Top chord 2x6 SP #2 Bot chord 2x6 SP #2 Webs 2x4 SP #3 Refer to DWG PIGBACKB0207 for piggyback deta PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. 110 mph wind, 27.25 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, Iw=1.00 GCpi(+/-)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ In lieu of rigid ceiling use purlins to brace BC @ 24" OC (7-140B--Isaac Construction Jeremy Cady ITW Building Components Group, Inc. Haines City, FL 33844 FI Cartificate of whorization # 447 TYP. ALPINE Wave **IMPORTANT**TURBISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BHILD THE TRUSS IN COMPORMANCE WITH FIT. OR FARRICATHIC, HANDLIGG. SHIPPIG., HINSALLING A BRACKLING OF TRUSSES.

DISTOR COMPORES WITH APPLICABLE PROVISIONS OF HOS (MATIONAL DESIGNS SPEC, BY ATREA) AND TOT. IT HIS DESIGN SPEC, BY ATREAD AND TOT. IT HE DESIGN COMPORTS WITH APPLICABLE PROVISIONS OF HOS (MATIONAL DESIGNS SPEC, BY ATREAD AND TOT. PLAYER OF THE SPECIAL OF THE SPECIAL SPECIAL PROVISION FOR THE SPECIAL SP **WARNING** IRUSEE RECHIRE LETREME CARE IN FARRICATION, HANDLING, SHIPPING, HISTALLING AND BRACING. RETER TO BEST (BUILDING COMPONENT SACTEY HORMATION), PUBLISHED BY TET (1805S PLAIE INSTITUTE, 218 HORTH LEE SIRLEI, SUITE 312, ALEXANDRIA, VA, 2231A) AND HICA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, HI \$52719) FOR SAFETY PRACTICES PRIOR TO PERFORMHIG THESE FUNCTIONS. UNLESS OHHERIES HORTAGED FOR FORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE R = 140 $2X4(A1) \equiv$ پ -3 W-8.201" details. 7-8-15 Over 4 Supports Design Crit: 10 2X3 III 2X3 III W AP8) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 2X3 III 2X3 III M 3 X 3 ≡ 2×4 III MX SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 66 PLF at -0.86 to 66 PLF at 6.89
BC - From 4 PLF at -0.86 to 4 PLF at 6.89 Bottom chord checked for 20.00 psf non-concurrent live load. Right end vertical not exposed to wind pressure. Wind reactions based on MWFRS pressures. .36.0424 6 CEN 12 BC LL BC DL TC DL TC LL SPACING DUR.FAC. TOT.LD. FL/-/4/-/-/R/-24.0" 1.25 40.0 20.0 10.0 PSF 10.0 PSF 0.0 PSF PSF PSF DATE JREF-SEQN-REF HC-ENG DRW HCUSR8228 07197012 Scale = .375"/Ft. R8228- 81245 1T938228Z01 TCE/WHK 07/16/07 2834 6

140B Isaac Construction Jeremy Cady SP4)

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

110 mph wind, 21.07 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/ \cdot)=0.18

In lieu of rigid ceiling use purlins to brace BC @ 24"

Bottom chord checked for 20.00 psf non-concurrent live load

Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

2X2 III

2 X 3 ⊞ 2X3 III ф 12 Z _19-0-0

3X5 (B2) ≡ 20 3×6/ 3X3≡

0 0

8 15 2-1-10

R=137 W=3.79" A 3-3-8 Over 2 Supports > R=113 U=108 W=3.5"

WARNING TRUSSES REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RELIA TO BEST (BUILDING COMPONENT SAFELY INFORMATION), PUBLISHED BY PET (TRUSS PLATE INSTITUTE, 218 INSTITUTE, 218 INSTITUTE, 218 INSTITUTE, 218 INSTITUTE, 3010 INSTITUTE, SHIFE 312, ALEXANDRÍA, VA, 22314) AND HICA (MODD TRUSS COUNCEL OF AMERICA, 6300 CHIEBRENSE LANE, MADISON, NE 53719) FOR SAFELY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OFHEREMENTS, HOLDSCHOLDED SON ALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SINALL HAVE A PROPERLY ATTACHED REGION OF THE SAFELY ATTACHE

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Wave

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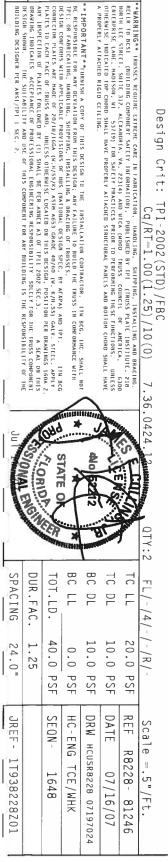
ALPINE

SPECIAL LOADS From From (LUMBER ER DUR.FAC.=1.25 / PLATE D 79 PLF at -0.42 to 79 68 PLF at 0.74 to 68 4 PLF at -0.42 to 4 TE DUR.FAC.=1.25)
79 PLF at 0.74
68 PLF at 2.88
4 PLF at 2.88

Wind reactions based on MWFRS pressures

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. 110 mph wind, 24.67 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi (+/-)=0.18 In lieu of structural panels or rigid ceiling use purlins brace all flat TC @ 24" 0C, all BC @ 24" 0C. Note: All Plates Are 2X3 Except As Shown. PLT TYP. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 7-140B--Isaac Construction Haines City, FL 33844 ALPINE

IMPORTANT*IURIISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BECG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DELYLATION FROM HIS DESIGN. ANY FAILURE TO BUILD HE TRUSS IN COMPORMANCE WITH FIP: OR FARRICATING, HANDLING, SHEPPING, HISFALLING A BRACILIG OF TRUSSES.

IMPORTANT*IURIISH A COPY OF THIS DESIGN FOR THIS DESIGN FOR THE PROPERTY OF 0 5 Wave $2X4(B1) \equiv$ #2 Dense #2 Dense #3 98 U=158 W=5.077" R=82 PLF U=36 PLF W=24-7-2 10 **HARNING** BRUSE'S REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.

HARNING BRUSE'S REQUIRE EXTREME CARE IN FABRICATION, HANDLING BY INCREME THIS FLATE INSTITUTE, 218

HORRI LEE STREET, SUITE 317, ALEXANDRIA, VA. 22314) AND WICA (MODD TRUSS COUNCIL OF AREELA, 6300

CHIERRETSE, LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS

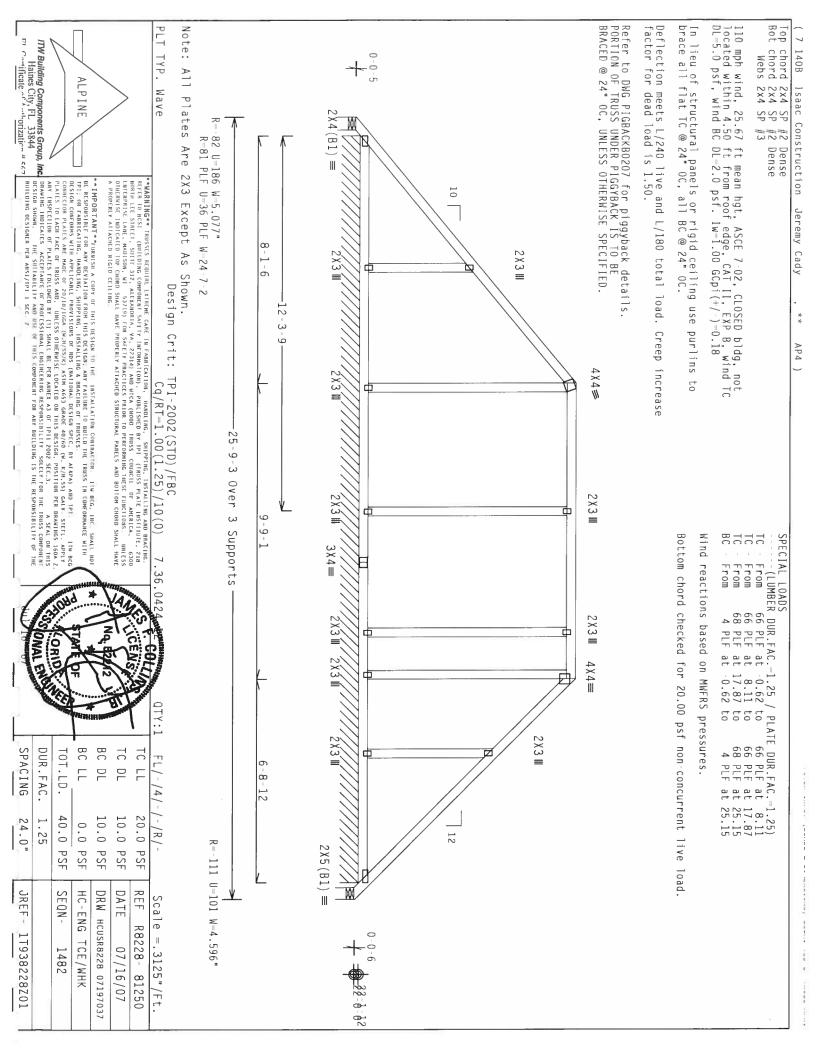
OTHERRETSE, LONGLATED OF CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAVE

A PROPERLY ATTACHED RIGIO CETLING. BUILDING DESIGNER PER ANSI/IPI 1 SEC. Jeremy Cady α 9 2 X 3 III 2′X′3 Ⅲ Design Crit: 4 X 4 == 2X3 III 12-3-9 AP3 2 X 3 III TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 25 9 0ver 2 X 3 III 2 X 3 Ⅲ 14-1-14 3 Supports Bottom chord checked for 20.00 psf non concurrent live load. Wind reactions based on MWFRS pressures. SPECIAL LOADS $3 \times 4 \equiv$ 7.36.0424 From From From (LUMBER 66 PLF at 66 PLF at 68 PLF at 4 PLF at 2×3 Ⅲ 2X3 **Ⅲ** DUR.FAC.=1.25 CORIOR -0.62 to 5.71 to 19.87 to -0.62 to PLATE 2 X 3 Ⅲ 4×4= 2X3 III 2 X 3 Ⅲ 66 PLF at 66 PLF at 68 PLF at 4 PLF at E DUR.FAC.-1.25)
66 PLF at 5.71
66 PLF at 19.87
68 PLF at 25.15
4 PLF at 25.15 BC LL BC DL TC DL TC LL SPACING DUR.FAC. TOT.LD. FL/ -/4/-/ 4-8-12 40.0 10.0 /-/R/ 24.0" 1.25 10.0 20.0 0.0 R PSF PSF PSF PSF PSF $2X5(B1) \equiv$ 12 116 U-103 W-4.596" JREF-DATE REF SEQN-HC-ENG DRW HCUSR8228 07197025 Scale = .3125"/Ft. R8228- 81247 1T938228Z01 TCE/WHK 07/16/07 1463 -32 d d2

l op Bot Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. PLT TYP. Note: All Plates Are Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ In lieu of structural panels or rigid ceiling use purlins brace all flat TC @ 24" 0C, all BC @ 24" 0C. 110 mph wind, 23.67 ft mean hgt, ASCE 7 02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL 5.0 psf, wind BC DL=2.0 psf. Iw 1.00 GCpi(+/ \cdot)=0.18 Haines City, FL 33844
Fi Codificate of Authorization 4 647 7-140B - Isaac Construction ITW Building Components Group, Inc. p chord 2x4 SP #2 I t chord 2x4 SP #2 I Webs 2x4 SP #3 0 0 5 ALPINE Wave 2X4(B1) R 50 10 0 U Dense Dense Ш 93 W 5.077" PLF U 29 PLF W-24 7 2 2X3 Except As Shown. **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BC RESPONSIBLE FOR ANY OFFVALION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORANCE WITH FPI; OR FARBECKTHG, INADULES, SIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN COMPORES WITH APPLICABLE PROVISIONS OF MDS (MATIONAL DESIGN SECE, BY ATERA) AND TPI. ITH BCG COMMICCION PEATES ARE HADE OF 70/19/16GA (M.H/SS/K) ASIH A653 GRADE 40/50 (M. K/M.59) GAUY. SIEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNICES OTHERNISE LOCATED ON THIS DESIGN, POSITION PER DEAAHINGS 16GA Z. ANY INSPECTION OF PLATES FOLOHOLD BY (I) SHALL BE PER ANHEX A OF TPI] 7002 SEC.3. A SEAL ON THIS DESIGN AND THE SUBJECT OR THE SECONDOMENT OF THE CONTRACT OF THE SECONDOMENT OF THE CONTRACT OF THE SECONDOMENT Ü **WARNING** IRUSYIS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, RETER TO BEST (BUILDING COMPONENT SAFLY INFORMATION), PHBLISHIGD BY TPI (TRUSS PLATE INSTITUTE, 218 HOBIN HIL STREET, SUITE 13.7 ALEXANDRA, VA, 22:14) AND HICKA (MODD TRUSS COUNCELL OF AMERICA, 6300 ENTERPRISE LAME, HADISON, HI 53/19) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE TUNCTIONS. UNICESS OTHERHISC, LUDICALID TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE BUILDING DESIGNER PER AUSI/TPI T SEC. 12 Jeremy Cady 4 X 4 == ŹX′3 Ⅲ 2 X 3 III 2 X 3 Ⅲ Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 12 - 3 - 9AP2 2 X 3 2×3 25 9 ω Over 3 X 4 ≡ 2 X 3 W 2X3 III 18-6-10 3 Supports Bottom chord checked for 20.00 psf non concurrent live load Wind reactions based on MWFRS pressures SPECIAL LOADS 3 \ 4 ≡ From From .36.0424 (LUMBER 66 PLF at -0.62 to 66 PLF at 3.31 to 68 PLF at 21.87 to 4 PLF at -0.62 to 2́X′3 ∭ 2 X 3 III DUR.FAC. =1.25 PLATE ŹX́3́∭ 2X3 III FE DUR.FAC.—1.25)
66 PLF at 3.31
66 PLF at 21.87
68 PLF at 25.15
4 PLF at 25.15 BC LL BC DL SPACING TC DL DUR.FAC. TOT.LD. FL/-/4/-. 2×3 ₩ $4 \times 4 =$ 24.0" 1.25 40 10.0 10.0 PSF 20.0 0.0 2-8-12 . 12 PSF PSF PSF PSF $2X5(B1) \equiv$ 35 U=34 W=4.596" JREF-SEQN-DATE REF HC-ENG DRW HCUSR8228 07197026 Scale R8228- 81248 1T938228Z01 =.3125"/ft. TCE/WHK 1486 07/16/07

Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 PLT TYP. 110 mph wind, 33.62 ft mean hgt, ASCE 7-02, CLOSED located within 4.50 ft from roof edge, CAT II, EXP DL=5.0 psf, wind BC DL=2.0 psf. Deflection meets L/240 live and L/180 total load. factor for dead load is 1.50. ITW Building Components Group, Inc. Haines City, FL 33844 FI Condificate of Authorization # 567 140B ALPINE Isaac Construction Jeremy Cady Wave **IMPORTANT*** URMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, AFF YALLURE TO BHILD THE TRUSS IN COMPORNANCE WITH IP: OR FABRICATHIG. INMIDITES, SUPPINE, HISTALLING & BRACHING OF TRUSSES, DESIGN CONTROPS WITH APPLICABLE PROVISIONS OF DRDS (MATIONAL DESIGN SPEC, BY ATRY) AND TP!. THE BCG CONNECTOR PLATES ARE HADE OF ZO/TH//166A (H.H/SS/K) ASTH AGS3 GRANDE 40/60 (H.K/H.SS) GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSSES, AND. UNICSS OHILRISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE FER ANNEX AS OF TPIL 2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL CHARGES AND TAKE AS OF TPIL 2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF THE TRUSS CORPORENT OF THE SULFACE OF THE SULFACE OF THE TRUSS CORPORENT OF THE SULFACE OF THE TRUSS CORPORENT OF THE SULFACE OF THE TRUSS CORPORENT OF THE SULFACE OF THE SULFACE OF THE TRUSS CORPORENT OF THE SULFACE OF THE SULFACE OF THE TRUSS CORPORENT OF THE SULFACE OF THE SU **WARNING** IRUSGES REQUIRE EXTREME CARF IN FABRICATION, INMOUTING, SHIPPING, INSTALLING AND BRACING, RETER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 219 MORTH LEE STREIT, SUIT 2127, ALEXANDRIA, WA, 22214) AND MICHA (MODD TRUSS COUNCIL OF AMERICA, 6300 CHILERRYSE LANF, MADISON, HI 53710) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNICESS OTHERWISE FLORICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2X4(B1) = 2 U=180 W=5.077" R=75 PLF U=38 PLF W=14-1-6 Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 1.5X4 III Creep increase) bldg, not B, wind TC 5 X 4 III BP3) 7-0-11 -0-11 10 15-4-6 Over 3 Supports ..5X4 Ⅲ 4 X 4≡ TC - From BC - From Bottom chord checked for 20.00 psf non-concurrent live load In lieu of rigid ceiling use purlins to brace BC @ 24" OC. SPECIAL LOADS 7.25.0411 (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25) rom 66 PLF at -0.62 to 66 PLF at 14.74 rom 4 PLF at -0.62 to 4 PLF at 14.74 10 -0 - 111.5X4 III 5×4 III The state of the s 2X4(B1) = 2 U=180 W=5.077" SPACING BC LL BC DL TC DL DUR.FAC. TOT.LD. TC LL FL/-/4/-/-/R/-24.0" 1.25 40.0 10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF PSF JREF DATE REF SEQN-HC-ENG DRW HCUSR8228 07197028 Scale = .375"/Ft. R8228- 81249 1T938228Z01 TCE/WHK 07/16/07 107149 REV

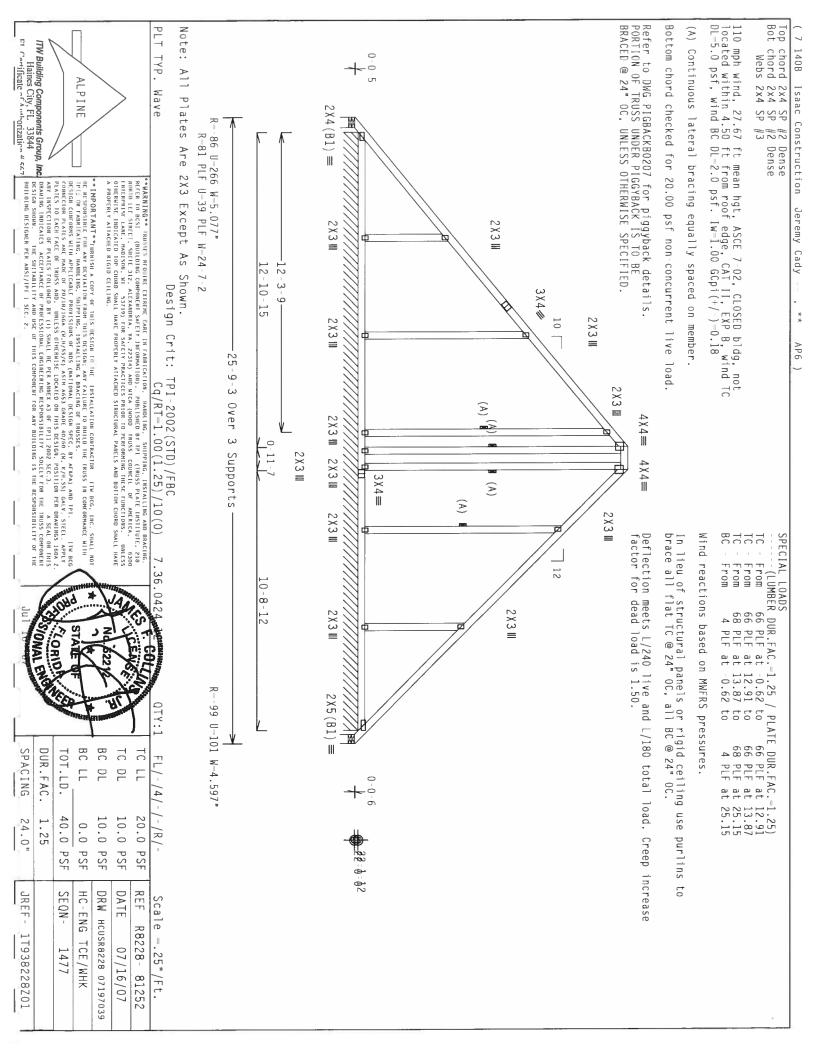


Bot 110 mph wind, 27.88 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18 Note: All Plates Are BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 3 7 140B Isaac Construction Jeremy Cady Haines City, FL 33844

rt Carificate of antionization a second chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 Continuous lateral bracing equally spaced on member TYP. ALPINE Wave 0 0 2X3 Except As Shown. **IMPORTANT**FURBISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE FRUSS IN CONTORNANCE WITH IPI. OR FLORE CHILLING. SHARLING ANY FAILURE TO BUILD THE FRUSSES.

DESIGN OR FLORE CHILLING. SHAPLING. SHEAVELING AS BRACER OF TRUSSES.

