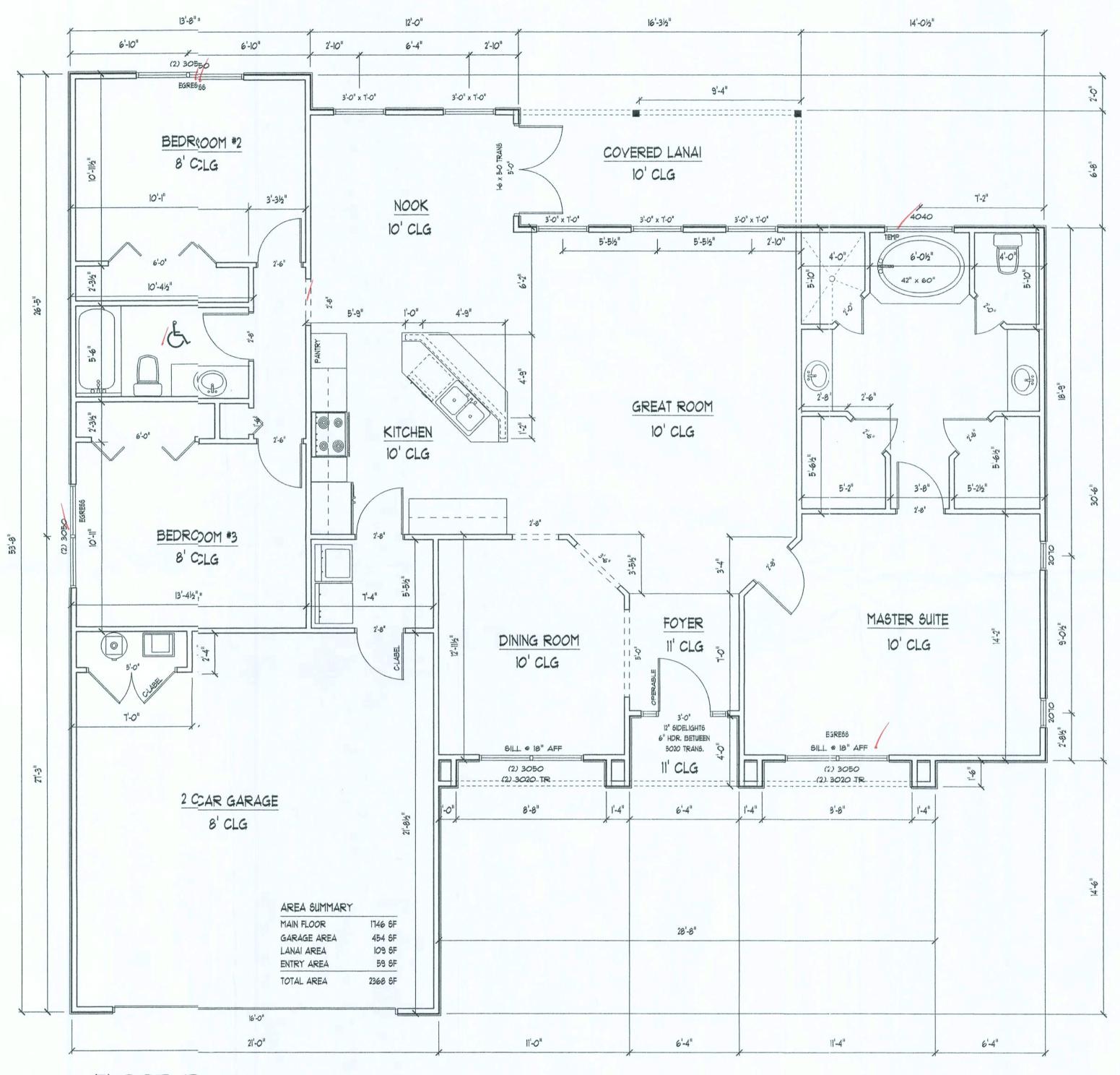




SCALE: 1/4" := 1'



Daniel Shaheen

July 13, 2006 ARCHITECTURAL © DESIGN P.O. BOX 273 LAKE CITY FL. 32055 (386) 754-0181 COPYRIGHTED BY: ENGINEERED BY: CREEK ANNON DESIGNED FOR: SHEET NUMBER 2 of 3 All work shall comply with the standard building code, and all applicable local codes and ordinances.

> Contractor shall verify all dimensions prior to commencing construction.

