

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 21.07 ft mean hgt, ASCE 7 02, CLOSED bldg, located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC
DL=2.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Bottom chord checked for 20.00 psf non concurrent live load.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

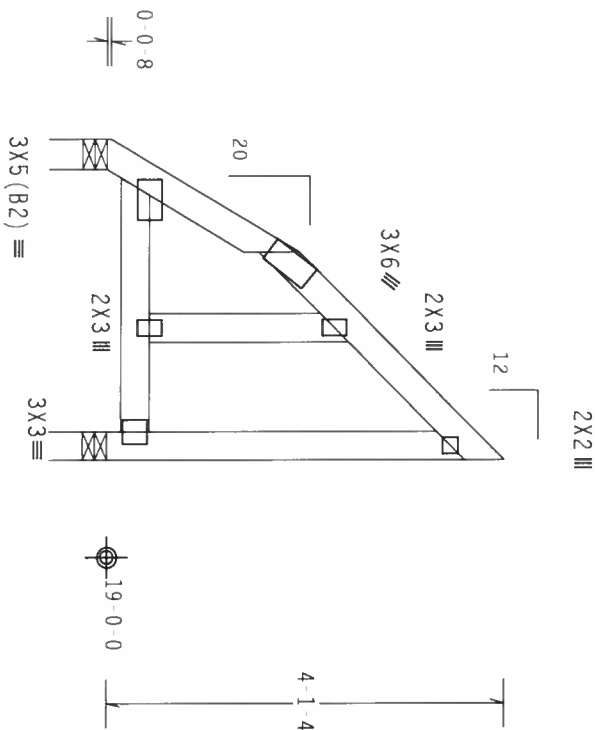
SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 79 PLF at -0.42 to 79 PLF at 0.74
TC - From 68 PLF at 0.74 to 68 PLF at 2.88
BC - From 4 PLF at -0.42 to 4 PLF at 2.88

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.



0-8-15 2-1-10
3-3-8 over 2 Supports
R=137 W=3.79" R=113 U=108 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

7.36.0424.12 QTY:2 FL/-/4/-/-/R/-

Scale =.5"/Ft.

