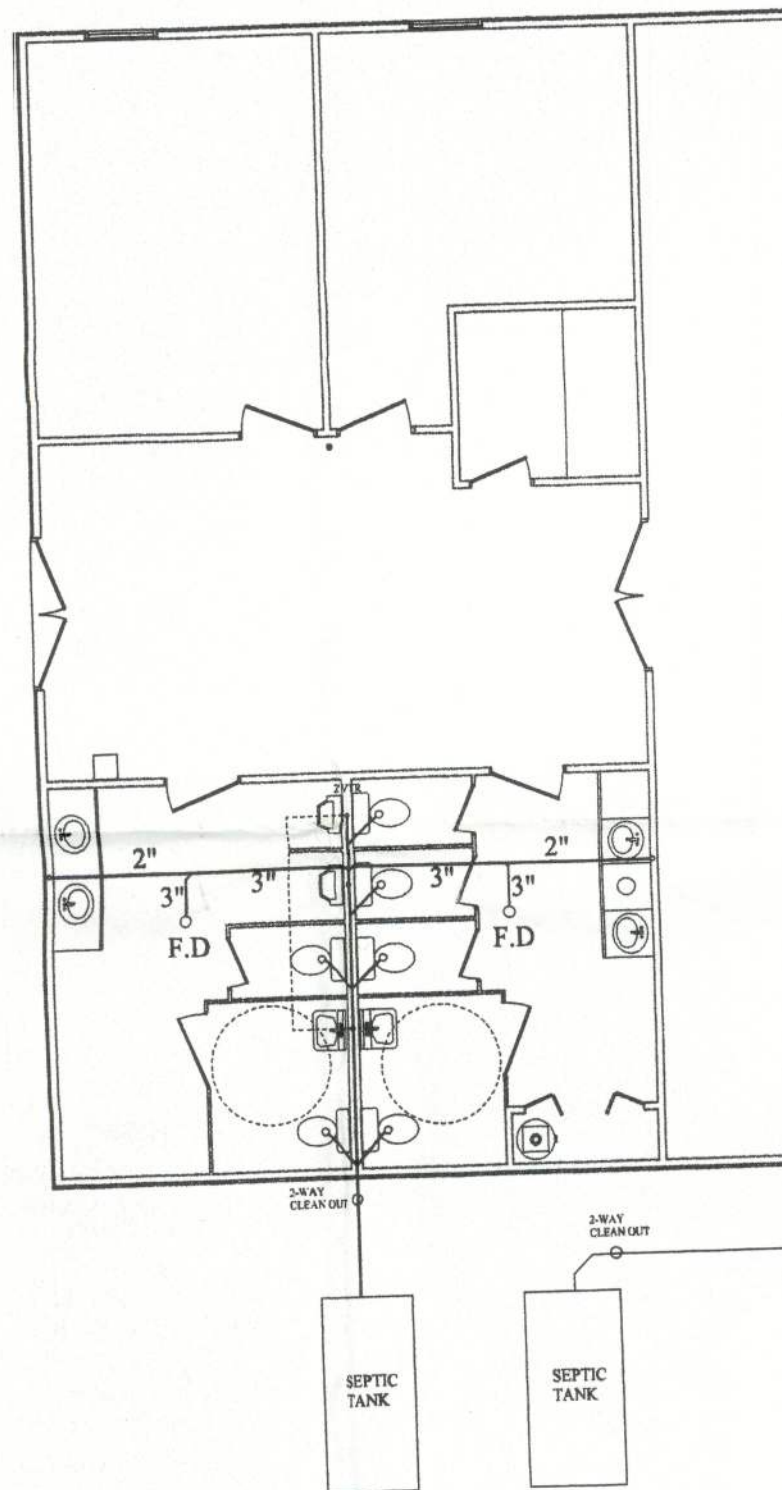




PROJECT No.
92-1301
DATE: SEPT. 2013
SCALE: AS NOTED

DRAWN BY:
J. WELLS
APPROVED BY:
P. SANTORA

REVISIONS:

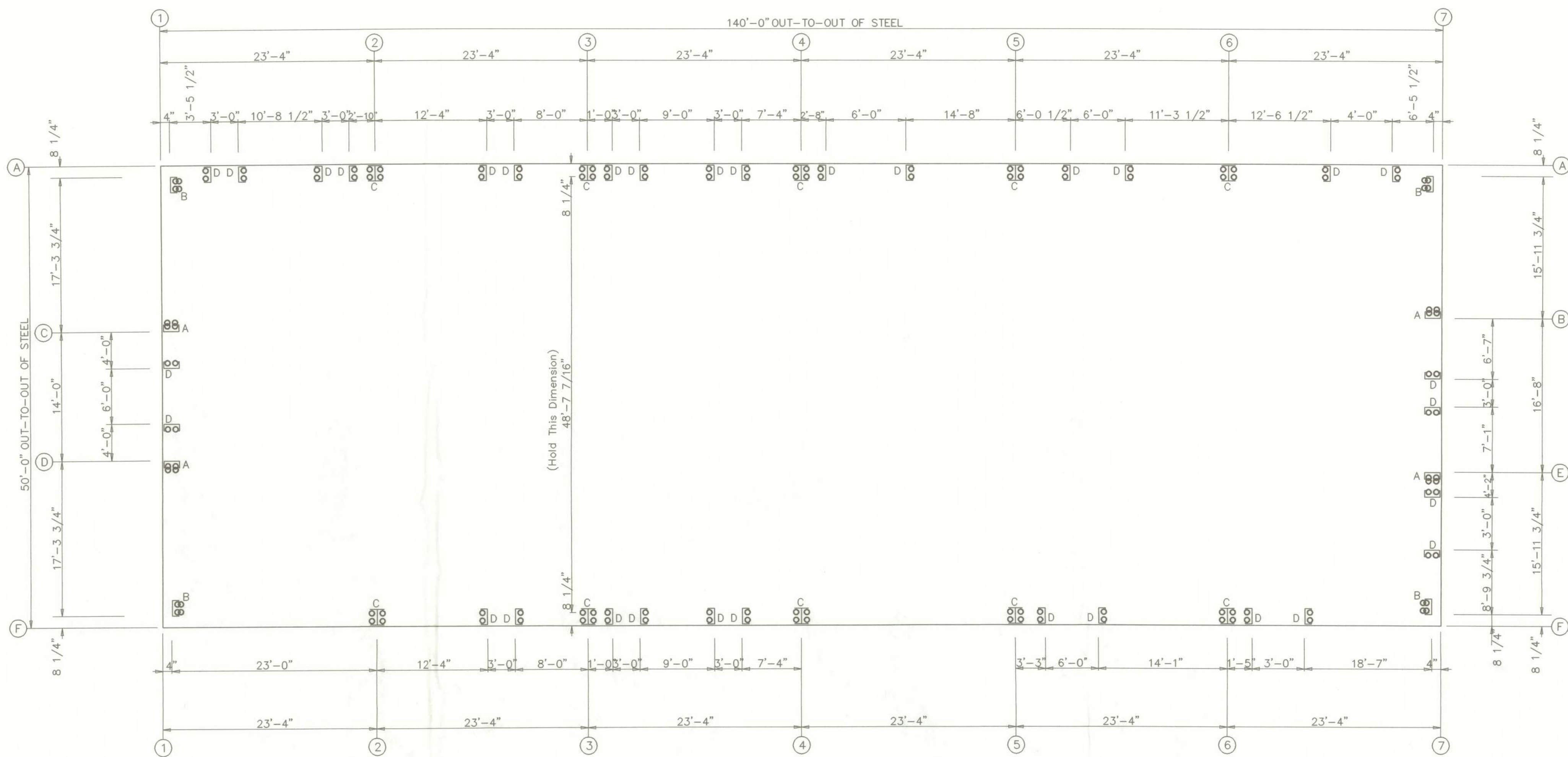
UNION LASTEEL
LULU BAPTIST CHURCH
A.B. PLAN
50'-0" X 140'-0" BUILDING
LAKE CITY, FL

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SHEET 1
OF 5



ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)
SCALE: 1/8" = 1'-0"

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	=	50.0
Length (ft)	=	140.0
Eave Height (ft)	=	14.0 / 14.0
Roof Slope (rise/12)	=	4.0 / 4.0
Dead Load (psf)	=	5.0
Collateral Load (psf)	=	2.0
Roof Live Load (psf)	=	20.0
Frame Live Load (psf)	=	12.0
Wind Speed (mph)	=	120.0
Wind Code	=	FBC 10 (IBC 12)
Exposure	=	B
Closed/Open	=	C
Importance Wind	=	1.00
Importance Seismic	=	1.00
Seismic Zone	=	B
Seismic Coeff (Fa*Se)	=	0.15

