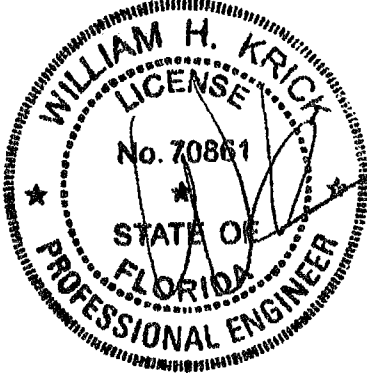




Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineltw.com



This item has been digitally signed by William H. Krick on the date adjacent to the seal.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

COA #0 278

Florida Certificate of Product Approval #FL 1999

04/03/2025

Site Information:	Page 1:
Customer: W B. Howland Company, Inc.	Job Number: 25-2400
Job Description: SPEC HOUSE	
Address: Lake City, FL	

Job Engineering Criteria:	
Design Code: FBC 8th Ed 2023 Res.	IntelliVIEW Version: 23 02 04
	JRef #: 1Y8T2150001
Wind Standard: ASCE 7-22	Wind Speed (mph): 130
Building Type: Closed	Design Loading (psf): 40 00

This package contains general notes pages, 77 truss drawing(s) and 5 detail(s)

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	093.25.0842.37410	A01	2	093.25.0836.13313	A02
3	093.25.0836.19490	A03	4	093.25.0836.23143	A04
5	093.25.0836.27840	A05	6	093.25.0836.31420	A06
7	093.25.0836.44767	A07	8	093.25.0836.48057	A08
9	093.25.0836.51673	A09	10	093.25.0836.53787	A10
11	093.25.0836.59667	A11	12	093.25.0837.01610	A12
13	093.25.0837.03840	A13	14	093.25.0837.05970	A14
15	093.25.0837.08420	A15	16	093.25.0837.12430	A16
17	093.25.0837.15007	A17	18	093.25.0837.21627	A18
19	093.25.0837.26320	A19	20	093.25.0842.42307	A20
21	093.25.0837.43293	A21	22	093.25.0838.43037	A22
23	093.25.0838.44157	B01	24	093.25.0838.45327	B02
25	093.25.0838.46387	B03	26	093.25.0838.47487	B04
27	093.25.0838.56413	B05	28	093.25.0842.59160	B06
29	093.25.0839.05777	C01	30	093.25.0839.14083	C02
31	093.25.0839.18550	C03	32	093.25.0839.31343	C04
33	093.25.0839.39773	C05	34	093.25.0840.03327	C06
35	093.25.0840.06453	C07	36	093.25.0840.08997	C08
37	093.25.0840.11403	C09	38	093.25.0840.14277	C10
39	093.25.0840.19270	C11	40	093.25.0840.21280	C12
41	093.25.0840.25410	C13	42	093.25.0840.30033	C14
43	093.25.0840.32770	C15	44	093.25.0840.34913	D01
45	093.25.0840.41330	D02	46	093.25.0840.42887	D03
47	093.25.0840.44083	D04	48	093.25.0840.45390	D05
49	093.25.0840.47600	D06	50	093.25.0841.02460	J01

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:

The bearing area factor, C_b , is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

General Notes (continued)

Key to Terms (continued).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

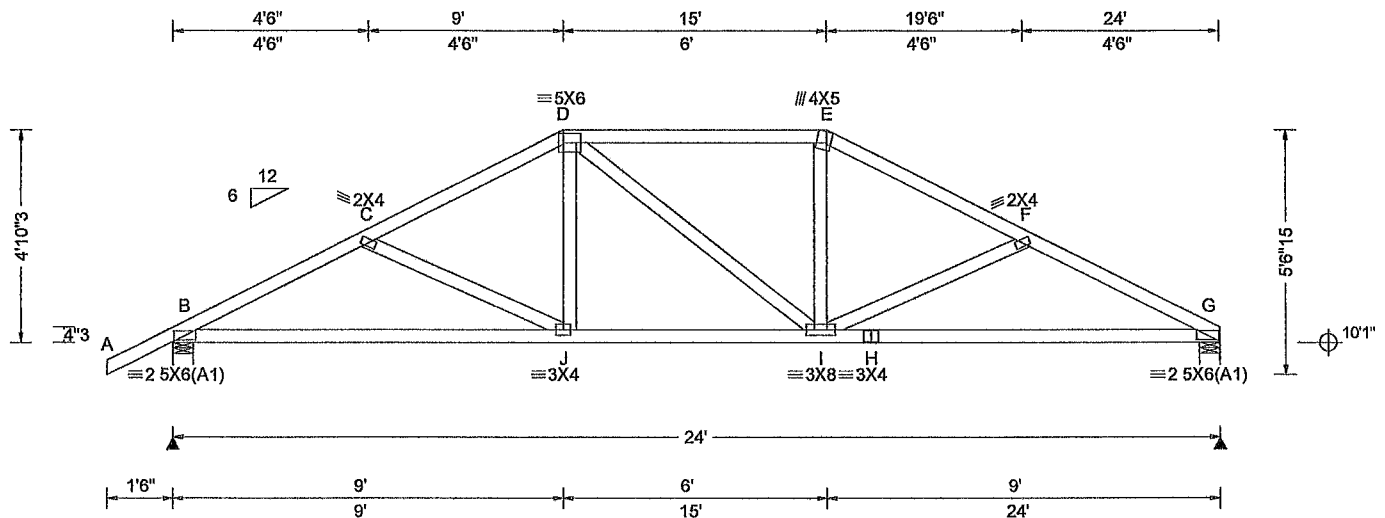
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1

References:

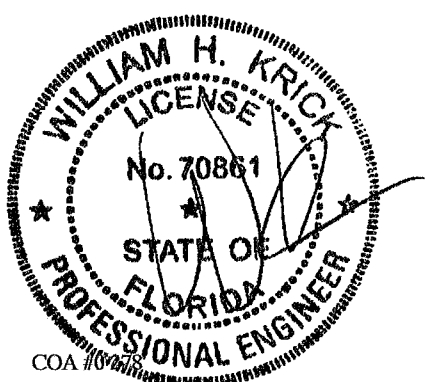
1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com

SEQN: 487745 FROM:	HIPS Qty: 1	Ply: 1 Qty: 1	Job Number: 25-2400 SPEC HOUSE Truss Label: A02	Cust: R215 JRef 1Y8T2150001 T58 DrwNo: 093.25.0836.13313 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL 20 00 TCDL 10 00 BCLL 0 00 BCDL 10 00 Des Ld: 40 00 NCBCLL 10 00 Soffit: 2.00 Load Duration: 1.25 Spacing 24.0 "	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category II EXP B Kzl. NA Mean Height: 15.00 ft TCDL 5 0 psf BCDL 5 0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a. 3.00 ft Loc. from endwall not in 9 00 ft GCPr 0 18 Wind Duration 1.60	Pg NA Ct NA CAT NA Pf NA Ce NA Lu NA Cs NA Snow Duration NA Building Code: FBC 8th Ed. 2023 Res TPI Std 2014 Rep Fac: Yes FT/RT 20(0)/10(0) Plate Type(s) WAVE	PP Deflection in loc L/defl L/# VERT(LL) 0.058 J 999 240 VERT(C/L) 0.119 J 999 180 HORZ(LL) 0.026 G - - HORZ(TL) 0.053 G - - Creep Factor: 2.0 Max TC CSI 0.429 Max BC CSI 0.741 Max Web CSI 0.166 VIEW Ver 23 02 04 0123.13	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1092 /- /- /615 /47 /94 G 985 /- /- /546 /34 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Truss) G Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings B & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens Comp. Chords Tens Comp. B - C 331 - 1709 E - F 311 - 1432 C - D 311 - 1430 F - G 336 - 1732 D - E 312 - 1237

Lumber Top chord 2x4 SP #2, Bot chord 2x4 SP #2, Webs 2x4 SP #3,	Purlins In lieu of structural panels use purlins to brace all flat TC @ 24" oc.	Wind Wind loads based on MWFRS with additional C&C member design Wind loading based on both gable and hip roof types	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens Comp. Chords Tens Comp. B - J 1479 - 265 I - H 1506 - 257 J - I 1227 - 202 H - G 1506 - 257
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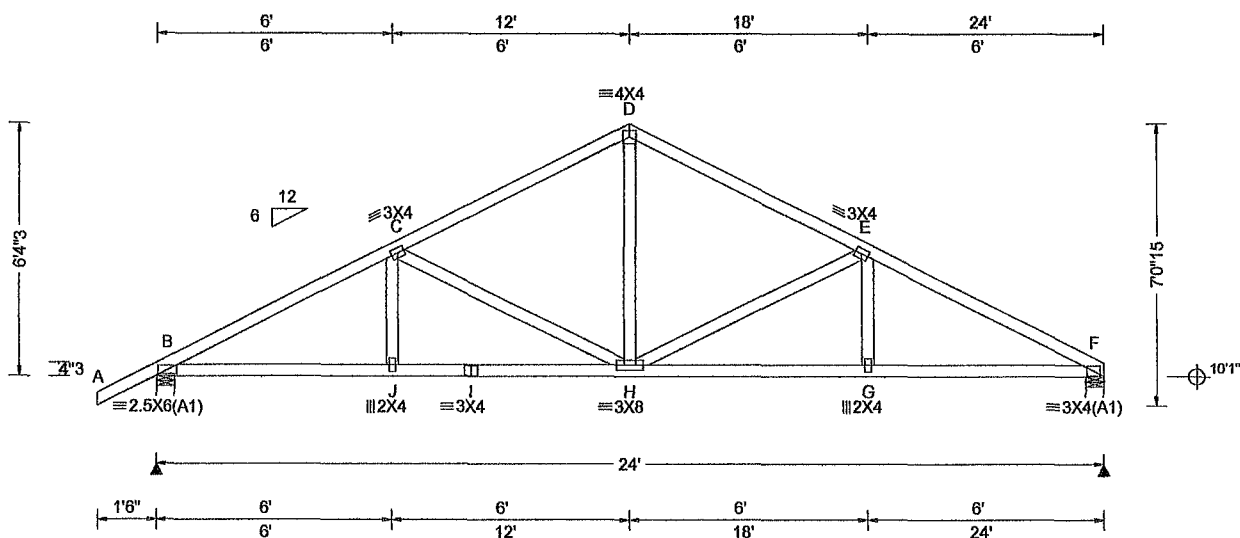
COA #0301235
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****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2
For more information see these web sites Alpine: alpineitw.com, TPI tpinst.org, SBCA sbacompcomponents.com, ICC iccsafe.org, AWC awc.org



155 Harlem Ave
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Glenview, IL 60025

SEQN. 487747 FROM	COMM Ply 1 Qty 3	Job Number: 25-2400 SPEC HOUSE Truss Label: A04	Cust: R 215 JRef: 1Y8T2150001 T20 DrwNo: 093 25.0836.23143 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL 20.00 TCDL 10.00 BCDL 0.00 BCDL 10.00 Des Ld 40.00 NCBCLL 10.00 Soffit 2.00 Load Duration 1.25 Spacing 24.0"	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category II EXP B Kzt: NA Mean Height: 15.00 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from end wall not in 9.00 ft GCpi 0.18 Wind Duration 1.60	Pg NA Ct: NA CAT: NA Pf NA Ce: NA Lu NA Cs: NA Snow Duration NA Building Code: FBC 8th Ed. 2023 Res TPI Std 2014 Rep Fac. Yes FT/RT 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL) 0.061 H 999 240 VERT(CL) 0.124 H 999 180 HORZ(LL) 0.027 F - - HORZ(TL) 0.055 F - - Creep Factor 2.0 Max TC CSI 0.363 Max BC CSI 0.478 Max Web CSI 0.472 VIEW Ver 23.02.04.0123.13	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1092 /- /- /618 /44 /120 F 985 /- /- /549 /30 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Truss) F Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens Comp Chords Tens. Comp. B - C 124 -1704 D - E 146 -1189 C - D 144 -1188 E - F 132 -1724

Lumber

Top chord 2x4 SP #2;
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,

Wind

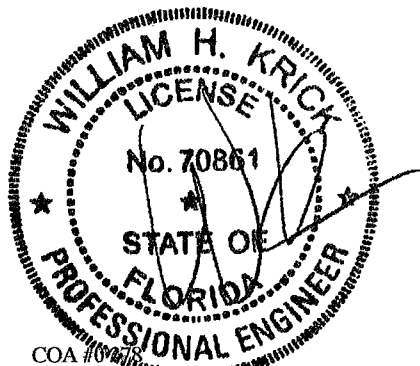
Wind loads based on MWFRS with additional C&C member design
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp.	Chords	Tens.	Comp.
B - J	1457	-75	H - G	1475	-69
J - I	1454	-76	G - F	1478	-68
I - H	1454	-76			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - H	80	-525	H - E	86	-549
D - H	623	-9			



COA #09218

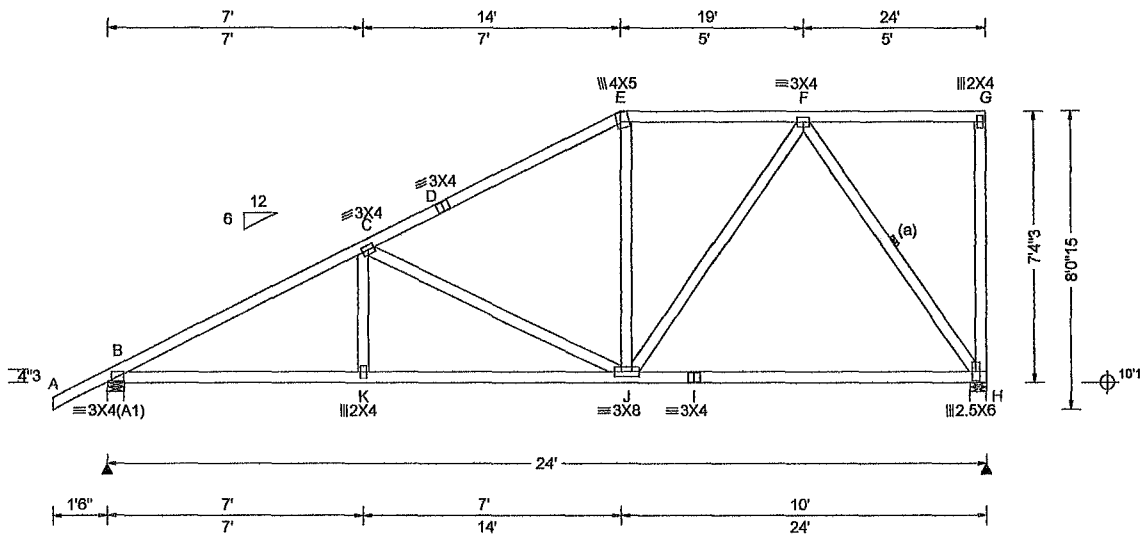
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For more information see these web sites. Alpine: alpineitw.com, TPI tpinst.org; SBCA: sbccomponents.com, ICC iccsafe.org, AWC awc.org

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN. 487749 FROM:	COMN Qty: 1	Ply: 1 Job Number: 25-2400 SPEC HOUSE Truss Label: A06	Cust: R 215 JRef 1Y8T2150001 T40 DrwNo: 093.25.0836.31420 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg Pf In PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL 20.00 TCDL 10.00 BCDL 0 00 BCDL 10 00 Des Ld 40.00 NCBCLL 10 00 Soffit 2 00 Load Duration 1 25 Spacing 24.0 "	Wind Std: ASCE 7-22 Speed 130 mph Enclosure Closed Risk Category II EXP B Kzt: NA Mean Height 15.00 ft TCDL 5.0 psf BCDL 5 0 psf MWFRS Parallel Dist: h to 2h C&C Dist a. 3 00 ft Loc. from endwall not in 9 00 ft GCp: 0.18 Wind Duration 1 60	Pg NA Ct: NA CAT NA Pf NA Ce NA Lu NA Cs NA Snow Duration NA Building Code: FBC 8th Ed 2023 Res TPI Std 2014 Rep Fac: Yes FT/RT 20(0)/10(0) Plate Type(s). WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0 049 K 999 240 VERT(CL) 0 101 K 999 180 HORZ(LL) 0 020 H - - HORZ(TL) 0 040 H - - Creep Factor 2.0 Max TC CSI 0.532 Max BC CSI 0.979 Max Web CSI 0.780 VIEW Ver: 23 02.04.0123 13	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1098 /- /- /655 /- /131 H 979 /- /- /534 /34 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Truss) H Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings B & H are a rigid surface Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 170 -1674 D - E 176 -990 C - D 152 -1049 E - F 189 -847

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,

Bracing

(a) Continuous lateral restraint equally spaced on member

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

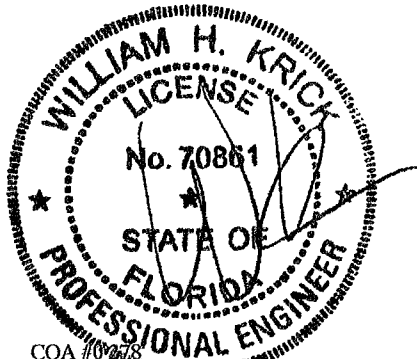
Wind loads based on MWFRS with additional C&C member design
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens Comp	Chords	Tens. Comp.
B - K	1423 -273	J - I	547 -177
K - J	1420 -274	I - H	547 -177

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp	Webs	Tens. Comp
C - J	94 -644	F - H	306 -946
J - F	528 -21		



COA #09478

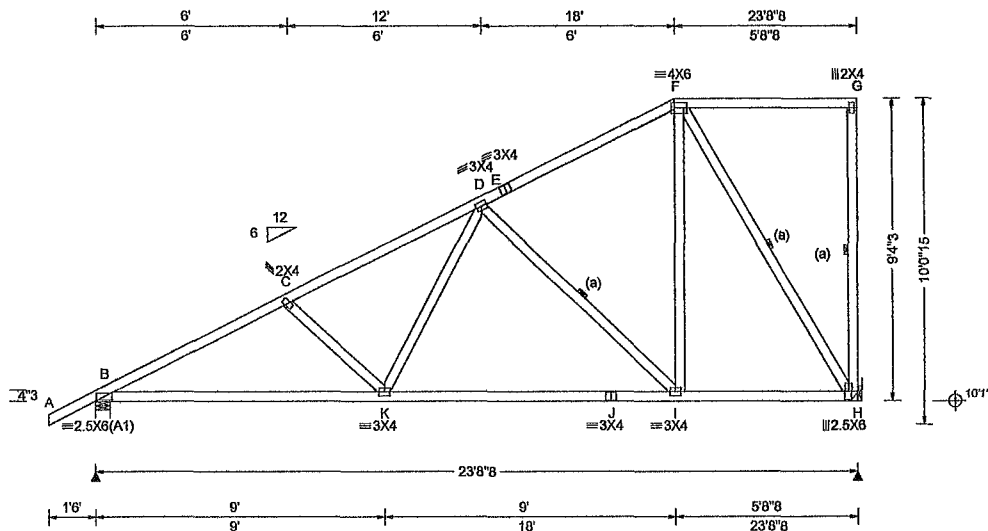
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For more information see these web sites. Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbacompnents.com, ICC iccsafe.org, AWC awc.org

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155 Harlem Ave
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Glenview, IL 60025

SEQN 487751 FROM	COMN Qty 1	Ply 1 Qty 1	Job Number: 25-2400 SPEC HOUSE Truss Label: A08	Cust: R 215 JRef 1Y8T2150001 T1 DrwNo: 093.25.0836.48057 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL 20 00 TCDL 10 00 BCDL 0 00 BCDL 10 00 Des Ld. 40 00 NCBCLL 10 00 Soffit 2 00 Load Duration 1 25 Spacing 24 0 "	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category II EXP B Kzt: NA Mean Height 15.00 ft TCDL 5 0 psf BCDL 5 0 psf MWFRS Parallel Dist. h to 2h C&C Dist at 3 00 ft Loc. from endwall not in 9 00 ft GCPr 0 18 Wind Duration: 1 60	Pg NA Ct NA CAT NA Pf NA Ce NA Lu NA Cs NA Snow Duration NA Building Code: FBC 8th Ed 2023 Res. TPI Std. 2014 Rep Fac: Yes FT/RT.20(0)/10(0) Plate Type(s) WAVE	PP Deflection in loc L/defl L/# VERT(LL) 0 051 K 999 240 VERT(CL) 0 104 K 999 180 HORZ(LL) 0 019 H - - HORZ(TL) 0 038 H - - Creep Factor 2.0 Max TC CSI 0.623 Max BC CSI 0.803 Max Web CSI 0 504 VIEW Ver: 23.02.04 0123 13	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1086 /- /- /656 /- /166 H 966 /- /- /563 /24 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Truss) H Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp B - C 59 -1660 D - E 44 -632 C - D 56 -1398 E - F 70 -609

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2;
Webs. 2x4 SP #3,

Bracing

(a) Continuous lateral restraint equally spaced on member

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

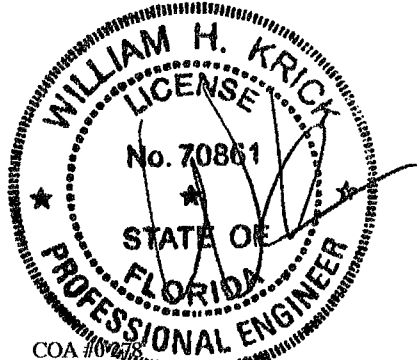
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens Comp.	Chords	Tens. Comp.
B - K	1421 -229	J - I	975 -170
K - J	975 -170	I - H	474 -87