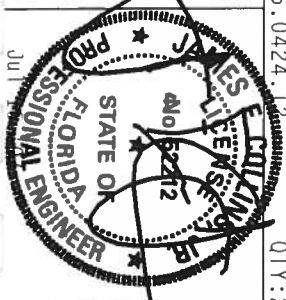
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN LIFTING, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BCSI (CONSULTING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218
NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WICK WOOD TRUSS COUNCIL OF AMERICA, 6300
ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT
BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH
THIS OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (AISC 360), AISC 360S, AISC 360M, AISC 360T, AISC 360U, AISC 360V, AISC 360W, AISC 360X, AISC 360Y, AISC 360Z, AISC 360AA, AISC 360AB, AISC 360AC, AISC 360AD, AISC 360AE, AISC 360AF, AISC 360AG, AISC 360AH, AISC 360AI, AISC 360AJ, AISC 360AK, AISC 360AL, AISC 360AM, AISC 360AN, AISC 360AO, AISC 360AP, AISC 360AQ, AISC 360AR, AISC 360AS, AISC 360AT, AISC 360AU, AISC 360AV, AISC 360AW, AISC 360AX, AISC 360AY, AISC 360AZ, AISC 360BA, AISC 360BB, AISC 360BC, AISC 360BD, AISC 360BE, AISC 360BF, AISC 360BG, AISC 360BH, AISC 360BI, AISC 360BJ, AISC 360BK, AISC 360BL, AISC 360BM, AISC 360BN, AISC 360BO, AISC 360BP, AISC 360BQ, AISC 360BR, AISC 360BS, AISC 360BT, AISC 360BU, AISC 360BV, AISC 360BW, AISC 360BX, AISC 360BY, AISC 360BZ, AISC 360CA, AISC 360CB, AISC 360CC, AISC 360CD, AISC 360CE, AISC 360CF, AISC 360CG, AISC 360CH, AISC 360CI, AISC 360CJ, AISC 360CK, AISC 360CL, AISC 360CM, AISC 360CN, AISC 360CO, AISC 360CP, AISC 360CQ, AISC 360CR, AISC 360CS, AISC 360CT, AISC 360CU, AISC 360CV, AISC 360CW, AISC 360CX, AISC 360CY, AISC 360CZ, AISC 360DA, AISC 360DB, AISC 360DC, AISC 360DD, AISC 360DE, AISC 360DF, AISC 360DG, AISC 360DH, AISC 360DI, AISC 360DJ, AISC 360DK, AISC 360DL, AISC 360DM, AISC 360DN, AISC 360DO, AISC 360DP, AISC 360DQ, AISC 360DR, AISC 360DS, AISC 360DT, AISC 360DU, AISC 360DV, AISC 360DW, AISC 360DX, AISC 360DY, AISC 360DZ, AISC 360EA, AISC 360EB, AISC 360EC, AISC 360ED, AISC 360EE, AISC 360EF, AISC 360EG, AISC 360EH, AISC 360EI, AISC 360EJ, AISC 360EK, AISC 360EL, AISC 360EM, AISC 360EN, AISC 360EO, AISC 360EP, AISC 360EQ, AISC 360ER, AISC 360ES, AISC 360ET, AISC 360EU, AISC 360EV, AISC 360EW, AISC 360EX, AISC 360EY, AISC 360EZ, AISC 360FA, AISC 360FB, AISC 360FC, AISC 360FD, AISC 360FE, AISC 360FF, AISC 360FG, AISC 360FH, AISC 360FI, AISC 360FJ, AISC 360FK, AISC 360FL, AISC 360FM, AISC 360FN, AISC 360FO, AISC 360FP, AISC 360FQ, AISC 360FR, AISC 360FS, AISC 360FT, AISC 360FU, AISC 360FV, AISC 360FW, AISC 360FX, AISC 360FY, AISC 360FZ, AISC 360GA, AISC 360GB, AISC 360GC, AISC 360GD, AISC 360GE, AISC 360GF, AISC 360GG, AISC 360GH, AISC 360GI, AISC 360GJ, AISC 360GK, AISC 360GL, AISC 360GM, AISC 360GN, AISC 360GO, AISC 360GP, AISC 360GQ, AISC 360GR, AISC 360GS, AISC 360GT, AISC 360GU, AISC 360GV, AISC 360GW, AISC 360GX, AISC 360GY, AISC 360GZ, AISC 360HA, AISC 360HB, AISC 360HC, AISC 360HD, AISC 360HE, AISC 360HF, AISC 360HG, AISC 360HH, AISC 360HI, AISC 360HJ, AISC 360HK, AISC 360HL, AISC 360HM, AISC 360HN, AISC 360HO, AISC 360HP, AISC 360HQ, AISC 360HR, AISC 360HS, AISC 360HT, AISC 360HU, AISC 360HV, AISC 360HW, AISC 360HX, AISC 360HY, AISC 360HZ, AISC 360IA, AISC 360IB, AISC 360IC, AISC 360ID, AISC 360IE, AISC 360IF, AISC 360IG, AISC 360IH, AISC 360II, AISC 360IJ, AISC 360IK, AISC 360IL, AISC 360IM, AISC 360IN, AISC 360IO, AISC 360IP, AISC 360IQ, AISC 360IR, AISC 360IS, AISC 360IT, AISC 360IU, AISC 360IV, AISC 360IW, AISC 360IX, AISC 360IY, AISC 360IZ, AISC 360JA, AISC 360JB, AISC 360JC, AISC 360JD, AISC 360JE, AISC 360JF, AISC 360JG, AISC 360JH, AISC 360JI, AISC 360JJ, AISC 360JK, AISC 360JL, AISC 360JM, AISC 360JN, AISC 360JO, AISC 360JP, AISC 360JQ, AISC 360JR, AISC 360JS, AISC 360JT, AISC 360JU, AISC 360JV, AISC 360JW, AISC 360JX, AISC 360JY, AISC 360JZ, AISC 360KA, AISC 360KB, AISC 360KC, AISC 360KD, AISC 360KE, AISC 360KF, AISC 360KG, AISC 360KH, AISC 360KI, AISC 360KJ, AISC 360KK, AISC 360KL, AISC 360KM, AISC 360KN, AISC 360KO, AISC 360KP, AISC 360KQ, AISC 360KR, AISC 360KS, AISC 360KT, AISC 360KU, AISC 360KV, AISC 360KW, AISC 360KX, AISC 360KY, AISC 360KZ, AISC 360LA, AISC 360LB, AISC 360LC, AISC 360LD, AISC 360LE, AISC 360LF, AISC 360LG, AISC 360LH, AISC 360LI, AISC 360LJ, AISC 360LK, AISC 360LL, AISC 360LM, AISC 360LN, AISC 360LO, AISC 360LP, AISC 360LQ, AISC 360LR, AISC 360LS, AISC 360LT, AISC 360LU, AISC 360LV, AISC 360LW, AISC 360LX, AISC 360LY, AISC 360LZ, AISC 360MA, AISC 360MB, AISC 360MC, AISC 360MD, AISC 360ME, AISC 360MF, AISC 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ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
P.O. Box 1000, Haines City, FL 33844



TC LL	20.0 PSF	REF R8228- 81246
TC DL	10.0 PSF	DATE 07/16/07
BC DL	10.0 PSF	DRW HCUR8228 07197024
BC LL	0.0 PSF	HC-ENG TCE/WHK
TOT. LD.	40.0 PSF	SEON- 1648
DUR. FAC.	1.25	
SPACING	24.0"	JREF - 1T938228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 24.67 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