ID Description

- | | |
|---|--|
| 1 | Dead+Collateral+Live |
| 2 | 0.6Dead+0.6Wind_Left1 |
| 3 | 0.6Dead+0.6Wind_Right1 |
| 4 | 0.6Dead+0.6Wind_Left1+0.6Wind_Suction |
| 5 | 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1 |
| 6 | 0.6Dead+0.6Wind_Left2+0.6Wind_Suction |
| 7 | 0.6Dead+0.6Wind_Right2+0.6Wind_Suction |
| 8 | 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2 |
| 9 | 0.6Dead+0.6Wind_Right1+0.6Wind_Suction |

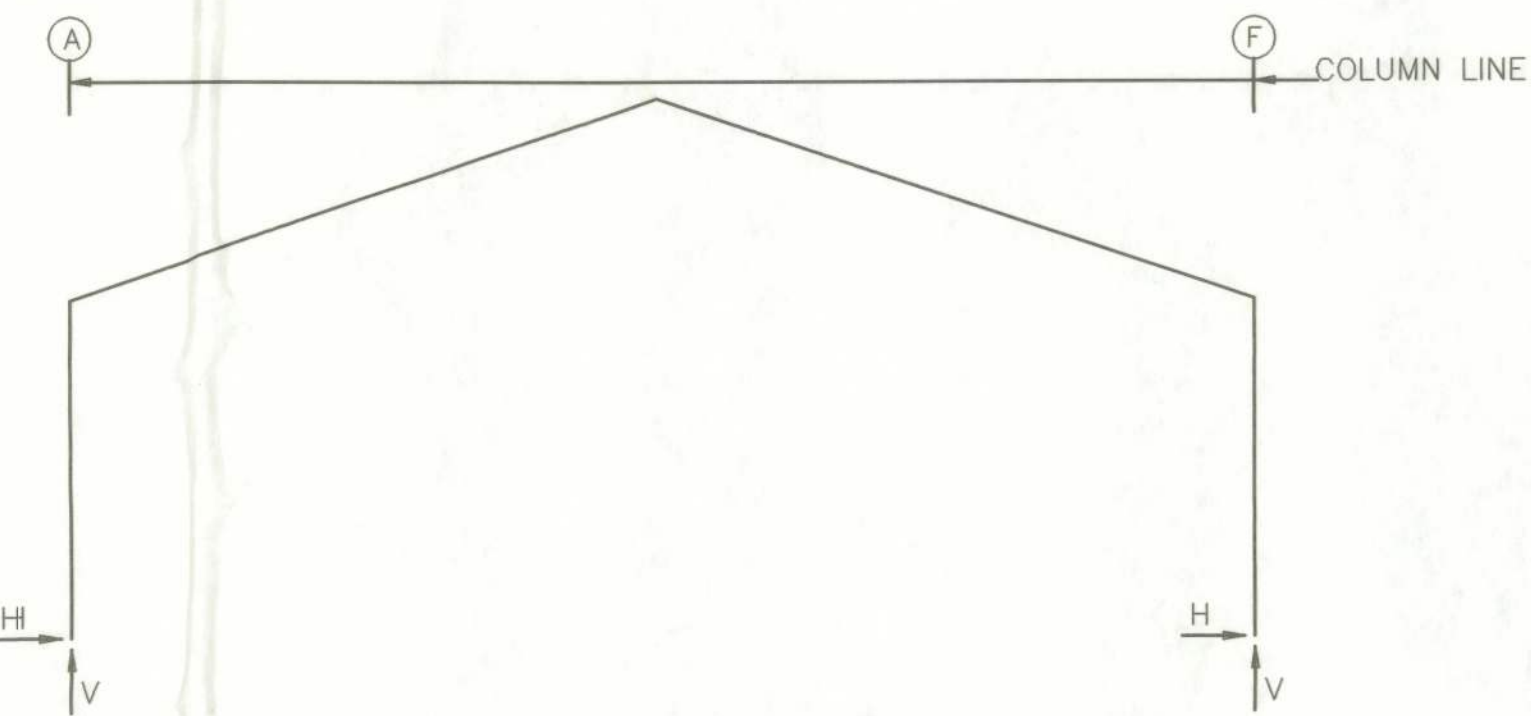
ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
64	Jamb	1 1/2"	A307	2.00
32	Endwall	5/8"	A307	3.00
40	Frame	3/4"	A307	3.00

BUILDING BRACING REACTIONS

—Wall—		Col	± Reactions (k)				Panel	Shear
Loc	Line	Line	—Wind—		—Seismic—		—	(lb/ft)
			Horz	Vert	Horz	Vert	Wind	Seis
L_EW	1						47	3
F_SW	F						20	5
R_EW	7						47	3
B_SW	A						22	5

FRAME LINES: 2 3 4 5 6



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

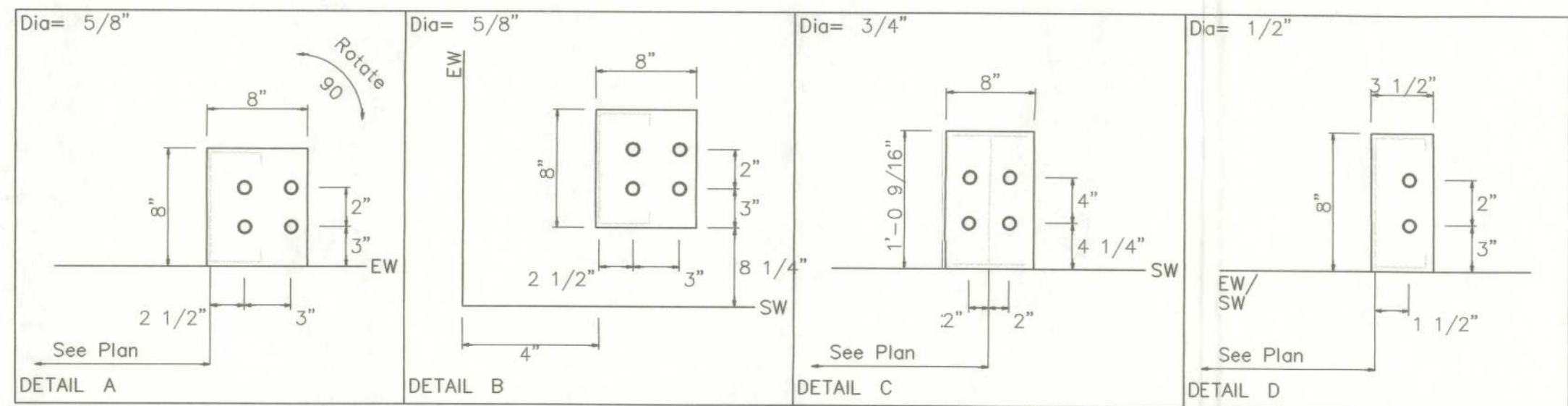
Frm Line	Col Line	Column Reactions (k)						Anc. Bolt Qty	Dia	Base Plate (in)			Grout (in)
		Load ID	Hmax H	V Vmax	Load ID	Hmin H	V Vmin			Width	Length	Thick	
2*	A	1	4.6	10.2	2	-3.5	-5.4	4	0.750	8.000	12.56	0.375	0.0
2*	F	3	3.5	-5.4	1	-4.6	10.2	4	0.750	8.000	12.56	0.375	0.0
2*	Frame lines: 2 3 4 5 6												

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead	Collateral	Live	Wind_Left1	Wind_Right1	Wind_Left2
2*	A	0.8	2.0	0.6	1.2	3.2	7.0
2*	F	-0.8	2.0	-0.6	1.2	-3.2	7.0
2*							

ENDWALL COLUMN REACTIONS(k)

MAXIMUM VERTICAL Dead+Collateral+Live = 5.4
MAXIMUM VERTICAL Dead+Wind = -4.9
MAXIMUM HORIZONTAL Dead+Wind = 3.4



ANCHOR BOLT DETAILS

NOTE: All Base Plates @ 100'-0" (U.N.)
N.T.S.



GENERAL NOTES

- A. ALL MATERIALS AND METHODS OF CONSTRUCTION SHALL BE IN ACCORDANCE WITH CONTRACT DOCUMENTS AND SPECIFICATIONS.
- B. THE CONTRACTOR SHALL WORK STRUCTURAL DRAWINGS TOGETHER WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND CIVIL DRAWINGS TO LOCATE DEPRESSED SLABS, SLOPES, DRAINS, GRADES, ETC. ANY DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE WORK.
- C. ALL DETAILS AND SECTIONS SHOWN ON THE DRAWING ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE ON THE PROJECT, EXCEPT WHERE A DIFFERENT DETAIL IS SHOWN.
- D. THE CONTRACTOR SHALL PROVIDE ADEQUATE BRACING, SHORING, AND OTHER TEMPORARY SUPPORTS AS REQUIRED TO SAFELY COMPLETE THE WORK.