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp	Webs	Tens. Comp
K - D	529 0	F - I	662 -2
D - I	118 -705	F - H	162 -888

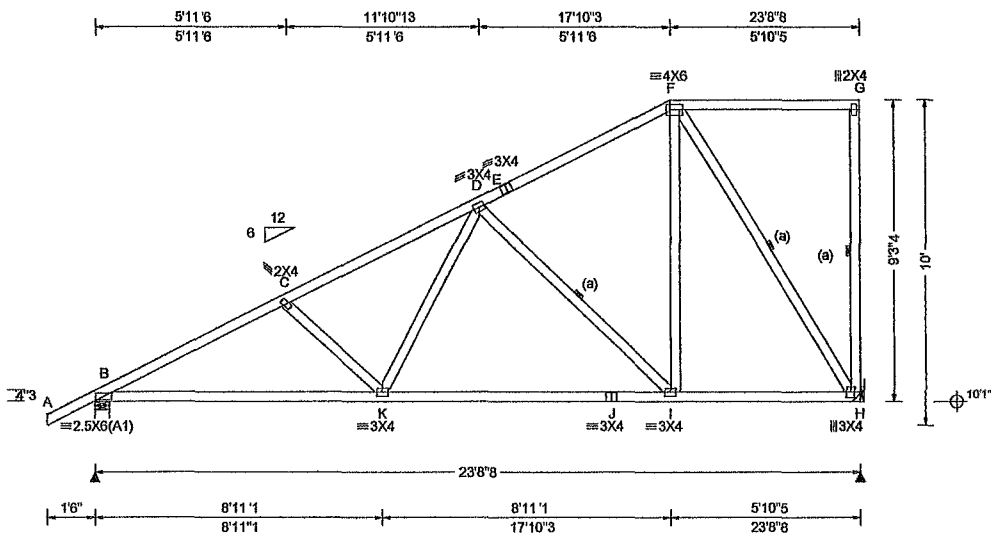


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ALPINE
 A DIVISION OF ITW
 155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 487753 FROM:	COMM Qty: 1	Ply: 1 Job Number: 25-2400 SPEC HOUSE Truss Label: A10	Cust: R 215 JRef 1Y8T2150001 T47 DrwNo: 093.25.0836.53787 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL 20 00 TCDL 10 00 BCDL 0 00 BCDL 10 00 Des Ld: 40.00 NCBCLL 10 00 Soffit: 2 00 Load Duration 1.25 Spacing 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.56 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall not in 9 00 ft GCPr 0 18 Wind Duration: 1 60	Pg NA Ct NA CAT: NA Pf NA Ce NA Lu NA Cs NA Snow Duration: NA Building Code: FBC 8th Ed 2023 Res. TPI Std 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL) 0 051 K 999 240 VERT(CL) 0 104 K 999 180 HORZ(LL) 0 019 H - - HORZ(TL) 0 038 H - - Creep Factor: 2.0 Max TC CSI 0 661 Max BC CSI 0 790 Max Web CSI 0 506 VIEW Ver 23.02 04 0123.13	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1086 -/- /667 - /166 H 966 -/- /654 - /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1 5 (Truss) H Brg Wid = - Min Req = - Bearing B is a rigid surface Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 -1662 D - E 0 -645 C - D 0 -1403 E - F 0 -619

Lumber

Top chord 2x4 SP #2;
Bot chord 2x4 SP #2;
Webs. 2x4 SP #3,

Bracing

(a) Continuous lateral restraint equally spaced on member

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

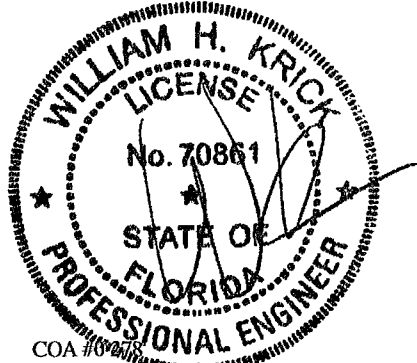
In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #09278

Florida Certificate of Product Approval #FL 1999

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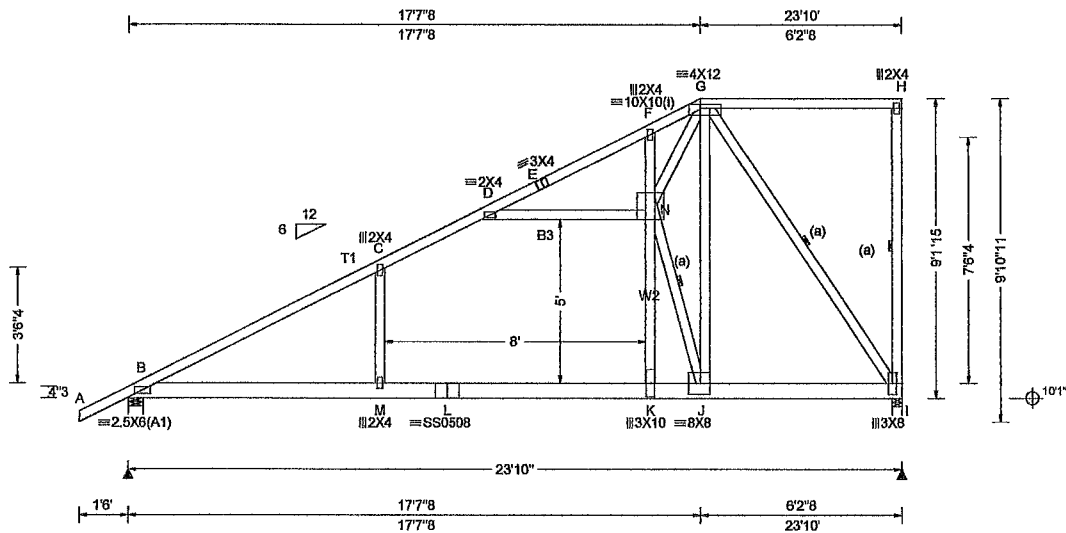
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 487757 FROM:	ATIC	Ply 1 Qty 1	Job Number: 25-2400 SPEC HOUSE Truss Label: A14	Cust: R 215 JRef: 1Y8T2150001 T60 DrwNo: 093 25.0837.05970 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL 20 00 TCDL 10 00 BCLL 0 00 BCDL 10 00 Des Ld 40 00 NCBCLL 10 00 Soffit 2.00 Load Duration 1 25 Spacing 24 0 "	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP B Kzt. NA Mean Height 15.00 ft TCDL 5 0 psf BCDL 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall not in 9.00 ft GCpi 0 18 Wind Duration: 1.60	Pg NA Ct NA CAT NA Pf NA Ce NA Lu NA Cs NA Snow Duration NA Building Code FBC 8th Ed 2023 Res. TPI Std 2014 Rep Fac: Yes FT/RT 20(0)/10(0) Plate Type(s): WAVE, 18SS	PP Deflection in loc L/def L/# VERT(LL) 0.560 M 507 240 VERT(CL) 1 173 M 242 180 HORZ(LL) 0.270 C - - HORZ(TL) 0.568 C - - Creep Factor 2 0 Max TC CSI 0.768 Max BC CSI 0.903 Max Web CSI 0.943 VIEW Ver 23.02.04.0123 13	Gravity Loc R+ /R- /Rh /Rw /U /RL B 1541 - /- /659 - /163 I 1423 - /- /561 /25 - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Truss) I Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens Comp. B - C 28 -2131 C - D 87 -1912

Lumber
Top chord 2x4 SP #2, T1 2x4 SP M-31,
Bot chord 2x6 SP 2400f-2.0E, B3 2x4 SP #2;
Webs 2x4 SP #3, W2 2x4 SP #2,

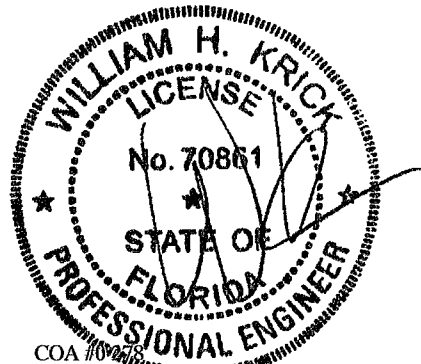
Bracing
(a) Continuous lateral restraint equally spaced on member

Plating Notes
(I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

Loading
BC attic loading. LL = 25 00 psf; DL = 10.00 psf; from 7-11-0 to 15-11-0

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling

Wind
Wind loads based on MWFRS with additional C&C member design
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

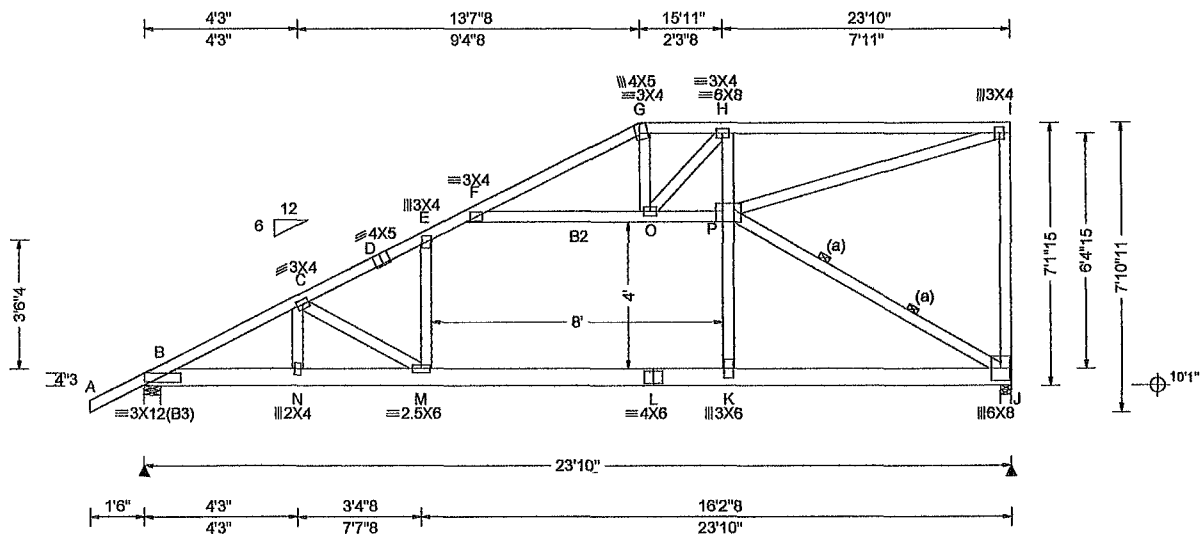


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 487823 FROM	ATIC Qty 1	Ply 1 Qty 1	Job Number: 25-2400 SPEC HOUSE Truss Label: A16	Cust: R 215 JRef 1Y8T2150001 T73 Dwno: 093.25.0837.12430 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL 20 00	Wind Std ASCE 7-22	Pg NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL 10 00	Speed. 130 mph	Pf NA Ce: NA	VERT(LL) 0.207 M 999 240	Loc R+ /R- /Rh /Rw /U /RL
BCLL 0 00	Enclosure: Closed	Lu NA Cs: NA	VERT(CL) 0.434 M 655 180	B 1910 -/- /- /B13 -/- /160
BCDL 10 00	Risk Category: II	Snow Duration NA	HORZ(LL) 0.086 E - -	J 1757 -/- /- /660 /42 -/-
Des Ld 40 00	EXP B Kzt: NA		HORZ(TL) 0.182 E - -	Wind reactions based on MWFRS
NCBCLL 10 00	Mean Height. 15.00 ft	Building Code:	Creep Factor 2.0	B Brg Wid = 5.5 Min Req = 1.6 (Truss)
Soffit 2 00	TCDL 5.0 psf	FBC 8th Ed 2023 Res.	Max TC CSI 0.645	J Brg Wid = 3.5 Min Req = 1.5 (Truss)
Load Duration: 1 25	BCDL 5.0 psf	TPI Std. 2014	Max BC CSI 0.548	Bearings B & J are a rigid surface.
Spacing 30 0"	MWFRS Parallel Dist: h to 2h	Rep Fac: No	Max Web CSI 0.969	Members not listed have forces less than 375#
	C&C Dist a. 3.00 ft	FT/RT.20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall not in 11 00 ft	Plate Type(s)	VIEW Ver: 23.02 04.0123.13	Chords Tens.Comp. Chords Tens Comp.
	GCpi 0.18	WAVE		B - C 169 -3426 E - F 214 -2521
	Wind Duration: 1 60			C - D 175 -3029 F - G 91 -512
				D - E 182 -2953

Lumber

Top chord 2x4 SP M-31,
Bot chord 2x6 SP 2400F-2.0E; B2 2x4 SP #2;
Webs 2x4 SP #3;

Bracing

(a) Continuous lateral restraint, equally spaced on member

Loading

BC attic loading LL = 25 00 psf; DL = 10 00 psf; from 7-11-0 to 15-11-0

Purlins

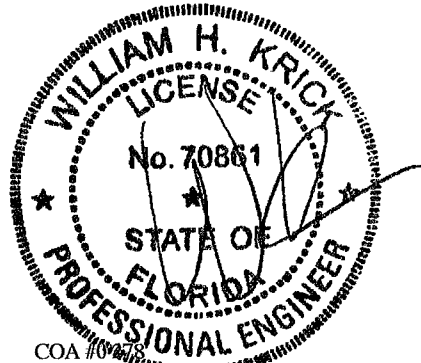
In lieu of structural panels use purlins to brace TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design
Right end vertical not exposed to wind pressure
Wind loading based on both gable and hip roof types

Additional Notes

The overall height of this truss excluding overhang is 7'-1-15



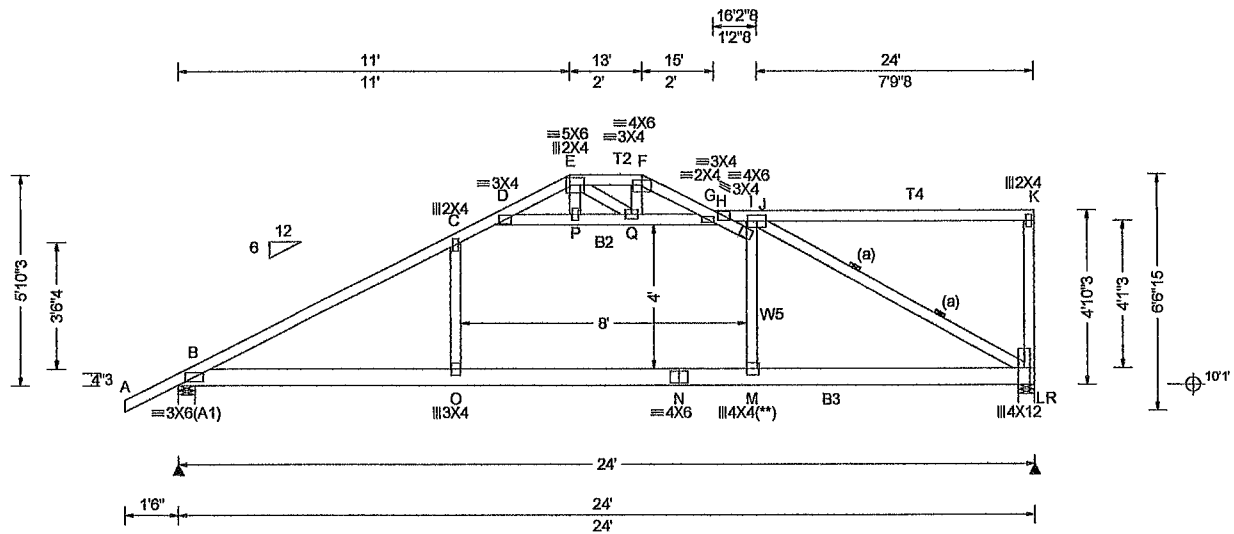
COA #09278

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ALPINE
ANTW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 487828 FROM:	ATC	Ply 1 Qty: 1	Job Number: 25-2400 SPEC HOUSE Truss Label: A18	Cust: R 215 JRef: 1Y8T2150001 T103 Dwno: 093.25.0837.21627 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL. 20 00 TCDL. 10 00 BCDL. 0 00 BCDL. 10.00 Des Ld. 40 00 NCBCLL. 10 00 Soffit. 2.00 Load Duration. 1.25 Spacing: 24.0 "	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP B Kzt: NA Mean Height: 15.00 ft TCDL 5 0 psf BCDL 5 0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9 00 ft GCp1 0 18 Wind Duration: 1.60	Pg NA Ct: NA CAT NA Pf NA Ce NA Lr NA Cs NA Snow Duration NA Building Code FBC 8th Ed 2023 Res. TPI Std 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s). WAVE	PP Deflection in loc L/def L/# VERT(LL) 0 230 O 999 240 VERT(CL) 0 578 O 495 180 HORZ(LL) 0 104 C - - HORZ(TL) 0.266 C - - Creep Factor: 2.0 Max TC CSI 0.939 Max BC CSI 0.826 Max Web CSI 0.810 VIEW Ver: 23.02.04.0123.13	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1531 -/- /636 /35 /122 R 1398 -/- /508 /47 -/- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.8 (Truss) R Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings B & R are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp Chords Tens Comp B - C 264 -2502 H - I 165 -382 C - D 292 -2098 H - J 414 -2079 G - H 292 -1982

Lumber
Top chord. 2x4 SP M-31, T2, T4 2x4 SP #2,
Bot chord 2x6 SP #2, B2 2x4 SP #2,
B3 2x6 SP 2400F-2.0E,
Webs 2x4 SP #3, W5 2x4 SP #2,

Bracing
(a) Continuous lateral restraint equally spaced on member

Plating Notes
(**) 1 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements.

Loading
BC attic loading LL = 25 00 psf; DL = 10.00 psf; from 7-11-0 to 15-11-0

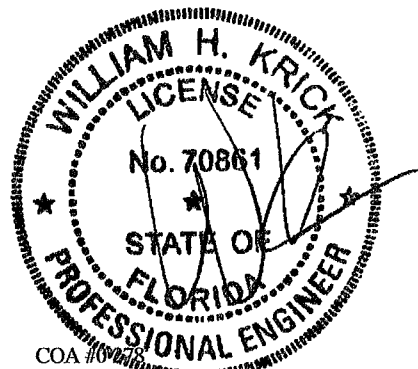
Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling

Wind
Wind loads based on MWFRS with additional C&C member design
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 5'-10-3
Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point)

Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp. Chords Tens Comp.
B - O 2120 -293 N - M 2129 -292
O - N 2129 -292 M - L 2104 -312

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp Webs Tens. Comp.
C - O 584 0 I - M 777 0
D - P 229 -2107 I - J 968 0
P - Q 226 -2072 J - L 359 -2426
Q - G 231 -1948



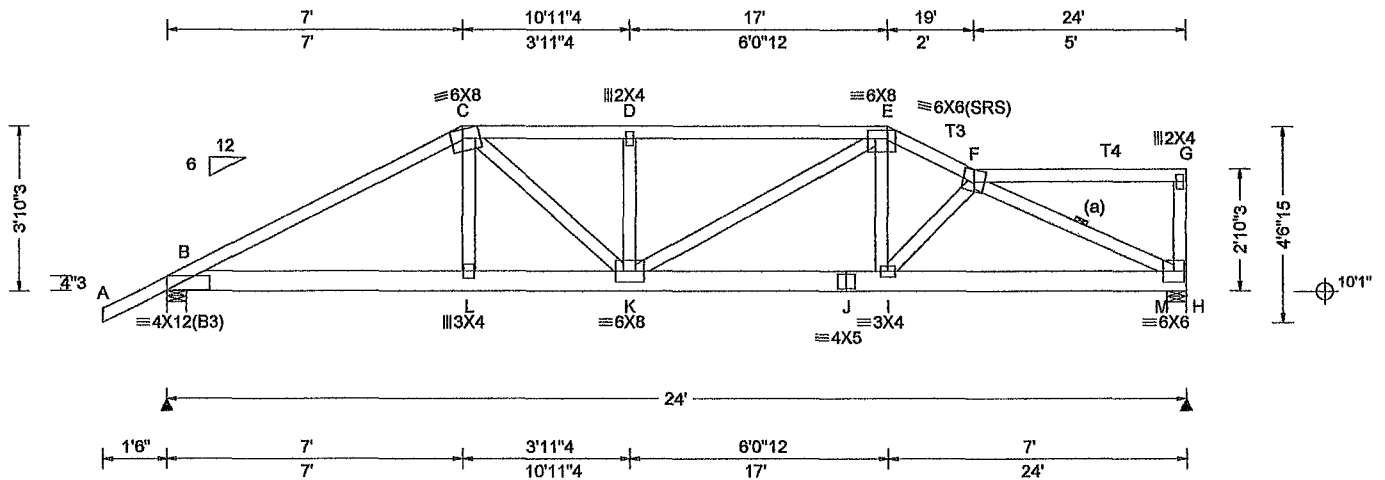
COA #09078

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 487817 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 25-2400 SPEC HOUSE Truss Label: A20	Cust: R 215 JRef: 1Y8T2150001 T13 DrwNo: 093.25.0842.42307 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL 20 00 TCDL 10.00 BCLL 0 00 BCDL 10 00 Des Ld. 40 00 NCBCLL 10 00 Soffit: 2 00 Load Duration 1.25 Spacing 24 0"	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP B Kzi: NA Mean Height: 15.00 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist. 0 to h/2 C&C Dist a 3.00 ft GCpi 0.18 Wind Duration 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration NA Building Code: FBC 8th Ed 2023 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.162 D 999 240 VERT(CL): 0.326 D 877 180 HORZ(LL) 0.045 H - - HORZ(TL) 0.091 H - - Creep Factor 2.0 Max TC CSI 0.494 Max BC CSI 0.867 Max Web CSI 0.833 VIEW Ver: 23 02.04.0123 13	Gravity Loc R+ / R- / Rh / Rw / U / RL B 2497 /- /- /- /131 /- M 1833 /- /- /- /79 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 2.9 (Truss) M Brg Wid = 5.5 Min Req = 2.2 (Truss) Bearings B & M are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens Comp. Chords Tens Comp B - C 240 -4756 D - E 234 -4894 C - D 235 -4896 E - F 149 -3542

Lumber

Top chord 2x4 SP M-31, T3, T4 2x4 SP #2,
Bot chord 2x6 SP #2,
Webs 2x4 SP #3,

Bracing

(a) Continuous lateral restraint equally spaced on member

Special Loads

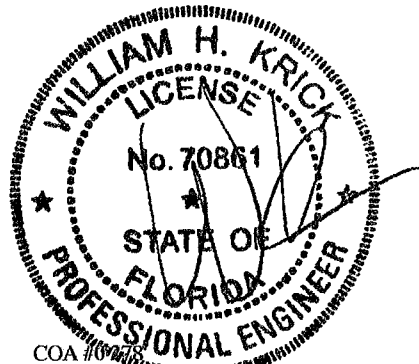
----- (Lumber Dur Fac.=1.25 / Plate Dur Fac.=1.25)
TC From 62 plf at -1.50 to 62 plf at 7.00
TC From 31 plf at 7.00 to 31 plf at 10.94
TC From 62 plf at 10.94 to 62 plf at 24.00
BC From 4 plf at -1.50 to 4 plf at 0.00
BC From 20 plf at 0.00 to 20 plf at 7.03
BC From 10 plf at 7.03 to 10 plf at 10.94
BC From 20 plf at 10.94 to 20 plf at 24.00
TC 542 lb Conc. Load at 7.03
TC 187 lb Conc. Load at 9.06
BC 394 lb Conc. Load at 7.03
BC 129 lb Conc. Load at 9.06
BC 1161 lb Conc. Load at 10.94

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS
Right end vertical not exposed to wind pressure
Wind loading based on both gable and hip roof types.