In lieu of structural panels or rigid ceiling use purtins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

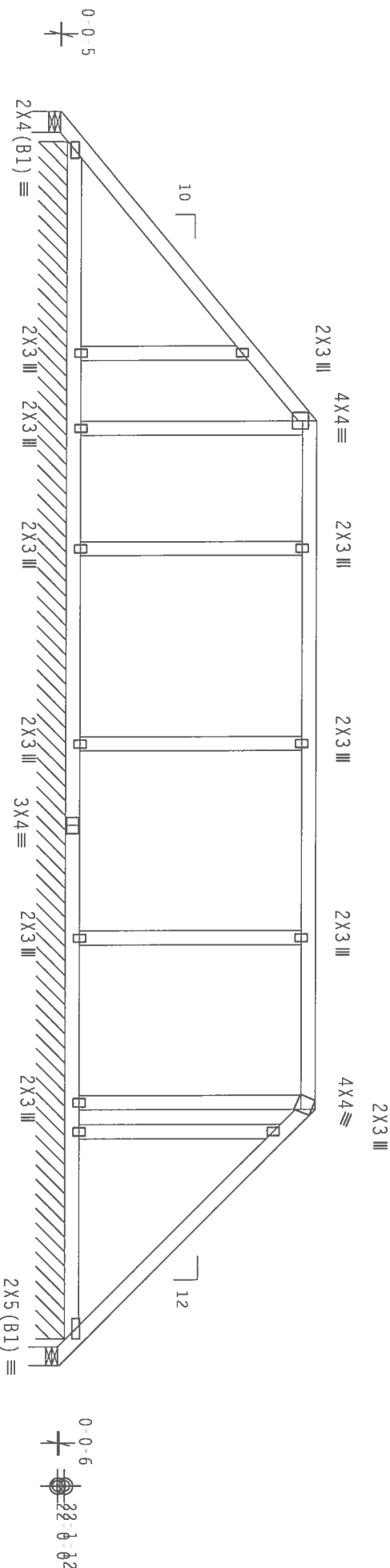
Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS

LUMBER		DUR.FAC. = 1.25 / PLATE DUR.FAC. = 1.25
TC - From	66 PLF at -0.62 to 66 PLF at 5.71	
TC - From	66 PLF at 5.71 to 66 PLF at 19.87	
TC - From	68 PLF at 19.87 to 68 PLF at 25.15	
BC - From	4 PLF at -0.62 to 4 PLF at 25.15	

Wind reactions based on MWFRS pressures.

Bottom chord checked for 20.00 psf non-concurrent live load.



$$R = 98 \text{ U-158 W=5.077"}$$

$$R = 82 \text{ PLF U=36 PLF W=24 7-2}$$

$$R = 116 \text{ U-103 W=4.596"}$$

Note: All Plates Are 2X3 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

7.36.0424 12

QTY:1 FL/-/4/-/-/R/-

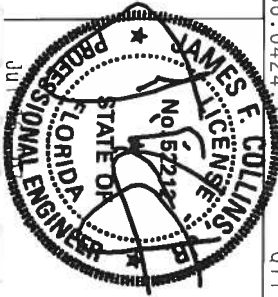
Scale = .3125"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN COMPROMISES WITH APPLICABLE PROVISIONS OF 2003 NATIONAL DESIGN SPEC. BY AIA/RA AND TPI. DESIGNER SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY DEVIATION FROM THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE DESIGNER. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC 3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
Toll-free 1-800-44-ALPINE



TC LL	20.0 PSF	REF R8228- 81247
TC DL	10.0 PSF	DATE 07/16/07
BC DL	10.0 PSF	DRW HCUSR8228 07197025
BC LL	0.0 PSF	HC-ENG TCE/WHK
TOT. LD.	40.0 PSF	SEON- 1463
DUR.FAC.	1.25	
SPACING	24.0"	UREF- 1T938228201

110 mph wind, 23.67 ft mean hgt, ASCE 7-02, closed bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, $I_w=1.00$ $G_{p1}(+/-)$ -0.18