DESIGN CRITERIA

- BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318 - 89/92))
- BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530 - 92)
- AISC "MANUAL OF STEEL CONSTRUCTION" - NINTH EDITION
- AWS D 1.1 "STRUCTURAL WELDING CODE" - LATEST EDITION
- AISI DESIGN FOR COLD FORMED STEEL STRUCTURAL MEMBERS 1996 W/1999 ADDENDUM
- ANSI / ASCE 7-10" MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES"

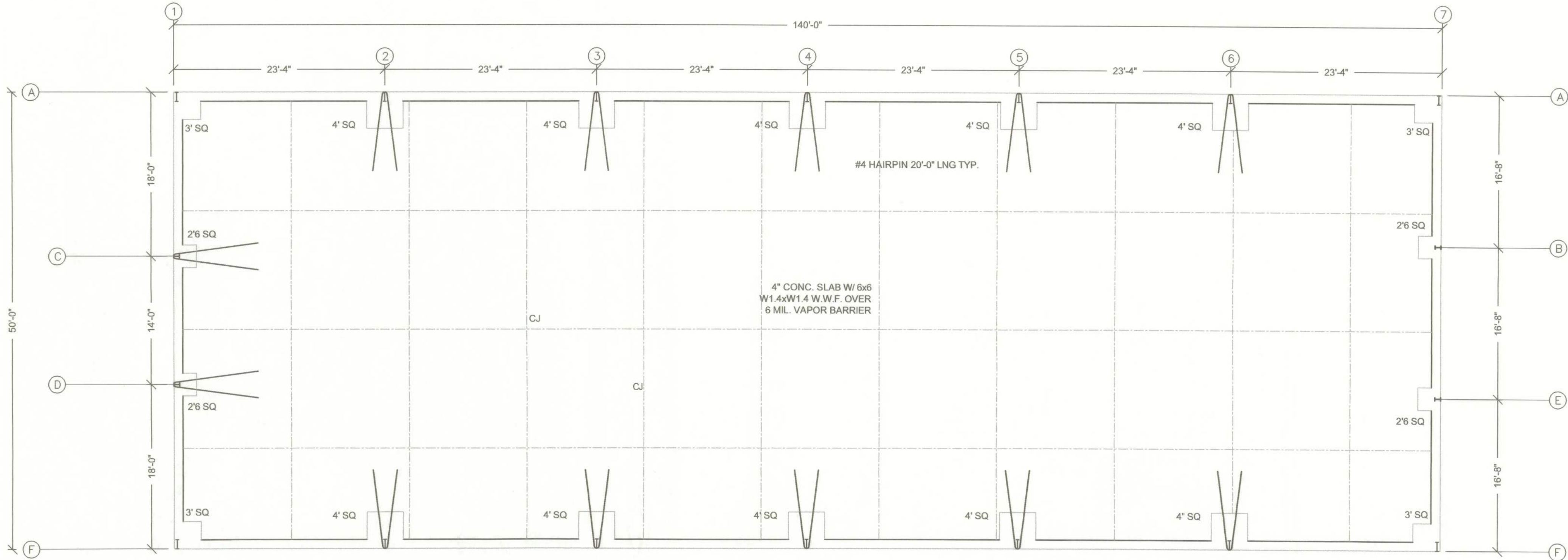
CONCRETE

- ALL CONCRETE SHALL BE DESIGNED TO SECURE A STRENGTH OF 2000 PSI AT 28 DAYS.
- PROVIDE MINIMUM COVER FOR REINFORCING BARS, UNLESS OTHERWISE INDICATED:
- FOOTINGS (TO GROUND) 3"
- FOOTINGS (TOP AND SIDE) 3"
- SLABS ON GRADE 2"
- SIDEWALKS 2"
- ALL BAR SPLICES AND DOWELS SHALL LAP 30 BAR DIAMETERS (MINIMUM) UNLESS NOTED OTHERWISE.
- ALL REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMED TO ASTM-A615, GRADE 60.
- ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
- ALL WELDED WIRE FABRIC SHALL BE LAPPED ONE FULL MESH, PANEL PLUS TWO INCHES AT SIDES AND ENDS AND SHALL BE WIRED TOGETHER.
- ALL CONCRETE REINFORCEMENT SHALL BE DETAILED, FABRICATED, LABELED, SUPPORTED, AND SPACED IN FORMS AND SECURED IN PLACE IN ACCORDANCE WITH THE PROCEDURES AND REQUIREMENTS OUTLINED IN THE LATEST EDITION OF ACI 318 AND ACI 315. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION.

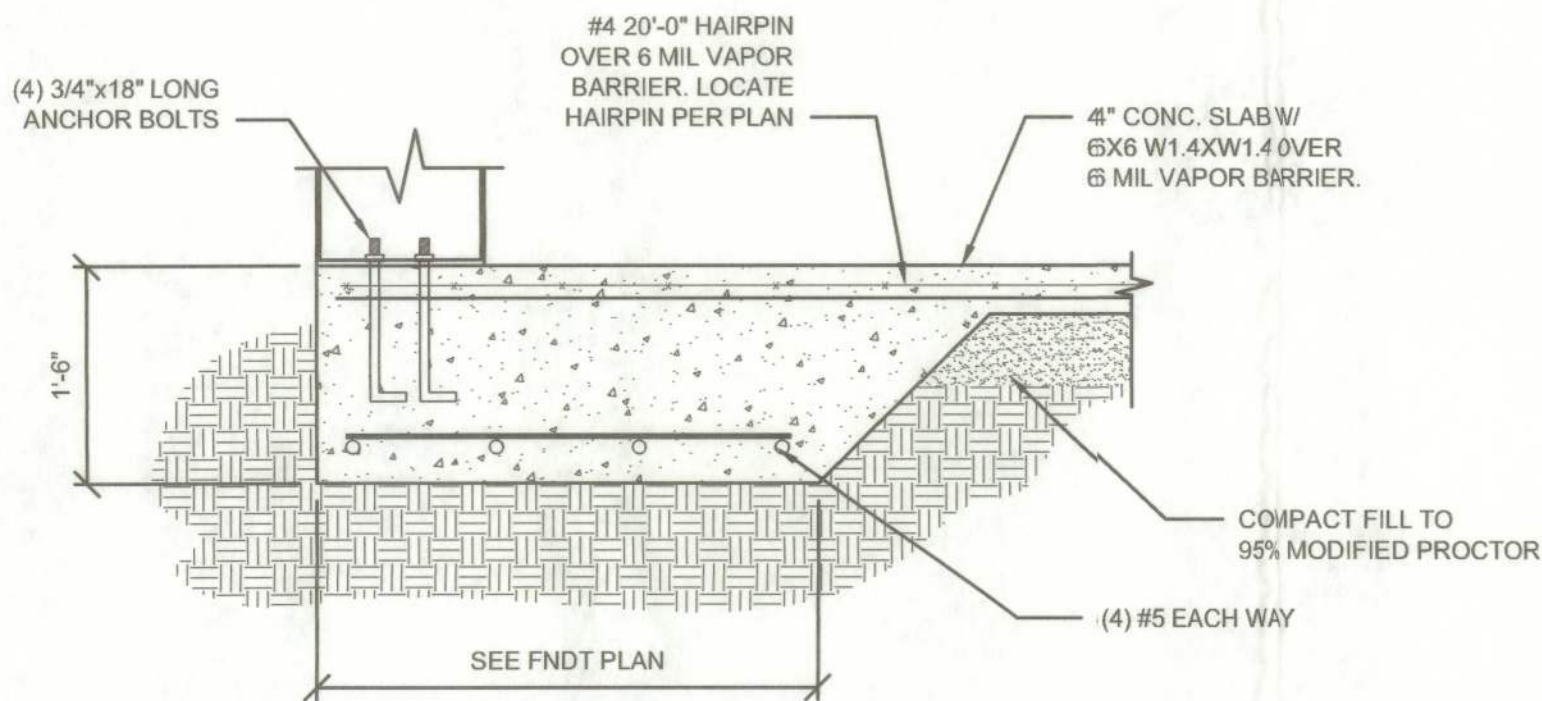
EARTHWORK

- ALL EXCAVATION AND BACK FILL SHALL BE PERFORMED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.
- CONCRETE SHALL BE PLACED AS SOON AS PRACTICAL AFTER SOIL PREPARATION AND COMPACTION SO AS NOT TO ALLOW THE ELEMENTS OR CONSTRUCTION ACTIVITY TO DISTURB THE PREPARED AREA.
- UNDER NO CIRCUMSTANCES WILL DIGGING, TUNNELING, OR TRENCHING BE ALLOWED AT OR NEAR ANY CONCRETE STRUCTURE WHICH MIGHT ACT TO UNDERMINE THE STRUCTURE.