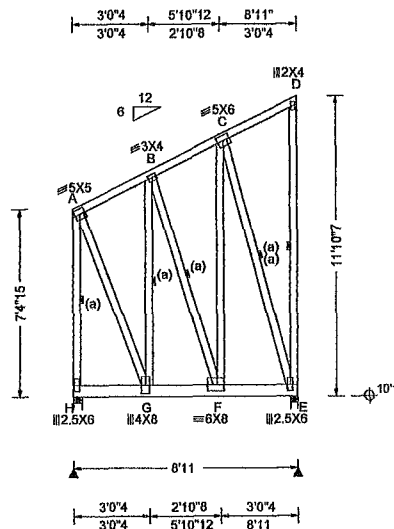


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ALPINE
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN- 487763 FROM:	MONO Qty 1	Ply 1 Qty 1	Job Number: 25-2400 SPEC HOUSE Truss Label: A22	Cust: R 215 JRef: 1Y8T2150001 T64 DrwNo: 093 25.0838.43037 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL 20.00 TCDL 10 00 BCLL 0 00 BCDL 10.00 Des Ld 40 00 NCBCLL 10 00 Soffit 2 00 Load Duration 1 25 Spacing 24.0 "	Wind Std ASCE 7-22 Speed 130 mph Enclosure. Closed Risk Category II EXP B Kzt. NA Mean Height. 19.72 ft TCDL 5 0 psf BCDL 5 0 psf MWFRS Parallel Dist. 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall not in 9 00 ft GCpi. 0 18 Wind Duration 1 60	Pg NA Ct: NA CAT: NA Pf NA Ce. NA Lu: NA Cs: NA Snow Duration NA Building Code: FBC 8th Ed 2023 Res. TPI Std 2014 Rep Fac. Varies by Ld Case FT/RT-20(0)/10(0) Plate Type(s). WAVE	PP Deflection in loc L/defl L/# VERT(LL) 0.029 F 999 240 VERT(CL) 0.059 F 999 180 HORZ(LL) 0.017 B - - HORZ(TL) 0.033 B - - Creep Factor 2.0 Max TC CSI 0 113 Max BC CSI 0 746 Max Web CSI 0 940 VIEW Ver: 23 02 04 0123.13	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL H 1909 /- /- /- /43 /- E 2324 /- /- /82 /- /- Wind reactions based on MWFRS H Brg Wid = 4.0 Min Req = 2.3 (Truss) E Brg Wid = 3.5 Min Req = 2.7 (Truss) Bearings H & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens Comp. Chords Tens Comp A - B 17 -645 B - C 7 -596

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #3,

Bracing

(a) Continuous lateral restraint equally spaced on member

Special Loads

—(Lumber Dur Fac.=1.25 / Plate Dur Fac.=1.25)
TC From 31 plf at 0 00 to 31 plf at 8 92
BC From 10 plf at 0 00 to 10 plf at 8 92
BC. 966 lb Conc. Load at 1 94, 3.94, 5 94, 7 94

Wind

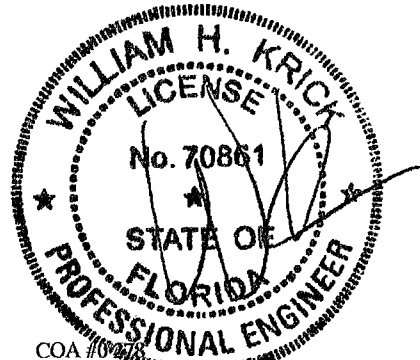
Wind loads and reactions based on MWFRS
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens	Comp.	Chords	Tens	Comp.
G - F	555	-11	F - E	480	-2

Maximum Web Forces Per Ply (lbs)

Webs	Tens	Comp.	Webs	Tens	Comp.
A - H	44	-1687	F - C	1779	0
A - G	1468	-29	C - E	5	-1703



COA #0000000000

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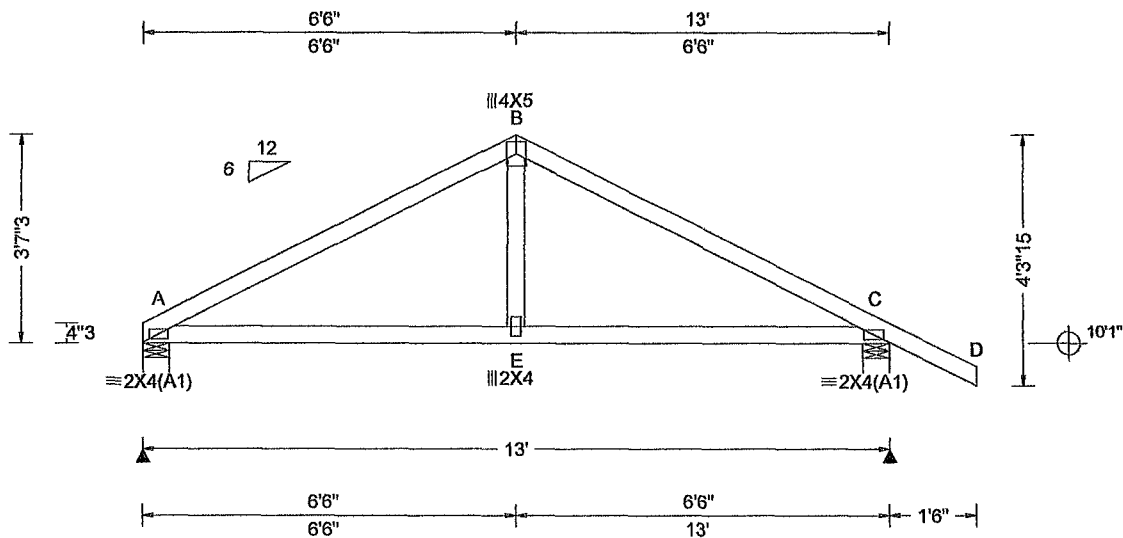
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For more information see these web sites. Alpine: alpineitw.com; TPI: tpiinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org



SEQN. 487765 FROM	COMN	Ply 1 Qty 4	Job Number: 25-2400 SPEC HOUSE Truss Label: B02	Cust: R 215 JRef: 1Y8T2150001 T67 DrwNo: 093.25.0838.45327 AK / WHK 04/03/2025
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Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg Pf in PSF)			Defl/CSI Criteria		▲ Maximum Reactions (lbs)							
TCLL	20.00	Wind Std ASCE 7-22	Pg NA	Ct: NA	CAT NA	PP Deflection in loc L/defl L/#		Gravity		Non-Gravity					
TCDL	10.00	Speed: 130 mph	Pf NA		Ce. NA	VERT(LL) 0.011 A 999 240		Loc	R+	/R-	/Rh	/Rw	/U	/RL	
BCLL	0.00	Enclosure: Closed	Lur NA	Cs NA		VERT(CL) 0.021 A 999 180		A	528	/-	/-	/297	/15	/72	
BCDL	10.00	Risk Category: II	Snow Duration NA			HORZ(LL) 0.007 A - -		C	643	/-	/-	/368	/29	/-	
		EXP B Kzt. NA				HORZ(TL) 0.013 A - -		Wind reactions based on MWFRS							
Des Ld	40.00	Mean Height: 15.00 ft	Building Code:			Creep Factor 2.0		A Brg Wid = 5.5 Min Req = 1.5 (Truss)							
NCBCLL	10.00	TCDL 5.0 psf	FBC 8th Ed 2023 Res.			Max TC CSI 0.420		C Brg Wid = 5.5 Min Req = 1.5 (Truss)							
Soffit	2.00	BCDL 5.0 psf	TPI Std 2014			Max BC CSI 0.421		Bearings A & C are a rigid surface.							
Load Duration	1.25	MWFRS Parallel Dist. 0 to h/2	Rep Fac: Yes			Max Web CSI 0.110		Members not listed have forces less than 375#							
Spacing	24.0 "	C&C Dist a. 3.00 ft	FT/RT.20(0)/10(0)			VIEW Ver 23.02 04 0123.13		Maximum Top Chord Forces Per Ply (lbs)							
		Loc. from endwall: Any	Plate Type(s)					Chords		Tens.Comp		Chords		Tens. Comp.	
		GCpi 0.18	WAVE					A - B		225 - 730		B - C		222 - 734	
		Wind Duration 1.60													

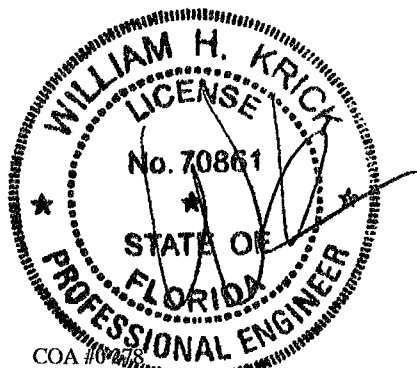
Lumber

Top chord. 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs: 2x4 SP #3,

Wind

Wind loads based on MWFRS with additional C&C member design
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
A - E	588 - 77	E - C	588 - 77



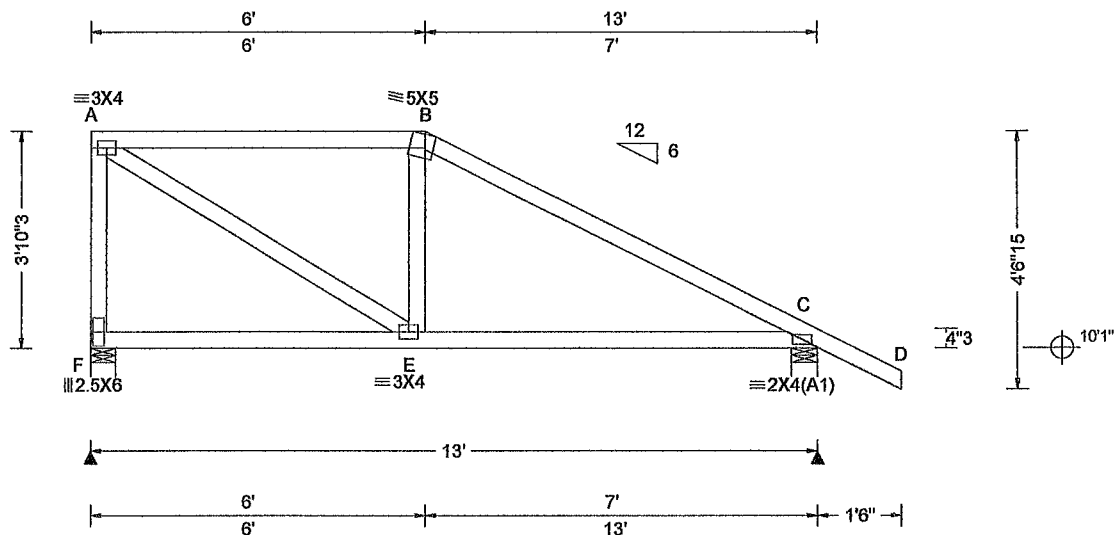
COA #0478

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For more information see these web sites. Alpine: alpineitw.com, TPI: tpinst.org, SBCA: sbacomp.com, ICC: iccsafe.org, AWC: awc.org

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN. 487767 FROM	COMN Qty: 1	Ply: 1 Qty: 1	Job Number: 25-2400 SPEC HOUSE Truss Label: B04	Cust: R 215 JRef: 1Y8T2150001 T51 DrwNo: 093.25.0838.47487 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL 20 00 TCDL 10 00 BCLL 0.00 BCDL 10 00 Des Ld: 40 00 NCBCLL 10 00 Soffit: 2 00 Load Duration 1 25 Spacing 24 0 "	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP B Kzt: NA Mean Height: 15 00 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall not in 9 00 ft GCpi 0.18 Wind Duration: 1 60	Pg NA Ct: NA CAT: NA Pf: NA Ce NA Lu NA Cs NA Snow Duration NA Building Code FBC 8th Ed 2023 Res. TPI Std 2014 Rep Fac. Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL) 0.011 B 999 240 VERT(CL) 0.021 C 999 180 HORZ(LL) -0.004 C - - HORZ(TL) 0.009 C - - Creep Factor 2.0 Max TC CSI 0.471 Max BC CSI 0.457 Max Web CSI 0.237 VIEW Ver 23.02.04.0123 13	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL F 522 /- /- /280 /33 /97 C 649 /- /- /388 /20 /- Wind reactions based on MWFRS F Brg Wid = 5.5 Min Req = 1.5 (Truss) C Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings F & C are a rigid surface Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 228 -539 B - C 212 -701

Lumber

Top chord 2x4 SP #2;
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,

Purlins

In lieu of structural panels use purlins to brace all flat
TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C
member design
Left end vertical not exposed to wind pressure
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

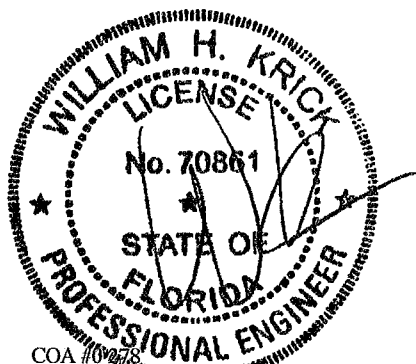
Chords Tens.Comp

E - C 553 -93

Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp. Webs Tens. Comp.

A - F 281 -482 A - E 623 -260



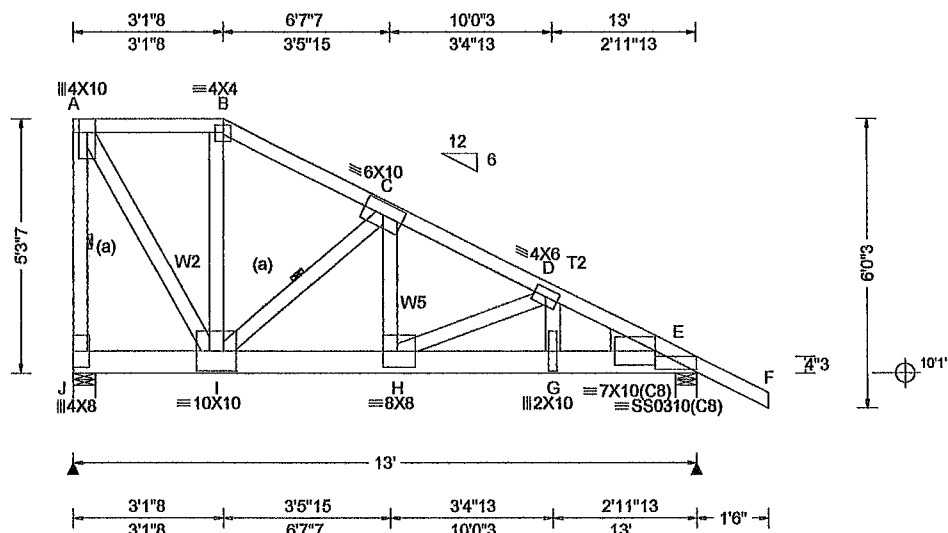
COA #09278

04/03/2025
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For more information see these web sites Alpine: alpineitw.com, TPI: tpinst.org, SBCA: sbcacomponents.com, ICC: iccsafe.org, AWC: awc.org

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 487835 FROM:	COMN Qty: 1	Ply: 1 Qty: 1	Job Number: 25-2400 SPEC HOUSE Truss Label: B06	Cust: R 215 JRef 1Y8T2150001 T45 DrwNo: 083.25.0842.59160 AK / WHK 04/03/2025
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Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defll/CSI Criteria	▲ Maximum Reactions (lbs)					
TCLL.	20 00	Wind Std ASCE 7-22	Pg NA Ct: NA CAT- NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity		
TCDL.	10 00	Speed. 130 mph	Pf NA Cc: NA	VERT(LL). 0.098 H 999 240	Loc	R+	/ R-	/ Rh	/ Rw	/ U / RL
BCLL.	0 00	Enclosure. Closed	Lu NA Cs. NA	VERT(CL). 0.196 H 788 180	J	4121	-	-	-	/26 /-
BCDL	10 00	Risk Category- II	Snow Duration NA	HORZ(LL). 0.035 A - -	E	3930	-	-	-	/79 /-
		EXP B Kzt: NA		HORZ(TL). 0.070 A - -	Wind reactions based on MWFRS					
Des Ld.	40 00	Mean Height. 15.00 ft	Building Code:	Creep Factor: 2.0	J	Brg Wid = 5.5		Min Req = 3.4 (Truss)		
NCBCLL.	10 00	TCDL. 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI 0.440	E	Brg Wid = 5.5		Min Req = 3.3 (Truss)		
Soffit:	2 00	BCDL. 5.0 psf	TPI Std: 2014	Max BC CSI 0.692	Bearings J & E are a rigid surface.					
Load Duration.	1.25	MWFRS Parallel Dist. 0 to h/2	Rep Fac: Varies by Ld Case	Max Web CSI 0.868	Members not listed have forces less than 375#					
Spacing.	24.0 "	C&C Dist a: 3.00 ft	FT/RT-20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)					
		Loc. from endwall. not in 9 00 ft	Plate Type(s)		Chords	Tens.Comp		Chords	Tens. Comp.	
		GCpi. 0.18	WAVE, 18SS	VIEW Ver 23.02.04 0123.13	A - B	12 -1995		C - D	18 -5557	
		Wind Duration: 1 60								

Lumber

Top chord 2x4 SP #2, T2 2x4 SP M-31,
Bot chord 2x6 SP 2400F-2 0E,
Webs. 2x4 SP #3, W2, W5 2x4 SP #2;
Rt Wedge. 2x6 SP #2,

Additional Notes

The overall height of this truss excluding overhang is
5-3-7

Bracing

(a) Continuous lateral restraint equally spaced on
member

Special Loads

----- (Lumber Dur Fac.=1.25 / Plate Dur Fac.=1.25)

TC From 62 plf at 0.00 to 62 plf at 3 12
TC From 31 plf at 3.12 to 62 plf at 10.94
TC From 62 plf at 10.94 to 62 plf at 14.50
BC From 10 plf at 0.00 to 10 plf at 13.00
BC From 4 plf at 13.00 to 4 plf at 14.50
BC 940 lb Conc. Load at 0.60
BC 887 lb Conc. Load at 2.60
BC 870 lb Conc. Load at 4.60
BC 1172 lb Conc. Load at 6.56
BC 1167 lb Conc. Load at 7.06, 8.94
BC 1051 lb Conc. Load at 10.94