DESIGN CONTROL OF THE PROPERTY OF THE BCG CONTROL OF THE PROPERTY OF THE BCG CONTROL OF THE PROPERTY OF THE BCG CONTROL OF THE ANDE OF ZO/JU/JGGA (W.M/SS/M) ASIM AGS3 GRADE AD/GO (W. K/M.SS) GALV SITELA APPLY PLATES TO LACH FACE OF TRUSS AND. UNITESS OTHERNISE LOCATED ON THIS DESIGN, POSITION PER DRAWHIGS HOA Z. ANY THIS POSITION OF PLATES FOLLOWED BY (I) SHALL BE PER ANIRY AS OF THIS ZOCE SEC.3. A SEAL ON THIS DRAWHIGH INDICATES ACCEPTANCE OF PROFESSIONAL ENGLINE AND RESPONSIBILITY SOLLY FOR THE THESE COMPONENT OF THE PROPERTY OF TH **WARNING** IRUSEYS REQUIRE EXTREME CARE IN FARRICATION, MANDLING, SHIPPING, INSTALLING AND BRACING, RETER TO REST. (BUILDING COMPONENT SAFETY INTORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, ZIB MORTH HEE STREET, SUITE JIZ, ALEXANDRIA, VA, ZZIJA) AND HICAG (WOOD TRUSS COUNCIL OF AMERICA, 6300 CHICERPRISE LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERHISE LIDICATIO TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANICES AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHT DESCRIPTION. BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2 2X4(B1) = R=-85 U=274 W=5.077" R=81 PLF U=40 PLF I Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/ 2X3 **Ⅲ** 2 X 3 III * .2-3-9 10 | 3X4₩ 13-5-₩=24-7 AP7) ٦ 2X3 III 25 9-3 0ver 2 X 3 III 2X3 Ⅲ 3 Supports 2 X 3 Ⅲ 3 \ 4 ≡ /10(0) $\widehat{\mathbb{A}}$ 2X3 III 2 X 3 III 4X5(R) III Bottom chord checked for 20.00 psf non-concurrent live load Wind reactions based on MWFRS pressures 12 SPECIAL LOADS 3×4// 1-1-15 7.36.042 From From CLUMBER DUR.FAC.=1.25 / PL rom 66 PLF at -0.62 to rom 68 PLF at 13.43 to rom 4 PLF at -0.62 to 2X3 III 2 X 3 III 20 2X5(B1) =99 U=102 W=4.596" OTY:6 PLATE 0 9-0-6 TE DUR.FAC.=1.25)
66 PLF at 13.43
68 PLF at 25.15
4 PLF at 25.15 SPACING BC LL BC DL TC DL DUR.FAC. TC LL TOT.LD. Equit direct date of the contract of the contr FL/-/4/-/-/R/-24.0" 40.0 10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF PSF JREF -DATE REF SEQN-HC-ENG DRW HCUSR8228 07197038 Scale =.1875"/Ft. R8228- 81251 1T938228Z01 TCE / WHK 07/16/07 1473



(A) 1x4 With 8d Top Bot Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. 110 mph wind, 26.67 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi (+/-)=0.18Note: All Plates Are 2X3 Except As Shown. Bottom chord checked for 20.00 psf non-concurrent live load. Haines City, FL 33844
Fit Carlificate of Ambarization # 447 chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 140B Isaac Construction TYP. 0-0-5 #3 or better "L" brace. 80% length of web member. Attach Box or Gun (0.113"x2.5",min.)nails @ 6" 0C. ALPINE Wave 2X4(B1) =-80 U=207 W=5.077" R=80 PLF U=36 PLF W=24-7-2 **IMPORTANT***CHRISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE 10 BUILD THE TRUSS IN COMPORMANCE WITH PL. OR FABRICATION, MANDING, SHIPPING, HISTALLIG A BRACHING OF TRUSSES.

DESIGN CONFORMS HITH APPLICABLE PROVISIONS OF DDS (MATIONAL DESIGN SPEC, BY AFRAYA AND TPI. THE BEG CONNECED REVAILES ARE HADE OF ZO/ID/16GA (M.H/SS/K) ASIH A653 GRADE 40/500 (M. K/M.SS) GALV. SIEEL APPLY PLANES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER BRACHES HOA Z. ANY HISTICION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF THIS ZOCE YEE OR THE RUSS COMPONENT BE ADMINISTED FOR THE PLATES ACCUPANACE OF PROTESSIONAL ENGINEER HOR RESPONSIBILITY SOLELY OR THE RESPONSIBILITY OF THE **WARNING** INUSSES REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, RETER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, ZIB HORN LEE STREET, SUITE JAZ ALEXANDRIA, VA, 22-214) AND MICHA (MODD IRUSS COUNCIL OF ARREIGA, 6300 CHITERPRISE LANE, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERSISE LINDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE ARROPERLY ATTACHED REGION SHALL HAVE PROPERLY ATTACHED REGION SHALL HAVE PROPERLY ATTACHED BY CHORD SHALL HAVE PROPERLY ATTACHED REGION PROPERLY ATTACHED REGION PROPERLY ATTACHED REGION PROPERLY ATTACHED REG BUILDING DESIGNER PER ANSI/IPI I SEC. Jeremy Cady 2 X 3 III 2X3 III 10-6-3 12-3-9 10 Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)* 2X3 **Ⅲ** 2X3 III AP5 25 4 X 4 == Ġ 2 X 3 Ⅲ ώ 0ver $\widehat{\mathbb{A}}$ 2X3 III 2X3 III 3 Supports -4-4 Ξ $3 \times 4 \equiv$ (A) 2 X 3 III 4×4 🐙 2 X 3 Ⅲ Wind reactions based on MWFRS pressures. In lieu of structural panels use purlins 24° OC. Deflection meets L/240 live and L/180 total load. factor for dead load is 1.50. SPECIAL LOADS (A) 2X3 **Ⅲ** 7.36.042 From From From (LUMBER 12 ER DUR.FAC.=1.25 / PL/
66 PLF at -0.62 to
66 PLF at 10.51 to
68 PLF at 15.87 to
4 PLF at -0.62 to 2 X 3 III 2X3 III ά -12 R 2X5(B1) =97 U=72 W=4.597" PLATE E DUR.FAC.=1.25)
66 PLF at 10.51
66 PLF at 15.87
68 PLF at 25.15
4 PLF at 25.15 BC LL BC DL SPACING TC DL DUR.FAC. TOT.LD. FL/-/4/-/-/R/-0-0-6 to brace all flat 24.0" 20.0 1.25 40.0 PSF 10.0 PSF 10.0 PSF 0.0 PSF Creep increase PSF JREF-SEQN-DATE REF HC-ENG DRW HCUSR8228 07197040 C Scal le =.25"/Ft. R8228- 81253 1T938228Z01 TCE/WHK 07/16/07

110 mph wind, 22.44 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi (+/-)=0.18 Top chord 2x4 SP #2 Bot chord 2x4 SP #2 Webs 2x4 SP #3 7 140B Isaac Construction Jeremy Cady ? Dense Dense *

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Bottom chord checked for 20.00 psf non-concurrent live load

Wind reactions based on MWFRS pressures

SPECIAL LOADS

(LUMBER

From From

68 PLF at 68 PLF at 4 PLF at DUR.FAC.

.=1.25 / PL t -0.51 to t 1.88 to t -0.51 to

TE DUR.FAC.=1.25)
68 PLF at 1.88
68 PLF at 4.29
4 PLF at 4.26

PLATE

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$

Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

4 X 4 == 12

12

0-0-6

1-10-8-1-10-8

2X4(B1)

2 X 3

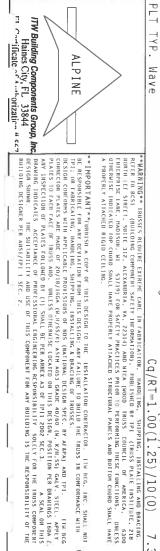
2X4 (B1)

4-9-9 Over W Supports 1 - 10 - 8

R=36 U=29 W=7.425* (1.328" Effective Contact)

R-86 PLF U=36 PLF W=3-2-14

R=16 U=2 W=7.425" (1.328" Effective Contact)



MARNING TRUSSY'S REQUIRE EXTREME CARE IN FARRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. RETER TO REST. (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 JOURNAL LE STRETT, SUITE 127, ALEXANDRIA, NA. 22213) AND HEAVE (PODD TRUSS COUNCIL OF AMERICA. 6300 THIERRE SE LAWE, MADISON, H. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE HOUSEALDED OF CHOODS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING. Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

NAL ENGINE QTY:11 FL/-/4/-/-/R/-SPACING BC LL DUR.FAC. # DE TC LL TOT.LD. 40.0 1.25 10.0 PSF 10.0 PSF 20.0 PSF 24.0" 0.0 PSF PSF JREF-DATE SEQN-REF HC-ENG DRW HCUSR8228 07197044 Scale =.5"/Ft. R8228- 81254 1T938228Z01 TCE / WHK 1564 07/16/07

Bot Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Bottom chord checked for 20.00 psf non-concurrent live load Wind reactions based on MWFRS pressures PLT TYP. Note: All Plates Are 2X3 Except As Shown. 7·140B--Isaac Construction Jeremy Cady chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 0-0-5 ALPINE Wave (1.024" Effective Contact) R-49 U-16 W-8.201" 2X4(A1) =10 က် **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE THISTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT UR. RESPONSIBLE FOR MAY DEFIATION FROM THIS DESIGN, MAY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH FPL. OR FLOREFACTING, LANDING, SHEPPIG, HISTALLING & BRACHIG OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF ANDS (MATIONAL DESIGN SPEC, BY AFRY) AND TPL. THE BCG CONNECTION PLATES ARE HADE OF 70/18/1/GAG (M. H/SS/M). ASTH AGS GRADE 40/60 (M. KM. SS) GALV SICEL APPLY PLATES TO EACH FACE OF TRUSS AND, MULLSS OHERHISE LOCATIO ON THIS DESIGN, POSITION PER DRAWINGS 150A-Z. PLATES TO EACH FACE OF TRUSS AND, MULLSS OHERHISE LOCATIO ON THIS DESIGN, POSITION PER DRAWINGS 150A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANNEXS OF FPIL 2002 SEC.3. A SEAL ON THIS DESIGN SHOWN.

DESIGN SHOWN. THE SUITABLLITY AND DUST OF THIS COMPONENT FOR MAY BUILDING IS THE RESPONSIBILITY OF THE DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING R DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT BUILDING DESIGNER PER ANSI/IPL 1 SEC. 2 **WARNING** IRUSSES REQUIRE EXTREME CARE IN FABRICATION. THANDELING. SHIPPING, UISTALLING AND BRACING, RETER TO BEST (BUILDING COMPONENT SAFETY HERMANION). PUBLISHED BY IT! (TRUSS PLATE INSTITUTE, 210 HORTH LEE STREIT, SUITE 315, ALEXANDRAIA, VA. 22314) AND ATCA (MODOD TRUSS COUNCIL OF AMERICA. 6300 FRITERPRISE LONE, MUNISON, NY 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDUSTRIED FOR THE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE _11 4 X 4 == 2X3 III Ф 2X3 **Ⅲ** 2X3 III Design Crit: Ф ф * ட் 0 BP1) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) R=73 PLF U=35 PLF W=13-7-2 15-4-6 Over 3 Supports 11-2-10 2X3 III 2X3 III 贞 市 110 mph wind, 31.27 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. In lieu of structural panels or rigid ceiling use purlins brace all flat TC @ 24" OC, all BC @ 24" OC. 7.36.0424 Ju SONAL ENGLISE STATE 2X3 III 2X3 III 贞 2X3 III 4 X 4 == ВС ВС TC DL TC LL SPACING DUR.FAC. TOT.LD. ф FL/-/4/-/-/R/-므 1-5-11 (1.024" Effective Contact) $\overline{\lambda}$ =26 U=11 W=8.201" 2X4(A1) =10 40.0 20.0 24.0" 10.0 PSF 1.25 10.0 PSF 0.0 PSF PSF PSF 0.1 REF JREF -SEQN-DATE HC-ENG DRW HCUSR8228 07197045 Scale R8228-1T938228Z01 =.5"/Ft. TCE / WHK 07/16/07 81255 -6-8-12

140B Isaac Construction Jeremy Cady SP3)

Top Bot chord 2x4 SP #2 Dense :T2 2x8 SP #2: chord 2x4 SP #2 Dense Webs 2x4 SP #3

110 mph wind, 20.12 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/ \cdot)=0.18

In lieu of structural panels or rigid ceiling brace all flat TC @ 24" OC, all BC @ 24" OC. use purlins

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

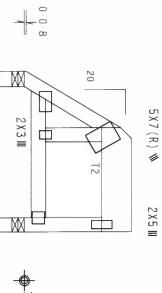
Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL --(LUMBER From 7 From 7 From 7 LOADS ER DUR.FAC.=1.25 / PL/79 PLF at -0.42 to 79 PLF at 0.97 to 4 PLF at -0.42 to PLATE E DUR.FAC.=1.25)
79 PLF at 0.74
79 PLF at 2.88
4 PLF at 2.88 0.74 2.88 2.88

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 20.00 psf non-concurrent live load



_19-0-0 9

-8-15 2-1-10

3X5(B2) =

3X3=

3-3-8 Over 2 Supports R=133 U=9 W=3.79" __V

R=122 U=45 W=3.5"

PLT TYP. Wave

TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0)

Design Crit:

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BGG, THC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY TALLIRE TO BHILD THE BRUSS IN COMPORMANCE WITH FILE OR FABRICATING, MANDING, SHEPPING, INSTALLING A BRACING OF BRUSSES.

DESIGN CONTRORS WITH APPLICABLE PROVISIONS OF HIS SCIENCE SPECE, BY ATAPA, AND IPI. COSTIGN CONTROL AND THE APPLICABLE PROVISIONS OF HIS SCIENCE SPECE, BY ATAPA, AND IPI. STEEL, APPLY DIALES TO EACH FACE OF TRUSS AND. HIMESS OTHERNISE LOCATED ON THIS DESIGN, POSITION FOR BRAHINGS 160A-Z. ANY THIS PETITION OF PLATES FOLLOWED BY CITY SOUTH AND THE STEEL APPLY DRAINED HID OF PLATES FOLLOWED BY CITY SOUTH AND THE STEEL APPLY DRAINED HID OF PLATES FOLLOWED BY CITY SOUTH AND THE STEEL AND THIS DESIGN AS A SEAL ON THIS DRAINED HID STEEL ASSESSMENT AND THE STEEL ASSESSMENT AND THE STEEL OF PROFISSIONAL ENGLINE FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BRUSS COMPONENT OF STEEL SHOWN.

DESIGN SHOWN IN THE SULFABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BRUSS COMPONENT OF THE STEEL SHOWN.

Haines City, FL 33844
Haines City, FL 33844
Haines City, FL 3844

ALPINE

7.36.042 SIONAL ENGINEE TATE BC LL TC DL SPACING ***** 25 TC LL DUR.FAC. TOT.LD. FL/-/4/-/-/R/ 10.0 20.0 24.0" 40.0 PSF 10.0 PSF 1.25 0.0 PSF PSF PSF JREF -SEQN DATE REF DRW HCUSR8228 07197047 HC-ENG Scale R8228-1T938228Z01 =.5"/Ft. TCE/WHK 07/16/07 81256

7 1408 Isaac Construction Jeremy Cady CPI)

Top Bot chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3

110 mph wind, 20.48 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

In lieu of structural panels use purlins $24\mbox{\,{}^{"}}\mbox{\,}0\mbox{\,{}^{"}}.$ to brace all flat

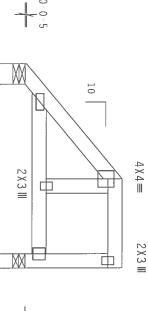
Refer to DWG PIGBACKB0207 for piggyback deta PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. details.

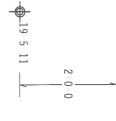
> SPECIAL LOADS From From (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25) rom 66 PLF at -0.62 to 66 PLF at 1.74 rom 66 PLF at 1.74 to 66 PLF at 3.57 rom 4 PLF at -0.62 to 4 PLF at 3.57

Wind reactions based on MWFRS pressures

Bottom chord checked for 20.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$





1-8 -15 1-9-14

2X4(B1) =

3X3=

R-158 U-28 W-5.077" 4-2-5 Over 2 Supports --> R-136 U-51 W-3.5"

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT

TYP.

Wave

IMPORTANTTURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN, ANY FAILURE TO BUILD THE TRUES IN COMPORMANCE WITH FPE; OR FARREATHE, HANDLING, SHIPPING, HISALLING & BRACHIG OF TRUESES.

DESIGN COMPORES WITH APPLICABLE PROVISIONS OF HDS (MALIONAL DESIGNE SPEC, BY ATRA) AND TPI. THE BCCCOMPORTED BY ALLES ARE AND TO TRUESES.

PLATES TO EACH FACE OF TRUES AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN. POSITION FER BRAVHIGS 160A-Z. ANY INSPECTION OF PLATES TOLLOHOUR BY (1) SHALL BE PER AIMER AS OF TPIL 2002 SEC. 3. A SEAL ON THIS DESIGN SHOWN OF THATES ALT ALLEY AND THE BLAVE OF THE RESPONSIBILITY OF THE BRAVHIGS COMPONENT DESIGN SHOWN. THE SULFABLING OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE RESPONSIBILITY OF THE BUILDING DESIGNER FER AMESI/IPI I SEC. 2.

Haines City, FL 33844
Haines City, FL 33844

"" originate of a monigation with care of the care of the

ALPINE

7.36.042 SPACING BC LL BC DL TC DL DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/-20.0 40.0 24.0" 10.0 PSF 10.0 PSF 1.25 0.0 PSF PSF PSF JREF -SEQN-DATE

DRW HCUSR8228 07197054

Scale =.5"/Ft.

R8228- 81257

07/16/07

HC-ENG

TCE/WHK

1T938228Z01

Top Bot PLT Refer to DWG PIGBACKB0207 for piggyback deta PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ In lieu of rigid ceiling use purlins to brace BC @ 24" OC. 110 mph wind, 21.66 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, Iw=1.00 GCpi(+/-)=0.18 ALPINE

IMPORTANT* UBRISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCC. LIC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCC. LIC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN AND FALLURG TO BUILD THE RRISS IN CONFORMANCE WITH THE CONFORMANCE WITH A COPY OF TRUSSES.

IMPORTANT* UBRISH A COPY OF THIS DESIGN. POST TRUSSES. IN CONFORMANCE WITH BCC. LIC. SHALL NOT THE CONFORMANCE WITH BCC. CONFORMANCE WITH A COPY OF TRUSSES. BEACH AS SPEC. BY AGRAY AND FILE CONFORMANCE OF TRUSSES.

IMPORTANT* UBRISH TO SUBJECT THE CONFORMANCE WITH BCC. CONF chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 140B Isaac Construction TYP. Wave 0-0-5 Jeremy Cady 2X4(B1) =W=5.077" -5-2-5 details. 10 Design Crit: 0ver * CP3) 1/2 Supports TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) R=171 U=113 W=3.5" 3X3≡ 2 X 4 III M 19-5-10 SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 66 PLF at -0.62 to 66 PLF at 4.57
BC - From 4 PLF at -0.62 to 4 PLF at 4.57 Bottom chord checked for 20.00 psf non-concurrent live load. Right end vertical not exposed to wind pressure. Wind reactions based on MWFRS pressures. 4 SOUND TO SOUND SOU STATE BC LL BC DL SPACING TC DL DUR.FAC. TOT.LD. FL/-/4/-/-/R/-40.0 24.0" 10.0 PSF 20.0 PSF 1.25 10.0 PSF 0.0 PSF PSF JREF-SEQN-DATE REF HC-ENG DRW HCUSR8228 07197075 Scale =.5"/Ft. R8228- 81259 1T938228Z01 TCE/WHK 07/16/07

Bot End Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ In lieu of rigid ceiling use purlins to brace BC @ 24" OC 110 mph wind, 25.18 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/)=0.18 7-140B Isaac Construction Jeremy Cady ITW Building Components Group, Inc. Haines City, FL 33844 "I C--ifficate - " " " orizatic - " " " " " verticals not exposed to wind pressure chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 TYP. ALPINE Wave ***IMPORTANT**TURNISH A FORY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR NAW DEVIATION FROM THE DESIGN AND TAILURE TO BUILD HE TRUSS IN COMPORMANCE WITH FP: OR TABRICATION. AND BLG. SHEPTHAC, INSTALLIG & BRACHIGO OF TRUSSES.

DESIGN COMPORNS WITH APPLICABLE PROVISIONS OF NNS (MAIDONAL DESIGN SPEC. BY ATARA) AND IPI. ITH BCG CONNICCION FALES ARE AND OF 707/19/16/A (M.1/555) AND ADDRESS OF NNS (MAIDONAL DESIGN SPEC. BY ATARA) AND IPI. ITH BCG CONNICCION FALES ARE AND OF 707/19/16/A (M.1/555) AND ADDRESS OF NNS (MAIDONAL DESIGN SPEC. BY ATARA AND IPI. SHALL APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER BRANHESS 160A Z. ANY INSPECTION OF FALES FOLLOWED BY (1) SHALL BE FER ANIEX A 30° IPI. 2002 SEC 3. A SEA ON INTESTITUTE OF THE THE SECONDARY OF THE STANDARY OF T **MARNING** TRUSSES REQUIRE EXTRINE CARE IN FABRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO ESCI. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 UNDERLY SHIPPING AND TRUSS COUNCIL OF AMERICA. 6300 ENTERPRISE LANG. MOTSON, H. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE LINGLID TO CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAMELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING. 4X4 // R = 467U=133 W=3.5" 3 X 4 ≡ 2X3 **Ⅲ** Design Crit: 9-14 * 12 13-0-0 BP8) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 0ver 2X3 III 2 X 3 3 \ 4 == 2 Supports 10 6-2-2 2X3 III 3 X 4 ≡ Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Calculated horizontal deflection is 0.17" $0.16\,^{\rm m}$ due to dead load. Wind reactions based on MWFRS pressures. Bottom chord checked for 20.00 psf non-concurrent live load SPECIAL LOADS R-459 From 6 From 3X3<u>=</u> 3×4// U=170 W=3.5" M 68 PLF at 66 PLF at 4 PLF at DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
68 PLF at 0.00 to 68 PLF at 6.82
66 PLF at 6.82 to 66 PLF at 13.00
4 PLF at 0.00 to 4 PLF at 13.00 SAN SAN TANOIS 19-5-11 3-11 10 SPACING BC LL BC DL TC DL DUR FAC TC LL TOT.LD. FL/-/4/-/-/R/due to 40.0 1.25 10.0 PSF 10.0 PSF 20.0 PSF 24.0" 0.0 PSF PSF load and JREF-DATE REF SEQN-HC-ENG DRW HCUSR8228 07197077 Scale = .3125"/Ft. R8228- 81260 1T938228Z01 TCE/WHK 07/16/07

Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / PLT 110 mph wind, 25.24 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi (+/-)=0.18Deflection meets L/240 live and L/180 total load. Creep increxem factor for dead load is 1.50. End verticals not exposed to wind pressure In lieu of rigid ceiling use purlins to brace BC @ ITW Building Components Group, Inc. Haines City, FL 33844 or Carlificate of Amborization # 667 TYP. 140B Isaac Construction Jeremy Cady ALPINE Wave #2 Dense #2 Dense #3 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEPARTON FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH PPI; OR FARRECTING, HANDLING, SHIPPING, INSTALLING & BRACHIGO F TRUSSES.