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

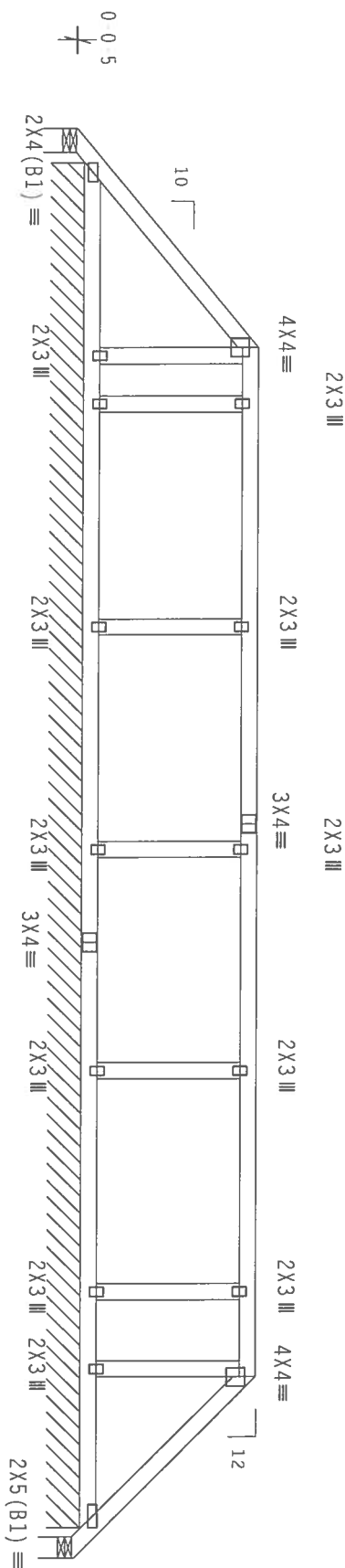
Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS
(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 66 PLF at -0.62 to 66 PLF at 3.31
TC - From 66 PLF at 3.31 to 66 PLF at 21.87
TC - From 68 PLF at 21.87 to 68 PLF at 25.15
BC - From 4 PLF at -0.62 to 4 PLF at 25.15

Wind reactions based on MMFRS pressures.

Bottom chord checked for 20.00 psf non concurrent live load

 $R = 50 \cup 93 \text{ W } 5.077''$

R=76 PLF U-29 PLF W=24 7-2

 $R = .35 \quad U = 34 \quad W = 4.596$

Note: All Plates Are 2X3 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

 $Cq/RT=1.00(1.25)/10(0)$

7.36.0424

QTX:1

FL/4/-/R/-

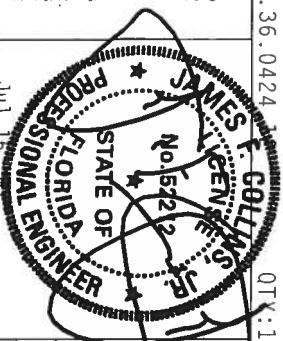
Scale = .3125"/ft.

WARNING—FIRE'S RESCUE EXTINGUISHING CASE IN INVESTIGATION. HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO GC&I (DOWDING COMPONENT IN SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH 1ST STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WICA (WOOD TRUSS CONSULTING), 65000 CREEPER LAKE, MADISON, WI, 53719 FOR SAFETY PRACTICES RELATING TO PERFORMING THESE OPERATIONS. UNDESIGNED, UNDESIGNED, INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CHANG.

A PROPERLY ATTACHED RIGID CEILING

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
F1 Certificate # 140107

[illegible]

Jul 16 07

DUR.FAC.	1.25
SPACING	24.0"

JREF - 1T938228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 33.62 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf.

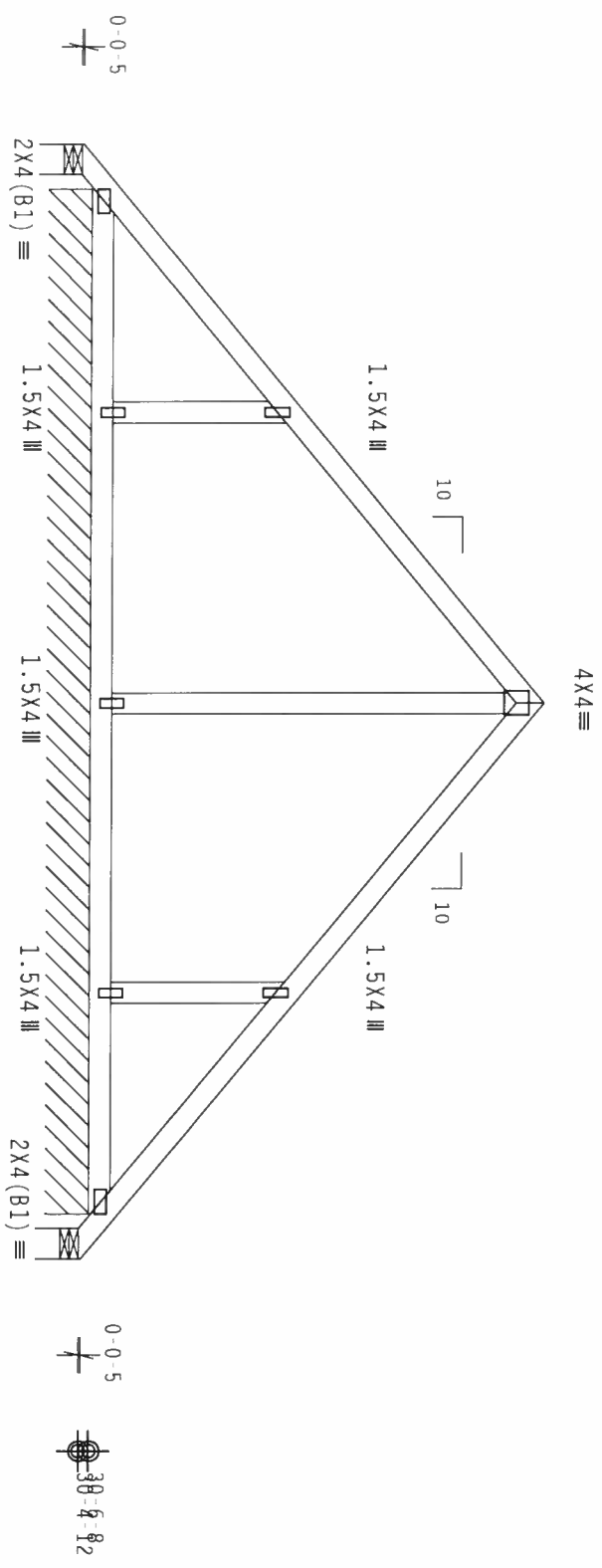
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS

