DATE 11/07/2005

Columbia County Building Permit

PERMIT 000023823

This Permit Expires One Year From the Date of Issue

APPLICANT HUGO ESCALANTE PHONE 386-288-8666

ADDRESS 6210 SW CR 18 FORT WHITE FL 32038

OWNER KAPTAIN 2, LLC PHONE 386-288-8666

ADDRESS 398 SW WISE DRIVE LAKE CITY FL 32025

CONTRACTOR HUGO ESCALANTE PHONE 386-288-8666

LOCATION OF PROPERTY 47 S, R 242, R WISE DRIVE, FOLLOW TO END ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 72900.00

HEATED FLOOR AREA 1458.00 TOTAL AREA 2104.00 HEIGHT 15.10 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 24-4S-16-03113-145 SUBDIVISION WISE ESTATES

LOT 15 BLOCK C PHASE UNIT TOTAL ACRES .58

000000879		CRC1326967	Hugo Escalante	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
PERMIT	05-0979-N	BK	JH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: NOC ON FILE, MINIMUM FLOOR ELEVATION SET @ 93 FT, NEED ELEVATION LETTER BEFORE SLAB

Check # or Cash 3348

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 365.00 CERTIFICATION FEE $ 10.52 SURCHARGE FEE $ 10.52

MISC. FEES $ .00 ZONING CERT. FEE $ 50.00 FIRE FEE $ .00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 486.04

INSPECTORS OFFICE _____ CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Donald F. Lee & Associates, Inc.

Surveyors & Engineers

140 NW Ridgewood Avenue
Lake City, Florida 32055
(386) 755-6166
Fax (386) 755-6167
dfla@suwanneevalley.net

Wednesday, December 28, 2005

TO: EWPL, Inc. – Hugo Escalante

CC: Columbia County Building Department

FROM: Tim Delbene, P.L.S. – Donald F. Lee & Associates, Inc.

23823

RE: Lot 15, Block C, Wise Estates - Elevation check

This letter is to certify that the elevation was measured for the finished floor (at Stemwall) for a house under construction on the above referenced Lot in Wise Estates. The Elevations are as follows:

House Floor: 93.52 - Adjacent grades: 91.4 (lowest) & 91.7 (highest)

The property lies in Flood Zone "X" per Flood Insurance Rate Maps (FIRM). No base flood elevation (BFE) is established for this area. The project Engineer for Wise Estates subdivision, has set the minimum floor elevation for Lot 15, Block C at 93.0 feet (data per record plat).

Timothy A. Delbene, P.L.S.
Florida Cert. No. LS 5594

DATE: 12/28/2005

Donald F. Lee & Associates, Inc.

FEDERAL EMERGENCY MANAGEMENT AGENCY
NATIONAL FLOOD INSURANCE PROGRAM

O.M.B. No. 3067-0077
Expires December 31, 2005

ELEVATION CERTIFICATE

Important: Read the instructions on pages 1 - 7.

SECTION A - PROPERTY OWNER INFORMATION

For Insurance Company Use:

BUILDING OWNER'S NAME
EWPL, Inc. - Hugo Escalante

Policy Number

BUILDING STREET ADDRESS (Including Apt., Unit, Suite, and/or Bldg. No.) OR P.O. ROUTE AND BOX NO.
SW Wise Drive

Company NAIC Number

CITY	STATE	ZIP CODE
Lake City	FL	32025

PROPERTY DESCRIPTION (Lot and Block Numbers, Tax Parcel Number, Legal Description, etc.)
Lot 15, Block "C" - Wise Estates - Plat Bk 7, Pages 164-167

BUILDING USE (e.g., Residential, Non-residential, Addition, Accessory, etc. Use a Comments area, if necessary.)
Residential

LATITUDE/LONGITUDE (OPTIONAL) (##° - ## - ##.## or ##.#####°) HORIZONTAL DATUM: ☐ NAD 1927 ☐ NAD 1983 SOURCE: ☐ GPS (Type):____ ☐ USGS Quad Map ☐ Other: ____

SECTION B - FLOOD INSURANCE RATE MAP (FIRM) INFORMATION

B1. NFIP COMMUNITY NAME & COMMUNITY NUMBER	B2. COUNTY NAME	B3. STATE
Columbia County, Florida 120070	Columbia	Florida

B4. MAP AND PANEL NUMBER	B5. SUFFIX	B6. FIRM INDEX DATE	B7. FIRM PANEL EFFECTIVE/REVISED DATE	B8. FLOOD ZONE(S)	B9. BASE FLOOD ELEVATION(S) (Zone AO, use depth of flooding)
120070 0175	B	1/6/1988	1/6/1988	X	Sec.D

B10. Indicate the source of the Base Flood Elevation (BFE) data or base flood depth entered in B9.
☐ FIS Profile ☐ FIRM ☐ Community Determined ☒ Other (Describe): No BFE

B11. Indicate the elevation datum used for the BFE in B9: ☐ NGVD 1929 ☐ NAVD 1988 ☒ Other (Describe): No BFE

B12. Is the building located in a Coastal Barrier Resources System (CBRS) area or Otherwise Protected Area (OPA)? ☐ Yes ☒ No Designation Date____

SECTION C - BUILDING ELEVATION INFORMATION (SURVEY REQUIRED)

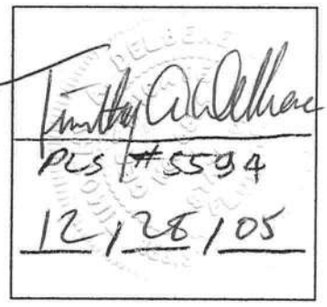
C1. Building elevations are based on: ☐ Construction Drawings* ☒ Building Under Construction* ☐ Finished Construction
*A new Elevation Certificate will be required when construction of the building is complete.

C2. Building Diagram Number 1 (Select the building diagram most similar to the building for which this certificate is being completed - see pages 6 and 7. If no diagram accurately represents the building, provide a sketch or photograph.)

C3. Elevations – Zones A1-A30, AE, AH, A (with BFE), VE, V1-V30, V (with BFE), AR, AR/A, AR/AE, AR/A1-A30, AR/AH, AR/AO
Complete Items C3.-a-i below according to the building diagram specified in Item C2. State the datum used. If the datum is different from the datum used for the BFE in Section B, convert the datum to that used for the BFE. Show field measurements and datum conversion calculation. Use the space provided or the Comments area of Section D or Section G, as appropriate, to document the datum conversion.
Datum NAVD 1988 Conversion/Comments per subdivision design benchmarks
Elevation reference mark used Local Does the elevation reference mark used appear on the FIRM? ☐ Yes ☒ No

- o a) Top of bottom floor (including basement or enclosure) 93. 52 ft.(m)
- o b) Top of next higher floor N/A. __ ft.(m)
- o c) Bottom of lowest horizontal structural member (V zones only) N/A. __ ft.(m)
- o d) Attached garage (top of slab) N/A. _ ft.(m)
- o e) Lowest elevation of machinery and/or equipment servicing the building (Describe in a Comments area) N/A. _ ft.(m)
- o f) Lowest adjacent (finished) grade (LAG) 91. 4 ft.(m)
- o g) Highest adjacent (finished) grade (HAG) 91. 7 ft.(m)
- o h) No. of permanent openings (flood vents) within 1 ft. above adjacent grade N/A
- o i) Total area of all permanent openings (flood vents) in C3.h N/A sq. in. (sq. cm)

License Number, Embossed Seal, Signature, and Date



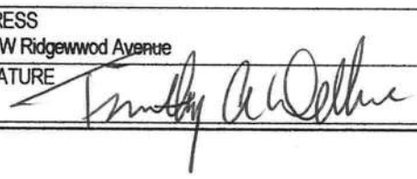
SECTION D - SURVEYOR, ENGINEER, OR ARCHITECT CERTIFICATION

This certification is to be signed and sealed by a land surveyor, engineer, or architect authorized by law to certify elevation information.
I certify that the information in Sections A, B, and C on this certificate represents my best efforts to interpret the data available.
I understand that any false statement may be punishable by fine or imprisonment under 18 U.S. Code, Section 1001.

CERTIFIER'S NAME Timothy A. Delbene, PSM LICENSE NUMBER LS 5594

TITLE Land Surveyor COMPANY NAME Donald F. Lee & Associates, Inc.

ADDRESS	CITY	STATE	ZIP CODE
140 NW Ridgewwod Avenue	Lake City	FL	32055

SIGNATURE  DATE 12/28/2005 TELEPHONE 386-755-6166

IMPORTANT: In these spaces, copy the corresponding information from Section A.			For Insurance Company Use:
BUILDING STREET ADDRESS (Including Apt., Unit, Suite, and/or Bldg. No.) OR P.O. ROUTE AND BOX NO. SW Wise Drive - Lot 15, Blk C Wise Estates			Policy Number
CITY Lake City	STATE FL	ZIP CODE 32025	Company NAIC Number

SECTION D - SURVEYOR, ENGINEER, OR ARCHITECT CERTIFICATION (CONTINUED)

Copy both sides of this Elevation Certificate for (1) community official, (2) insurance agent/company, and (3) building owner.

COMMENTS

Foundation is under construction. Elevation is on stemwall.

Minimum Floor Elevation is 93.0 - per subdivision engineer and as shown on plat of record.

No Base Flood Elevation (BFE) is established in this area. Lot is in Flood Zone "X". ☐ Check here if attachments

SECTION E - BUILDING ELEVATION INFORMATION (SURVEY NOT REQUIRED) FOR ZONE AO AND ZONE A (WITHOUT BFE)

For Zone AO and Zone A (without BFE), complete Items E1 through E4. If the Elevation Certificate is intended for use as supporting information for a LOMA or LOMR-F, Section C must be completed.

E1. Building Diagram Number _(Select the building diagram most similar to the building for which this certificate is being completed – see pages 6 and 7. If no diagram accurately represents the building, provide a sketch or photograph.)

E2. The top of the bottom floor (including basement or enclosure) of the building is__ ft.(m) __in.(cm) ☐ above or ☐ below (check one) the highest adjacent grade. (Use natural grade, if available).

E3. For Building Diagrams 6-8 with openings (see page 7), the next higher floor or elevated floor (elevation b) of the building is __ ft.(m) __in.(cm) above the highest adjacent grade. Complete items C3.h and C3.i on front of form.

E4. The top of the platform of machinery and/or equipment servicing the building is__ ft.(m) __in.(cm) ☐ above or ☐ below (check one) the highest adjacent grade. (Use natural grade, if available).

E5. For Zone AO only: If no flood depth number is available, is the top of the bottom floor elevated in accordance with the community's floodplain management ordinance?
☐ Yes ☐ No ☐ Unknown. The local official must certify this information in Section G.

SECTION F - PROPERTY OWNER (OR OWNER'S REPRESENTATIVE) CERTIFICATION

The property owner or owner's authorized representative who completes Sections A, B, C (Items C3.h and C3.i only), and E for Zone A (without a FEMA-issued or community-issued BFE) or Zone AO must sign here. *The statements in Sections A, B, C, and E are correct to the best of my knowledge.*

PROPERTY OWNER'S OR OWNER'S AUTHORIZED REPRESENTATIVE'S NAME

ADDRESS | CITY | STATE | ZIP CODE

SIGNATURE | DATE | TELEPHONE

COMMENTS

☐ Check here if attachments

SECTION G - COMMUNITY INFORMATION (OPTIONAL)

The local official who is authorized by law or ordinance to administer the community's floodplain management ordinance can complete Sections A, B, C (or E), and G of this Elevation Certificate. Complete the applicable item(s) and sign below.

G1. ☐ The information in Section C was taken from other documentation that has been signed and embossed by a licensed surveyor, engineer, or architect who is authorized by state or local law to certify elevation information. (Indicate the source and date of the elevation data in the Comments area below.)

G2. ☐ A community official completed Section E for a building located in Zone A (without a FEMA-issued or community-issued BFE) or Zone AO.

G3. ☐ The following information (Items G4-G9) is provided for community floodplain management purposes.

G4. PERMIT NUMBER	G5. DATE PERMIT ISSUED	G6. DATE CERTIFICATE OF COMPLIANCE/OCCUPANCY ISSUED

G7. This permit has been issued for: ☐ New Construction ☐ Substantial Improvement

G8. Elevation of as-built lowest floor (including basement) of the building is: ____.__ft.(m) Datum: ____

G9. BFE or (in Zone AO) depth of flooding at the building site is: ____.__ ft.(m) Datum: ____

LOCAL OFFICIAL'S NAME | TITLE

COMMUNITY NAME | TELEPHONE

SIGNATURE | DATE

COMMENTS

☐ Check here if attachments

Columbia County Building Permit Application

left message 11/3/05

Revised 9-23-04

For Office Use Only Application # 0510-82 Date Received 10/27/05 By G Permit # 879/23823

Application Approved by - Zoning Official BLK Date 03.11.05 Plans Examiner OK JTH Date 11-2-05

Flood Zone X pw plat Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res. Low Den.

Comments PLAT MIN Finished FF EV Letter required 93° Need EH

Applicants Name Hugo Escalante Phone 386-288-8666

Address 6210 S.W. CR 18, Ft White, FL 32038

Owners Name Kaptain 2 LLC Phone 386-288-8666

911 Address 398 S.W. WISE DR, Lake City, FL

Contractors Name Hugo Escalante (CEWPL INC) Phone 386-288-8666

Address 6210 S.W. CR 18, Ford White, FL 32038

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Daniel Shaheen, Lake City, FL 32038

Mortgage Lenders Name & Address None

Circle the correct power company - FL Power & Light - ~~Clay Elec.~~ - Suwannee Valley Elec. - Progressive Energy

Property ID Number 24-4S-16-03113-145 Estimated Cost of Construction 110,000 -

Subdivision Name WISE Estates Lot 15 Block C Unit ____ Phase ____

Driving Directions 47 South to 242 make right, go 2 miles make right at WISE DR follow to end Lot 15 on left.

Type of Construction New Single Family Construction Number of Existing Dwellings on Property 0

Total Acreage .58 Lot Size 1/2 Acre Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 65' Side 30' Side 30' Rear 65'

Total Building Height 15'-10" Number of Stories 1 Heated Floor Area 1458 Sq Ft Roof Pitch 6-12

Porches 181 GARAGE 465 TOTAL 2104

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Hugo Escalante
Owner Builder or Agent (Including Contractor)

05-979-N

Hugo Escalante
Contractor Signature

Contractors License Number CRC1326967

Competency Card Number ____

NOTARY STAMP/SEAL

CARRIE L. REVELLE
MY COMMISSION # DD 181697
EXPIRES: February 13, 2007
Bonded Thru Notary Public Underwriters

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 27th day of October 2005.

Personally known X or Produced Identification ____

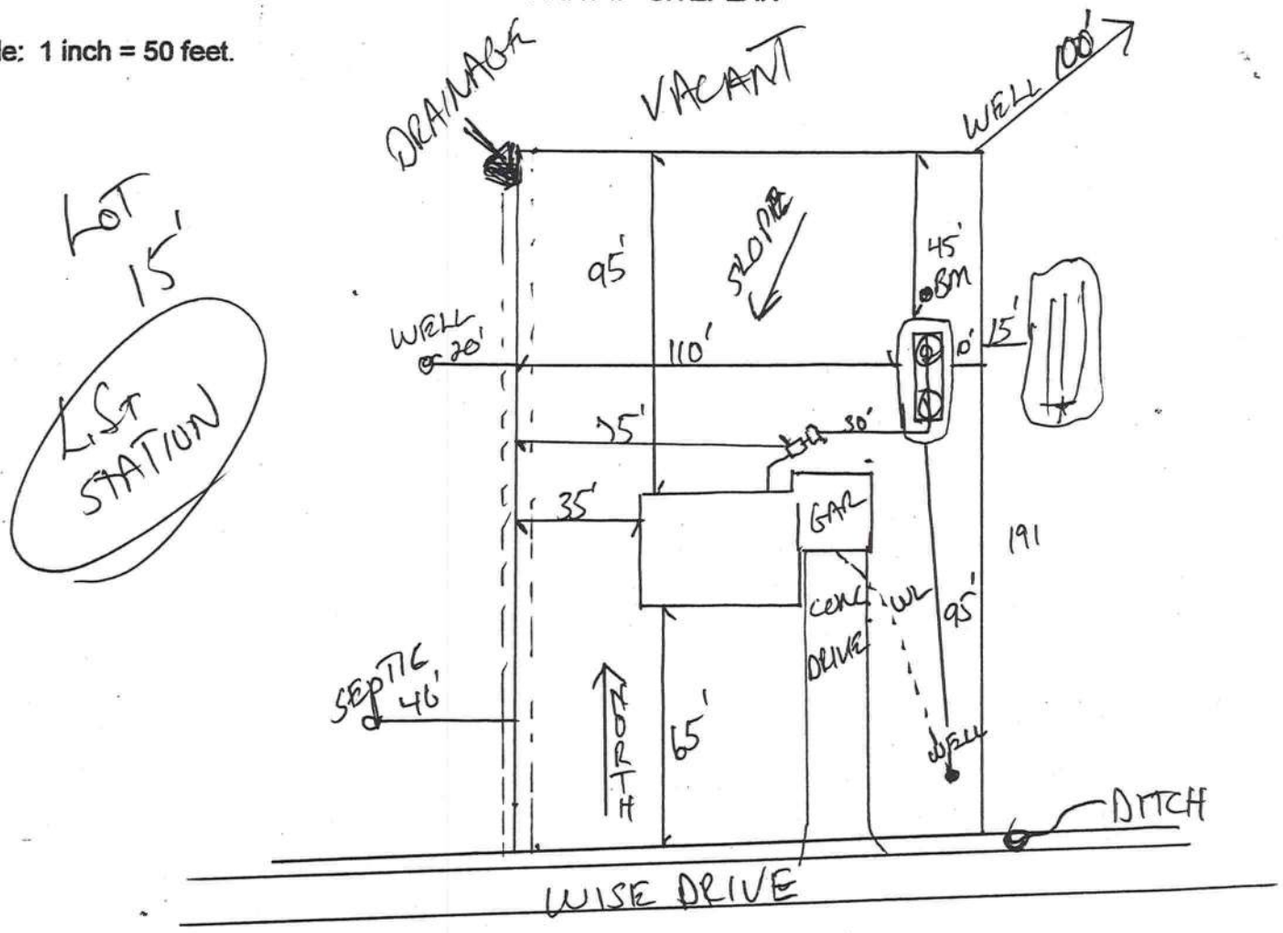
Notary Signature

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 05-0979N

---------- PART II - SITEPLAN ----------

Scale: 1 inch = 50 feet.



Notes: _____

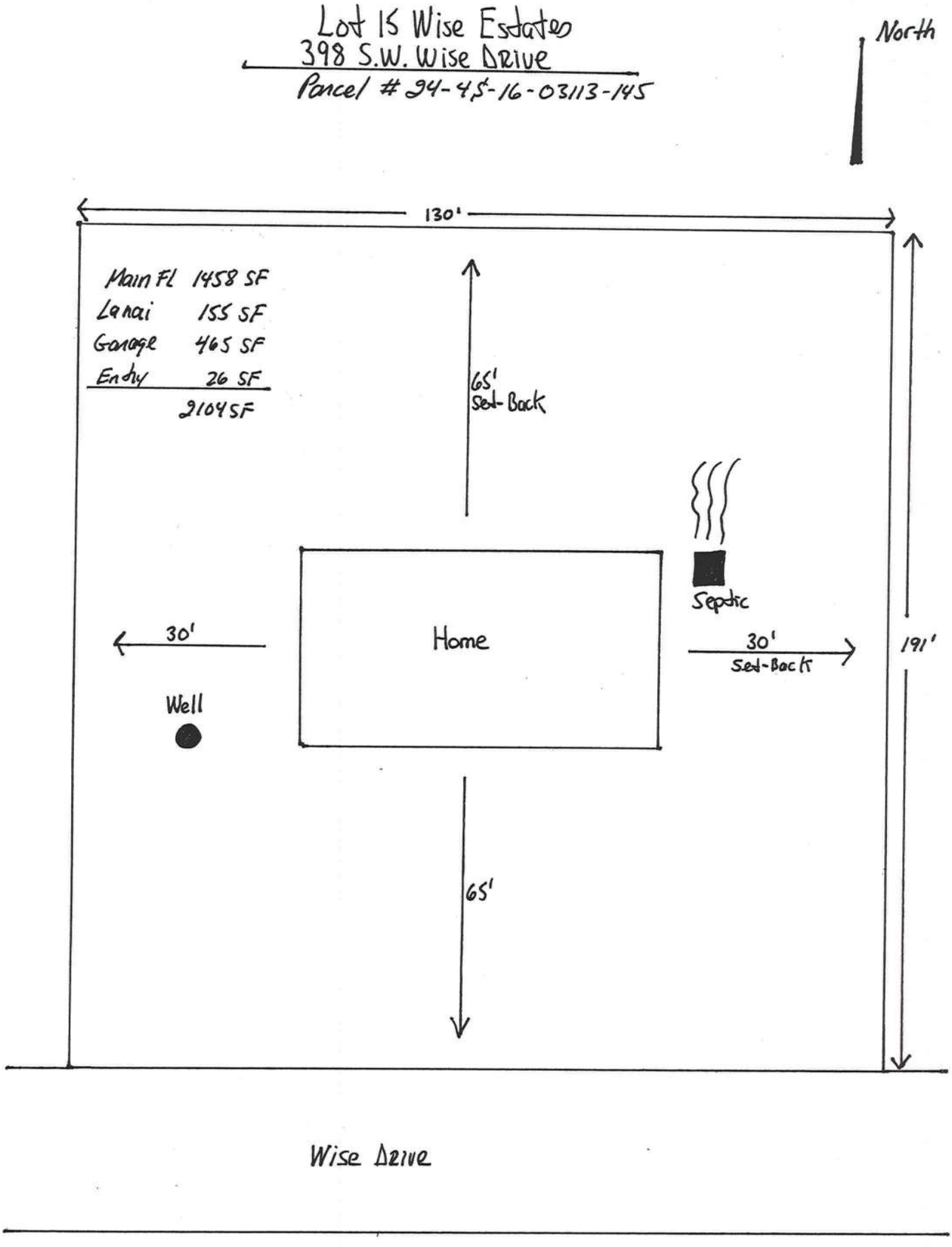
Site Plan submitted by: Rocky Tyo MASTER CONTRACTOR

Plan Approved ✓ Not Approved ____ Date 9-29-05

By _____ Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

Lot 15 Wise Estates
398 S.W. Wise Drive
Parcel # 94-45-16-03113-145



Columbia County Property Appraiser

DB Last Updated: 10/21/2005

Parcel: 24-4S-16-03113-145

398 SW WISE PR

2006 Proposed Values

Tax Record | Property Card | Interactive GIS Map | Print

<< Prev Search Result: 2 of 4 Next >>

Owner & Property Info

Owner's Name	KAPTAIN 2 LLC
Site Address	WISE
Mailing Address	P O BOX 1510 LAKE CITY, FL 32056
Brief Legal	LOT 15 BLOCK C WISE ESTATE S/D WD 1017-499, 1036-2159. WD 1043-1098.

Use Desc. (code)	VACANT (000000)
Neighborhood	24416.00
Tax District	2
UD Codes	MKTA06
Market Area	06
Total Land Area	0.580 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	$20,500.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$20,500.00

Just Value	$20,500.00
Class Value	$0.00
Assessed Value	$20,500.00
Exempt Value	$0.00
Total Taxable Value	$20,500.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
4/14/2005	1043/1098	WD	V	U	01	$100.00
1/28/2005	1036/2159	WD	V	Q		$82,500.00
5/28/2004	1017/499	WD	V	Q		$165,800.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1.000 LT - (.580AC)	1.00/1.00/1.00/1.00	$20,500.00	$20,500.00

Columbia County Property Appraiser

DB Last Updated: 10/21/2005

<< Prev 2 of 4 Next >>

LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave
Lake City, FL. 32025
Phone 386-752-6677
Fax 386-752-1477

Lot 15 WISE

Building Permit # _____ Owner's Name Kapkun 2LLC

Well Depth _____ Ft. Casing Depth _____ Ft. Water Level _____ Ft.

Casing Size 4 inch Steel Pump Installation: Deep Well Submersible

Pump Make Red Jacket Pump Model 100F21120G8 HP 1

System Pressure (PSI) _____ On 30 Off 50 Average Pressure ~~XXX~~ 40

Pumping System GPM at average pressure and pumping level 20 (GPM)

Tank Installation: Precharged Bladder Make Challenger Model PC244 Size _____

Tank Draw-down per cycle at system pressure 26.1 gallons

I HEREBY VERTIFY THAT THIS WATER WELL SYSTEM HAS BEEN INSTALLED AS PER THE ABOVE INFORMATION.

Linda Newcomb
Signature

Linda Newcomb
Print Name

2609
License Number

6-6-05
Date

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Lot 15 Wise, Thomas 2-Car**	Builder:	**EWPL INC.**
Address:	**Lot: 15, Sub: Wise Estates, Plat:**	Permitting Office:	Columbia County
City, State:	**Lake City, FL**	Permit Number:	23823
Owner:	**Kingdom Properties**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 30.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 12.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	Yes		
6. Conditioned floor area (ft²)	1458 ft²	c. N/A	
7. Glass area & type			
a. Clear - single pane	0.0 ft²	13. Heating systems	
b. Clear - double pane	230.5 ft²	a. Electric Heat Pump	Cap: 30.0 kBtu/hr
c. Tint/other SHGC - single pane	0.0 ft²		HSPF: 6.80
d. Tint/other SHGC - double pane	0.0 ft²	b. N/A	
8. Floor types			
a. Slab-On-Grade Edge Insulation	R=0.0, 232.0(p) ft	c. N/A	
b. N/A			
c. N/A		14. Hot water systems	
9. Wall types		a. Electric Resistance	Cap: 50.0 gallons
a. Frame, Wood, Exterior	R=13.0, 1972.0 ft²		EF: 0.88
b. Frame, Wood, Adjacent	R=13.0, 120.0 ft²	b. N/A	
c. N/A			
d. N/A		c. Conservation credits	
e. N/A		(HR-Heat recovery, Solar DHP-Dedicated heat pump)	
10. Ceiling types		15. HVAC credits	CF,
a. Under Attic	R=30.0, 1458.0 ft²	(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 92.0 ft		
b. N/A			

Glass/Floor Area: 0.16

Total as-built points: 23705
Total base points: 26057

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 7-13-05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA
IN GOD WE TRUST

BUILDING OFFICIAL: _____

DATE: _____

EnergyGauge® (Version: FLRCPB v3.2)

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 15, Sub: Wise Estates, Plat: , Lake City, FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 15, Sub: Wise Estates, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2746.00	8238.0	50.0	0.88	3	1.00	2746.00	1.00	8238.0	
				As-Built Total:						**8238.0**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
7649	**10170**	**8238**	**26057**	**5541**	**9925**	**8238**	**23705**

PASS

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 15, Sub: Wise Estates, Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT						
Winter Base Points:		**16210.4**	**Winter As-Built Points:**						**17030.7**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
16210.4	**0.6274**	**10170.4**	17030.7 **17030.7**	1.000 **1.00**	(1.069 x 1.169 x 0.93) **1.162**	0.501 **0.501**	1.000 **1.000**	9925.5 **9925.5**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 15, Sub: Wise Estates, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1458.0	12.74	3343.5	Double, Clear	NW	1.5	6.0	60.0	14.03	1.00	844.4	
				Double, Clear	SW	1.5	6.0	50.0	7.17	1.06	380.1	
				Double, Clear	SW	1.5	6.0	15.0	7.17	1.06	114.0	
				Double, Clear	SE	1.5	6.0	15.0	5.33	1.10	87.7	
				Double, Clear	SE	1.5	6.0	20.0	5.33	1.10	116.9	
				Double, Clear	NE	1.5	6.0	60.0	13.40	1.01	809.0	
				Double, Clear	NE	1.5	4.5	10.5	13.40	1.01	142.5	
				As-Built Total:				**230.5**			**2494.6**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	120.0	3.60	432.0	Frame, Wood, Exterior			13.0	1972.0	3.40		6704.8	
Exterior	1972.0	3.70	7296.4	Frame, Wood, Adjacent			13.0	120.0	3.30		396.0	
Base Total:	**2092.0**		**7728.4**	**As-Built Total:**				**2092.0**			**7100.8**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	18.0	11.50	207.0	Adjacent Wood				18.0	11.50		207.0	
Exterior	60.0	12.30	738.0	Exterior Wood				60.0	12.30		738.0	
Base Total:	**78.0**		**945.0**	**As-Built Total:**				**78.0**			**945.0**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1458.0	2.05	2988.9	Under Attic			30.0	1458.0	2.05 X	1.00	2988.9	
Base Total:	**1458.0**		**2988.9**	**As-Built Total:**				**1458.0**			**2988.9**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	232.0(p)	8.9	2064.8	Slab-On-Grade Edge Insulation			0.0	232.0(p	18.80		4361.6	
Raised	0.0	0.00	0.0									
Base Total:			**2064.8**	**As-Built Total:**				**232.0**			**4361.6**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	1458.0	-0.59	-860.2					1458.0	-0.59		-860.2	

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 15, Sub: Wise Estates, Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT					
Summer Base Points:		**17929.4**	**Summer As-Built Points:**					**18026.2**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
17929.4	**0.4266**	**7648.7**	18026.2 **18026.2**	1.000 **1.00**	(1.090 x 1.147 x 0.91) **1.138**	0.284 **0.284**	0.950 **0.950**	5541.3 **5541.3**

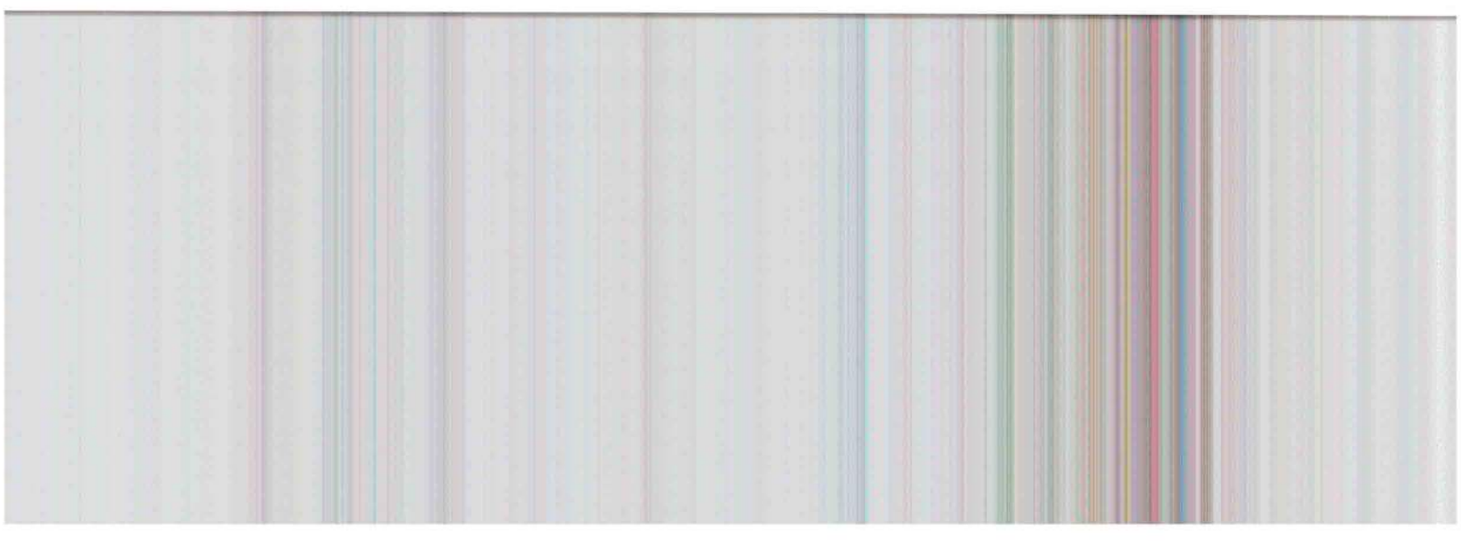
FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 15, Sub: Wise Estates, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	1458.0	20.04	5259.3	Double, Clear	NW	1.5	6.0	60.0	25.46	0.93	1413.8
				Double, Clear	SW	1.5	6.0	50.0	38.46	0.89	1702.2
				Double, Clear	SW	1.5	6.0	15.0	38.46	0.89	510.7
				Double, Clear	SE	1.5	6.0	15.0	40.86	0.88	541.4
				Double, Clear	SE	1.5	6.0	20.0	40.86	0.88	721.9
				Double, Clear	NE	1.5	6.0	60.0	28.72	0.92	1586.6
				Double, Clear	NE	1.5	4.5	10.5	28.72	0.86	260.4
				As-Built Total:				**230.5**			**6736.9**
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Adjacent	120.0	0.70	84.0	Frame, Wood, Exterior			13.0	1972.0	1.50		2958.0
Exterior	1972.0	1.70	3352.4	Frame, Wood, Adjacent			13.0	120.0	0.60		72.0
Base Total:	**2092.0**		**3436.4**	**As-Built Total:**				**2092.0**			**3030.0**
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points
Adjacent	18.0	2.40	43.2	Adjacent Wood				18.0	2.40		43.2
Exterior	60.0	6.10	366.0	Exterior Wood				60.0	6.10		366.0
Base Total:	**78.0**		**409.2**	**As-Built Total:**				**78.0**			**409.2**
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points
Under Attic	1458.0	1.73	2522.3	Under Attic			30.0	1458.0	1.73 X 1.00		2522.3
Base Total:	**1458.0**		**2522.3**	**As-Built Total:**				**1458.0**			**2522.3**
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Slab	232.0(p)	-37.0	-8584.0	Slab-On-Grade Edge Insulation			0.0	232.0(p	-41.20		-9558.4
Raised	0.0	0.00	0.0								
Base Total:			**-8584.0**	**As-Built Total:**				**232.0**			**-9558.4**
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points
	1458.0	10.21	14886.2					1458.0	10.21		14886.2



ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 83.7

The higher the score, the more efficient the home.

Kingdom Properties, Lot: 15, Sub: Wise Estates, Plat: , Lake City, FL,

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family	a.	Central Unit	Cap: 30.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 12.00
4.	Number of Bedrooms	3	b.	N/A	
5.	Is this a worst case?	Yes			
6.	Conditioned floor area (ft²)	1458 ft²	c.	N/A	
7.	Glass area & type				
a.	Clear - single pane	0.0 ft²	13.	Heating systems	
b.	Clear - double pane	230.5 ft²	a.	Electric Heat Pump	Cap: 30.0 kBtu/hr
c.	Tint/other SHGC - single pane	0.0 ft²			HSPF: 6.80
d.	Tint/other SHGC - double pane	0.0 ft²	b.	N/A	
8.	Floor types				
a.	Slab-On-Grade Edge Insulation	R=0.0, 232.0(p) ft	c.	N/A	
b.	N/A				
c.	N/A		14.	Hot water systems	
9.	Wall types		a.	Electric Resistance	Cap: 50.0 gallons
a.	Frame, Wood, Exterior	R=13.0, 1972.0 ft²			EF: 0.88
b.	Frame, Wood, Adjacent	R=13.0, 120.0 ft²	b.	N/A	
c.	N/A				
d.	N/A		c.	Conservation credits	
e.	N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
a.	Under Attic	R=30.0, 1458.0 ft²	15.	HVAC credits	CF,
b.	N/A			(CF-Ceiling fan, CV-Cross ventilation,	
c.	N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 92.0 ft		RB-Attic radiant barrier,	
b.	N/A			MZ-C-Multizone cooling,	
				MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

EnergyGauge® (Version: FLRCPB v3.2)

From: The Columbia County Building Department
Plans Review
135 NE Hernando Av.
P. O Box 1529
Lake City Florida, 32056-1529

Reference to: Build permit application Number: 0510-82

Hugo Escalate Owner Kaptain 2 LLC Lot 15 Block C of Wise Estates

On the date of November 2, 2005 application 0510-82 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0510-82 when making reference to this application.

1. Application 0510-82 which was filed with the building department on the date of October 27, 2005 will be reviewed under the Florida Building Code 2004. The Wind Load design by Mr. Mark Disosway was design under the Florida Building Code 2001. The wind Load design should reflect the code sections of the Florida Building Code 2004 that relate to wind Load design code requirements.

2. Please show compliance with sections R309 of the FRC-2004 R309.1 Opening protection. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors. R309.1.1 Duct penetration: Ducts in the garage and ducts penetrating the

walls or ceilings separating the dwelling from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other approved material and shall have no openings into the garage. R309.2 Separation required: The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent.