DESIGN CONFEDENCE WITH APPLICABLE PROVISIONS OF DROS (MATIONAL DESIGN SPEC, BY ATRY) AND TPI. ITH BCG COMMECTOR PLATES ARE HADE OF 70/19/16/GA (M.H/SS/K) ASTH A683 GRADU 40/60 (M. K/H.SS) GALV. STEEL APPLY PRATES TO EACH FACE OF TRUSS AND, UNILSS OHHERSISE LOCALED ON HIS DESIGN, POSITION FER DRAWHIGS 160A Z. ANY HISPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANHEX AS OF TPIT 2002 SEC.3. A SEAL ON THIS BESIGN AND THIS SHALL SECONDARY HISPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANHEX AS OF TPIT 2002 FEC.3. A SEAL ON THIS BRANDED FINANCE OF PROFESSIONAL GROUNDERS HILLY AND THE SHELLEY AND USE OF THIS COMPONENT FOR HIS DESIGN AND THE SHELLEY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE **WARNING** IRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING, SHIPPING, INSIALLING AND BRACING, BETER 10 BEST (BRIEDING COMPONENT SATETY INFORMATION), PUBLISHED BY TPT (TRUSS PLATE INSTITUTE, 218 HORSH LEE STREET, SUITE 312, ALEXANDREAL, NA, 25214) AND KINCA (MOOD TRUSS COUNCIL OF AMERICA, 6300 CHICKERS IS LAWE, MADISON, H. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HEST FUNCTIONS. UNLESS OTHERWISE HOLGLATED NO PUBB SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED ATTACHED RIGHD CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING. BRITBIRG 3×3= 3X4 // DESIGNER PER ANSI/TPI 1 SEC. **1** 2 - 5 - 4 → =116U=250 W=3.5" R=229 U=400 W=3.5" 3X4 =2 X 3 Ⅲ Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) ά 6 12 12 4-0-0-24" OC. BP7) 10 ά 0ver 2x'3'**Ⅲ** 3 \ 4 ≡ R=243 U=45 W=3.5" W 5 Supports A -4-0-0— 10 -2-2 2X3 III 3 X 4 ≡ MX Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. (A) 1x4~#3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.) nails @ 6" 0C. Wind reactions based on MWFRS pressures. SPECIAL LOADS Bottom chord checked for 20.00 psf non-concurrent live load. From From (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25) rom 68 PLF at 0.00 to 68 PLF at 6.70 rom 66 PLF at 6.70 to 66 PLF at 12.87 rom 4 PLF at 0.00 to 4 PLF at 12.87 3 X 4 // M TATE O w -10 SPACING BC LL BC DL TC DL TC LL DUR . FAC. TOT.LD. FL/-/4/-/-/R/ 40.0 10.0 10.0 20.0 24.0" 1.25 0.0 PSF PSF PSF PSF PSF JREF DATE REF SEQN-HC-ENG DRW HCUSR8228 07197088 Scale =.3125"/Ft. R8228-1T938228Z01 TCE/WHK 07/16/07 81261

H op 110 mph wind, 21.99 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18 Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. In lieu of structural panels or rigid ceiling use purlins brace all flat TC @ 24" 0C, all BC @ 24" 0C. PLT ITW Building Components Group, Inc. Haines City, FL 33844 Ft Contificate of Amborization # 447 chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 140B Isaac Construction TYP. 0 - 0 - 6ALPINE Wave 2X5 (B1) = R=177 W=4.596" 12 **IMPORTANT**THRHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BGG, INC. SHALL NOT BE RESPONSIBLE FOR ANY BUILDING FROM HIS DESIGN, ANY TAILURE TO BHILD THE TRUSS IN COMPORMANCE WITH THIS OF CARRICALING, WANDLING, SHIPPING, HISTALLING A BRACHING OF TRUSSES.

DESIGN COMPORES WITH APPLICABLE PROVISIONS OF HOS (MATIONAL DESIGNS SPEC, BY ATAPA) AND IPI.

CITY OF CARRICALING, WANDLING, SHIPPING, HISTALLING A BRACHING OF H.X. ATAPAY AND IPI.

HATES TO EACH FACE OF TRUSS AND. UNLESS OTHERNISL LOCATED ON HISTOSIGN, POSITION PER BRAHHING 160A Z.

ANY HISPECTION OF PALIES FOLLOWED BY (1) SHALL BE PER ANHER AS OF THIS DOES SEC.3.

ASSAULD HISTOSICH SHOULD AND SHOULD HEAD SHOULD AND SHOULD HE RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOULD HE SHOULD HEAD SHOULD HE SHEET HAS HELD HE RESPONSIBILITY OF THE A PROPERLY ATTACHED RIGID CETTING. 4-6-12-4-5-0 Jeremy Cady 4×4= Design Crit: 2X3 III 13-7-10 R = 316M 币 * U=200 W=3.5" Over DP1) ∞ TPI-2002(STD)/FBC Cq/RT=1.00(1.25) 4 œ Supports 2X3 III 2X3 III R=344 U=137 W=3.5" W 贞 ∞ ά 0 /10(0)Bottom chord checked for 20.00 psf non-concurrent live load Right end vertical not exposed to wind pressure. Wind reactions based on MWFRS pressures SPECIAL 7.36.042 From From 2 X 3 III 3 X 3 ≡ 2X3 III R=148 U=52 W=3.5" (LUMBER DUR.FAC.=1.25 中 LOADS 卓 2X3 **Ⅲ** M 68 PLF at -0.55 to 68 PLF at 4.42 to 4 PLF at -0.55 to ORIONAL ENGINEE -15 PLATE TE DUR.FAC.=1.25)
68 PLF at 4.42
68 PLF at 13.08
4 PLF at 13.08 SPACING ВС ВС TC DL TC LL DUR.FAC. TOT.LD. FL/-/4/-/-/R/-Ε P 10.0 20.0 40.0 PSF 10.0 PSF 24.0" 1.25 0.0 PSF PSF PSF SEQN-DATE REF HC-ENG DRW HCUSR8228 07197095 Scale R8228-1T938228Z01 =.375"/Ft. TCE/WHK 07/16/07 81262

JREF -

7 140B Isaac Construction Jeremy Cady CP2)

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

110 mph wind, 21.24 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Bottom chord checked for 20.00 psf non-concurrent live load

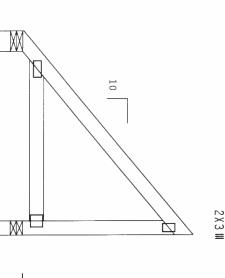
Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24° OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS

From (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25) rom 66 PLF at -0.62 to 66 PLF at 3.57 rom 4 PLF at -0.62 to 4 PLF at 3.57

Wind reactions based on MWFRS pressures

Right end vertical not exposed to wind pressure.



0-0-5

2X4(B1) =

3X3≡



R=158 W=5.077" 4-2-5 Over 2 Supports -> R=136 U=79 W=3.5"

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

7.36.042

QTY:2

FL/-/4/-/-/R/-

Scale

=.5"/Ft.

81263

ITW Building Components Group, Inc. Haines City, FL 33844 FT Cartificate of Amborization # 447 ALPINE

PLT

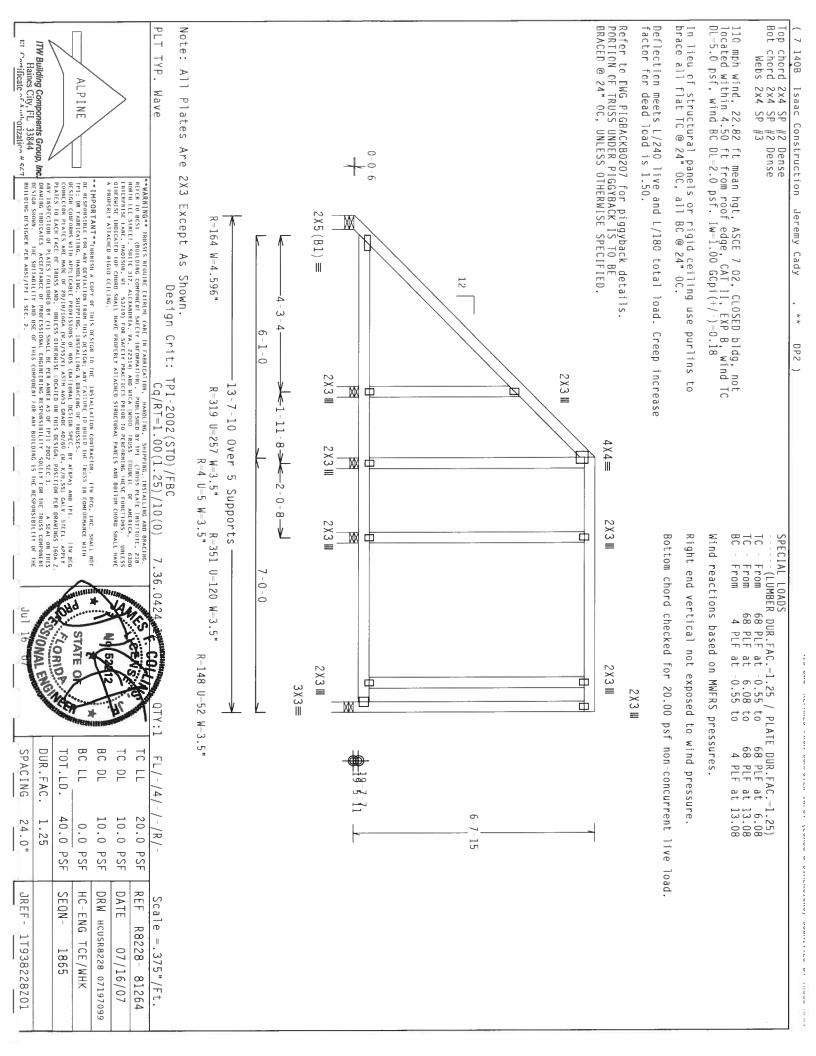
TYP.

Wave

#ARNING IRUSSIS REQUIRE EXIRENE CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING, REIER TO BCSI. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TIPI (TRUSS PLATE INSTITUIC, ZIB UNDERLING LES SEREIT, SUIC ALIZ ALEXANDRIA, NA, AZZIA) AND HICA (MODO TRUSS COUNCIL OF AMERICA. 6300 CHILERREISE LANE, HADISON, H. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE HIDICATED TOP CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAMELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED TO THE CONTROL OF THE CONTROL

IMPORTANTTURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY TALLER TO BUILD THE TRUSS IN COMPORANCE WITH PRISON FARREACHING, MANDLIGE. SHEPPING, INSTALLIGA & BRACHING OF TRUSSES. BY AFAPA, AND TPI. CREEKE PROVISIONS OF HIS SCHOOL OF TRUSSES. BY AFAPA, AND TPI. IT IN BCG CONNECTION PARTES AND THE ANALYSES OF TRUSSES, AND THE STATE AND THE STATE OF TRUSS, AND THE STATE OF TRUSS OF THE TRUSS OF TRUSS OF

SPACING BC LL BC DL TC DL DUR.FAC. TOT.LD. 40.0 PSF 10.0 PSF 20.0 PSF 24.0" 10.0 PSF 0.0 PSF JREF-SEQN-DATE REF HC-ENG DRW HCUSR8228 07197096 R8228-1T938228Z01 TCE/WHK 07/16/07



Top Bot Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\mathrm{.}$ Bottom chord checked for 20.00 psf non-concurrent live load (A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 110 mph wind, 23.65 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18 Note: All Plates Are (7 140B rı ~--ificate p chord 2x4 SP #2 I t chord 2x4 SP #2 I Webs 2x4 SP #3 Haines City, FL 33844

Haines City, FL 33844

"I O'--iffcate - C' A ...'t orizatic - # 607 TYP. Isaac Construction ALPINE Wave Dense Dense 2X3 Except As **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN TO HILD THE BUSSS IN COMPORMANCE WITH FP1; OR FLABELGHIGH, SHAPLING, HYACLING, A BRACING OF TRUSSES.

DESIGN CONTORNS HITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC, BY AFAPA) AND FP1. ITH BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (H.M/SS/K) ASTH A633 GRANDE 40/60 (H.K/H.SS) GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSSS AND; HUESS OHERHISE COCATE ON THIS DESIGN, POSITION FER DRAWHIGS 16GA Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE FER ANNEXS OF FP11 2002 SEC.3. A SEA, ON THIS DESIGN SHOWN.

DRAWHIGH HODICATES ACCEPTANCE OF PROFESSIONAL CHORDICATION RESPONSIBILITY SOLLY OR THE HUESS COMPONENT OF THE SOLIAN FOR THE HUESS COMPONENT OF THE SOLIAN FOR THE DRAHING INDICATES ACCEPTANCE OF PROTESSIONAL ENGINEERING RESPONSIBILITY DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ARY BUILD BUILDING DESIGNER PER ANSI/IPT I SEC. 2. **WARNING** TRUSSES REQUIRE EXTRINE CARE IN FARRICATION, HANDIENG, SUPPING, INSTALLING AND BRACING, REFER TO RESE. (BUILDING COMPONENT SAFETY INFORMATION), POBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 HORBIN LE STREET, SUTIE 112. ALEXANDRIA, VA. ZZZIA) AND WICA (MODO TRUSS COUNCIL DE AMERICA. 6300 ENTERPRISE LANE, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OFMENUST HORIZOTED IDE CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SMALL HAVE A PROPERLY ATTACHED RIGID CEILING. Jeremy Cady 0-0-6 Shown. Design Crit: 2×5′(B1) * R=155 DP3) 0 = 1512 W=4.596" TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 6 ώ 0C. 13-7-10 Over -9-0 2X3 III 2 X 3 Ⅲ R=384 U=287 W 4 4 Supports W=3.5" -2-12-Right end vertical not exposed to wind pressure. Wind reactions based on MWFRS pressures In lieu of structural panels use purlins to brace all flat 24" OC. SPECIAL 2 X 3 -- (LUMBER From 6 \equiv LOADS 2X3 III 2X3 III ER DUR.FAC.=1.25 / PLA
68 PLF at -0.55 to
68 PLF at 9.35 to
4 PLF at -0.55 to R=355 U=159 W=3.5" R=92 U=46 W=3.5" WX. 'n ATE O 2X3 III 2X3 III 3X3≡ PLATE (A)TE DUR.FAC.=1.25)
68 PLF at 9.35
68 PLF at 13.08
4 PLF at 13.08 BC LL SPACING TC DL ВС TC LL DUR . FAC TOT.LD. FL/-/4/-/-/R/ PL 40.0 10.0 10.0 20.0 24.0" 1.25 0.0 8 PSF 0 PSF PSF PSF PSF JREF -SEQN-DRW DATE Ţ HC-ENG Sca HCUSR8228 07197013 l e R8228-1T938228Z01 =.3125"/Ft. TCE/WHK 07/16/07 œ 81265 Ń

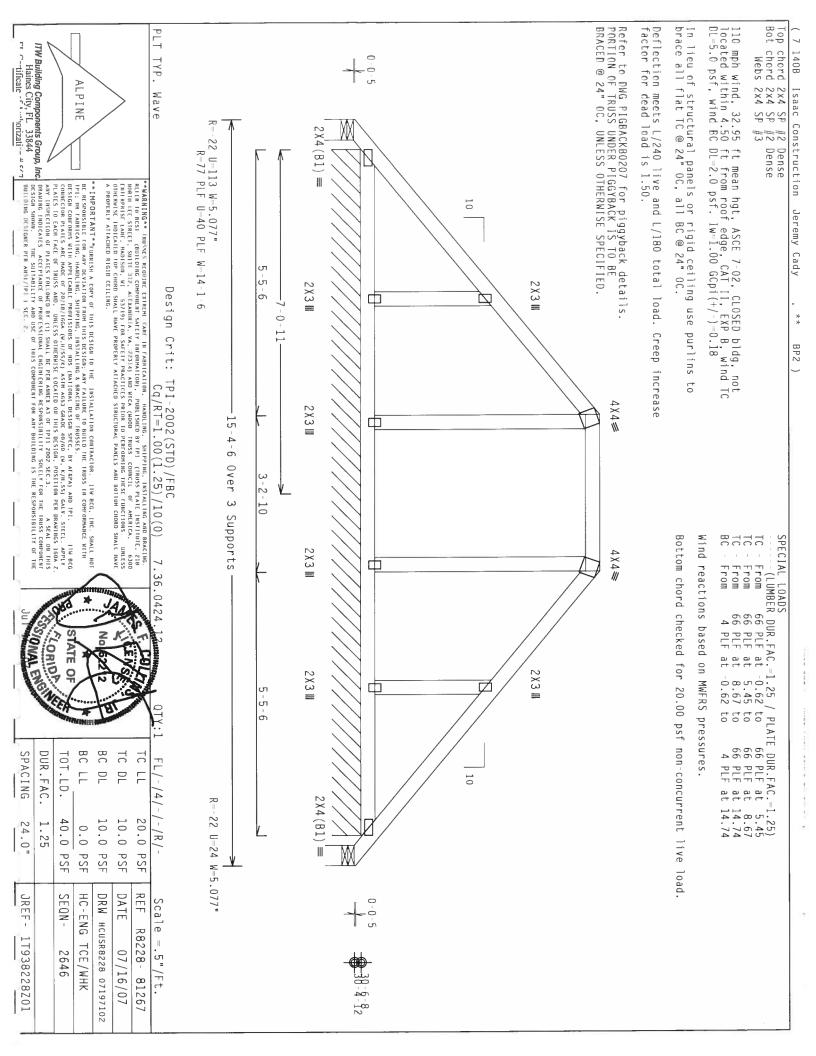
Bot 110 mph wind, 23.46 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18 Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Deflection meets L/240 live and L/180 total load. factor for dead load is $1.50.\,$ In lieu of structural panels or rigid ceiling use purlins brace all flat TC @ 24" 0C, all BC @ 24" 0C. ITW Building Components Group, Inc. Haines City, FL 33844 FI Cardificate of Amborization # 567 p chord 2x4 SP #2 [t chord 2x4 SP #2 [Webs 2x4 SP #3 140B Isaac Construction TYP. ALPINE Wave Dense Dense 0 0 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH IP: OR FABREACHING, HANDLING, SHAPPING, HISTALLING A BRACHING OF TRUSSES.

DESIGN CONTRESS HITH APPLICABLE PROVISIONS OF THOS (MATIONAL DESIGN SPEC, BY AFRA) AND IP!. ITH BCG CONTRECTOR PLATES ARE HADE OF 20/14/166A, MALMYSKY, SATH A653 GRADE 40/166 (M. KM. 55) GALV STEEL. APPLY PLATES TO LACH FACE OF TRUSS AND. UNICESS ONLERNISE LOCATED ON THIS DESIGN, POSITION PER DRAWNING 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AND THIS DESIGN, POSITION PER DRAWNING 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AND THIS DESIGN SHOWN.

DRAWNING HODICATES ACCEPTANCE OF PROFESSIONAL EMPLOYED FOR THE TRUSS COMPONENT DESIGN SHOWN.

DRAWNING HODICATES ACCEPTANCE OF PROFESSIONAL EMPLOYED FOR THE TRUSS COMPONENT DESIGN SHOWN.