(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 66 PLF at -0.62 to 66 PLF at 14.74
BC - From 4 PLF at -0.62 to 4 PLF at 14.74

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.
Bottom chord checked for 20.00 psf non-concurrent live load.



7-0-11
7-0-11
15-4-6 Over 3 Supports
R=2 U=180 W=5.077"
R=75 PLF U=38 PLF W=14-1-6
R=2 U=180 W=5.077"

PLT TYP. Wave

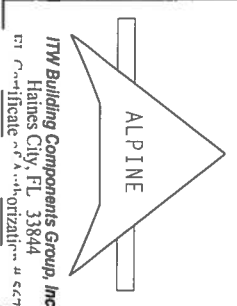
Design Crit: TPI-2002 (STD) /FBC
Cq/RT=1.00(1.25)/10(0)

7.25.0411
OTY:6 FL/-/4/-/-/R/-

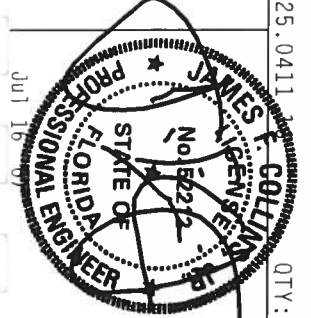
Scale = .375"/Ft.

WARNING TRUSSES REQUIRING EXTERNAL CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR GOOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.



ITW Building Components Group, Inc.
Haines City, FL 33844
Tel: 888.444.4444
Fax: 888.444.4444



TC LL	20.0 PSF	REF	R8228 - 81249
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197028
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEQN	107149 REV
DUR.FAC.	1.25		
SPACING	24.0"	JREF	1T938228Z01

100

Bottom chord checked for 20.00 psf non-c














 $Cq/RT=1.00(1.25)/10(0)$

7.36.0424 QTY:1

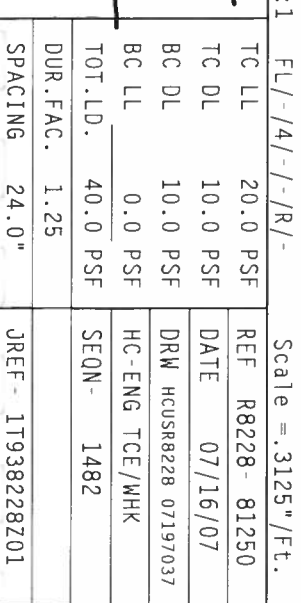
QTY:1 FL/-/4/-/-/R/-

Scale = .3125"/Ft.

JAMES F. COLLINS
LIBRARIAN
No. 12012

Year	Actual (%)	Projected (%)
1950	7.5	-
1960	8.5	-
1970	9.5	-
1980	10.5	-
1990	11.5	-
2000	12.5	12.5
2010	-	14.5
2020	-	16.5
2030	-	17.5
2040	-	18.5
2050	-	18.5

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE STABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2



JREF - 1T938228Z01

110 mph wind, 27.67 ft mean hgt, ASCE 7-02, closed bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ gcpi(+/-)=0.18

(A) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 20.00 psf non-concurrent live load.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