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 24-4S-16-03113-145

1. **Description of property: (legal description of the property and street address or 911 address)**
Lot 15 Block "C" WISE ESTATE S/D WD 1017-499, 1036-259
WD 1043-1098

2. **General description of improvement:** New Single Family Dwelling

3. **Owner Name & Address** Kaptain 2 LLC, P.O. BOX 1510 Lake City, FL 32056 **Interest in Property** 100%

4. **Name & Address of Fee Simple Owner (if other than owner):** None

5. **Contractor Name** Hugo Escalante (EWPL INC) **Phone Number** 386-288-8666
Address 6210 S.W. CR 18, Fort White, FL 32038

6. **Surety Holders Name** None **Phone Number** N/A
Address
Amount of Bond

7. **Lender Name** **Phone Number**
Address

8. **Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:**
Name Hugo Escalante **Phone Number** 386-288-8666
Address 6210 S.W. CR 18, Fort White, FL 32038

9. **In addition to himself/herself the owner designates** Hugo Escalante **of** Fort White, FL **to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) – (a) 7. Phone Number of the designee** 386-288-8666

10. **Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified)**

NOTICE AS PER CHAPTER 713, Florida Statutes:
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Hugo Escalante
Signature of Owner

Sworn to (or affirmed) and subscribed before day of 27th October, 2005

NOTARY STAMP/SEAL

CARRIE L. REVELLE
MY COMMISSION # DD 181697
EXPIRES: February 11, 2007
Bonded Thru Notary Public Underwriters

Carrie L. Revelle
Signature of Notary

Inst:2005026826 Date:10/27/2005 Time:10:30
MK DC,P.DeWitt Cason,Columbia County B:1063 P:668

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By Marcel Keen
Deputy Clerk
Date Oct 27, 2005

CLERK CIRCUIT COURT
COLUMBIA COUNTY, FLORIDA

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000000879

DATE 11/07/2005 PARCEL ID # 24-4S-16-03113-145

APPLICANT HUGO ESCALATE PHONE 386-288-8666

ADDRESS 6210 SW CR 18 FORT WHITE FL 32038

OWNER KAPTAIN 2, LLC PHONE 386-288-8666

ADDRESS 398 SW WISE DRIVE LAKE CITY FL 32025

CONTRACTOR HUGO ESCALANTE PHONE 386-288-8666

LOCATION OF PROPERTY 47 S, R 242, R WISE DRIVE, FOLLOOW TO THE END ON THE LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT WISE ESTATES 15 C

SIGNATURE Hugo Escalante

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

Mark Disosway, P.E.
POB 868, Lake City, FL 32056, Ph (386) 754-5419, Fax (386) 269-4871

November 2, 2005

Building Department

Re: Permit 0510-82, Ewpl Inc / Hugo Escalate, Kaptain Residence, Lot 15 Wise Estates S/D Columbia County, Florida

Dear Building Official:

Please accept this letter as addendum to the plans for the above referenced house to change all references to FBC 2001 to FBC 2004.

- The plan was drawn prior to the effective date for FBC 2004, 01 October 2005.
- Since the wind load requirements of FBC 2004 remain basically unchanged from FBC 2001 there are no structural changes required to this plan.

M h Disosway
07 NOV 05

Mark Disosway, PE
Florida Registered Professional Engineer

Cc Ewpl Inc / Hugo Escalate (Builder)

Residential System Sizing Calculation

Summary

Kingdom Properties

Lake City, FL

Project Title:
Lot 15 Wise, Thomas 2-Car

Code Only
Professional Version
Climate: North

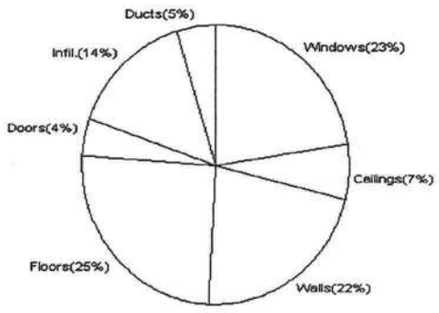
9/13/2005

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**28849**	**Btuh**	**Total cooling load calculation**	**27963**	**Btuh**
Submitted heating capacity	30000	Btuh	Submitted cooling capacity	30000	Btuh
Submitted as % of calculated	104.0	%	Submitted as % of calculated	107.3	%

WINTER CALCULATIONS

Winter Heating Load (for 1458 sqft)

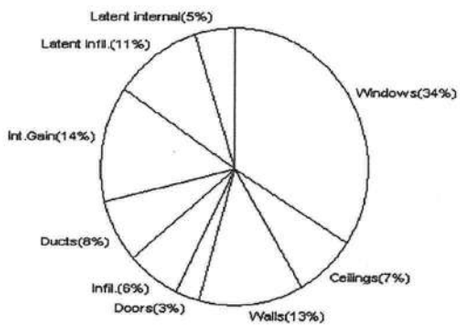
Load component			Load	
Window total	231	sqft	6523	Btuh
Wall total	2092	sqft	6305	Btuh
Door total	78	sqft	1242	Btuh
Ceiling total	1458	sqft	1895	Btuh
Floor total	232	ft	7331	Btuh
Infiltration	97	cfm	4178	Btuh
Subtotal			**27475**	**Btuh**
Duct loss			1374	Btuh
TOTAL HEAT LOSS			**28849**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1458 sqft)

Load component			Load	
Window total	231	sqft	9587	Btuh
Wall total	2092	sqft	3556	Btuh
Door total	78	sqft	778	Btuh
Ceiling total	1458	sqft	2070	Btuh
Floor total			0	Btuh
Infiltration	85	cfm	1687	Btuh
Internal gain			3800	Btuh
Subtotal(sensible)			**21479**	**Btuh**
Duct gain			2148	Btuh
Total sensible gain			**23627**	**Btuh**
Latent gain(infiltration)			2955	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**4335**	**Btuh**
TOTAL HEAT GAIN			**27963**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY:
DATE: 9-13-05

System Sizing Calculations - Winter

Residential Load - Component Details

Kingdom Properties

Lake City, FL

Project Title:
Lot 15 Wise, Thomas 2-Car

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

9/13/2005

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	E	60.0	28.3	1698 Btuh
2	2, Clear, Metal, DEF	N	50.0	28.3	1415 Btuh
3	2, Clear, Metal, DEF	N	15.0	28.3	424 Btuh
4	2, Clear, Metal, DEF	W	15.0	28.3	424 Btuh
5	2, Clear, Metal, DEF	W	20.0	28.3	566 Btuh
6	2, Clear, Metal, DEF	S	60.0	28.3	1698 Btuh
7	2, Clear, Metal, DEF	S	10.5	28.3	297 Btuh
	Window Total		231		6523 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Exterior	13.0	1972	3.1	6113 Btuh
2	Frame - Adjacent	13.0	120	1.6	192 Btuh
	Wall Total		2092		6305 Btuh
Doors	Type		Area X	HTM=	Load
1	Wood - Adjac		18	9.2	166 Btuh
2	Wood - Exter		60	17.9	1076 Btuh
	Door Total		78		1242Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1458	1.3	1895 Btuh
	Ceiling Total		1458		1895Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	232.0 ft(p)	31.6	7331 Btuh
	Floor Total		232		7331 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	14580(sqft)	97	4178 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			97	4178 Btuh

Totals for Heating	**Subtotal**	**27475 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1374 Btuh**
	Total Btuh Loss	**28849 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

Manual J Summer Calculations

Residential Load - Component Details (continued)

Kingdom Properties
Lake City, FL

Project Title:
Lot 15 Wise, Thomas 2-Car

Code Only
Professional Version
Climate: North

9/13/2005

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

System Sizing Calculations - Summer

Residential Load - Component Details

Kingdom Properties
Lake City, FL

Project Title:
Lot 15 Wise, Thomas 2-Car

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F 9/13/2005

Window	Type Panes/SHGC/U/InSh/ExSh	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, DEF, N, N	E	1.5	6	60.0	12.5	47.6	22	72	3698 Btuh
2	2, Clear, DEF, N, N	N	1.5	6	50.0	0.0	50.0	22	22	1100 Btuh
3	2, Clear, DEF, N, N	N	1.5	6	15.0	0.0	15.0	22	22	330 Btuh
4	2, Clear, DEF, N, N	W	1.5	6	15.0	0.7	14.3	22	72	1043 Btuh
5	2, Clear, DEF, N, N	W	1.5	6	20.0	0.5	19.5	22	72	1416 Btuh
6	2, Clear, DEF, N, N	S	1.5	6	60.0	30.0	30.0	22	37	1770 Btuh
7	2, Clear, DEF, N, N	S	1.5	4.5	10.5	10.5	0.0	22	37	231 Btuh
	Window Total				231					9587 Btuh

Walls	Type	R-Value	Area	HTM	Load
1	Frame - Exterior	13.0	1972.0	1.7	3431 Btuh
2	Frame - Adjacent	13.0	120.0	1.0	125 Btuh
	Wall Total		2092.0		3556 Btuh

Doors	Type	Area	HTM	Load
1	Wood - Adjac	18.0	10.0	180 Btuh
2	Wood - Exter	60.0	10.0	599 Btuh
	Door Total	78.0		778 Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load
1	Under Attic/Dark	30.0	1458.0	1.4	2070 Btuh
	Ceiling Total		1458.0		2070 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab-On-Grade Edge Insulation	0.0	232.0 ft(p)	0.0	0 Btuh
	Floor Total		232.0		0 Btuh

Infiltration	Type	ACH	Volume	CFM=	Load
	Natural	0.35	14580	85.2	1687 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			85	1687 Btuh

Internal gain		Occupants		Btuh/occupant		Appliance	Load
		6	X	300	+	2000	3800 Btuh

Totals for Cooling	**Subtotal**	**21479 Btuh**
	Duct gain(using duct multiplier of 0.10)	**2148 Btuh**
	Total sensible gain	**23627 Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**2955 Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380 Btuh**
	Latent other gain	**0 Btuh**
	TOTAL GAIN	**27963 Btuh**

ELK

PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE™

Prestique Plus *High Definition* and Prestique Gallery Collection™

Product size	13¼"x 39⅜"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	11

50-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Raised Profile

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique I *High Definition*

Product size	13¼"x 39⅜"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	14

40-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique *High Definition*

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/*FLX*™

Size: 12"x 12"
Exposure: 6⅓"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors: Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood**, Forest Green, Wedgewood**, Birchwood**, Sandalwood. Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge roofing products contain Elk WindGuard® sealant. WindGuard activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae. Not available in Sablewood.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles meet the latest Metro Dade building code requirements.

***See actual limited warranty for conditions and limitations.**
****Check for product availability.**

SPECIFICATIONS

SCOPE: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color).

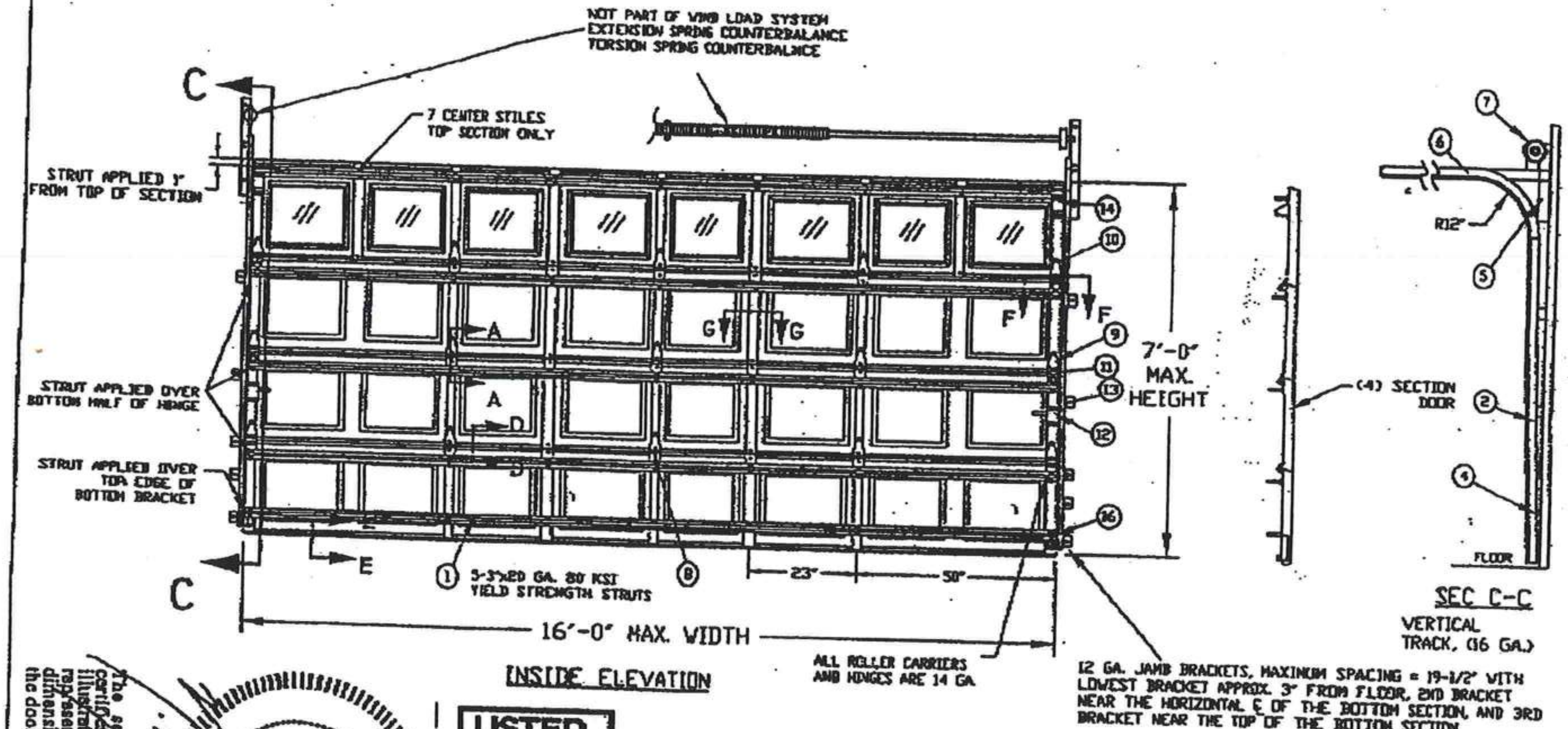
MATERIALS: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater: apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. Eaves

warranties are contingent upon the correct installation as shown on the instructions. These instructions are the

REV.	DATE	BY	DESCRIPTION
A—A	11-10-00	DW	SEE E.C.N. 831

NE

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT= 21"
3. SECTION HEIGHTS OF 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION. (AS TESTED WITH 1/8" DSB GLASS OR EQUIVALENT) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MINIMUM LENGTH OF ROLLER STEM IS 5½" (7" AS TESTED)
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOWN.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE 0,1, OR (2) AS TESTED.
9. △ DROP IN TYPE OF INSULATION IS OPTIONAL



INSIDE ELEVATION

NORTH CAROLINA PROFESSIONAL SEAL PE No. 024280 ENGINEER YASER R. KEYVAN

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.

LISTED SBCCI PST & ESI REPORT No. 2202

TEST REPORTS ON FILE VIDEO 10/19/00 (00293)

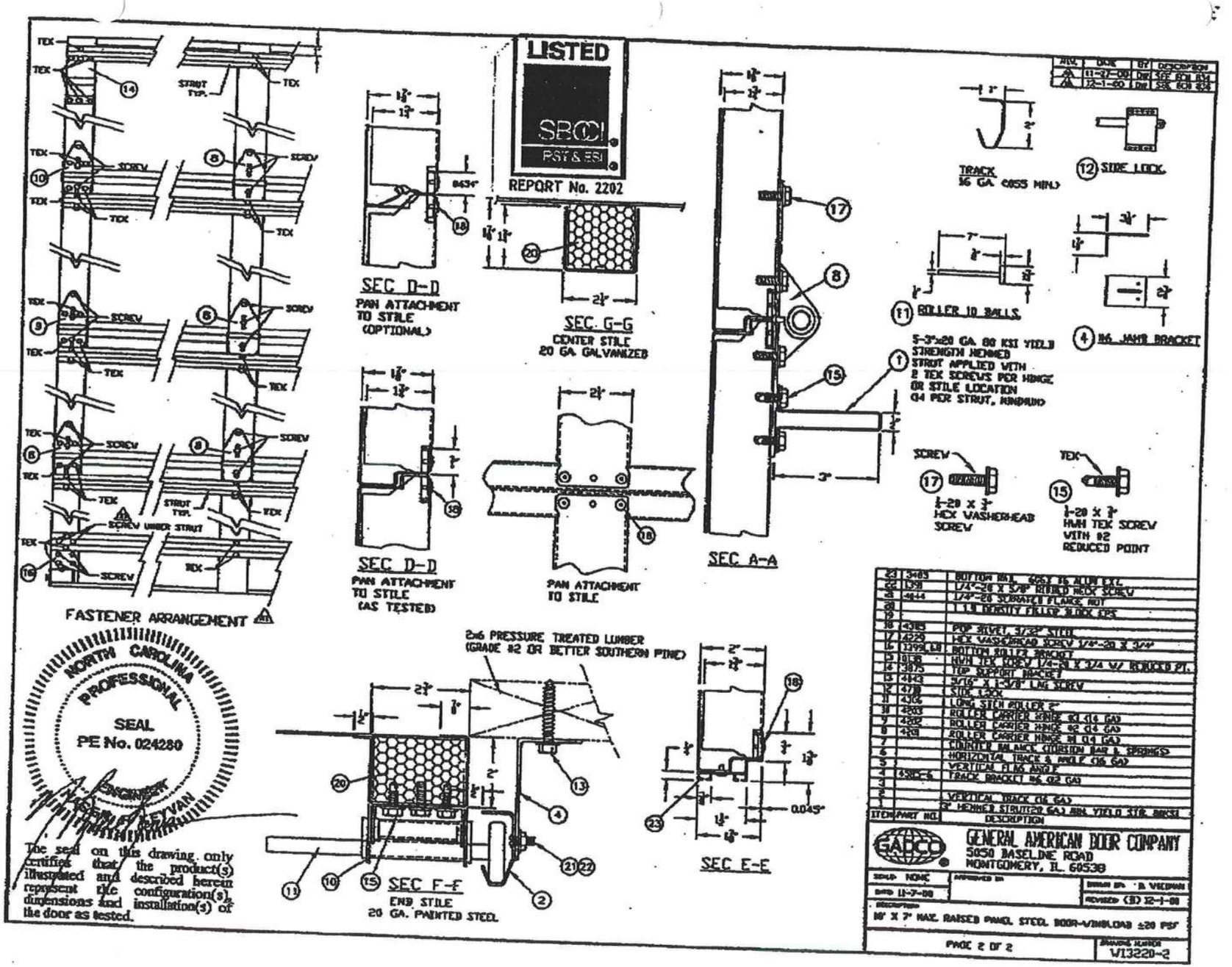
DESIGN LOAD +20.0 PSF & -20.0 PSF
TEST LOAD +30.0 PSF & -30.0 PSF

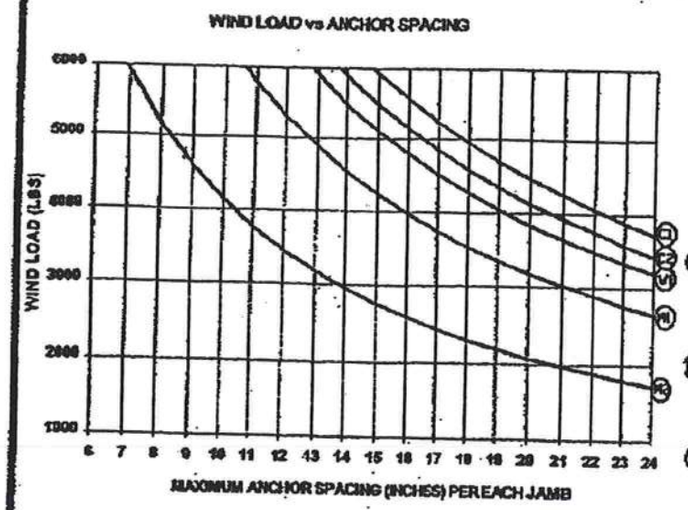
GADCO DOORS
SERIES 7480, EXTERIOR STEEL =.017 MIN (AS TESTED)
SERIES 7825, EXTERIOR STEEL =.019" MIN △
SERIES 7524, EXTERIOR STEEL =.024" MIN △
(TESTED WITH WINDOWS

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CTR. STILE SPACING	STRUTS 80 KSI SIZE	STRUTS 80 KSI QTY.	VERTICAL TRACK
16'	7'	23"	3"	5	2 IN.

GADCO GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: 1/25 | APPROVED BY: | DRAWN BY: D. WIEDMAN
DATE: 10-20-00 | REVISED: (A) 11-10-00
DESCRIPTION: 16' X 7' MAX RAISED PANEL STEEL DOOR -WINDLOAD ±20 PSF
PART NUMBER: | PAGE 1 OF 2 | DRAWING NUMBER: W13220-1





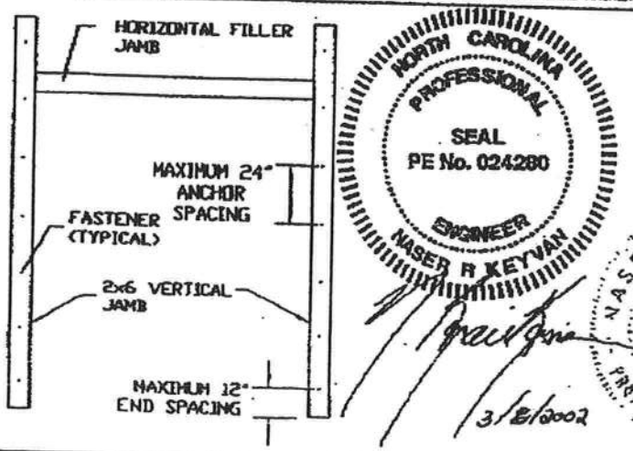
DESIGN LOAD (LBS)/FT² X GARAGE DOOR AREA(WIDTH-FT X HEIGHT-FT) = WIND LOAD(LBS)

EXAMPLE

30 LBS/FT² X (16 FT WIDE X 8 FT HIGH) = 3840 LBS

- C1 USE 22" SPACING
- C2 USE 21" SPACING
- W1 USE 19" SPACING
- M1 USE 16" SPACING
- M2 USE 10" SPACING

SEE NOTE 11 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS



2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION SSTD 10," CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE (#2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.

5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2150 PSI; GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI; REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4"

8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.

9) WASHERS ARE REQUIRED ON ALL FASTENERS.

10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.

11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

GADCO

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

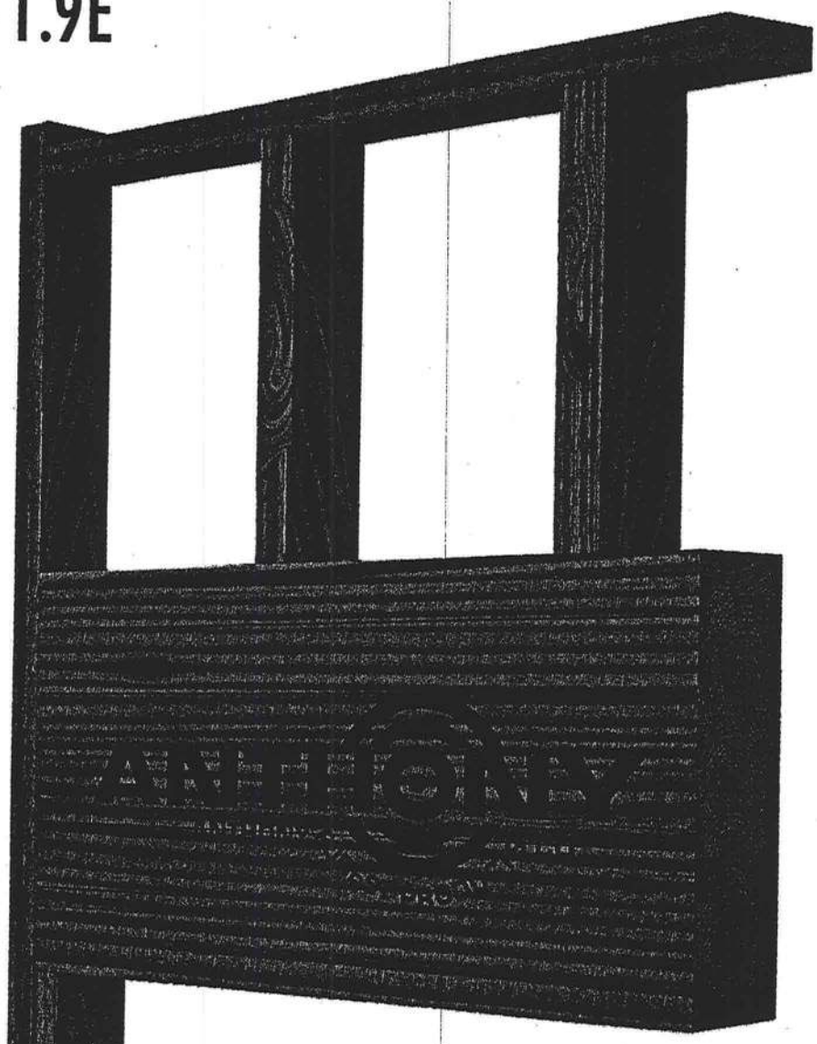
SCALE: NONE	APPROVED BY:	DRAWN BY: DV
DATE: 8-30-99		REVISED

DESCRIPTION: JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS

VENDOR DRAWING NUMBER: A10560

Anthony POWER HEADER®

2600F_b - 1.9E



Anthony POWER HEADER® Advantages

- Less Expensive than LVL or PSL
- Lighter than Steel, LVL or PSL
- Pre-Cut Lengths
- Renewable Resource
- Cambered or Non-cambered
- 3-1/2" Width to Match Framing
- One Piece - No Nail Laminating
- Lifetime Warranty

Garage Header Sizing Tables

ANTHONY®
ANTHONY FOREST PRODUCTS CO.

Anthony Power Header®

$26F_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER APPLICATION - SINGLE STORY

HEADER SUPPORTING: 1/2 ROOF SPAN

SNOW LOAD AREAS USING LOAD DURATION FACTOR = 1.15

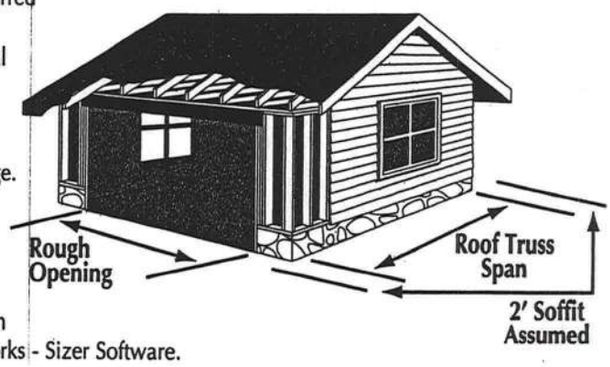
ROOF LIVE LOAD (PSF)	20																	
ROOF DEAD LOAD (PSF)	10																	
ROOF OPENING	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
ROOF RUN SPAN (FT) 6	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	16-3/4
8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	
10	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	9-3/4	15-3/8	
12	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8		9-3/4		
SOFFIT ASSUMED	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	16-3/4	9-3/4	15-3/8		9-3/4		
2'	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4			9-3/4		
	8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4		
	8-3/8	14	15-3/8	8-3/8	15-3/8		8-3/8	15-3/8		9-3/4			9-3/4			11-1/4		
	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4			11-1/4		

NON SNOW LOAD AREAS USING LOAD DURATION FACTOR = 1.25

ROOF LIVE LOAD (PSF)	30														
ROOF DEAD LOAD (PSF)	10														
ROOF OPENING	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
ROOF RUN SPAN (FT) 6	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14
8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14
10	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14
12	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14
SOFFIT ASSUMED	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8
2'	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8
	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8
	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	
	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	

NOTES:

1. Table assumes a simple span header supporting a uniform load transferred from 1/2 the roof span plus a 2' soffit.
2. Roof live and dead loads shown are applied vertically to the horizontal projection. No reductions in roof live loads or snow loads were considered. The header weight is accounted for in the table.
3. Deflection is limited to L/240 for live load and L/180 for total load.
4. Headers are assumed to have continuous lateral support along top edge.
5. Bearing length based on full width bearing is indicated as follows:
   Non-shaded sizes require two trimmers (3" bearing).
   Shaded sizes require three trimmers (4.5" bearing).
   Shaded & outlined sizes require four trimmers (6" bearing).
6. ** Applications where load carrying capacity of 16-3/4" depth has been exceeded. See AFP $30F_b$ POWER BEAM® literature or AFP's WoodWorks - Sizer Software.



Anthony Power Header®

$26F_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

GARAGE HEADER SUPPORTING ROOF LOADS ONLY - 115% SNOW LOAD AREA

844	896		1216		1573						
161	207	254	330	390	510	552	669	752	824		
114	145	180	231	277	359	391	510	534	653	707	789

GARAGE HEADER SUPPORTING ROOF LOADS ONLY - 125% NON-SNOW LOAD AREA

844	975		1322								
161	207	254	330	390	510	552	724	752	897		
114	145	180	231	277	359	391	510	534	699	693	

GARAGE HEADER SUPPORTING ROOF, WALL AND FLOOR LOADS - 100% LOAD DURATION

562	778	888	1056	1363	1367		1582						
107	153	169	245	260	380	368	540	501	715	664	864	840	
76	107	120	171	185	267	261	380	356	521	471	684	609	813

NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

Anthony Power Header®

$26F_b$ - 1.9E

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS (F_b) =	2600
COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) =	740
HORIZONTAL SHEAR (F_v) =	225
MODULUS OF ELASTICITY (MOE) =	1.9×10^6

Standard Beam Width (in.)							
Standard Beam Depth (in.)							
WEIGHT (lbs/ft)	7.7	9.0	10.4	11.7	12.9	14.2	15.5
[illegible]	326	514	789	1115	1521	2014	2604
MOMENT CAPACITY (ft-lbs)	8865	12015	15996	20145	24772	29877	35460
SHEAR CAPACITY (lbs)	3908	4550	5250	5892	6533	7175	7817

NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b, shall be modified by the Volume Factor, C_v, as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1997.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for Power Header®.

GARAGE HEADER COMPARISONS

[illegible]	GARAGE HEADER REQUIRED				
[illegible]					
810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-5/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
640 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-3/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"*
765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"*
750 / 480	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"*
900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	-----

For more information on Power Header®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

Distributed by:

Anthony Forest Products Company
Post Office Box 1877 • El Dorado, Arkansas 71731
Internet address: http:// www.anthonyforest.com
e-mail: info@anthonyforest.com

MI Home Products, Inc.
650 West Market St.
P.O. Box 370
Gratz, PA 17030-0370

(717) 365-3300
(717) 362-7025 Fax

740/744 Single Hung (Fin & Flange)
165 Single Hung (Fin & Flange)
BB165/740/744 Fixed (Fin & Flange)

- Test Reports
  - 165 Single Hung
    - #CTLA-787W (Fin)
    - #CTLA-787W-1 (Flange)
  - 740/744 Single Hung
    - #01-40351.03 (Fin)
    - #01-40351.04 (Flange)
  - 165/740/744 Fixed
    - #NCTL-310-0005-2.1 (Fin)
    - # NCTL-310-0005-5.1 (Flange)
    - #01-40486.03 (2-Panel Fixed)
- Installation Instructions
- Sample 110/120/140 MPH Labels

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 740/744
TYPE: Aluminum Single Hung Window with Nail Fin

Title of Test	Results
Rating	H R45 52 x 72
Overall Design Pressure	45 psf
Operating Force	24 lb max.
Air Infiltration	0.10 cfm/ft^2
Water Resistance	6.75 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-40351.03 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess

Mark A. Hess, Technician

MAH:baw

ALLEN N. REEVES
CERTIFICATE
NO. 19[illegible]
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
15 FEBRUARY 2002

THIS FENESTRATION PRODUCT COMPLIES* WITH THE

NEW FLORIDA BUILDING CODE

FOR RESIDENTIAL BUILDINGS WITH A MEAN ROOF HEIGHT OF 30 FT. OR LESS, *EXPOSURE "B"* (WHICH IS INLAND OF A LINE THAT IS 1500 FT. FROM THE COAST), AND *WALL ZONE "5"* (INSTALLED NEAR THE CORNER OF THE BUILDING).

PER *ASTM E1300*, THE CORRECT GLASS THICKNESS, BASED ON THE *NEGATIVE* DESIGN PRESSURE (DP) LISTED BELOW, HAS BEEN INSTALLED IN THIS UNIT. THE GLASS THICKNESS IS BASED ON ITS' WIDTH, HEIGHT, AND ASPECT RATIO.

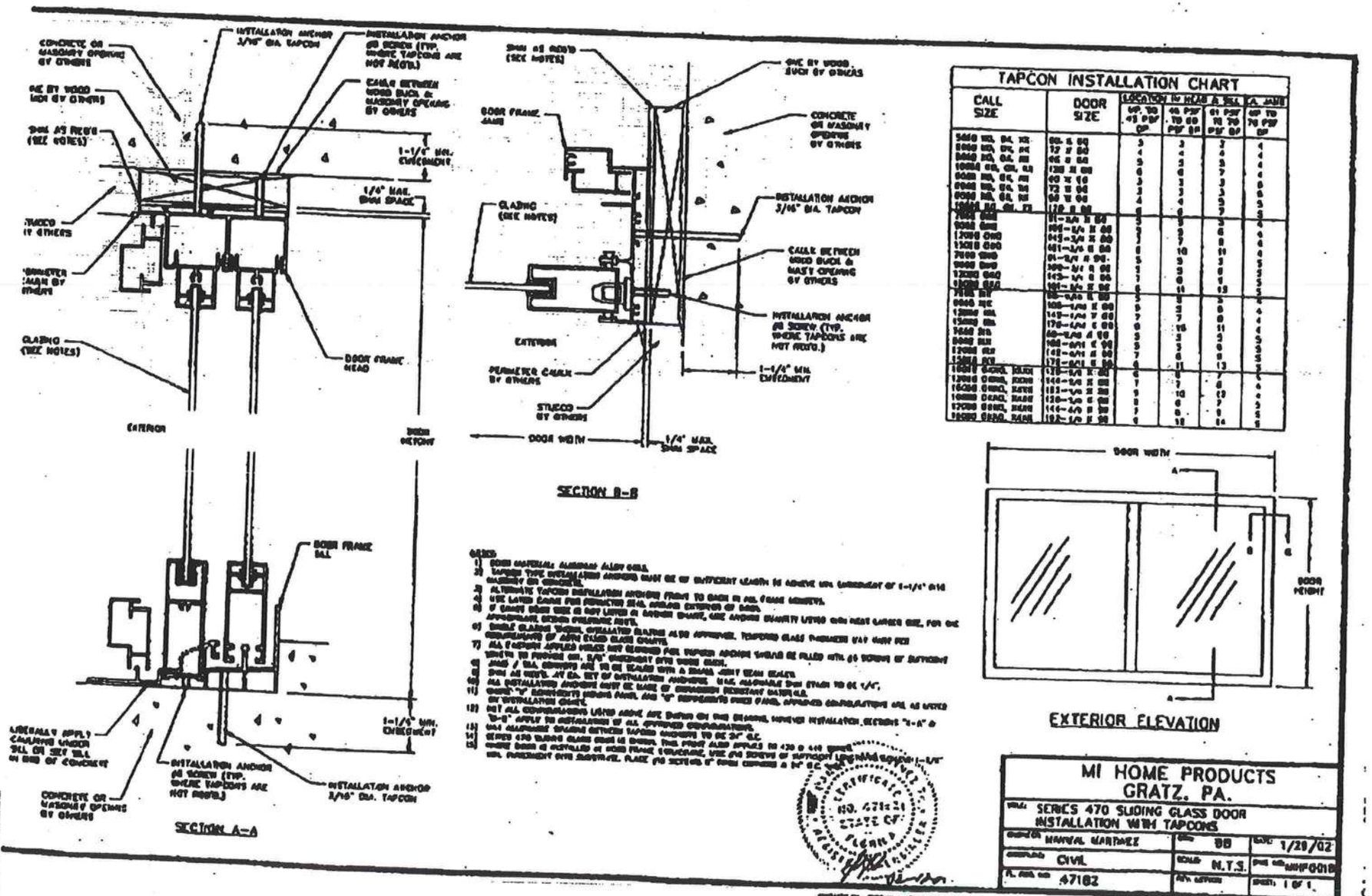
Series 470HP SLIDING GLASS DOOR – all 6'- 8" High Panels

- **2'- 6" WIDE** DP + 40.0 / - 55.4
- **3'- 0" WIDE** DP + 40.0 / - 48.5
- **4'- 0" WIDE** DP + 40.0 / - 40.3

THIS PRODUCT MEETS THE REQUIREMENTS FOR STRUCTURAL LOADS, WATER AND AIR INFILTRATION PER ATTACHED *AAMA* PERFORMANCE LABEL. BE ADVISED THAT IF LOADS ARE PLACED UP TO OR EXCEEDING THE TESTED LEVELS, THIS PRODUCT MAY BE ALTERED IN SUCH A WAY THAT FUTURE PERFORMANCE WILL BE REDUCED.

* COMPLIANCE MUST INCLUDE INSTALLATION ACCORDING TO MANUFACTURER'S INSTRUCTIONS AND FLORIDA CODE REQUIREMENTS.