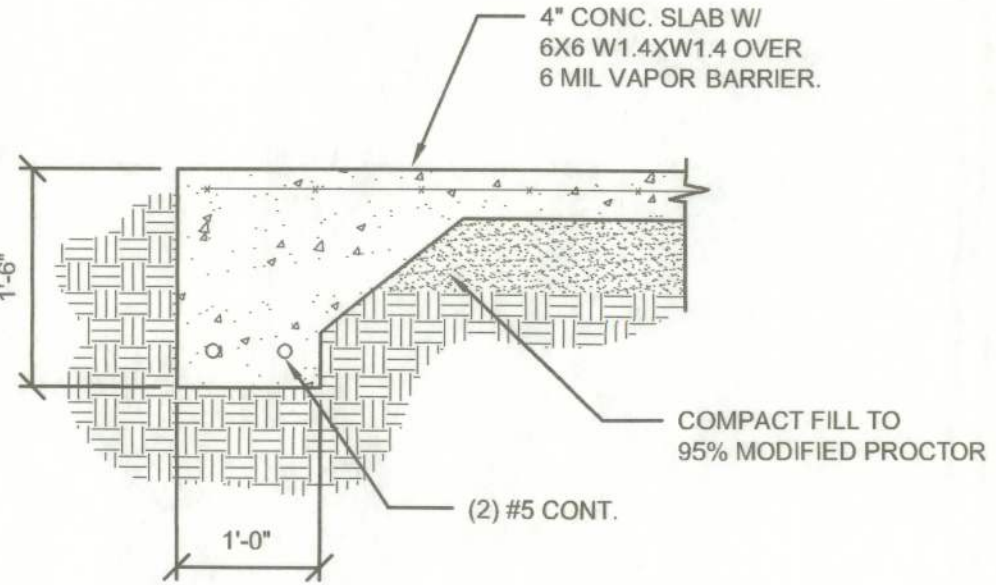
FOUNDATIONS ARE DESIGNED TO BEAR ON SOIL WHICH PROVIDES A SAFE BEARING CAPACITY OF 2,000 PSF.



FOUNDATION PLAN
SCALE: 1/8" = 1'-0"



COLUMN FOOTER DETAIL
SCALE: 3/4" = 1'-0"



CONT. FOOTER DETAIL
SCALE: 3/4" = 1'-0"

PROJECT No.
92-1301

DATE: SEPT. 2013

SCALE: AS NOTED

DRAWN BY:
J. WELLS

APPROVED BY:
P. SANTORA

REVISIONS:

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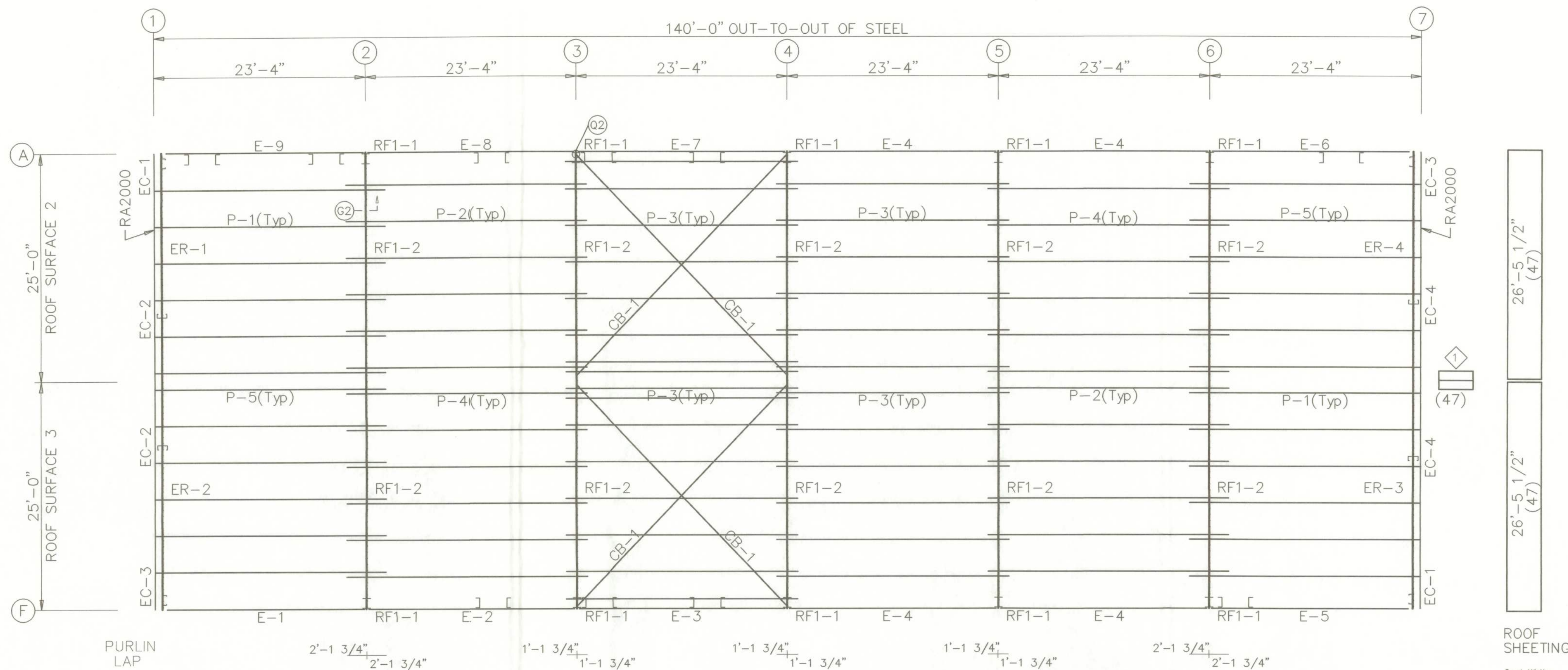
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SHEET 2
OF 5



ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"

MARK	PART	LENGTH
P-1	8X25Z16	24'-10 3/4"
P-2	8X25Z16	26'-7 1/2"
P-3	8X25Z16	25'-7 1/2"
P-4	8X25Z16	26'-7 1/2"
P-5	8X25Z16	24'-10 3/4"
ER-1	8E14LOW	23'-8 1/4"
ER-2	8E14LOW	23'-3 1/2"
ER-3	8E14LOW	23'-5 1/2"
ER-4	8E14LOW	23'-6 1/2"
ER-5	8E14LOW	23'-6 1/2"
ER-6	8E14LOW	23'-6 1/2"
ER-7	8E14LOW	23'-6 1/2"
ER-8	8E14LOW	23'-6 1/2"
ER-9	8E14LOW	23'-8 1/4"
CB-1	0.25 CBI	35'-0"

MARK	PART	LENGTH
1	SSRC	3'-0"

ROOF SHEETING
Cool White

PANELS: 26 Ga. SX

GENERAL NOTES

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DESIGN CRITERIA

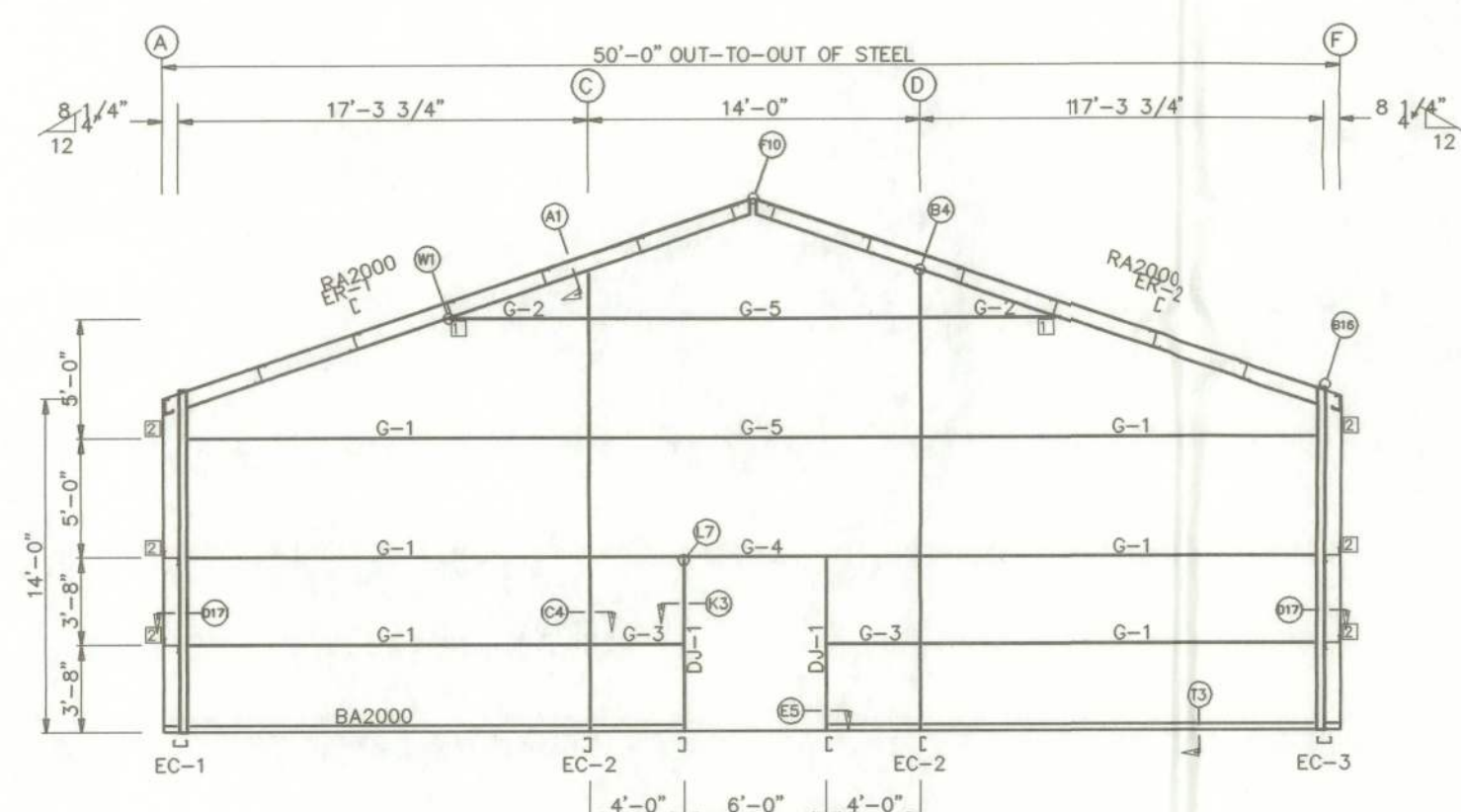
- FLORIDA BUILDING CODE - 2001 2ND EDITION
- AISC "MANUAL OF STEEL CONSTRUCTION" - NINTH EDITION
- AWS D 1.1 "STRUCTURAL WELDING CODE" - LATEST EDITION
- AISI DESIGN FOR COLD FORMED STEEL STRUCTURAL MEMBERS 1996 W/1999 ADDENDUM
- ANSI / ASCE 7-10 "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES"