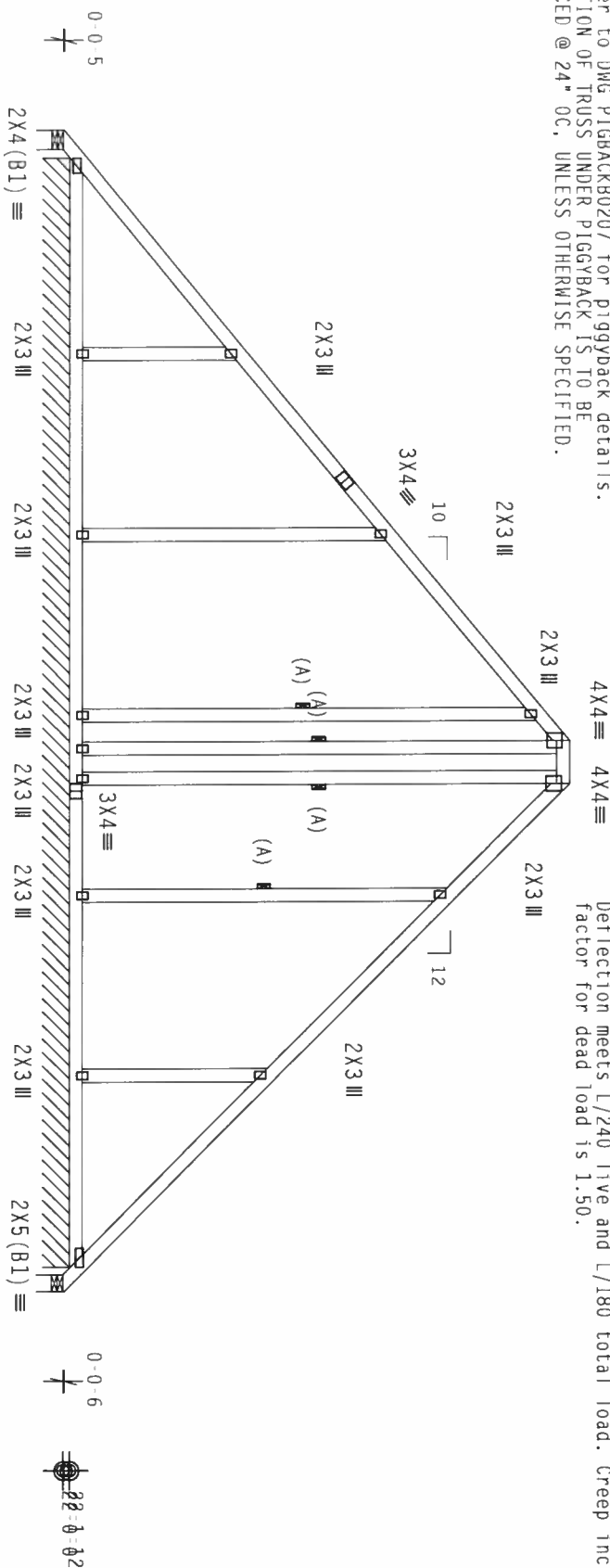
SPECIAL LOADS

-----LUMBER	DUR.FAC.=1.25	/	PLATE	DUR.FAC.=1.25
TC - From	66 PLF at -0.62	to	66 PLF at 12.91	
TC - From	66 PLF at 12.91	to	66 PLF at 13.87	
TC - From	68 PLF at 13.87	to	68 PLF at 25.15	
BC - From	4 PLF at -0.62	to	4 PLF at 25.15	

Wind reactions based on MMFRS pressures.

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

 $R = -.86 \quad U = 266 \quad W = 5.077''$

R=81 PLF U=39 PLF W=24-7-2

Note: All Plates Are 2X3 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/10(0)$

7.36.0424

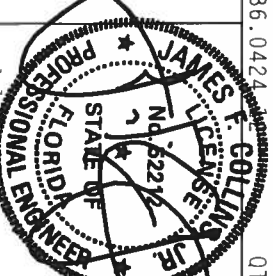
QTY:1 FL/-/4/-/-/R/-

Scale = .25"/Ft.

WARNING: THESE PANELS REQUIRE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND PATCHING. RETURN TO SCS1 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE CRUSS PAPER INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 65000 INTERSTATE LANE, MIDLAND, MI 48691) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIDGE CEILING.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
E1 Certificate of Authorization # 567



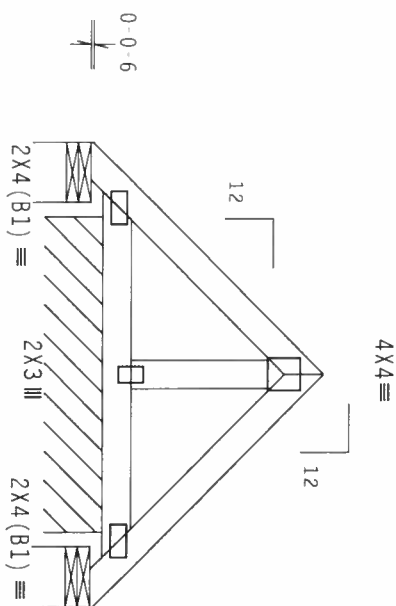
TC LL	20.0 PSF	REF	R8228- 81252
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197039
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEQN-	1477
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1T938228Z01

	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense	
	webs	2x4	SP	#3		

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.