MIP-686


SECTION A-A
SECTION B-B
EXTERIOR ELEVATION
DOOR WIDTH
DOOR HEIGHT
TAPCON INSTALLATION CHART
MI HOME PRODUCTS
GRATZ, PA.
SERIES 470 SLIDING GLASS DOOR
INSTALLATION WITH TAPCONS
MANUAL MARTINEZ
1/29/02
CIVIL
N.T.S.
47182

Series V83 HORIZONTAL MULLION for SINGLE UNITS - Florida Flange

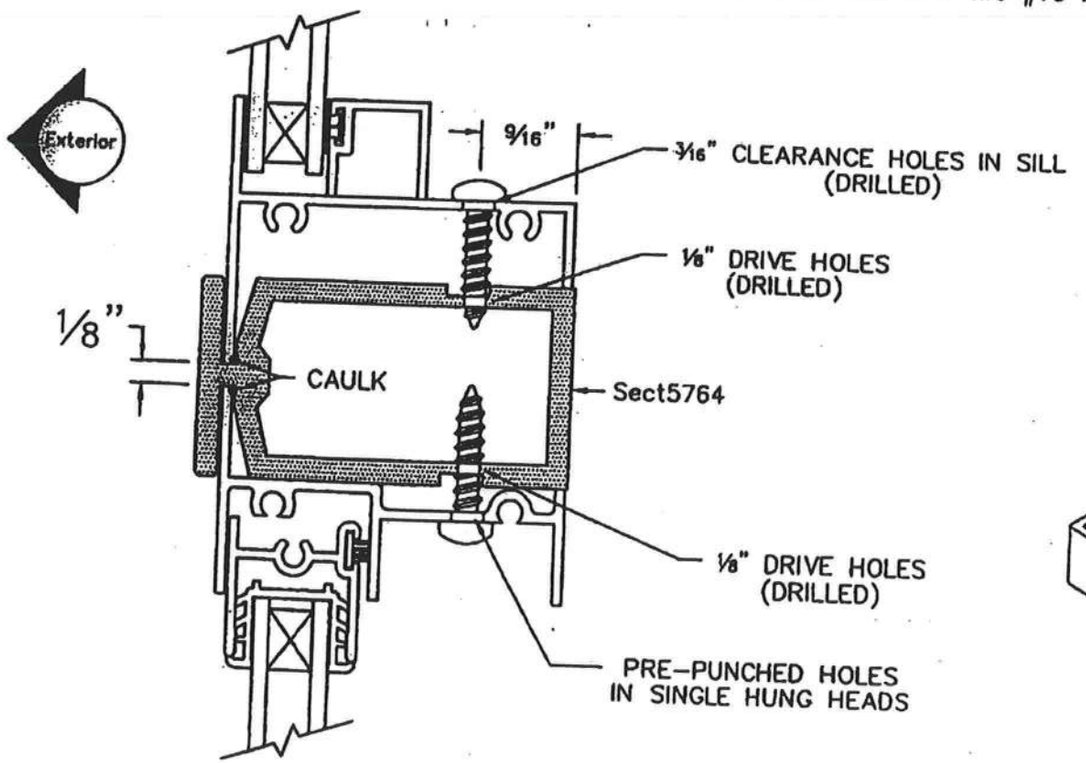
165 & 740/744

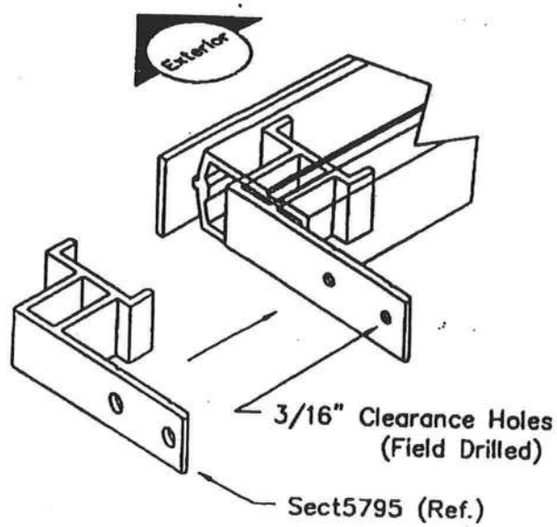
NOTE: LENGTHS FOR STANDARD WIDTH UNITS ARE 19⅛", 26½", 37", AND 53⅛".

Step 1. Position horizontal mull on top of lower unit as shown below. With 1/8" drill, drill up through pre-punched holes in the single hung heads into the mull. Before attaching with #8 x 3/4" screws (not included), run a full length bead of caulk in area shown.

Step 2. Position top unit on top of mull and drill 1/8" holes, in position shown, on same centers as lower unit. With 3/16" drill, re-drill holes in sill only and fasten with screws.

Step 3. Before lifting into rough opening, Drill two holes in each clip #SECT5795 and insert into each end of mull as shown below with tab pointing to inside. Fasten each clip tab to construction with two #10 x 1 1/2" screws for structural integrity.





M.I. HOME PRODUCTS

NOTE: SEE REVERSE SIDE FOR FASTENING REQUIREMENTS.

MULLV83B

DOCUMENT CONTROL ADDENDUM #01-40351.00

Current Issue Date: 02/15/02

Report No.: 01-40351.01

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 744 aluminum single hung window with flange.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Report No.: 01-40351.02

Requested by: William Emley, MI Home Products, Inc.
Purpose: Change of glass type.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories.

Report No.: 01-40351.03

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 740/744 aluminum single hung window with nail fin.
Issued Date: 02/15/02
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

ALLEN N. REEVES
CERTIFICATE
NO. 13354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
15 FEBRUARY 2002

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meting rail) (Loads were held for 52 seconds)		
	@ 45.0 psf (positive)	0.91"*	0.29" max.
	@ 45.0 psf (negative)	0.97"*	0.29" max.
4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads held for 10 seconds)		
	@ 67.5 psf (positive)	0.14"	0.20" max.
	@ 67.5 psf (negative)	0.19"	0.20" max.
4.4.2	@ 70.8 psf (negative)	0.20"	0.20" max.

** Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:baw
01-40351.03

Allen N. Reeves, P.E.
Director - Engineering Services
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
NO. 19354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into the #2 2 x 8 Spruce-Pine-Fir wood buck with 1" galvanized roofing nails through the nail fin every 8" on center. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	24 lbs	30 lbs max.
2.1.2	Air Infiltration (ASTM E 283) @ 1.57 psf (25 mph)	0.10 cfm/ft^2	0.30 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-96) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.86"*	0.29" max.
	@ 15.0 psf (negative)	0.81"*	0.29" max.

*Note: * Exceeds L/175 for deflection, but meets all other test requirements.*

2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.20" max.
	@ 22.5 psf (negative)	<0.01"	0.20" max.
2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction at 70 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.03"/6%	0.50"/100%
	Right stile	0.03"/6%	0.50"/100%

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES CERTIFICATE NO. 19354 STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.330" high by 0.187" backed polypile with center fin	1 Row	Fixed meeting rail interlock
0.170" high by 0.187" backed polypile with center fin	1 Row	Fixed lite, stiles and top rail
3/8" diameter hollow bulb gasket	1 Row	Bottom rail
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash stiles
0.150" high by 0.187" wide polypile	1 Row	Active sash stiles

Frame Construction: All frame members were constructed of extruded aluminum with coped, butted and sealed corners fastened with two screws each. Fixed meeting rail was secured utilizing one screw in each end directly through exterior face into jamb. Silicone was utilized around exterior meeting rail/jamb joinery.

Sash Construction: All sash members were constructed of extruded aluminum with coped and butted corners fastened with one screw each.

Screen Construction: The screen frame was constructed from roll-formed aluminum members with plastic keyed corners. The screening consisted of a fiberglass mesh and was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Plastic tilt latch	2	One each end of the interior Meeting rail
Metal sweep lock	2	13" from meeting rail ends
Balance assembly	2	One per jamb
Screen tension spring	2	One per end of screen stile
Tilt pin	2	One each end of bottom rail

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
NO. 19354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-40351.03
Test Dates: 10/22/01
And: 10/23/01
Report Date: 02/15/02
Expiration Date: 10/23/05

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness performance testing on a Series/Model 740/744, aluminum single hung window at MI Home Products, Inc.'s test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R45 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 740/744

Type: Aluminum Single Hung Window With Nail Fin

Overall Size: 4' 4-1/8" wide by 5' 11-5/8" high

Active Sash Size: 4' 2-3/4" wide by 2' 11-5/8" high

Fixed Daylight Opening Size: 4' 1-1/8" wide by 2' 9" high

Screen Size: 4' 1-7/8" wide by 2' 11-5/16" high

Finish: All aluminum was polished.

Glazing Details: The active sash and fixed lite were glazed with one sheet of 1/8" thick clear tempered glass. Each sash was channel glazed using a flexible vinyl gasket.

ALLEN M. REEVES
CERTIFICATE
NO. 19354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen M. Reeves

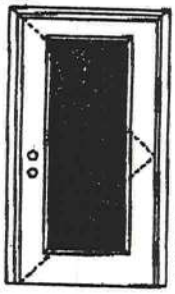
130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.testatl.com

X
Glazed Inswing Unit

COP-WL EN4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Test Data Review Certificate #3028447C and COP/Test Report Validation Matrix #3028447C-001 provides additional information - available from the ITS/WH website (www.atlaslabs.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+50.5/-50.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:


106 Series


133, 135 Series


135 Series


650 Series


822 Series

1/2 GLASS:


105 Series*


108, 160 Series*


120 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


197 Series*


106 Series


204 Series

*This glass kit may also be used in the following door styles: 6-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 6-panel with scroll.

Entergy
Entry Systems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Glazed Inswing Unit

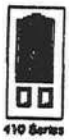
COP-WI FN4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


100 Series


114, 180, 122 Series


152 Series


148 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Test Data Review Certificate #3028447C and COP/Test Report Validation Matrix #3028447C-001 provides additional information - available from the ITS/WH website (www.wdsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

2

Entergy
Entry Systems

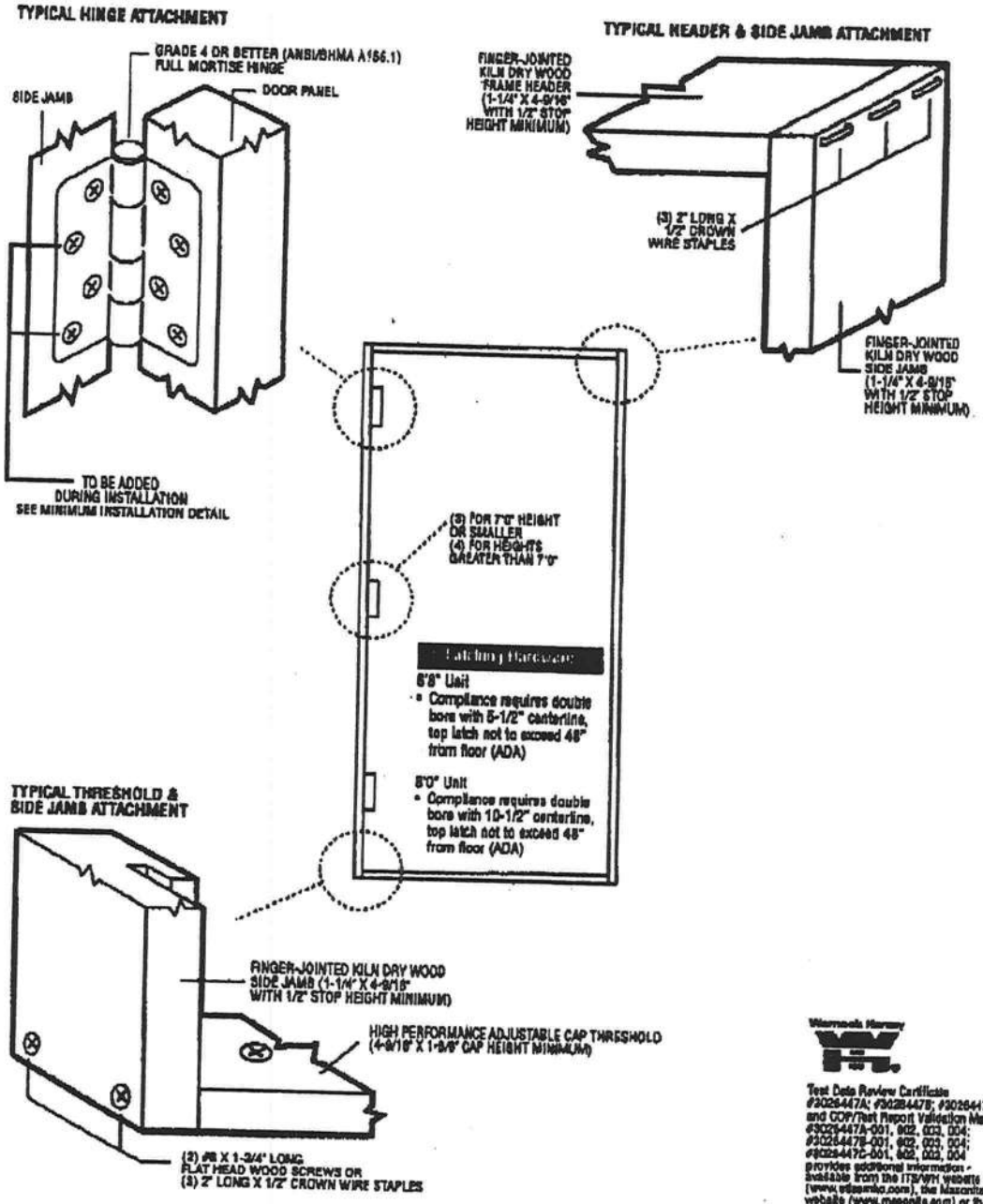
June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

Exclusively from Masonite®
Masonite International Corporation

X
Unit

MAD-WI-MA0001-02

INSWING UNIT WITH SINGLE DOOR

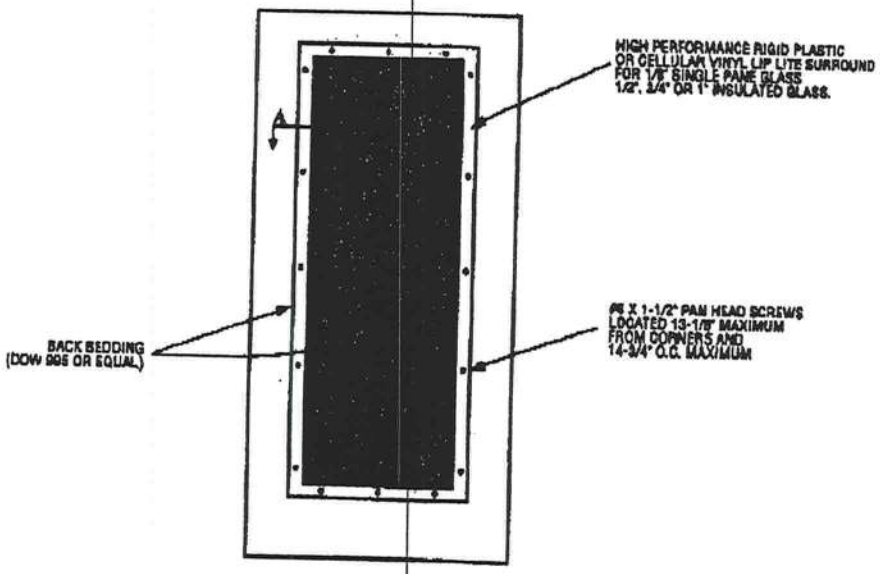


Test Data Review Certificate #30284478; #30284479; #30284470 and CCP/Test Report Validation Matrix #30284478-001, 002, 003, 004; #30284479-001, 002, 003, 004; #30284470-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etsamko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

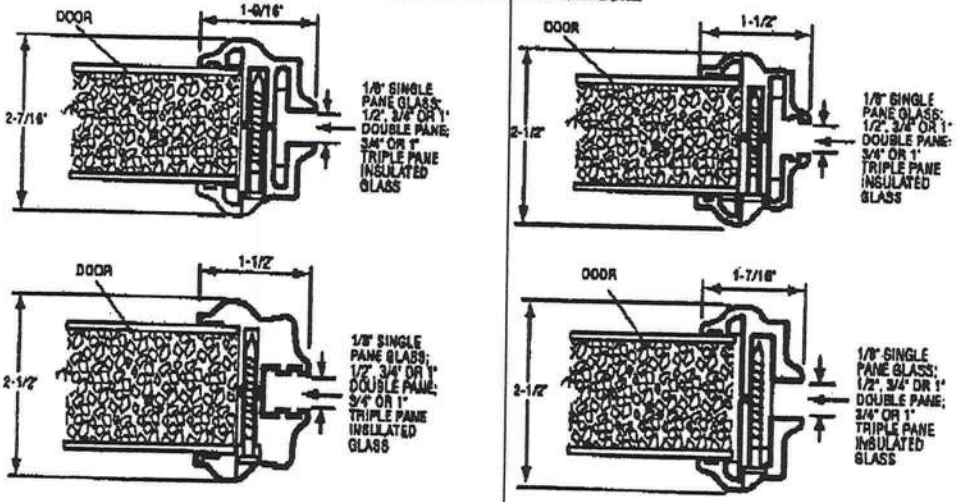
Masonite

MAD-WL-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

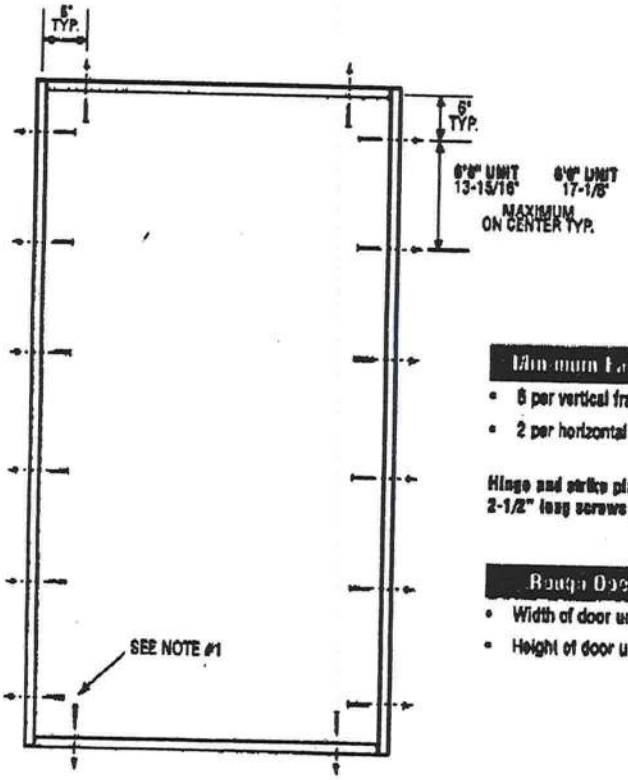
Test Data Review Certificate #3026447A; #3026447B; #3026447C and CDF/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 2-1/2" lag screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Intertek Testing Services NA Inc. Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etsamko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0246*, 0256*, 3241*, 3248, 3251* or 3256**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

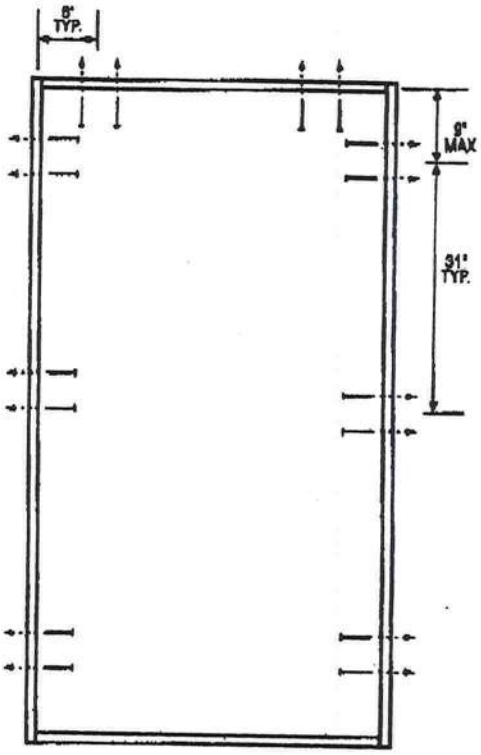
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0" height and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 4 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey Test Data Review Certificate #30284474; #30284475; #30284476 and COP/Test Report Validation Matrix #30284474-001, 002, 003, 004; #30284475-001, 002, 003, 004; #30284476-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etsconline.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0246*, 8286*, 3241*, 3248, 3261* or 3266**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

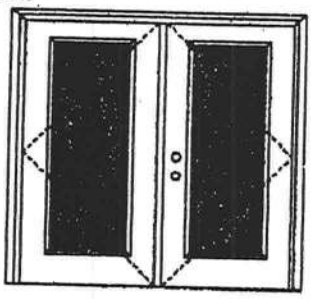
Masonite®

XX
Glazed Outswing Unit

COP-WI-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Underwriters Laboratories

This Data Review Certificate #20035447C and COP/Test Report Validation Matrix #20035447C-001 provides additional information - available from the ITS/WHI website (www.wherni.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size - 6'0" x 6'8"

Design Pressure
+50.5/-50.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:
1/4 GLASS:


100 Series


133, 135 Series


136 Series


880 Series


822 Series

1/2 GLASS:


105 Series*


106, 160 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


167 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 6-panel; Eyebrow 5-panel with scroll.

Entergy
Entry Systems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

FIREDOOR Corporation Premium Quality Doors

Exclusively from Masonite
Masonite International Corporation

XX
Glazed Outswing Unit

COP-WI-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:
3/4 GLASS:


404 Series


410 Series


430 Series

FULL GLASS:


100 Series


114, 129, 132 Series


152 Series


140 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Weyerhaeuser

Test Data Review Certificate #3028447C and COP/Test Report Validation Matrix #3028447C-001 provide additional information - available from the ITS/WH website (www.atlasmtn.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Entergy
Entry Systems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

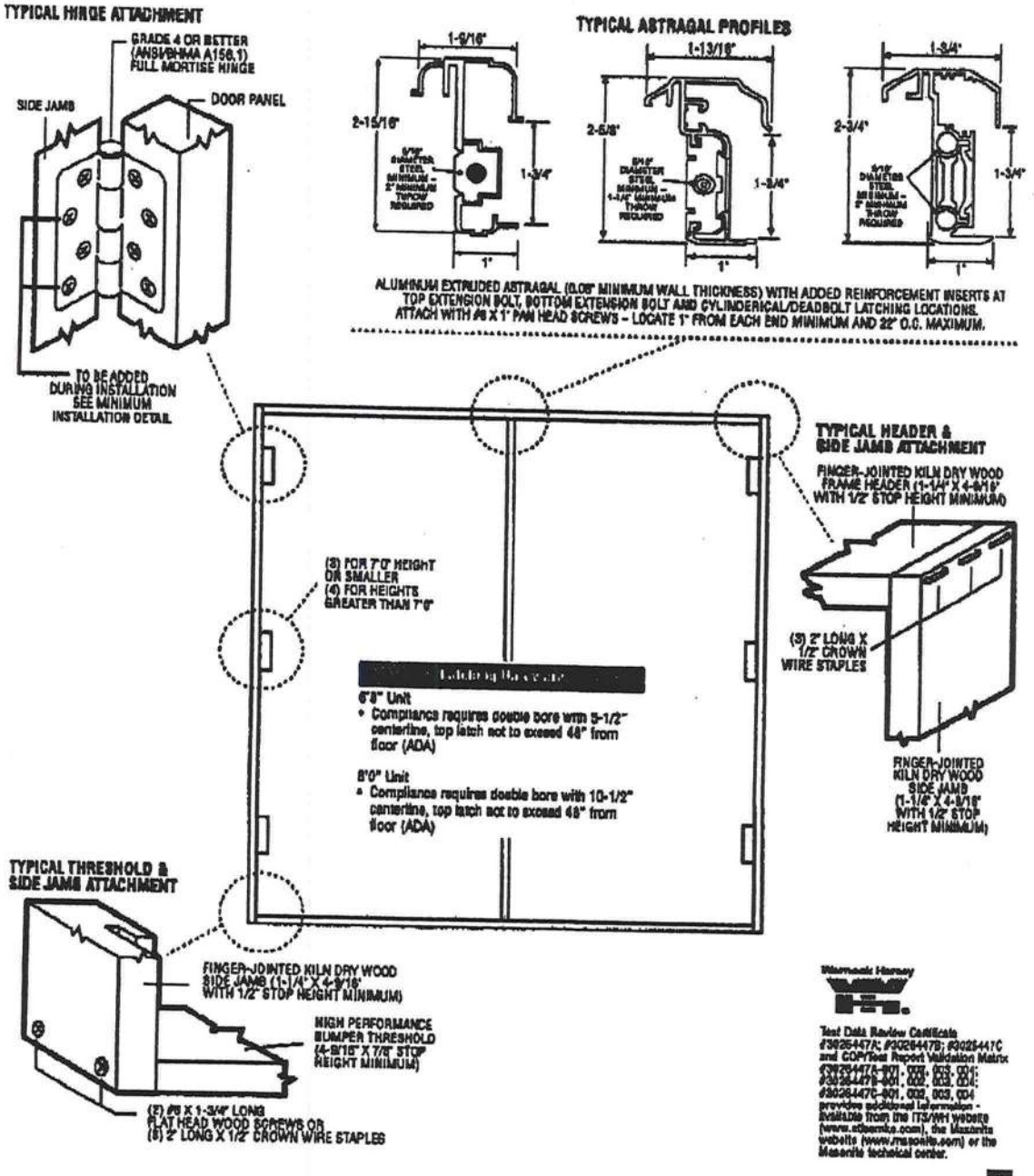
PREMDOR Collection Premium Quality Doors

Exclusively from Masonite
Masonite International Corporation

XX
Unit

MAD-WL-MA0012-02

OUTSWING UNITS WITH DOUBLE DOOR

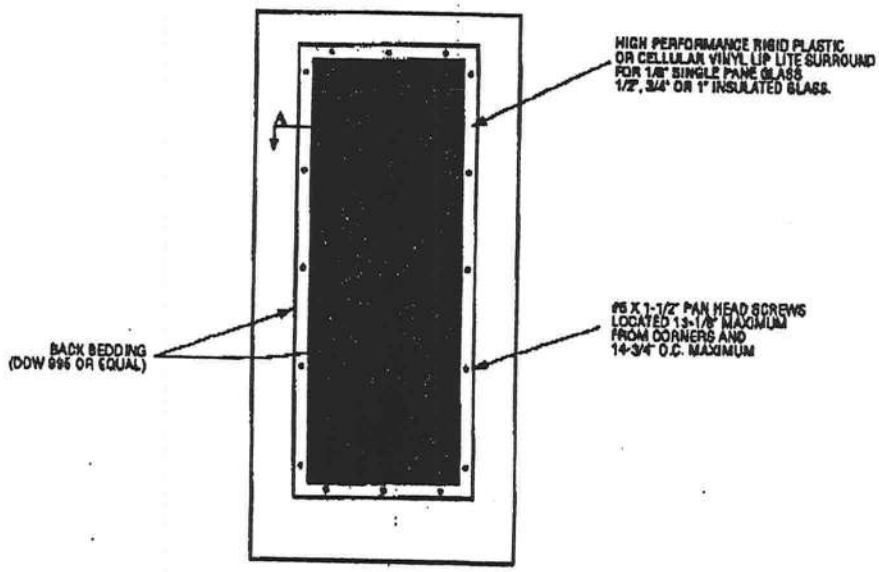


Test Data Review Certificate #30264478; #30264478; #30264478 and CORTest Report Validation Matrix #30264478-001; 002; 003; 004; #30264478-001; 002; 003; 004; #30264478-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.ccbcmks.com), the Masonite website (www.masonite.com) or the Masonite technical center.

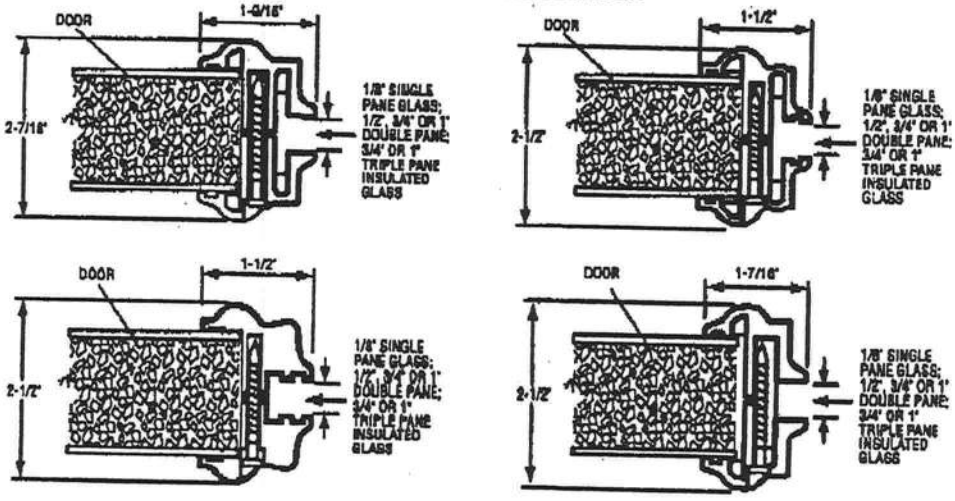
Masonite

MAD-WI-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



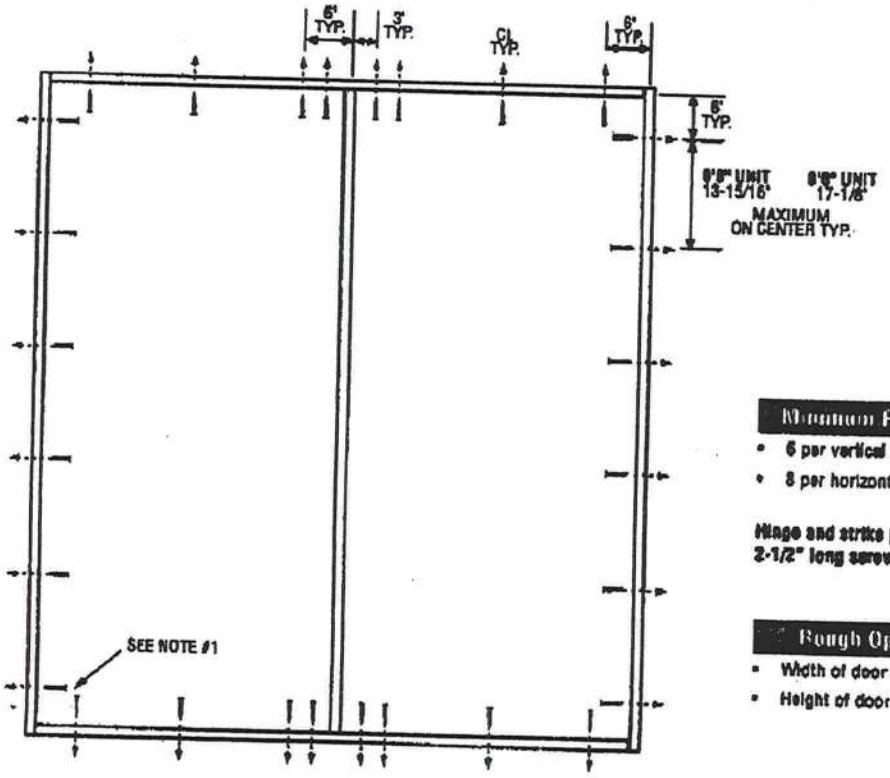
*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

Test Data Review Certificate #30284474; #30284478; #30284476 and COP/Test Report Validation Matrix #30284474-001, 002, 003; #30284478-001, 002, 003; #30284476-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4" .

Warnock Hersey Test Data Review Certificate #3025447A; #3025447B; #3025447C and COP/Test Report Validation Matrix #3025447A-001, 002, 003, 004; #3025447B-001, 002, 003, 004; #3025447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 8247*, 8287*, 3242*, 3247, 3282* or 3267**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

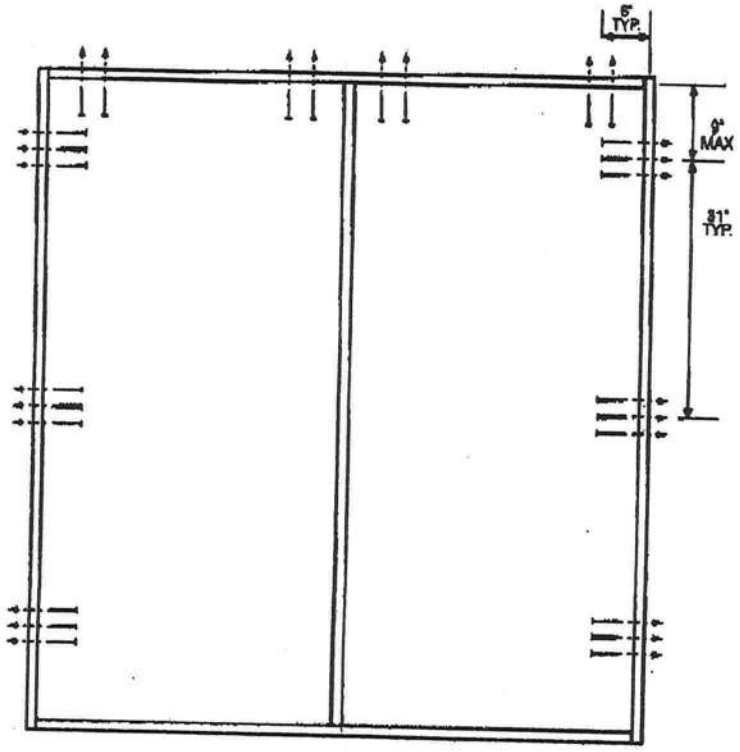
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite

XX
Unit

MID-WL-MA0002 02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0" heights and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

That Data Review Certificate #30256447A; #30256447B; #30256447C and COP/Test Report Validation Matrix #30256447A-001, 002, 003, 004; #30256447B-001, 002, 003, 004; #30256447C-001, 002, 003, 004 provides additional information - available from the ITS/AAH website (www.etsamko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0247*, 0267*, 3242*, 3247, 3262* or 3267**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 wood screws and 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw and common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite

Project Information for:	L135121		
Builder:	HUGO ESCALANTE	Date:	10/19/2005
Lot:	N/A	Start Number:	195
Subdivision:	LOT 15 WISE ESTATES		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	51		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	120		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ESCALANTE, HUGO CRC 1326967
Address: P.O. BOX 280
FORT WHITE, FL. 32038 Designer: 26

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:
1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	101905195	10/19/2005	41	T15	101905235	10/19/2005
2	CJ1T	101905196	10/19/2005	42	T16	101905236	10/19/2005
3	CJ2	101905197	10/19/2005	43	T17	101905237	10/19/2005
4	CJ2A	101905198	10/19/2005	44	T18	101905238	10/19/2005
5	CJ3	101905199	10/19/2005	45	T18G	101905239	10/19/2005
6	CJ3A	101905200	10/19/2005	46	V02	101905240	10/19/2005
7	CJ3B	101905201	10/19/2005	47	V05	101905241	10/19/2005
8	CJ3T	101905202	10/19/2005	48	V08	101905242	10/19/2005
9	CJ4	101905203	10/19/2005	49	V11	101905243	10/19/2005
10	CJ5	101905204	10/19/2005	50	V14	101905244	10/19/2005
11	CJ5A	101905205	10/19/2005	51	V17	101905245	10/19/2005
12	CJ5T	101905206	10/19/2005				
13	CJ6	101905207	10/19/2005				
14	CJ7	101905208	10/19/2005				
15	CJ9	101905209	10/19/2005				
16	EJ6	101905210	10/19/2005				
17	EJ7	101905211	10/19/2005				
18	EJ7T	101905212	10/19/2005				
19	EJ7TA	101905213	10/19/2005				
20	F01	101905214	10/19/2005				
21	F02	101905215	10/19/2005				
22	F03	101905216	10/19/2005				
23	F04	101905217	10/19/2005				
24	HJ7	101905218	10/19/2005				
25	HJ9	101905219	10/19/2005				
26	HJ15	101905220	10/19/2005				
27	T01	101905221	10/19/2005				
28	T02	101905222	10/19/2005				
29	T03	101905223	10/19/2005				
30	T04	101905224	10/19/2005				
31	T05	101905225	10/19/2005				
32	T06	101905226	10/19/2005				
33	T07	101905227	10/19/2005				
34	T08	101905228	10/19/2005				
35	T09	101905229	10/19/2005				
36	T10	101905230	10/19/2005				
37	T11	101905231	10/19/2005				
38	T12	101905232	10/19/2005				
39	T13	101905233	10/19/2005				
40	T14	101905234	10/19/2005				

Dwg.#101905194

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02:00:39 PM 10/6/2004

Licensee Details

Licensee Information

Name:	**ESCALANTE, HUGO** (Primary Name)
	EWPL INC (DBA Name)
Main Address:	**P.O. BOX 280** **FORT WHITE, Florida 32038**

License Information

License Type:	**Certified Residential Contractor**
Rank:	**Cert Residental**
License Number:	**CRC1326967**
Status:	**Current, Active**
Licensure Date:	**11/24/2003**
Expires:	**08/31/2006**

Special Qualifications	Effective Date
Qualified Business License Required	11/24/2003

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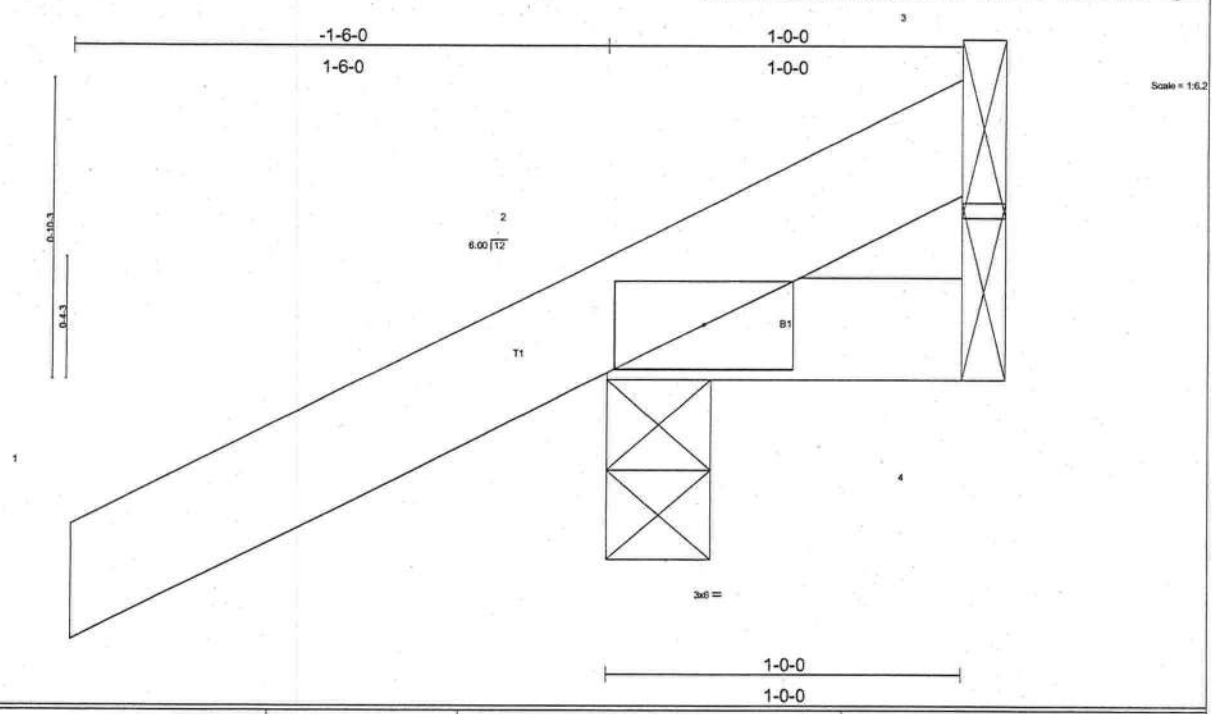
OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	CJ1	MONO TRUSS	6	1	Job Reference (optional)

Dwg.#101905195

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:46:05 2005 Page 1



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TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=189/0-3-8, 4=14/Mechanical, 3=-40/Mechanical
Max Horz 2=84(load case 5)
Max Uplift2=-220(load case 5), 3=-40(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=73(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/41
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 220 lb uplift at joint 2 and 40 lb uplift at joint 3.