DESIGN NOTE:

FRAMED OPENINGS AND ALL OTHER CLADDING AND COMPONENTS MUST BE DESIGNED FOR THE FOLLOWING WIND PRESSURES (psf):

ROOF:
INTERIOR ZONE: (+11.5) (-29.0)
END ZONE: (+11.8) (-43.6)
CORNER: (+11.8) (-63.6)

WALL:
INTERIOR ZONE: (+29.0) (-31.6)
END ZONE: (+29.0) (-38.0)



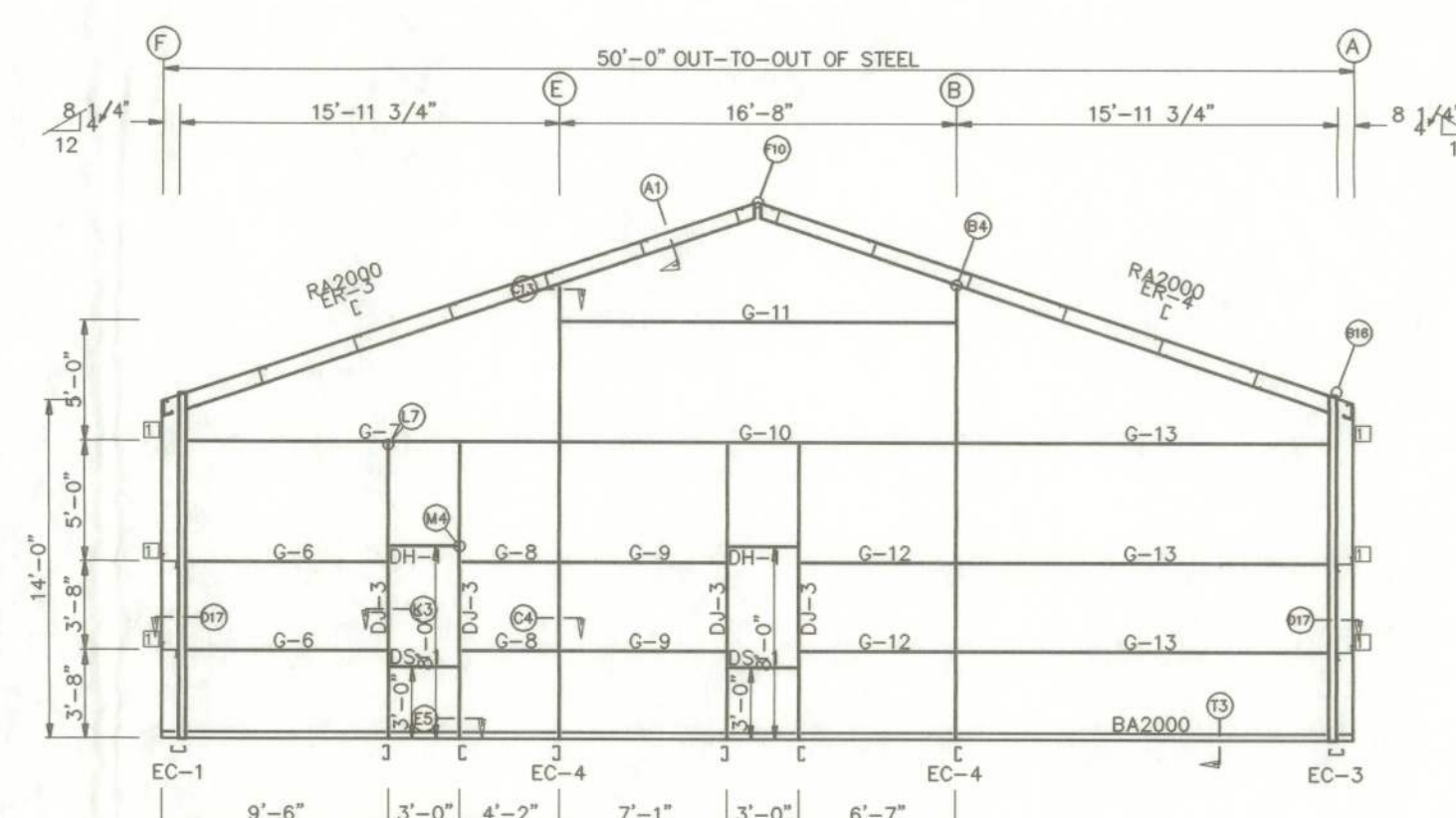
ENDWALL FRAMING: FRAME LINE 1

LOCATION	QUAN	TYPE	DIA	LENGTH
Cor. Column/Raf	2	A325	5/8"	1 3/4"
ER-1/ER-2	6	A325	5/8"	1 3/4"
Int. Column/Raf	2	A325	5/8"	1 1/2"

MARK	PART	LENGTH
EC-1	8X25C16	14'-5 7/16"
EC-2	8X25C12	19'-1 9/16"
EC-3	8X25C16	14'-5 7/16"
ER-1	8X35C12	25'-1 3/4"
ER-2	8X35C12	25'-1 3/4"
DJ-1	8X35C16	7'-4"
C-1	8X25Z16	16'-3 1/4"
C-2	8X25Z16	6'-5 7/16"
C-3	8X25Z16	3'-7 1/2"
C-4	8X25Z16	13'-11 1/2"
C-5	8X25Z16	13'-11 1/2"

MARK	PART	LENGTH
1	BT-101	10'-3"
2	CT-102	10'-3"
3	RT-101	10'-3"
4	RT-101	6'-8 1/4"
5	SPB4	8"
6	JT-101	10'-3"
7	HT-101	10'-3"