Purlins

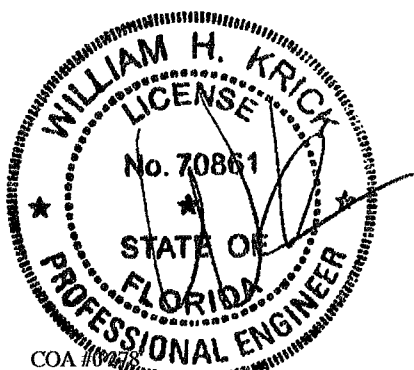
In lieu of structural panels use purlins to brace all flat
TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

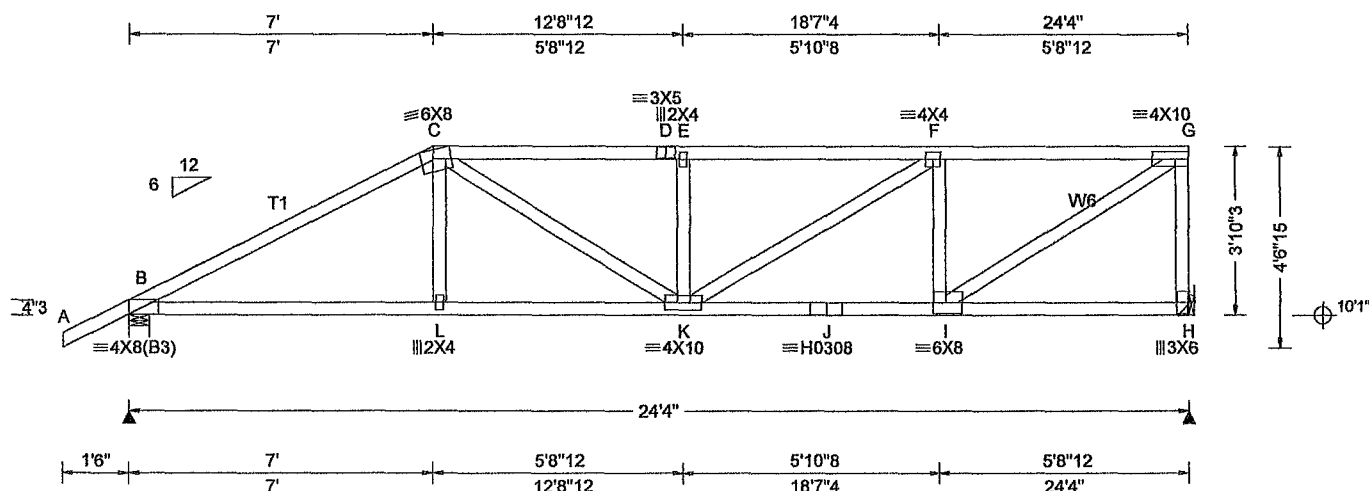


COA #04/03/2025
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 487819 FROM:	HIPM Qty: 1	Ply: 1 Qty: 1	Job Number: 25-2400 SPEC HOUSE Truss Label: C02	Cust: R 215 JRef: 1Y8T2150001 T21 DrwNo: 093.25.0839 14083 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL 20 00 TCCL 10 00 BCLL 0 00 BCDL 10.00 Des Ld: 40.00 NCBCLL 10 00 Soffit: 2.00 Load Duration: 1 25 Spacing: 24.0 "	Wind Std. ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category II EXP B Kzt: NA Mean Height: 15.00 ft TCDL 5 0 psf BCDL 5 0 psf MWFRS Parallel Dist. 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4 50 ft GCpi 0 18 Wind Duration 1.60	Pg NA Ct NA CAT NA Pf NA Ce NA Lr NA Cs NA Snow Duration NA Building Code: FBC 8th Ed 2023 Res. TPI Std 2014 Rep Fac: Varies by Ld Case FT/RT.20(0)/10(0) Plate Type(s) WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL) 0 172 E 999 240 VERT(CL) 0 344 E 842 180 HORZ(LL) 0 041 H - - HORZ(TL) 0 083 H - - Creep Factor: 2.0 Max TC CSI 0.877 Max BC CSI 0.547 Max Web CSI 0.999 VIEW Ver 23.02.04.0123 13	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 2391 -/- /- /- /157 -/ H 2464 -/- /- /- /182 -/ Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 2.0 (Truss) H Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 293 -4449 E - F 320 -4558 C - D 320 -4559 F - G 228 -3212 D - E 320 -4557

Lumber

Top chord 2x4 SP M-31, T1 2x4 SP #2;
Bot chord 2x4 SP M-31,
Webs: 2x4 SP #3, W6 2x4 SP #2,

Special Loads

---(Lumber Dur Fac.=1 25 / Plate Dur Fac.=1.25)
TC From 62 plf at -1.50 to 62 plf at 7 00
TC From 31 plf at 7.00 to 31 plf at 24.33
BC From 4 plf at -1.50 to 4 plf at 0 00
BC From 20 plf at 0.00 to 20 plf at 7 03
BC From 10 plf at 7 03 to 10 plf at 24 33
TC 542 lb Conc. Load at 7 03
TC 187 lb Conc. Load at 9 06,11 06,13 06,15 06
17 06,19 06,21 06,23 06
BC 394 lb Conc. Load at 7 03
BC 129 lb Conc. Load at 9.06,11 06,13 06,15 06
17 06,19.06,21 06,23 06

Hangers / Ties

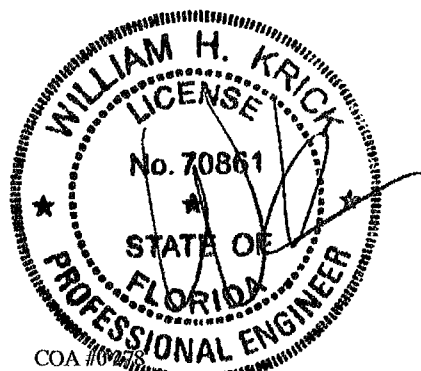
(J) Hanger Support Required, by others

Purlins

In lieu of structural panels use purlins to brace all flat
TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



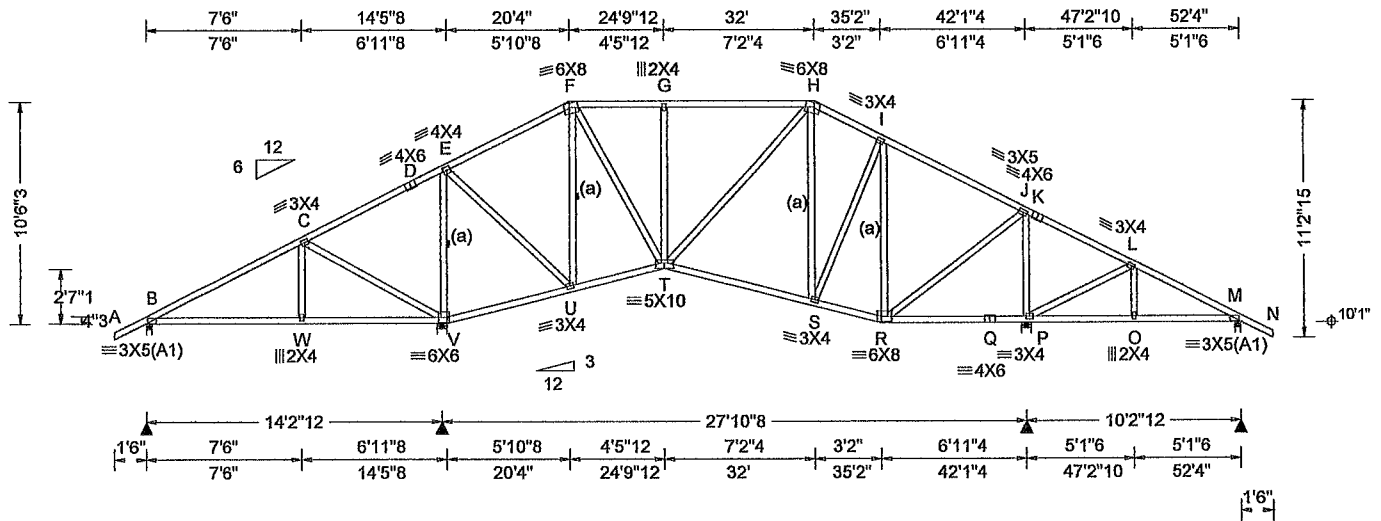
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN. 487771 FROM	COMN Qty 1	Ply 1 Qty 1	Job Number: 25-2400 SPEC HOUSE Truss Label: C04	Cust: R 215 JRef: 1Y8T2150001 T29 DrwNo: 093.25.0839.31343 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL. 20 00 TCDL. 10 00 BCDL. 0 00 BCDL. 10 00 Des Ld. 40 00 NCBCLL. 10 00 Soffit: 2.00 Load Duration 1.25 Spacing. 24.0 "	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP B Kzt: NA Mean Height: 15 14 ft TCDL. 5 0 psf BCDL. 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a. 5.23 ft Loc. from endwall: not in 13 00 ft GCpl 0 18 Wind Duration 1.60	Pg NA Ct NA CAT: NA Pf NA Ce: NA Lu NA Cs. NA Snow Duration NA Building Code FBC 8th Ed. 2023 Res. TPI Std 2014 Rep Fac: Yes FT/RT 20(0)/10(0) Plate Type(s) WAVE	PP Deflection in loc L/defl L/# VERT(LL) 0 039 G 999 240 VERT(CL) 0 080 G 999 180 HORZ(LL) 0 019 M - - HORZ(TL) 0 040 R - - Creep Factor: 2 0 Max TC CSI 0.638 Max BC CSI 0.458 Max Web CSI 0.825 VIEW Ver 23 02 04.0123.13	Gravity Loc R+ /R- /Rh /Rw /U /RL B 554 -/- /296 -/- /220 V 2001 -/- /1188 -/- P 1664 -/- /983 -/- M 415 -/- /261 /1 -/- Non-Gravity Wind reactions based on MWFRS B Brg Wid = 3 0 Min Req = 1 5 (Truss) V Brg Wid = 5.5 Min Req = 2.4 (Truss) P Brg Wid = 5.5 Min Req = 1 6 (Truss) M Brg Wid = 3 0 Min Req = 1 5 (Truss) Bearings B, V, P, & M are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs. 2x4 SP #3,

Bracing

(a) Continuous lateral restraint equally spaced on member

Purlins

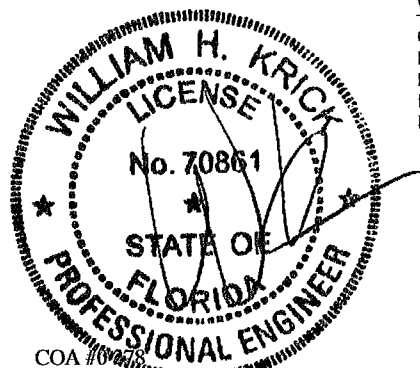
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design
Wind loading based on both gable and hip roof types.

Additional Notes

WARNING. Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below



COA 100000

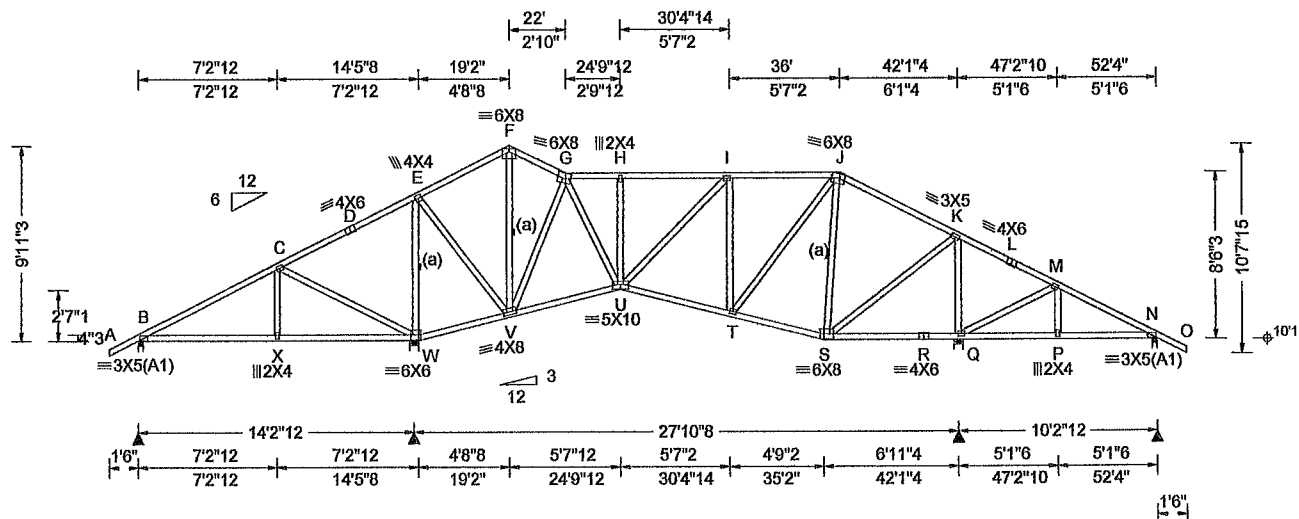
04/03/2025 Florida Certificate of Product Approval #FL 1999

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 487773 FROM	COMN	Ply: 1 Qty: 1	Job Number: 25-2400 SPEC HOUSE Truss Label: C06	Cust: R 215 JRef 1Y8T2150001 T36 DrwNo: 093.25.0840.03327 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL 20 00 TCDL 10 00 BCDL 0 00 BCDL 10 00 Des Ld. 40 00 NCBCLL 10 00 Soffit 2.00 Load Duration 1 25 Spacing 24 0 "	Wind Std: ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category II EXP B Kzt: NA Mean Height: 15.00 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist. h to 2h C&C Dist a: 5.23 ft Loc. from endwall, not in 13 00 ft GCpi 0 18 Wind Duration 1.60	Pg NA Ct: NA CAT NA Pf NA Ce NA Lu NA Cs NA Snow Duration NA Building Code FBC 8th Ed. 2023 Res. TPI Std 2014 Rep Fac: Yes FT/RT 20(0)/10(0) Plate Type(s) WAVE	PP Deflection in loc L/defl L/# VERT(LL) 0 043 H 999 240 VERT(CL) 0 091 H 999 180 HORZ(LL) 0 019 S - - HORZ(TL) 0 041 S - - Creep Factor 2.0 Max TC CSI 0.702 Max BC CSI 0.422 Max Web CSI 1.000 VIEW Ver: 23 02 04.0123 13	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 499 - / - / 1244 - / 1209 W 2163 - / - / 1245 - / - Q 1660 - / - / 952 - / - N 396 - / - / 1245 / 9 - Wind reactions based on MWFRS B Brg Wid = 3.0 Min Req = 1.5 (Truss) W Brg Wid = 5.5 Min Req = 2 6 (Truss) Q Brg Wid = 5.5 Min Req = 1 6 (Truss) N Brg Wid = 3.0 Min Req = 1.5 (Truss) Bearings B, W, Q, & N are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs: 2x4 SP #3,

Bracing

(a) Continuous lateral restraint equally spaced on member

Plating Notes

All plates are 3X4 except as noted

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

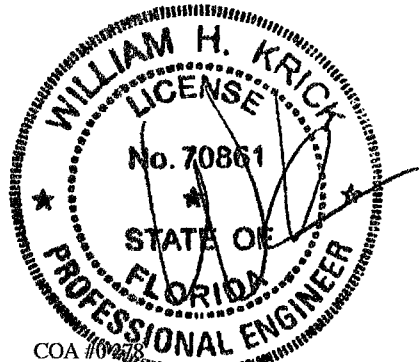
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types

Additional Notes

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COA #09248

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Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	230 -389	H - I	214 -806
C - D	649 -42	I - J	245 -752
D - E	745 -24	J - K	203 -633
G - H	214 -806	K - L	426 0

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
W - V	208 -629	U - T	804 -27
V - U	504 0	T - S	465 0

Maximum Web Forces Per Ply (lbs)

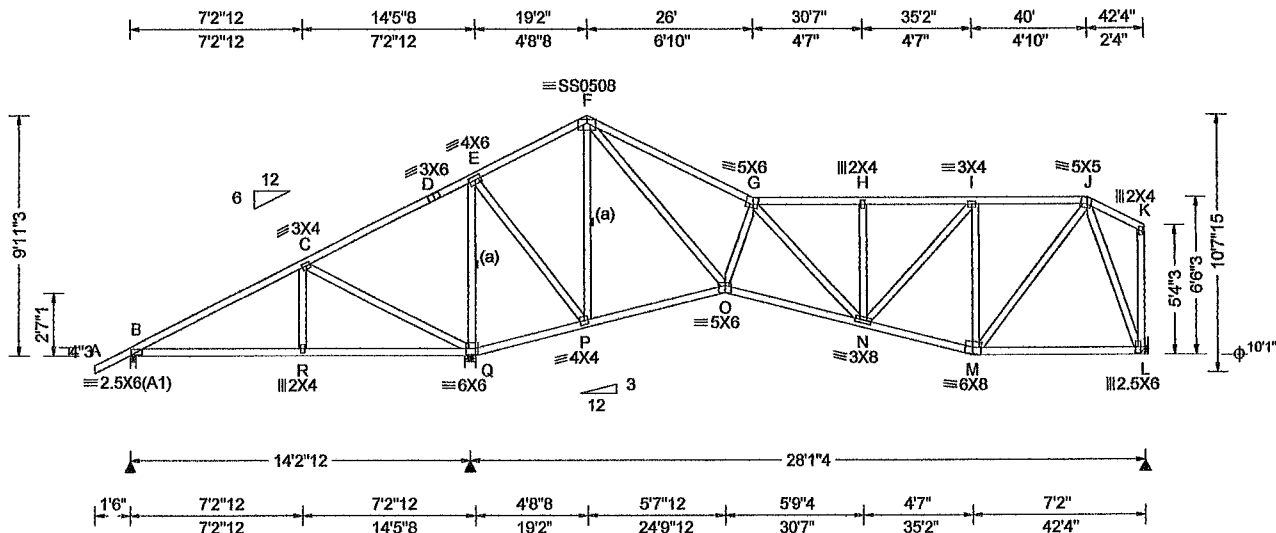
Webs	Tens.Comp.	Webs	Tens. Comp.
C - W	89 -730	T - J	536 -84
E - W	244 -1563	S - J	108 -547
E - V	1081 -78	S - K	920 -95
V - G	204 -920	K - Q	229 -1313
G - U	763 -108	Q - M	64 -470
I - T	132 -406		



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SEQN. 487775 FROM:	COMN Qty: 1	Ply 1 Qty: 1	Job Number: 25-2400 SPEC HOUSE Truss Label: C08	Cust: R 215 JRef 1Y8T2150001 T23 DrwNo: 093.25.0840.08997 AK / WHK 04/03/2025
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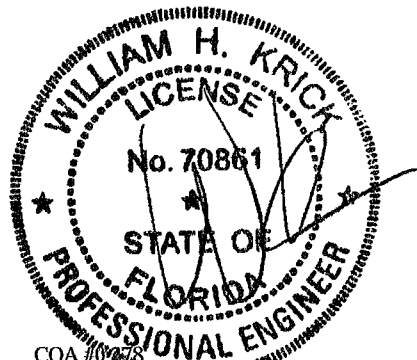


Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL. 20 00 TCDL. 10.00 BCLL. 0 00 BCDL. 10 00 Des Ld. 40 00 NCBCLL. 10.00 Soffit: 2 00 Load Duration 1 25 Spacing 24.0 "	Wind Std ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP B Kzt: NA Mean Height: 15.00 ft TCDL. 5.0 psf BCDL. 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a. 4.23 ft Loc. from endwall. not in 13.00 ft GCpi 0 18 Wind Duration. 1.60	Pg NA Ct: NA CAT: NA Pf NA Ce: NA Lu NA Cs. NA Snow Duration NA Building Code FBC 8th Ed 2023 Res TPI Std 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, 18SS	PP Deflection in loc L/def L/# VERT(LL): 0 074 G 999 240 VERT(CL): 0 158 G 999 180 HORZ(LL): 0 032 F - - HORZ(TL): 0 067 F - - Creep Factor: 2.0 Max TC CSI 0 783 Max BC CSI 0.549 Max Web CSI 0.894 VIEW Ver 23 02.04 0123 13	Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh Non-Gravity / Rw / U / RL B 434 -161 - / 188 /3 /181 Q 2423 - / - /1340 - / - L 940 - / - /489 - / - Wind reactions based on MWFRS B Brg Wid = 3 0 Min Req = 1.5 (Truss) Q Brg Wid = 5 5 Min Req = 2.9 (Truss) L Brg Wid = - Min Req = - Bearings B & Q are a rigid surface Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens Comp.