PROJECT NUMBER

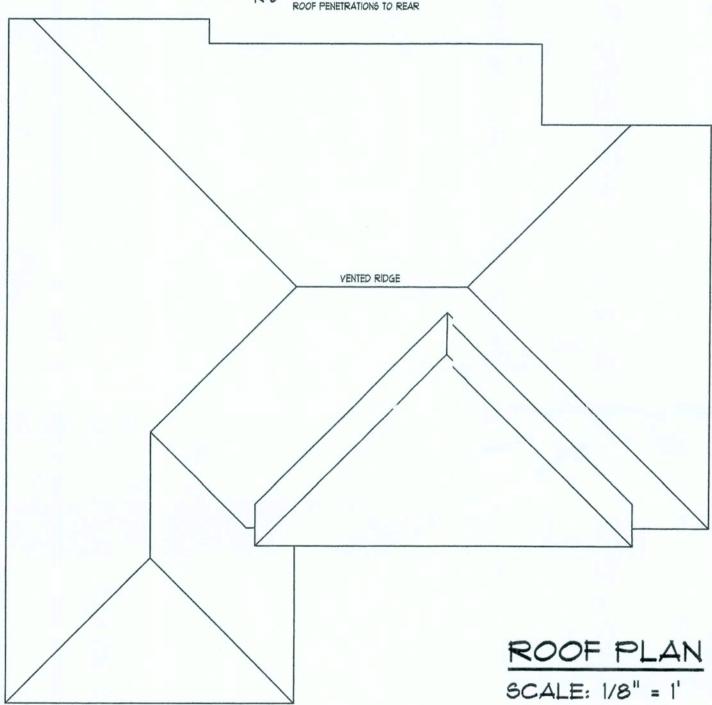
801010

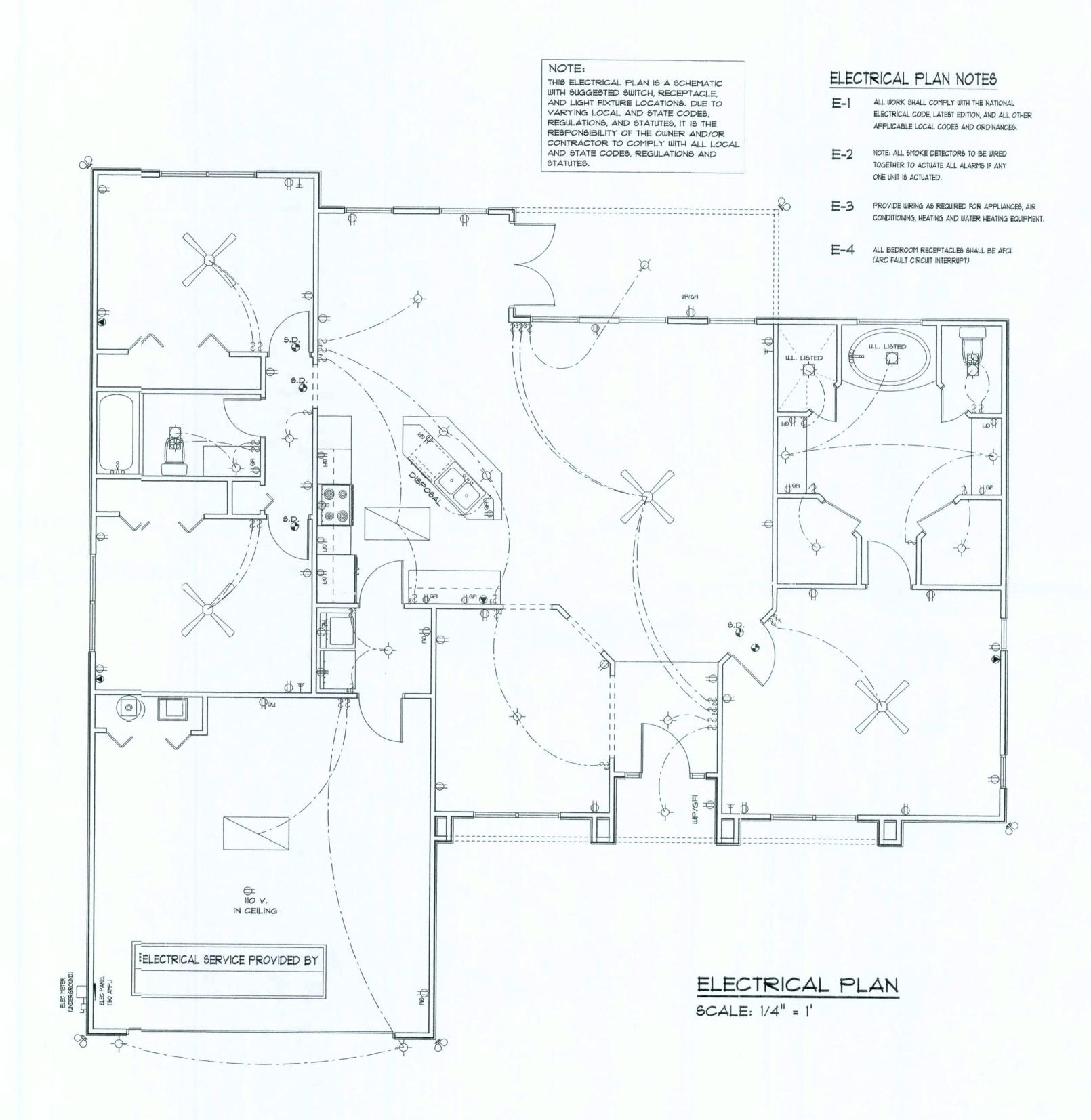
ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

SCALE: 1" = 1'0"



- R-I ALL ROOF PITCH 6/12 UNLESS OTHERWIS NOTED
- R-2 ALL OVERHANG 18" AND 12" AT GABLES UNLESS OTHERWISE NOTED
- R-3 PROVIDE ATTIC VENTILATION IN AC-CORDANCE WITH CODE REQUIREMENTS
- R-4 6EE EXTERIOR ELEVATIONS AND FLOOF PLANS TO VERIFY PLATE AND HEEL HEIHTS
- R-5 MOVE ALL VENTS AND OTHER ROOF PENETRATIONS TO REAR





July 13, 2006

D.D.S. Studioe

ARCHITECTURAL

© **DESIGN**P.O. BOX 273
LAKE CITY FL. 32056
(386) 754-D181

COPYRIGHTED EY:

AARK DISOSWAY, P.E.
POB 868, LAKE CITY, FL 32056
Ph: (904) 754-5419
Ph: (904) 754-6749
FLORIDA P.E. 53915

ENGINEERED BY:

CANNON CREEK, LOT 12

DESIGNED FOR:

ELECTRICAL PLAN
ROOF PLAN

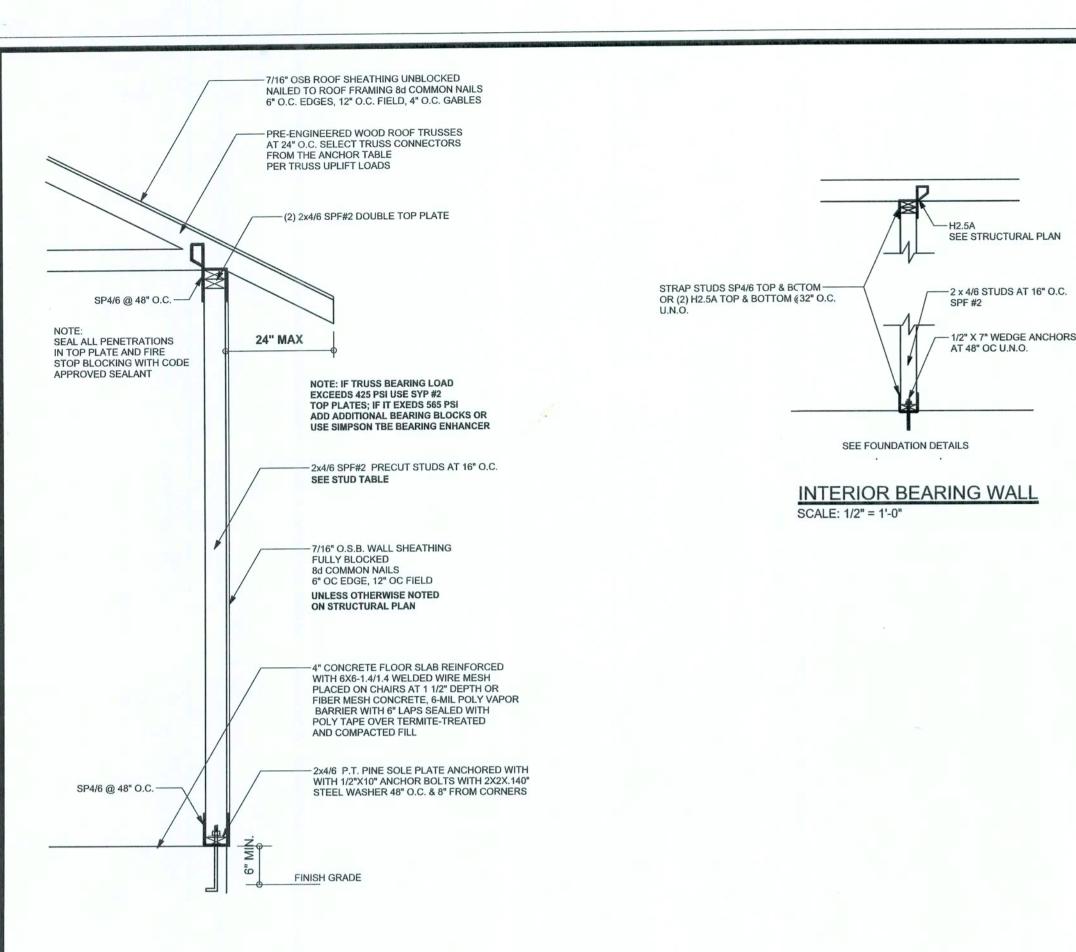
3 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances.

Contractor shall verify all dimensions prior to commencing construction

PROJECT NUMBER

Daniel Shaheen



ONE STORY WALL SECTION

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC TO 11'-9" STUD HEIGHT

(1) 2x4 @ 12" OC TO 13'-0" STUD HEIGHT

(1) 2x6 @ 16" OC TO 18'-10' STUD HEIGHT

(1) 2x6 @ 12" OC TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B,

STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

SIMPSON H2.5A U.N.O.-

(2) SIMPSON LSTA21-

w/ (8) -16d TO HEADER

AND (8) -16d TO POST

SEE STRUCTURAL PLAN

EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS

RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B.

LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.

(2) 2X10 SYP #2 U.N.O.

-6X6 SYP #2 POST

SEE STRUCTURAL PLAN

-SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

SEE FOOTING DETAILS

ANCHOR BOLT

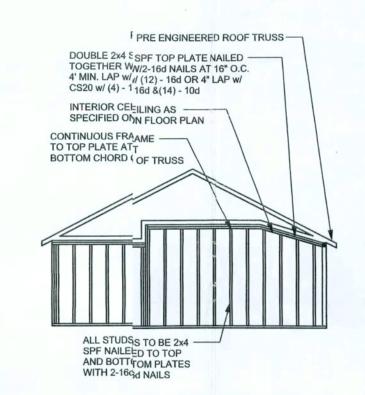
-(2) 2X4 SPF #2 TOP PLATE (2) SIMPSON SPH4 w/ (6) - 10d ----SIMPSON | SP4 @ 48" O.C. (2) SIMPSON LSTA21w/ (8) -16d TO HEADER (2) 2X12 SYP #2 HEADEFR U.N.O AND (8) -16d TO STUD PACK SEE STRUCTURAL PLANN (2) JACKS STUDS (2) KINGS STUDS w/ (2) ROWS 10d @ w/ (2) ROWS 10d @ 12" O.C. EACH SIDE 12" O.C. EACH SIDE SIMPSON LTTI31 w/ (18) - 10d & 5/8" x 10" ANCHOR BOLT FOUNDATION SEE SEE FOOTING DETAILS