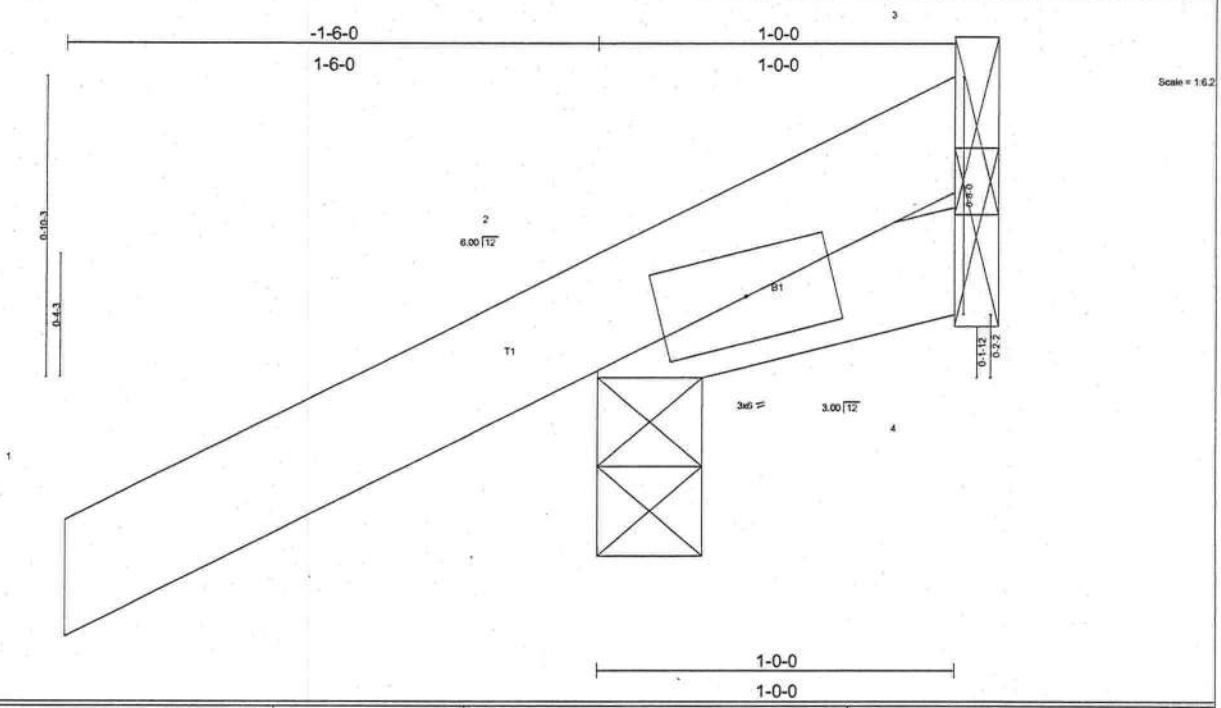
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	CJ1T	MONO SCISSOR	2	1	Job Reference (optional)

Dwg.#101905196

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:46:09 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.18	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=189/0-3-8, 4=14/Mechanical, 3=-40/Mechanical
Max Horz 2=83(load case 5)
Max Uplift2=-214(load case 5), 3=-40(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=68(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/32, 2-3=-48/41
BOT CHORD 2-4=-3/3

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 214 lb uplift at joint 2 and 40 lb uplift at joint 3.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	CJ2	MONO TRUSS	4	1	Job Reference (optional)

Dwg.#101905197

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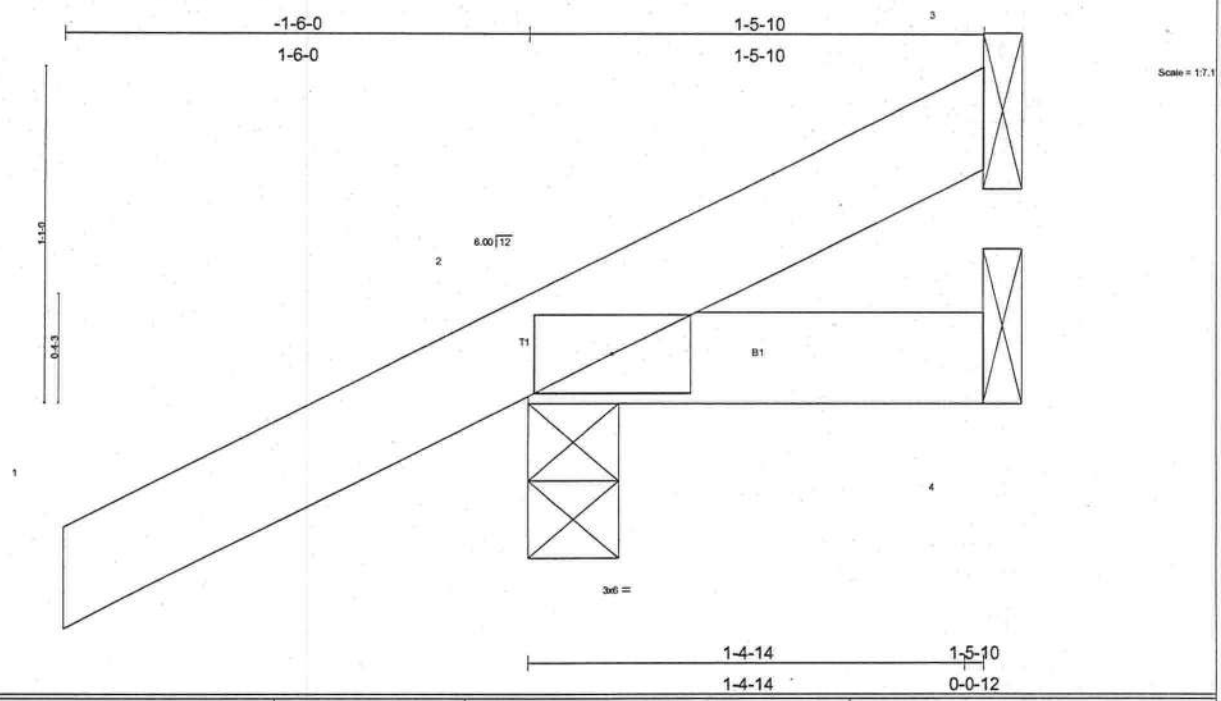

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TCLL	20.0	Plates Increase	1.25	TC	0.18	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-5-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=187/0-3-8, 4=21/Mechanical, 3=-6/Mechanical
Max Horz 2=97(load case 5)
Max Uplift2=-213(load case 5), 4=-17(load case 3), 3=-7(load case 8)
Max Grav 2=187(load case 1), 4=21(load case 1), 3=25(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-39/11
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 213 lb uplift at joint 2, 17 lb uplift at joint 4 and 7 lb uplift at joint 3.

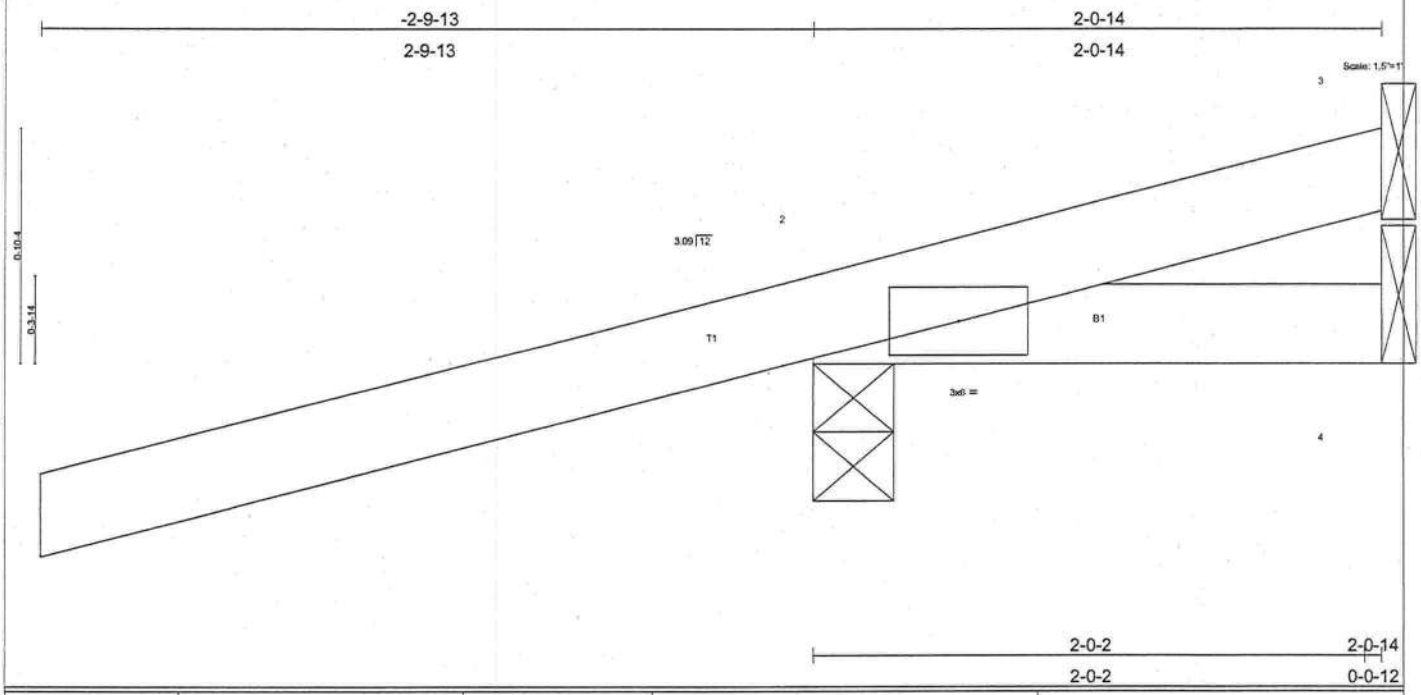
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	CJ2A	MONO TRUSS	2	1	Job Reference (optional)

Dwg.#101905198

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:46:16 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.00	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 11 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=-77/Mechanical, 2=365/0-3-8, 4=28/Mechanical
Max Horz 2=84(load case 3)
Max Uplift3=-77(load case 1), 2=-435(load case 3), 4=-22(load case 3)
Max Grav 3=123(load case 3), 2=365(load case 1), 4=28(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-50/40
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 77 lb uplift at joint 3, 435 lb uplift at joint 2 and 22 lb uplift at joint 4.

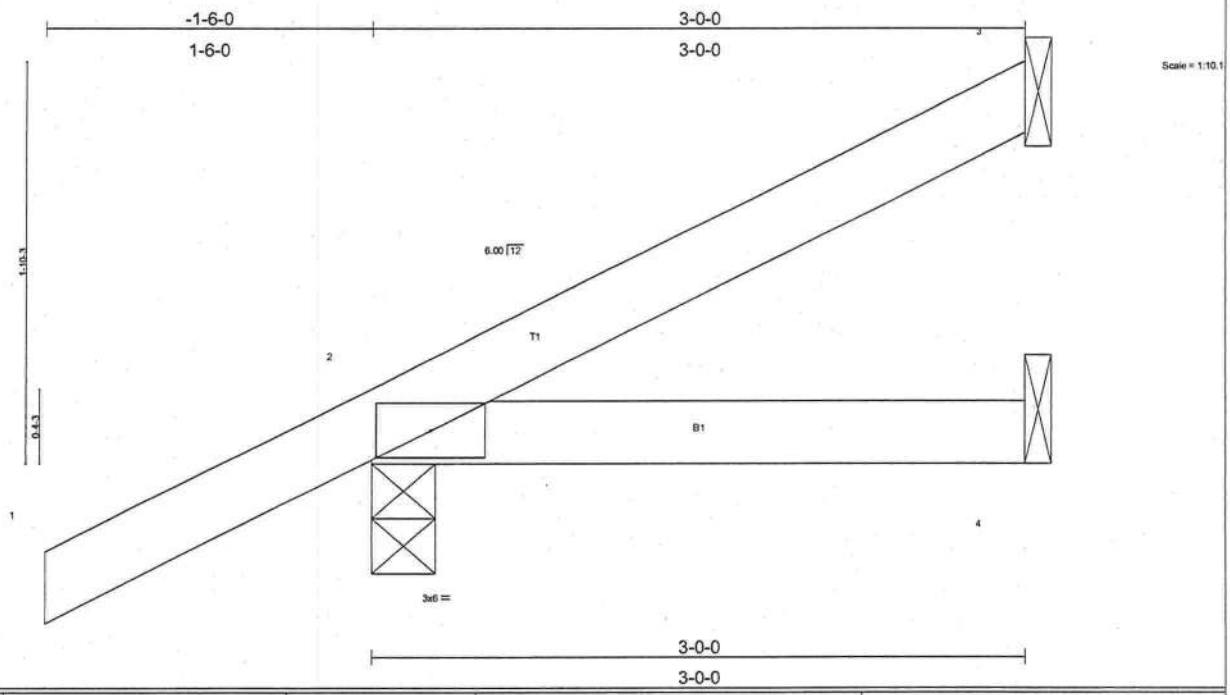
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	CJ3	MONO TRUSS	6	1	Job Reference (optional)

Dwg.#101905199

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:46:19 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical
Max Horz 2=137(load case 5)
Max Uplift3=-47(load case 5), 2=-187(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-52/16
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 3 and 187 lb uplift at joint 2.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	CJ3A	MONO TRUSS	2	1	Job Reference (optional)

Dwg.#101905200

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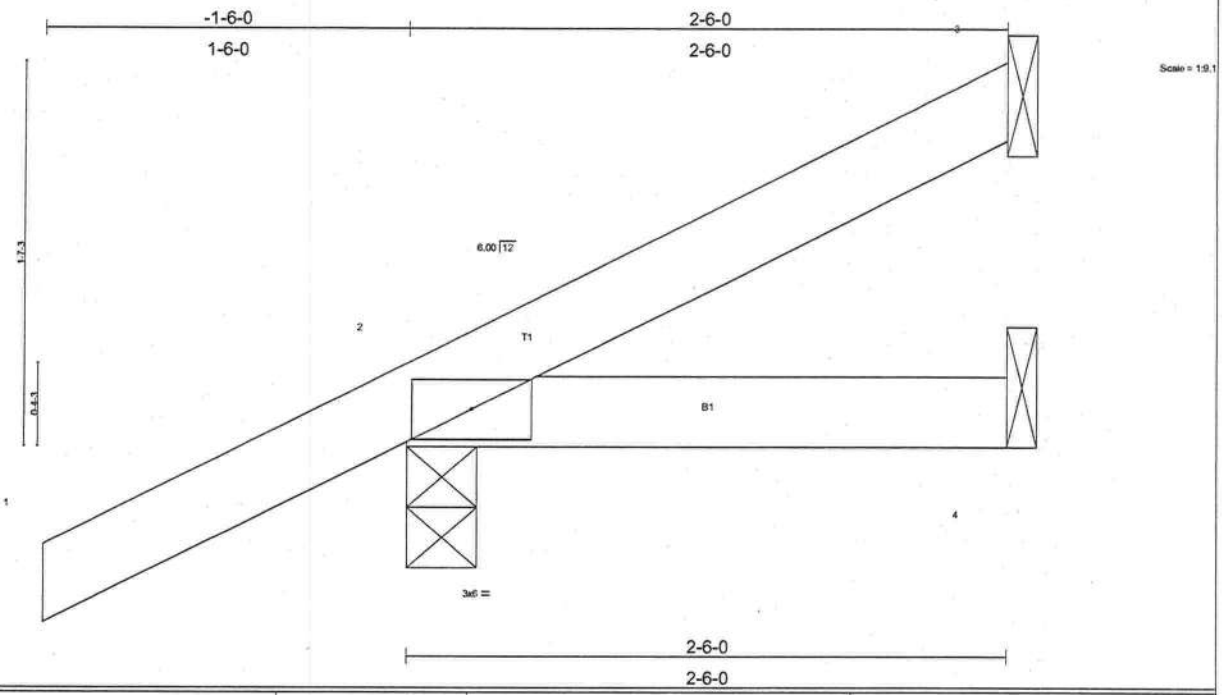

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.00	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 11 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=30/Mechanical, 2=217/0-3-8, 4=34/Mechanical
Max Horz 2=124(load case 5)
Max Uplift3=-31(load case 6), 2=-223(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/8
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 3, 223 lb uplift at joint 2 and 27 lb uplift at joint 4.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	CJ3B	MONO TRUSS	1	1	Job Reference (optional)

Dwg.#101905201

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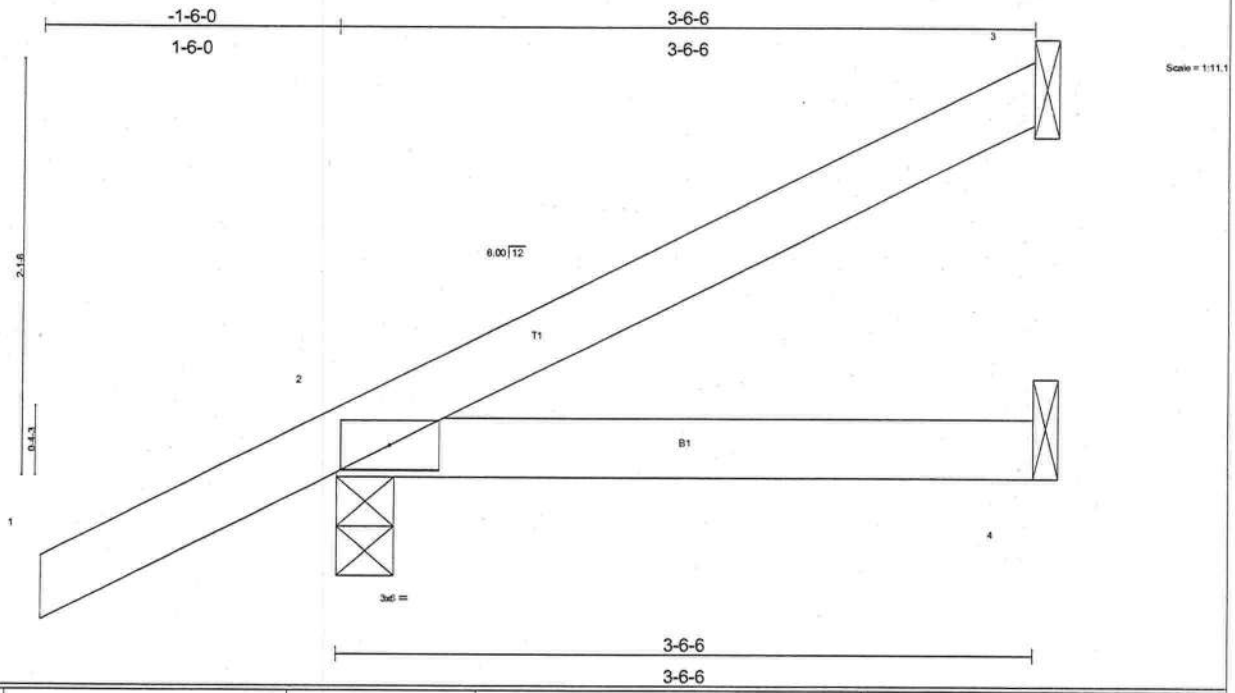

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 14 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=68/Mechanical, 2=250/0-3-8, 4=50/Mechanical
Max Horz 2=152(load case 5)
Max Uplift3=-69(load case 5), 2=-188(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-69/23
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 3 and 188 lb uplift at joint 2.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	CJ3T	MONO SCISSOR	2	1	Job Reference (optional)

Dwg.#101905202

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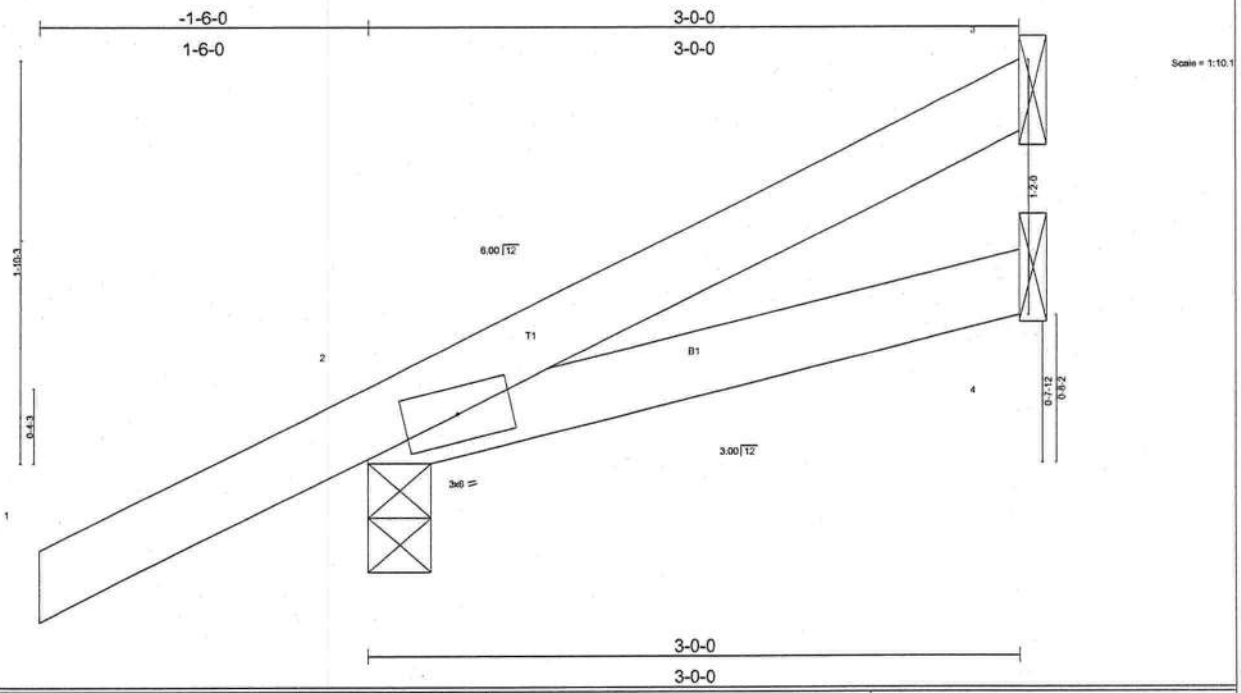


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical
Max Horz 2=137(load case 5)
Max Uplift3=-49(load case 5), 2=-185(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-54/16
BOT CHORD 2-4=-8/8

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 3 and 185 lb uplift at joint 2.

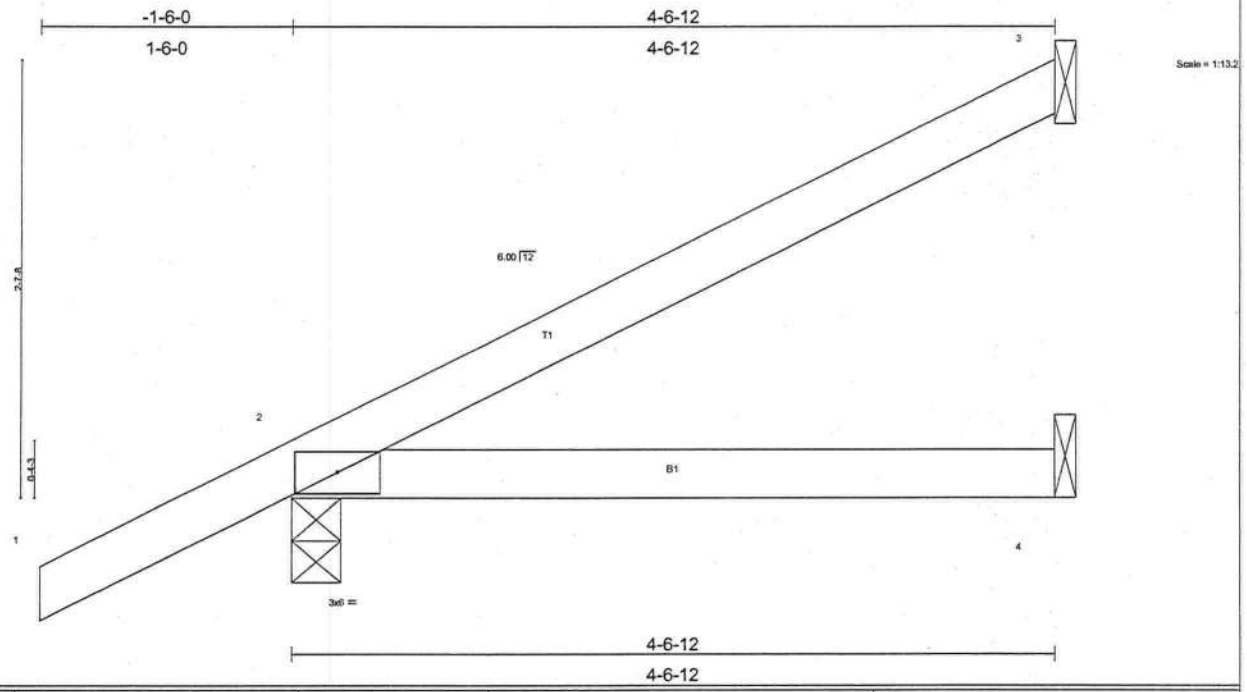
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	CJ4	MONO TRUSS	1	1	Job Reference (optional)

Dwg.#101905203

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:46:33 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.02	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.03	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 17 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=101/Mechanical, 2=288/0-3-8, 4=65/Mechanical
Max Horz 2=180(load case 5)
Max Uplift3=-108(load case 5), 2=-194(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-100/36
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 3 and 194 lb uplift at joint 2.

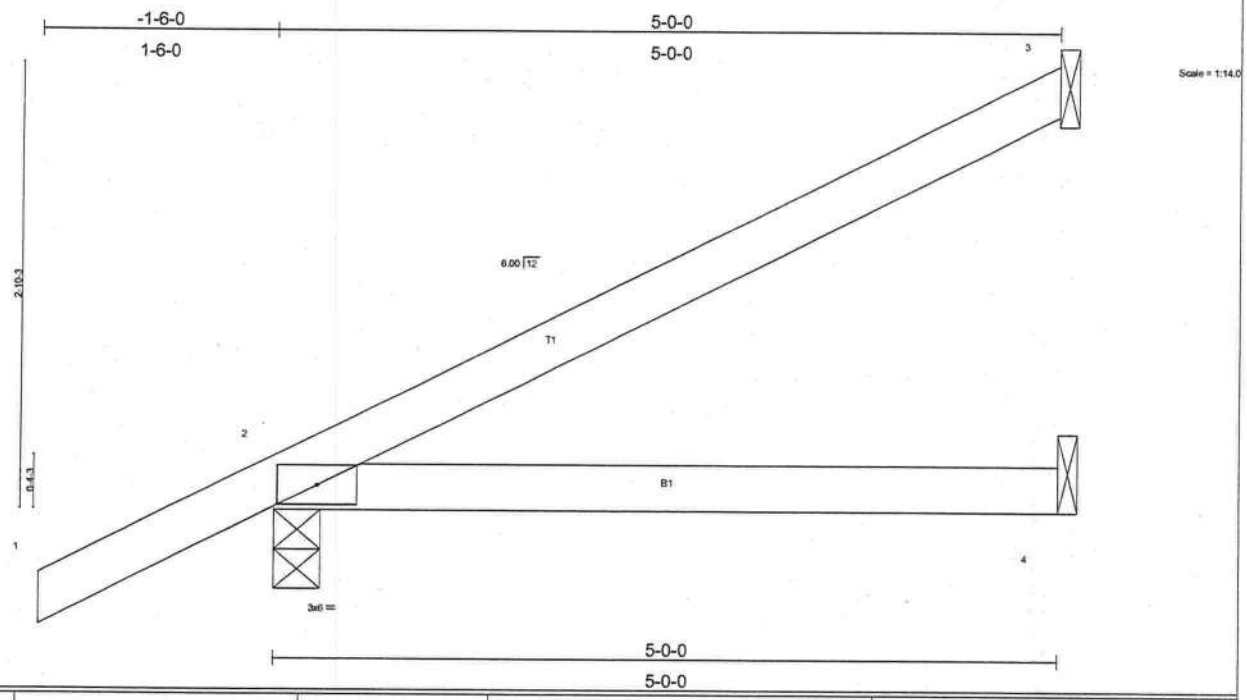
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	CJ5	MONO TRUSS	6	1	Job Reference (optional)

Dwg.#101905204

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:46:37 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=192(load case 5)
Max Uplift3=-124(load case 5), 2=-197(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-112/41
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 3 and 197 lb uplift at joint 2.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	CJ5A	MONO TRUSS	1	1	Job Reference (optional)

Dwg.#101905205

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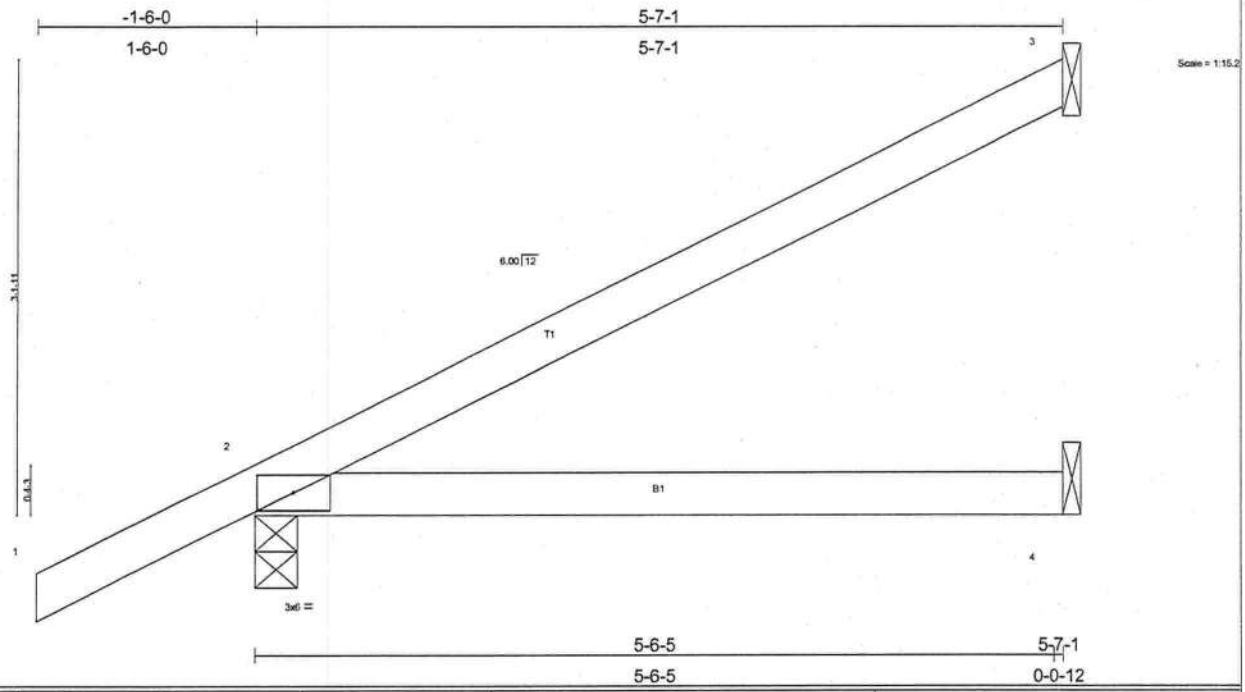

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.04	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.08	2-4	>838	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 20 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=132/Mechanical, 2=329/0-3-8, 4=81/Mechanical
Max Horz 2=208(load case 5)
Max Uplift3=-144(load case 5), 2=-202(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-127/47
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 3 and 202 lb uplift at joint 2.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	CJ5T	SPECIAL	2	1	Job Reference (optional)

Dwg.#101905206

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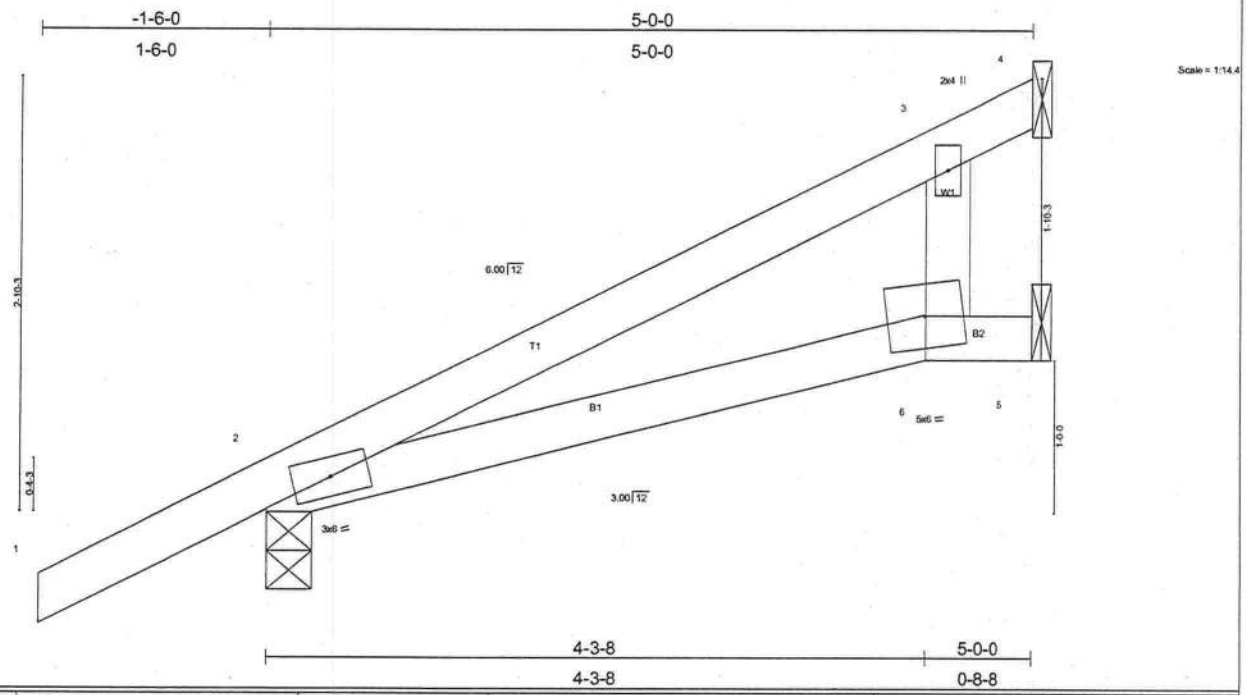


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.04	6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.04	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(TL)	-0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 20 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=176/Mechanical, 2=305/0-3-8, 5=10/Mechanical
Max Horz 2=192(load case 5)
Max Uplift4=-113(load case 5), 2=-196(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-116/17, 3-4=-87/78
BOT CHORD 2-6=-3/21, 5-6=-0/0
WEBS 3-6=0/75

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 113 lb uplift at joint 4 and 196 lb uplift at joint 2.