THE SULTABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BULLDING Jeremy Cady 2X5(B1) =DESIGNER PER ANSI/TPI 1 SEC. R = 146U=41 9 W=4.596Design Crit: 0 2X3 III 2 X 3 III Creep increase R=264 U=246 W=3.5" -4-4 DP4) 12 TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 10 -0-0-0ver 4 2X3<u>₩</u> 4×4= Supports R 398 U 199 G W=3Right end vertical not exposed to wind pressure. Wind reactions based on MWFRS pressures. Bottom chord checked for 20.00 psf non-concurrent live load ά SPECIAL LOADS P' -12 . 5 2 X 3 ■ 2 X 3 R-177 U-68 W-3.5" = 3X3≡ 2X3 III 68 PLF at 68 PLF at 4 PLF at DUR.FAC.=1.25 M CEN =1.25 / PL =0.55 to 7.35 to =0.55 to PLATE TE DUR.FAC.=1.25)
68 PLF at 7.35
68 PLF at 13.08
4 PLF at 13.08 BC LL BC DL TC DL SPACING TOT.LD. DUR.FAC. FL/-/4/-1.25 20.0 24.0" 40.0 PSF 10.0 PSF 10.0 PSF 0.0 PSF PSF JREF SEQN-DATE HC-ENG DRW HCUSR8228 07197100 Scal æ R8228-=.3125"/Ft. 1T938228Z01 TCE/WHK 07/16/07 1885 81266



l op Bot 110 mph wind, 22.46 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi (+/-)=0.18 Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. In lieu of structural panels or rigid ceiling use purlins brace all flat TC @ 24" 0C, all BC @ 24" 0C. PLT Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\mathrm{cm}$ Note: All Plates Are 7 140B Isaac Construction Haines City, FL 33844
FI Conficate of Ambridge # 567 chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 TYP. ALPINE Wave 0-0-6 2 X 3 **IMPORTANT**FURNISH A COPY OF THIS DESIGN 10 THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH TO, OR FARRICATHUR, NANDLUG, SHEPPING, HISTALLING A BRACTING OF TRUSSES, DESIGN OF TRUSSES, DESIGN OF TRUSSES, AND SHEEL ARDE OF ZO/IN/160A (M.H/SS/M.) ASH A653 GRADE 40/60 (M. K/M.SS) GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND. UNICSS OTHERNISE LOCATED ON THIS DESIGN, POSITION PER BRAVINGS 160A Z. ANY INSPECTION OF LATES FOLLOWED BY (1) SHALL BE FER ANHEX AS OF TPI) ZOOZ SEC 3. A SEAL ON THIS DESIGN OF THIS DESIGN OF THE TRUSS COMPONENT OF THE BOST OCCUPONENT OF THE SHALL AND THE SHALL ACCUPANT OF THE TRUSS COMPONENT OF THE SHALL AND THE SHAL BUILDING DISIGNER PER ANSI/IPI 1 SEC. **MARNING** HRUSCES REQUIRE EXTREME CARE IN FARBICATION, HARDING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY HUPOWALIDE), POBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2019 MORTHLEE STREET, SUITE 312, ALEXANDRIA, VAA, 22314) ABD HICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LAME, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HEES FUNCTIONS, UNLESS OTHERWISE, HUDICALID HOP CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING. Except As Shown. Jeremy Cady 2X5 (B1) ≡ R = 165W=4.59612 Design Crit: * 0P5 2X3 III TPI-2002 (STD) /FBC Cq/RT=1.00(1.25) /10(0) 2X3 III 13-7-10 Over 6 Supports R=319 U=261 W=3.5" R=283 U=95 R=21 U=61 W=3.5" \mathbb{M} 卣 4×4= 2X3 III W 2-9-4 2X3 **Ⅲ** 2X3 III W Right end vertical not exposed to wind pressure. Wind reactions based on MWFRS pressures Ф Bottom chord checked for 20.00 psf non-concurrent live load SPECIAL -8-12 From (LUMBER LOADS 4-0-0 ¥=3. 68 PLF at 68 PLF at 4 PLF at DUR.FAC.=1.25 CORIO R = -2 X 3 Ⅲ 2 X 3 Ⅲ =1.25 / PL -0.55 to 5.35 to -0.55 to R=250 U=81 W=3.5" 53 U=14 W=3.5" W 車 中 2X3 III 3X3≡ W / PLATE DUR.FAC.=1.25)
to 68 PLF at 5.35
to 68 PLF at 13.08
to 4 PLF at 13.08 BC LL BC DL TC DL SPACING DUR.FAC. TOT.LD. FL/-/4/-/-/R/ 40.0 10.0 20.0 10.0 PSF 24.0" 1.25 0.0 PSF PSF PSF SEQN-JREF-DATE HC-ENG DRW HCUSR8228 07197103 Scale R8228- 81268 1T938228Z01 =.375"/Ft. TCE/WHK 07/16/07

Bot Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. In lieu of structural panels or rigid ceiling use purlins brace all flat TC @ 24" 0C, all BC @ 24" 0C. 110 mph wind, 21.46 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\mathrm{.}$ ITW Building Components Group, Inc.
Haines City, FL 33844

Pri Cardificate of Authorization # 647 chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 140B Isaac Construction Jeremy Cady TYP. ALPINE Маче "orizatic" " ec7 R=151 W=4.596' 2X5 (B1) ≡ 12 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DIVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH PI; DR FARBLEACHING, MAND LING, SHIPPIDG, INSTALLING A BRAIDE OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MOS (MATIONAL DESIGN SPEC, BY AFAPA) AND IPI. ITH BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (H.M/SS/K). ASTH AGAS GRANDE 40/26G (M.K/M.SS) GALV. SIEELA APPLY PLATES TO EACH FACE OF TRUSS AND, UNICESS OTHERWISE COCALD BY HIS DESIGN, POSITION PER DRAHINGS 160A A. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER RAHEZ A OF IPI) 200E YEC.3. A SEAL ON THIS DESIGN AND ANY INSPECTION OF PLATES FOLCOWED BY (I) SHALL BE PER RAHEZ AS OF IPI) 200E YEC.3. A SEAL ON THIS DRAHING HOLICATES ACCEPTANCE OF PROTESSIONAL UNCLUSTED HE RESPONSIBILITY OF THE BUSS COMPONENT DESIGN SHOWN.

THE SUITABLITY AND MES OF THIS COMPONENT FOR MAY BUILDING IS THE RESPONSIBILITY OF THE ANY INSPECTION OF PLATES FOLLOWED BY (1) SMALL BE PER ANNE)
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING &
DESIGN SHOWN. THE SUITABLITY AND USE OF THIS COMPONENT
BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. **WARNING** TRUSSES REQUIRE EXPREME CARE IN FABRICATION, IMBULIDE, SHIPPING, INSTALLING AND BRACHIG, REFER TO BEST. (BUILDING COMPONETY SAFETY INFOMENTION), PUBLISHED BY PIP (TRUSS PLATE INSTITUTE, 210 HORTH LEE STREET, SUITE 312, ALEXANDRÍA, VA, 22314) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LAME, MAISSON, MI 53719) FOR SAFETY PRACTICES, PRIOR TO PREFORMING MEST FUNCTIONS. UNLESS OTHERWISE HOLDSCHED FOR BOOD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE w 3-6-0 -4-4 Design Crit: 4×4 € 2X3 **Ⅲ** R=250 U=147 W=3.5" M 中 * DP6 13-7-10 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) ò 0ver 4 Supports 2X3 III 2X3 **Ⅲ** R = 411W 中 ف Right end vertical not exposed to wind pressure. Bottom chord checked for 20.00 psf non-concurrent live load Wind reactions based on MWFRS pressures SPECIAL LOADS œ U=141 W=3.5" 12 7.36.042 --(LUMBER from 6 From 6 From 6 ER DUR.FAC.=1.25 / PL/ 68 PLF at -0.55 to 68 PLF at 3.35 to 4 PLF at -0.55 to CORION MAIN 2X3 III 2 X 3 Ⅲ Ф Ф / PLATE DUR.FAC.=1.25)
to 68 PLF at 3.35
to 68 PLF at 13.08
to 4 PLF at 13.08 R=174 U=57 W=3.5" BC LL BC DL TC DL TC LL 3×3≡ SPACING DUR.FAC. TOT.LD. FL/-/4/-/-/R/-2X3 III 中 10.0 20.0 40.0 PSF 10.0 PSF 1.25 24.0" 0.0 PSF PSF PSF JREF-SEQN-DATE -11-3DRW HCUSR8228 07197104 HC-ENG Scale =.5"/Ft. R8228-1T938228Z01 TCE/WHK 07/16/07 81269

BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Refer to DWG PIGBACKB0207 for piggyback PORTION OF TRUSS UNDER PIGGYBACK IS TO F In lieu of structural panels use purlins to brace all flat TC $24\mbox{\ensuremath{^{\circ}}}\xspace$ 0C. 110 mph wind, 20.46 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/ $^{\prime}$)=0.18 Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 ALPINE

IMPORTANTTHRIISH A COPY OF THIS DESIGN TO THE IDSTALLATION CONTRACTOR. ITH BCG. LIC. SHALL HAVE

IMPORTANTTHRIISH A COPY OF THIS DESIGN TO THE IDSTALLATION CONTRACTOR. ITH BCG. LIC. SHALL HAVE

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ANY HAVE CITY OF THE SHOWN HAVE DESIGNED BY (1) SHALL BE PER AMER X3 DE FIT) 2002 SEC. 3.

ANY HAVE CITY OF THE SHOWN HAVE DESIGNED BY (1) SHALL BE PER AMER X3 DE FIT) 2002 SEC. 3.

BUILDING DESIGNER PER AMESI/FPT 1 SEC. 2.

**IMPORTANT **THREE TO MAKE AS DESIGNED BUILDING IS THE RESPONSED TO A SEAL ON THIS

DESIGNES SHOWN. HAVE

BUILDING DESIGNER PER AMESI/FPT 1 SEC. 2. PLT TYP. Note: All Plates Are 2X3 Except As Shown. 7 140B Isaac Construction Jeremy Cady Wave R=90 W=4.596" 2X5(B1) =12 1-6-0-1 1-4-4 **MARNING** RUSSES REQUIRE EXTREME CARE IN FABRICATION, MANDING, SHIPPING, INSTALLING AND BRACING, RETER TO BESI (BUILDING COMPONIENT SAFETY INFORMATION), PUBLISHED BY TPI (RUSS PLATE INSTITUTE, 218 UNGHI LEE SHRETE, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (MODD TRISS COUNCIL OF AMERICA, 6300 CHIERBASE LONE, MAISSON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS UNLESS OTHERMISE INFORMATION OF THE PROPERTY ATTACHED STRUCTURAL PAMELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PAMELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PAMELS AND BOTTOM CHORD SHALL HAVE $4 \times 4 =$ 2 X 3 III \mathbb{M} Ф 140 U=63 W=3.5" 2-9-4 details. Design Crit: * 2 X 3 III 2X3 Ⅲ DP7) M 由 Ф =242 U=74 W=3.5" 13-7-10 Over TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 4-0-0 5 Supports 11-8-12 2X3 III 2X3 III Bottom chord checked for 20.00 psf non-concurrent live load Wind reactions based on MWFRS pressures SPECIAL LOADS Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\cdot$ \mathbb{W} ф R=364 U=108 W=3. \Box 7.36.0424 From From (LUMBER R DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
68 PLF at -0.55 to 68 PLF at 1.35
68 PLF at 1.35 to 68 PLF at 13.08
4 PLF at -0.55 to 4 PLF at 13.08 CORIOR STATE * R=150 U=44 W=3.5" 2 X 3 III 2 X 3 III ф 中 SPACING BC LL BC DL TC DL TC LL 3X3≡ DUR.FAC. TOT.LD. FL/-/4/-2X3 III / - /R/ 40.0 10.0 20.0 10.0 PSF 1.25 0.0 PSF PSF PSF PSF DATE REF SEQN-HC-ENG DRW HCUSR8228 07197105 Scale =.5"/Ft. R8228-TCE/WHK 07/16/07 1940 81270

24.0"

JREF-

1T938228Z01

(7-140B--Isaac Construction Jeremy Cady * KP1) SPECIAL LOADS

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

110 mph wind, 21.32 located within 4.50 DL=5.0 psf, wind BC ft mean hgt, ASCE 7-02, CLOSED ft from roof edge, CAT II, EXP DL=1.2 psf.) bldg, not B, wind TC

Bottom chord checked for 20.00 psf non-concurrent live load

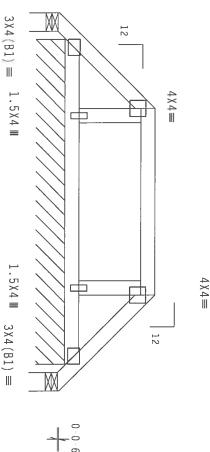
Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

From From (LUMBER ER DUR.FAC.=1.15 / PL 68 PLF at -0.55 to 68 PLF at 5.25 to 4 PLF at -0.55 to / PLATE DUR.FAC.=
to 68 PLF at
to 68 PLF at
to 4 PLF at .=1.15) t 5.25 t 7.22 t 7.22

their eath eith that frames a number and againthe at these

In lieu of brace all f structural panels or flat TC @ 24" OC, all rigid ceiling BC @ 24" OC. use purlins to

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



0-0-6

1-10-4

=26 U=180 W=4.596" R=72 PLF U=27 PLF 9-4 M=6-8 0ver ω Supports \mathbb{R} 26 U=180 W=4.596"

æ

ပ်

-9-15

1-5-1

3-4-0

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25) /10(0)

PLT

TYP.

Wave

MARNIG IRUSCIS REQUIRE EXTREMÉ EARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER NO BEST. (BUILDING COMPONENT SATETY IN DOMAITON), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, ZIB NORTH LEE STREET, SUITE 312. ALEXANDRIA, VA, 22314) AND HEAT (AVOID TRUSS COUNCIL DE AMERICA. 6300 ERHIERRISE LAME, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE HOLGLIFD FOR PHOROS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING.

IMPORTANTrurrish a copy of this design to the installation contractor. The beg, inc. shall not be received by any deviation from this design, any fallure to build the thuss in comformance with the received by any deviation. Annothing, sumpplie, installing a bracing of thusses, which are conformed by the period bested conformed with applicable provisions of 1005 (national design spec, by Atra) and Del. I've bested conformed with applicable provisions of 1005 (national design spec, by Atra) and Del. I've bested conformed with a specific provision of 20/18/166A (M.1/55/K) ASTA A653 GRADE 40/60 (M. K/M.SS) GALV. SIEEL APPLY PLAIES DE EACH FACE OF TRUSS AND. UNLESS OTHERRISE LOCATED ON THIS DESIGN, POSITION PRE DRAWINGS 160A-Z. ANY INSPECTION OF PLAIES FOLLOWED BY (I) SHALL BE PER AHIREX A0 IF THI 2002 SEC.3. A STAL ON THIS DESIGN SHOULD BE COMPORED TO THE SULFACE OF PROFESSIONAL ENGINEER A0 OF FPI1 2002 SEC.3. BEACH THE SULFACE OF PROFESSIONAL ENGINEER ACCORDING TO THE SULFACE OF PROFESSIONAL ENGINEER ACCORDING TO THE SULFACE OF THE TRUST OF THE SULFACE OF THE SULFA DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

7.25. CORIOR 0TY:1SPACING TC DL BC LL BC DL TC LL DUR . FAC TOT.LD. FL/-/4/-/-/R/-40.0 10.0 10.0 20.0 24.0" 1.15 0.0 PSF PSF PSF PSF PSF JREF -REF SEQN-DATE DRW HCUSR8228 07197106 HC-ENG Scale = R8228-

TCE/WHK 21605

REV

07/16/07

81271

.5"/Ft.

1T938228Z01

Bot 110 mph wind, 22.67 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. lw=1.00 GCpi $(\pm/-)$ =0.18 Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. In lieu of structural panels or rigid ceiling use purlins brace all flat TC @ 24" 0C, all BC @ 24" 0C. Note: All Plates Are 2X3 Except As Shown. ri C-tificate p chord 2x4 SP #2 I t chord 2x4 SP #2 I Webs 2x4 SP #3 140B Isaac Construction 0-0-5 TYP. ALPINE Wave $2 \times 4 (B1) \equiv$ 10 R=118 U=30 W=5.077" 0-10-15 Dense Dense $4 \times 4 \equiv$ 2 X 3 Ⅲ -3-0-2-**IMPORTANT**TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG. INC. SHALL NOT RE RESPONSIBLE FOR ANY DEPLATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE BRUSS IN COMPORMANCE WITH IP) OR FARECALING, ANDLING. SHEPPIG. INSTALLING A BRACLING OF TRUSSES. BY ATAMA AND FFI. DESIGN CONTROL OF TRUSSES. BY ATAMA AND FFI. ITH BCG CONNECTION PARTS ARE AND FOR PROPERLY AND FROM THE CONTROL OF TRUSSES. BY ATAMA AND FFI. ITH BCG CONNECTION PARTS AND THE CONTROL OF ZOLOGIA GROAD AND AND THE SECOND OF ZOLOGIA GROAD AND AND THE SECOND OF ZOLOGIA GROAD AND AND THE SECOND OF PARTS TOLOGRAD BY C1) SHALL BE FER ANNEX A 30 FPD1 2002 SCC.3. AS SEAL ON THIS DESIGN SHOWN. THE SULFABLITY OF THE FOR THE SECOND USE OF THIS COMPONENT OF SHORT SHOWN AND THE SULFABLITY OF THE SECOND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2 **MARNING** HRISTSE REQUIRE EXTREME CARE IN FABRICATION, PROBLEME, SHIPPING, INSTALLING AND BRACHIG,
REFER TO BEST (BUILDING COMPONIENT SAFETY INFORMATION), PROBLEMED BY TPI (TRUSS PLATE HISTITUTE, 218
UNDITH LEE SHREET, SUITE 312, ALEXANDRIA, VA, 22314) AND HICA (MOOD TRUSS COUNCIL OF ARERICA, 6300
ERHEAPISE LANGE, MAISSON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERATISE LANGERD TOP COMBOS SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED REGID CELLING. Jeremy Cady 2 X 3 III 2X3 III R=276 U=102 W=3.5" MX ф ф 4-0-0-Design Crit: * 2X3 III 2X3 AP1) 4 =282 U=99 W=3.5" TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 22-3-14 Over -4-0-0-3X4≡ 2X______ 2X3 W 20-9-7 7 Supports 44 R=278 U=97 W=3.5" 3 X 4 ≡ 4-0-0-Ε Wind reactions based on MWFRS pressures Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Bottom chord checked for 20.00 psf non-concurrent live load SPECIAL LOADS --(LUMBER From 6 From 6 From 7.36.0424 2X3 W 2X3 III æ 4 -287 66 PLF at -0.62 to 66 PLF at 0.91 to 4 PLF at -0.62 to DUR.FAC.=1.25 U=99 No | 522 STATE O 4-0-0-<u>₹</u>=3 .5 2 X 3 2X3 III / PLATE DUR.FAC.=1.25)
to 66 PLF at 0.91
to 66 PLF at 21.70
to 4 PLF at 21.70 4 R=251 U=87 W=3 R=61 U=22 W=3.5" 1 U=87 W=3.5" BC LL BC DL TC DL SPACING DUR FAC TOT.LD. FL/-/4/-/-/R/-3X3≡ 2 X 3 III M 40.0 10.0 20.0 10.0 PSF 24.0" 1.25 0.0 PSF PSF PSF PSF JREF-SEQN-DATE DRW HCUSR8228 07197107 HC-ENG Scale =.3125"/Ft. R8228-1T938228Z01 TCE/WHK 07/16/07 81272

7 140B Isaac Construction Jeremy Cady * R2)

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 :W1, W2, W9, W10 2x4 SP #2 Dense:

Wind reactions based on MWFRS pressures

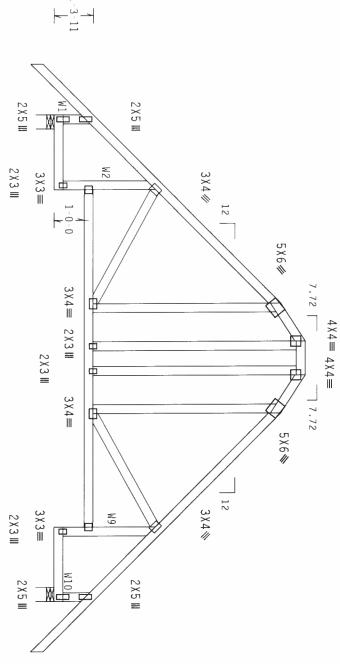
In lieu of structural panels use purlins to brace all flat TC @ $24\ensuremath{^{\circ}}\xspace$ 0C.

110 mph wind, 15.97 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Calculated horizontal deflection is 0.15" due to live load and 0.31" due to dead load.

Bottom chord checked for 20.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT

TYP.

Wave

MARNING RUSSES REQUIRE EXTREME CARE IN FABRICATION, IMADILING, SHIPPING, INSTALLING AND BRACING RETER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), POULISHED BY THE (TRUSS PLATE INSTITUTE, 218 DORIN LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND RICA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LOUICE, SUITE 312, ALEXANDRIA, VA, 22314) AND RICA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE (ADDICATE) AND SOUTH OF SAFETY PRACTICES, BRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE UNDICATED TOP CORONS SMALL MAVE PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL MAVE PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL MAVE

ALPINE

ITW Building Components Group, Inc. Haines City, FL 33844 **IMPORTANT**FURNISH A COPY OF THIS DESIGN 10 THE INSTALLATION CONTRACTOR. THE BCG, INC. SMALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, FOR FAILURE TO BULLD THE TRUES IN COMPORMANCE WITH FPI; OR FARRICKING, HANDLIGG, SUPPING, HISTALLING A BRACHE OF TRUESES.

DESIGN CONTRORS WITH APPLICABLE PROVISIONS OF DIDS (MALIONAL DESIGN SPEC, BY ATAPA) AND TPI.

DESIGN CONTROLS ARE ALGE OF PROVISIONS OF DIDS (MALIONAL DESIGN SPEC, BY ATAPA) AND TPI.

PLATES TO FACT FOR THE STATE ARE ALGE OF TRUES OTHERWISE LOCATED ON THIS DESIGN, POSITION FER DRAMHIGS 160A Z.