1900 05/190

TYPICAL GARAGE DOOR HEADER STRAPING LOETAIL

GRAIDE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO
CEILING DIAPHRAGM DETAIL
SCALE: N.T.S.3

-NAIL SHEATHING TO HEADER AND TOP

PLATE WITH 8d AT 4" O.C. FOR UPLIFT

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

DESIGN DATA

ANCHOR TABLE

< 420

< 455

< 360

< 455

< 600

< 950

< 745

< 1465

< 1465

< 990

< 760

< 1470

< 1470

< 1000

< 1450

< 2900

< 2050

< 3965

< 10980

< 10530

< 9250

< 455

< 825

< 825

< 885

< 1240

< 885

< 1240

< 1235

< 1235

< 1030

< 1350

< 2310

< 2775

< 4175

< 1400

< 3335

< 2200

< 2300

< 2320

< 1705

MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP UPLIFT LBS. SPF

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

< 245

< 265

< 235

< 320

< 535

< 820

< 565

< 1050

< 1050

< 850

< 655

< 1265

< 1265

< 860

< 2490

< 1785

< 6485

< 9035

< 9250

< 420

< 825

< 600

< 1065

< 1065

< 1165

< 1235

< 1030

< 1705

< 1305

< 2310

< 2570

< 3695

< 3335

< 2200

< 2300

< 2320

TRUSS CONNECTOR*

H5A

H2.5

H2.5A

H14-1

H10-1

H10-2

H16-1

H16-2

MTS24C

HTS24

2 - HTS24

LGT2

HEAVY GIRDER TIEDOWNS

MGT

HGT-2

HGT-3

HGT-4

STUD STRAP CONNECTOR*

SSP SINGLE SILL PLATE

DSP DOUBLE TOP PLATE

DSP SINGLE SILL PLATE

SPH4

SP6

LSTA18

LSTA21

CS20

STUD ANCHORS*

LTT19

HTT16

PAHD42

ABU44

ABU66

ABU88

SSP DOUBLE TOP PLATE

TO PLATES TO RAFTER/TRUSS

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

12-8d, 1 1/2"

12-8d, 1 1/2"

8-8d, 1 1/2"

6-10d

2-10d, 1 1/2"

2-10d, 1 1/2"

7-10d 1 1/2"

12-10d 1 1/2"

16 -10d

3-8d

4-8d

4-8d

4-8d

5-8d

5-8d

5-10d, 1 1/2"

13-8d

15-8d

8-8d, 1 1/2"

6-10d

10-10d, 1 1/2"

10-10d, 1 1/2"

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

3 -10d

1 -10d

6 -10d

2-10d

14-10d

16-10d

18-8d

TO STUDS

8-16d

18-10d, 1 1/2"

2-5/8" BOLTS

18 - 16d

16-16d

16-16d

12-16d

12-16d

18 - 16d

TO STUDS

TO FOUNDATION

1-5/8" THREADED ROD

12" EMBEDMENT

2-5/8" THREADED ROD

12" EMBEDMENT

2-5/8" THREADED ROD

2-5/8" THREADED ROI

12" EMBEDMENT

12" EMBEDMENT

TO STUDS

4 -10d

4-10d

8 -10d

8 -10d

6-10d, 1 1/2"

10-10d, 1 1/2"

6-10d, 1 1/2"

10-10d, 1 1/2"

TO FOUNDATION

1/2" AB

1/2" AB

5/8" AB

5/8" AB

1/2" AB

2-5/8" AB

WIND L	OADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
MEAN F	OSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT PER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.
BUILDIN	NG IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDIN	NG IS NOT IN THE WIND-BORNE DEBRIS REGION

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH
2.) WIND EXPOSURE = B

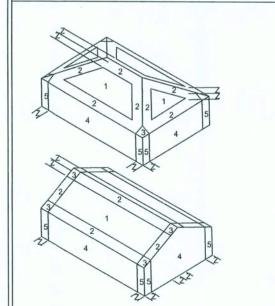
3.) WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II
5.) ROOF ANGLE = 10-45 DEG

5.) ROOF ANGLE = 10-45 DEGREES6.) MEAN ROOF HEIGHT = <30 FT

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Z.UHE	Lilico		iid Aid	ובוון הי	
	1	0	1	100	
1	19.9	-21.8	18.1	-18.1	
2	19.9	-25.5	18.1	-21.8	ı
2 O'hg		-40.6		-40.6	
3	19.9	-25.5	18.1	-21.8	
3 O'hg		-68.3		-42.4	
4	21.8	-23.6	18.5	-20.4	1
5	21.8	-29.1	18.5	-22.6	1
Doors	& Wind	awob	21.8	-29.1	
Wor	st Cas	е			
(Zone	5, 10	ft2)			ĺ
8x7 Gar	age D	oor	19.5	-22.9	
16x7 Ga	rage [Door	18.5	-21.0	
					١
					1
				-	

SIGN	LOADS	_
OR	40 PSF (ALL OTHER DWELLING ROOMS)	_
	30 PSF (SLEEPING ROOMS)	_

30 PSF (SLEEPING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

ROOF 20 PSF (FLAT OR <4:12)

16 PSF (4:12 TO <12:12)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

SOFTPIXN ARCHITECTURAL DEED SOFTWARE

CERTIFICATION: I hereby certify that I have examined this plan, and tlat the applicable portions of the plan, relating to wind engineeric comply with section R3012.1, florida building code residential 2004, to he best of my knowledge.