21-4-42

R=36 U=29 W=7.425"
(1.328" Effective Contact)

R=86 PLF U=36 PLF W=3-2-14

R=16 U=2 W=7.425"
(1.328" Effective Contact)

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

7.36.0424.12

QTY:11 FL/-/4/-/-/R/-

Scale = .5" / Ft.

WARNING: FIRST-ALONGER EXTENSIVE CASE IN PRACTITIONER, HANDLING, SHIPPING, INSTALLING AND BRACING RIGS TO DECK (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY THE (STEEL PLATE INSTITUTE, 218 NORTH 1ST STREET, SUITE 312, ALEXANDRIA, VA 22314) AND WEA (WOOD TRUSS COUNCIL OF AMERICA, 65000 ENTERPRISE LANE, HANOVER, NH 03749) FOR SAFETY PRACTICES PRIOR TO REINFORCING THESE COMPONENTS. UNDESIGNED INDIVIDUAL TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE USER. THE USER SHALL BE RESPONSIBLE FOR THE PROPER DESIGN, INSTALLATION, MAINTENANCE, AND REMOVAL OF THE TRUSS. THE USER SHALL BE RESPONSIBLE FOR THE PROPER DESIGN, INSTALLATION, MAINTENANCE, AND REMOVAL OF THE TRUSS. THE USER SHALL BE RESPONSIBLE FOR THE PROPER DESIGN, INSTALLATION, MAINTENANCE, AND REMOVAL OF THE TRUSS.

CONNECTIONS WITH APPLICABLE PROVISIONS FOR JOBS (NATIONAL DESIGN SPEC., BY AIAA) AND PPL. THE BRIDGE CONNECTOR PLATES ARE MADE OF 20/18/16GA (H, H/55/K) ASTM A653 GRADE 40/60 (H, K/H, 55) GALV. STEEL. APPLY PLATES TO EACH FACE OF RIBS AND UNLESS OTHERWISE NOTED ON THIS DESIGN POSITION PER DRAWING TEND.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF IP11 2002 SEC.3
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE
A SEAL ON THIS

DESIGN SHOW THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

SPECIAL LOADS

	DUR.	FAC.=1.25	/	PLATE	DUR.	FAC.=1.25
TC - From	68 PLF	at -0.51	to	68 PLF	at	1.88
TC - From	68 PLF	at	1.88	to	68 PLF	at 4.29
BC - From	4 PLF	at -0.51	to	4 PLF	at	4.26

Wind reactions based on MFRS pressures.

Bottom chord checked for 20.00 psf non-concurrent live load

FL/-/4/-/-/R/-/

Scale = .5" / Ft.

TC LL 20.0 PSF

REF R8228 - 81254

~~100~~ 100 PSE

DATE 07/16/07

500

[illegible]

8
5
10
10
10

DIN 11603A0220

0.0 psf 77 lb

HC-ENG ICE/W


TOT.LD. 40.0 PSF

SEQN- 1564

DUR.FAC. 1.25

SPACING 24 0"

REF ID: A638228701



ITW Building Components Group, Inc.
Haines City, FL 33844
To Qualify: Call 800-Organization 4422

[illegible]

Professional Engineer Seal for the State of Florida, No. B2242, signed by J. M. Williams.

TCE DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197044
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEON-	1564
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1T938228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

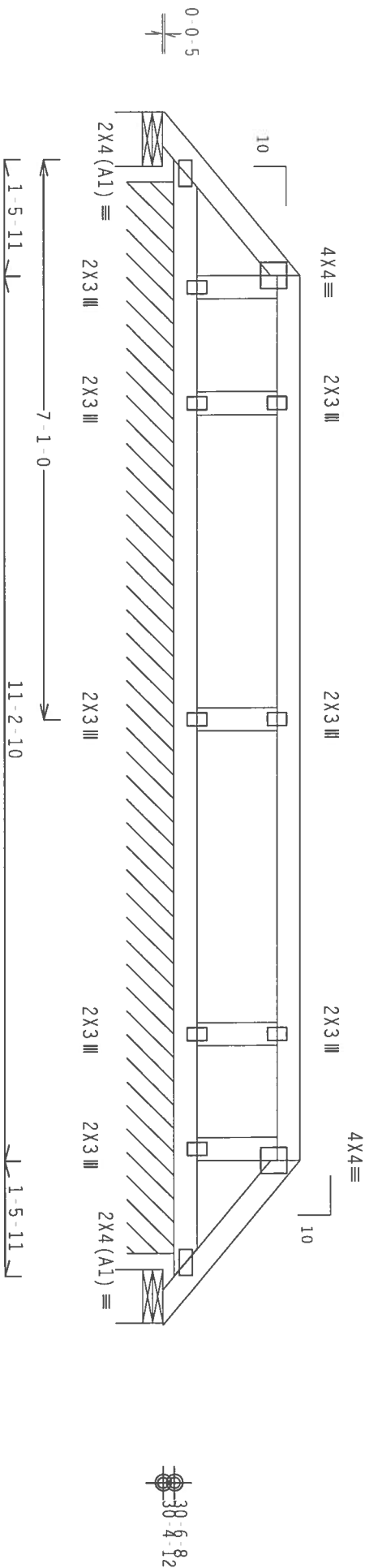
Bottom chord checked for 20.00 psf non-concurrent live load.