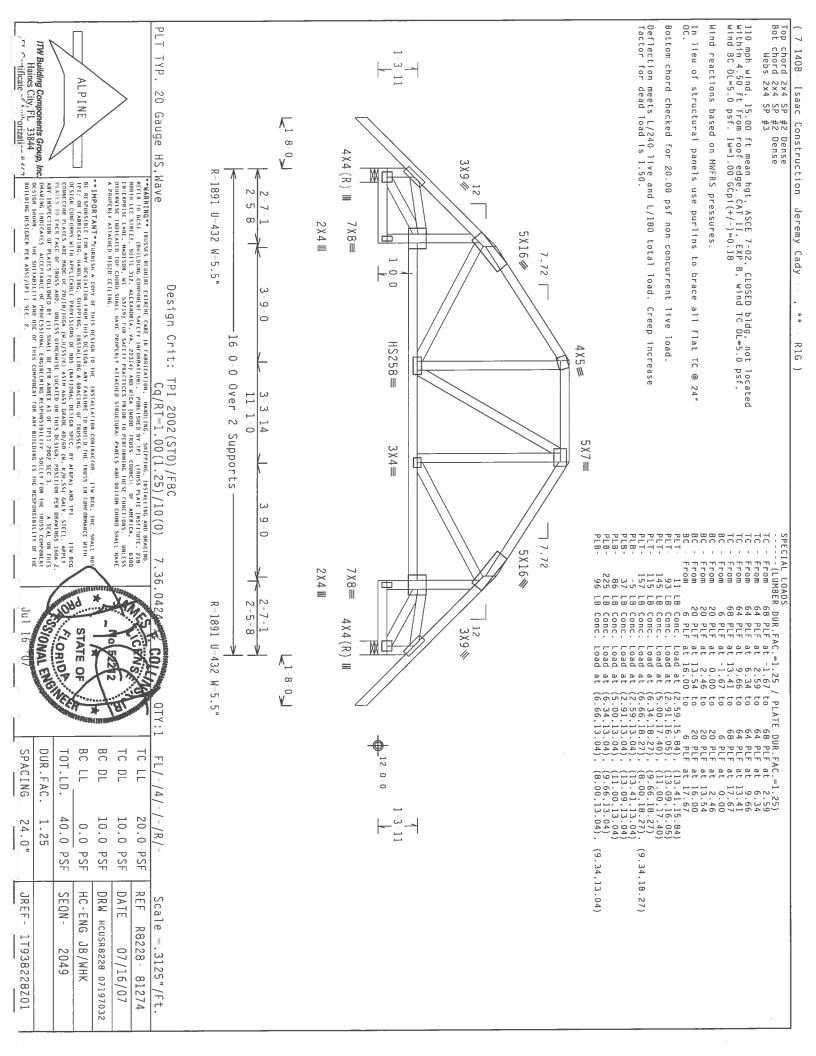
ANY HISTOLICION OF PLATES TOLLOWED BY C1) SHALL BE FPR ANIER X3 OF TPI1 2002 SEC 3.

AS S.A. ON THIS STATEMENT OF THE SULTABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUSINGLE OF PROFISSIONAL ENGLIEFERING RESPONSIBILITY SOLELY FOR THE RESPONSIBILITY OF THE BUSINGLE OF PROFISSIONAL ENGLIEFERING RESPONSIBILITY SOLELY FOR THE RESPONSIBILITY OF THE BUSINGLE OF PROFISSIONAL ENGLIEFERING RESPONSIBILITY SOLELY FOR THE RESPONSIBILITY OF THE BUSINGLE OF PROFISSIONAL ENGLIEFERING RESPONSIBILITY SOLELY FOR THE RESPONSIBILITY OF THE BUSINGLE OF PROFISSIONAL ENGLIEFERING RESPONSIBILITY SOLELY FOR THE SULTABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE



	NG.		- American		The same of the sa	SA
SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	TC LL
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
JREF- 1T938228Z01		SEQN- 2014	HC-ENG JB/WHK	DRW HCUSR8228 07197010	DATE 07/16/07	REF R8228- 81273

Scale = .3125"/Ft.



7 140B Isaac Construction Jeremy Cady * R3)

l op Bot chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense :W3, W4, W5 2x4 SP

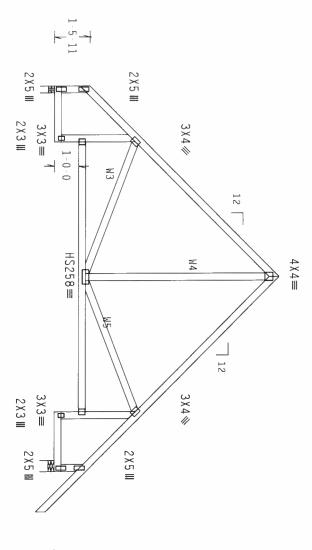
Wind reactions based on MWFRS pressures

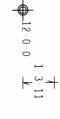
Bottom chord checked for 20.00 psf non-concurrent live load.

110 mph wind, 16.47 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Calculated horizontal deflection is 0.17" due to live load and 0.31" due to dead load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.







Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT

TYP.

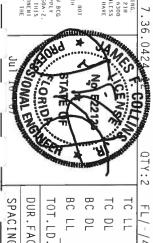
20 Gauge HS, Wave

WARNING TRUSSES REQUIRE EXTRINE CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, RETER TO BEST (BUILDING COMPORENT SAFETY INFORMATION), PUBLISHED BY FIP] (TRUSS PLATE INSTITUTE, ZIB HORTH LEE STREET, SUITE IZZ ALEXANDRAL, VA, 223.14) AND HICKA (MODO TRUSS COUNCIL OF AMERICA, 6300 CHITEPRESE LAME, MADISON, HI 53719) FOR SAFETY PRACTICES PHIOR TO PERFORMING THESE FUNCTIONS, UNITESS OTHERNISE HOUSE, CALED TO PEORDE SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGHD CELLING.

ALPINE

Haines City, FL 33844
Haines City, FL 33844
Haines City, FL 3844 **IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BLE R SPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FALLURE FOR BUILD THE TRUES IN COMPORNANCE WITH PIC OR FARECKING. NAMELIGE, SHEPPIDE, HISALLING & BRACHING OF TRUESES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF HIDS (MATIONAL DESIGN SPEC, BY AFAPA) AND FILL HIS DESIGN CONFICENCY PLATES OF EACH FACE OF TRUESS. AND THIS DESIGN APPLY PLATES TO EACH FACE OF TRUESS, AND THIS LINES OF THE STREET OF



SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	10 11
VG 24.0"	AC. 1.25). 40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
JREF 1T938228Z01		SEQN- 2011	HC-ENG JB/WHK	DRW HCUSR8228 07197064	DATE 07/16/07	REF R8228- 81275

Scale = .

25"/Ft

TC - From
TC - From
BC - From
BC - From Top See DWGS All015EC0207 & GBLLETIN0207 for more requirements. Deflection factor for SPECIAL LOADS C - From 7 140B Isaac Construction Jeremy Cady chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 :W1 2x8 SP TYP. -(LUMBER DUR.FAC.=1.: From 110 PLF at -1 From 110 PLF at 6 From 10 PLF at 6 From 6 PLF at -1 From 20 PLF at 0 ALPINE meets L/240 live and L/180 total load. Creep increase dead load is 1.50. 20 Gauge HS, Wave 1-3-12 1.25 / -1.67 0.96 6.23 -1.67 0.00 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, THC. SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN, ANY TAILURE TO BUILD THE RUSS IN COMPORMANCE WITH IP: OR FARRICATHIG, ANNIHIG, SHAPPING, HISTALLING A BRACTHO OF TRUSSES.

DESIGN CONFERENCE AND THOSE OF TRUSS OF HOS (MATIONAL DESIGN SPEC, BY AFRAY) AND IPI. ITH BCG CONNECTION PLATES AND THAT DESIGN AND ANY APPLY PLATES TO EACH ACC OF TRUSS AND AND APPLY PLATES TO EACH ACC OF TRUSS AND ANY APPLY PLATES TO EACH ACC OF TRUSS AND. UNITESS OTHERWISE ENGATED ON THIS DESIGN, POSITION PER BRAKHES HOS AND APPLY PLATES TO EACH ACC TRUSS AND. BUILDING DESIGNER PER ANSI/TPI 1 SEC. DESIGN SHOWN. I ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE to #2: NTE DUR.FAC.=1.25)
110 PLF at 0.96
110 PLF at 6.23
110 PLF at 13.13
6 PLF at 0.00
20 PLF at 13.13 R-145 PLF U-16 PLF W-13-1-8 HS610(R) / ¥ T 3X3 **Ⅲ** 3X3 ₩ Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/ 2 X 3 III 2X3 III S 12 5-0-14 13-1-8 Over Continuous Support 2 X 3 III 2 X 3 Ⅲ DESIGN SPEC, BY AFRPA) AND IPI.

3 GRADE 40/60 (N. K/H.SS) GALV. SIEEL. APPLY
ON THIS DESIGN, POSITION PER DRAHINGS 160A Z. 4 X 4 == 2 X 3 III 2 X 3 III 2X3 **||** A SEAL ON THIS FOR THE TRUSS COMPONENT THE RESPONSIBILITY OF THE /10(0)2 X 3 III 2 X 3 Ⅲ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 Fasten rated sheathing to one face of this frame Bottom chord checked for 20.00 psf non-concurrent live load. In lieu of structural panels use purlins to brace all flat TC @ 24" OC. Right end vertical not exposed to wind pressure. Wind reactions based on MWFRS pressures. 7.36.0424 2 X 3 2 X 3 III STONAL ENGREE 2 X 3 III 2 X 3 III SPACING BC LL BC DL TC DL TC LL DUR.FAC. TOT.LD. FL/-/4/-/-/R/-9 40.0 PSF 10.0 PSF 10.0 PSF 20.0 PSF 24.0" 1.25 0.0 PSF DATE JREF-REF SEQN-HC-ENG DRW HCUSR8228 07197016 Scale = .3125"/Ft. R8228- 81276 1T938228Z01 JB/WHK 07/16/07 2327

Top chord 2x4 SP #2 Dense :T2 2x6 SP #1 Dense: :T3 2x6 SP #2: Bot chord 2x8 SP SS :B1 2x8 SP #2: :B3 2x4 SP #2 Dense: Webs 2x4 SP #3 Calculated horizontal deflection is 0.25" due to live load and 0.46" due to dead load. Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling. In lieu of structural panels use purlins to brace all flat TC @ 24" OC. PLT TYP. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50.\,$ 7 140B Isaac Construction Jeremy Cady Haines City, FL 33844
Fit Conficate of Authorization # 567 ALPINE 18 Gauge HS, Wave 1-8-0 3×5 III 4X7/ \Box W =2059 U=45 W=5.5' **WARNING** HRUSERS BEQUIRE CERTERE CARE IN FARRICATION, HANDLING, SHIPPING, HISTAILING AND BRACING.
RETER TO BESS! GUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY FIT (TRUSS PLATE HISTIDUTE, 218
HORRIN LEE SIREE, SUITE 312, ALEXANDRIA, VA, 22314) AND NICA (MODD TRUSS COUNCIL OF AMERICA, 6300
ENTERPRISE LINIE, ANDISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERHISE HIDTOLATED FOR COMED SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING. ANY INSPECTION OF PLATES FOLLOHED BY DRAWLING INDICATES ACCEPTANCE OF PROFESSIONS. THE SUITABILITY AND BUILDING DESIGNER PER ANSI/TPI 1 SEC **IMPORTANT** TRANSIS A CONY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BGG. HE SMALL NOT BU RESPONSIBLE TERM ANY DEVILLION FROM HIS DESIGN, ANY FALLING TO BRILLD THE TRUSS IN COMPORMANCE WITH 191: OR FARRICATING, MANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFIDENT AND THE OF TO/LEPING A (H.1/55X) ASTH AGS GRADE 40/50 (K. K/M.35) GALV. STEEL, APPLY COMMICTOR THATES ARE AND OF TO/LEPING A (H.1/55X) ASTH AGS GRADE 40/50 (K. K/M.35) GALV. STEEL, APPLY 12 B 1 4 X 5 == 3X4 ∕ 7-11-5 4×5 / $6 \times 4 \equiv$ Design Crit: 6 X 4 == 2 X 8 III 2X3≡ S4 2 X 3 III TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 24-6-0 Over 2 Supports 5 X 6 == α Ö 83 -12-0 10-4-11 2X3 III 4 X 7 = 3. A SEAL ON THIS FOR THE TRUSS COMPONENT HE RESPONSIBILITY OF THE 110 mph wind. 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from $6 \cdot 4 \cdot 0$ to $18 \cdot 4 \cdot 0$. (A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" Bottom chord checked for 20.00 psf non-concurrent live load. Right end vertical not exposed to wind pressure. Wind reactions based on MWFRS pressures. $3X3 \equiv$.36.0424 4X10(R) III $3X4 \equiv$ SS0612= Jul 16 (A)ò CORIO 4-10-0 R-1939 U-142 W-3.5" 5 X 6 (R) Ⅲ 3X5 Ⅲ BC DL SPACING ВС DUR.FAC. TC C TOT.LD. FL/-/4/-/-/R/- $\widehat{\mathbb{A}}$ Ε DL 10-0-0 24.0" 1.25 40.0 10.0 10.0 PSF 20.0 0.0 PSF PSF PSF PSF 10 _ 00. DATE JREF SEQN REF DRW HCUSR8228 07197041 HC-ENG Scale = .25"/Ft. R8228-17938228201 JB/WHK 07/16/07 81277

Top choru :: 13 2x6 SP #2: Bot chord 2x8 SP #2: Webs 2x4 SP #3 PLT BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from $6 \cdot 4 \cdot 0$ to $18 \cdot 4 \cdot 0$. Calculated horizontal deflection is 0.11" due to live load and 0.20" due to dead load. Bottom chord checked for 20.00 7 140B Isaac Construction Jeremy Cady ITW Building Components Group, Inc. Haines City, FL 33844 chord 2x4 SP #2 Dense :T2, TYP. ALPINE Wave :B3 2x4 SP #2 Dense: **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH TP: OR FARREACHING, MANDLING, SHIPPIDE, HISTALLING A BRACING OF TRUSSES, DESIGN CONTREMS HITH APPLICABLE PROVISIONS OF ANDS (MATIONAL DESIGN SECC. BY ACTAPA) AND IPI. THE BCG CONNECTOR PLATES ARE HADE OF 20/19/166A (M.H/SS/K) ASTH AGS3 GRADE 40/60 (M. K/M:SS) GALV. STEEL, APPLY PLATES TO LACH FACE OF TRUSS AND. UNICSS OHERHISE LOCATED ON THIS DESIGN, POSITION PER DRAHINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF FPI1 2002 SEC. 3. A SEAL ON THIS DRAHING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLLY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SULFAMILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE A PROPERLY ATTACHED RIGID CEILING. T4 2x6 SP #1 psf non-concurrent live load. 1-8-0 DESIGNER PER ANSI/IPI 1 SEC. 4X7// 12 · 3X5 **Ⅲ** R=2066 4 X 5 == Design Crit: U=85 W=5.5" 3 X 5 ₪ Dense: 4×5% 6X6≡ 4X10(R) Ⅲ ဟ် 2 X 9 III S2 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 3X3= 24-8-0 Over 2 Supports 3 X 4 ≡ 5 X 4 ≡ œ Ö 4-9-14 8-9-6 12-0 ВЗ ò 2X3 III 5×6= 3X3≡ Wind reactions based on MWFRS pressures 110 mph wind, 15.44 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC Dl=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 Collar tie braced with continuous lateral bracing at 24" rigid ceiling. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. In lieu of structural panels use purlins to 24" OC. 4X10(R) III 2 X 9 III -11-5 €X6= 4 X 5 // 4 X 5 ≡ SONAL ENGRAPHIC 3 X 5 // ഗ R=2066 U=85 W=5.5" -0-0 ¹ 12 4 X 7 **⟨** 3X5 **Ⅲ** 1-8-0 SPACING BC LL 90 DF TC DL **4** = DUR.FAC. TOT.LD. FL/-/4/-/-/R/-brace all flat TC 1.25 40.0 24.0" 10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF PSF JREF-DATE REF SEQN-HC-ENG DRW HCUSR8228 07197059 Scale =.1875"/Ft. 9 R8228- 81278 1T938228Z01 JB/WHK 07/16/07

7 140B Isaac Construction Jeremy Cady 2x4 SP #2 Dense :T2, T4 2x6 SP #1 Dense: S3 יוים משת ושבו ששבם ושמוד במנו מוכנו לבתשמת ש מזוובשת במשל תמחודו בר מו ווחסים ווושי

Top chord 2x4 SP #2 Dense :T2, T4 :T3 2x6 SP #2: Bot chord 2x8 SP #2 :B2 2x8 SP SS: :B3 2x4 SP #2 Dense: Webs 2x4 SP #3

Calculated horizontal deflection is 0.11" due to live load and 0.20" due to dead load.

Bottom chord checked for 20.00 psf non-concurrent live load

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-4-0 to 18-4-0 .

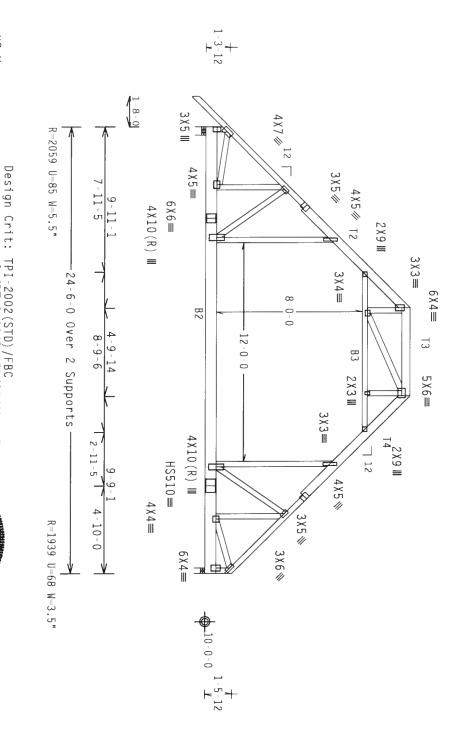
110 mph wind, 15.44 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

In lieu of structural panels use purlins to brace all flat TC 24" OC.

Collar tie braced with continuous lateral bracing at 24 $\!\!^{\circ}$ OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



20 Gauge HS, Wave **WARNING** IRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING, MILER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TIP (TRUSS PLATE INSTITUTE, 218 HORTH LEE STREET, SUITE 312, ALEXANDRA, NA. 22314) AND HICA (MODD TRUSS COUNCIL OF AMERICA. 6300 ENTERPRISE LAWE, MADISON, WI 53710) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNITSS OTHERNISE INDICATED FOR CHERD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING. TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT TYP.

ALPINE

| **Important**rubilish a copy of this design to the installation contractor. The beg. the shall hot be accomposed to the controlled from the design to the installation begins the conformance with definition controlled the reports of the controlled from the state of the controlled from the control



	ANN THE PROPERTY OF					
SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	וכ רר
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
JREF - 1T938228Z01		SEQN- 1715	HC-ENG JB/WHK	DRW HCUSR8228 07197065	DATE 07/16/07	REF R8228- 81279

Scale =.1875"/Ft.

(A) 1x4 with 8d l op Bot 110 mph wind, 15.42 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. iw=1.00 GCpi(+/-)=0.18 PLT TYP. Bottom chord checked for 20.00 psf non-concurrent live load Wind reactions based on MWFRS pressures ITW Building Components Group, Inc. Haines City, FL 33844 Ft Cartificate of Amborization # 547 7-140B -- Isaac Construction Jeremy Cady chord 2x4 SP #2 Dense chord 2x6 SP #2 Webs 2x4 SP #3 #3 or better "I" brace. 80% length of web member. Attach Box or Gun (0.113"x2.5",min.)nails @ 6" OC. ALPINE Wave 1-3-12 **IMPORTANT***URBISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. FAILURE 10 BHILD THE RUSS IN COMPORMANCE WITH IP: OR FARBICATION, HANDLING, SHEPPIG, HISTALLING & BRACING OF FRUSSES.

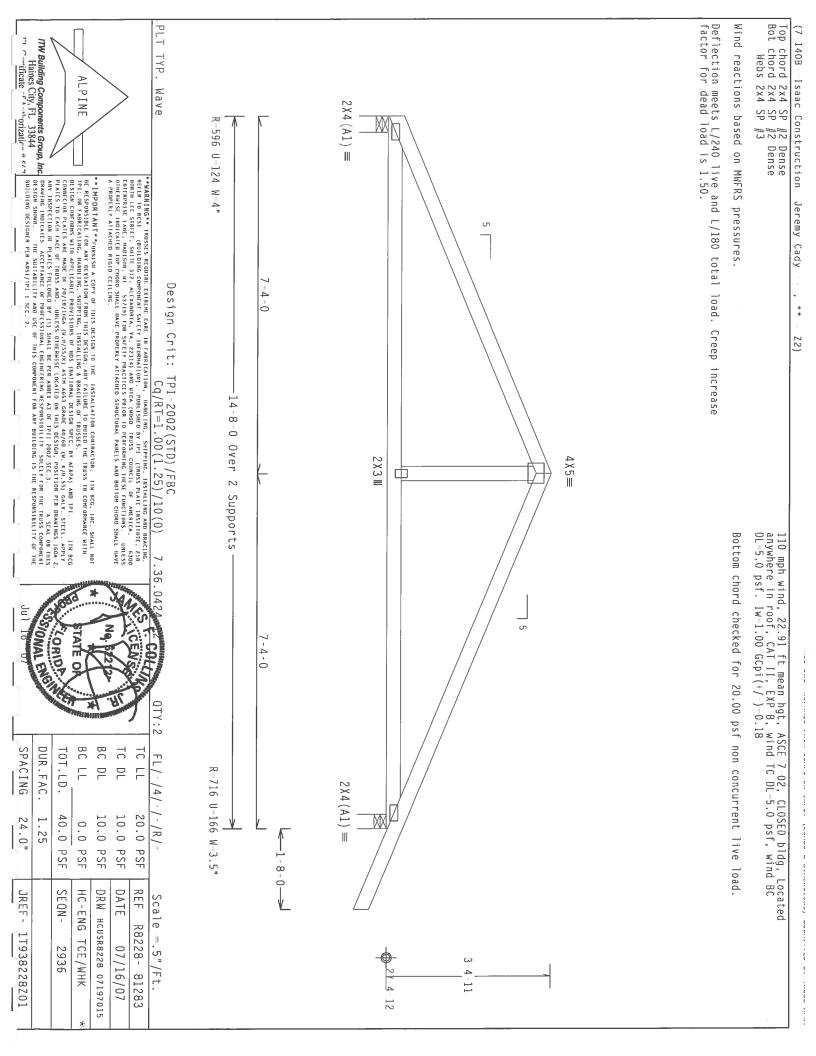
DESIGN CONTORNS HITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC, BY ATAPA) AND IPI. ITH BCG CONNECTOR PLATES ARE HADE OF 20/10/1662, 40.H/55/K) ASTH HASS GRADE 40/60 (M. X/H.55) GALV. SIETEA APPLY PLATES TO EACH FACT OF TRUSS AND. UNCESS OFFICENS HITS DESIGN, POSITION PER DRAWHIGS 160A Z. ANY LISPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A SOF IPI) 2002 SEC.3. A SEAL ON HIS DESIGN SHOWN.