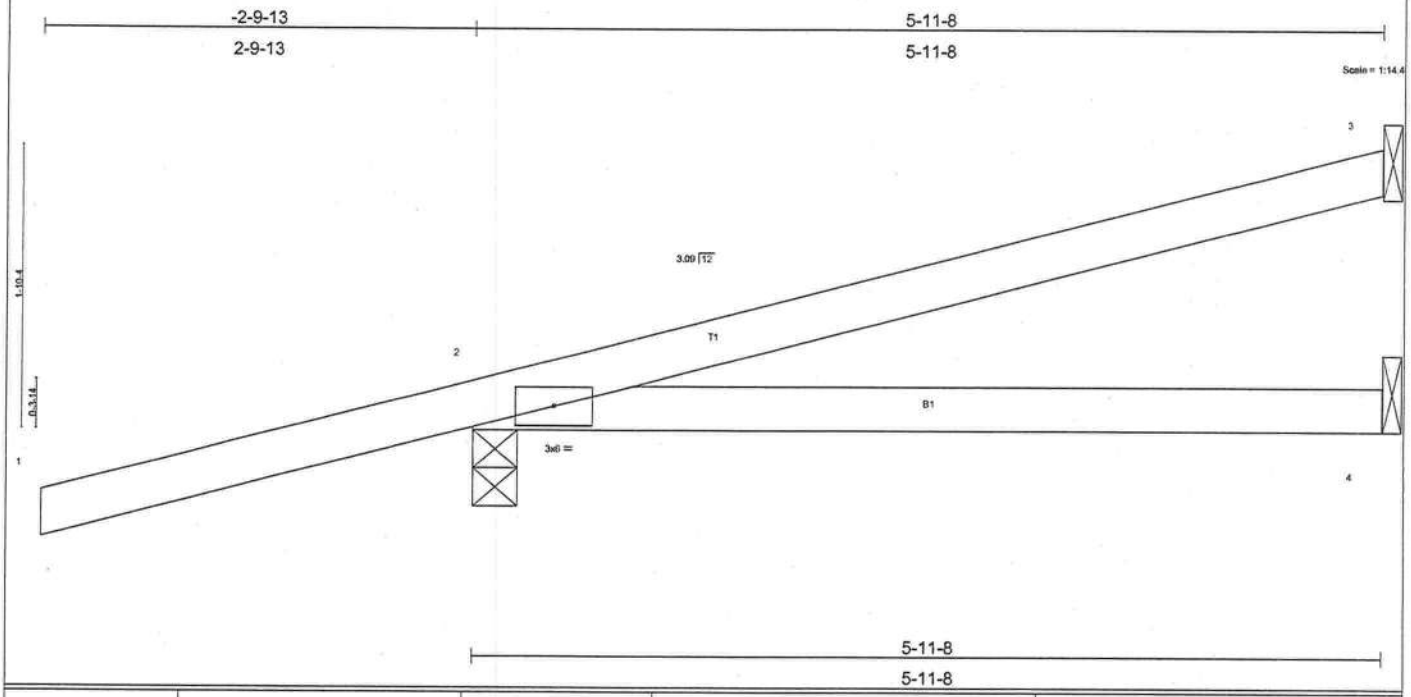
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	CJ6	MONO TRUSS	2	1	Job Reference (optional)

Dwg.#101905207

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:46:48 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	0.21	2-4	>324	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	0.19	2-4	>369	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=443/0-3-8, 4=86/Mechanical
Max Horz 2=138(load case 3)
Max Uplift3=-90(load case 3), 2=-449(load case 3), 4=-69(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-52/22
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 90 lb uplift at joint 3, 449 lb uplift at joint 2 and 69 lb uplift at joint 4.

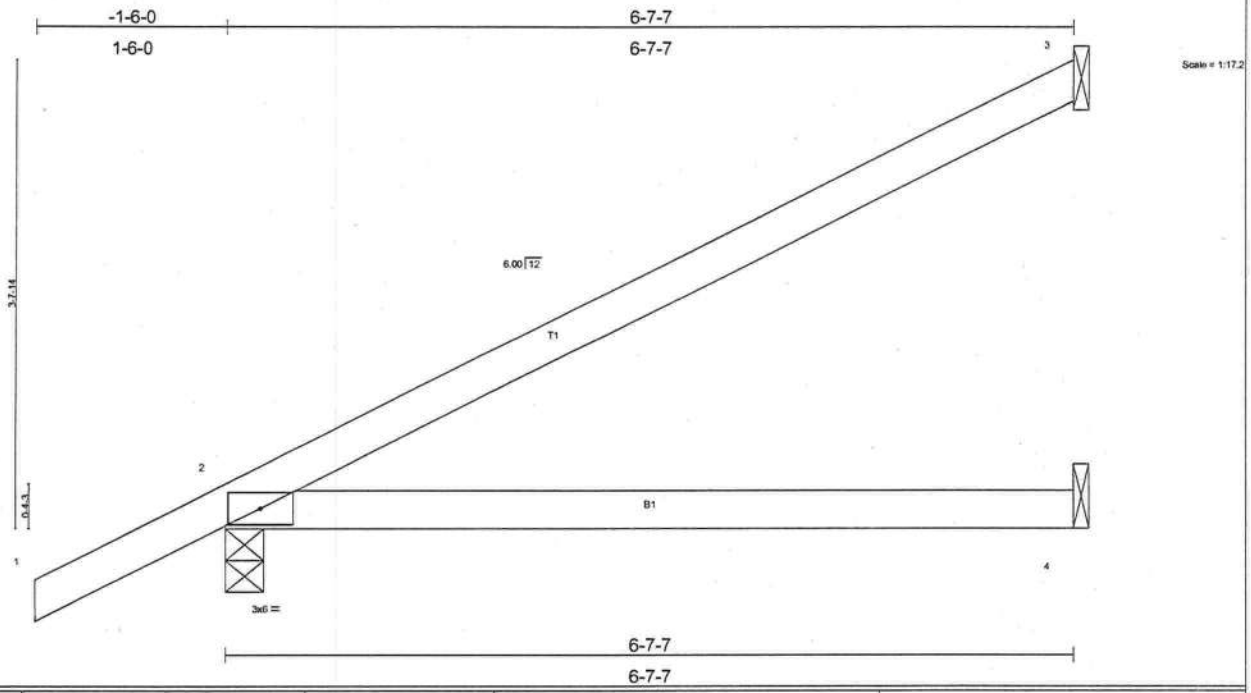
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	CJ7	MONO TRUSS	1	1	Job Reference (optional)

Dwg.#101905208

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:46:52 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.09	2-4	>867	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.16	2-4	>496	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 24 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=370/0-3-8, 4=96/Mechanical
Max Horz 2=237(load case 5)
Max Uplift3=-179(load case 5), 2=-213(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-149/58
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 179 lb uplift at joint 3 and 213 lb uplift at joint 2.

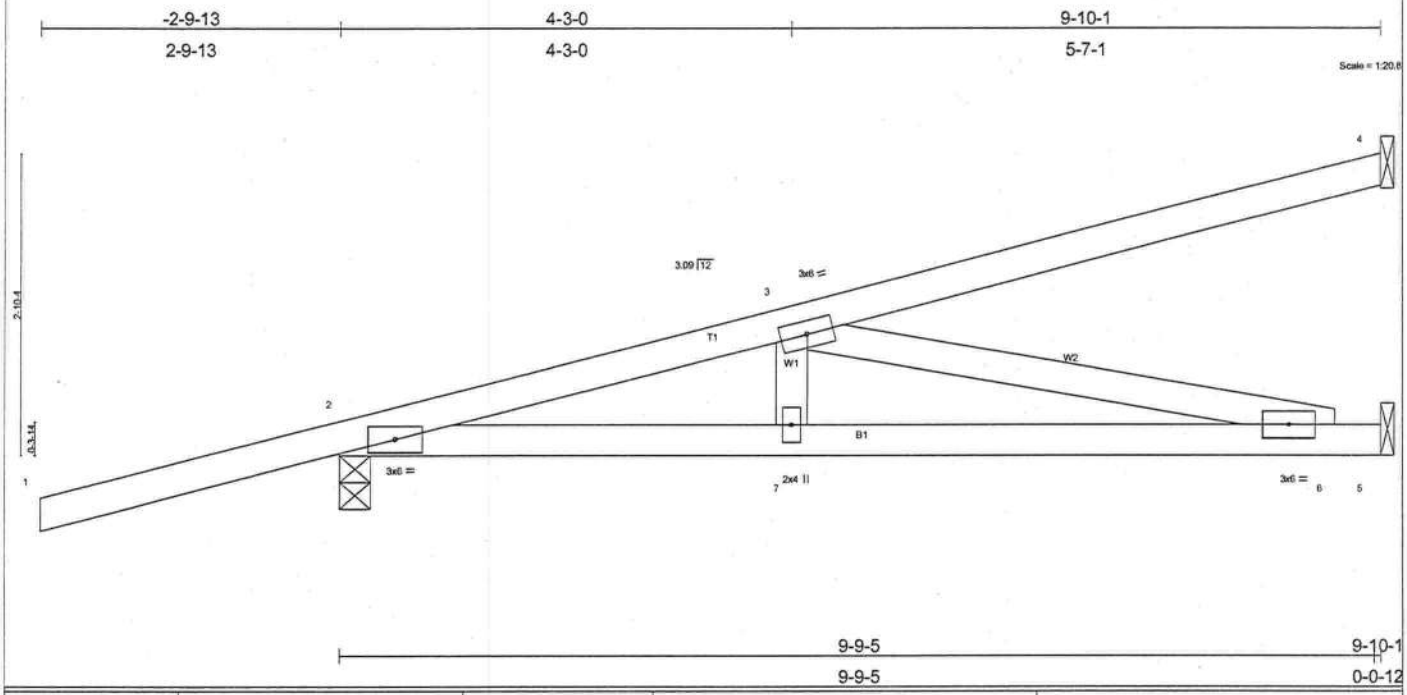
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	CJ9	MONO TRUSS	1	1	Job Reference (optional)

Dwg.#101905209

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:46:56 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.08	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.13	6-7	>886	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.46	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-3-8 oc bracing.

REACTIONS (lb/size) 4=131/Mechanical, 2=589/0-3-8, 5=249/Mechanical
Max Horz 2=193(load case 3)
Max Uplift4=-120(load case 3), 2=-412(load case 3), 5=-61(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-926/369, 3-4=-52/25
BOT CHORD 2-7=-488/863, 6-7=-488/863, 5-6=0/0
WEBS 3-6=-883/500, 3-7=0/152

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 120 lb uplift at joint 4, 412 lb uplift at joint 2 and 61 lb uplift at joint 5.

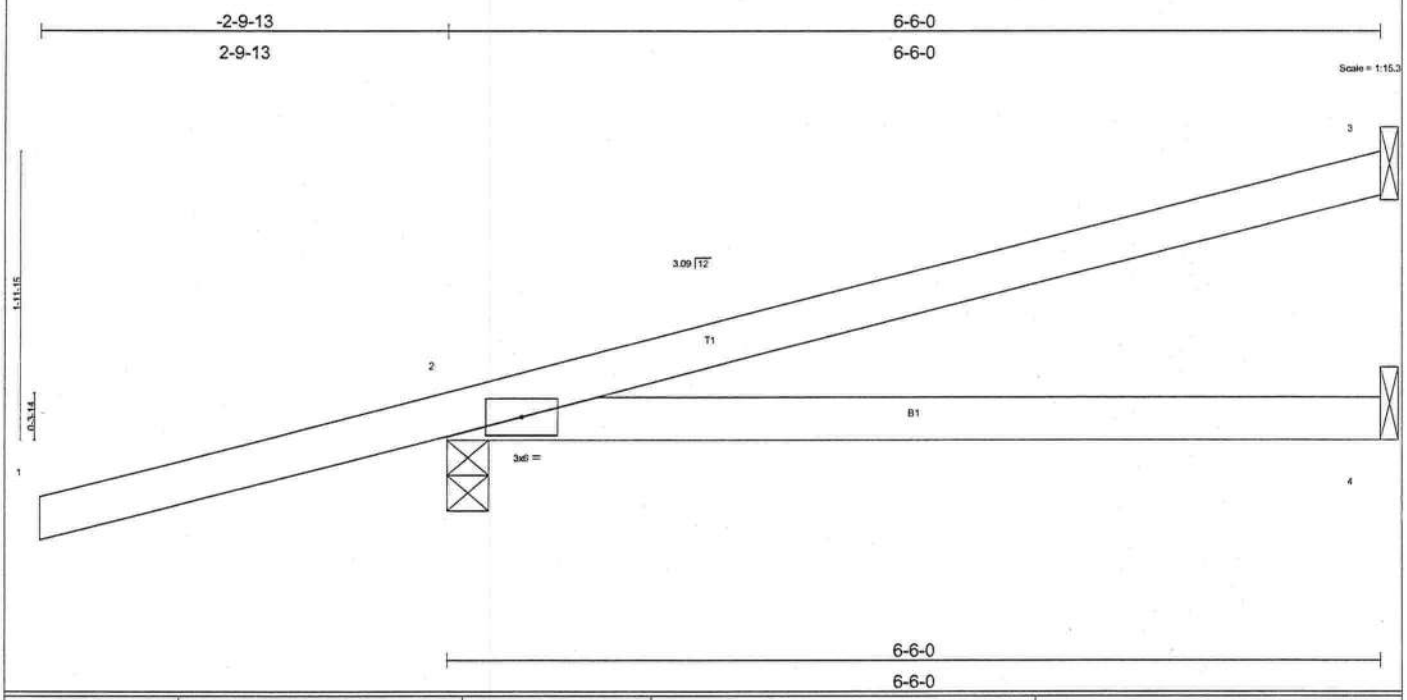
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	EJ6	MONO TRUSS	10	1	Job Reference (optional)

Dwg.#101905210

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:46:59 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	0.30	2-4	>248	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	0.27	2-4	>283	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 24 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=132/Mechanical, 2=462/0-3-8, 4=94/Mechanical
Max Horz 2=146(load case 3)
Max Uplift3=-108(load case 3), 2=-462(load case 3), 4=-75(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-59/25
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 3, 462 lb uplift at joint 2 and 75 lb uplift at joint 4.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	EJ7	MONO TRUSS	13	1	Job Reference (optional)

Dwg.#101905211

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:47:03 2005 Page 1

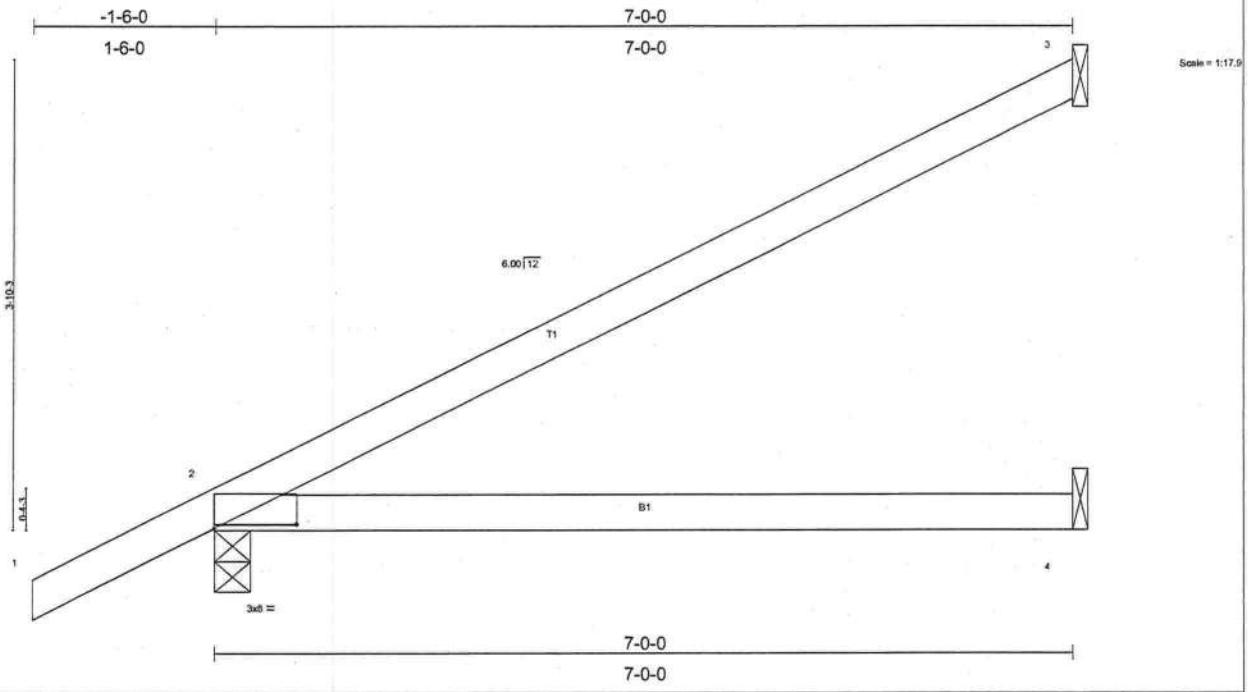


Plate Offsets (X,Y): [2:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.13	2-4	>606	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.22	2-4	>365	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=166/Mechanical, 2=385/0-3-8, 4=108/Mechanical
Max Horz 2=247(load case 5)
Max Uplift3=-170(load case 5), 2=-217(load case 5), 4=-1(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-142/59
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 3, 217 lb uplift at joint 2 and 1 lb uplift at joint 4.

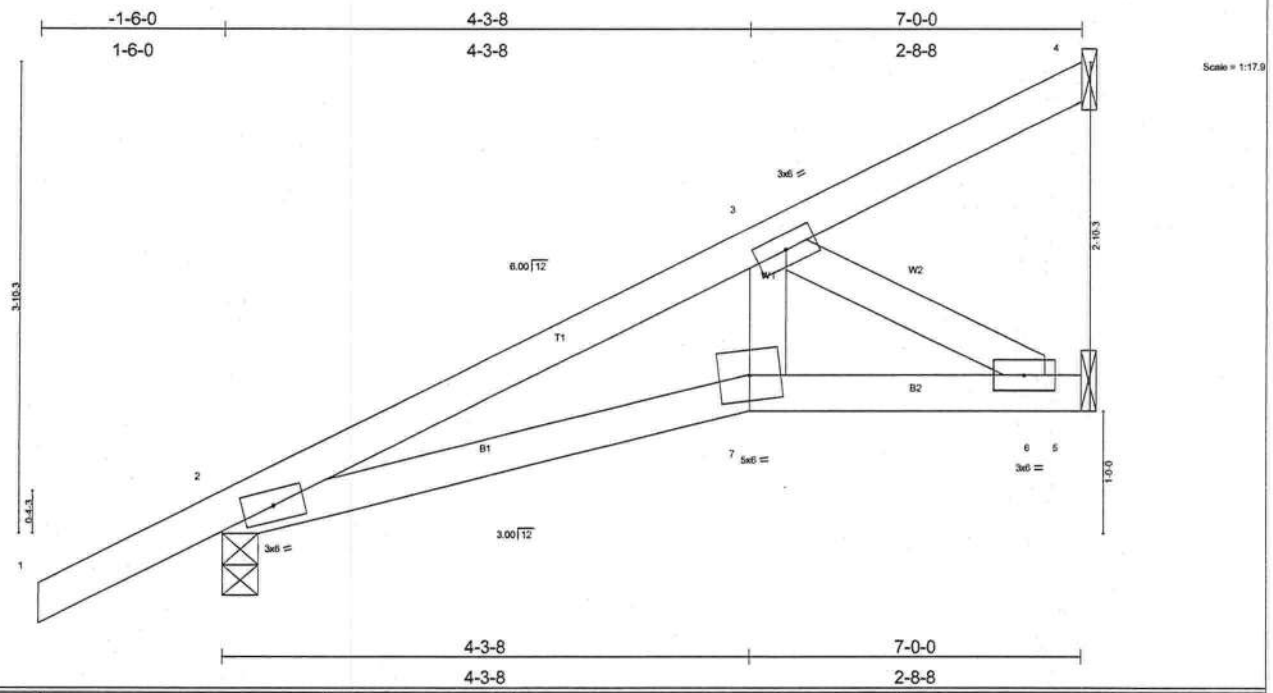
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	EJ7T	SPECIAL	6	1	Job Reference (optional)

Dwg.#101905212

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:47:07 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.02	2-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.03	2-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 30 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-13 oc bracing.

REACTIONS (lb/size) 4=45/Mechanical, 2=385/0-3-8, 5=230/Mechanical
Max Horz 2=246(load case 5)
Max Uplift4=-50(load case 5), 2=-216(load case 5), 5=-122(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-544/258, 3-4=-42/14
BOT CHORD 2-7=-416/461, 6-7=-390/403, 5-6=0/0
WEBS 3-7=-95/250, 3-6=-461/446

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 4, 216 lb uplift at joint 2 and 122 lb uplift at joint 5.

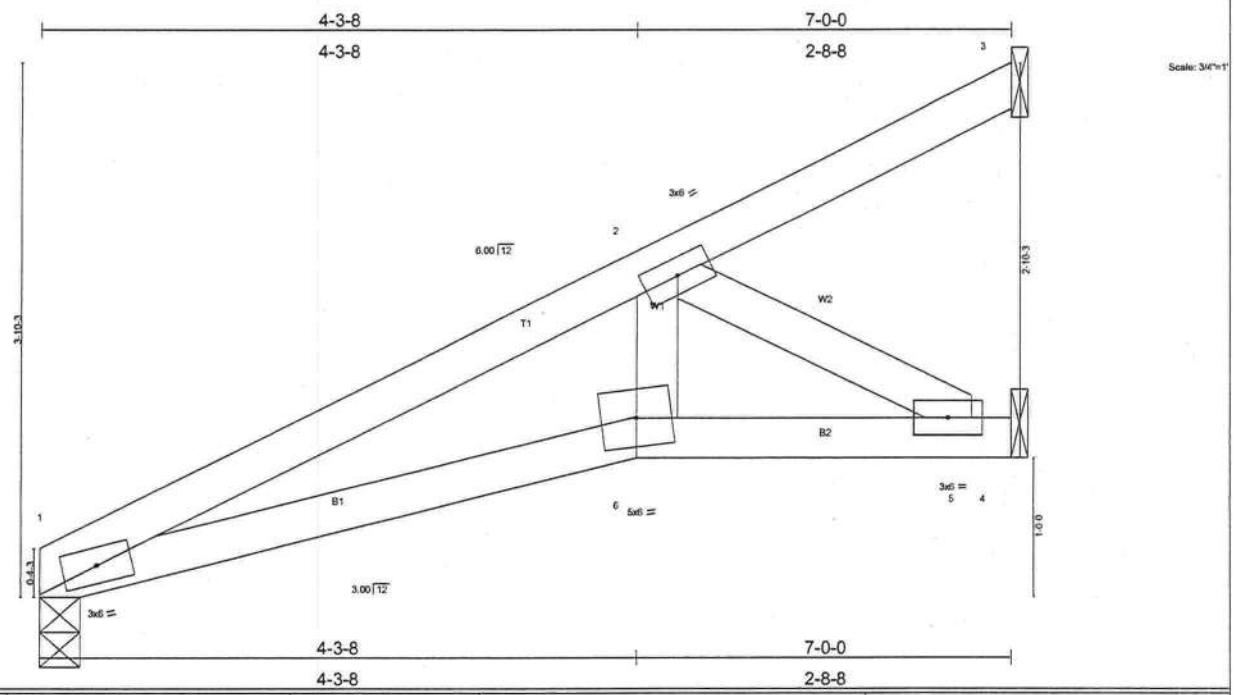
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	EJ7TA	SPECIAL	3	1	Job Reference (optional)

Dwg.#101905213

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:47:11 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	0.03	1-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.03	1-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 28 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-7-10 oc bracing.

REACTIONS (lb/size) 1=285/0-3-8, 3=41/Mechanical, 4=244/Mechanical
Max Horz 1=194(load case 5)
Max Uplift1=-87(load case 5), 3=-44(load case 5), 4=-144(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-585/338, 2-3=-42/12
BOT CHORD 1-6=-503/506, 5-6=-462/440, 4-5=0/0
WEBS 2-6=-150/278, 2-5=-503/528

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 1, 44 lb uplift at joint 3 and 144 lb uplift at joint 4.

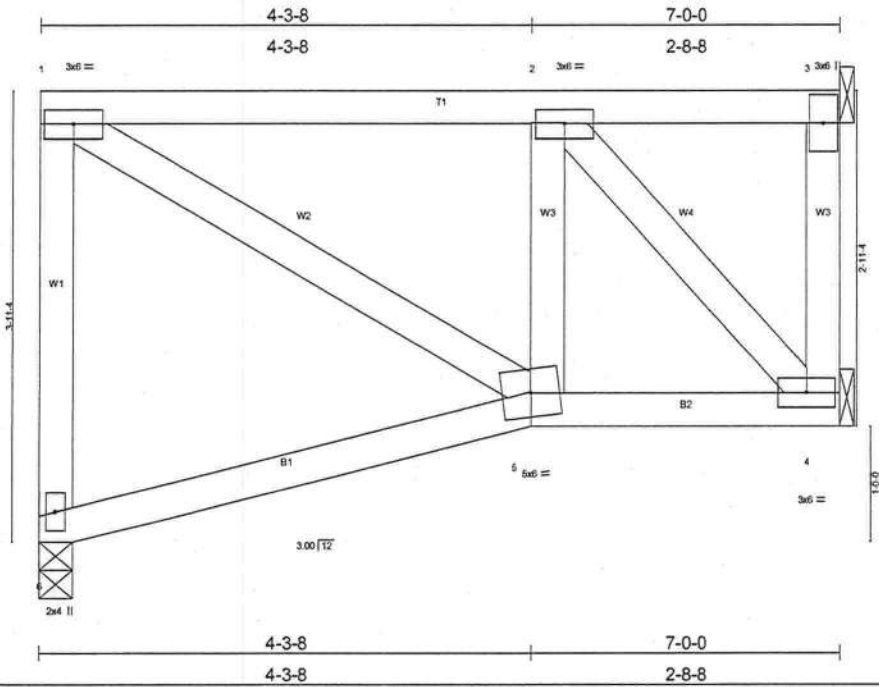
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	F01	SPECIAL	1	1	Job Reference (optional)

Dwg.#101905214

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:47:14 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.01	5-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	5-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=282/0-3-8, 3=40/Mechanical, 4=242/Mechanical
Max Uplift6=-136(load case 3), 3=-31(load case 3), 4=-105(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-6=-223/231, 1-2=-200/154, 2-3=-6/4, 3-4=0/0
BOT CHORD 5-6=-22/41, 4-5=-160/200
WEBS 1-5=-157/206, 2-5=0/118, 2-4=-287/233

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 136 lb uplift at joint 6, 31 lb uplift at joint 3 and 105 lb uplift at joint 4.
6) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

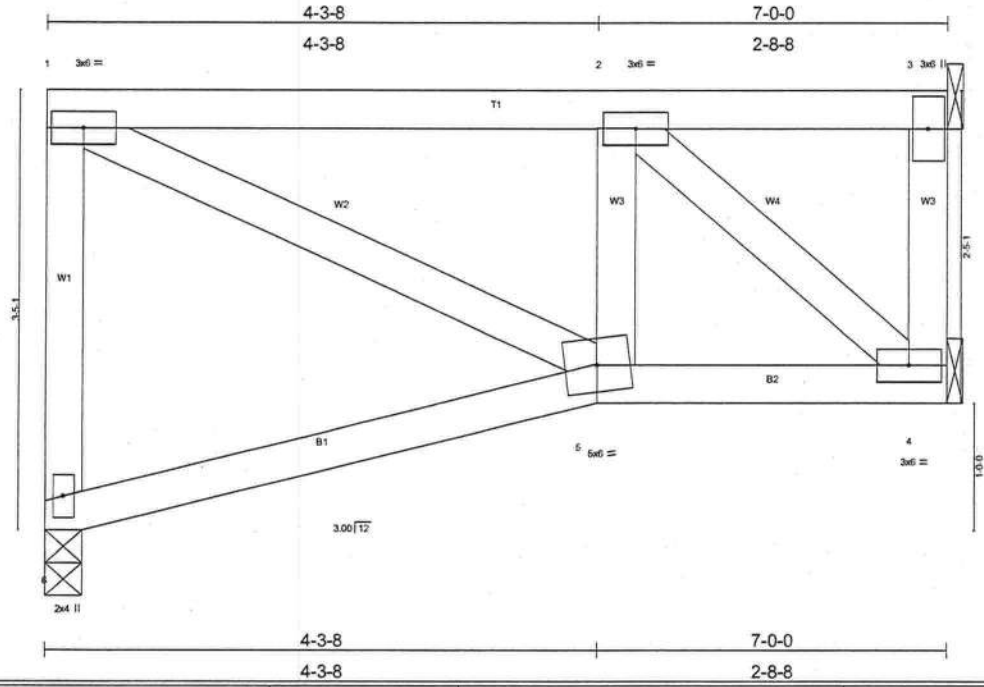
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	F02	SPECIAL	1	1	Job Reference (optional)

Dwg.#101905215

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:47:18 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.01	5-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	5-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 42 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=282/0-3-8, 3=42/Mechanical, 4=240/Mechanical
Max Uplift6=-136(load case 3), 3=-32(load case 3), 4=-104(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-6=-224/231, 1-2=-245/189, 2-3=-9/6, 3-4=0/0
BOT CHORD 5-6=-28/48, 4-5=-197/245
WEBS 1-5=-183/239, 2-5=0/117, 2-4=-315/255

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 136 lb uplift at joint 6, 32 lb uplift at joint 3 and 104 lb uplift at joint 4.
6) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

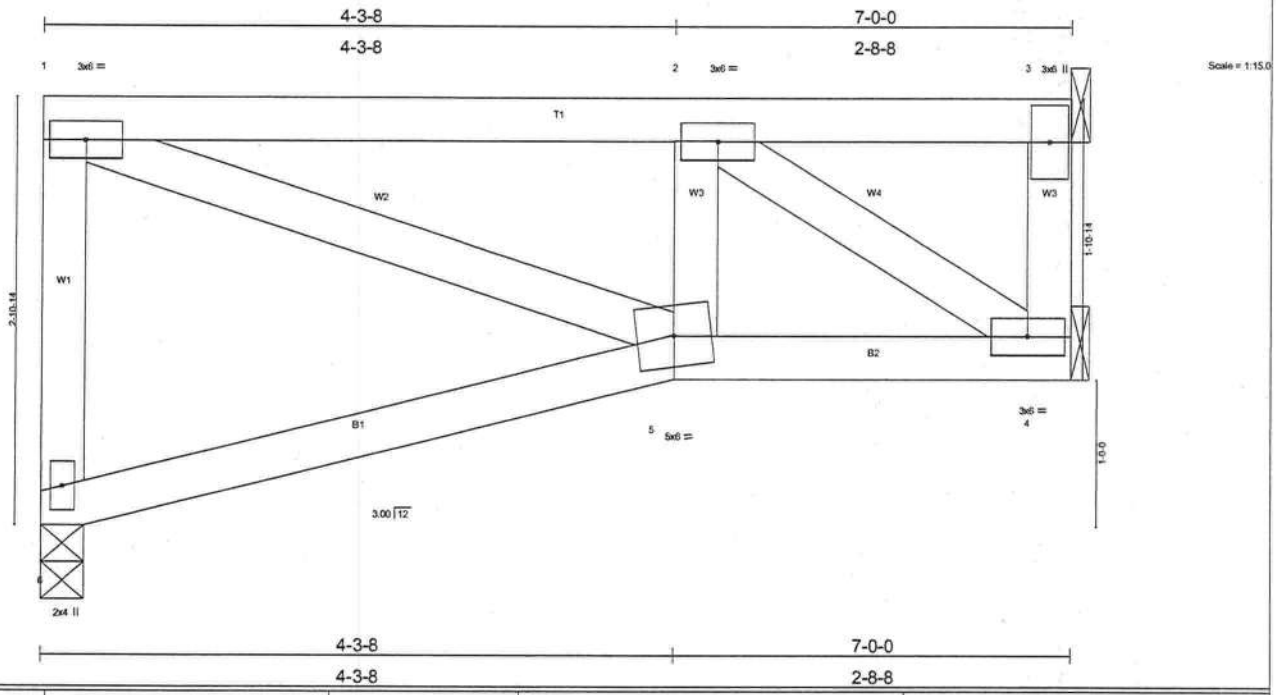
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	F03	SPECIAL	1	1	Job Reference (optional)

Dwg.#101905216

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:47:22 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	0.01	5	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.02	5-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 39 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=282/0-3-8, 4=236/Mechanical, 3=46/Mechanical
Max Uplift6=-136(load case 3), 4=-102(load case 3), 3=-34(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-6=-224/232, 1-2=-317/245, 2-3=-17/11, 3-4=0/0
BOT CHORD 5-6=-36/58, 4-5=-255/317
WEBS 1-5=-225/295, 2-5=0/117, 2-4=-361/293

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 136 lb uplift at joint 6, 102 lb uplift at joint 4 and 34 lb uplift at joint 3.
6) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

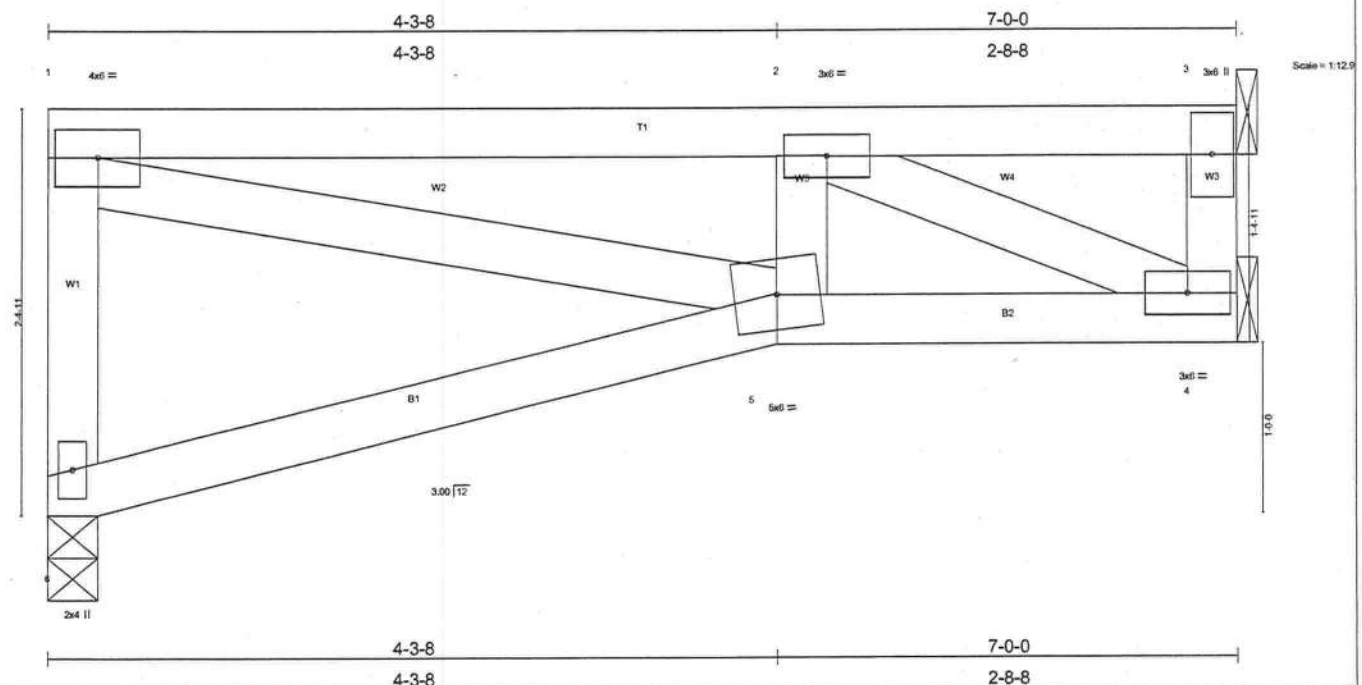
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	F04	SPECIAL	1	1	Job Reference (optional)

Dwg.#101905217

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:47:25 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.01	5	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	5-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code	FBC2004/TPI2002	(Matrix)							Weight: 36 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=282/0-3-8, 3=55/Mechanical, 4=227/Mechanical
Max Uplift6=-136(load case 3), 3=-39(load case 3), 4=-97(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-6=-224/233, 1-2=-444/342, 2-3=-41/29, 3-4=0/0
BOT CHORD 5-6=-51/76, 4-5=-357/444
WEBS 1-5=-302/396, 2-5=-1/118, 2-4=-443/360

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 136 lb uplift at joint 6, 39 lb uplift at joint 3 and 97 lb uplift at joint 4.
6) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

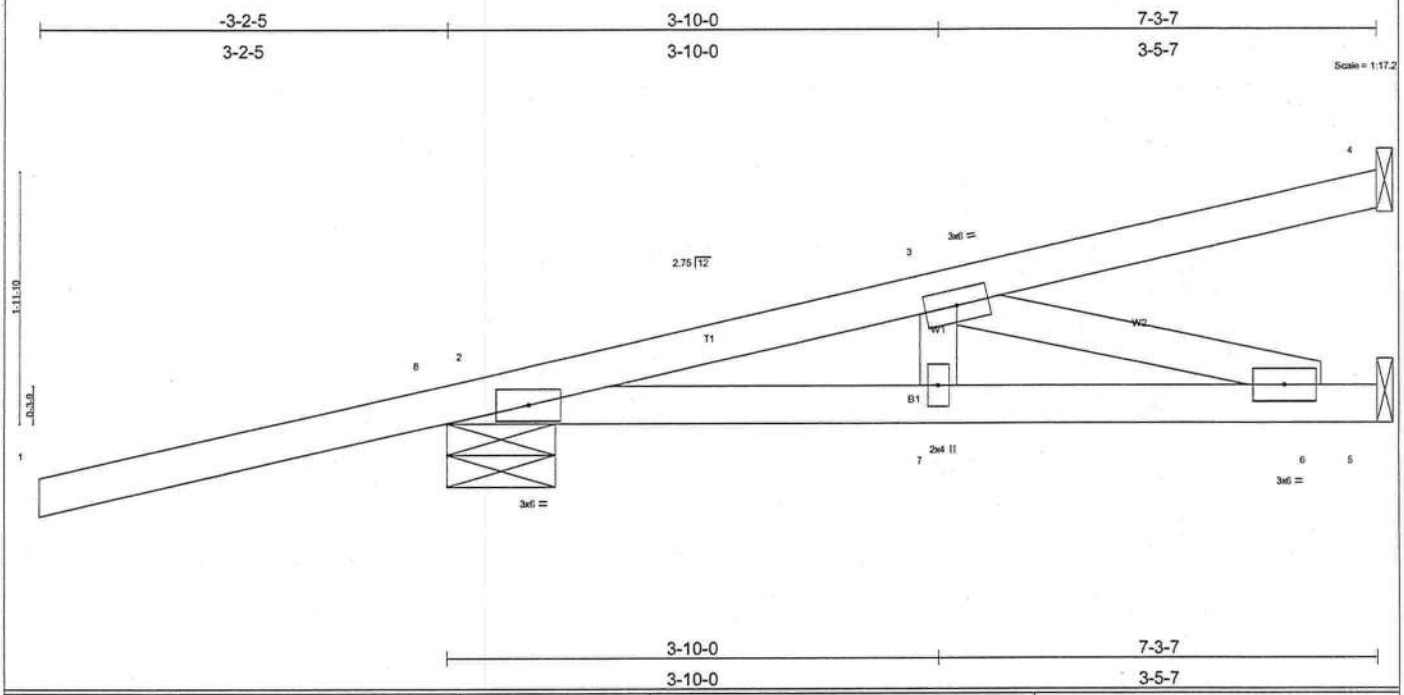
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	HJ7	MONO TRUSS	1	1	Job Reference (optional)