LIMITATION: This designis valid for one building, at specified localon.

MARK DISISSWAY
P.E. 53915

SEA

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, lake City, FL

Mark Disosway, P.E. hereby expressly reserves ts common law copyright and property right in

nese instruments of service. This document is

not to be reproduced, alteed or copied in any

form or manner without fist the express written

tated dimensions superede scaled

dimensions. Refer all questions to Mark Disosway, P.E. for esolution.

Do not proceed without chrification.

COPYRIGHTS AND PROPERTY RIGHTS

Petersen Construction

Spec Fouse Lot 12 Cannon Creek S/D

ADDRESS: Lot 12 Cannoi Creek S/D Columbia County, Florida

Mark Disosway P.E. P.O. Bcx 868 Lake City, Florida 32056 Phone: (386)754 - 5419 Fax: (386) 269 - 4871

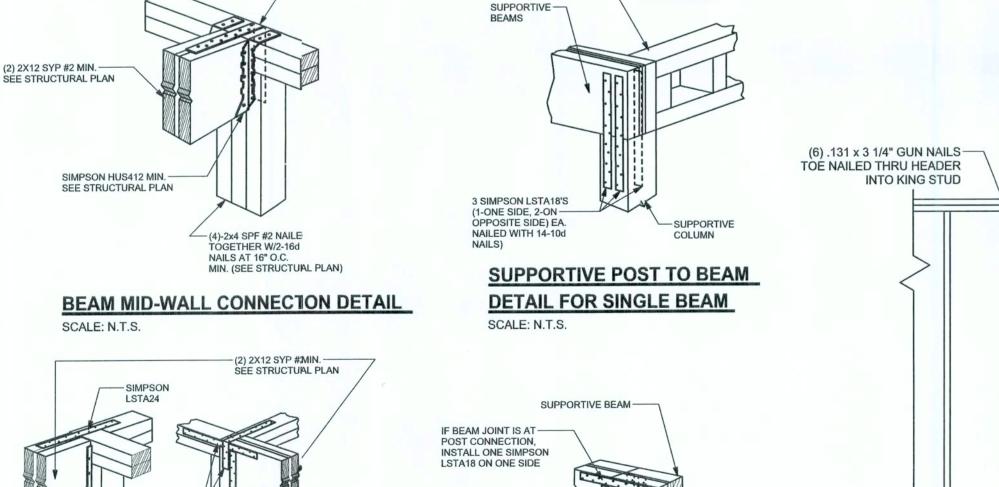
PRINTEDDATE:
July 21, 206

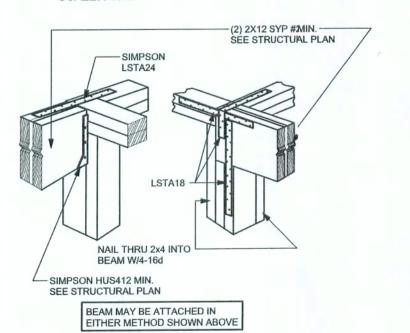
DRAWN BY: CHECKED BY:
David Disosway

FINALS DATE: 19 / Jul / 06

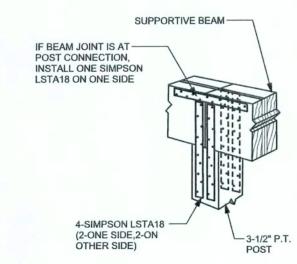
JOB NUMBER: 607'45 DRAWING NUMBER

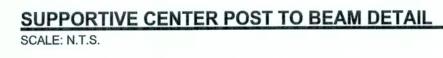
OF 3 SHEETS





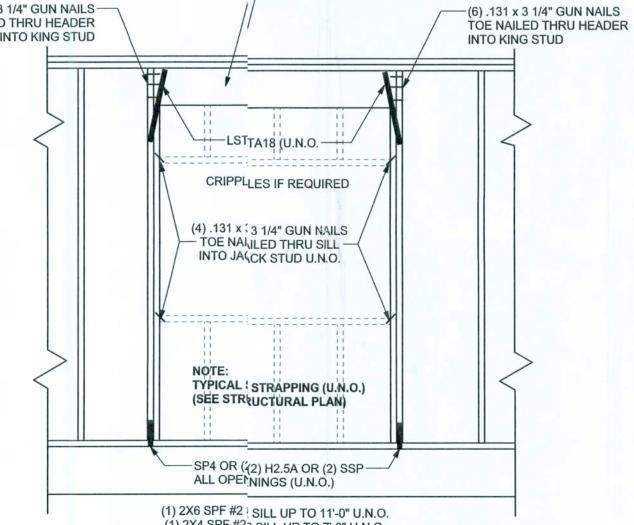
BEAM CORNER CONNECTION. D:TAIL





— NON-SUPPORTIVE

2X4 LADDER BEAM

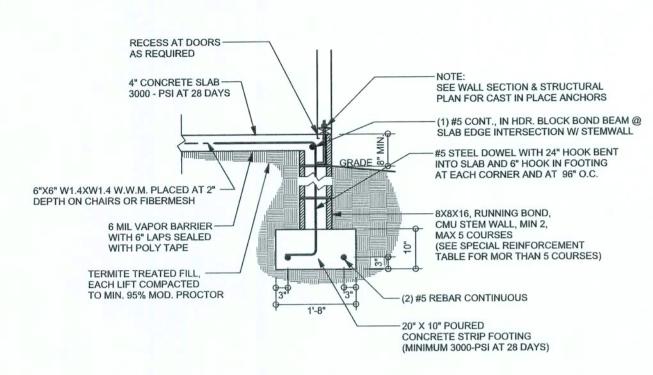