DRAWHIG INDICATES ACCEPTANCE OF PROTESSIONAL ENGINEERING RESPONSIBILITY SOLETY FOR THE RUSS COMPOUNT DESIGN SHOWN.

THE SULTABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. **WARNING** IRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (FRUSS PLATE HISTITULE, 218 HORN LEE STREET, SUITE 312, ALEXANDRAL, VA. 22314) AND WIGE ALOOD TRUSS COUNCIL OF AMERICA. 6300 ENTERPRISE LANE, MADISON, WI 537(9) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE THOLEACH AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING. 1 8 0 4X10 12 4 X 5 ∇ = 2884 Design Crit: U-876 W-5.5" 6 X 4 == 5×6/ -10 - 10S1) TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/ 24-8-0 \mathcal{A} 5 X 7 == 5×6/ 0ver 4-10-11 2 $4 \times 4 \equiv$ Supports 4 X 7 (R) III 4 X 7 == /10(0)(A)Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. In lieu of structural panels use purlins to brace all flat TC $24\ensuremath{\text{m}}$ OC. SPECIAL From (LUMBER 1408 265 LOADS 5 X 6 // 68 PLF at -1.67 to 68 PLF at 9.89 to 68 PLF at 14.78 to 68 PLF at -1.67 to 68 PLF at -1.67 to 20 PLF at -0.00 to 20 PLF at 24.67 to 68 PLF at 24.6 $6 \times 4 \equiv$ 10-10 DUR.FAC.=1.25 CORNO R=2884 U=876 W=5.5" 112 4X10// 4 X 5 III 1-8-0 PLATE TE DUR.FAC.=1.25)
68 PLF at 9.89
68 PLF at 14.78
68 PLF at 26.33
6 PLF at 0.00
20 PLF at 24.67
6 PLF at 26.33
, 14.78
, 12.72 100 L SPACING BC DL TC DL DUR.FAC. TOT.LD. FL/-/4/-/-/R/-₩10-0-0 24.0" 1.25 40.0 PSF 10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF JREF SEQN-DATE REF HC-ENG DRW HCUSR8228 07197091 Scale = .1875"/Ft. R8228- 81280 17938228201 JB/WHK 1684 07/16/07

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 PLT TYP. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Wind reactions based on MWFRS pressures (7-140B--Isaac Construction Haines City, FL 33844
Haines City, FL 33844
Haines City, FL 3844 ALPINE Wave **★**1-8-0→ $2 \times 4^{\circ} (A1) =$ **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE NGG, THC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY TAILURE TO BUILD THE READS IN COMPORMANCE WITH PI: OR TARREATING, ANNOTHIG. SHIPPING, HISTALLING & BRACHING OF TRUSSES, BY ATRYA AND IPI. LINE CONTROLLED THE APPLICABLE PROVISIONS OF HIS CHAINGAL DESIGN SPEC, BY ATRYA AND IPI. THE NGC. CONTROLLED AND THE STREAM CHAINGACH, MINISTRYA SHAND HAD SHADE ADJOE OF A TRYA AND INDICTOR PLATES ARE MADE OF ZOJADJAGAC (MINISTRYA) AND THIS OFFICIAN POSITION PER DRAMHINGS LOCAL PLATES OFFICENCY OF THE STREAM OF THIS OFFICENCY STREAM OF THE TRUSS COMPONENT OF THE THE TRUSS COMPONENT OF THE TRUSS COMPONENT OF THE THIS OFFICENCY STREAM OF THE TRUSS COMPONENT OF THE THIS OFFICENCY STREAM OF THE TRUSS COMPONENT OF THE STREAM OF THE TRUSS COMPONENT OF THE STREAM OF THE TRUSS COMPONENT OF THE TRUSS COM R=654 U=71 W=5.5' **MARNING** NUSSES REQUIRE CYPREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, METER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLAIE INSTITUTE, 218 MORTH LEE STREE, SUITE 317, ALEXANDRIA, VA, 22314) AND HICA (MODO) TRUSS COUNCIL OF AMERICA, 6300 ETHERPRIST LAND, MAISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED OF CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE Jeremy Cady បា Design Crit: ω Ö X2) 13-4-0 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) Over 2 X 3 III $4 \times 4 =$ 2 Supports لـ 5 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 Bottom chord checked for 20.00 psf non-concurrent live load ά 0 7.36.0424 R=654 U=71 W=5.5" 2X4 (A1) CORIOF TATE M **1**-8-0→ QTY:1 נואמנו ממנוו מובט זווו מו לרמטמים פי מזונרוויסזמנים ל ממנודוורם מו נוצמים ונוצי SPACING BC LL BC DL TC DL TC LL DUR.FAC. TOT.LD. FL/-/4/-/-/R/-10-0-0 24.0" 1.25 40.0 10.0 PSF 20.0 PSF 10.0 PSF 0.0 PSF PSF SEQN-DATE JREF-REF HC-ENG DRW HCUSR8228 07197018 Scale = .375"/Ft. R8228- 81281 1T938228Z01 TCE/WHK 2888 07/16/07

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Bot chord 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 D
:Stack Chord SC2 2x4 SP #2 D See Note: All Plates Are Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ Bottom chord checked for 20.00 psf non-concurrent live load (7 140B Isaac Construction rr C--ificate DWGS All015EE0207 & GBLLETIN0207 for more requirements TYP. ALPINE Wave **★**1-8-0→ 98 PLF U=1 PLF W=13-4-0 2 X 4 (C 5 2X3 Except As Shown. **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BEGG, THE SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY TAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH PPI OR TANERCATION. ANADIDIGS. SHEPTHALLING A BRACTING OF TRUSSES. DESIGN SPEC, BY ATRAN AND TPI. CABLE PROVISIONS OF THIS CONTRIBUTED OF TRUSSES. DESIGN SPEC, BY ATRAN AND TPI. ITH BEGCONTICTION PLATES ARE AND OF ZOTABLE PROVISIONS OF THIS CONTRIBUTED OF THIS DESIGN SPEC, BY ATRAN AND TPI. ITH BEGCONTICTION PLATES AND THE STATE OF THE **WARNING** TRUSSES REQUIRE LYMEME DARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), POBLISHED BY PPI (TRUSS PLAIE INSTITUIT, ZIB MORTH LEE STREEL, SUITE 137. ALEXANDRIA, VA, ZZJIA) AND MICA (MODO TRUSS COUNCIL O' AMERICA, 6300 ENTERPRIST LANE, MAINSON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNICESS OFHEM MISS. INDICATED FOR COMBES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE $2 \times 6 (C5) =$ Dense: 4 8 0 -5 (NNL Jeremy Cady 2 X 3 III 2×6 || Design Crit: α Ö 13-4-0 Over Continuous Support 2X3 III 2 X 3 III ഗ X1-GE) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/ 2X3 III 4 X 4 == /- 4 - 0 2 X 3 2 X 3 /10(0)σ Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 Wind reactions based on MWFRS pressures. chord in notchable area using 3x6. ω Ö 2X3 III 2 X 6 /// * Z $2 \times 6 (C5) =$ 2X4(C5) =4-8-0 3-5 (NNL CORIOR **★**1-8-0**≯** BC DL SPACING TC DL C DUR.FAC. TOT.LD. FL/-/4/-40.0 24.0" 1.25 10.0 10.0 PSF 20.0 0.0 PSF PSF PSF PSF DATE JREF SEQN-REF HC-ENG DRW HCUSR8228 07197014 Scale = .375"/Ft. R8228- 81282 1T938228Z01 TCE/WHK 07/16/07 2898



Top Bot professional engineer regarding conventional framing. Conventional framing is not the responsibility of the Truss Designer, Plate Manufacturer, or Truss Fabricator. Persons erecting trusses are cautioned to seek advice from a local Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\mathrm{.}$ Bottom chord checked for 20.00 psf non-concurrent live load. (B) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC. (A) 1×4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC. Wind reactions based on MWFRS pressures 110 mph wind, 26.59 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/)=0.18 End verticals not exposed to wind pressure (7-140B--Isaac Construction Jeremy Cady Haines City, FL 33844

Fi Calificate Canada and Cara p chord 2x4 SP #2 (t chord 2x6 SP #2 Webs 2x4 SP #3 lieu of structural panels use purlins to brace TC @ 24" TYP. ALPINE Wave Dense :T2 2x6 SP #2: ***IMPORTANT***URRISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, THC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE 10 BHILD THE TRUSS IN COMPORMANCE WITH PI: OR FARBLOATHE, HANDLURG, SHIPPIRE, HISTALLING A BRACHEN OF TRUSSES.

DESIGN CONTRONS HITH APPLICANCE PROVISIONS OF DDS (MATIONAL DESIGN SEC. BY AFRAY) AND PI. THE BCG CONNECTOR PLATES ARE HADE OF ZO/TB/16GA (M.H/SS/K) ASIH A653 GRADE 40/60 (M. K/M.SS) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND. UNICESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWHELS AND THE SOUTH OF THE SOUTH OF THE SOUTH OF THE TRUSS COMPONENT OF THE TRUSS ACCOMPONENT OF THE TRUSS ACCOMPONENT OF THE TRUSS ACCOMPONENT OF THE TRUSS COMPONENT AND USE OF THIS DESIGN SHOWN.

BRANING INDICATES ACCOMPONENT AND USE OF THIS COMPONENT FOR ANY BRUILDING IS THE TRUSS COMPONENT OF THE **MARNING** IRUSSES REQUIRE EXTREME CARE IN FARRICATION. INAUDING, SHIPPING, INSIALLING AND BRACING.

BETER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TYPI (TRUSS PLATE INSTITUTE, 218

JOHN HILE STREET, SUITE JIZY ALEXANDRIA, NA, 22214) AND HIVE ATTACK (MODO TRUSS CONNICLIO OF AMERICA. 6300

ENTERPRISE LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING INESE FUNCTIONS. UNICESS

OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE

A PROPERLY ATTACHED REGID CELLING. BUILDING DESIGNER PER ANSI/IPI 1 SEC. Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) The Building Designer shall evaluate and approve load magnitudes and locations as shown under "SPECIAL LOADS" Iruss Engineer & Fabricator are not responsible for SPECIAL LOADS load magnitudes and locations. From 88 PLF at 11.33 From 20 PLF at 0.00 PLF at 14.79 From 10 PLF at 14.79 E65 LB Conc. Load at 143 LB Conc. Load at 155 LB Conc. Load at 155 LB Conc. Load at 3X10 W 7.36 From From 3X5 (LUMBER 26 -18453-5-9 88 PLF at 0.00 t 88 PLF at 3.46 t 88 PLF at 11.33 t 88 PLF at 0.00 t 20 PLF at 14.79 t DUR.FAC.=1. CONAL ENGINEE U=1662 W=5.5" Ξ 4 X 7 ≡ 5 X 9 🕼 TATEOF (B) . 25 2X3 III 4 X 7 == to to 3.52, 5.52, 5.52, to 10-7 PLATE \sim Supports E DUR.FAC.=
88 PLF at
88 PLF at
88 PLF at
20 PLF at
11 27
7.40, 9.
11.33
7.40, 9. BC LL BC DL TC DL SPACING DUR.FAC. TOT.LD FL/-/4/-/-/R/ (B) 5 X 8 // 4 X 7 = at 3.46 at 11.33 at 16.46 at 14.79 at 16.46 SEE ABOVE $\widehat{\Xi}$ R=2026 U=1724 W=3.5" . 27 . 27 40.0 20.0 -5-9 . 25) 1.25 10.0 PSF 10.0 PSF 0.0 26 4 X 4 III PSF PSF PSF 4 X 1 0 \| 8-0 DATE JREF-SEQN-HC-ENG DRW HCUSR8228 07197016 Scal e R8228 81284 1T938228Z01 =.1875" TCE/WHK 2911 07/16/07 10 ىب

Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / :Stack Chord SC1 :Stack Chord SC2 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Bottom chord checked for 20.00 psf non-concurrent live load ī See DWGS All030EE0207 & GBLLETIN0207 for more requirements. Note: All Plates Are 2X3 Except As Shown. (7-1408 - Isaac Construction Haines City, FL 33844
Ft Confificate of Authorization # 567 lieu of structural panels use purlins to brace TC @ 24" OC TYP. ALPINE Wave P #2 Dense P #2 Dense P #3 11 2x4 SP #2 Dense: 12 2x4 SP #2 Dense: **★**1-8-0→ R=97PLF U=11 PLF W=14-8 2X4 (C5) **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR ITH BCG, INC. SHALL NOT BE RESPONSIBL FOR ANY DEVIATION FROM THIS DESIGN, MY FAILURE TO BUILD THE TRUSS IN COPPORAMICE WITH IP., OR FABRICATING, HANDING, SHIPPING, HISTALLING A BRACING OF TRUSSES.

DESIGN CONTROMS HITH APPLICABLE PROVISIONS OF THIS CREATER SPECE, BY AREA AND TEST. ITH BCG CONNICTOR FLATES ARE HADE OF 20/18/16GA (M.H/SS/K) ASIM A653 GRADE 40/60 (M. K/M.SS) AGAV. STEEL. APPLY PLATES TO CACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION FER DRAWINGS 160A Z. ANY HISPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANNEX AND THE SOLITABLE SECE, BY A SCAL ON THIS DESIGN. BY A STAL ON THIS DESIGN AND THE SOLITABLE AND THE SOLITABLE SECENDATION OF PLATES ACCUPICATED BY (1) SHALL BE FER ANNEX AND THE SOLITABLE TO CACHE OF THE DESIGN AND THE SOLITABLE TO CACHE ON THIS DESIGN. BY AND THE SOLITABLE TO CACHE ON THE SOLITABLE TO CACHE ON THE SOLITABLE TO CACHE ON THE SOLITABLE TO SOLITABLE TO THE RESPONSIBILITY OF THE DESIGN SHOWN. $2 \times 6 (C5) =$ ****MARNING*** RUIS (S REQUIRE LIFERE CARE IN FABRICATION: IMMODILING, SHIPPING, HISTALLING AND BRACING.
REFER TO BLOSI (BUILDING COMPONENT SER Y INFORMATION), PUBLISHED BY THE (FRUNS P ATE INSTITUTE, 210
MARTH LEE STRITT. SUITE 137: ALEXANDRIA, WA, 22314) AND MICA, (MODO TRUISS COMMENT) OF AMERICA, 6300
CHIERBRYSE LANG, MODOR SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE
OTHERBRYSE LANG, ATD OF CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY AT ACHED RIGID CEILING 4-8-0 - 0 (NNI Jeremy Cady 3X3# 2X3 III 2X3 III Design Crit: 卣 - 4 - 0 Z3-GE) 14 J 2 X 3 Ⅲ -8-0 2 X 3 III TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 0ver Continuous Support 2X3 III 4 X 4 == -8-0 2X3 III 2X3 III Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6. 110 mph wind, 23.08 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 Wind reactions based on MWFRS pressures 7.36.0424 -4 2 X 3 III 2X3 III 0 3X3₩ CORIO IATE O 2X6(C5) =-8-0 (NNL) 2X4(C5) =4-8-0 0TY:1 **1**-8-0¥ BC DL TC DL DUR.FAC. TC LL TOT.LD. FL/-/4/-/ - /R / -10.0 10.0 20.0 40.0 PSF 1.25 0.0 PSF PSF PSF PSF DATE REF SEQN-HC-ENG DRW HCUSR8228 07197017 Scale =.375"/Ft. R8228- 81285 TCE/WHK 2942 07/16/07 3 - 0 - 15

BUILDING

DESIGNER PER ANSI/IPI I SEC.

SPACING

24.0"

JREF-

1T938228Z01

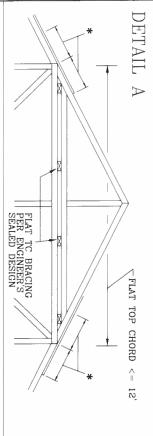
PIGGYBACK DETAIL

100 MPH WIND, 30.00 FT MEAN HCT, ASCE 7-02 OR ASCE 7-05, CLOSED BLGD, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

80 MPH WIND, 30.00 FT MEAN HGT, SBC, ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

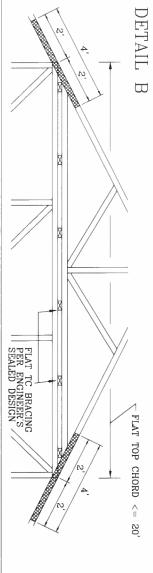
100 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS. MUST BE ADEQUATLY BRACED BY SHEATHING OR PURLINS. PROVIDE DIAGONAL BRACING OR OTHER SUITABLE

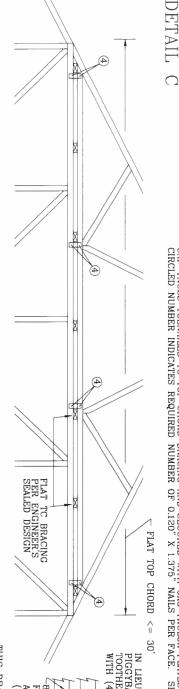


PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"x3") NAILS.

12" MIN RIGID SHEATHING OVERLAP WITH 8d COMMON (0.131"x2.5") OR GUN NAILS IN OVERLAP ZONE SPACED AT 4" O.C.



PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"X3") NAILS AND SECURED WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY) ATTACHED WITH 10d COMMON NAILS AT 4" O.C.



CAP TRUSS TOENAILED TO TOP CHORD CIRCLED NUMBER INDICATES REQUIRED

IN LIEU OF TRULOX CONNECTORS, ALPINE 62PB SPECIAL PIGGYBACK CONNECTORS MAY BE USED. SHOP APPLY TOOTHED PORTION, FIELD ATTACH TO MATING TRUSS WITH (4) 0.120" X 0.375" NAILS MINIMUM EACH FACE.

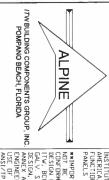
BRACING AND SECURED WITH 3X8 TRULOX PLATES (EACH FACE) AT EACH END AND AT 1/3 POINTS. NUMBER OF 0.120" X 1.375" NAILS PER FACE. SEE DRAWING 160TL FOR TRULOX INFORMATION.

(4) 8d COMMON NAILS (0.131"X2.5")

ζθ" χ θ" χ'1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES, ATTACH WITH (θ) Bd COMMON NAILS PER GUSSET, (4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

THIS DRAWING REPLACES DRAWINGS 581,670 & 961,860

Salau	DING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER STATE OF	FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND	ROVISIONS OF NDS (MATIGMAL DESIGN SPEC, BY AFEPA) AND TPI. E OF ZOVIBYIGHS (VLMYSSYN) ASTIM ASSIS GRADE AVAGO (VLMYSSY) H FACE OF TRUSS AND UNLESS OTHERWISE LUGATED ON THE AVERAGE H FACE OF TRUSS AND UNLESS OTHERWISE TOWN IN THE AVERAGE AVAILABLE OF TRUSS AND THE AVAILABLE OF TRUSS AND	IS DESIGN ID INSTALLATION CONTRACTOR. THE BULG IN SHALL STORM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN ING. SHIPPING, INSTALLING & BRACING OF TRUSSES.		E 312. ALEXANDRÍA, VA. 22319 AND WTCA KUDDO TRUSS COUNCIL D NW. WI 33719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE ATED TOP FHORD SHAIL HAVE PROPERTY ATTACHED STRUCTURA	EHE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND DEPARTMENT SAFETY INFORMATION), PUBLISHED BY TPI CTRUSS PLATE
60	R	WHIII *	21	4			1
	SPACING	DUR. FAC.	TOT. LD	BC LL	BC DL	TC DL	TC LL
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The references	24.0"	C. 1.15	. MAX 60 PS	P;	Pi	Po	
	24.0"	C. 1.15	TOT. LD. MAX 60 PSF	PSF -E	PSF DR	PSF DA	
	24.0"	C. 1.15	. MAX 60 PSF	PSF -ENG DLJ/KAR	PSF DRWG PIGBACKA0207	PSF DATE 2/23/07	L PSF REF PIGGYBACK



#WARKHING## TRUSSES REDUIRE EXTRE BRACING. REFER TO BCS! (BUILDING CDI INSTITUTE, 218 NURTH LEE STR., SUITE AMERICA, 6300 ENTERPRISE LIN, HADISUN, FUNCTIONS. UNLESS OTHERWISE NODGE PANELS AND BOTTOM CHORD SHALL HAVE

| WINDERTANITM FURNISH CORY OF THIS OR CONTORNATION TO ATTOM CONTORNATION THAT PROPERTY OF THE PROPERTY OF TH

TOP CHORD CHORD WEBS 2X4 2X4 2X4 ### %% 222 R BETTER
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PIGGYBACK

(4) 6d BOX (0.099"X 2.", MIN) NAILS.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX. REFER TO SEALED DESIGN FOR DASHED PLATES

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS. REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

130 MPH WIND, 30' MEAN HGT, ASCE 7-98, ASCE 7-02 OR ASCE 7-05, CLOSED BLGD, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

F

MAX SIZE OF 2X12 #2 OR BETTER

20' FLAT TOP CHORD MAX SPAN

EITHER PLATE LOCATION IS ACCEPTABLE

OPTIONAL SPLICE

В A

C8" X 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES, ATTACH WITH (8) 6d BOX (0.099"X 2.",MIN) NAILS PER GUSSET. (4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

JOINT A H D C В 4X6 1.5X3 5X4 2X4 4X6 30 OR 3X6 TRULOX AT 4' 2.5X4 SPANS 5X5 .5X4 5X6 34 ЧU 2.5X4 .5X4 5X5 5X6 38 To 1.5X4 3X5 5X6 5X6 52 000