Lumber Top chord 2x4 SP #2; Bot chord 2x4 SP #2, Webs. 2x4 SP #3,	Bracing (a) Continuous lateral restraint equally spaced on member	Hangers / Ties (J) Hanger Support Required, by others	Purlins In lieu of structural panels use purlins to brace all flat TC @ 24" oc.	Wind Wind loads based on MWFRS with additional C&C member design Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens Comp. Chords Tens. Comp. B - R 150 -445 O - N 1199 -179 R - Q 146 -448 N - M 813 -143 Q - P 172 -910
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Maximum Bot Chord Forces Per Ply (lbs) Chords Tens Comp. Chords Tens. Comp.	B - R 150 -445 R - Q 146 -448 Q - P 172 -910	O - N 1199 -179 N - M 813 -143
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Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp.	C - Q 88 -740 E - Q 273 -1754 E - P 1282 -152 P - F 195 -1110 F - O 1474 -252	O - G 245 -791 N - I 449 -79 I - M 194 -668 M - J 775 -117 J - L 174 -862
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COA #09478

04/03/2025
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Lumber		B - C	838	-202	G - H	173	-782
Top chord	2x4 SP #2,	C - D	1299	-184	H - I	101	-793
Bot chord	2x4 SP #2,	D - E	1341	-157	I - J	217	-1530
Webs.	2x4 SP #3,	E - F	1280	-102	J - K	190	-1082

Hangers / Ties	B - R	129	-711	O - N	1618	-237
	R - Q	127	-715	N - M	1105	-203
(J) Hanger Support Required, by others	Q - P	142	-608			

Wind		C - Q	88 - 738	O - I	185 - 951
Wind loads based on MWFRS with additional C&C member design		Q - F	234 - 1768	N - J	547 - 32
Right end vertical not exposed to wind pressure.		F - P	1029 - 135	J - M	230 - 749
Wind loading based on both gable and hip roof types.		P - G	202 - 1010	M - K	1233 - 221
		G - O	1272 - 194	K - L	193 - 808

Additional Notes
Negative reaction(s) of -199# MAX from a non-wind load case requires uplift connection See Maximum Reactions.

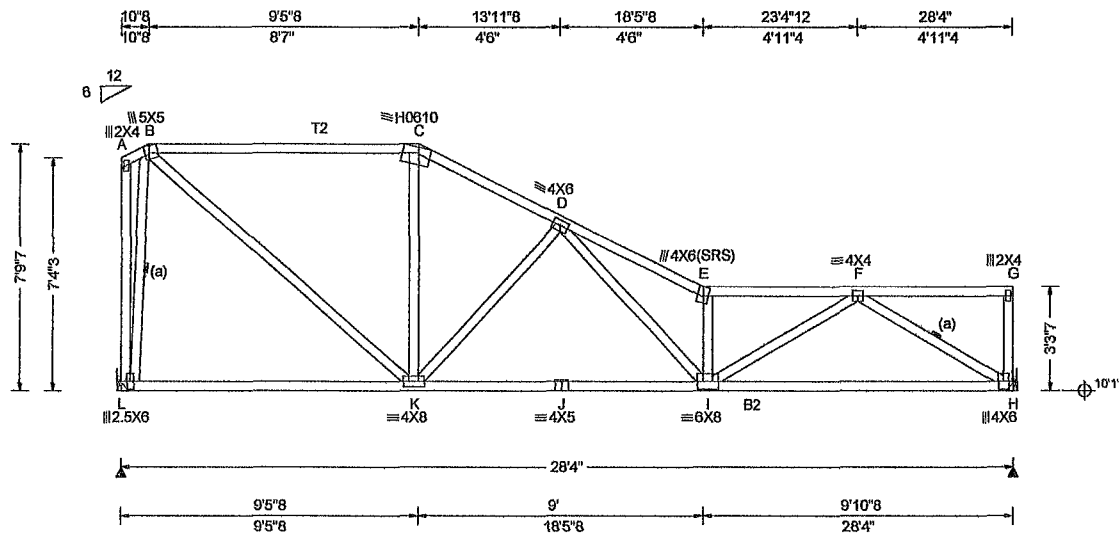


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155 Harlem Ave.
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Glenview, IL 60025

SEQN: 487779 FROM:	SPEC Qty: 1	Ply: 1	Job Number: 25-2400 SPEC HOUSE Truss Label: C12	Cust: R215 JRef: 1Y8T2150001 T41 DrwNo: 093.25.0840.21280 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL 20.00 TCDL 10.00 BCLL 0.00 BCDL 10.00 Des Ld: 40.00 NCBCLL 10.00 Soffit: 2.00 Load Duration 1.25 Spacing: 24.0"	Wind Std ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP B Kzt. NA Mean Height: 15.62 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi 0.18 Wind Duration: 1.60	Pg NA Ct: NA CAT NA Pf NA Ce: NA Lu. NA Cs. NA Snow Duration NA Building Code: FBC 8th Ed 2023 Res. TPI Std 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL) 0.131 E 999 240 VERT(CL) 0.270 E 999 180 HORZ(LL) 0.045 B - - HORZ(TL) 0.092 B - - Creep Factor: 2.0 Max TC CSI 0.614 Max BC CSI 0.935 Max Web CSI 0.782 VIEW Ver: 23.02.04 0123.13	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL L 1167 /- /- /627 /11 /79 H 1167 /- /- /614 /- /- Wind reactions based on MWFRS L Brg Wid = - Min Req = - H Brg Wid = - Min Req = - Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 249 -1034 D - E 559 -3003 C - D 237 -1197 E - F 436 -2557

Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31,
Bot chord: 2x4 SP #2; B2 2x4 SP M-31,
Webs: 2x4 SP #3,

Bracing

(a) Continuous lateral restraint equally spaced on member

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

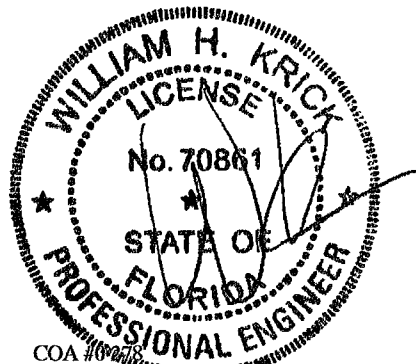
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0000000000

Florida State of Product Approval #FL 1999

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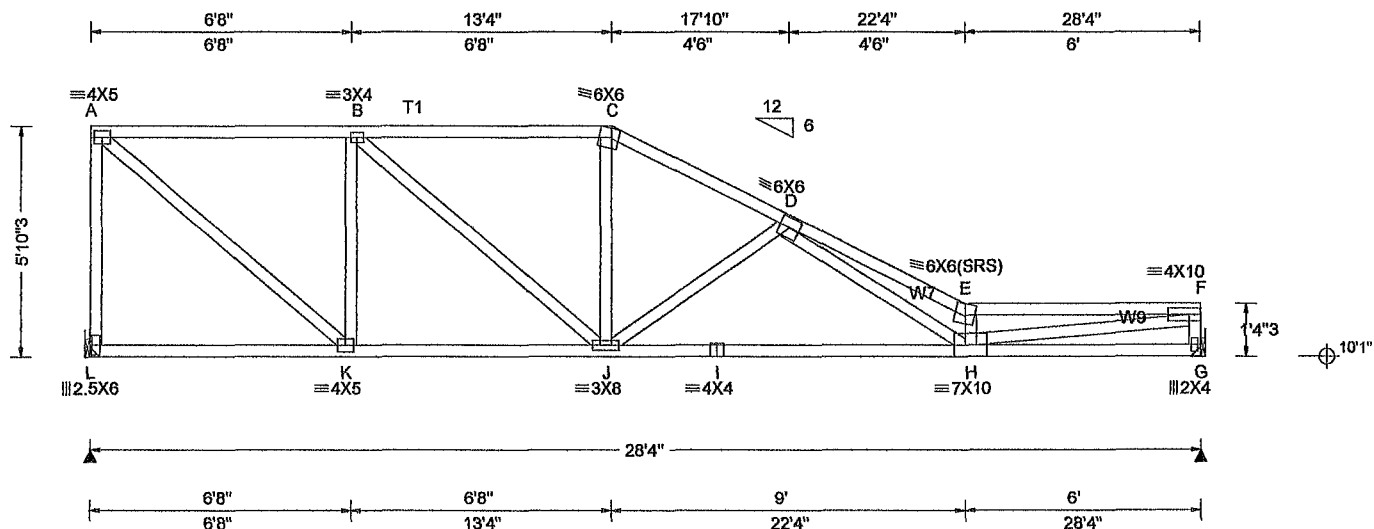
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 487813 FROM:	HIPS Qty: 1	Ply: 1 Job Number: 25-2400 SPEC HOUSE Truss Label: C14	Cust: R 215 JRef 1Y8T2150001 T18 DrwNo: 093.25.0840.30033 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL 20 00 TCDL 10 00 BCLL 0 00 BCDL 10.00 Des Ld 40 00 NCBCLL 10 00 Soffit 2 00 Load Duration 1.25 Spacing: 24.0 "	Wind Std ASCE 7-22 Speed 130 mph Enclosure Closed Risk Category II EXP B Kzt. NA Mean Height 15.00 ft TCDL 5 0 psf BCDL 5 0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall, not in 9 00 ft GCpi 0 18 Wind Duration 1.60	Pg NA Ct: NA CAT: NA Pf NA Ce NA Lu NA Cs NA Snow Duration NA Building Code: FBC 8th Ed. 2023 Res. TPI Std 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s) WAVE	PP Deflection in loc L/defl L/# VERT(LL) 0.350 E 971 240 VERT(CL) 0.727 E 467 180 HORZ(LL) 0.088 A - - HORZ(TL) 0.183 A - - Creep Factor 2.0 Max TC CSI 0.924 Max BC CSI 0.415 Max Web CSI 0.785 VIEW Ver: 23 02 04.0123.13	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL L 1154 -/-/-/-/37 -/- G 1051 -/-/-/-/38 -/- Wind reactions based on MWFRS L Brg Wid = - Min Req = - G Brg Wid = - Min Req = - Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp Chords Tens Comp. A - B 35 -1092 D - E 155 -5764 B - C 40 -1502 E - F 110 -4748 C - D 58 -1735

Lumber

Top chord: 2x4 SP M-31, T1 2x4 SP #2,
Bot chord 2x4 SP M-31,
Webs: 2x4 SP #3, W7 2x4 SP #2; W9 2x4 SP M-31,

Special Loads

----- (Lumber Dur Fac.=1.25 / Plate Dur Fac.=1.25)
TC From 62 plf at 0 00 to 62 plf at 22.33
TC From 31 plf at 22.33 to 31 plf at 28.33
BC From 20 plf at 0 00 to 20 plf at 24.40
BC From 10 plf at 24.40 to 10 plf at 28.33
TC: 21 lb Conc. Load at 24.40,26.40
BC 28 lb Conc. Load at 24.40,26.40

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

In lieu of structural panels use purlins to brace all flat
TC @ 24" oc.

The TC of this truss shall be braced with attached
spans at 24" oc in lieu of structural sheathing

Wind

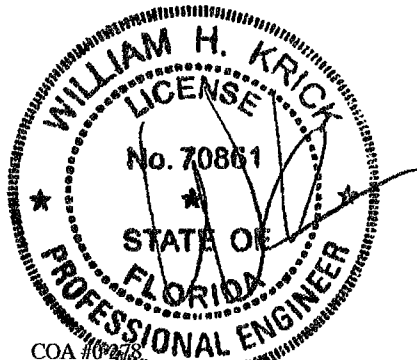
Wind loads and reactions based on MWFRS.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Deflection

Max JT VERT DEFL LL 0 34" DL 0.40" See detail
DEFLCAMB1014 for camber recommendations.
Provide for adequate drainage of roof



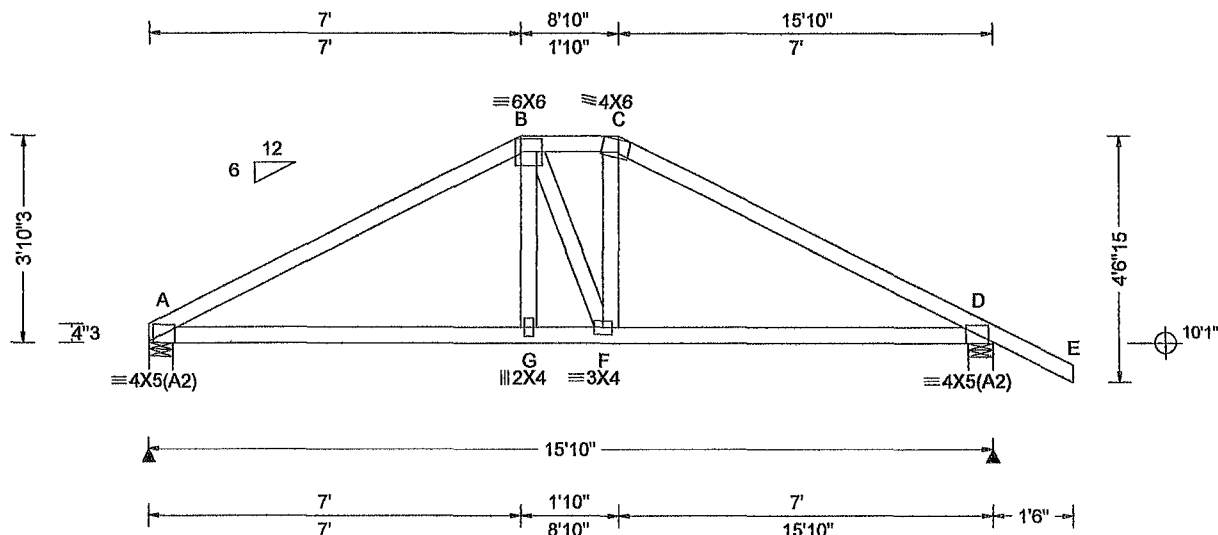
COA #092618

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North Building, 4th Floor
Glenview, IL 60025

SEQN 487820 FROM	HIPS	Ply 1 Qty: 1	Job Number: 25-2400 SPEC HOUSE Truss Label: D01	Cust: R 215 JRef: 1Y8T2150001 T30 DrwNo: 093.25.0840.34913 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg Pf In PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL 20 00 TCDL 10 00 BCLL 0 00 BCDL 10 00 Des Ld: 40.00 NCBCLL 10 00 Soffit 2 00 Load Duration: 1 25 Spacing 24.0 "	Wind Std ASCE 7-22 Speed: 130 mph Enclosure Closed Risk Category II EXP B Kzt: NA Mean Height: 15 00 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall. not in 4.50 ft GCpi 0.18 Wind Duration 1 60	Pg NA Ct: NA CAT: NA Pf NA Ce NA Lu NA Cs NA Snow Duration NA Building Code: FBC 8th Ed 2023 Res. TPI Std 2014 Rep Fac: Varies by Ld Case FT/RT.20(0)/10(0) Plate Type(s) WAVE	PP Deflection in loc L/defl L/# VERT(LL) 0.071 G 999 240 VERT(CL) 0.142 G 999 180 HORZ(LL) 0.030 D - - HORZ(TL) 0.059 D - - Creep Factor 2.0 Max TC CSI 0.918 Max BC CSI 0.849 Max Web CSI 0.180 VIEW Ver: 23.02 04.0123 13	Gravity Loc R+ / R- / Rh Non-Gravity / Rw / U / RL A 1546 /- /- /- /66 /- D 1657 /- /- /- /88 /- Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.8 (Truss) D Brg Wid = 5.5 Min Req = 2.0 (Truss) Bearings A & D are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp Chords Tens Comp. A - B 144 -2850 C - D 143 -2837 B - C 103 -2489

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2;
Webs: 2x4 SP #3,

Special Loads

---(Lumber Dur Fac.=1.25 / Plate Dur Fac.=1.25)

TC From 62 plf at 0.00 to 62 plf at 7.00	TC From 31 plf at 7.00 to 31 plf at 8.83	TC From 62 plf at 8.83 to 62 plf at 17.33	BC From 20 plf at 0.00 to 20 plf at 7.03	BC From 10 plf at 7.03 to 10 plf at 8.80	BC From 20 plf at 8.80 to 20 plf at 15.83	BC From 4 plf at 15.83 to 4 plf at 17.33
TC 542 lb Conc. Load at 7.03, 8.80	BC 394 lb Conc. Load at 7.03, 8.80					

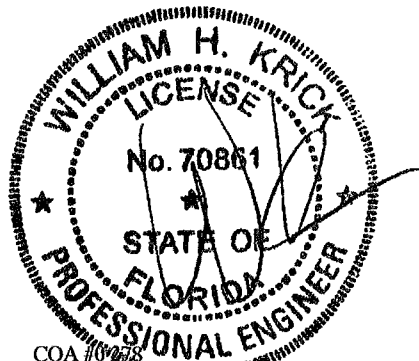
Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS

Wind loading based on both gable and hip roof types.



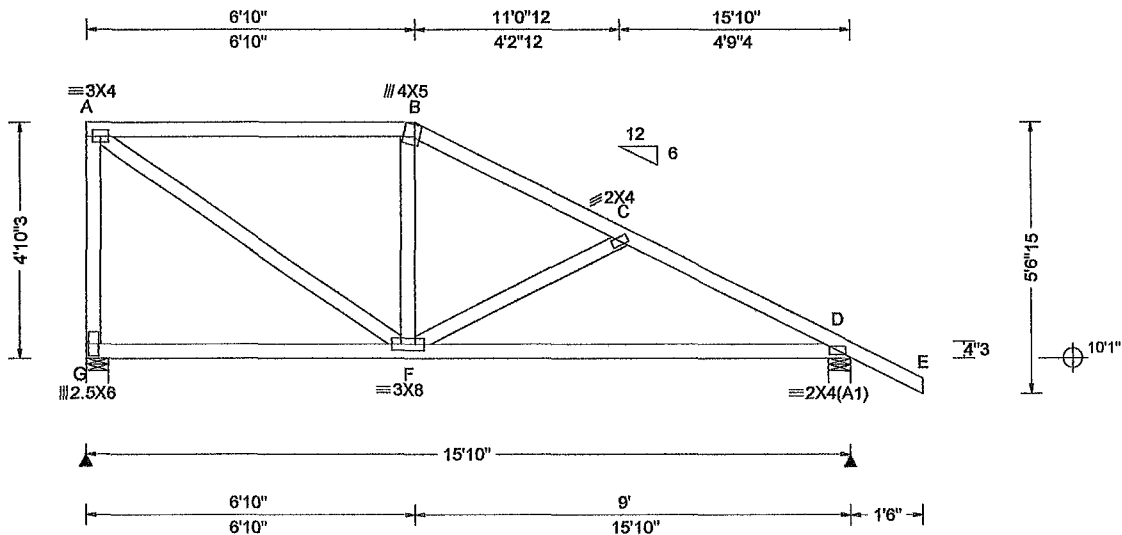
COA #0908

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 155 Harlem Ave
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 Glenview, IL 60025

SEQN: 487783 FROM	COMN Qty: 1	Ply: 1 Qty: 1	Job Number: 25-2400 SPEC HOUSE Truss Label: D03	Cust: R215 JRef: 1Y8T2150001 T35 DrwNo: 093.25.0840.42887 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL 20 00 TCDL 10 00 BCLL 0 00 BCDL 10 00 Des Ld 40 00 NCBCLL 10.00 Soffit: 2 00 Load Duration 1 25 Spacing 24 0 "	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP B Kzt: NA Mean Height: 15.00 ft TCDL 5 0 psf BCDL 5 0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a 3.00 ft Loc. from endwall not in 9 00 ft GCpi: 0.18 Wind Duration: 1 60	Pg NA Ct: NA CAT: NA Pf NA Ce: NA Lu NA Cs: NA Snow Duration NA Building Code: FBC 8th Ed 2023 Res. TPI Std. 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s) WAVE	PP Deflection in loc L/defl L/# VERT(LL) 0 020 C 999 240 VERT(CL) 0 040 C 999 180 HORZ(LL) 0 006 D - - HORZ(TL) 0 012 D - - Creep Factor 2.0 Max TC CSI 0 763 Max BC CSI 0 674 Max Web CSI 0 307 VIEW Ver: 23.02.04 0123 13	Gravity Loc R+ /R- /Rh Non-Gravity /Rw /U /RL G 640 /- /- /347 /42 /121 D 764 /- /- /457 /19 /- Wind reactions based on MWFRS G Brg Wid = 5.5 Min Req = 1.5 (Truss) D Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings G & D are a rigid surface Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp A - B 214 -604 C - D 217 -994 B - C 196 -724

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2;
Webs 2x4 SP #3,

Purlins

In lieu of structural panels use purlins to brace all flat
TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C
member design

Left end vertical not exposed to wind pressure

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords Tens Comp.

F - D 839 -118

Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp Webs Tens. Comp.