(1) 2X6 SPF #2 : SILL UP TO 11'-0" U.N.O. (1) 2X4 SPF #22 SILL UP TO 7'-3" U.N.O. (FOR: 110 MPH, 1₁₀'-0" WALL HIGHT U.N.O.) TYPICAL HEADEER STRAPING DETAIL

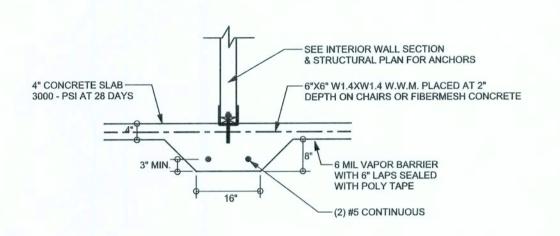
MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

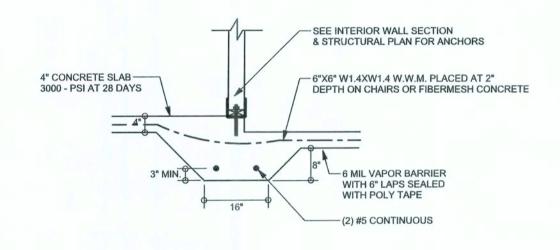
	ACI530.1-02 Section	Specific Requirements		
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi		
2.1	Mortar	ASTM C 270, Type N, UNO		
2.2	Grout	ASTM C 476, admixtures require approva		
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block		
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"		
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)		
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS		
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet me ties not completely embedded in mortal grout, ASTM A153, Class B2, 1.50 oz/ff or 304SS		
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.		
3.3.E.7	Movement joints	Contractor assumes responsibility for typ and location of movement joints if not detailed on project drawings.		



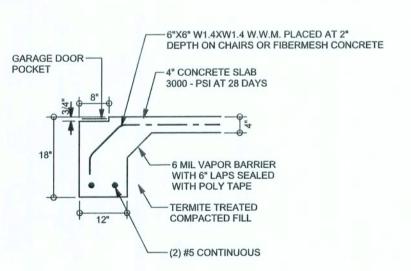
F9 STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"



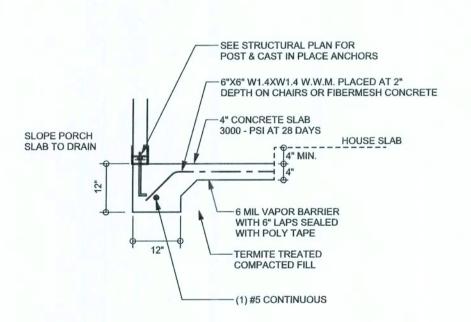
F2 INTERIOR BEARING FOOTING S-2 SCALE: 1/2" = 1'-0"



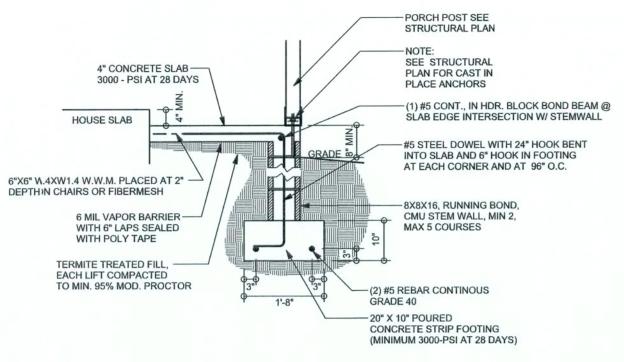
F3 INTERIOR BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"