Dwg.#101905218

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:47:32 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	0.03	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.04	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.08	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=138/Mechanical, 2=413/0-10-2, 5=174/Mechanical
Max Horz 2=137(load case 2)
Max Uplift4=-130(load case 2), 2=-468(load case 2), 5=-113(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-8=0/35, 2-8=0/35, 2-3=-383/261, 3-4=-37/23
BOT CHORD 2-7=-304/352, 6-7=-304/352, 5-6=0/0
WEBS 3-7=-85/55, 3-6=-367/317

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 130 lb uplift at joint 4, 468 lb uplift at joint 2 and 113 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-8=-54
Trapezoidal Loads (plf)
Vert: 8=0(F=27, B=27)-to-4=-98(F=-22, B=-22), 2=-3(F=14, B=14)-to-5=-55(F=-12, B=-12)

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	HJ9	MONO TRUSS	4	1	Job Reference (optional)

Dwg.#101905219

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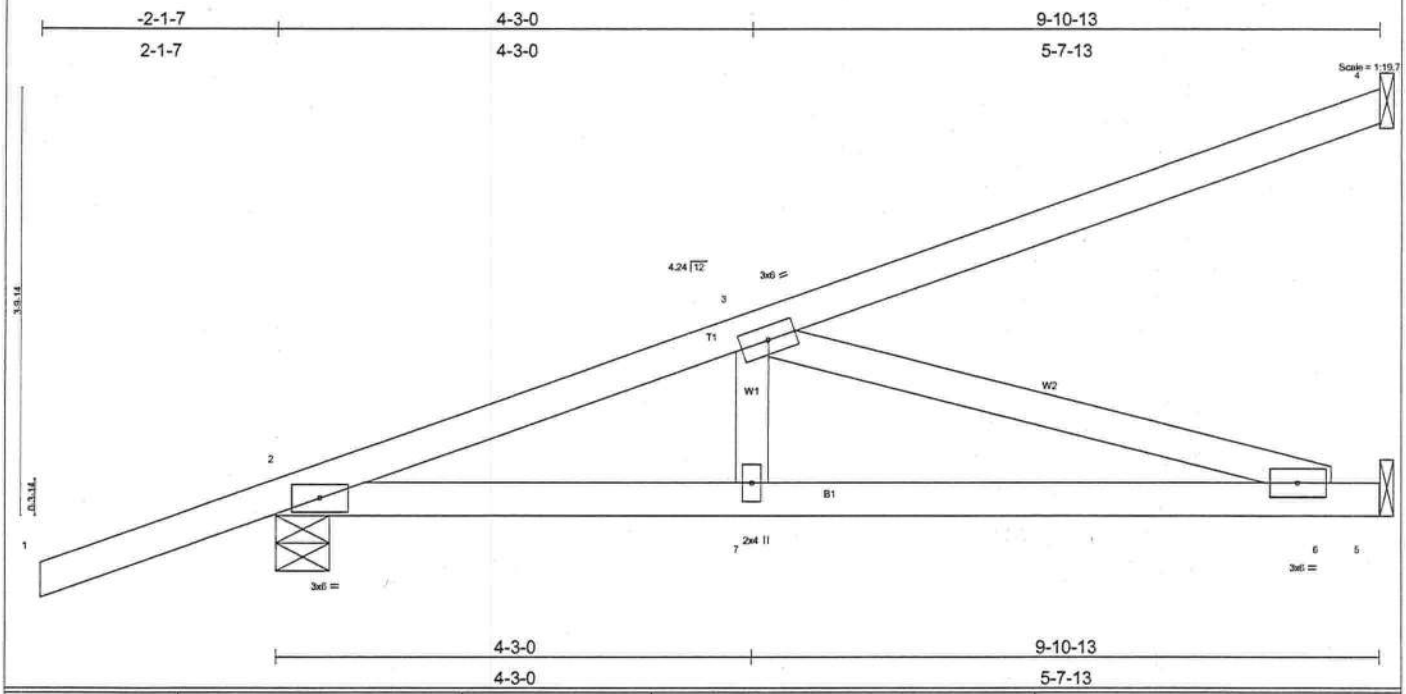

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.18	6-7	>626	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.50	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-7-8 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=486/0-5-11, 5=386/Mechanical
Max Horz 2=301(load case 2)
Max Uplift4=-281(load case 2), 2=-287(load case 2), 5=-105(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-934/243, 3-4=-121/65
BOT CHORD 2-7=-468/869, 6-7=-468/869, 5-6=0/0
WEBS 3-7=0/208, 3-6=-906/488

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 281 lb uplift at joint 4, 287 lb uplift at joint 2 and 105 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	HJ15	MONO TRUSS	1	1	Job Reference (optional)

Dwg.#101905220

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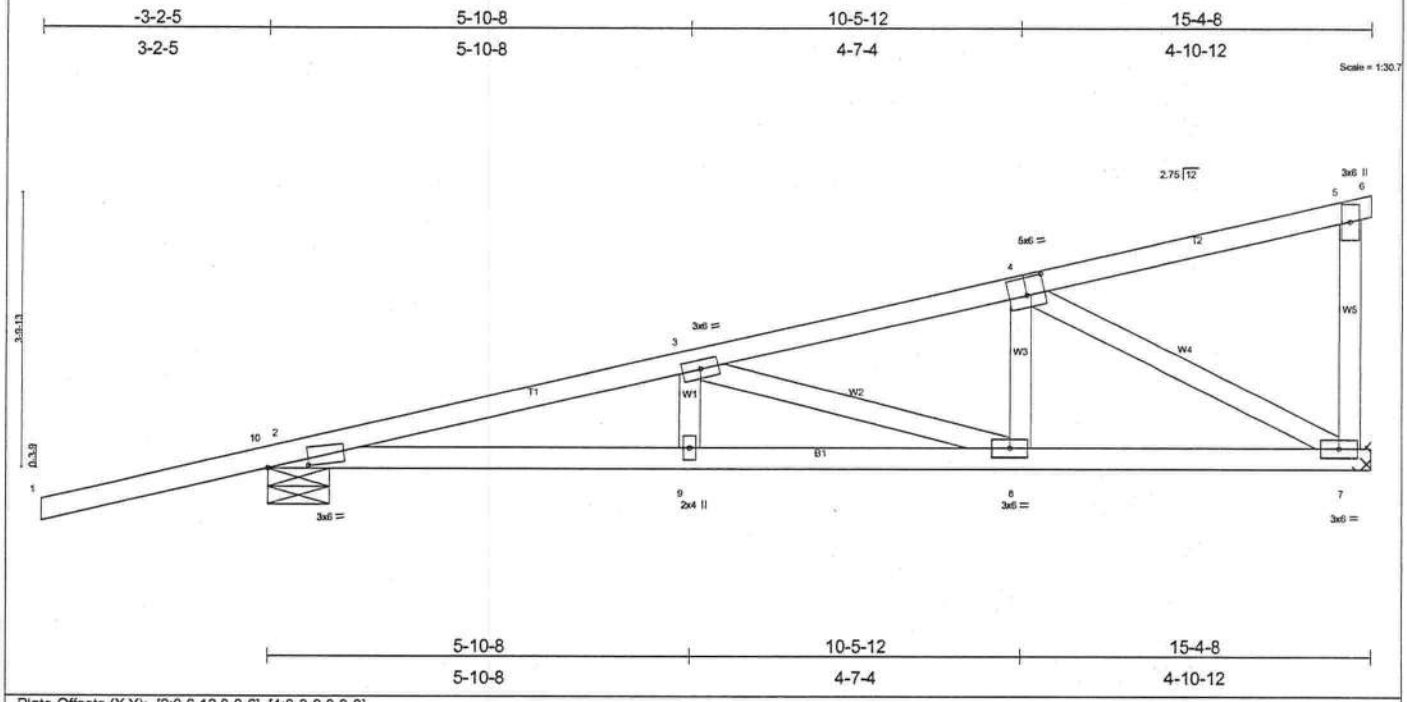

Plate Offsets (X,Y): [2:0-6-13,0-0-6], [4:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.73	Vert(LL)	-0.12	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.19	8-9	>914	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.73	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 75 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-1-2 oc bracing.

REACTIONS (lb/size) 7=1226/Mechanical, 2=822/0-10-2
Max Horz 2=350(load case 2)
Max Uplift7=-687(load case 2), 2=-547(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-10=0/35, 2-10=0/35, 2-3=-2093/660, 3-4=-1511/562, 4-5=-149/0, 5-6=-8/0, 5-7=-322/330
BOT CHORD 2-9=-913/2013, 8-9=-913/2013, 7-8=-765/1439
WEBS 3-9=0/100, 3-8=-586/203, 4-8=-43/479, 4-7=-1538/818

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 687 lb uplift at joint 7 and 547 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-10=-54
Trapezoidal Loads (plf)
Vert: 10=0(F=27, B=27)-to-5=-156(F=-51, B=-51), 5=-116(F=-51, B=-51)-to-6=-119(F=-52, B=-52), 2=-2(F=14, B=14)-to-7=-88(F=-29, B=-29)

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T01	HIP	1	1	Job Reference (optional)

Dwg.#101905221

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-1-6-0	3-9-4	7-0-0	11-7-6	16-1-0	20-6-10	25-2-0	28-4-12	32-2-0	33-8-0
1-6-0	3-9-4	3-2-12	4-7-6	4-5-10	4-5-10	4-7-6	3-2-12	3-9-4	1-6-0

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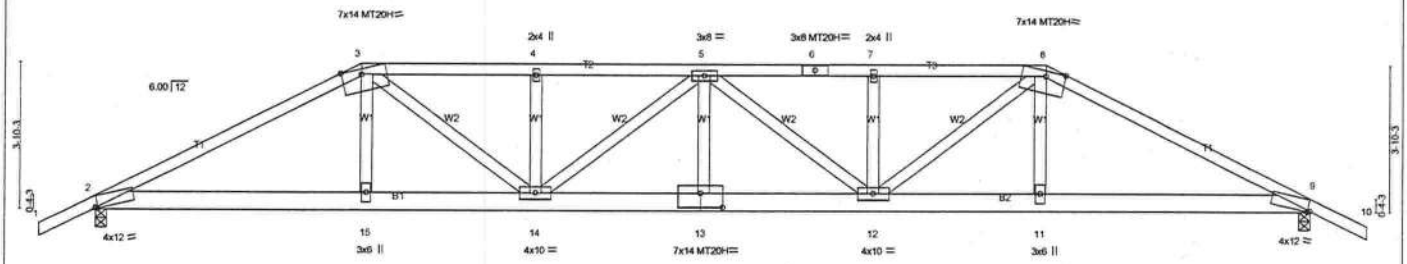

3-9-4	7-0-0	11-7-6	16-1-0	20-6-10	25-2-0	28-4-12	32-2-0
3-9-4	3-2-12	4-7-6	4-5-10	4-5-10	4-7-6	3-2-12	3-9-4

Plate Offsets (X,Y): [2:0-0-13,Edge], [3:0-6-3,Edge], [8:0-6-3,Edge], [9:0-0-13,Edge], [13:0-7-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.77	Vert(LL)	0.47	13	>817	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.75	13	>510	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.66	Horz(TL)	0.16	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 185 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except* T2 2 X 4 SYP No.2, T3 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-11-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-3-2 oc bracing.

REACTIONS (lb/size) 2=2860/0-3-8, 9=2860/0-3-8
Max Horz 2=-95(load case 5)
Max Uplift2=-1522(load case 4), 9=-1522(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-5646/3014, 3-4=-6575/3657, 4-5=-6575/3657, 5-6=-6577/3657, 6-7=-6577/3657, 7-8=-6577/3657, 8-9=-5646/3014, 9-10=0/39
BOT CHORD 2-15=-2654/4969, 14-15=-2667/5004, 13-14=-3864/7147, 12-13=-3865/7139, 11-12=-2617/5004, 9-11=-2604/4969
WEBS 3-15=-318/842, 3-14=-1220/2066, 4-14=-535/551, 5-14=-755/421, 5-13=0/341, 5-12=-742/422, 7-12=-535/551, 8-12=-1221/2069, 8-11=-319/840

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1522 lb uplift at joint 2 and 1522 lb uplift at joint 9.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 348 lb up at 25-2-0, and 539 lb down and 348 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-8=-118(F=-64), 8-10=-54, 2-15=-30, 11-15=-65(F=-35), 9-11=-30
Concentrated Loads (lb)
Vert: 15=-539(F) 11=-539(F)

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T02	HIP	1	1	Job Reference (optional)

Dwg.#101905222

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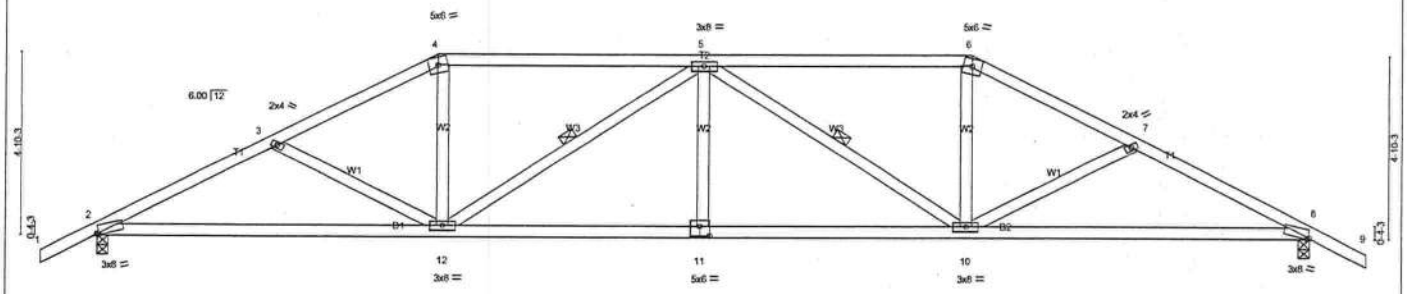
-1-6-0	4-9-4	9-0-0	16-1-0	23-2-0	27-4-12	32-2-0	33-8-0
1-6-0	4-9-4	4-2-12	7-1-0	7-1-0	4-2-12	4-9-4	1-6-0

Scale = 1:58.3



9-0-0	16-1-0	23-2-0	32-2-0
9-0-0	7-1-0	7-1-0	9-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-0-10,Edge], [11:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.22	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.37	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.21	Horz(TL)	0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 161 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-3-6 oc bracing.
WEBS 1 Row at midpt 5-12, 5-10

REACTIONS (lb/size) 2=1428/0-3-8, 8=1428/0-3-8
Max Horz 2=-109(load case 6)
Max Uplift2=-616(load case 5), 8=-616(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2435/1289, 3-4=-2210/1159, 4-5=-1957/1112, 5-6=-1958/1112, 6-7=-2211/1159, 7-8=-2436/1289, 8-9=0/35
BOT CHORD 2-12=-985/2127, 11-12=-1006/2396, 10-11=-1007/2393, 8-10=-984/2127
WEBS 3-12=-220/252, 4-12=-219/649, 5-12=-621/340, 5-11=0/175, 5-10=-617/341, 6-10=-219/650, 7-10=-219/252

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 616 lb uplift at joint 2 and 616 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T03	HIP	1	1	Job Reference (optional)

Dwg.#101905223

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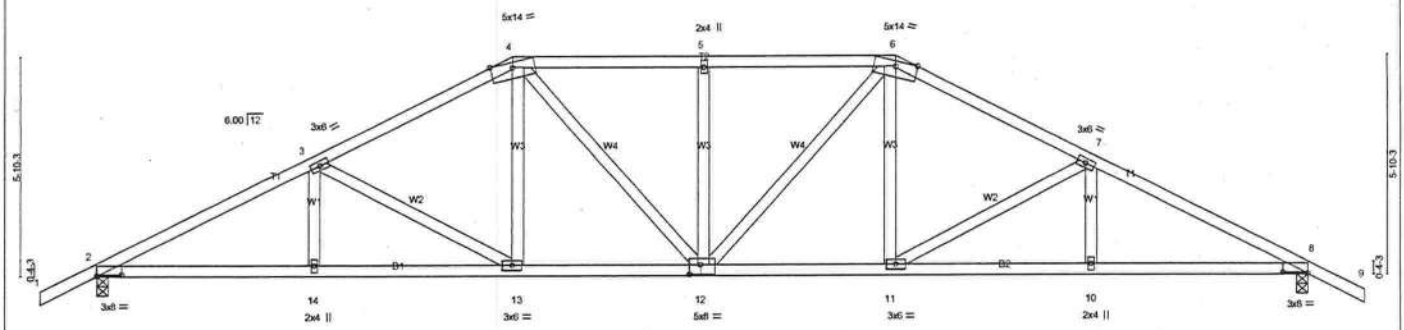
-1-6-0	5-9-4	11-0-0	16-1-0	21-2-0	26-4-12	32-2-0	33-8-0
1-6-0	5-9-4	5-2-12	5-1-0	5-1-0	5-2-12	5-9-4	1-6-0

Scale = 1:58.3



5-9-4	11-0-0	16-1-0	21-2-0	26-4-12	32-2-0
5-9-4	5-2-12	5-1-0	5-1-0	5-2-12	5-9-4

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [8:0-8-0,0-0-6], [12:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.16	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.26	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.32	Horz(TL)	0.11	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 174 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-5-8 oc bracing.

REACTIONS (lb/size) 2=1428/0-3-8, 8=1428/0-3-8
Max Horz 2=-126(load case 6)
Max Uplift2=-636(load case 5), 8=-636(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2503/1274, 3-4=-2010/1116, 4-5=-1933/1154, 5-6=-1937/1157, 6-7=-2010/1116, 7-8=-2502/1274, 8-9=0/35
BOT CHORD 2-14=-963/2162, 13-14=-963/2162, 12-13=-665/1750, 11-12=-665/1750, 10-11=-963/2162, 8-10=-963/2162
WEBS 3-14=0/179, 3-13=-487/343, 4-13=-138/387, 4-12=-202/387, 5-12=-277/247, 6-12=-206/389, 6-11=-137/390, 7-11=-486/343, 7-10=0/178

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 636 lb uplift at joint 2 and 636 lb uplift at joint 8.

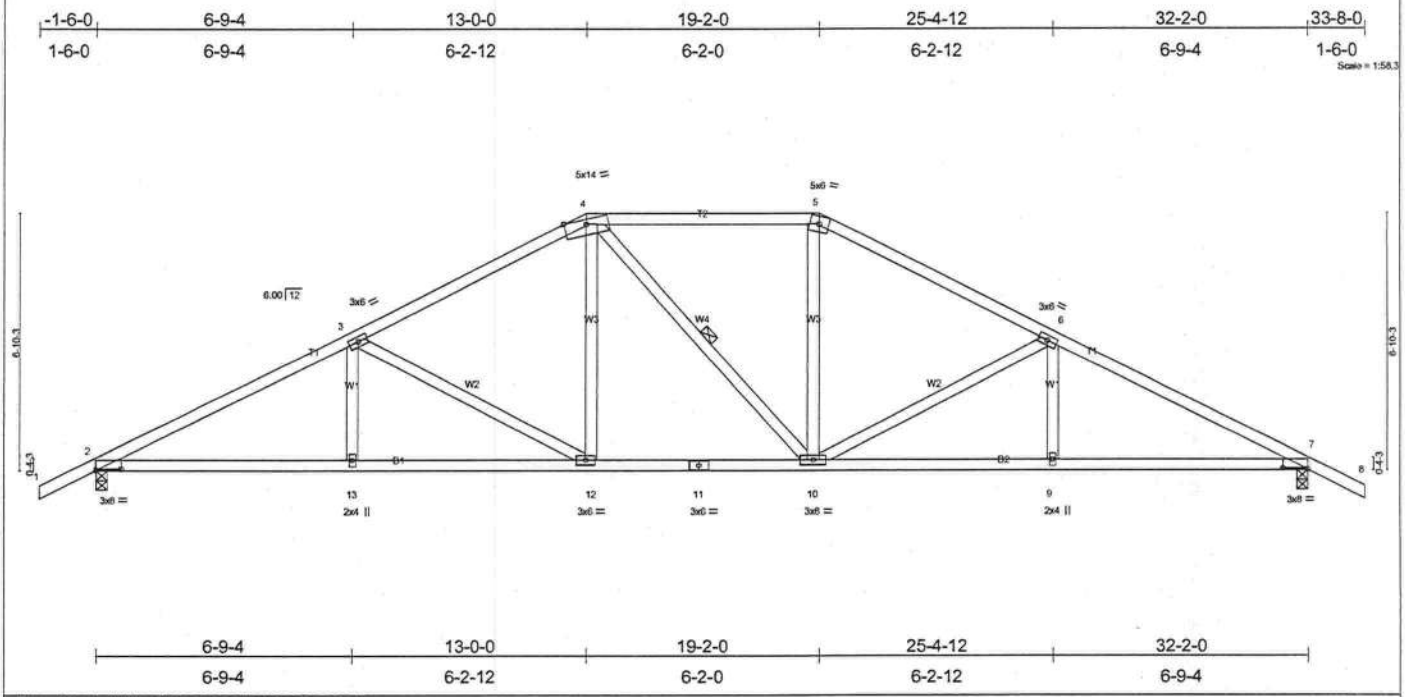
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T04	HIP	1	1	Job Reference (optional)

Dwg.#101905224

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:47:51 2005 Page 1



6-9-4	13-0-0	19-2-0	25-4-12	32-2-0
6-9-4	6-2-12	6-2-0	6-2-12	6-9-4

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [7:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.16	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.25	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.11	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-4-11 oc bracing.
WEBS 1 Row at midpt 4-10

REACTIONS (lb/size) 2=1428/0-3-8, 7=1428/0-3-8
Max Horz 2=-142(load case 6)
Max Uplift2=-654(load case 5), 7=-654(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2463/1277, 3-4=-1853/1065, 4-5=-1598/1035, 5-6=-1854/1065, 6-7=-2462/1277, 7-8=0/35
BOT CHORD 2-13=-952/2121, 12-13=-952/2121, 11-12=-570/1598, 10-11=-570/1598, 9-10=-952/2121, 7-9=-952/2121
WEBS 3-13=0/221, 3-12=-607/438, 4-12=-171/465, 4-10=-154/156, 5-10=-171/465, 6-10=-606/438, 6-9=0/220

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 654 lb uplift at joint 2 and 654 lb uplift at joint 7.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T05	HIP	1	1	Job Reference (optional)

Dwg.#101905225

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:47:54 2005 Page 1

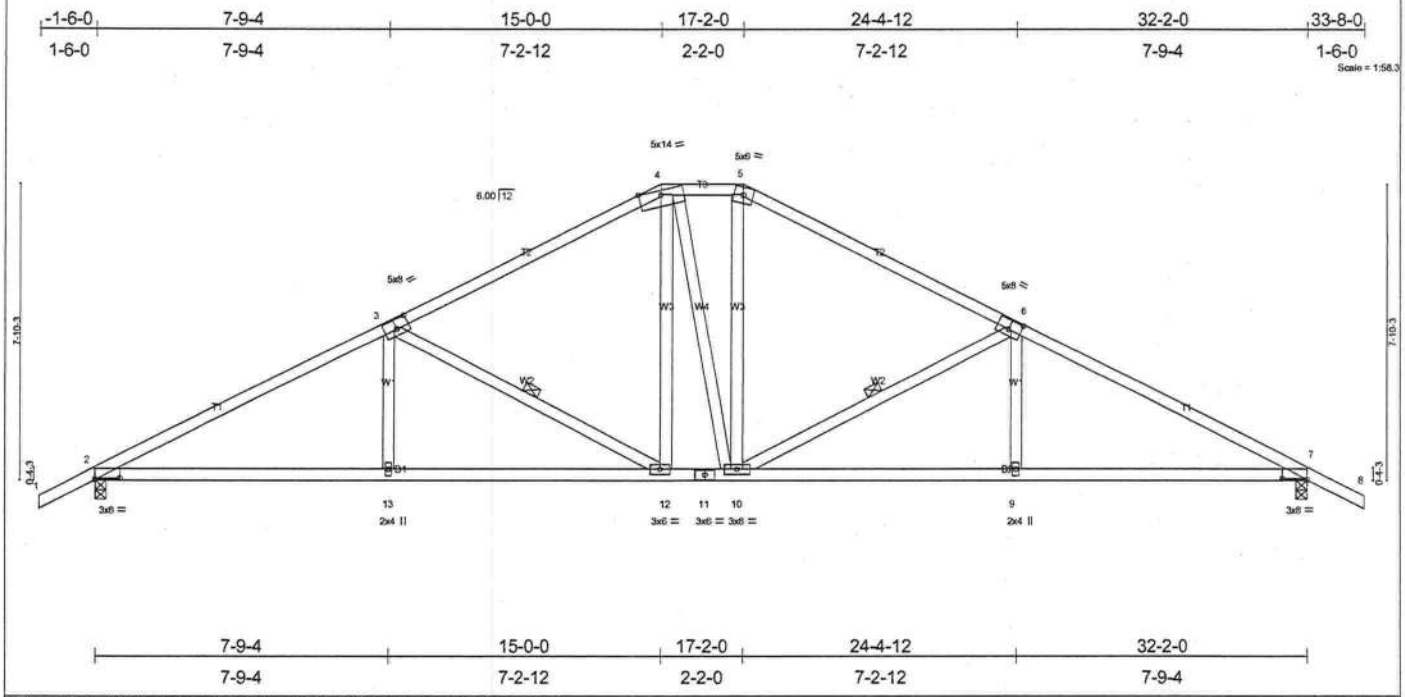


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [3:0-4-0,0-3-0], [6:0-4-0,0-3-0], [7:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.18	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.30	12-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.26	Horz(TL)	0.11	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 174 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-3-12 oc bracing.
WEBS 1 Row at midpt 3-12, 6-10

REACTIONS (lb/size) 2=1428/0-3-8, 7=1428/0-3-8
Max Horz 2=-159(load case 6)
Max Uplift2=-669(load case 5), 7=-687(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2422/1275, 3-4=-1682/1009, 4-5=-1431/997, 5-6=-1686/1011, 6-7=-2421/1275, 7-8=0/35
BOT CHORD 2-13=-938/2080, 12-13=-938/2080, 11-12=-469/1426, 10-11=-469/1426, 9-10=-938/2078, 7-9=-938/2078
WEBS 3-13=0/267, 3-12=-756/536, 4-12=-220/471, 4-10=-185/212, 5-10=-217/505, 6-10=-748/533, 6-9=0/265

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 669 lb uplift at joint 2 and 687 lb uplift at joint 7.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T06	COMMON	2	1	Dwg.#101905226

Job Reference (optional)

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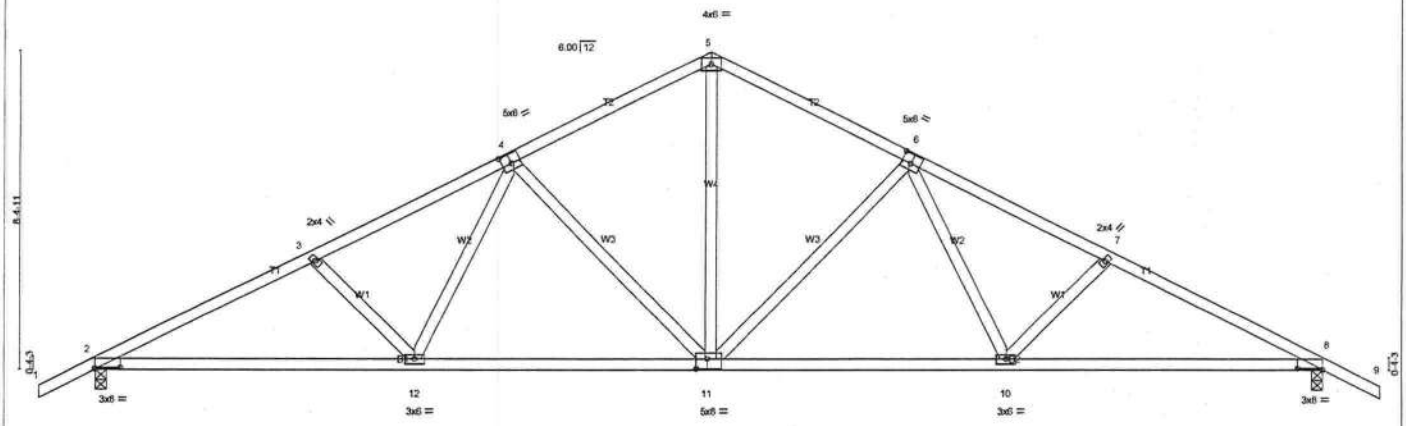
-1-6-0	5-8-11	10-10-13	16-1-0	21-3-3	26-5-5	32-2-0	33-8-0
1-6-0	5-8-11	5-2-3	5-2-3	5-2-3	5-2-3	5-8-11	1-6-0

Scale = 1:57.7



8-3-12	16-1-0	23-10-4	32-2-0
8-3-12	7-9-4	7-9-4	8-3-12

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [8:0-8-0,0-0-6], [11:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.19	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.30	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 168 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-3-5 oc bracing.

REACTIONS (lb/size) 2=1428/0-3-8, 8=1428/0-3-8
Max Horz 2=168(load case 5)
Max Uplift2=-677(load case 5), 8=-677(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2438/1339, 3-4=-2242/1270, 4-5=-1570/1008, 5-6=-1558/1001, 6-7=-2243/1269, 7-8=-2440/1338, 8-9=0/35
BOT CHORD 2-12=-1021/2125, 11-12=-722/1736, 10-11=-722/1736, 8-10=-1021/2126
WEBS 3-12=-264/314, 4-12=-196/493, 4-11=-582/452, 5-11=-617/1053, 6-11=-588/454, 6-10=-195/497, 7-10=-264/314

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 677 lb uplift at joint 2 and 677 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T07	SPECIAL	2	1	Job Reference (optional)

Dwg.#101905227

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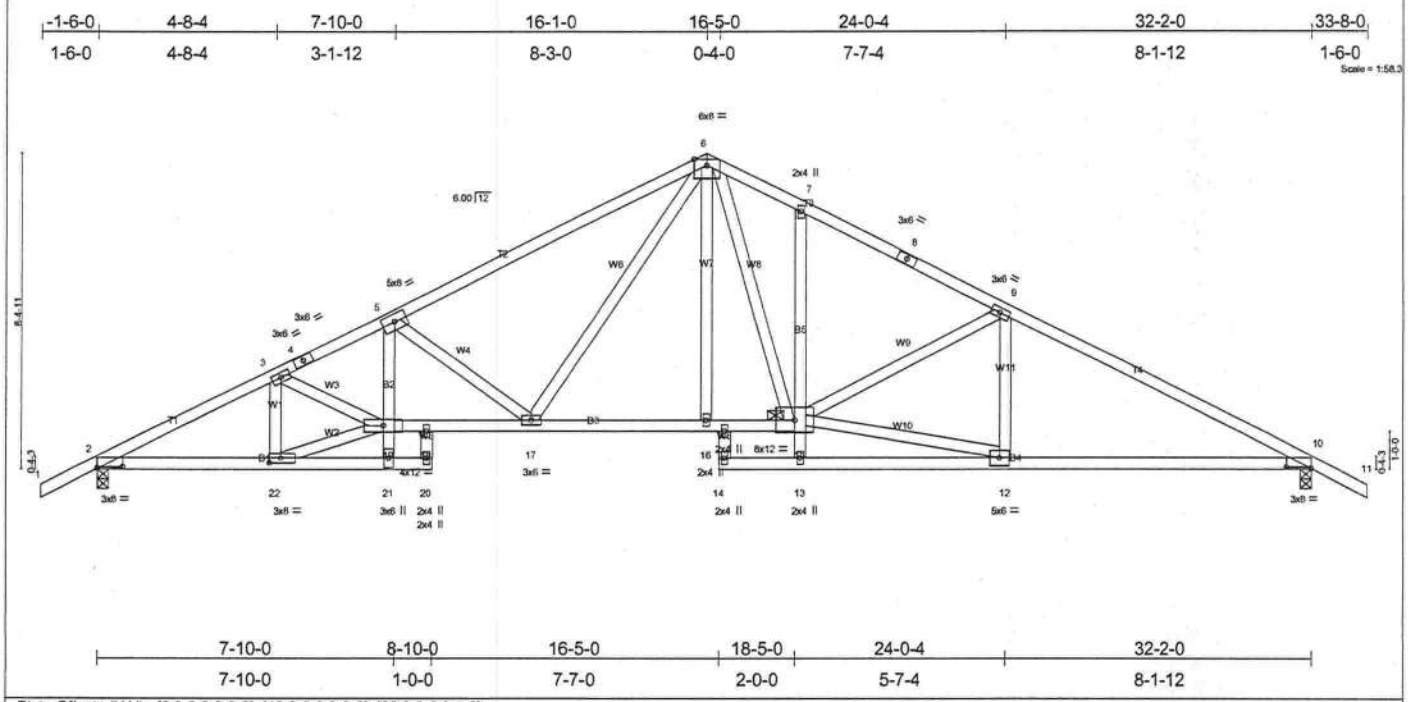

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [10:0-8-0,0-0-6], [22:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.23	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.38	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.14	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 202 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except*
T1 2 X 4 SYP No.2, T4 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B3 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-11-11 oc bracing.
JOINTS 1 Brace at Jt(s): 15

REACTIONS (lb/size) 2=1457/0-3-8, 10=1463/0-3-8
Max Horz 2=168(load case 5)
Max Uplift2=-671(load case 5), 10=-670(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2580/1303, 3-4=-3027/1558, 4-5=-2962/1568, 5-6=-2414/1319, 6-7=-2026/1192, 7-8=-2058/1164, 8-9=-2147/1146, 9-10=-2461/1252, 10-11=0/35
BOT CHORD 2-22=-996/2232, 21-22=-190/292, 20-21=0/0, 19-21=0/55, 5-19=-180/521, 18-19=-1261/2793, 17-18=-1261/2793, 16-17=-486/1612, 15-16=-486/1612, 13-15=0/135, 7-15=-138/110, 13-14=0/0, 12-13=-69/151, 10-12=-908/2110
WEBS 3-19=-177/500, 12-15=-853/1991, 9-15=-368/343, 9-12=-119/177, 6-16=-15/189, 18-20=-58/91, 3-22=-551/315, 19-22=-850/2045, 6-17=-454/840, 6-15=-345/766, 5-17=-908/662

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 671 lb uplift at joint 2 and 670 lb uplift at joint 10.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T08	CAL. DUAL PITCH	1	2	Job Reference (optional)

Dwg.#101905228

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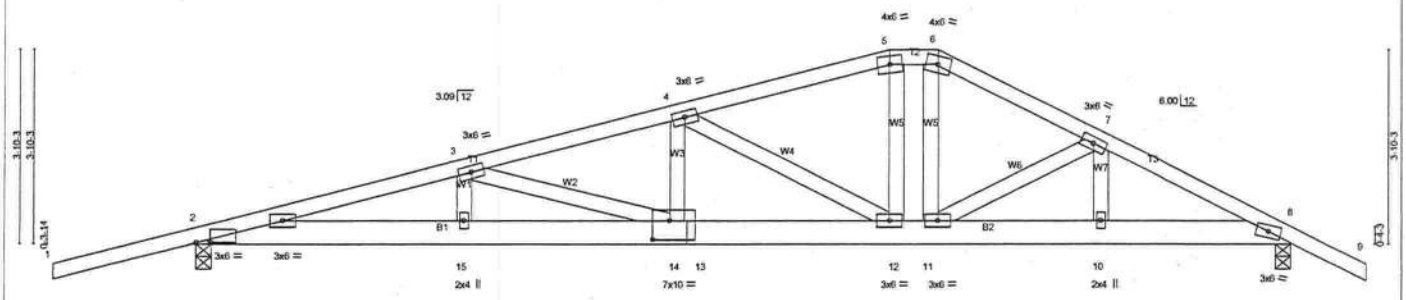
-2-9-13 | 5-3-3 | 9-5-12 | 13-8-4 | 14-8-0 | 17-10-12 | 21-8-0 | 23-2-0

2-9-13 | 5-3-3 | 4-2-9 | 4-2-9 | 0-11-12 | 3-2-12 | 3-9-4 | 1-6-0

Scale = 1:43.5



5-3-3 | 9-5-12 | 13-8-4 | 14-8-0 | 17-10-12 | 21-8-0

5-3-3 | 4-2-9 | 4-2-9 | 0-11-12 | 3-2-12 | 3-9-4

Plate Offsets (X,Y): [2:0-3-7,0-0-2], [13:0-4-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	0.13	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.21	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.23	Horz(TL)	0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 251 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1651/0-3-8, 8=2073/0-3-8
Max Horz 2=113(load case 2)
Max Uplift2=-1001(load case 4), 8=-1181(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-4765/2400, 3-4=-4414/2396, 4-5=-3595/2040, 5-6=-3486/2015, 6-7=-3849/2180, 7-8=-3900/2073, 8-9=0/39
BOT CHORD 2-15=-2314/4580, 14-15=-2314/4580, 13-14=-2251/4256, 12-13=-2251/4256, 11-12=-1858/3485, 10-11=-1780/3431, 8-10=-1780/3431
WEBS 3-15=-46/174, 3-14=-348/67, 4-14=-98/395, 4-12=-930/469, 5-12=-451/845, 6-11=-826/1447, 7-11=-148/130, 7-10=-112/148

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1001 lb uplift at joint 2 and 1181 lb uplift at joint 8.
7) Girder carries hip end with 7-0-0 right side setback, 13-8-4 left side setback, and 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 348 lb up at 14-8-0, and 1054 lb down and 680 lb up at 13-8-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-6=-118(F=-64), 6-9=-54, 2-12=-30, 11-12=-65(F=-35), 8-11=-30
Concentrated Loads (lb)
Vert: 12=-1054(F) 11=-539(F)

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T09	DUAL PITCH	7	1	Job Reference (optional)

Dwg.#101905229

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:48:09 2005 Page 1

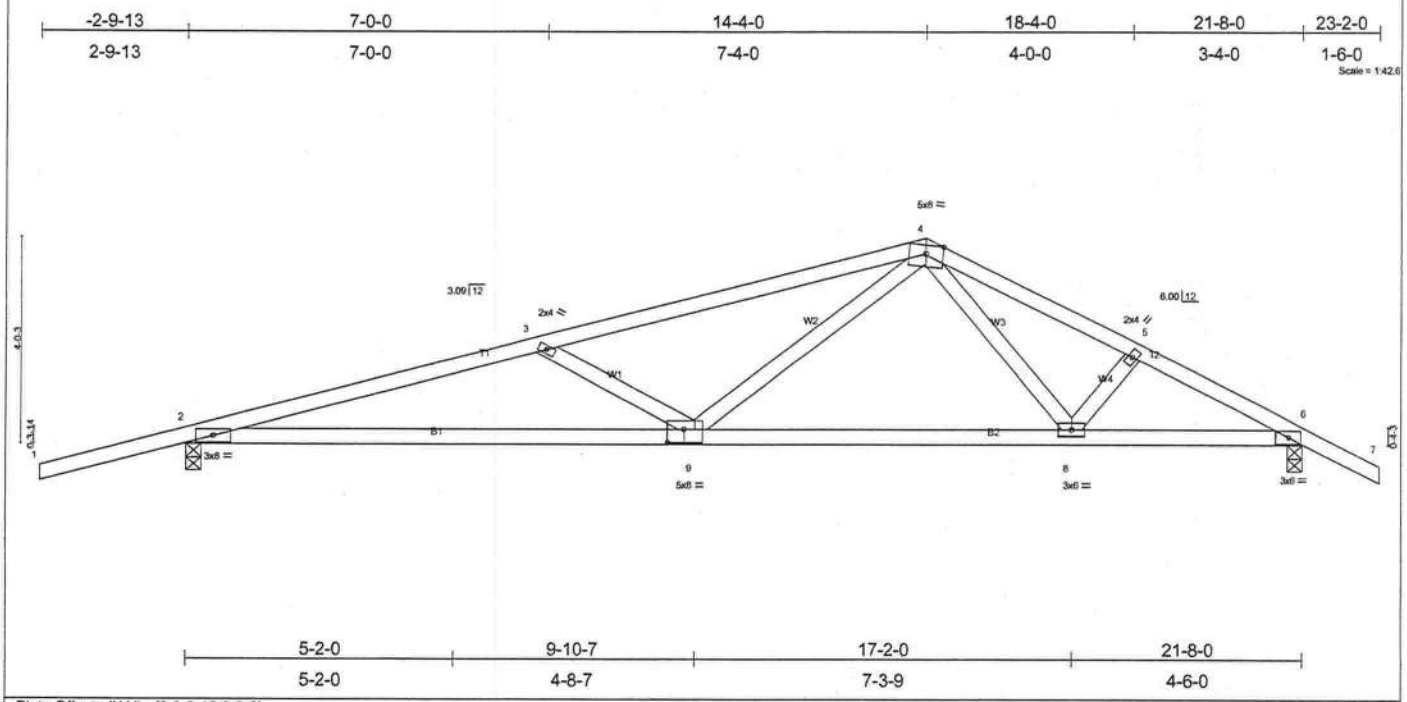


Plate Offsets (X,Y): [9:0-3-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	-0.24	2-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.41	2-9	>622	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.28	Horz(TL)	0.06	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 96 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-3-6 oc bracing.