A - G 294 -592 A - F 730 -258



COA #0925

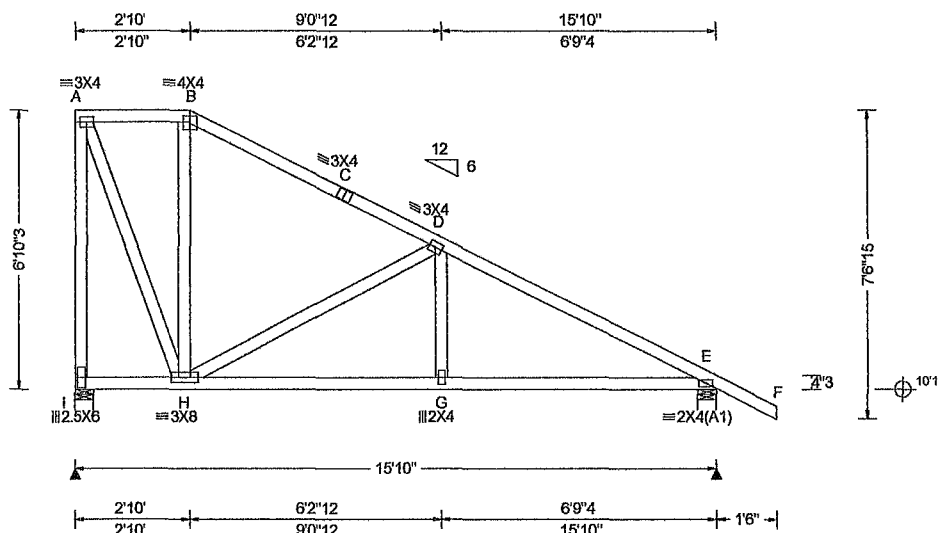
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 487785 FROM.	COMN Qty: 1	Ply: 1 Qty: 1	Job Number: 25-2400 SPEC HOUSE Truss Label: D05	Cust: R 215 JRef: 1Y8T2150001 T2 DrwNo: 093.25.0840.45390 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL 20 00 TCDL 10 00 BCLL 0 00 BCDL 10 00 Des Ld: 40.00 NCBCLL 10 00 Soffit: 2.00 Load Duration: 1 25 Spacing 24 0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP B Kzt: NA Mean Height: 15.00 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist. h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9 00 ft GCpi 0 18 Wind Duration 1 60	Pg NA Ct: NA CAT: NA Pf NA Ce NA Lu NA Cs NA Snow Duration NA Building Code: FBC 8th Ed 2023 Res. TPI Std. 2014 Rep Fac: Yes FT/RT 20(0)/10(0) Plate Type(s). WAVE	PP Deflection in loc L/defl L/# VERT(LL) 0 022 G 999 240 VERT(CL) 0 045 G 999 180 HORZ(LL) 0.008 A - - HORZ(TL) 0 017 A - - Creep Factor: 2.0 Max TC CSI 0.469 Max BC CSI 0.444 Max Web CSI 0.627 VIEW Ver: 23 02 04 0123 13	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I 640 /- /- /383 /14 /123 E 764 /- /- /464 /- /- Wind reactions based on MWFRS I Brg Wid = 5.5 Min Req = 1 5 (Truss) E Brg Wid = 5.5 Min Req = 1 5 (Truss) Bearings I & E are a rigid surface Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp D - E 49 -971

Lumber

Top chord: 2x4 SP #2,
Bot chord: 2x4 SP #2,
Webs: 2x4 SP #3,

Purlins

In lieu of structural panels use purlins to brace all flat
TC @ 24" oc.

Wind

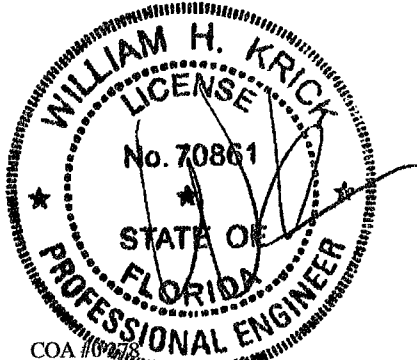
Wind loads based on MWFRS with additional C&C
member design.
Left end vertical not exposed to wind pressure
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp	Chords	Tens Comp
H - G	794 0	G - E	797 0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp	Webs	Tens Comp
A - I	193 -628	H - D	129 -630
A - H	627 -151		



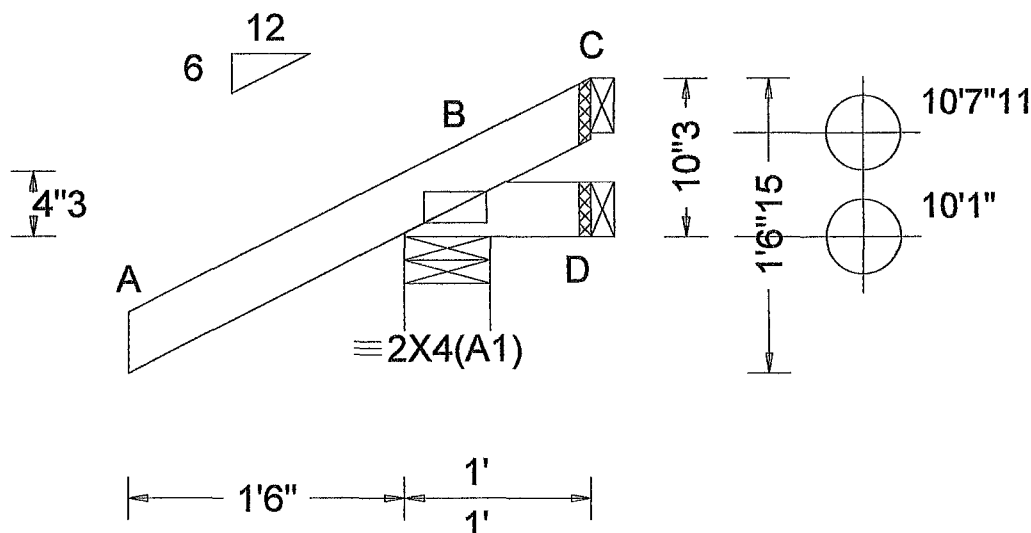
COA # 09018

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 487786 FROM:	JACK	Ply: 1 Qty: 12	Job Number: 25-2400 SPEC HOUSE Truss Label: J01	Cust: R 215 JRef: 1Y8T2150001 T8 Dwno: 093.25.0841.02460 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL 20.00 TCDL 10.00 BCLL 0.00 BCDL 10.00 Des Ld 40.00 NCBCLL 10.00 Soffit 2.00 Load Duration 1.25 Spacing 24.0"	Wind Std. ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category II EXP B Kzt. NA Mean Height: 15.00 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall Any GCpl. 0.18 Wind Duration 1.60	Pg NA Ct NA CAT NA Pf NA Ce NA Lu NA Cs NA Snow Duration NA Building Code FBC 8th Ed 2023 Res TPI Std 2014 Rep Fac: Yes FT/RT-20(0)/10(0) Plate Type(s) WAVE	PP Deflection in loc L/defl L/# VERT(LL) NA VERT(CL) NA HORZ(LL) -0.000 B - - HORZ(TL) 0.001 B - - Creep Factor 2.0 Max TC CSI 0.159 Max BC CSI 0.022 Max Web CSI 0.000 VIEW Ver: 23 02 04 0123.13	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 254 /- /- /167 /32 /26 D 4 /-18 /- /10 /11 /- C - /-53 /- /20 /40 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

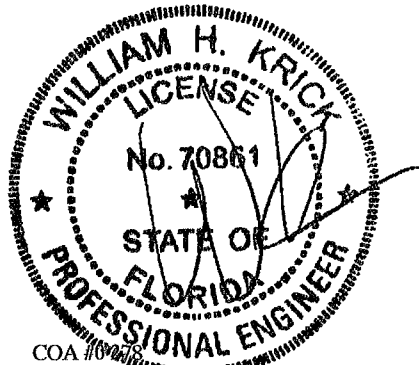
Lumber

Top chord 2x4 SP #2;
Bot chord 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design

Wind loading based on both gable and hip roof types



COA #00000000

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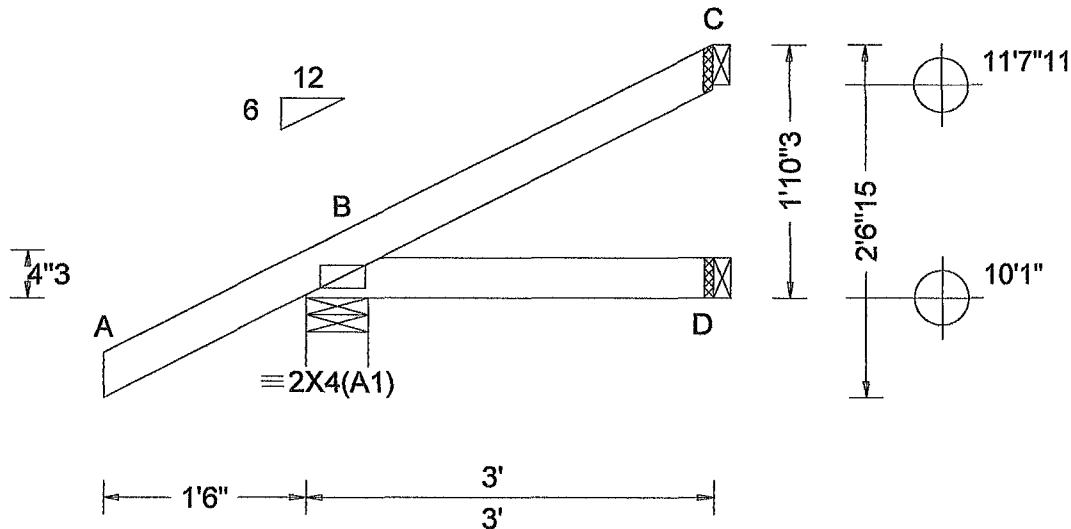
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Glenview, IL 60025

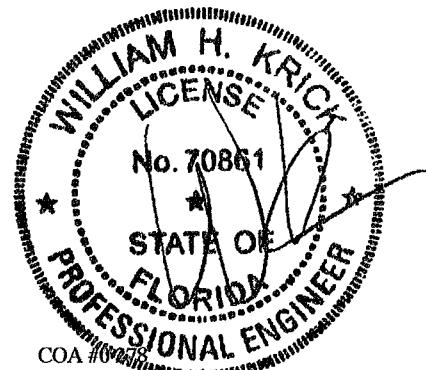
SEQN: 487787 FROM:	JACK Qty: 12	Ply: 1 Qly: 12	Job Number: 25-2400 SPEC HOUSE Truss Label: J02	Cust: R 215 JRef: 1Y8T2150001 T8 DwnNo: 083.25.0841.05257 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL 20 00 TCDL 10 00 BCLL 0 00 BCDL 10 00 Des Ld: 40 00 NCBCLL 10 00 Soffit: 2.00 Load Duration: 1.25 Spacing 24.0 "	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP B Kzt: NA Mean Height: 15.00 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist. 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi. 0 18 Wind Duration 1 60	Pg NA Ct: NA CAT: NA Pf NA Ce: NA Lu NA Cs: NA Snow Duration NA Building Code: FBC 8th Ed 2023 Res. TPI Std 2014 Rep Fac: Yes FT/RT.20(0)/10(0) Plate Type(s) WAVE	PP Deflection in Loc L/defl L/# VERT(LL) NA VERT(CL) NA HORZ(LL) 0 000 B - - HORZ(TL) 0 001 B - - Creep Factor: 2.0 Max TC CSI 0 159 Max BC CSI 0.064 Max Web CSI 0.000 VIEW Ver 23 02 04.0123.13	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 262 /- /- /164 /10 /50 D 49 /- /- /26 /- /- C 62 /- /- /32 /19 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1 5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface Members not listed have forces less than 375#

Lumber
Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,

Wind
Wind loads based on MWFRS with additional C&C member design
Wind loading based on both gable and hip roof types.



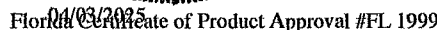
COA #04438
04/03/2025
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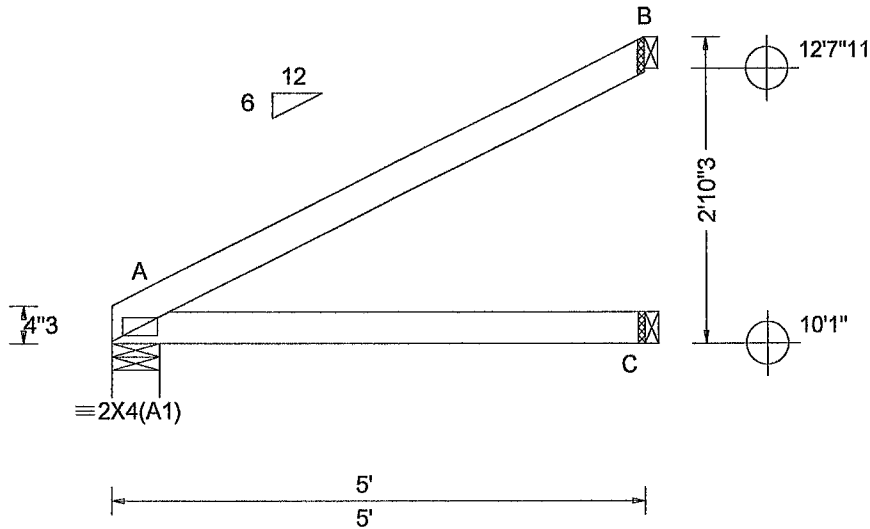
Diagram illustrating a structural frame with an inclined member AC and a horizontal member BD. The frame is supported by a column at B and a fixed support at D. The inclined member AC has a slope of 12/6. The horizontal distance from A to B is 1'6", and from B to D is 5'. The vertical distance from the base to point C is 2'10"3, and from the base to point D is 3'6"15. The cross-section at C is 12'7"11, and the cross-section at D is 10'1". The column at B is labeled 2X4(A1).

Lumber Top chord 2x4 SP #2, Bot chord 2x4 SP #2, Wind Wind loads based on MWFRS with additional C&C member design Wind loading based on both gable and hip roof types.



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SEQN: 487790 FROM	JACK Qty: 1	Ply 1 Job Number: 25-2400 SPEC HOUSE Truss Label: J05	Cust: R 215 JRef: 1Y8T2150001 T4 DrwNo: 093.25.0841.12770 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL 20.00	Wind Std ASCE 7-22	Pg NA Ct: NA CAT NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL 10.00	Speed 130 mph	Pf NA Ce NA	VERT(LL) NA	Loc R+ / R- / Rh / Rw / U / RL
BCLL 0.00	Enclosure: Closed	Lu NA Cs NA	VERT(CL) NA	
BCDL 10.00	Risk Category: II	Snow Duration NA	HORZ(LL) 0.006 A - -	A 212 /- /- /126 /- /59
Des Ld 40.00	EXP B Kzt: NA		HORZ(TL) 0.012 A - -	C 93 /- /- /54 /- /-
NCBCLL 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor 2.0	B 137 /- /- /76 /37 /-
Soffit: 2.00	TCDL 5.0 psf	FBC 8th Ed 2023 Res.	Max TC CSI 0.361	Wind reactions based on MWFRS
Load Duration 1.25	BCDL 5.0 psf	TPI Std 2014	Max BC CSI 0.251	A Brg Wid = 5.5 Min Req = 1.5 (Truss)
Spacing 24.0"	MWFRS Parallel Dist. 0 to h/2	Rep Fac: Yes	Max Web CSI 0.000	C Brg Wid = 1.5 Min Req = -
	C&C Dist a: 3.00 ft	FT/RT 20(0)/10(0)		B Brg Wid = 1.5 Min Req = -
	Loc. from endwall not in 4.50 ft	Plate Type(s)		Bearing A is a rigid surface.
	GCpl 0.18	WAVE	VIEW Ver: 23.02 04.0123 13	Members not listed have forces less than 375#
	Wind Duration 1.60			

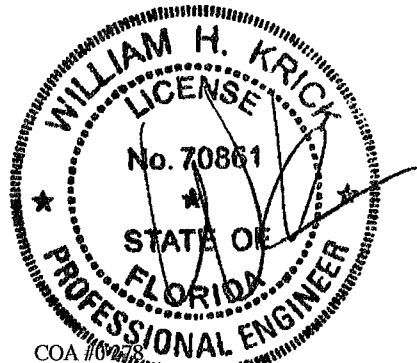
Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,

Wind

Wind loads based on MWFRS with additional C&C
member design

Wind loading based on both gable and hip roof types

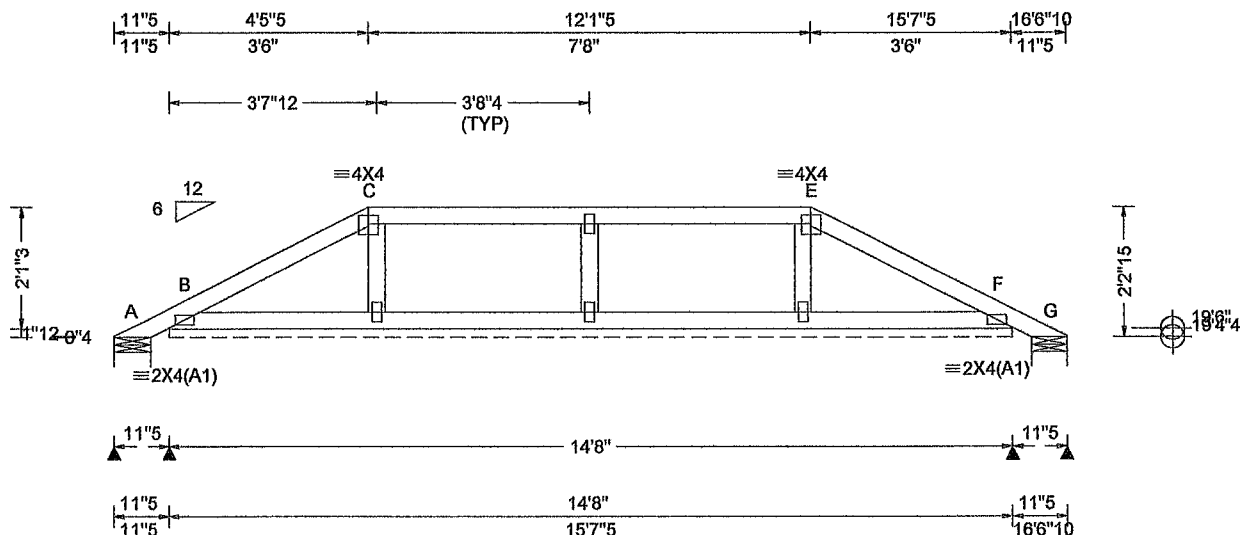


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 487792 FROM:	COMN Qty: 1	Ply: 1 Qty: 1	Job Number: 25-2400 SPEC HOUSE Truss Label: PB01	Cust: R 215 JRef 1Y8T2150001 T63 DrwNo: 093.25.0841 15547 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL 20 00 TCDL 10 00 BCLL 0 00 BCDL 10 00 Des Ld 40 00 NCBCLL 10 00 Soffit: 2.00 Load Duration 1.25 Spacing 24 0 "	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category: II EXP B Kzt: NA Mean Height: 15.64 ft TCDL 5.0 psf BCDL 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a. 3.35 ft Loc. from endwall: not in 13 00 ft GCpi: 0 18 Wind Duration 1 60	Pg NA Ct: NA CAT: NA Pf NA Ce NA Lu NA Cs NA Snow Duration NA Building Code FBC 8th Ed 2023 Res TPI Std 2014 Rep Fac: Yes FT/RT.20(0)/10(0) Plate Type(s). WAVE	PP Deflection in loc L/defl L/# VERT(LL) 0 002 B 999 240 VERT(CL) 0 003 B 999 180 HORZ(LL) 0 001 B - - HORZ(TL) 0 002 B - - Creep Factor 2 0 Max TC CSI 0 215 Max BC CSI 0 056 Max Web CSI 0 057 VIEW Ver 23 02 04.0123.13	Gravity Loc R+ / R- / Rh Non-Gravity / Rw / U / RL A - /-32 /- /24 /39 /40 B* 77 /- /- /50 /1 /- G - /-32 /- /7 /21 /- Wind reactions based on MWFRS A Brg Wid = 7.3 Min Req = 1.5 (Truss) B Brg Wid = 176 Min Req = - G Brg Wid = 7.3 Min Req = 1.5 (Truss) Bearings A, B, & G are a rigid surface Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2;
Bot chord 2x4 SP #2,
Webs. 2x4 SP #3,

Plating Notes

All plates are 2X4 except as noted

Loading

Gable end supports 8" max rake overhang Top
chord must not be cut or notched

Purlins

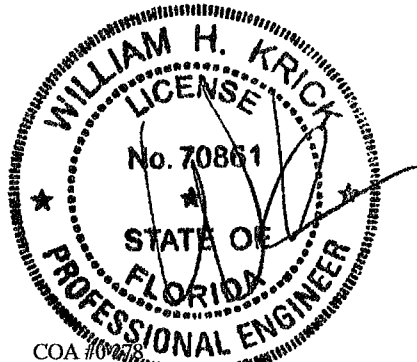
In lieu of structural panels or rigid ceiling use purlins to
brace all flat TC @ 24" oc, all BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C
member design
Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160220723 for piggyback details.