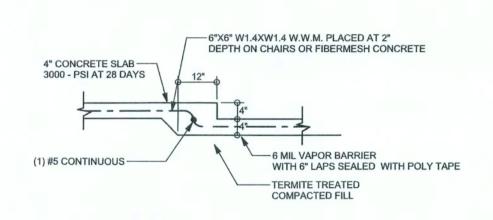


F12 OPTIONAL STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

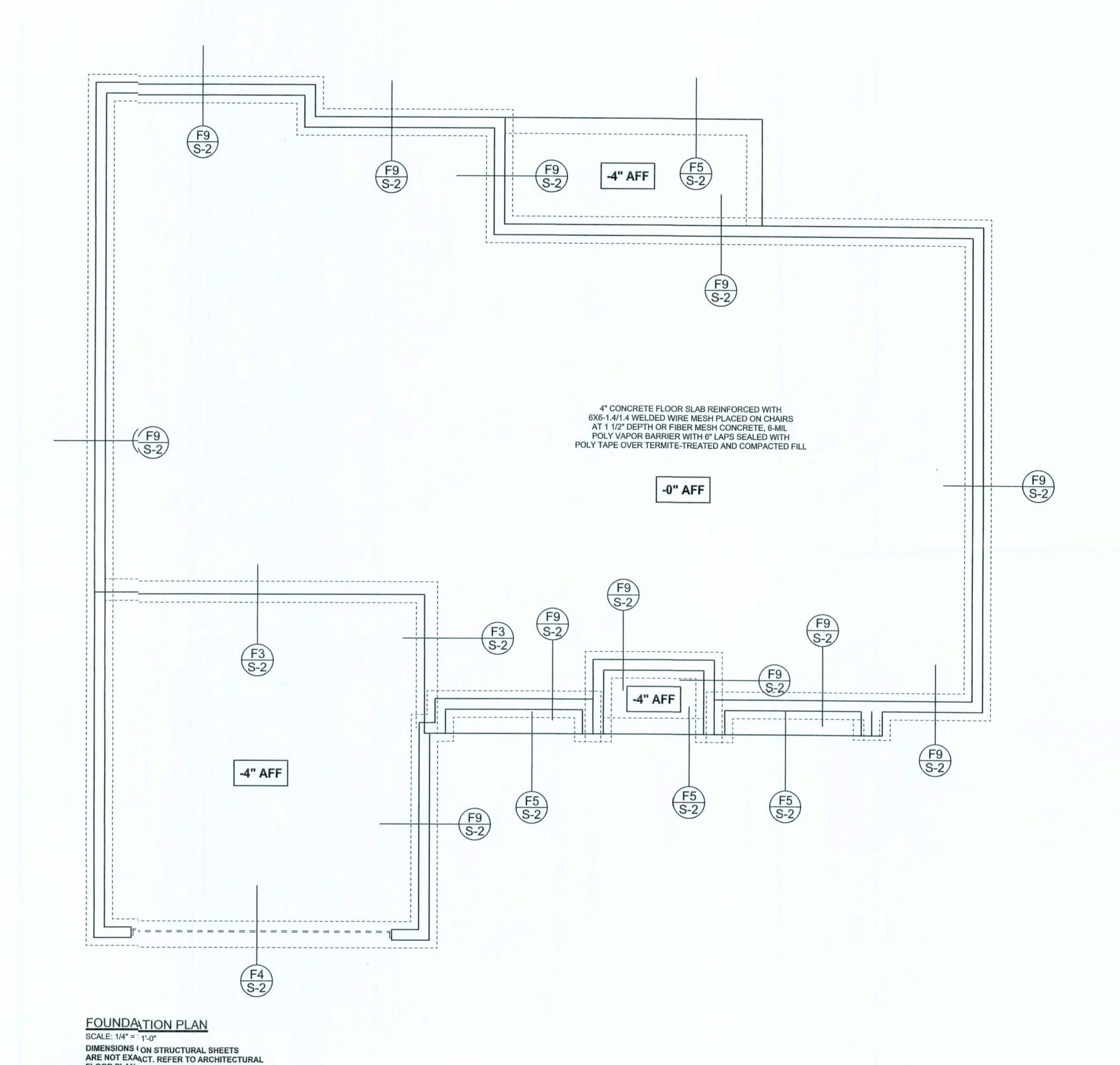
The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)		CKFILL FOR 8" CMU STEMWALL FOR 12" CMU STEMWALL			MWALL
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



F6 TYPICAL NON - BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"

FLOOR PLAN | FOR ACTUAL DIMENSIONS



DESIGN SOFTING

SOFIPIAN

REVISIONS

WINDLOAD ENGINER: Mark Disosway, PE No.53915, POB 8/8, Lake City, FL 32056, 386-754-5419 Stated dimensions swercede scaled dimensions. Refer all questions to Mark Disosway, P.E. or resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. lereby expressly reserves its common law copyights and property right in these instruments of service. This document is not to be reproduced altered or copied in any form or manner without first the express written permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, aid that the applicable portions of the plan, relating to wind engineering comply with section F301.2.1, florida building code residential 2004 to the best of my LIMITATION: This delign is valid for one building, at specified bcation. MARK DISOSWAY P.E 53915