REACTIONS (lb/size) 2=1065/0-3-8, 6=979/0-3-8
Max Horz 2=114(load case 3)
Max Uplift2=-623(load case 5), 6=-462(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-2392/1267, 3-4=-1921/931, 4-5=-1502/799, 5-6=-1602/804, 6-7=0/35
BOT CHORD 2-9=-1089/2272, 8-9=-453/1156, 6-8=-569/1370
WEBS 3-9=-545/495, 4-9=-270/858, 4-8=-89/340, 5-8=-136/142

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 623 lb uplift at joint 2 and 462 lb uplift at joint 6.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

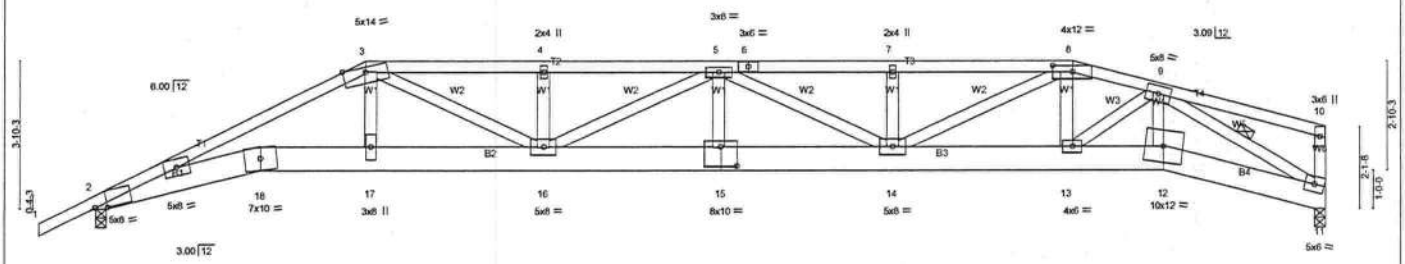
Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T10	SPECIAL	1	1	Job Reference (optional)

Dwg.#101905230

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:48:13 2005 Page 1

-1-6-0	4-3-8	7-0-0	11-8-5	16-2-14	20-9-7	25-5-12	27-10-8	32-2-0
1-6-0	4-3-8	2-8-8	4-8-5	4-6-9	4-6-9	4-8-5	2-4-12	4-3-8

Scale = 1:57.5



4-3-8	7-0-0	11-8-5	16-2-14	20-9-7	25-5-12	27-10-8	32-2-0
4-3-8	2-8-8	4-8-5	4-6-9	4-6-9	4-8-5	2-4-12	4-3-8

Plate Offsets (X,Y): [2:0-3-14,0-0-11], [8:0-6-0,0-1-15], [15:0-5-0,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.82	Vert(LL)	-0.41	15-16	>943	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.65	15-16	>589	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.52	Horz(TL)	0.25	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 210 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-8-3 oc bracing.
WEBS 1 Row at midpt 9-11

REACTIONS (lb/size) 2=1430/0-3-8, 11=1336/0-3-8
Max Horz 2=188(load case 4)
Max Uplift2=-593(load case 4), 11=-627(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-3652/1524, 3-4=-3950/1825, 4-5=-3950/1825, 5-6=-3933/1876, 6-7=-3933/1876, 7-8=-3933/1876, 8-9=-3103/1491, 9-10=-266/165, 10-11=-214/158
BOT CHORD 2-18=-1337/3236, 17-18=-1318/3196, 16-17=-1334/3250, 15-16=-2012/4466, 14-15=-2013/4473, 13-14=-1368/3008, 12-13=-1142/2470, 11-12=-1164/2542
WEBS 3-17=-261/891, 3-16=-509/933, 4-16=-240/215, 5-16=-619/316, 5-15=-51/296, 5-14=-659/259, 7-14=-240/216, 8-14=-475/1108, 8-13=-17/71, 9-13=-313/739, 9-12=-59/292, 9-11=-2727/1231

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 2, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 593 lb uplift at joint 2 and 627 lb uplift at joint 11.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T11	SPECIAL	1	1	Job Reference (optional)

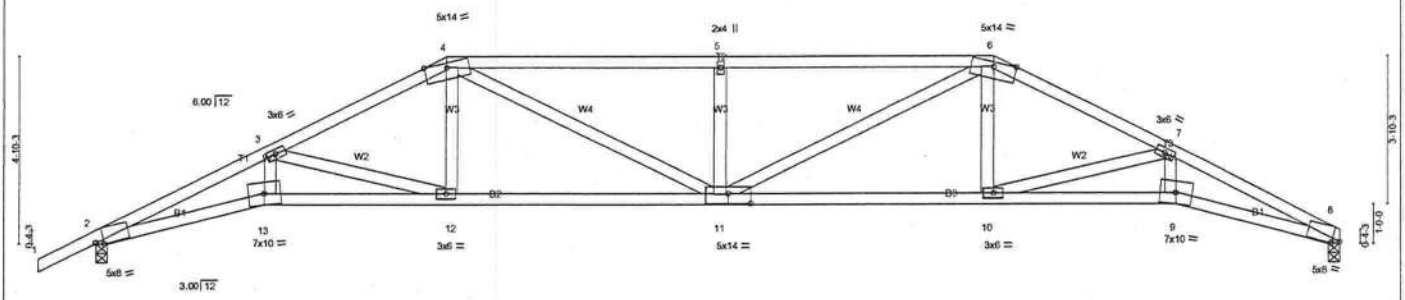
Dwg.#101905231

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:48:17 2005 Page 1

-1-6-0 | 4-3-8 | 9-0-0 | 16-1-0 | 23-2-0 | 27-10-8 | 32-2-0

1-6-0 | 4-3-8 | 4-8-8 | 7-1-0 | 7-1-0 | 4-8-8 | 4-3-8

Scale = 1:56.9



4-3-8 | 9-0-0 | 16-1-0 | 23-2-0 | 27-10-8 | 32-2-0

4-3-8 | 4-8-8 | 7-1-0 | 7-1-0 | 4-8-8 | 4-3-8

Plate Offsets (X,Y): [2:0-2-7,Edge], [8:0-2-7,Edge], [11:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.44	10-11	>878	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.70	10-11	>546	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.42	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 155 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 2=1430/0-3-8, 8=1336/0-3-8
Max Horz 2=129(load case 5)
Max Uplift2=-617(load case 5), 8=-500(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4502/2201, 3-4=-2784/1440, 4-5=-3078/1662, 5-6=-3078/1662, 6-7=-2793/1459, 7-8=-4551/2317
BOT CHORD 2-13=-1910/4040, 12-13=-1811/3794, 11-12=-1092/2494, 10-11=-1108/2502, 9-10=-1915/3848, 8-9=-2029/4101
WEBS 3-13=-397/1046, 3-12=-1370/752, 4-12=-182/578, 4-11=-399/777, 5-11=-390/349, 6-11=-393/771, 6-10=-201/588, 7-10=-1418/844, 7-9=-450/1073

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 617 lb uplift at joint 2 and 500 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T12	SPECIAL	1	1	Job Reference (optional)

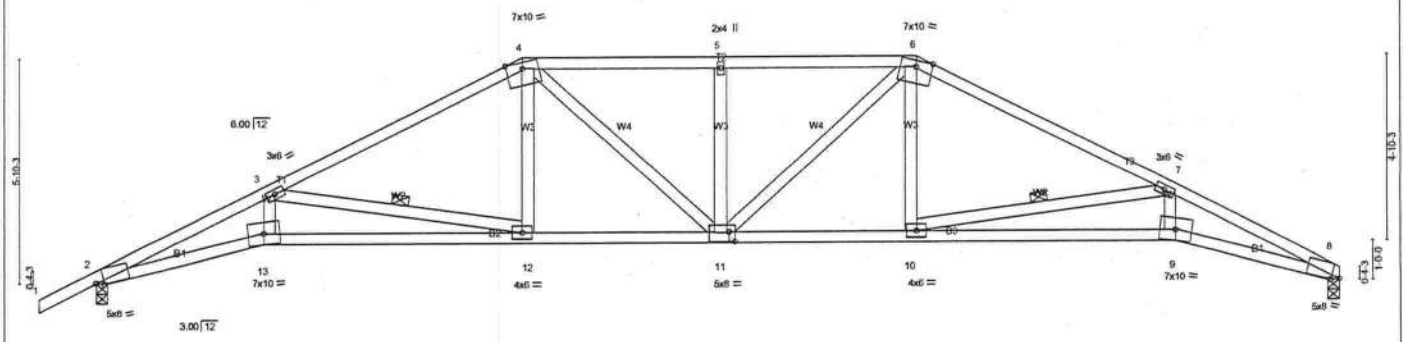
Dwg.#101905232

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:48:20 2005 Page 1

-1-6-0 | 4-3-8 | 11-0-0 | 16-1-0 | 21-2-0 | 27-10-8 | 32-2-0

1-6-0 | 4-3-8 | 6-8-8 | 5-1-0 | 5-1-0 | 6-8-8 | 4-3-8

Scale = 1:56.9



4-3-8 | 11-0-0 | 16-1-0 | 21-2-0 | 27-10-8 | 32-2-0

4-3-8 | 6-8-8 | 5-1-0 | 5-1-0 | 6-8-8 | 4-3-8

Plate Offsets (X,Y): [2:0-2-7,Edge], [8:0-2-7,Edge], [11:0-2-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.43	9-10	>886	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.70	9-10	>549	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.46	Horz(TL)	0.44	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 163 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 3-12, 7-10

REACTIONS (lb/size) 2=1430/0-3-8, 8=1336/0-3-8
Max Horz 2=145(load case 5)
Max Uplift2=-637(load case 5), 8=-520(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4609/2296, 3-4=-2471/1306, 4-5=-2347/1339, 5-6=-2347/1339, 6-7=-2476/1316, 7-8=-4655/2407
BOT CHORD 2-13=-2007/4148, 12-13=-1907/3888, 11-12=-929/2181, 10-11=-938/2185, 9-10=-2006/3940, 8-9=-2120/4207
WEBS 3-13=-404/1125, 3-12=-1755/1000, 4-12=-171/556, 4-11=-218/375, 5-11=-234/228, 6-11=-213/371, 6-10=-185/564, 7-10=-1803/1093, 7-9=-453/1151

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 637 lb uplift at joint 2 and 520 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

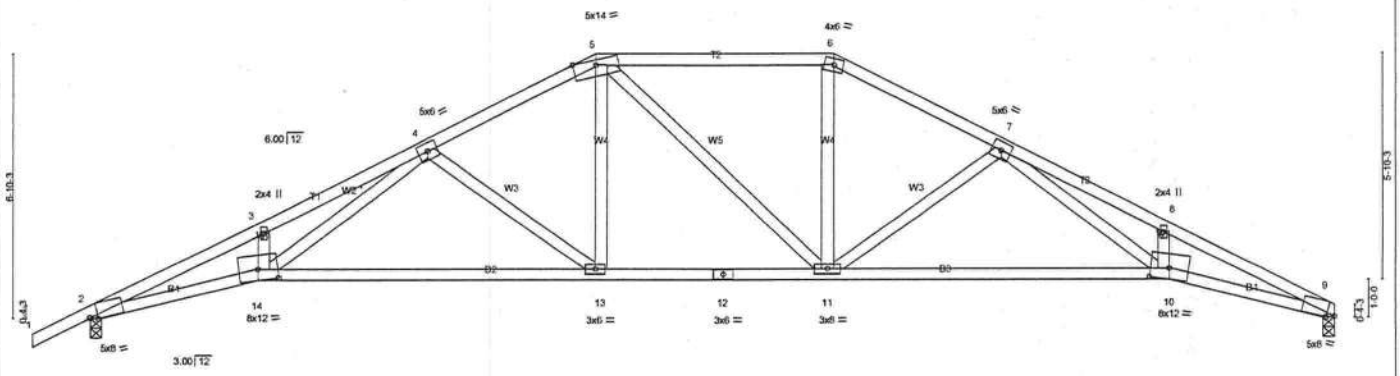
Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T13	SPECIAL	1	1	Job Reference (optional)

Dwg.#101905233

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:48:24 2005 Page 1

-1-6-0	4-3-8	8-7-12	13-0-0	19-2-0	23-6-4	27-10-8	32-2-0
1-6-0	4-3-8	4-4-4	4-4-4	6-2-0	4-4-4	4-4-4	4-3-8

Scale = 1:56.9



4-3-8	13-0-0	19-2-0	27-10-8	32-2-0
4-3-8	8-8-8	6-2-0	8-8-8	4-3-8

Plate Offsets (X,Y): [2:0-2-7,Edge], [9:0-2-7,Edge], [10:0-6-0,0-3-8], [14:0-6-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.53	13-14	>716	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.87	13-14	>438	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.65	Horz(TL)	0.41	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 165 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-3-8 oc bracing.

REACTIONS (lb/size) 2=1430/0-3-8, 9=1336/0-3-8
Max Horz 2=162(load case 5)
Max Uplift2=-655(load case 5), 9=-538(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4618/2243, 3-4=-4505/2365, 4-5=-2158/1216, 5-6=-1904/1159, 6-7=-2161/1220, 7-8=-4568/2488, 8-9=-4675/2356
BOT CHORD 2-14=-1947/4152, 13-14=-1160/2444, 12-13=-756/1901, 11-12=-756/1901, 10-11=-1180/2454, 9-10=-2061/4210
WEBS 3-14=-32/188, 4-14=-953/1979, 4-13=-699/514, 5-13=-273/653, 5-11=-158/165, 6-11=-278/655, 7-11=-708/533, 7-10=-1065/2036, 8-10=-42/208

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 2, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 655 lb uplift at joint 2 and 538 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T14	SPECIAL	1	1	Job Reference (optional)

Dwg.#101905234

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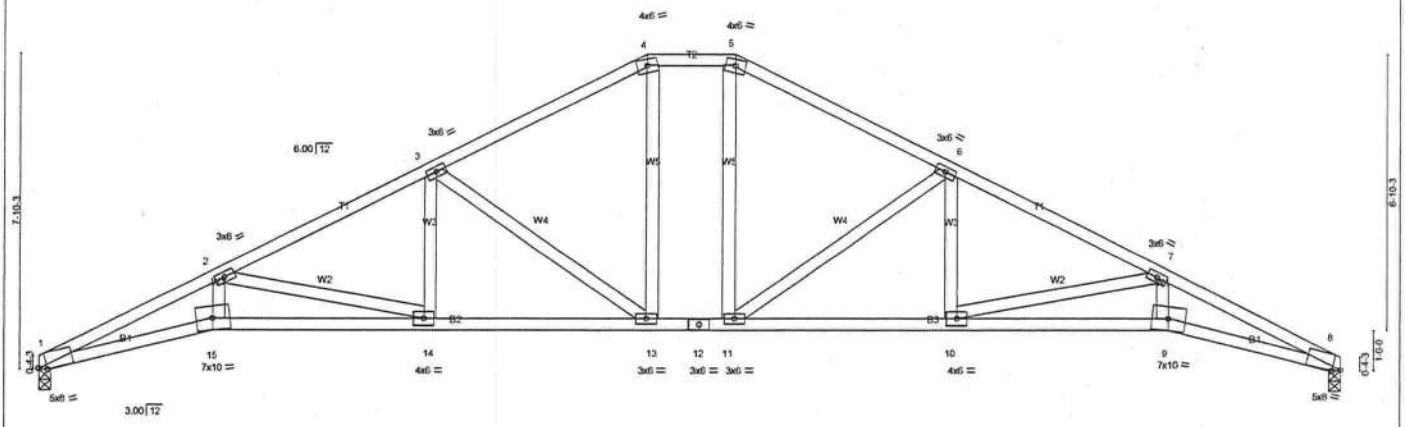
4-3-8	9-7-12	15-0-0	17-2-0	22-6-4	27-10-8	32-2-0
4-3-8	5-4-4	5-4-4	2-2-0	5-4-4	5-4-4	4-3-8

Scale = 1:54.4



4-3-8	9-7-12	15-0-0	17-2-0	22-6-4	27-10-8	32-2-0
4-3-8	5-4-4	5-4-4	2-2-0	5-4-4	5-4-4	4-3-8

Plate Offsets (X,Y): [1:0-2-7,Edge], [8:0-2-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.37	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.59	10-11	>646	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.42	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 168 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-2-10 oc bracing.

REACTIONS (lb/size) 1=1339/0-3-8, 8=1339/0-3-8
Max Horz 1=-131(load case 3)
Max Uplift1=-554(load case 5), 8=-554(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4600/2452, 2-3=-2696/1493, 3-4=-1892/1135, 4-5=-1642/1088, 5-6=-1892/1135, 6-7=-2696/1493, 7-8=-4600/2452
BOT CHORD 1-15=-2155/4150, 14-15=-2035/3892, 13-14=-1117/2379, 12-13=-629/1642, 11-12=-629/1642, 10-11=-1117/2379, 9-10=-2035/3892, 8-9=-2155/4150
WEBS 2-15=-476/1104, 2-14=-1556/944, 3-14=-203/570, 3-13=-927/605, 4-13=-325/627, 5-11=-325/627, 6-11=-927/605, 6-10=-203/570, 7-10=-1556/944, 7-9=-476/1104

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 554 lb uplift at joint 1 and 554 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T15	SPECIAL	2	1	Job Reference (optional)

Dwg.#101905235

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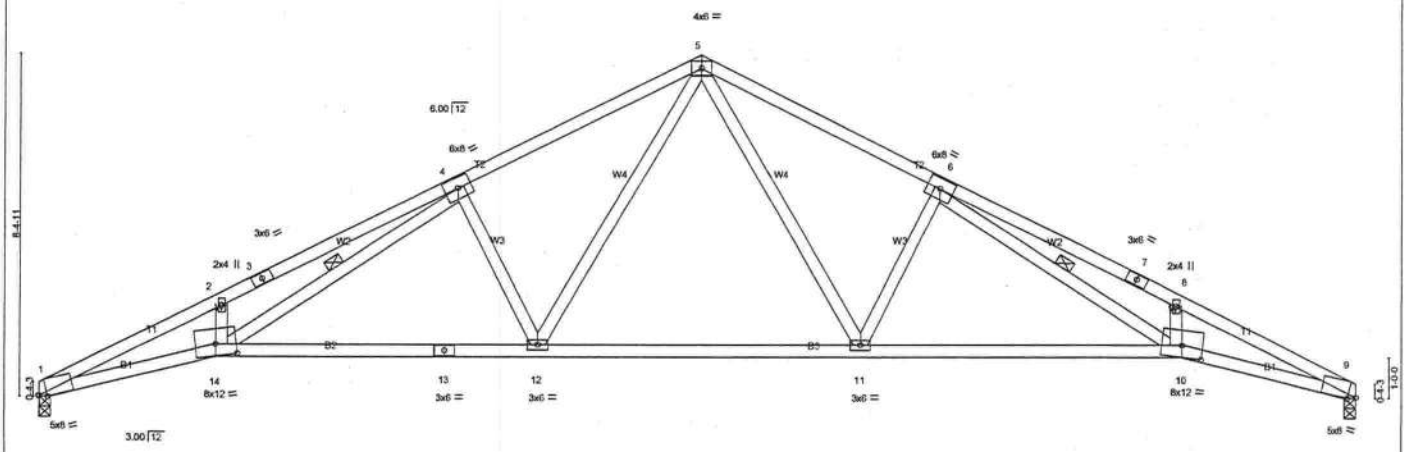
4-3-8	10-2-4	16-1-0	21-11-12	27-10-8	32-2-0
4-3-8	5-10-12	5-10-12	5-10-12	5-10-12	4-3-8

Scale = 1:53.8



4-3-8	12-1-13	20-0-3	27-10-8	32-2-0
4-3-8	7-10-5	7-10-5	7-10-5	4-3-8

Plate Offsets (X,Y): [1:0-2-7,Edge], [9:0-2-7,Edge], [10:0-6-0,0-3-8], [14:0-6-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	-0.51	10-11	>753	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.82	10-11	>465	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.43	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 162 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-2-4 oc bracing.
WEBS 1 Row at midpt 4-14, 6-10

REACTIONS (lb/size) 1=1339/0-3-8, 9=1339/0-3-8
Max Horz 1=140(load case 4)
Max Uplift1=-561(load case 5), 9=-561(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4632/2443, 2-3=-4554/2592, 3-4=-4518/2609, 4-5=-2299/1399, 5-6=-2299/1399, 6-7=-4519/2610, 7-8=-4555/2592, 8-9=-4632/2443
BOT CHORD 1-14=-2143/4167, 13-14=-1089/2276, 12-13=-1089/2276, 11-12=-578/1521, 10-11=-1089/2276, 9-10=-2143/4168
WEBS 2-14=-105/261, 4-14=-1249/2157, 4-12=-647/564, 5-12=-536/963, 5-11=-536/964, 6-11=-647/564, 6-10=-1249/2157, 8-10=-105/261

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 561 lb uplift at joint 1 and 561 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T16	SPECIAL	1	1	Job Reference (optional)

Dwg.#101905236

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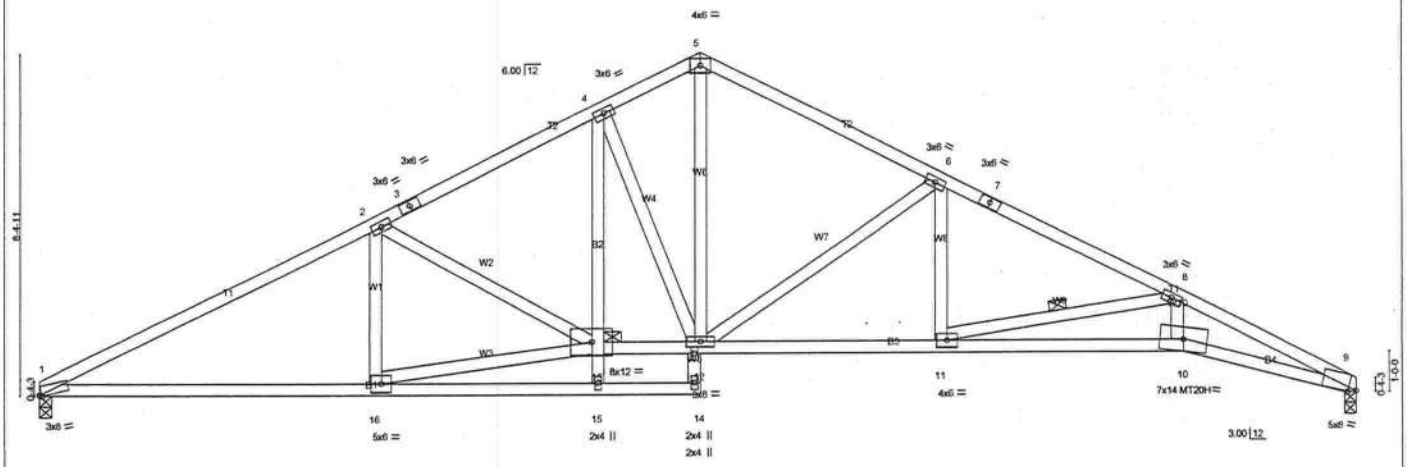
8-1-12 | 13-9-0 | 16-1-0 | 21-11-12 | 27-10-8 | 32-2-0
8-1-12 | 5-7-4 | 2-4-0 | 5-10-12 | 5-10-12 | 4-3-8

Scale = 1:53.8



8-1-12 | 13-9-0 | 15-9-0 | 21-11-12 | 27-10-8 | 32-2-0
8-1-12 | 5-7-4 | 2-0-0 | 6-2-12 | 5-10-12 | 4-3-8

Plate Offsets (X,Y): [1:0-0-10,Edge], [9:0-2-6,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.36	14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.58	14	>661	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.85	Horz(TL)	0.29	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 183 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 8-11
JOINTS 1 Brace at Jt(s): 13

REACTIONS (lb/size) 1=1379/0-3-8, 9=1373/0-3-8
Max Horz 1=-141(load case 3)
Max Uplift1=-554(load case 5), 9=-555(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2494/1295, 2-3=-2186/1187, 3-4=-2166/1206, 4-5=-1804/1105, 5-6=-1879/1083, 6-7=-2594/1441, 7-8=-2705/1429, 8-9=-4747/2454
BOT CHORD 1-16=-1017/2141, 15-16=-67/126, 14-15=0/0, 13-15=0/146, 4-13=-290/637, 12-13=-708/1873, 11-12=-1062/2381, 10-11=-2038/4017, 9-10=-2158/4285
WEBS 2-16=-128/182, 13-16=-967/2050, 2-13=-367/349, 5-12=-718/1339, 6-12=-938/623, 6-11=-188/570, 8-11=-1675/999, 8-10=-474/1149, 4-12=-688/400

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are MT20 plates unless otherwise indicated.
4) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 554 lb uplift at joint 1 and 555 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T17	MONO HIP	1	1	Job Reference (optional)

Dwg.#101905237

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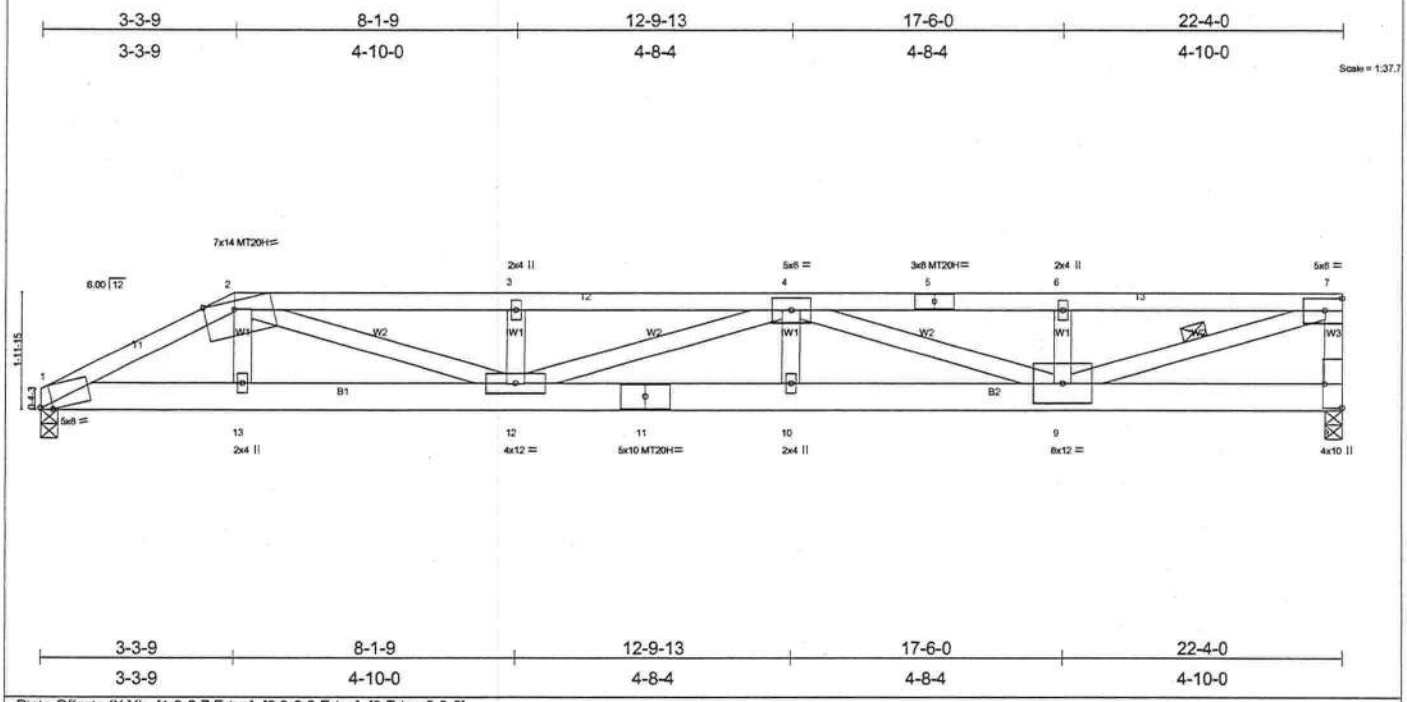

Plate Offsets (X,Y): [1:0-2-7,Edge], [2:0-6-3,Edge], [8:Edge,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	0.55	10-12	>482	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-0.69	10-12	>381	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.84	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 123 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.2 *Except*
W1 2 X 4 SYP No.3, W1 2 X 4 SYP No.3, W1 2 X 4 SYP No.3, W1 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 3-8-12 oc bracing.
WEBS 1 Row at midpt 7-9

REACTIONS (lb/size) 1=1833/0-3-8, 8=1914/0-3-8
Max Horz 1=89(load case 4)
Max Uplift1=-1300(load case 3), 8=-1410(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3893/2782, 2-3=-6012/4412, 3-4=-6011/4412, 4-5=-4311/3171, 5-6=-4311/3171, 6-7=-4311/3171, 7-8=-1661/1278
BOT CHORD 1-13=-2523/3461, 12-13=-2549/3507, 11-12=-4777/6490, 10-11=-4777/6490, 9-10=-4777/6490, 8-9=-271/344
WEBS 2-13=-288/512, 2-12=-1988/2640, 3-12=-504/516, 4-12=-507/399, 4-10=-158/383, 4-9=-2307/1700, 6-9=-480/492, 7-9=-3069/4197

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1300 lb uplift at joint 1 and 1410 lb uplift at joint 8.
5) Girder carries hip end with 0-0-0 right side setback, 3-3-9 left side setback, and 6-6-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 231 lb down and 149 lb up at 3-3-9 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-7=-111(F=-57), 1-13=-30, 8-13=-62(F=-32)
Concentrated Loads (lb)
Vert: 13=-231(F)

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T18	COMMON	2	1	Job Reference (optional)

Dwg.#101905238

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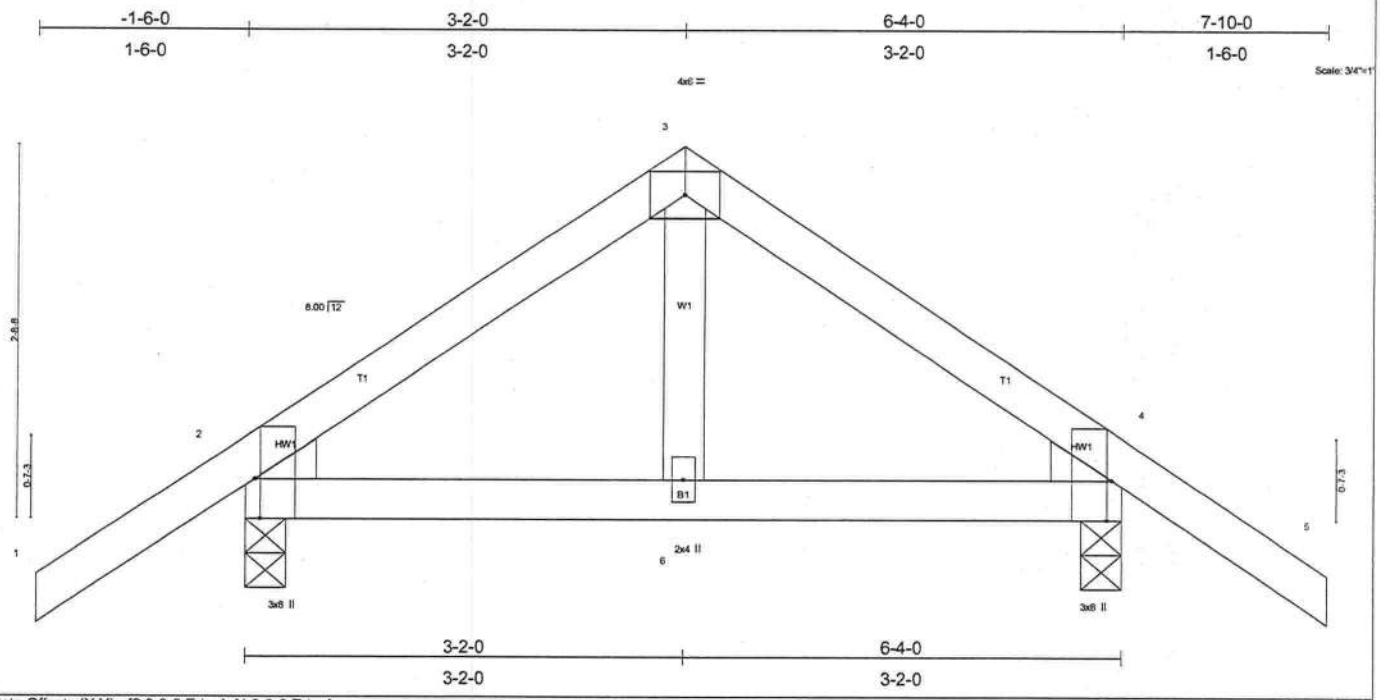