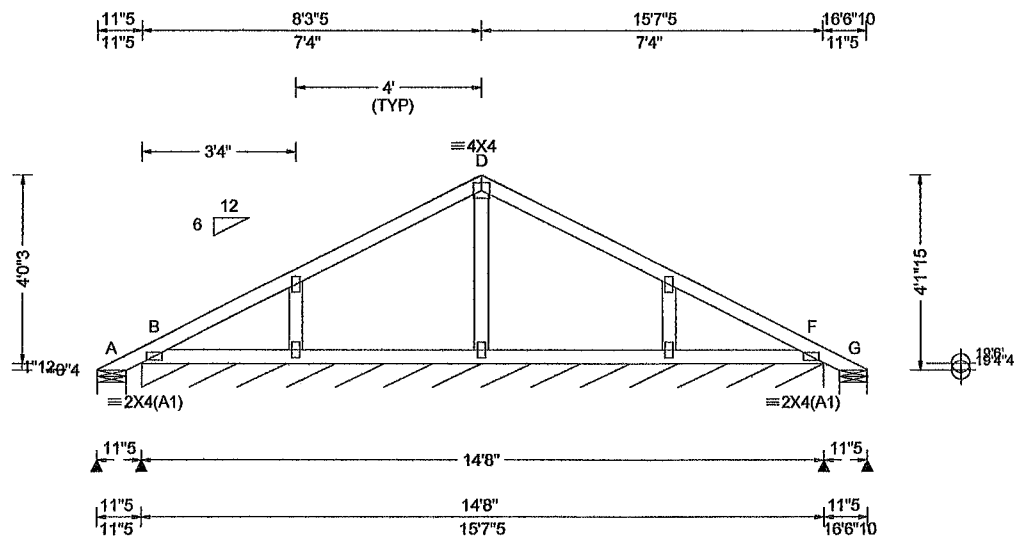
COA #00438

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 487794 FROM:	COMN Qty: 7	Job Number: 25-2400 SPEC HOUSE Truss Label: PB03	Cust: R215 JRef: 1Y8T2150001 T27 DrwNo: 093.25.0841.59937 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL 20 00 TCDL 10 00 BCLL 0 00 BCDL 10 00 Des Ld 40 00 NCBCLL 10 00 Soffit 2 00 Load Duration 1 25 Spacing 24.0 "	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category II EXP B Kzt: NA Mean Height: 21.44 ft TCDL 5 0 psf BCDL 5 0 psf MWFRS Parallel Dist 0 to h/2 C&C Dist a: 3.35 ft Loc. from endwall not in 9 00 ft GCpi 0 18 Wind Duration 1 60	Pg NA Ct: NA CAT NA Pf NA Ce NA Lu: NA Cs: NA Snow Duration NA Building Code: FBC 8th Ed 2023 Res. TPI Std 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s). WAVE	PP Deflection in loc L/defl L/# VERT(LL) 0 001 F 999 240 VERT(CL) 0 002 F 999 180 HORZ(LL) 0 000 B - - HORZ(TL) 0 001 E - - Creep Factor: 2 0 Max TC CSI 0 200 Max BC CSI 0 111 Max Web CSI 0 307 VIEW Ver: 23.02 04 0123.13	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL A 0 /-11 /- /29 /36 /70 B* 88 /- /- /47 /- /- G 0 /-11 /- /- /5 /- Wind reactions based on MWFRS A Brg Wid = 7 3 Min Req = 1.5 (Truss) B Brg Wid = 175 Min Req = - G Brg Wid = 7 3 Min Req = 1.5 (Truss) Bearings A, B, & G are a rigid surface Members not listed have forces less than 375#

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,

Plating Notes

All plates are 2X4 except as noted

Loading

Gable end supports 8" max rake overhang Top
chord must not be cut or notched

Wind

Wind loads based on MWFRS with additional C&C
member design

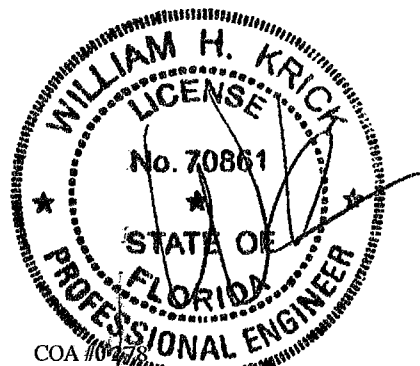
Wind loading based on both gable and hip roof types

Gable meets L/120 deflection criteria for wind load
applied to face Calculated deflection ratio is L/571

Additional Notes

Exposed portion of gable face shall be reinforced with
sheathing and the wind pressures shall be transferred
into lateral diaphragms. Connections and designs for
diaphragms is the responsibility of the Building
Designer in accordance with ANSI/TPI 1

See Detail PB160220723 for piggyback details.



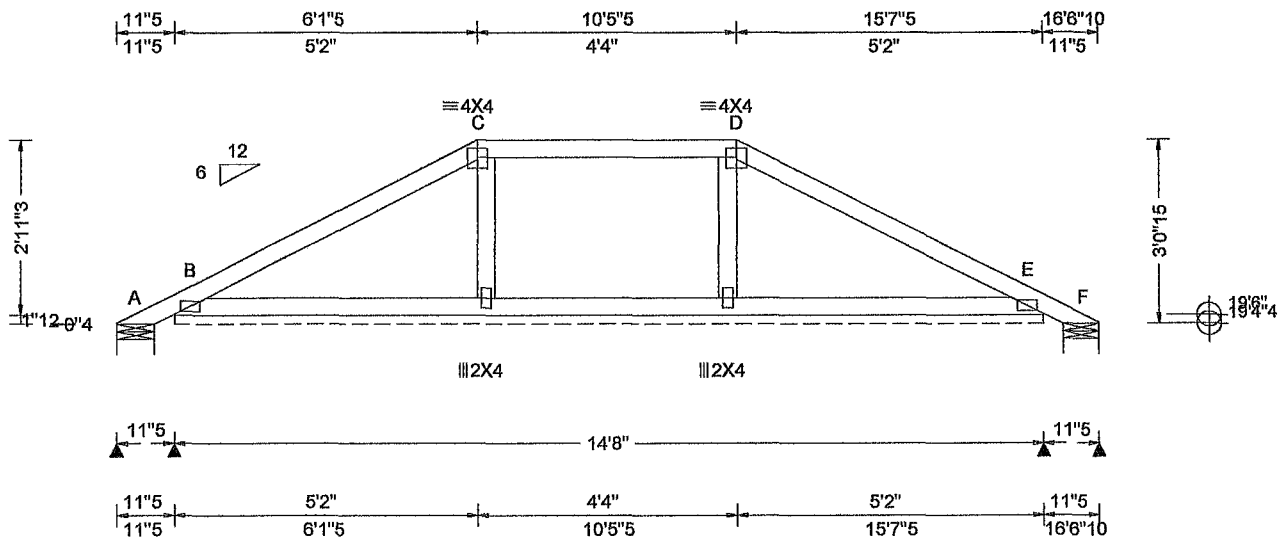
COA 10/1/2025

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155 Harlem Ave
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Glenview, IL 60025

SEQN. 487796 FROM	COMN Qty 1	Ply 1 Qty 1	Job Number: 25-2400 SPEC HOUSE Truss Label: PB05	Cust: R 215 JRef: 1Y8T2150001 T19 DrwNo: 093.25.0842.08663 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL 20 00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL 10 00	Speed 130 mph	Pf NA Ce NA	VERT(LL) 0 004 B 999 240	Loc R+ /R- /Rh /Rw /U /RL
BCLL 0 00	Enclosure: Closed	Lu NA Cs NA	VERT(CL) 0 007 B 999 180	A - /-127 /- /45 /120 /56
BCDL 10 00	Risk Category II	Snow Duration NA	HORZ(LL) 0 002 B - -	B* 87 /- /- /58 /2 /-
Des Ld 40.00	EXP B Kzt. NA		HORZ(TL) 0 004 B - -	F - /-84 /- /17 /75 /-
NCBCLL 10 00	Mean Height 16.43 ft	Building Code:	Creep Factor 2 0	Wind reactions based on MWFRS
Soffit: 2 00	TCDL 5.0 psf	FBC 8th Ed 2023 Res	Max TC CSI 0 291	A Brg Wid = 7.3 Min Req = 1.5 (Truss)
Load Duration 1.25	BCDL 5.0 psf	TPI Std 2014	Max BC CSI 0 095	B Brg Wid = 176 Min Req = -
Spacing 24 0 "	MWFRS Parallel Dist. h to 2h	Rep Fac: Yes	Max Web CSI 0 060	F Brg Wid = 7.3 Min Req = 1.5 (Truss)
	C&C Dist a. 3.35 ft	FT/RT 20(0)/10(0)		Bearings A, B, & F are a rigid surface
	Loc. from endwall not in 13 00 ft	Plate Type(s).	VIEW Ver: 23 02 04 0123 13	Members not listed have forces less than 375#
	GCpi 0 18	WAVE		
	Wind Duration: 1 60			

Lumber

Top chord 2x4 SP #2,
Bot chord 2x4 SP #2;
Webs. 2x4 SP #3,

Plating Notes

All plates are 2X4(A1) except as noted

Loading

Gable end supports 8" max rake overhang Top
chord must not be cut or notched

Purlins

In lieu of structural panels or rigid ceiling use purlins to
brace all flat TC @ 24" oc, all BC @ 24" oc.

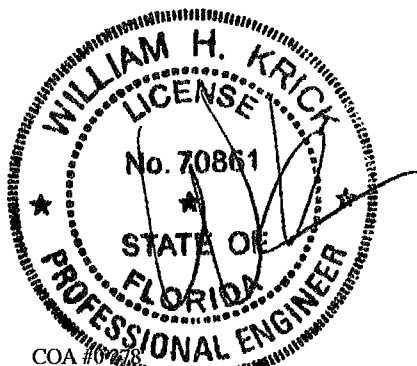
Wind

Wind loads based on MWFRS with additional C&C
member design

Wind loading based on both gable and hip roof types

Additional Notes

Refer to DWG PB160220723 for piggyback details



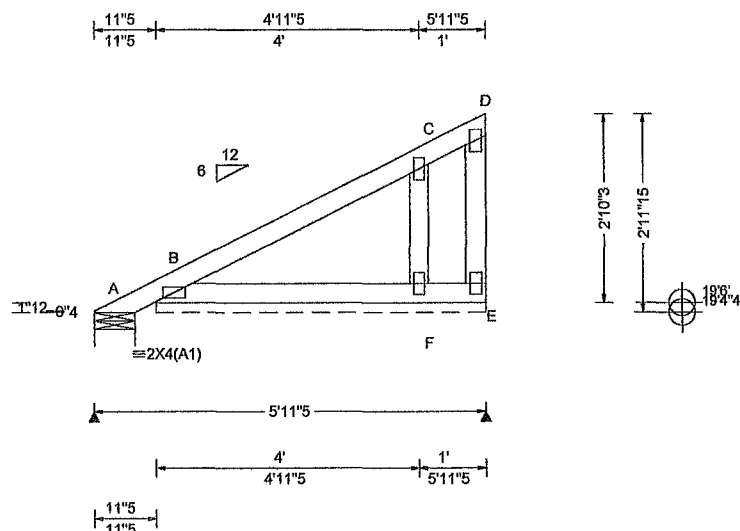
COA #0928

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 487798	MONO	Ply: 1 Qty: 3	Job Number: 25-2400 SPEC HOUSE Truss Label: PB07	Cust: R215 JRef 1Y8T2150001 T46 DrwNo: 093.25.0842.13460 AK / WHK 04/03/2025
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Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)		Defl/CSI Criteria		▲ Maximum Reactions (lbs), or *=PLF						
TCLL	20 00	Wind Std ASCE 7-22	Pg NA	Ct: NA CAT NA	PP Deflection in	loc L/defl L/#	Gravity			Non-Gravity			
TCDL	10 00	Speed: 130 mph	Pf NA	Ce NA	VERT(LL)	0 002 B 999 240	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL	0 00	Enclosure: Closed	Lu NA	Cs NA	VERT(CL)	0 003 B 999 180	A	-	/-43	/-	/27	/58	/50
BCDL	10.00	Risk Category: II	Snow Duration NA		HORZ(LL)	0 001 B - -	B*	84	/-	/-	/89	/4	/-
Des Ld	40 00	EXP B Kzt. NA			HORZ(TL)	0.002 B - -	Wind reactions based on MWFRS						
NCBCLL	10 00	Mean Height: 16.01 ft	Building Code:		Creep Factor:	2 0	A Brg Wid = 7 3 Min Req = 1.5 (Truss)						
Soffit:	2 00	TCDL 5 0 psf	FBC 8th Ed 2023 Res.		Max TC CSI	0 125	B Brg Wid = 60 0 Min Req = -						
Load Duration	1.25	BCDL 5 0 psf	TPI Std 2014		Max BC CSI	0 089	Bearings A & B are a rigid surface.						
Spacing:	24 0 "	MWFRS Parallel Dist. h to 2h	Rep Fac: Yes		Max Web CSI	0.043	Members not listed have forces less than 375#						
		C&C Dist a. 3.00 ft	FT/RT 20(0)/10(0)										
		Loc. from endwall not in 9 00 ft	Plate Type(s)										
		GCpi 0 18	WAVE		VIEW Ver:	23.02 04.0123.13							
		Wind Duration 1 60											

Lumber

Top chord. 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs. 2x4 SP #3,

Plating Notes

All plates are 2X4 except as noted

Loading

Gable end supports 8" max rake overhang Top
chord must not be cut or notched

Purlins

In lieu of rigid ceiling use purlins to brace BC @ 24"
oc.

Wind

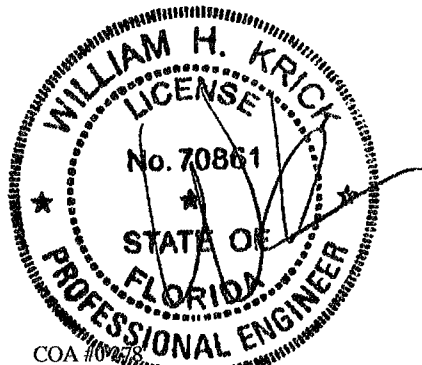
Wind loads based on MWFRS with additional C&C
member design

Right end vertical not exposed to wind pressure

Wind loading based on both gable and hip roof types

Additional Notes

Refer to DWG PB160220723 for piggyback details



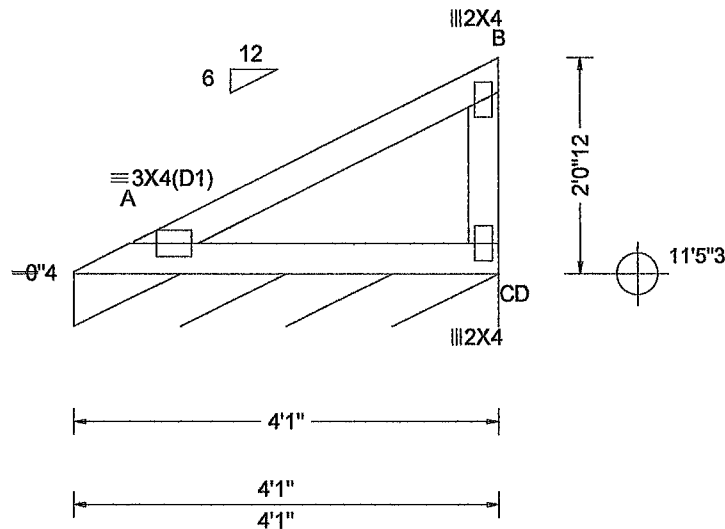
COA #0926127

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN 487800 FROM	VAL	Ply: 1 Qty: 1	Job Number: 25-2400 SPEC HOUSE Truss Label: V01	Cust: R 215 JRef 1Y8T2150001 T79 DrwNo: 093.25.0842 17547 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL 20 00 TCDL 10 00 BCLL 0 00 BCDL 10 00 Des Ld 40 00 NCBCLL 10 00 Soffit 2 00 Load Duration 1 25 Spacing 24 0 "	Wind Std ASCE 7-22 Speed 130 mph Enclosure: Closed Risk Category II EXP B Kzt NA Mean Height 15.00 ft TCDL 5 0 psf BCDL 5 0 psf MWFRS Parallel Dist. h/2 to h C&C Dist a: 3.00 ft Loc. from endwall. not in 9.00 ft GCpi 0 18 Wind Duration 1 60	Pg NA Ct: NA CAT: NA Pf NA Ce: NA Lu NA Cs. NA Snow Duration NA Building Code: FBC 8th Ed 2023 Res. TPI Std 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s). WAVE	PP Deflection in loc L/defl L/# VERT(LL) NA VERT(CL) NA HORZ(LL) 0 003 A - - HORZ(TL) 0 006 A - - Creep Factor 2.0 Max TC CSI 0.198 Max BC CSI 0.157 Max Web CSI 0.056 VIEW Ver: 23 02 04 0123 13	Gravity Loc R+ /R- /Rh /Rw /U /RL Non-Gravity D* 82 /- /- /47 /0 /10 Wind reactions based on MWFRS D Brg Wid = 49 0 Min Req = - Bearing A is a rigid surface Members not listed have forces less than 375#

Lumber

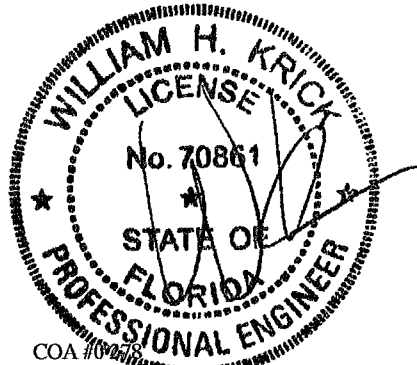
Top chord: 2x4 SP #2,
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3,

Wind

Wind loads based on MWFRS with additional C&C member design
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



COA #00018

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[illegible]

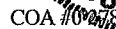
Lumber	
Top chord 2x4 SP #2,	
Bot chord 2x4 SP #2;	
Webs. 2x4 SP #3,	

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind loads based on MWFRS with additional C&C member design

Additional Notes

valley details.



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Attached rigid ceiling locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to Job's General Notes page for additional information.

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North Building, 4th Floor
Glenview, IL 60025

Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)		Defl/CSI Criteria		▲ Maximum Reactions (lbs), or * = PLF						
TCLL	20 00	Wind Std	ASCE 7-22	Pg NA	Ct: NA CAT: NA	PP Deflection in	loc L/defl L/#	Gravity			Non-Gravity			
TCDL	10 00	Speed	130 mph	Pf NA	Ce NA	VERT(LL) NA		Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL	0 00	Enclosure:	Closed	Lu NA	Cs. NA	VERT(CL) NA		D*	82	/-	/-	/44	/-	/9
BCDL	10 00	Risk Category:	II	Snow Duration NA		HORZ(LL) 0.001 A	- -	Wind reactions based on MWFRS						
		EXP B Kzt:	NA			HORZ(TL) 0.002 A	- -	D Brg Wid = 36.0 Min Req = -						
Des Ld	40.00	Mean Height:	15.00 ft			Creep Factor:	2.0	Bearing A is a rigid surface						
NCBCLL	10 00	TCDL	5 0 psf	Building Code		Max TC CSI	0.097	Members not listed have forces less than 375#						
Soffit	2 00	BCDL	5.0 psf	FBC 8th Ed	2023 Res	Max BC CSI	0.087							
Load Duration	1 25	MWFRS Parallel Dist. h/2 to h		TPI Std	2014	Max Web CSI	0.030							
Spacing	24 0 "	C&C Dist a: 3 00 ft		Rep Fac:	Yes									
		Loc. from endwall, not in 9 00 ft		FT/RT:	20(0)/10(0)									
		GCpi 0 18		Plate Type(s)										
		Wind Duration	1 60	WAVE		VIEW Ver 23.02 04.0123 13								

Lumber

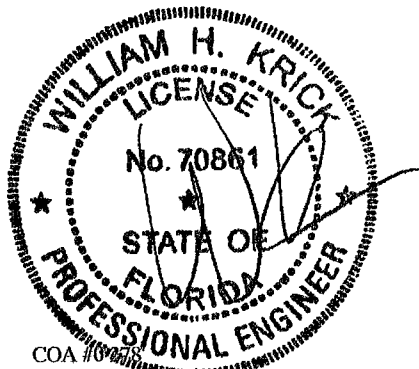
Top chord- 2x4 SP #2;
Bot chord. 2x4 SP #2,
Webs. 2x4 SP #3.