CONNECTION PLATES	FRAME LINE
1	SA005
2	SA005



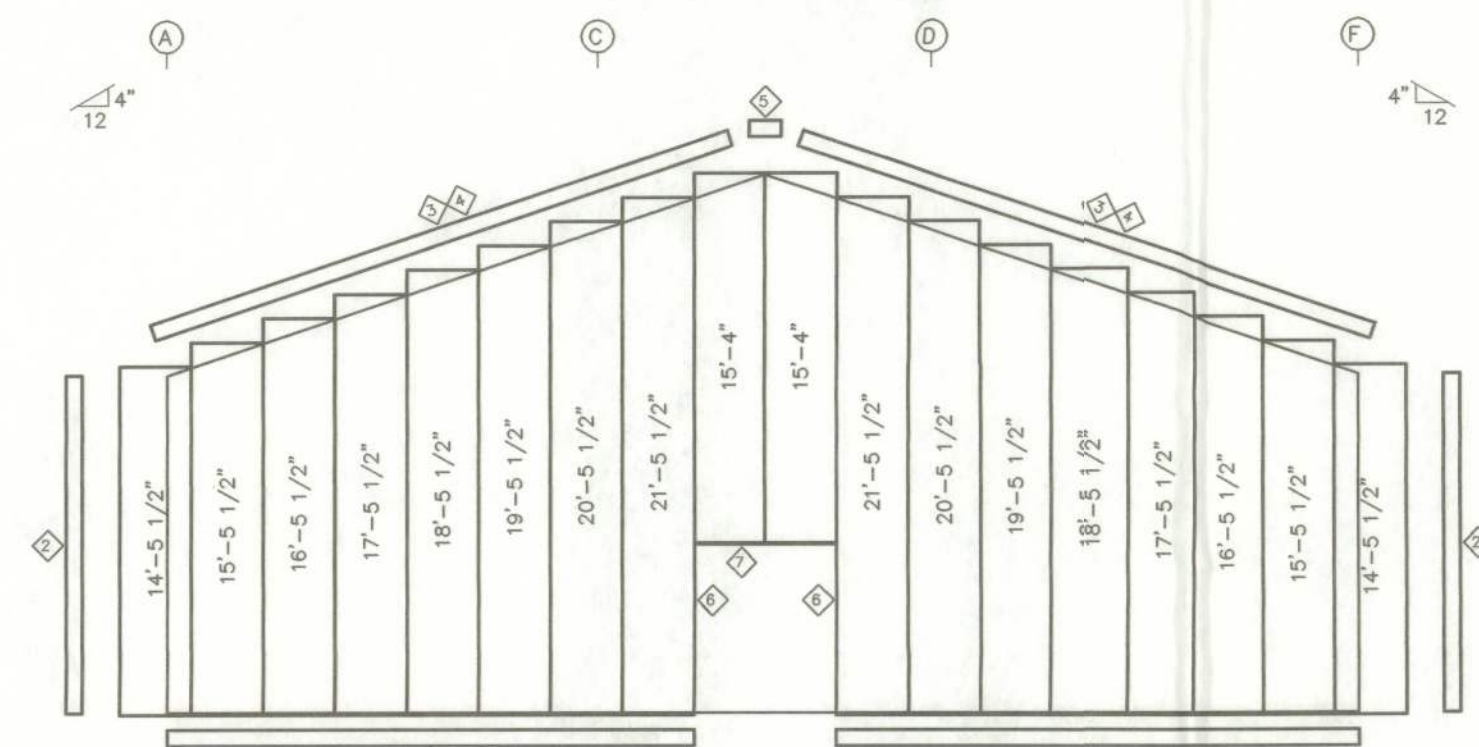
ENDWALL FRAMING: FRAME LINE 7

LOCATION	QUAN	TYPE	DIA	LENGTH
Cor. Column/Raf	2	A325	5/8"	1 3/4"
ER-3/ER-4	6	A325	5/8"	1 3/4"
Int. Column/Raf	2	A325	5/8"	1 1/2"

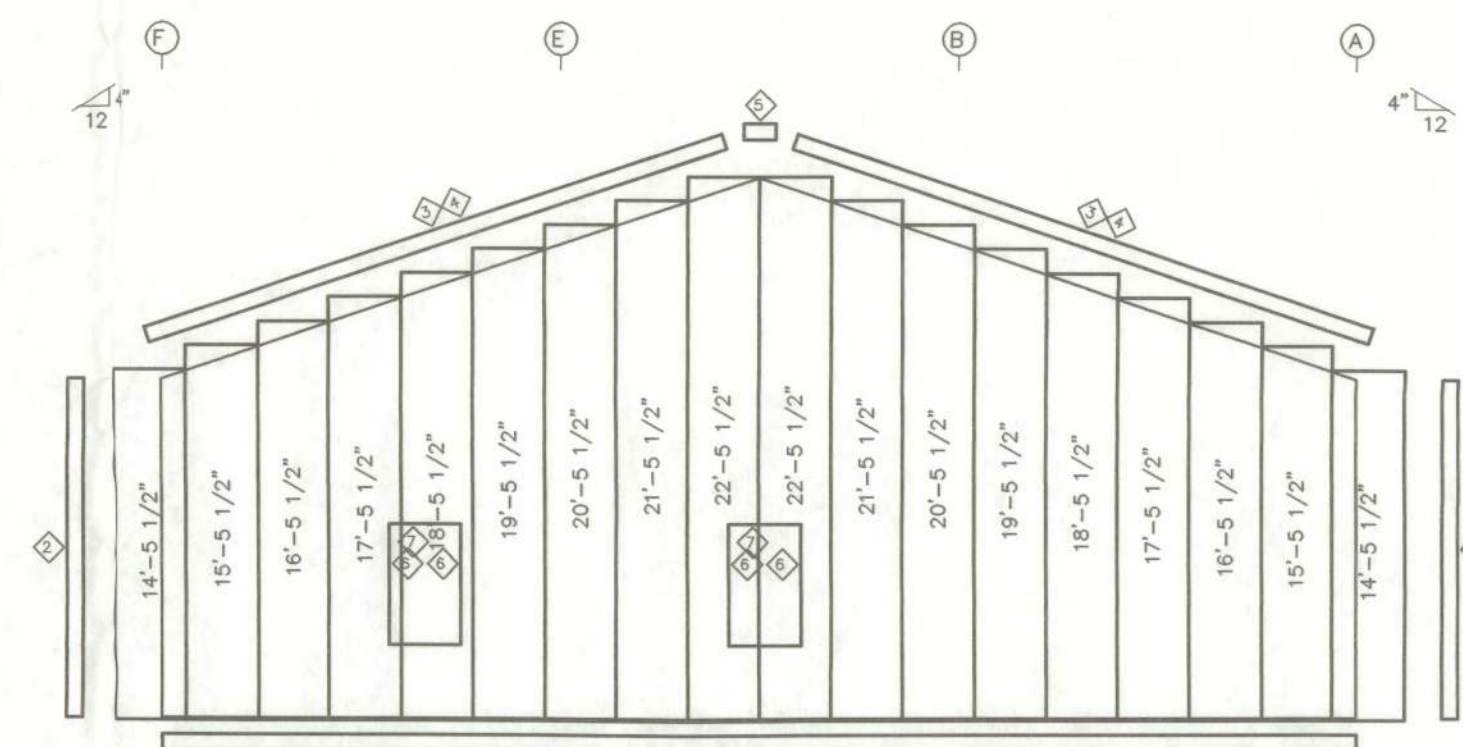
MARK	PART	LENGTH
EC-1	8X25C16	14'-5 7/16"
EC-2	8X25C12	19'-1 9/16"
EC-3	8X25C16	14'-5 7/16"
ER-1	8X35C12	25'-1 3/4"
ER-2	8X35C12	25'-1 3/4"
DJ-1	8X35C16	7'-4"
C-1	8X25Z16	16'-3 1/4"
C-2	8X25Z16	6'-5 7/16"
C-3	8X25Z16	3'-7 1/2"
C-4	8X25Z16	13'-11 1/2"
C-5	8X25Z16	13'-11 1/2"

MARK	PART	LENGTH
1	BT-101	10'-3"
2	CT-102	10'-3"
3	RT-101	10'-3"
4	RT-101	6'-8 1/4"
5	SPB4	8"
6	JT-101	10'-3"
7	HT-101	10'-3"

CONNECTION PLATES	FRAME LINE
1	SA005
2	SA005



ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 Ga. SX - Light Stone
SCALE: 1/8" = 1'-0"



ENDWALL SHEETING & TRIM: FRAME LINE 7
PANELS: 26 Ga. SX - Light Stone
SCALE: 1/8" = 1'-0"

PROJECT No.
92-1301

DATE: SEPT. 2013

SCALE: AS NOTED

DRAWN BY:
J. WELLS

APPROVED BY:
P. SANTORA

REVISIONS:

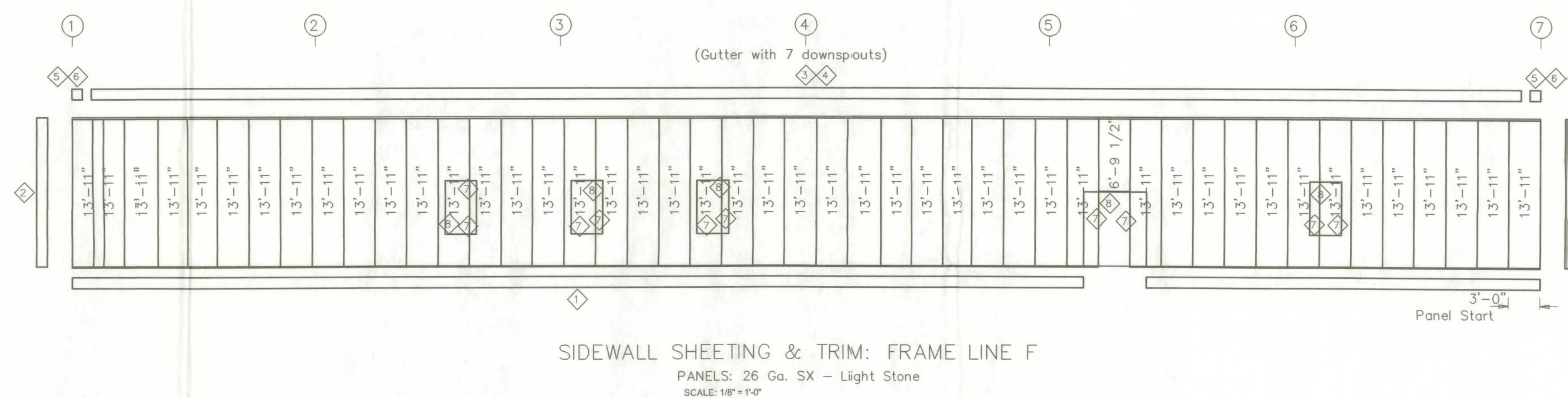
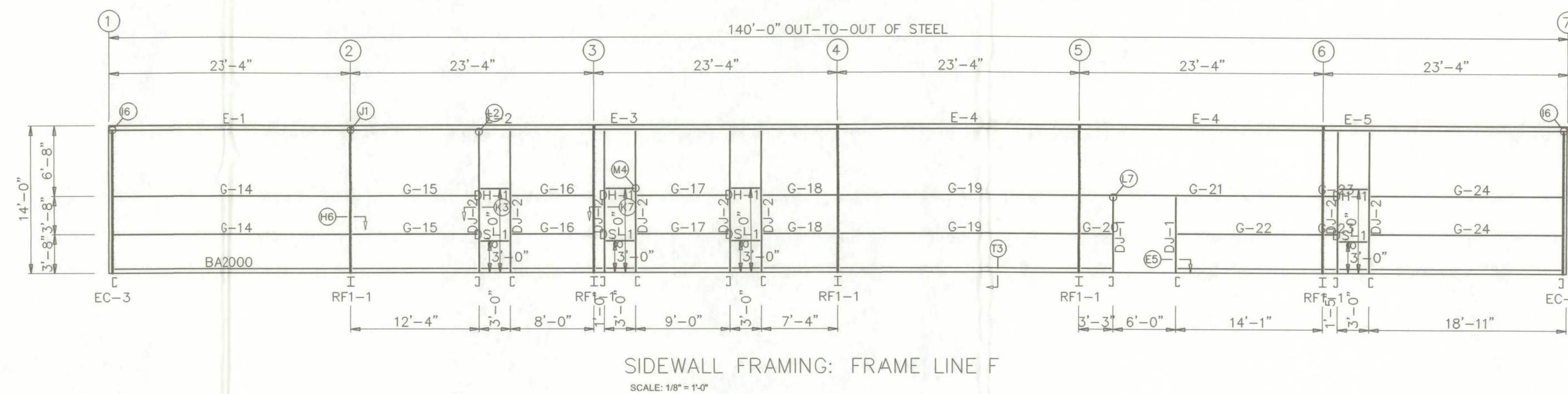
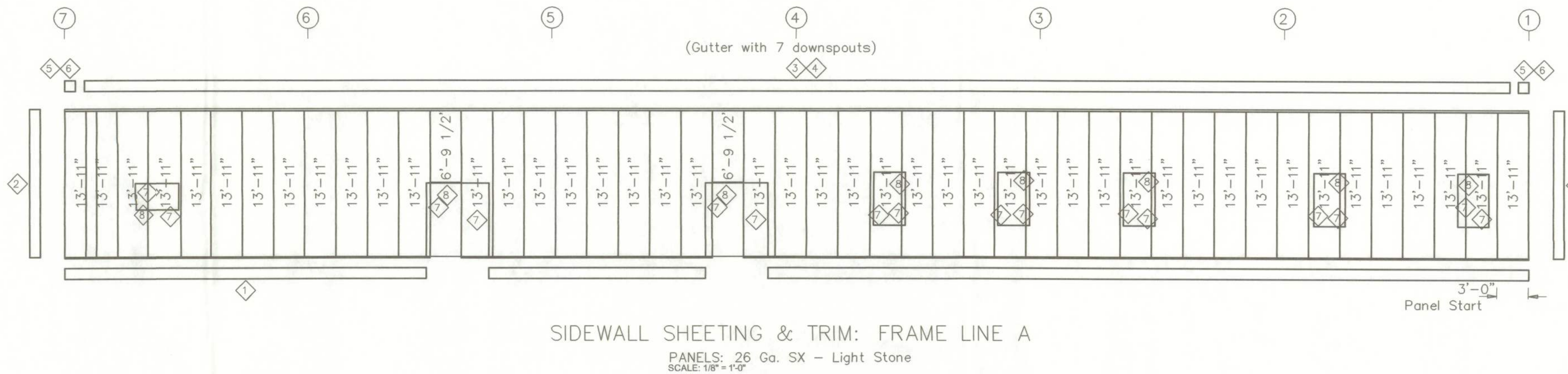
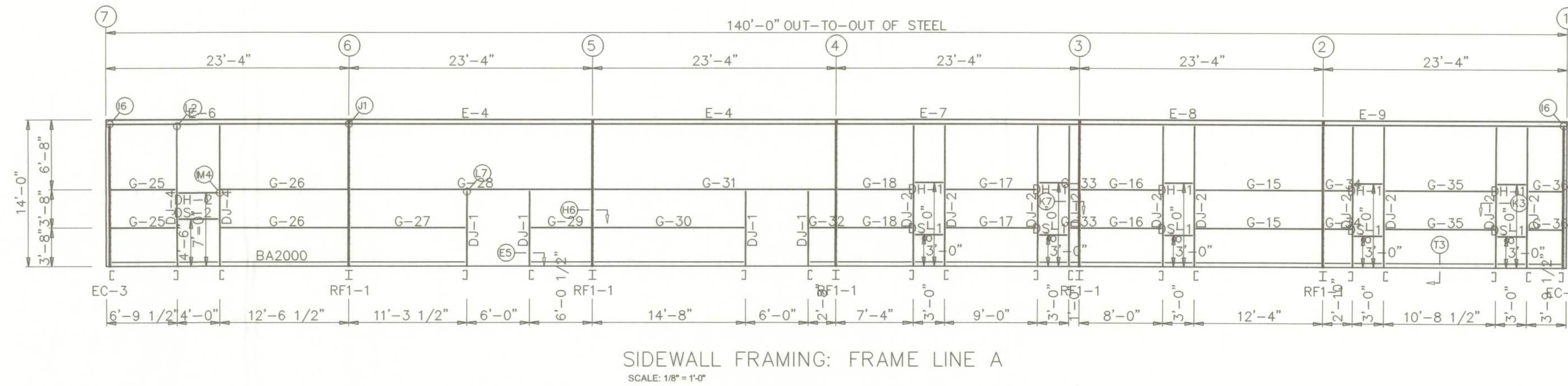
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SHEET 3
OF 5



MEMBER TABLE		
FRAME LINE A		
MARK	PART	LENGTH
DJ-1	8X35C16	7'-4"
DJ-2	8X35C16	13'-6 5/8"
DJ-4	8X35C16	13'-6 5/8"
DH-1	8X35C16	2'-11 1/2"
DH-2	8X35C16	3'-11 1/2"
DS-1	8X35C16	2'-11 1/2"
DS-2	8X35C16	3'-11 1/2"
E-4	8E14LOW	23'-3 1/2"
E-6	8E14LOW	22'-9 1/4"
E-7	8E14LOW	23'-3 1/2"
E-8	8E14LOW	23'-3 1/2"
E-9	8E14LOW	22'-8 1/4"
G-15	8X25Z16	11'-7 1/2"
G-16	8X25Z16	7'-3 1/2"
G-17	8X25Z16	8'-3 1/2"
G-18	8X25Z16	6'-7 1/2"
G-25	8X25Z16	6'-5"
G-26	8X25Z16	11'-10"
G-27	8X25Z12	10'-7"
G-28	8X25Z12	22'-7 1/2"
G-29	8X25Z12	5'-4"
G-30	8X25Z12	13'-11 1/2"
G-31	8X25Z12	22'-7 1/2"
G-32	8X25Z12	1'-11 1/2"
G-33	8X25Z16	3'-1 1/2"
G-34	8X25Z16	3'-1 1/2"
G-35	8X25Z16	10'-0"
G-36	8X25Z16	3'-5"

TRIM TABLE		
FRAME LINE A		
QID	PART	LENGTH
1	BT-101	10'-3"
2	CT-102	10'-3"
3	GU-124	10'-3"
4	GC-104	10'-3"
5	SCB	8"
6	EC-121	8"
7	JT-101	10'-3"
8	HT-101	10'-3"