Plate Offsets (X,Y): [2:0-3-8,Edge], [4:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	0.01	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	0.01	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 31 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=343/0-3-8, 4=343/0-3-8
Max Horz 2=-103(load case 3)
Max Uplift2=-317(load case 5), 4=-317(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-247/292, 3-4=-247/292, 4-5=0/36
BOT CHORD 2-6=-86/142, 4-6=-86/142
WEBS 3-6=-253/111

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 317 lb uplift at joint 2 and 317 lb uplift at joint 4.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	T18G	COMMON	1	1	Job Reference (optional)

Dwg.#101905239

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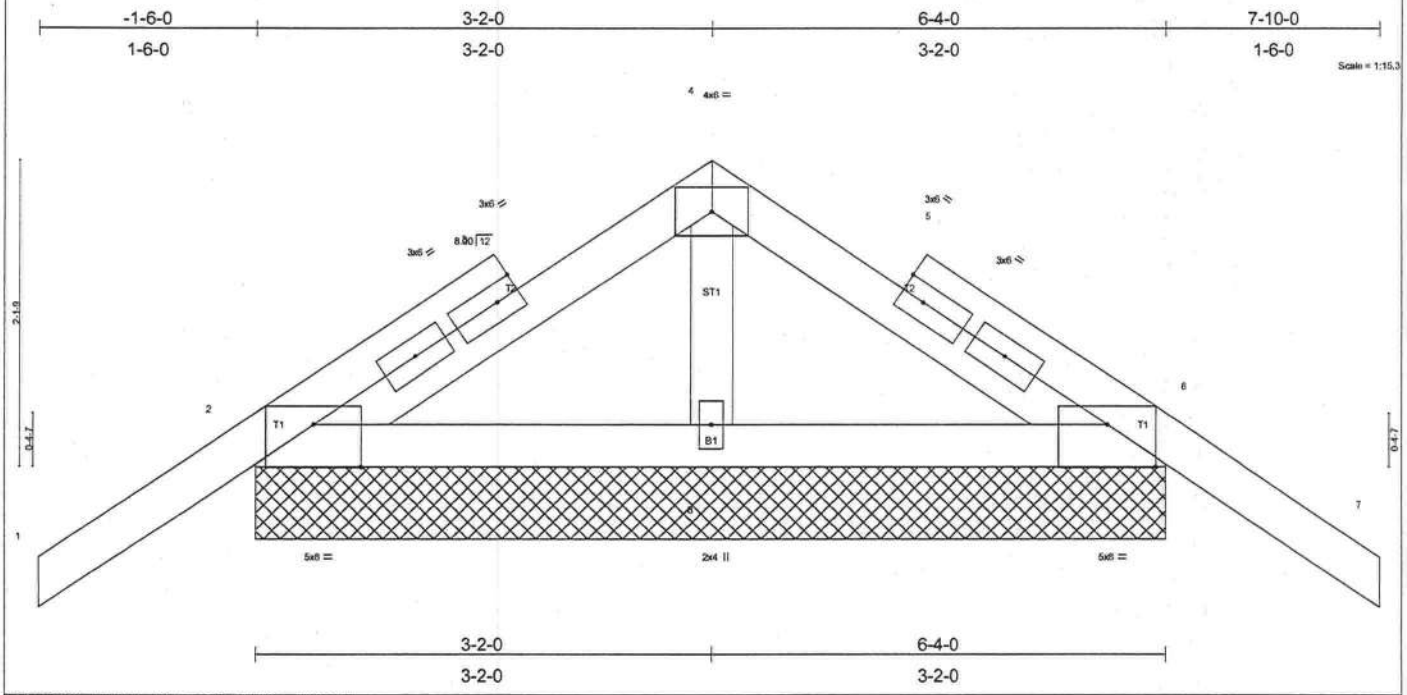

Plate Offsets (X,Y): [2:0-4-0,0-3-7], [6:0-4-0,0-3-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.00	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.00	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=288/6-4-0, 6=288/6-4-0, 8=119/6-4-0
Max Horz 2=-81(load case 3)
Max Uplift2=-237(load case 5), 6=-237(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-181/90, 3-4=-134/103, 4-5=-134/103, 5-6=-181/90, 6-7=0/43
BOT CHORD 2-8=-8/116, 6-8=-8/116

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 2 and 237 lb uplift at joint 6.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	3x6 =	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	V02	VALLEY		1	1	Job Reference (optional)

Dwg.#101905240

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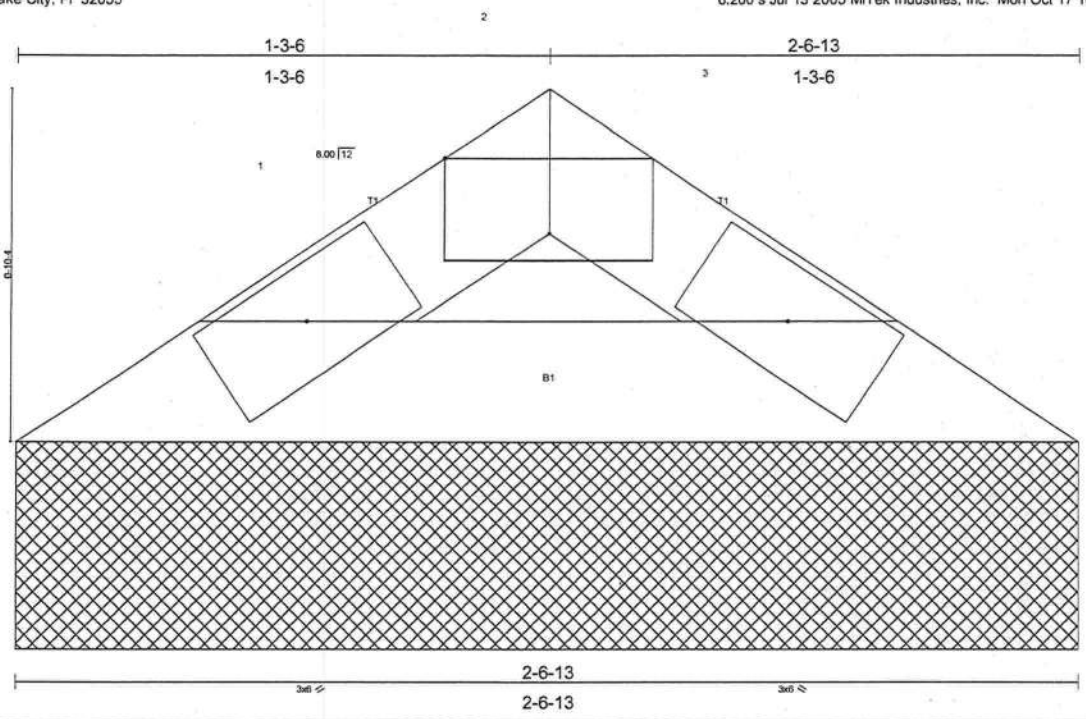

Plate Offsets (X,Y): [2:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.01	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=45/2-6-13, 3=45/2-6-13
Max Horz 1=22(load case 4)
Max Uplift1=-17(load case 5), 3=-17(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-29/21, 2-3=-29/21
BOT CHORD 1-3=-9/15

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed: MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 1 and 17 lb uplift at joint 3.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	V05	VALLEY	1	1	Job Reference (optional)

Dwg.#101905241

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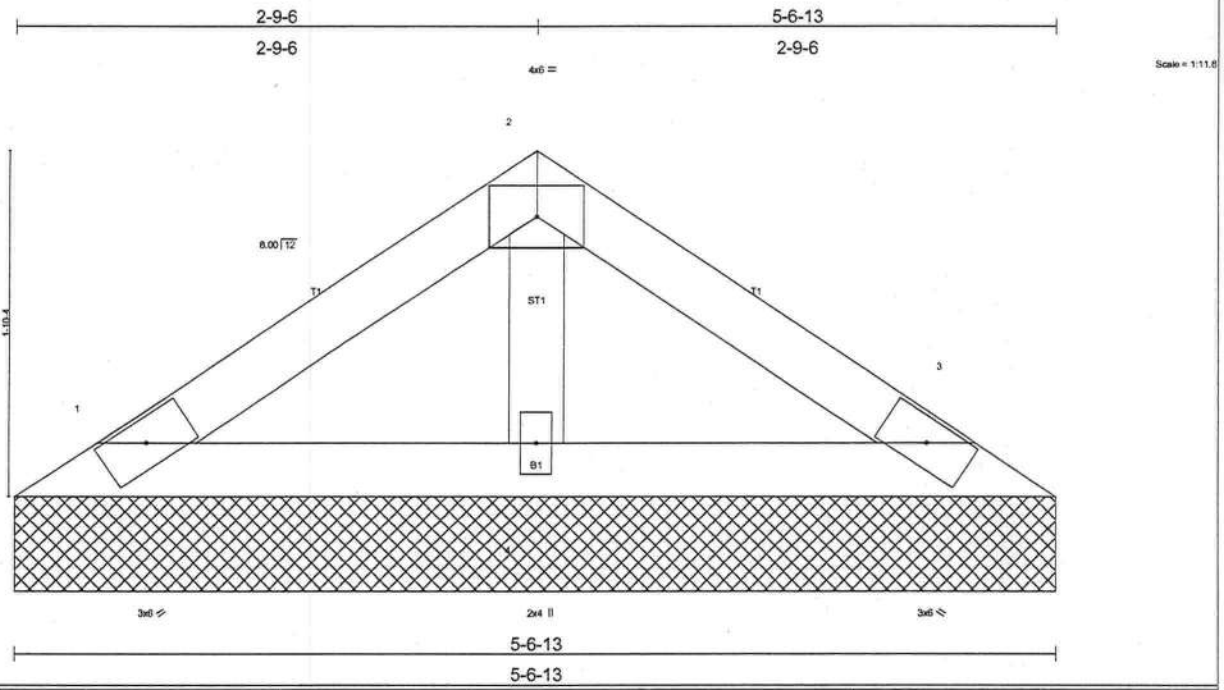


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.08	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-6-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=100/5-6-13, 3=100/5-6-13, 4=186/5-6-13
Max Horz 1=63(load case 4)
Max Uplift1=-57(load case 5), 3=-62(load case 6), 4=-44(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-50/45, 2-3=-50/42
BOT CHORD 1-4=-16/28, 3-4=-16/28
WEBS 2-4=-100/77

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 1, 62 lb uplift at joint 3 and 44 lb uplift at joint 4.

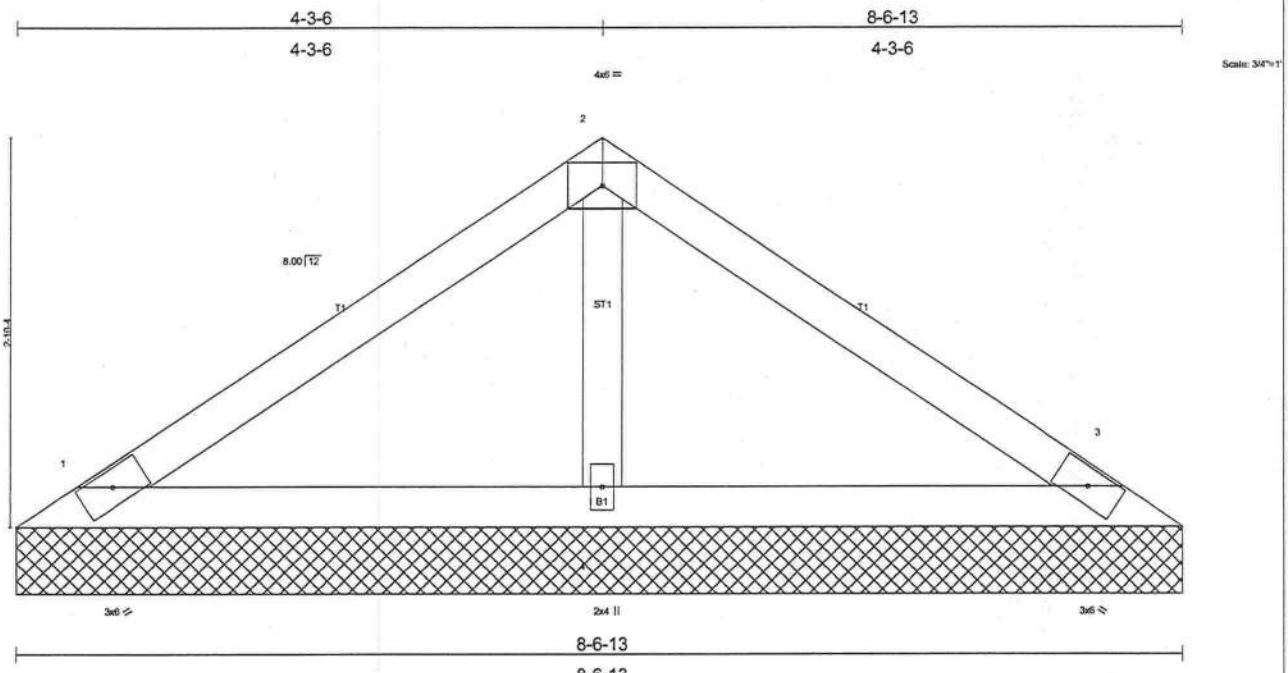
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	V08	VALLEY	1	1	Job Reference (optional)

Dwg.#101905242

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:48:57 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 30 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=147/8-6-13, 3=147/8-6-13, 4=346/8-6-13
Max Horz 1=-105(load case 3)
Max Uplift1=-71(load case 5), 3=-79(load case 6), 4=-121(load case 5)
Max Grav 1=147(load case 9), 3=147(load case 10), 4=346(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-95/75, 2-3=-95/69
BOT CHORD 1-4=-28/46, 3-4=-28/46
WEBS 2-4=-187/139

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 71 lb uplift at joint 1, 79 lb uplift at joint 3 and 121 lb uplift at joint 4.

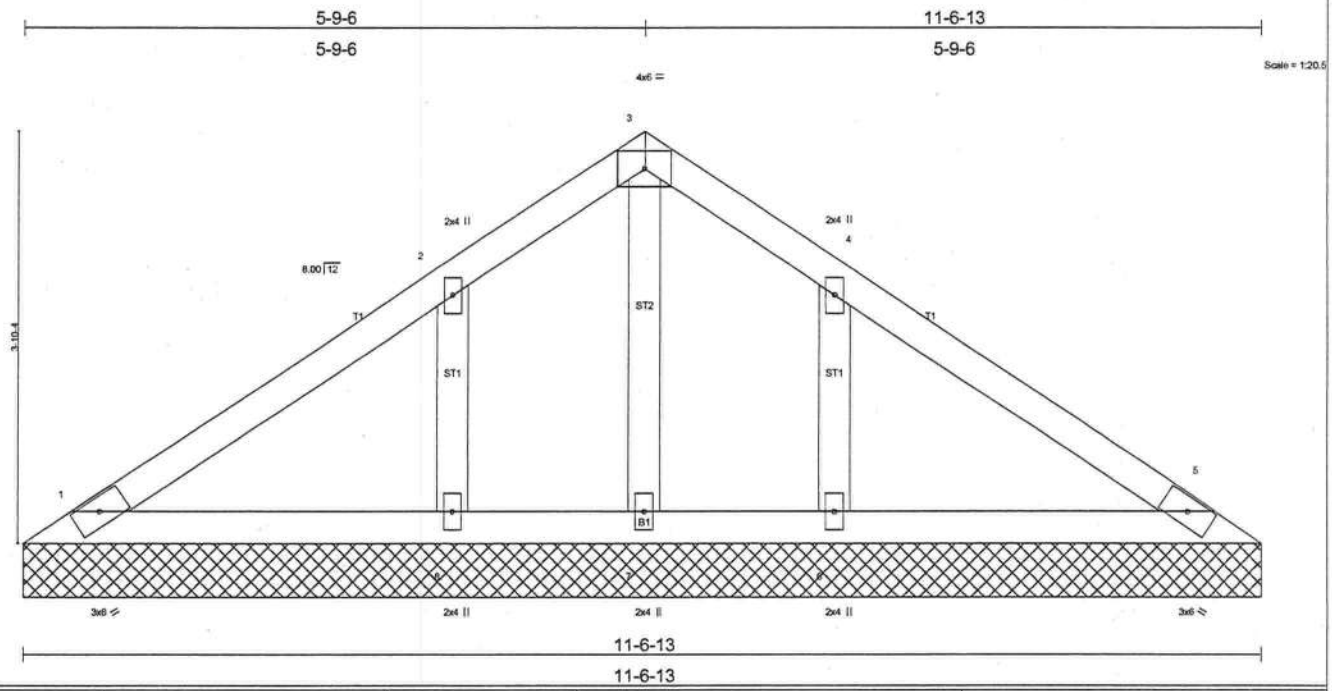
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	V11	VALLEY	1	1	Job Reference (optional)

Dwg.#101905243

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:49:01 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.10	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 47 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=122/11-6-13, 5=122/11-6-13, 7=20/11-6-13, 6=313/11-6-13, 8=313/11-6-13
Max Horz 1=-146(load case 3)
Max Uplift1=-38(load case 6), 5=-34(load case 6), 6=-236(load case 6), 8=-236(load case 5)
Max Grav 1=122(load case 1), 5=122(load case 1), 7=116(load case 6), 6=313(load case 1), 8=313(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-100/88, 2-3=-49/119, 3-4=-49/119, 4-5=-51/56
BOT CHORD 1-8=-27/89, 7-8=-27/89, 6-7=-27/89, 5-6=-27/89
WEBS 3-7=-91/0, 4-6=-187/225, 2-8=-187/225

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 1, 34 lb uplift at joint 5, 236 lb uplift at joint 6 and 236 lb uplift at joint 8.

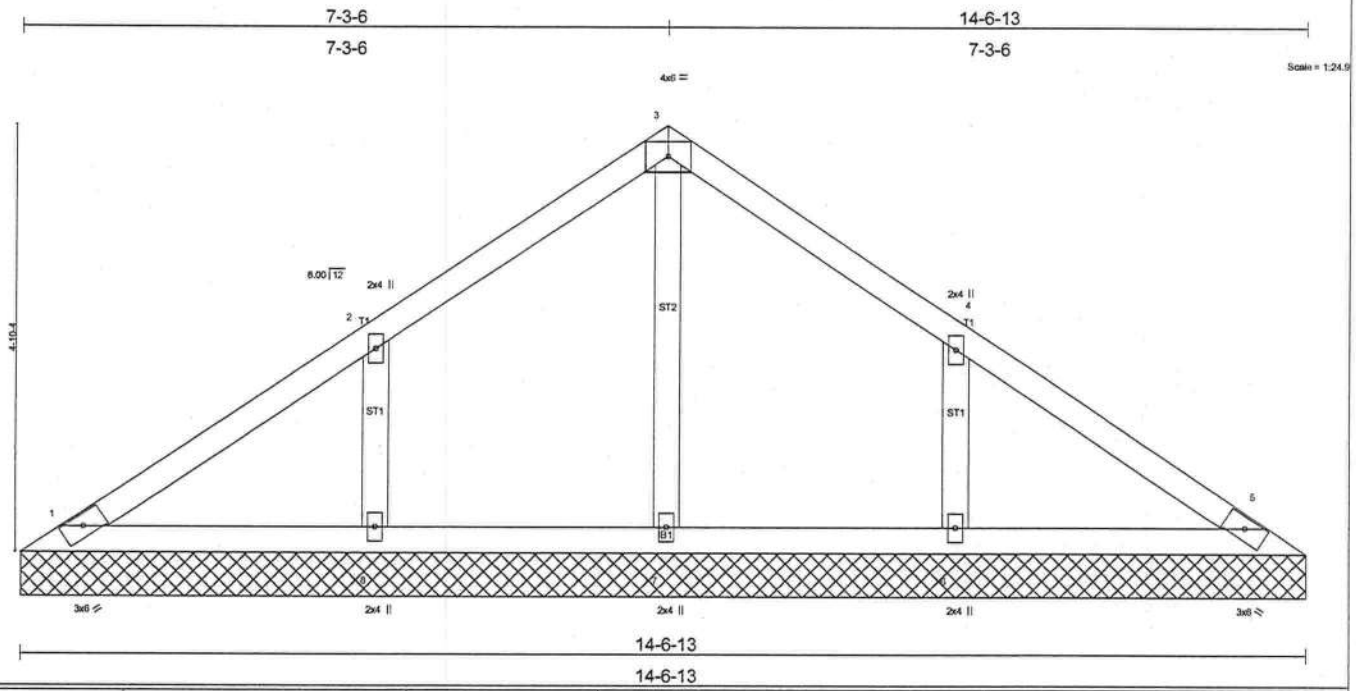
LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	V14	VALLEY	1	1	Job Reference (optional)

Dwg.#101905244

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Oct 17 16:49:05 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.09	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 59 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=134/14-6-13, 5=134/14-6-13, 7=213/14-6-13, 6=331/14-6-13, 8=331/14-6-13
Max Horz 1=-187(load case 3)
Max Uplift1=-45(load case 6), 5=-31(load case 5), 6=-257(load case 6), 8=-258(load case 5)
Max Grav 1=134(load case 1), 5=134(load case 1), 7=213(load case 1), 6=337(load case 10), 8=337(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-137/119, 2-3=-83/143, 3-4=-83/138, 4-5=-76/53
BOT CHORD 1-8=-40/94, 7-8=-40/94, 6-7=-40/94, 5-6=-40/94
WEBS 3-7=-130/21, 4-6=-211/259, 2-8=-211/259

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 1, 31 lb uplift at joint 5, 257 lb uplift at joint 6 and 258 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE- LOT 15 WISE ESTATES
L135121	V17	VALLEY	1	1	Job Reference (optional)

Dwg.#101905245

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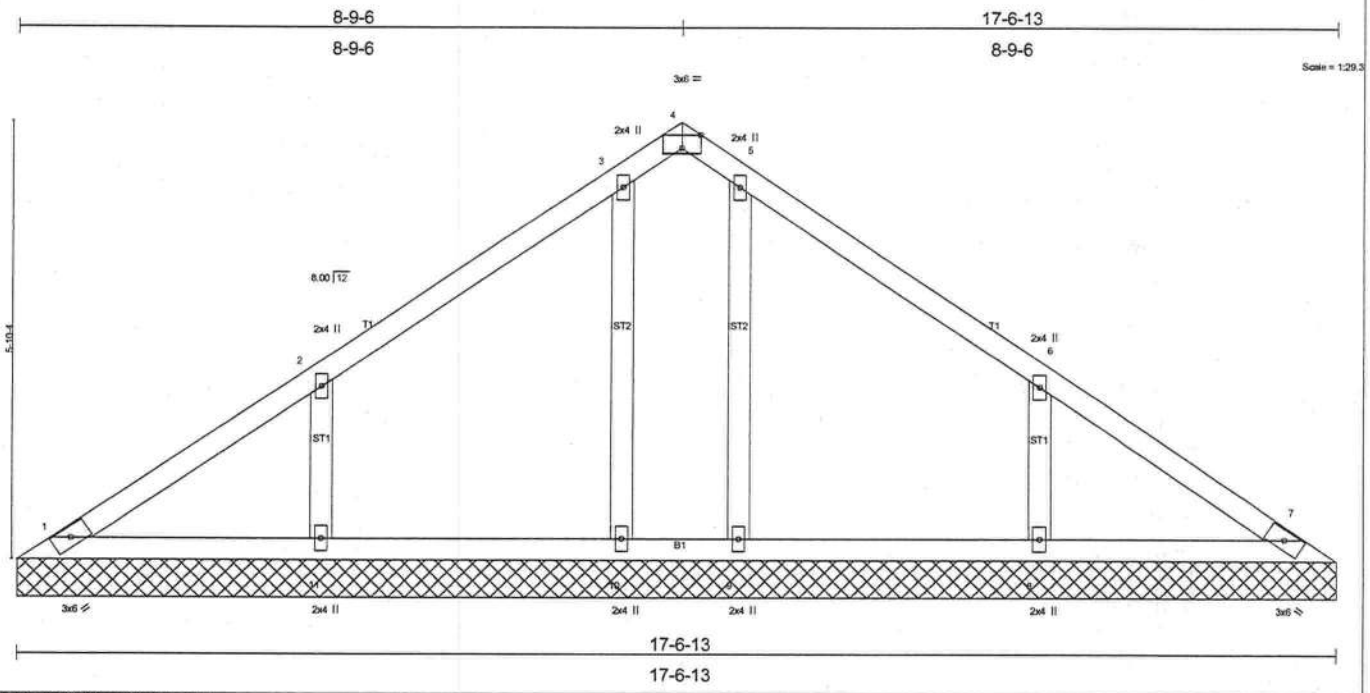


Plate Offsets (X,Y): [4:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 76 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=99/17-6-13, 7=99/17-6-13, 8=367/17-6-13, 9=231/17-6-13, 11=367/17-6-13, 10=231/17-6-13
Max Horz 1=-229(load case 3)
Max Uplift1=-32(load case 3), 8=-288(load case 6), 9=-102(load case 6), 11=-287(load case 5), 10=-120(load case 4)
Max Grav 1=104(load case 9), 7=104(load case 10), 8=367(load case 1), 9=243(load case 10), 11=367(load case 1), 10=243(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-201/112, 2-3=-79/97, 3-4=-24/92, 4-5=-24/98, 5-6=-51/76, 6-7=-173/84
BOT CHORD 1-11=-33/193, 10-11=-33/193, 9-10=-33/193, 8-9=-33/193, 7-8=-33/193
WEBS 6-8=-229/298, 5-9=-167/121, 2-11=-229/297, 3-10=-167/140

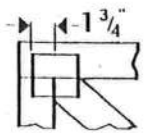
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 32 lb uplift at joint 1, 288 lb uplift at joint 8, 102 lb uplift at joint 9, 287 lb uplift at joint 11 and 120 lb uplift at joint 10.

LOAD CASE(S) Standard

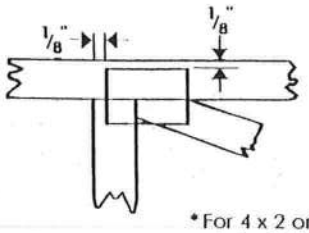
OCTOBER 19,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

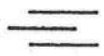
PLATE LOCATION AND ORIENTATION



* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

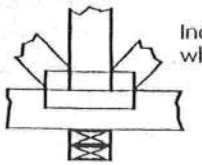
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

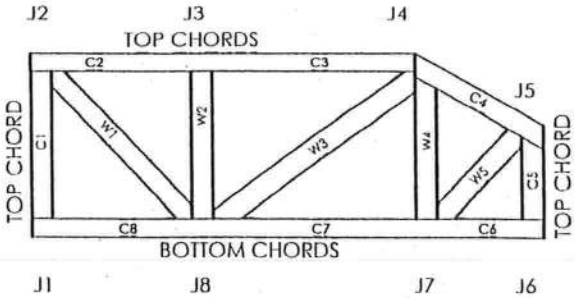
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

Lot 15 WISE

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☑	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☑	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☑	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☑ see NOTE 1	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
☑	☑	**Elevations including:** a) All sides
☑	☑	b) Roof pitch
☑	☑	c) Overhang dimensions and detail with attic ventilation
☑	☑	d) Location, size and height above roof of chimneys
☑	☑	e) Location and size of skylights
☑	☑	f) Building height
☑	☑	e) Number of stories

Floor Plan including:

a) Rooms labeled and dimensioned
b) Shear walls
c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
b) All posts and/or column footing including size and reinforcing
c) Any special support required by soil analysis such as piling
d) Location of any vertical steel

Roof System:

a) Truss package including:
    1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
    2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

b) Conventional Framing Layout including:
    1. Rafter size, species and spacing
    2. Attachment to wall and uplift
    3. Ridge beam sized and valley framing and support details
    4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

a) Masonry wall
    1. All materials making up wall
    2. Block size and mortar type with size and spacing of reinforcement
    3. Lintel, tie-beam sizes and reinforcement
    4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
    5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
    6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
    7. Fire resistant construction (if required)
    8. Fireproofing requirements
    9. Shoe type of termite treatment (termicide or alternative method)
    10. Slab on grade
        a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
        b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
    11. Indicate where pressure treated wood will be placed
    12. Provide insulation R value for the following:
        a. Attic space
        b. Exterior wall cavity
        c. Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
    a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
b) Floor joist size and spacing
c) Girder size and spacing
d) Attachment of joist to girder
e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
b) Ceiling fans
c) Smoke detectors
d) Service panel and sub-panel size and location(s)
e) Meter location with type of service entrance (overhead or underground)
f) Appliances and HVAC equipment
g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

a) Manual J sizing equipment or equivalent computation
b) Exhaust fans in bathroom

Energy Calculations (dimensions shall match plans)

Gas System Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

***Notice Of Commencement Required Before Any Inspections Will Be Done**

Private Potable Water

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued.
   (386) 758-1058 **(Toilet facilities shall be provided for construction workers)**

4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
   A development permit will also be required. Development permit cost is **$50.00**

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (**$25.00**) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (**$50.00**). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

NOTICE:

ADDRESSES BY APPOINTMENT ONLY!

TO OBTAIN A 9-1-1 ADDRESS THE REQUESTER MUST CONTACT THE COLUMBIA COUNTY 9-1-1 ADDRESSING DEPARTMENT AT (386) 752-8787 FOR AN APPOINTMENT TIME AND DATE:

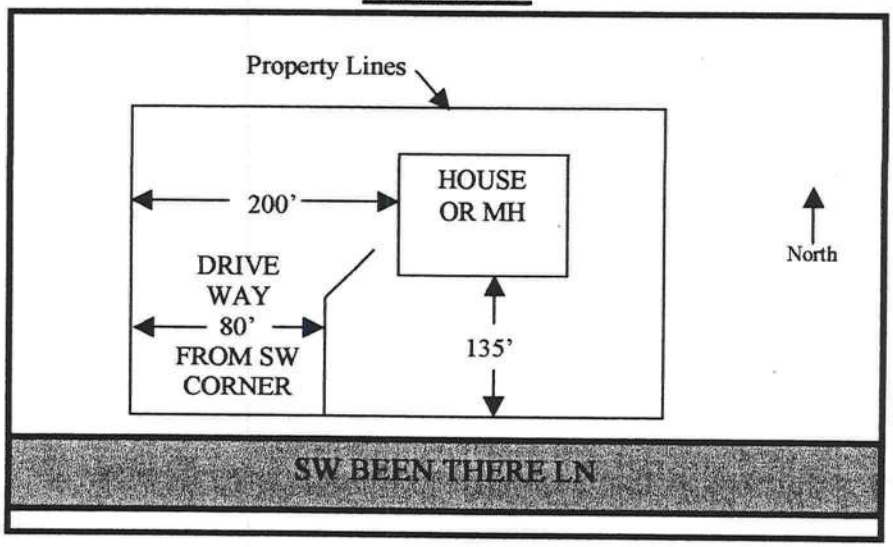
YOU CAN NOT OBTAIN A NEW ADDRESS OVER THE TELEPHONE. MUST MAKE AN APPOINTMENT!

THE ADDRESSING DEPARTMENT IS LOCATED AT 263 NW LAKE CITY AVENUE (OFF OF WEST U.S. HIGHWAY 90 WEST OF INTERSTATE 75 AT THE COLUMBIA COUNTY EMERGENCY OPERATIONS CENTER).

THE REQUESTER WILL NEED THE FOLLOWING:

1. THE PARCEL OR TAX ID NUMBER (SAMPLE: "25-4S-17-12345-123" OR "R12345-123) FOR THE PROPERTY.
2. A PLAT, PLAN, SITE PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
   a. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
   b. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
   c. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



NOTE: 5 TO 7 WORKING DAYS MAY BE REQUIRED IF ADDRESSING DEPARTMENT NEEDS TO CONDUCT AN ON SITE SURVEY.

CERTIFICATE OF

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 24-4S-16-03113-145 **Building permit No.** 000023823

Use Classification SFD,UTILITY **Fire:** 23.68

Permit Holder HUGO ESCALANTE **Waste:** 49.00

Owner of Building KAPTAIN 2, LLC **Total:** 72.68

Location: 398 SW WISE DR. (WISE ESTATES, LOT 15)

Date: 06/19/2006

COLUMBIA COUNTY · GATEWAY TO FLORIDA ·

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Notice of Treatment 11801

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: Bayard Ave
City Lake City Phone 752 1703

Site Location: Subdivision WISE EST
Lot # 15 Block# C Permit # 23823
Address 398 SW Wise Dr

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☑ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☑ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling	2104	613	4

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

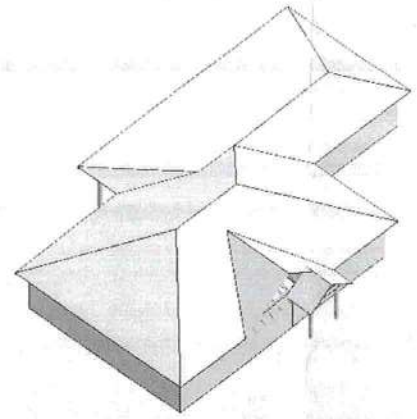
2/2/06 1400 F254
Date Time Print Technician's Name

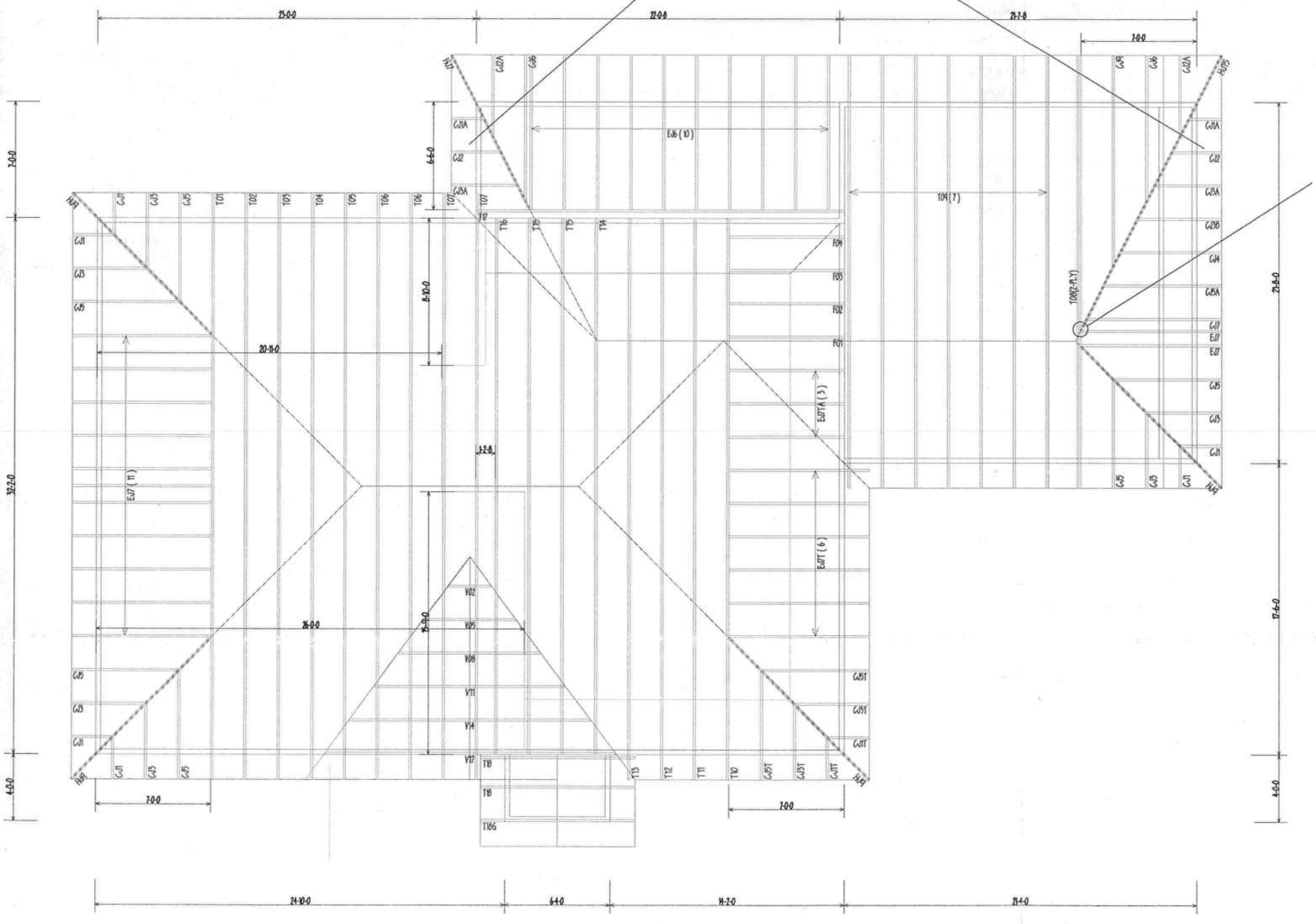
Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink

10/05 ©

CUT 2 CJ2'S
FOR CJ1A





BEARING HEIGHT SCHEDUL

8'-0"

8/12 PITCH
6/12 PITCH
3/12 PITCH
1'-4" OH

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BR/ REFER TO ENGINEERED DRAWINGS FOR PERMAN BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVA

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR C TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOU BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIF CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RE IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved By: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-

Lake City
PHONE: 904-755-6894 FAX: 904-755-

Sanford
PHONE: 407-322-0059 FAX: 407-322-

BUILDER: HUGO ESCALNTE		
LEGAL ADDRESS: LOT 15 WISE ESTATES		
MODEL: THOMAS		REVISION: SCALE: NT
DATE: 10-17-05	DRAWN BY: JRD	JOB #: L135