Petersen Construction

Spec House Lot 12 Cannon Creek S/D

> ADDRESS: Lot 12 Camon Creek S/D Columbia County, Florida

Mark Disosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
July 21,2006

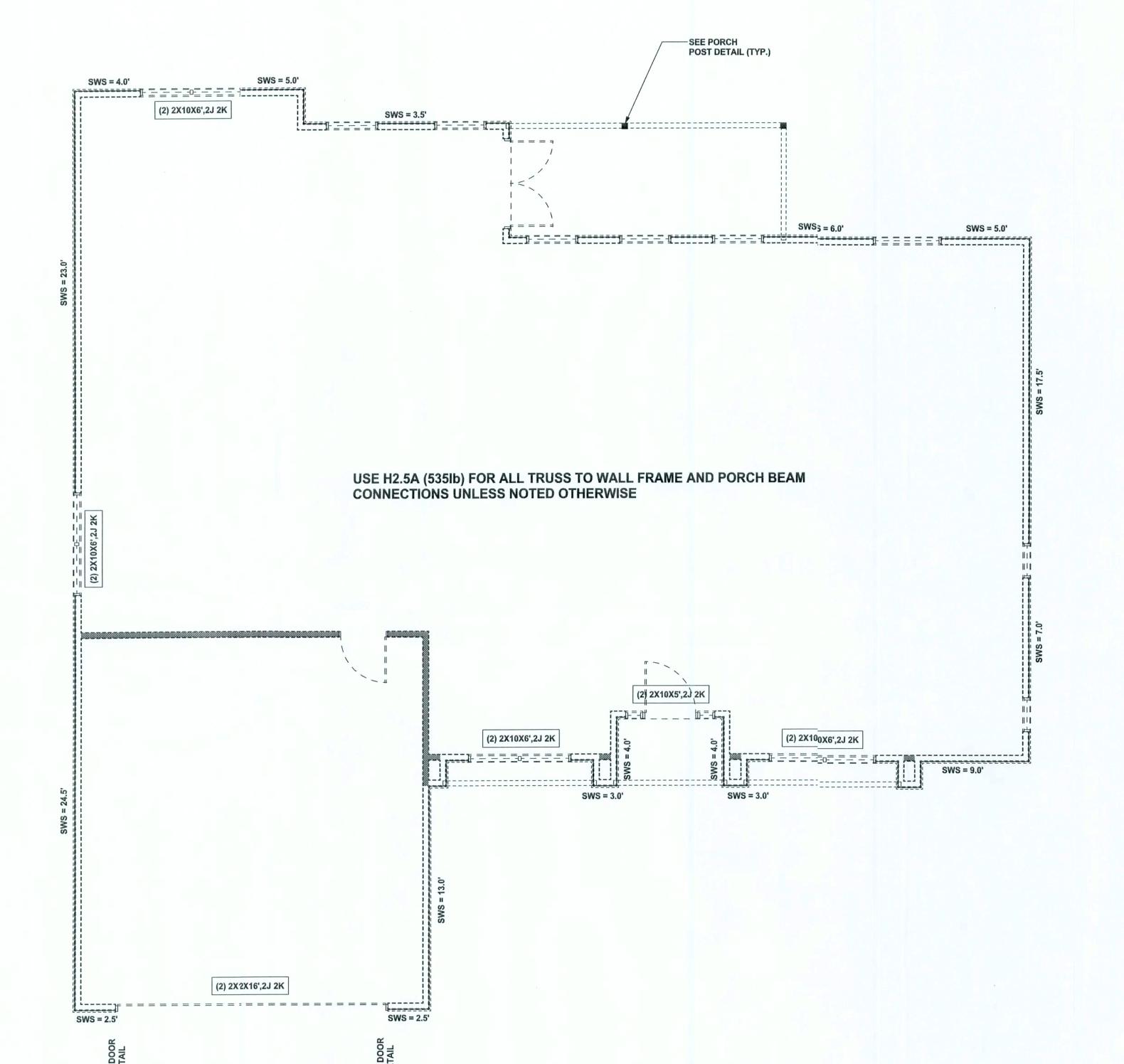
DRAWN BY: CHECKED BY:
David Disosway

FINALS DATE: 19 / Jul / 06

> JOB NJMBER: 607145 DRAWING NUMBER

> > S-2
> > OF 3 SHEETS





STRUCTURAL PLAN

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

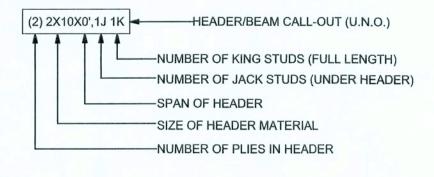
SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. SN-4 LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3
ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

sws = 0.0'	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
SWS = 0.0'	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

HEADER LEGEND



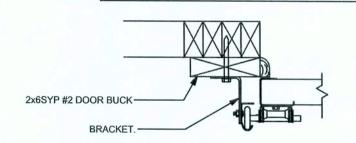
TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	34.5'	93.0'
LONGITUDINAL	31.5'	43.5'

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK ATTACH
ATTACH GARAGE DOOR BUCK TO STUD PACK AT
EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG
SCREWS w/ 1" WASHER LAG SCREWS MAY BE
COUNTERSUNK. HORIZONTAL JAMBS DO NOT
TRANSFER LOAD. CENTER LAG SCREWS OR
STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"
GN PER TABLE BELOW:

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



GARAGE DOOR BUCK INSTALLATION DETAIL

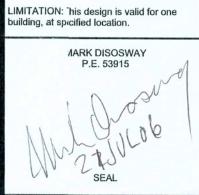
CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS CO. JOB #6-273

WINDLOAD EVGINEER: Mark Disosway, PE No.53915,POB 868, Lake City, FL DIMENSIONS

Stated dimensons supercede scaled dimensions. Refer all questions to Mark Disoswar, P.E. for resolution. Do not proceel without clarification. COPYRIGHTSAND PROPERTY RIGHTS:
Mark Disoswa; P.E. hereby expressly reserves
its common lav copyrights and property right in
these instruments of service. This document is
not to be reproduced, altered or copied in any
form or manne without first the express written
permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have

examined this ilan, and that the applicable portions of theplan, relating to wind engineerin comply with section R301.2.1, florida building code residential 2004, to the best of my



Petersen Construction

Spec House Lot 12 Canon Creek S/D

ADDRESS: Lot 12 Cannon Creek S/D Colunbia County, Florida

Mark Disosway P.E. F.O. Box 868 Lake City, Florida 32056 Phone (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: July 21, 2006 DRAWN BY: CHECKED BY: David Disoway

FINALS DATE: 19 / Jul /)6

JOB NUMBER: 607145 DRAWING NUMBER

> **S-3** OF 3 SHEETS