Wind

Wind loads based on MWFRS with additional C&C member design
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

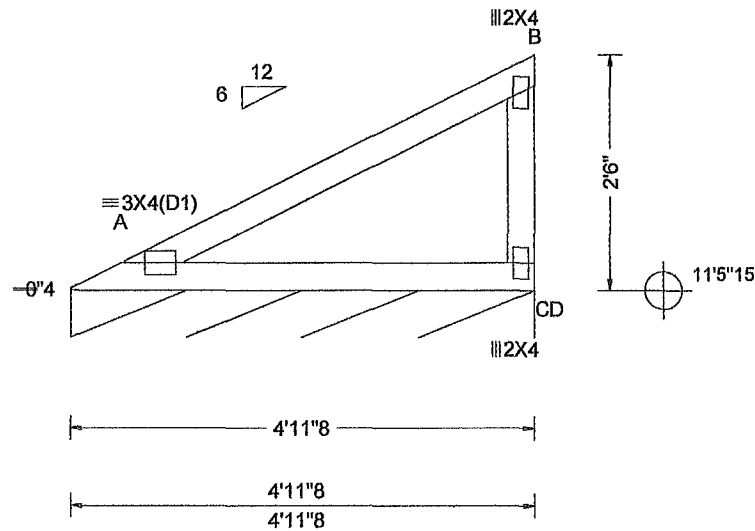


COA #0278

Florida Certificate of Product Approval #FL 1999

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety) information, by TPI and SBCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI, unless otherwise, top chord plates, purlins, and bottom chord plates shall have a properly installed rigid ceiling. Lateral restraint (LR) and permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to Job's General Notes page for additional information.
 Alpine, a division of ITW Building Components Group Inc, shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSIT/UP1 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSIT/UP1 1 Sec.2
 For more information see these web sites: Alpine: alpineitw.com, TPI: tpiinst.org, SBCA: sbcacomponents.com, ICC: iccsafe.org, AWC: awc.org

SEQN: 487806	VAL	Ply: 1	Job Number: 25-2400	Cust: R 215 JRef 1Y8T2150001 T56
FROM:		Qty: 1	SPEC HOUSE	DrwNo: 093.25.0842.28270
			Truss Label: V07	AK / WHK 04/03/2025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL 20 00	Wind Std ASCE 7-22	Pg NA Ct NA CAT NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL 10 00	Speed 130 mph	Pf NA Ce NA	VERT(LL) NA	Loc R+ / R- / Rh / Rw / U / RL
BCLL 0 00	Enclosure: Closed	Lu NA Cs NA	VERT(CL) NA	D* 82 /- /- /47 /- /8
BCDL 10 00	Risk Category: II	Snow Duration NA	HORZ(LL) 0 006 A - -	Wind reactions based on MWFRS
Des Ld 40 00	EXP B Kzt: NA		HORZ(TL) 0 012 A - -	D Brg Wid = 59 5 Min Req = -
NCBCLL 10 00	Mean Height 15 00 ft	Building Code.	Creep Factor: 2.0	Bearing A is a rigid surface
Soffit 2 00	TCDL 5.0 psf	FBC 8th Ed 2023 Res.	Max TC CSI 0.294	Members not listed have forces less than 375#
Load Duration 1.25	BCDL 5.0 psf	TPI Std 2014	Max BC CSI 0.277	
Spacing 24 0 "	MWFRS Parallel Dist: h to 2h	Rep Fac. Yes	Max Web CSI 0.079	
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	VIEW Ver: 23.02 04.0123 13	
	Loc. from endwall not in 9 00 ft	Plate Type(s)		
	GCpi 0 18	WAVE		
	Wind Duration: 1 60			

Lumber

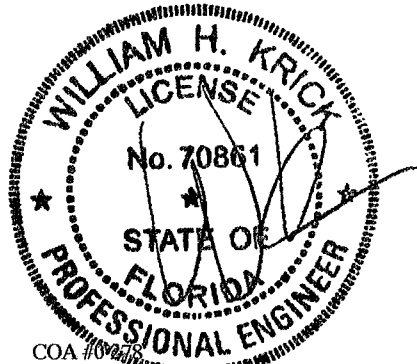
Top chord. 2x4 SP #2;
Bot chord 2x4 SP #2,
Webs. 2x4 SP #3,

Wind

Wind loads based on MWFRS with additional C&C member design
Right end vertical not exposed to wind pressure
Wind loading based on both gable and hip roof types

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details



COA #09478

Florida Certificate of Product Approval #FL 1999

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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2
For more information see these web sites. Alpine: alpineitw.com, TPI tpinst.org; SBCA: sbacompcomponents.com, ICC: iccsafe.org, AWC: awc.org

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Glenview, IL 60025

12
6

2X4

3X4(D1)
A

0'4

2X4

2X4

2'1"

2'1"

2'1"

1'0"12"

14'5"3

Lumber	
Top chord 2x4 SP #2;	
Bot chord 2x4 SP #2,	
Webs 2x4 SP #3,	

Wind loads based on MWFRS with additional C&C member design
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

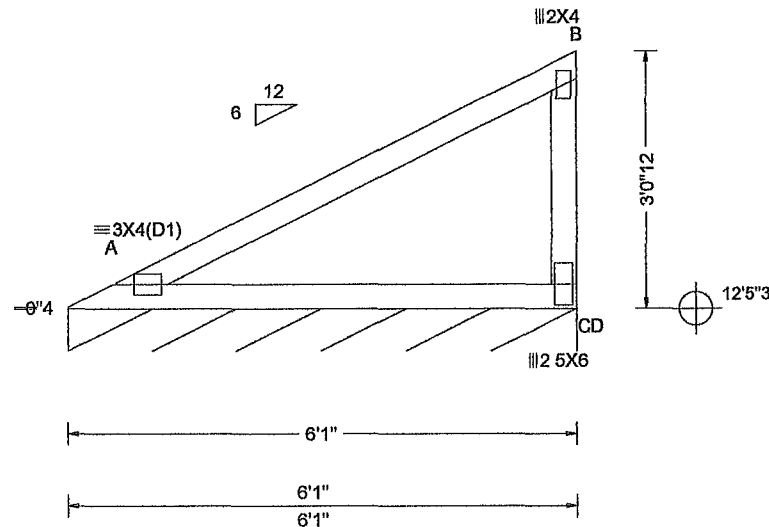
See DWGS VALTN220723 and VAL180220723 for valley details.



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 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety) Information for TPI and (BCSI) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI's Uniform Code of Practice (UCP) for all trusses. Heating and bottom chord shall have a properly installed rigid ceiling. Location for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
 Alpine, a division of ITW Building Components Group Inc, shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2
 For more information see these web sites. Alpine, alpinetw.com. TPI tpinst.org. SBCA sbcacomponents.com. ICC iccsafe.org. AWC awc.org

SEQN: 487810 FROM	VAL	Ply: 1 Qty: 1	Job Number: 25-2400 SPEC HOUSE Truss Label: V11	Cust: R 215 JRef: 1Y8T2150001 T24 DrwNo: 083.25.0842.34110 AK / WHK 04/03/2025
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL 20 00 TCDL 10 00 BCLL 0 00 BCDL 10 00 Des Ld 40 00 NCBCLL 10 00 Soffit 2.00 Load Duration 1 25 Spacing 24 0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt. NA Mean Height: 15.00 ft TCDL 5 0 psf BCDL 5 0 psf MWFRS Parallel Dist. h to 2h C&C Dist a: 3 00 ft Loc. from endwall not in 9 00 ft GCpl 0.18 Wind Duration 1 60	Pg NA Ct: NA CAT: NA Pf NA Ce. NA Lu NA Cs. NA Snow Duration NA Building Code: FBC 8th Ed 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT.20(0)/10(0) Plate Type(s). WAVE	PP Deflection in loc L/defl L/# VERT(LL) NA VERT(CL) NA HORZ(LL) 0.011 A - - HORZ(TL) 0.022 A - - Creep Factor 2.0 Max TC CSI 0.470 Max BC CSI 0.401 Max Web CSI 0.154 VIEW Ver: 23.02 04 0123 13	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity D* 82 /- /- /48 /- /8 Wind reactions based on MWFRS D Brg Wid = 73.0 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

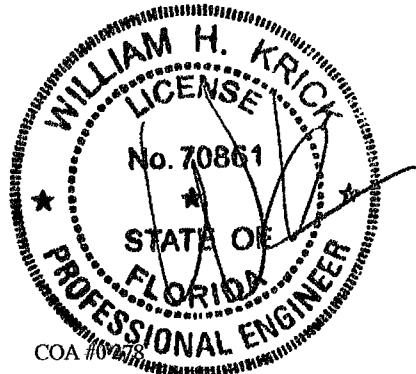
Top chord 2x4 SP #2,
Bot chord 2x4 SP #2,
Webs 2x4 SP #3,

Wind

Wind loads based on MWFRS with additional C&C member design
Right end vertical not exposed to wind pressure
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



COA #09678

Florida Certificate of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites Alpine: alpineitw.com, TPI: tpinst.org, SBCA: sbccomponents.com, ICC: iccsafe.org, AWC: awc.org

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Glenview, IL 60025

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

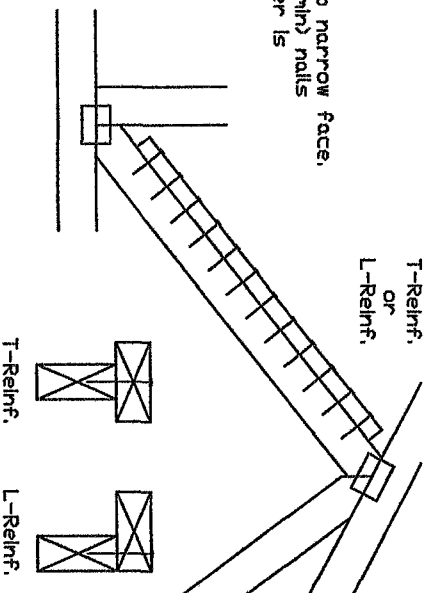
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

Center scab on wide face of web. Apply (1) scab to each face of web.

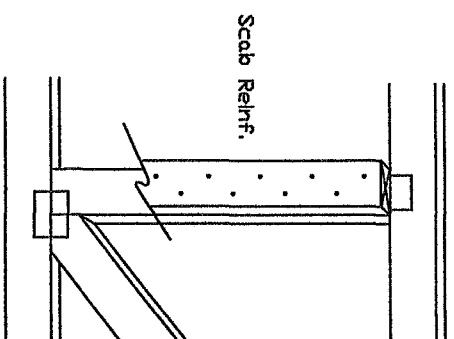
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scabs to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Before erecting, the fabricator shall follow the latest edition of BCSI Guiding Design Safety Information by TPI and BCSI for proper practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached roof ceiling. Locations shown for permanent internal restraint of web shall have bracing installed per BCSI sections 22, 27 or 28, as applicable. Apply plates to each end of the web member. Refer to drawings 10d-2 for standard plate details, unless noted otherwise.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in accordance with ANSI/TPI 1 or for handling, shipping, installation or bracing.

A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering by the State of Florida. The seal shall be used in accordance with the rules of the State of Florida. For more information, see the Florida Building Code, Part 9, Chapter 9, Section 901.2.1.1.

ALPINE BUILDING COMPONENTS GROUP INC. 175 Westchester Blvd. Westchester, NY 10598

PROFESSIONAL ENGINEER
STATE OF FLORIDA
No. 70861
WILLIAM H. KRICK
LICENSE

TC LL	PSF	REF	CLR Subst.
TC DL	PSF	DATE	01/02/19
BC DL	PSF	DRWG	BRCLBSUB0119
BC LL	PSF		
TDI, L.D.	PSF		

DLR 03/20/05
COA #0 278
Florida Certificate Approval #FL 1999



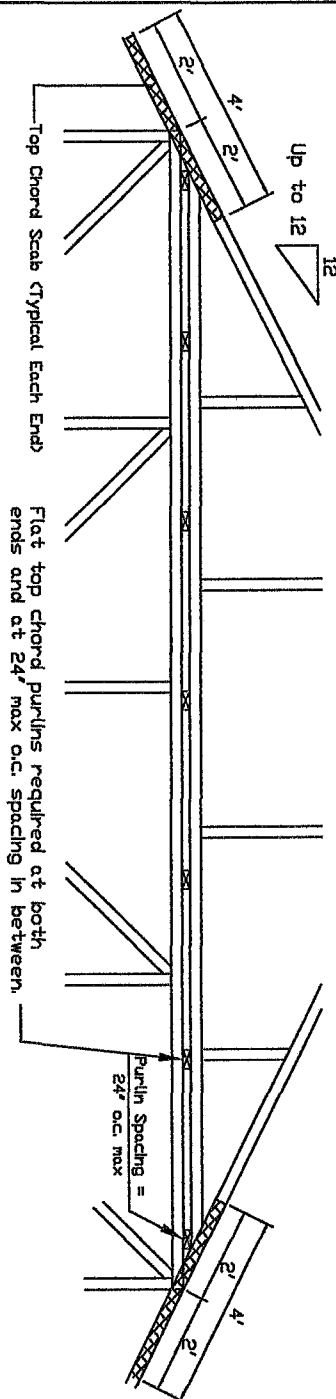
Piggyback Detail - ASCE 7-22: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-22, Enclosed Bldg, located anywhere in roof, Exp C, Wind III = 5.0 psf (min), Kzt=1.00, Dp 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-22, Enclosed Bldg, located anywhere in roof, Exp D, Wind III = 5.0 psf (min), Kzt=1.00.

Notes: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building designer shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends. Maximum truss spacing is 24' o.c. Detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

*** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

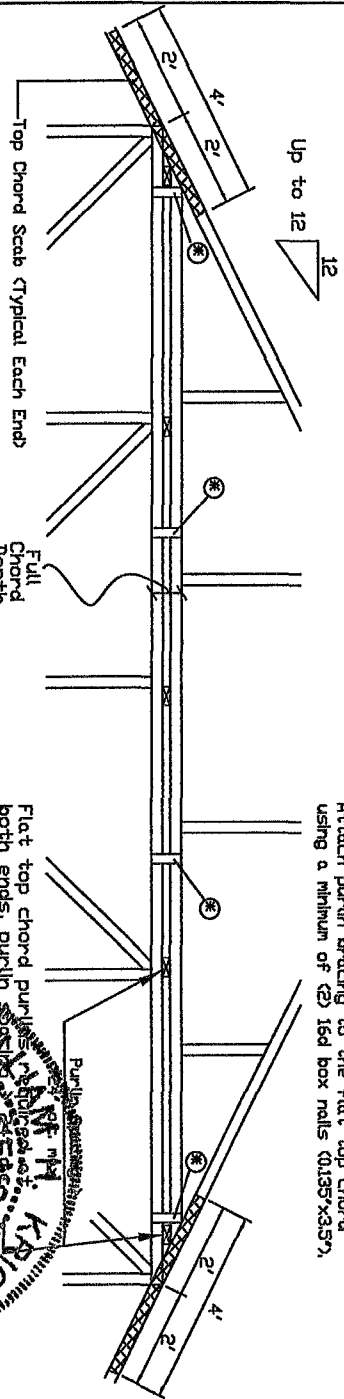
Detail A : Purlin Spacing = 24" o.c. or less



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c. Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3x8 Trulux plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate attached to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

Note: If purlins or sheathing are not specified on the flat top of the truss, purlins must be installed at 24' o.c. max and use Detail A.

* In addition, provide connection with one of the following methods:
Trulux: Use 3x8 Trulux plates for 2x4 chord member, and 3x10 Trulux plates for 2x6 and larger chord members. Attach to each face & 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulux plates may be staggered 4' o.c. front to back faces. APA Rated Gussset: 8"x8"x7/16" (min) APA rated sheathing gusssets (each face). Attach & 8' o.c. with (8) 5d common (0.113"x2") nails per gussset, (4) in cap bottom chord and (4) in base truss top chord. Gusssets may be staggered 4' o.c. front to back faces. 2x4 Vertical Scabs: 2x4 SPF #2, full chord depth scabs (each face). Attach & 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces. 28PB Wave Piggyback Plate: One 28PB wave piggyback plate to each face & 8' o.c. Attach teeth to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

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No. 70861

STATE OF FLORIDA

PROFESSIONAL ENGINEER

04/03/2025

REF PIGGYBACK

DATE 07/03/2023

DRWG PB160220723



155 Harten Ave
North Building 4th Floor
Glenview, IL 60025

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCS Guiding Component Safety Information, by ITW and SCSA for more information. Unless noted otherwise, top chord shall have properly attached structural sheathing and fasteners shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint for wind or truss and position as shown above and on the detail details, unless noted otherwise. Refer to drawings of truss for standard plate positions. A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the building designer per ASCE 7-22, Sec. 2. For more information see this job's general notes page and these web sites: ALPINE: www.alpine.com ITW: www.itw.com SCSA: www.scsa.com

COA #0 278
Florida Certified Professional Engineer
04/03/2025
APPENDIX 10 #FL 1999

Valley Detail - ASCE 7-22: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

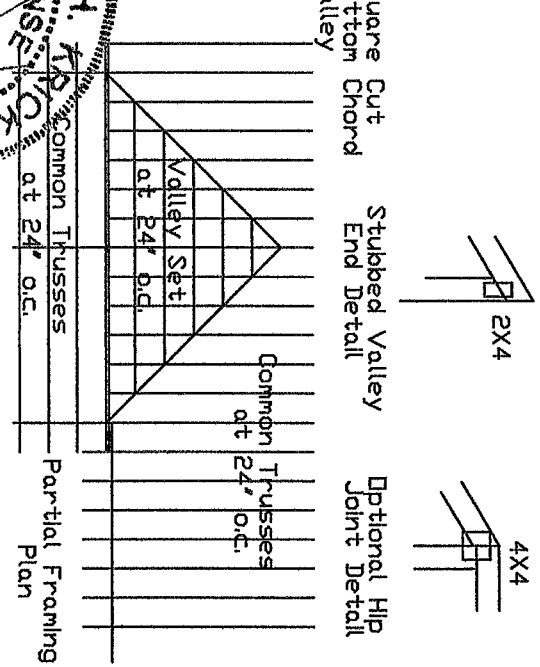
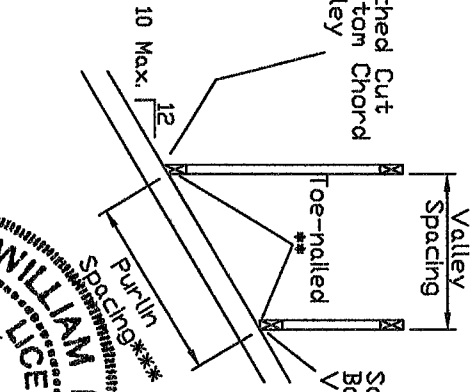
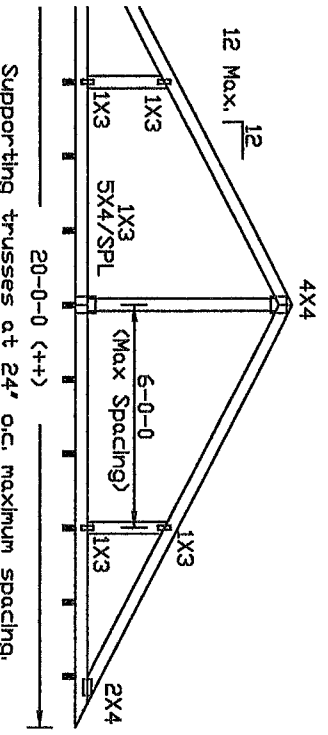
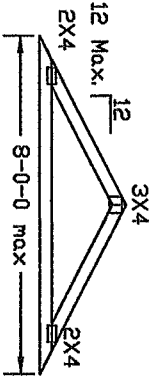
** Attach each valley to every supporting truss with
(2) 16d box (0.135' x 3.5') nails toe-nailed for
ASCE 7-22, 30' Mean Height, Enclosed Building, Exp. C,
Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
supporting truss material at connection location:
140 mph for SP (G = 0.55, min.), or
125 mph for DF-L (G = 0.50, min.), or
105 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses
below valley trusses.

Bottom chord of valley trusses may be square or
pitched cut as shown.

Valleys short enough to be cut as solid triangular
members from a single 2x6, or larger as required,
shall be permitted in lieu of fabricating from
separate 2x4 members.

All plates shown are Alpine Wave Plates.

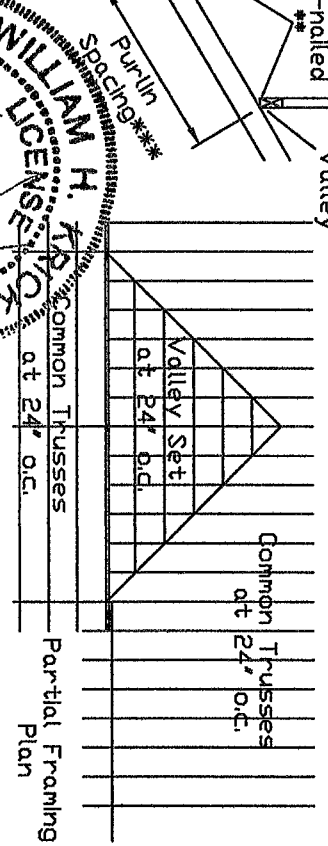
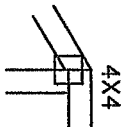
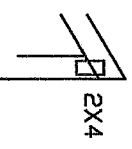


Unless specified otherwise on engineer's sealed design, for vertical
valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of
web, same species and grade or better, attached with 10d box
(0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous
Lateral Restraint applied at mid-length of web is permitted with diagonal
bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with
properly attached, rated sheathing applied prior to valley truss
installation.

Or
Purlins at 24' o.c. or as otherwise specified on engineer's sealed design
By valley trusses used in lieu of purlin spacing as specified on
Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss
beneath the valley is measured along the slope of the top chord.
+ Larger spans may be built as long as the vertical height does
not exceed 14'-0".



155 Haden Ave
North Building, 4th Floor
Glenview, IL 60025

WILLIAM H. HARRIS
PROFESSIONAL ENGINEER
FLORIDA
No. 70861

TC LL		30		40PSF		REF	
TC DL		20		7PSF		DATE 07/03/2023	
BC DL		10		10 PSF		DRWG VALTN20723	
BC LL		0		0 PSF			
TOT. L.D.		60		55 57PSF			
DATE 07/03/2023				COA #0278			
DRG. AC. 1.55/1.33 1.15 1.15				Florida Certified			
DATE OF PROJECT APPROVAL 11-1999							
SPACING 24.0"							