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

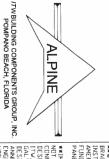
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	° °	0	0	ATTACH TEETH TO T FABRICATION. ATTAS (4) 0.120" X 1.375" APPLY PIGGYBACK S AND SPACE 4' OC O		10' TO 14'	7'9" TO 10'	0' TO 7'9"	WEB LENGTH	
0) °	0	0	OCK 375	* PIGGYB	2x4 "T" BRACE. SAME GRADE, SP MEMBER, OR BETTER, AND 80% LE MEMBER. ATTACH WITH 16d BOX (0.135"X 3.5",MIN) NAILS AT 4" OC		NO BRACING		WEB BI
0	0	0	0	THE PIGGYBACK AT THE TIME OF STANDARY TO SUPPORTING TRUSS WITH STANDARY FACE PER PLY. SPECIAL PLATE TO EACH TRUSS FACE OR LESS.	* PIGGYBACK SPECIAL PLATE	RACE. SAME GRADE, S OR BETTER, AND 80% L ATTACH WITH 16d BOX 3.5",MIN) NAILS AT 4" O	BRACE. SAME GRADE, OR BETTER, AND 80% ATTACH WITH 8d BO 2.5",MIN) NAILS AT 4"		REQUIR	BRACING CHART
0	0	0	0	GGYBACK AT THE TIN SUPPORTING TRUSS PER FACE PER PLY. PLATE TO EACH TR	AL PLATE	SAME GRADE, STER, AND 80% H WITH 16d BOS NAILS AT 4"	_×		REQUIRED BRACING	ART
0	0	0	0	TIME OF SS WITH LY. TRUSS F#		NGTH	SPECIES AS LENGTH OF X OC.		ត	
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		NTS GRO	<i>)</i> i	m_				*
		NTS GROUP, INC.						ATTACH
4	USE OF THE SUBPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.	GALV STEEL, APPLY PLATES 10 EACH FALE DE ENDS AND UNLESS DEFENDANCE DE PROFESSIONAL BE PER DESIGNA, POSITION PER BRANNES GOAZZ. ANY INSPECTION OF PLATES FOLLUTED BY O'S SHALL BE PER DE NAVEX AS DE TOT 1-2002 SEC. A SEAL DE HIS DRAVING INDICATES ACCEPTANCE DE PROFESSIONAL DE NAVEX AS DE TOT 1-2002 SEC. A SEAL DE HIS DRAVING INDICATES ACCEPTANCE DE PROFESSIONAL DE NAVEX AS DE TOT 1-2002 SEC. A SEAL DE HIS DRAVING INDICATES ACCEPTANCE DE PROFESSIONAL DE NAVEX AS DE TOT 1-2002 SEC. A SEAL DE HIS DRAVING INDICATES ACCEPTANCE DE PROFESSIONAL DE NAVEX DE L'ACCEPTANCE DE PROFESSIONAL DE NAVEX DE L'ACCEPTANCE DE PROFESSIONAL DE NAVEX DE L'ACCEPTANCE DE PROFESSIONAL DE L'ACCEPTANCE DE L'	LICE CONNECTOR PLATES ARE MADE OF 20/18/16/16/16/16/16/16/16/16/16/16/16/16/16/	***IMPDRTANI** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUST IN THE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUST IN THE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUST IN THE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUST IN THE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUST IN THE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUST IN THE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUST IN THE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUST IN THE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUST IN THE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUST IN THE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUST IN THE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUST IN THE RESPONSIBLE FOR ANY DEVIATION FROM THE TRUST IN THE TRUST	FUNCTIONS: UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERTY ATTACHED RIGID CEILING.	INSTITUTE, 218 NORTH LEE STR. SUITE 312, ALEXANDRIA, VA. 22314) AND WTCA (WOOD TRUSS COUNCIL ALEXANDRIA, VA. 22314) AND WTCA (WOOD TRUSS COUNCIL ALEXANDRIA, 6300 ENTERPRISE LM, HADISON, WI 53759 FOR SAFETY PRACTICES PRIOR TO PERFORMING MASS.	**VARRING** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE	*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.
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	S	Hilling			T			AWING
	PACING 24.0"	1.15 DUR. FAC.	1.25 DUK. FAC.	50 PSF AT	1.33 DUR. FAC.	55 PSF AT	MAX LOADING	REPLACES DRAWINGS 634,016 634,017 & 847,0
				-ENG	DRWG	DATE	REF	34,016
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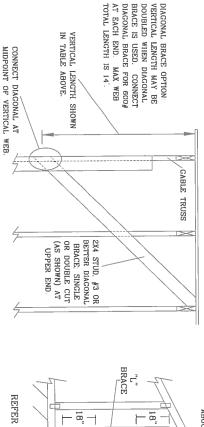
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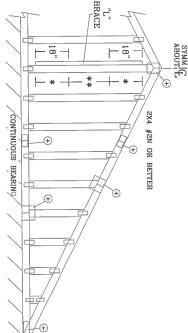
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D-SPLICE

ASCE 7-02: 110 MPH WIND SPEED, 15, MEAN HEIGHT, ENCLOSED, I | 1.00, EXPOSURE \Box

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	STANDARD	STUD	#3	2#	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	ü	#1 / #2	GRADE	BRACE
	4' 11"	5' 0"	5' 0"	5 3"	5' 4"	4' 9"	4' 9"	4' 9"	4' 11"	4' 5"	4' 6"	4' 6"	4' 9"	4' 10"	4, 4.	4' 4"	4' 4"	4' 5"	3' 10"	4' 0"	4. 0"	4' 2"	4' 3"	3' 9"	3' 9"	3' 9"	3' 10"	BRACES	ON
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	7' 5"	8' 7"	8' 5"	9' 1"	9' 1"	7' 3"	8' 5"	8, 5,	8' 8"	6' 5"	7' 6"	7' 7"	8' 3"	8' 3"	6' 4"	7' 4"	7' 4"	7' 10"	5, 3,,	6' 1"	6' 2"	7' 2"	7' 2"	ري دي	6' 0"	6' 0"	6' 10"	GROUP B	BRACE *
	9' 10"	10' 0"	10' 0"	10' 0"	10, 0,	9' 7"	10' 0"	10' 0"	10' 0"	8' 6"	9' 1"	9' 1"	9' 1"	9' 1"	8' 4"	9' 1"	9' 1"	9' 1"	6' 11"	7' 11"	7' 11"	7' 11"	7' 11"	6' 9"	7' 11"	7' 11"	7' 11"	GROUP A	(1) CX4
2	9' 10"	10' 6"	10' 6"	10' 9"	10' 9"	9' 7"	10' 0"	10' 0"	10' 3"	8' 6"	9' 6"	9' 6"	9' 9"	9' 9"	8' 4"	9' 1"	9' 1"	9' 4"	6' 11"	8'0"	8' 1"	8' 6"	8' 6"	6' 9"	7' 11"	7' 11"	8' 1"	GROUP B	L BRACE
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	12' 3"	12' 6"	12' 6"	12' 10"	12' 10"		11' 11"	11' 11"	12' 3"	11' 1"	11' 4"	11' 4"	11' 8"	11' 8"	10' 10"	10' 10"	10' 10"	11' 1"	9' 4"	9' 11"	9' 11"	10' 2"	10' 2"	9' 1"	9	9, 5,	9' 8"	GROUP B	DRACE
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OUTLOOKERS WITH 2' O" OVERHANG, OR 12"	GABLE END SUPPORTS LOAD FROM 4' 0"	CONTINUOUS BEARING (5 PSF TC DEAD LOAD).	PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER	LIVE LOAD DEFLECTION CRITERIA IS L/240.		GABLE TRUSS DETAIL NOTES:					SOUTHERN PINE DOUGLAS FIR-LARCH			#1 & BTR	Upu Pilo	GROUP B:			an Si	STUD	K-LARCH SOUTH		STUD	0 #2	SPRUCE-PINE-FIR HEM-FIR	GROUP A:	BRACING GROUP SPECIES AND GRADES		





PLYWOOD OVERHANG. BLE END SUPPORTS LOAD FROM 4' 0"
OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" WIDE UPLIFT CONNECTIONS FOR 80 PLF OVER ONTINUOUS BEARING (5 PSF TC DEAD LOAD).

ATTACH EACH "L" BRACE WITH 10d NAILS.

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.

* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

MEMBER LENGTH. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.	GREATER THAN 11' 6"	GREATER THAN 4' 0", BUT LESS THAN 11' 6"	LESS THAN 4' O"	VERTICAL LENGTH	GABLE VERTICAL PLATE SIZES
SS DESIGN FOR L PLATES.	2.5X4	JT 2X4	1X4 OR 2X3	NO SPLICE	ATE SIZES

/TWBUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA ALPINE

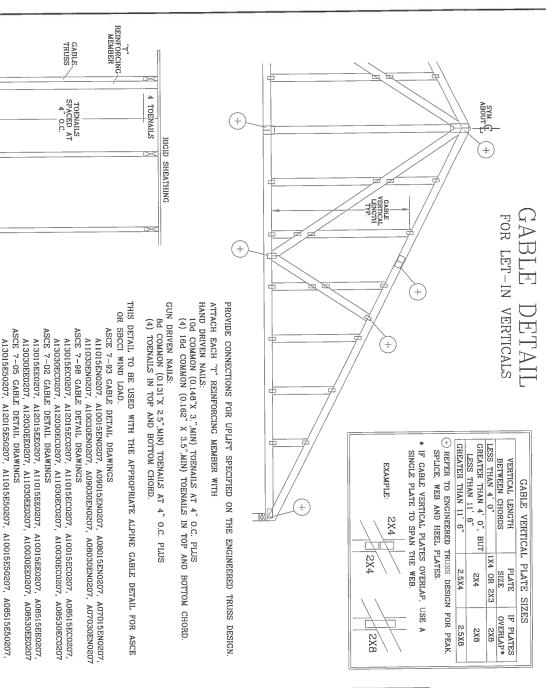
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HAND, ING. SHIPPING, INSTALLING BRACING. REFER TO BEST GBULDING COMPORENT SAFETY INFORMATION, PUBLISHED BY FIS (TRUSS F IBRACING, REFER TO BEST GBULDING COMPORENT SAFETY INFORMATION, PUBLISHED BY FIS (SCION INSTITUTE, 218 MORTH LEE SET, SUITE 312, ALEXANDRIA, VA. 2234) AND VICA (VOODD TRUSS COUN AMERICA, 6300 ENTERPRISE IN, HADISON, WI 53719) FOR SAFETY PRACTICES PROPERTY ATTACHED STRUCTUP CHORTONS. UNICESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERTY ATTACHED RIGID CEILING.

TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH

WMIMPDRIANTAW FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC. SHADT BE RESPONSIBLE FOR ANY DEVLATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS. AN CONTREHANCE WITH THE OF FABRICATING, HANDLING, SHIPPING, INSTALLING IS BRACING OF TRUSSED BESIGN CONFIDENCY WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY ATRADA AND THY BOG CONNECTIOR PLATES OF MEDE OF 20/18/19/16A V4H/25X/X) ASTM ASSI GARDE 40/60 V4X/H ITV, BCG CONNECTIOR PLATES OF EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS GALV STEEL APPLY PLATES TO BACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS GESTON, POSITION FER DRAVINGS 160A-Z. ANY INSPECTION OF PLATES FOLLUPED BY CIV SHALL BE ESIGN, POSITION FER DRAVINGS 160A-Z. ANY INSPECTION OF PLATES FOLLUPED BY CIV SHALL BE CHIGHNERING RESPONSIBILITY SUCLEY FOR THE TRUSS COMPONENT DESIGN SHOW. THE SUITABILITY USE OF THIS COMPONENT DESIGN SHOW. THE SUITABILITY USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PARTICIPAL IN THE SUITABILITY OF THE BUILDING DESIGNER, PARTICIPAL IN THE BUILDING DESIGNER.

* SOUNT ENGLISH 16 NO. 522 STATE OF × MAX. MAX. TOT. SPACING LD. 60 24.0"

PSF DRWG DATE REF -ENG A11015EE0207 2/23/07 ASCE7-02-GAB11015



WEB LENGTH INCREASE W/ "T" BRACE

TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE

TOENAIL

TOENAIL

2X4 "T" REINFORCING MEMBER

APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD. VERTICAL SPECIES, GRADE AND SPACING) FOR (1)

2X4 "L" BRACE, GROUP A, OBTAINED FROM THE

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

30	70	15	70	30	80	15	80	30	90	15	90	30	100	15	100	30	110	15	110	AND
30 FT	70 MPH	FT	70 MPH	F	MPH	F	HAM	30 FT	MPH	15 FT	90 MPH	FT	MPH	15 FT	MPH	FT	MPH	15 FT	MPH	AND MRH
2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	MBR. SIZE
10 %	10 %	0 %	0 %	20 %	20 %	10 %	2 01	30 %	10 %	20 %	20 %	40 %	10 %	30 %	10 %	50 %	10 %	40 %	10 %	ישטרכו
30 %	20 %		20 %	40 %	10 %	30 %	20 %	50 %	2 01	40 %	10 %	40 %	2 01	50 %	2 01	50 %	10 %	50 %	10 %	AUCE

NG REPLACES DRAWINGS GAB98117 876,719 & HC26294035 LET-IN VERT

mm * CENS STATE OF No. 52212 MAX TOT. MAX SPACING DUR. FAC. LD. ANY 60 24.0 PSF DRWG DATE ENG GBLLETIN0207 DLJ/KAR 2/23/07

OS VINOS

/TW BUILDING COMPONENTS GROUP, INC POMPANO BEACH, FLORIDA ALPINE

WAWARNINGAW TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, JNSTALLING AND BRACING, REFER TO BOST (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY THE CRUSS PLANSITURE, 218 MARTH LEE STER, SUITE 212, ALEXANDRIA, VA. 22314) AND VITA CYODD TRUSS COUNCE, ARERICA, 6300 ENTERRISE LN, HADISON, VI 53719) FIDS SAFETY PRACTICES PRIOR TO PERFORMING THE FUNCTIONS. UNLESS DIMENSEE INDICATED. THE CRUBB SHALL HAVE PROPERTY ATTACHED STRUCTURAL LINESS DIMENSEE INDICATED. THE CRUBB SHALL HAVE PROPERTY ATTACHED RIGHD CEILING.

TOENAILS

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI

A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08530E50207

(1) 2X4 "L" BRACE LENGTH = 6' 7" MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH 1.10 x 6' 7" = 7' 3" "T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"

GABLE VERTICAL = 24" O.C. SP ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT

#3

WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE

VERTICAL LENGTH.

CEILING

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED

NOTES

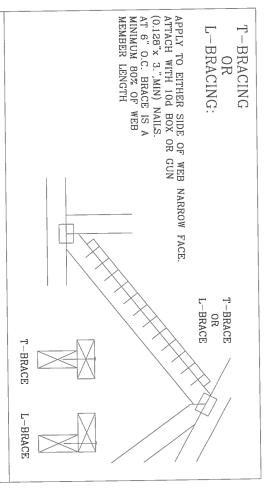
THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR BRACING SCAB

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING. WITH APPROPRIATE

*)9X2-2	2X6	2 ROWS	2X8
1-2X8	2X6	1 ROW	2X8
1-2X6	2X4	1 ROW	2X6
2-2X4(*)	2X6	2 ROWS	2X6
1-2X4	2X4	1 ROW	2X3 OR 2X4
2-2X4	2X6	2 ROWS	2X3 OR 2X4
E BRACING	ALTERNATIVE BRACING T OR L-BRACE SCAB BR	SPECIFIED CLB	WEB MEMBER
SCAB BRACE		BRACING	SIZE

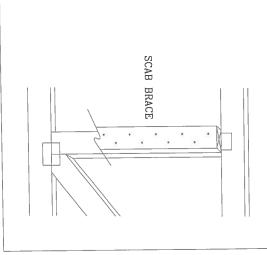
T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

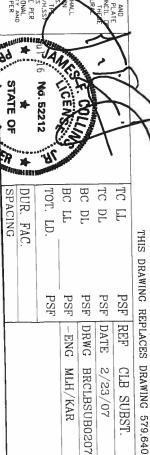
* FACE OF WEB. CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH



SCAB BRACING:

(0.128"x 3.",MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH ATTACH WITH 10d BOX OR GUN APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE.





PSF PSF PSF PSF PSF

> -ENG DRWG

MLH/KAR BRCLBSUB0207

REF DATE

2/23/07 CLB SUBST.



WWWARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACHING, REFER TO BEST (BUILDING COMPOBENT SAFETY INTERNATION), POBLISHED BY TRY CIRRUSS PLAY INSTALLING, ADD TRUSS COUNCIL (MSTITUTE, 218) MORPH LEE STR, SUITE 122, ALEXANDRIA, VA. 2234) AND "TOTA AVOID TRUSS COUNCIL AMERICA, 6300 ENTERPRISE IN, HADISON, VI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THE FUNCTIONS. INVESSO DHERWISE INDICATED, THE CHARD SHALL HAVE PROPERLY ATTACHED STRUCTURE FUNCTIONS. INVESSO OTHERWISE INDICATED, THE CHARD SHALL HAVE PROPERLY ATTACHED STRUCTURE PARIELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ASCE 7-02: 110 MPH WIND SPEED, 30° MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

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		1 %	S	,,	(С	. (7.			1 (6	,,	(С	.(7.			2	4	,,		0	. (7.		SPACING	2
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	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	ωI	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE
	4' 7"	4' 9"	4' 9"	4' 11"	5' 1"	4' 6"	4' 6"	4' 6"	4' 7"	4, 2,	4' 4"	4' 4"	4' 6"	4' 7"	4' 1"	4' 1"	4' 1"	4, 2,	3' 8"	3' 9"	3, 9,	3' 11"	4' 0"	3' 7"	3' 7"	3' 7"	ယ္	BRACES	N O
	6' 9"	7' 9"	7' 11"	8' 0"	8' 0"	6' 7"	7' 8"	7' B"	8'0"	5' 10"	6' 9"	6' 10"	7' 3"	7' 3"	5' 8"	8' 0"	6' 8"	7' 3"	4' 9"	5' 6"	5' 7"	6' 4"	6' 4"	4, 8,	5' 5"	5 5 "	6' 4"	GROUP A	(1) 1X4 "L"
	6' 9"	7' 9"	7' 11"	8' 7"	8' 7"	6' 7"	7' 8"	7' 8"	8, 5,	5' 10"	6' 9"	6' 10"	7' 9"	7' 9"	5' 8"	8' 0"	6' 8"	7' 5"	4' 9"	5 6	5' 7"	- 1	6' 10"	4' 8"	5' 5"	5, 2,	6' 6"	GROUP B	L" BRACE *
	8' 10"	9' 5"	9' 5"	9' 5"	9' 5"	8' 8"	9, 5,	9' 5"	9' 5"	7' 8"	8' 7"	8' 7"	8' 7"	8' 7"	7' 6"	8' 7"	8' 7"	8' 7"	6' 3"	7' 3"	7' 4"	7' 6"	7' 6"	6' 1"	7' 1"	7' 2"	7' 6"	GROUP A	(1) 2X4 "L"
IYS	8' 10"	9' 11"	9' 11"	10' 2"	10' 2"	8' 8"	9' 5"	9' 5"	9' 8"	7' 8"	8' 11"	9' 0"	9' 3"	9' 3"	7' 6"	8' 7"	8' 7"	8' 10"	6' 3"	7' 3"	7' 4"	8' 1"	8' 1"	6' 1"	7' 1"	7' 2"	7' 8"	GROUP B	L" BRACE *
SYMM IC	11' 3"	11' 3"	11' 3"	11' 3"	11' 3"	11' 3"	11' 3"	11' 3"	11' 3"	10' 3"	10' 3"	10' 3"	10' 3"	10' 3"	10' 1"	10' 3"	10' 3"	10' 3"	8' 5"	8' 11"	8' 11"	8' 11"	8' 11"	8' 3"	8' 11"	8' 11"	8' 11"	GROUP A	(2) 2X4 "L"
	11' 7"	11' 10"	11' 10"	12' 1"	12' 1"	11' 3"	11' 3"	11' 3"	11' 7"	10' 4"	10' 9"	10' 9"	11' 0"	11' 0"	10' 1"	10′ 3″	10' 3"	10' 6"	8, 2,	9' 5"	9' 5"	9' 7"	9' 7"	8 3"	8' 11"	8' 11"	9' 2"	GROUP B	BRACE **
	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	13' 6"	14' 0"	14' 0"	14' 0"	11' 11"	13' 5"	13′ 5″	13′ 5″	13′ 5″	11' 8"	13' 5"	13' 5"	13' 5"	9' 9"	11' 4"	11' 5"	11' 9"	11' 9"	9' 6"	11, 1,"	11' 2"	11' 9"	GROUP A	(1) 2X6 "L"
	13' 10"	1	14' 0"	14' 0"	1 -	13' 6"	14' 0"		14' 0"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"		13' 5"		13 10"		1 "	11' 5"	12' 8"	12' 8"	1		11' 2"	12' 1"	GROUP B	BRACE *
	14' 0"	14' 0"	14' 0"	14' 0"		14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		14' 0"	13' 3"		14' 0"	14' 0"	14' 0"	12' 11"		14' 0"		GROUP A	(2) 2X6 "L"
	14' 0"	14' 0"	14' 0"	14' 0"	-	14' 0"	14' 0"		14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14 0"	14' 0"	14' 0"	14, 0,	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	GROUP B	"L" BRACE **

DOUGLAS FIR-LARCH

SOUTHERN PINE

STANDARD

STANDARD #3

GROUP B:

HEM-FIR

STUD

#1 / #2 STANDARD
#3 STUD

13

STANDARD

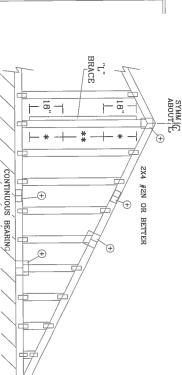
BRACING GROUP SPECIES

AND

GRADES:

GROUP

A



DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL

GABLE TRUSS

CONNECT

AT EACH END. MAX WEB TOTAL LENGTH IS 14'. DIAGONAL BRACE FOR 700# BRACE IS USED.