MEMBER TABLE		
FRAME LINE F		
MARK	PART	LENGTH
DJ-1	8X35C16	7'-4"
DJ-2	8X35C16	13'-6 5/8"
DH-1	8X35C16	2'-11 1/2"
DS-1	8X35C16	2'-11 1/2"
E-1	8E14LOW	22'-8 1/4"
E-2	8E14LOW	23'-3 1/2"
E-3	8E14LOW	23'-3 1/2"
E-4	8E14LOW	23'-3 1/2"
E-5	8E14LOW	22'-9 1/4"
E-14	8X35Z14	22'-11 1/2"
E-15	8X25Z16	11'-7 1/2"
E-16	8X25Z16	11'-7 1/2"
E-17	8X25Z16	8'-3 1/2"
E-18	8X25Z16	6'-7 1/2"
E-19	8X35Z14	22'-7 1/2"
E-20	8X25Z12	2'-6 1/2"
E-21	8X25Z12	22'-7 1/2"
E-22	8X25Z12	13'-4 1/2"
E-23	8X25Z16	8 1/2"
E-24	8X25Z16	18'-6 1/2"

TRIM TABLE		
FRAME LINE F		
QID	PART	LENGTH
1	BT-101	10'-3"
2	CT-102	10'-3"
3	GU-124	10'-3"
4	GC-104	10'-3"
5	SCB	8"
6	EC-121	8"
7	JT-101	10'-3"
8	HT-101	10'-3"

PROJECT No.
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SCALE: AS NOTED

DRAWN BY:
J. WELLS

APPROVED BY:
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REVISIONS:

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LULU BAPTIST CHURCH
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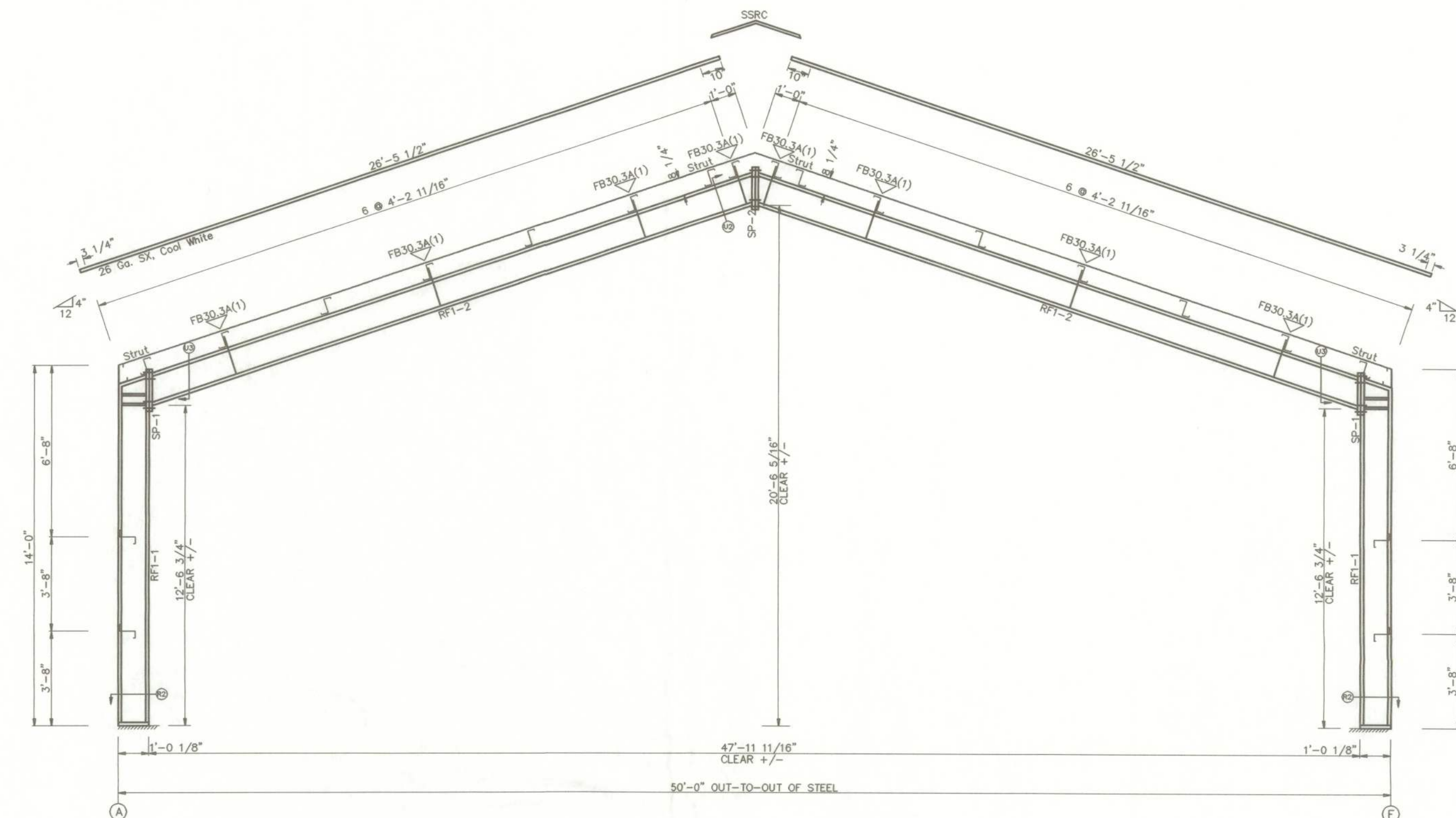


SHEET 4
OF 5

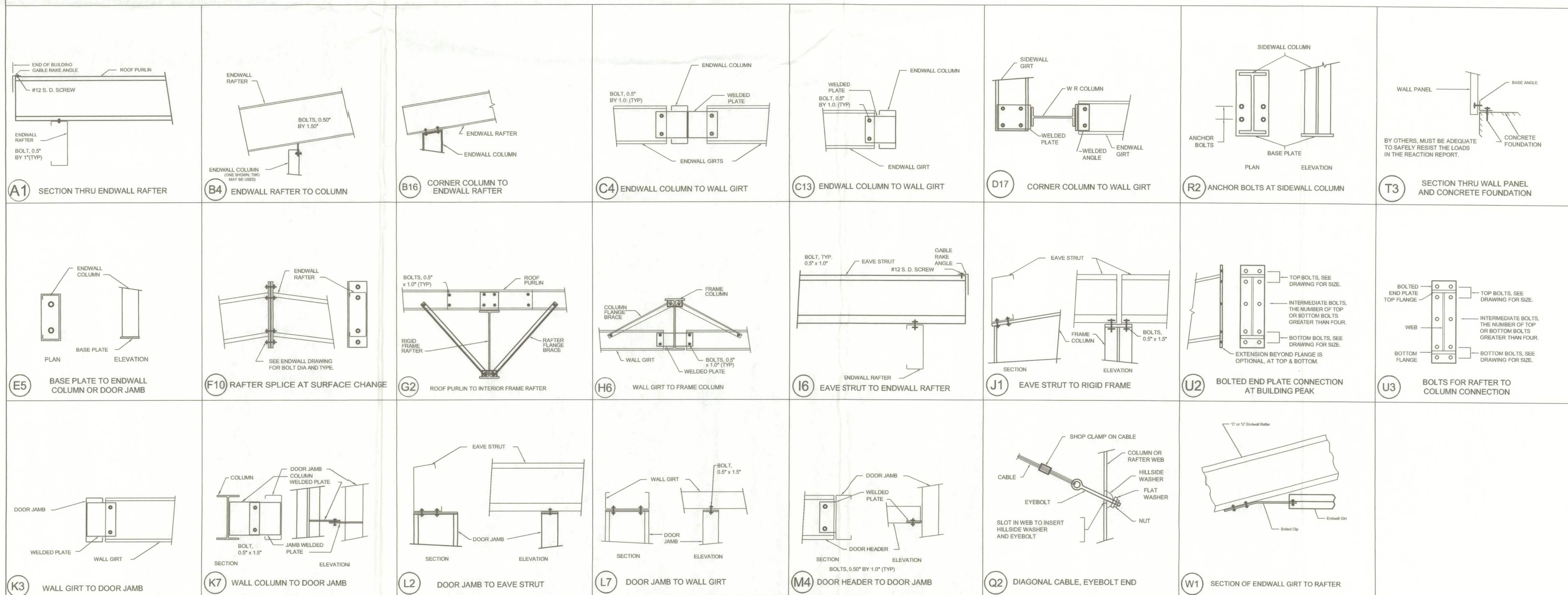
SPlice Bolt Table							
Mark	Qty	Top	Bot	Int	Type	Dia	Length
SP-1	4	4	0	0	A325	0.750	2.25
SP-2	4	4	0	0	A325	0.750	1.75

▽ FLANGE BRACES: Both Sides (U.N.)
 FBxxA(1): xx=length(in)
 A ~ L2X15X12

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF1-1	W12X19	13'-3 5/16"
RF1-2	W12X19	25'-3 7/16"



RIGID FRAME ELEVATION: FRAME LINE 2 3 4 5 6
 SCALE: 1/4" = 1'-0"



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APPROVED BY:
P. SANTORA

REVISIONS:

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 LULU BAPTIST CHURCH
 RIGID FRAME/DETAILS
 50'-0" X 140'-0" BUILDING
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