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

BETTER DIAGONAL BRACE; SINGLE OR DOUBLE CUT (AS SHOWN) AT 2X4 STUD, #3 OR

UPPER END.

REFER

TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH

CONNECT DIAGONAL AT MIDPOINT OF VERTICAL WEB.

GABLE TRUSS DETAIL NOTES:

SOUTHERN PINE

DOUGLAS FIR-LARCH

2

15

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" LIVE LOAD DEFLECTION CRITERIA IS L/240. PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

ATTACH EACH "L" BRACE WITH IOD NAILS.

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.
IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

MEMBER LE "L" BRACING IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

REFER TO COMMON TRUSS DESIGN FOR	GREATER THAN 11' 6"	GREATER THAN 4' 0", BUT LESS THAN 11' 6"	LESS THAN 4' 0"	VERTICAL LENGTH	GABLE VERTICAL PLATE SIZES	
DESIGN FOR	2.5X4	2X4	1X4 OR 2X3	NO SPLICE	E SIZES	

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDING, SHPPING, INSTALLING BRACING, REFER TO BESS (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY PIG (TRUSS INSTITUTE, 218 MORTH LEE STE, SUITE 212, ALEXANDRIA, VA. 22314) AND VICA (VOIDD TRUSS COMMERCIA, 6300 EXTERPRISE LN, HADISON, VI 53719) FOR SAFETY PRACTICES PRIDE TO PERCONNOC FUNCTIONS. UNLESS OTHER VISE INDICATOR. THE ALVE PROPERTY ATTACHED STRAIL PAVE PROPERTY ATTACHED STRAIL PAVELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED RIGHT CELLING. SONAL BUCKER 16 N@./52212 STATE OF * MAX. MAX.

SPACING

TOT. LD. 60 PSF

DRWG DATE -ENG 2/23/07

WAIMPORTANTAM FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC. SHALL

NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FALLURE TO BUILD THE TRUSS IN

CONTORNACE WITH THIS OF FABRICATION, HANDLING, SHIPPING, INSTALLING & BEAGING OF TRUSSES

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (MATIDNAL DESIGN SPEC, BY AFREA) AND PRI
THY, BGG CONNECTIOR PLATES ARE MADE OF PAVISIONS OF NDS (MATIDNAL DESIGN SPEC, BY AFREA) AND

EDGLIC STEEL APPLY PLATES 10 EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED BY HIS

DESIGN, POSITION PER DRAVINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOUED BY (1) SHALL BE PER

LYGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND

LYGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND

LYGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND

LYGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN.

ITWBUILDING COMPONENTS GROUP, INC POMPANO BEACH, FLORIDA

ALPINE

PLYWOOD OVERHANG.

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ZES	80%
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	IC MUST BE A MINIMUM OF 80% OF WEB ENGTH.

PEAK, SPLICE, AND HEEL PLATES.	REFER TO COMMON TRUSS DESIGN FOR	GREATER THAN 11' 6"	LESS THAN 11' 6"
PLATES.	DESIGN	2.5X4	244
	FOR		

REF A11030EE0207 ASCE7-02-GAB11030

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110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I |} 1.00, EXPOSURE \bigcirc

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			j	<u>V.</u>)	TIT	I I I	CT				j j	υ. Τ)	TIL	口 口 丁	ULL	2	,		1	<i>y</i> .) j	TIT.		ひてゴ	2	SPECIES GRADE	2X4	
	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE	
	4' 11"	5' 0"	5 0"	5 3	5' 4"	4' 9"	4' 9"	4' 9"	4' 11"	4' 5"	4, 6,"	4' 6"	4' 9"	4' 10"	4' 4"	4' 4"	4' 4"	4' 5"	3' 10"	4' 0"	4' 0"	4' 2"	4' 3"	3' 9"	3' 9"	3' 9"	3' 10"	BRACES	N O	
	7' 5"	8' 5"	8,5	& 5	8, 5,	7' 3"	8, 5,	B, 5,	8, 5,	6' 5"	7' 6"	7' 7"	7' 8"	7' 8"	6' 4"	7' 4"	7' 4"	7' 8"	5' 3"	6' 1"	ත <u>.</u>	6' 8"	6' 8"	5' 2"	6' 0"	6' 0"	6, 8,	GROUP A	(1) 1X4 "L"	
	7' 5"	8' 7"	8' 5"	9' 1"	9' 1"	7' 3"	8, 5,	8, 2,	8, 8,	6, 5,	7' 6"	7' 7"	8' 3"	8' 3"	6' 4"	7' 4"	7' 4"	7' 10"	5' 3"	6' 1"	6' 2"	7' 2"	7' 2"	ري 2	6' 0"	6'0"	6' 10"	GROUP B	" BRACE .	
	9' 10"	10' 0"	10' 0"	10' 0"	10' 0"	9' 7"	10' 0"	10' 0"	10' 0"	8' 6"	9' 1"	9' 1"	9, 1,,	9' 1"	8' 4"	9' 1"	9' 1"	9' 1"	6' 11"	7' 11"	7' 11"	7' 11"	7' 11"	6' 9"	7' 11"	7' 11"	7' 11"	GROUP A	(1) 2X4 "L"	
NAS	9' 10"	10' 6"	10' 6"	10' 9"	10' 9"	9' 7"	10' 0"	10' 0"	10' 3"	8' 6"	9' 6"	9' 6"	9' 9"	9' 9"	8' 4"	9' 1"	9' 1"	9' 4"	6' 11"	8' 0"	8' 1"	8' 6"	8' 6"	6' 9"	7' 11"	7' 11"	8' 1"	GROUP B	BRACE *	
CI MAAS	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	9' 4"	9' 5"	9' 5"	9' 5"	9' 5"	9' 1"	9 5	9' 5"	9, 5,	GROUP A	(2) 2X4 "L"	
	12' 3"	12' 6"	12' 6"	12' 10"	12' 10"	11' 11"	11' 11"	11' 11"	12' 3"	11' 1"	11' 4"	11' 4"	11' 8"	11' 8"	10' 10"	10' 10"	10' 10"	11' 1"	9' 4"	9' 11"	9' 11"	10' 2"	10' 2"	9' 1"	9, 5,	9' 5"	9 _,	GROUP B	" BRACE **	
	14' 0"	14' 0"	14'0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	10' 10"		12' 5"		12' 5"	10' 7"			12 5"	GROUP A	(1) 2X6 "L'	
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13′ 3″	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"			12' 8"	13' 5"	13' 5"	10' 7"	12' 3"	12' 4"	12' 9"	GROUP B	" BRACE *	
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		14' 0"	14' 0"			14' 0"		14' 0"	GROUP A	(2) 2X6 "L"	
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		14' 0"	14' 0"		14' 0"	14' 0"	14' 0"	14' 0"	GROUP B	BRACE *	

DOUGLAS FIR-LARCH

SOUTHERN PINE #3

STANDARD STUD

STANDARD

STUD

SPRUCE-PINE-FIR

#3

STUD

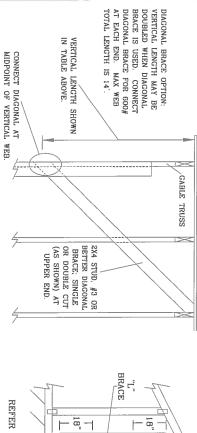
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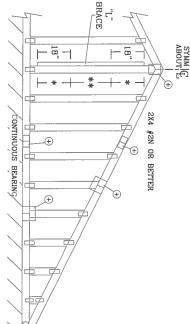
STANDARD

HEM-FIR

BRACING GROUP SPECIES AND GRADES:

GROUP A:





GABLE TRUSS DETAIL NOTES

SOUTHERN PINE

DOUGLAS FIR-LARCH

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HEM-FIR #1 & BTR #1 GROUP B:

12

GABLE END SUPPORTS LOAD FROM 4' 0"
OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER LIVE LOAD DEFLECTION CRITERIA IS L/240. CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

PLYWOOD OVERHANG.

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.
IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.
IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. ATTACH EACH "L" BRACE WITH 10d NAILS: MEMBER LENGTH. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB

TE SIZES NO SPLICE 1X4 OR 2X3 2X4 2.5X4 DESIGN FOR PLATES.	REFER TO COMMON TRUSS DESIGN FOR EAK, SPLICE, AND HEEL PLATES.	REATER THAN 11' 6"	REATER THAN 4' 0", BUT	SS THAN 4' 0"	VERTICAL LENGTH	GABLE VERTICAL PLATE SIZES
	S DESIGN FOR PLATES.	2.5X4	2X4	1X4 OR 2X3	NO SPLICE	TE SIZES

없음

구 # C

TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

MOT BE RESPONSIBLE FOR ANY BELLINK SERSION TO INSTALLATION CONTRACTIRE TITY BCG, INC., SYLL
MOT BE RESPONSIBLE FOR ANY BELLING FROM THIS DESIGN, ANY FAILURE IN BUILD IN FRIENDS.

CONFIDENCIAC THE PRINCE AND THE SERSION ANY FAILURE IN BUILD IN FRUSTSES.

DESIGN CONFIDENCY WITH APPLICABLE PROPUSIONS OF AUSTON ASSEMBLY AND ASSEMBLY AND ASSEMBLY AND ASSEMBLY ASSEMB **WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESS GUILDING COMPONENT SAFETY INGRANITION, PUBLISHED BY TEI CIRUSS PLATE INSTITUTE. 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND WTCA CUODO TRUSS COUNCIL MARKICA, 6300 ENTERRISE LN, MADISON, VI 53719) FOR SAFETY PRACTICES PRICK TO PERFORMING THESE FUNCTIONS. UNLESS OTHERSISE INDICATED. TOP CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED RIGID CEILING.

ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE

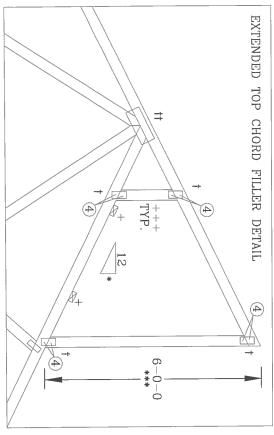
* 16 N6752212 SIONAL ENGINES d com STATE OF MAX. MAX. TOT. SPACING LD. 60 24.0"

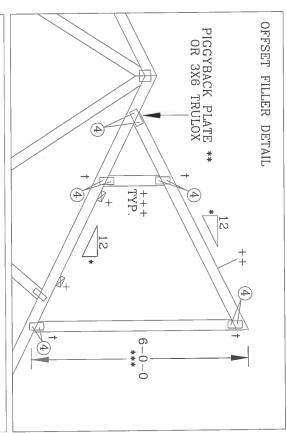
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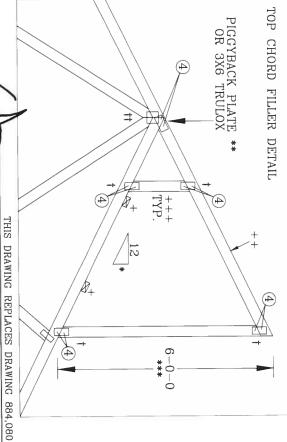
TOP CHORD FILLER DETAIL

- + 2X4 CONTINUOUS LATERAL BRACING AT 24" O.C. MAXIMUM SPACING. ATTACH TO EACH TOP CHORD WITH BRACING MATERIAL TO BE SUPPLIED AND ATTACHED (2) 16d COMMON (0.162"X 3.5", MIN) NAILS.
- AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR. 2X4 SO. PINE #2 N OR SPF #1/#2 FILLER TOP CHORD.
- +++ 2X4 SO. PINE #3 OR SPF #1/#2 VERTICAL WEBS SPACED OC MAXIMUM.
- 8/12 MAXIMUM PITCH.
- ** 2X8.25 PIGGYBACK SPECIAL PLATE. SEE DRAWING PIGBACKB0699 FOR PIGGYBACK SPECIAL PLATE INFORMATION.
- *** 6'0" MAXIMUM HEIGHT.
- † W2X4 OR 3X6 TRULOX.
- †† REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS SHOWN DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT

0.120"X 1.375" NAILS REQUIRED IN CIRCLES MUST BE APPLIED TO EACH FACE OF EACH TRUSS PLY. SEE DWG. 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS. FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED









WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANGLING, SHIPPING, INSTALLING SPACING, REFER TO BEST GBULDING COMPIDANT SAFETY INFORMATION, PUBLISHED BY TPF (FRUSS F BRACING, REFER TO BEST GBULDING COMPIDANT SAFETY NEGRONALIDA, PUBLISHED BY TPF (FRUSS F COMPINATIONE). AND BUTCH A CYCOD TRUSS COUNTAINT OF SAFETY PRACTICES PRIDE TO PERFORMING FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTUS PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

WAIMPORTANTAM FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW BCG, INC. NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN ANY FAILURE TO BUILD THE TRUSS CONFIDENCE WITH THE OF FABRICATING, HANDLING, SHEPPIN, INSTALLING & BEACING OF TRUSSE DESIGN CONFIDENC WITH APPLICABLE PROVISIONS OF NDS (MATIDINAL DESIGN SPEC, BY AFRAN AND ITW. BCG CONNECTOR PLATES ARE MADE OF POLYSIONS OF NDS (MATIDINAL DESIGN SPEC, BY AFRAN AND ITW. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THE GOLV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THE GOLV. STEEL FOR THE TRUSS CONFIDENCE OF PROPESS AND UNLESS TOLLOWED BY () SHALL B DESIGN AND AND ADDEDIES OF THIS CORPORATION OF THE BUILDING DESIGNER, AND THE SHAPPING OF THE SOURCE OF PROPESS ENGINEERING RESPONSIBILITY SCIELLY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABLE LISE OF THIS CORPORATION FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, ANSI/TPI I SEC. 2.

* SSONAL ENGINEE STATE OF No. 52212 ВС SPACING

TC DL TC TOT. BC DL DUR. FAC. EL LD. MAXMAX MAX MAX 30 PSF 1.15 OR 1.33 55 15 10 PSF 0 PSF PSF PSF DATE DRWG REF -ENG TCFILLER0207 2/23/07 SJP/KAR TC-FILLER

BOTTOM CHORD

OPTIONAL INTERIOR OR CANTILEVER BEARING. MINIMUM PLATE OR VERTICAL MEMBER MUST COINCIDE WITH BEARING LOCATION. SIZES (1X3 WAVE) MAY BE USED IF BEARING IS OMITTED. WEDGE

0.120" X 1.375", NAILS, REQUIRED FOR NAILING AND TRULOX PLATE REQUIREMENTS NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO EACH FACE OF THE TRUSS. SEE DWG. 160TL FOR TRULOX PLATE ATTACHMENT

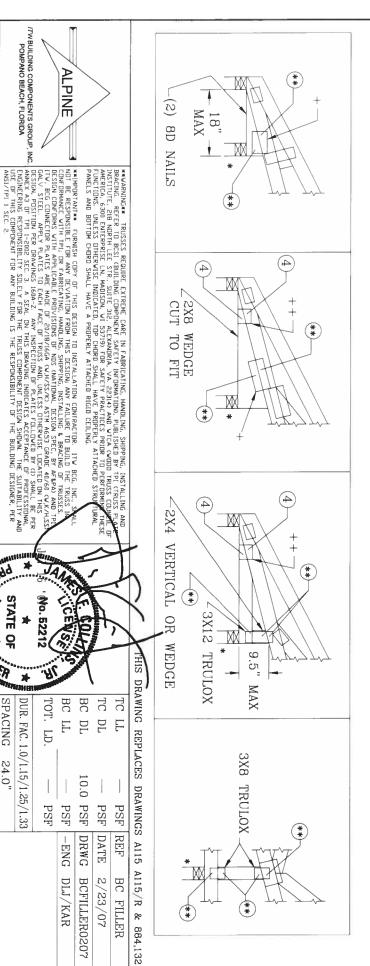
3X4 WAVE OR 4X8 TRULOX

++ 2X4 WAVE OR 3X6 TRULOX

DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN. REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS

MAY BE REQUIRED TO ACCOMODATE REQUIRED NAILS (**) TRULOX PLATES SHOWN ARE MINIMUMS. LARGER PLATES

FILLER BOTTOM CHORD	MAXIMUM REACTION	EACTION	MINIMIM	** REQUIRED NAI	Γ	PER FACE WITH TRULOX PLATES	1 TRULOX P	LATES
OR WEDGE SPECIES	DOWNWARD	UPLIFT	₹EA	1.00 D.O.L.	1.15 D.O.L.	1.25 D.O.L.	D.O.L. 1.33 D.O.L.	1.60 D.O.L
DOUGLAS FIR-LARCH	3281#	1656#	1.5" X 3.5"	12	11	10	9	8
HEM-FIR	2126#	1095#	1.5" X 3.5"	9	8	7	7	တ
SPRUCE-PINE-FIR	2231#	1192#	1.5" X 3.5"	10	9	8	8	6
SOUTHERN PINE DENSE	3465#	1791#	1.5" X 3.5"	12	11	10	9	8
SOUTHERN PINE	2966#	1492#	1.5" X 3.5"	10	9	8	8	7
SOUTHERN PINE NON-DENSE	2520#	1343#	1.5" X 3.5"	9	8	7	7	တ



SPACING

24.0"

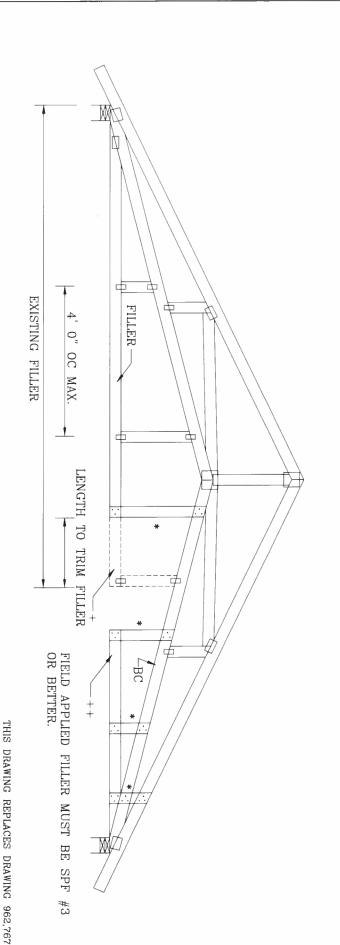
BOTTOM CHORD FILLER REPAIR

RECOMMENDED REPAIR PROCEDURE

- 1. MEASURE DISTANCE FOR NEW LENGTH OF FILLER
- 2. APPLY NEW 2X4 STUD GRADE OR BETTER VERTICAL SCAB TO BOTTOM CHORD AND FILLER WITH (3) NAILS 0.131" DIA. x 3.0" OR LARGER, (I.E. 10d OR 16d COMMON, SINKER, GUN, OR 16d BOX NAILS) TO EACH END OF VERTICAL.
- 3. CAREFULLY REMOVE EFFECTED CONNECTOR PLATES.
 USE CARE NOT TO DAMAGE THE REMAINING
 CONNECTOR PLATES OR LUMBER IN ANY WAY.
- 4. TRIM FILLER TO LENGTH, AT EDGE OF NEW VERTICAL SCAB.

- MAXIMUM BOTTOM CHORD LOAD IS 10 PSF.
- BOTTOM CHORD FILLER TO BE REMOVED. SEE NOTE #3.
- ++ FIELD APPLIED FILLER.
- * 2X4 STUD GRADE OR BETTER VERTICAL SCAB.
 ATTACH TO BOTTOM CHORD AND FILLER WITH (3)
 NAILS WITH A MIN. 0.131" DIA. X 3.0" LENGTH.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR ALLOWABLE FILLER DIMENSIONS, PLACEMENT, AND WEBBING





WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SMEPPING, INSTALLING AND BRACING. REFER TO BEST GUILLIONG COMPENENT SMETTY INSCRAWATING, PUBLISHED BY THE (TRUSS PLATE INSTITUTE, 218 MORTH, LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND WICA CHOOD TRUSS COUNCIL OF MERITAGE, 6300 ENTERPRISE LM, MANISON, WI 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING THE FUNCTIONS. UNLESSES OTHERWISE INDICATED, TOP CHOOD SHALL HAVE PROPERLY ATTACHED STRUCTUSE PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTUSE.

WATHER RAITH FURNISH COPY OF THIS DESIGN TO INSTALLATION COMPRACTOR. IT'Y BCG, INC. SHALL
NOT BE RESPONSIBLE FOR ANY DEVIATION FORM HIS DESIGNA MAY FAILURE TO BUILD THE TRUES IN
COMPONENCE WITH APPLICABLE PROVISIONS OF NIS (NATIONAL DESIGN SECRE BY AFEAN AND THE)
TITY, BCG CONNECTOR PLATES ARE HADE OF BOYLSTONS OF NIS (NATIONAL DESIGN SECRE BY AFEAN AND THE
OLIVEN STELL, APPLY PLATES TO EACH FACE OF TRUES AND UNLESS OF THE MASS ARE CONTROLLED ON THE
BESTON DOSITION PER DRAWNING 160A-Z. ANY INSPECTION OF PATES ACCUPATED WITH THE SUTTABLITY AND UNICATES ACCUPATED SHALL BE
CHINEERING RESPONSIBILITY SCIELLY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABLITY AND UNSET OF THIS COMPONENT FOR ANY BUILDING ISS THE RESPONSIBILITY OF THE BUILDING DESIGNARE, FER

STATE OF MINING

REF BC FILLER REP.

DATE 2/23/07

DRWG REPBCFIL0207

-ENG MLH/KAR