Refer to DWG PIGBACK0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

110 mph wind, 31.27 ft mean hgt, ASCE 7-02, CLOSED bldg, not
located within 4.50 ft from roof edge, CAT II, EXP B, wind TC
DL=5.0 psf, wind BC DL=2.0 psf. lw=1.00 Gcpi (+/-)-0.18

In lieu of structural panels or rigid ceiling use purlins to
brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.



R=49 U=16 W=8.201"
(1.024" Effective Contact)

15'-4'-6" Over 3 Supports
R=73 PLF U=35 PLF W=13'-7'-2

R=26 U=11 W=8.201"
(1.024" Effective Contact)

Note: All Plates Are 2X3 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

7.36.0424

QTY: 1

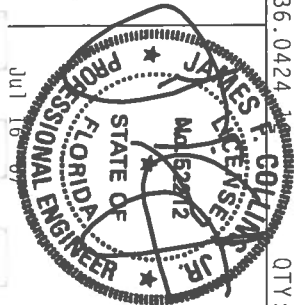
FL/-/4/-/-/R/-

Scale = .5"/ft.

****WARNING**** TRUSSES BEARING EXISTING LOAD TO FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH
TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (WOOD TRUSS) COUNCIL OF AMERICA, 6300
ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
Tel: 888-444-4444
Fax: 888-444-4444



TC LL	20.0 PSF	REF	R8228- 81255
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197045
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEON-	2681
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1T938228201

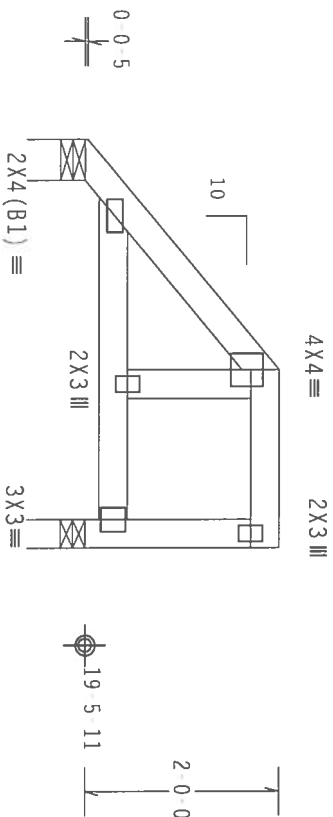
REF	R8228-81256
DATE	07/16/07
DRW	HCUSR8228 07197047
HC-ENG	TCE/WHK
SEQN-	1634
JREF-	1T938228Z01

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
Wbs	2x4	SP	#3	

110 mph wind, 20.48 ft mean hgt., ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $1w=1.00$ GCPI(+/-)=0.18

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Refer to DWG PIGBACK0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/10(0)$

7.36.0424 QTY:1

QTY:1 FL/-/4/-/-/R/-

Scale = .5" / Ft.

WARNING: THESE REINFORCING CABLES, IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO A631 (BUILDING CONSTRUCTION SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND AISC 4000 TRUSS CONNECTIONS, 65000 FORT LINDSEY LANE, MANASSAS, VA, 20108 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED GRID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH ITPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONDITIONS FOR APPLICABLE PORTIONS OF MODIFIED ADDITIONAL DESIGN SPEC. (BY AIRMA) AND THE CONDUCTOR PLATES ARE MADE OF 20/18/16GA (W/H/55/2) ASTM A653 GRADE 40/60 (W/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND ONE ORIENTED LAYER LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AIRMA AS OF 11/1/2007 SEC. 3. A SEAL ON THIS DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLICIT FOR THE TRUSS COMPONENTS OF THE DESIGN SHOW. THE SATISFACTION AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AIN/1/1 SEC. 2.

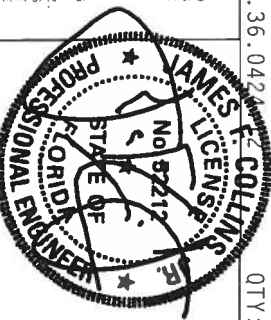
SPECIAL LOADS

TC	From	66 PLF at 0.62 to	66 PLF at 1.74
TC	From	66 PLF at 1.74 to	66 PLF at 3.57
BC	From	4 PLF at 0.62 to	4 PLF at 3.57

Wind reactions based on MIFRS pressures.

Bottom chord checked for 20.00 psf non-concurrent live load

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



TC LL	20.0 PSF	REF	R8228- 81257
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197054
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEQN-	1835
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1T938228Z01

SPECIAL LOADS

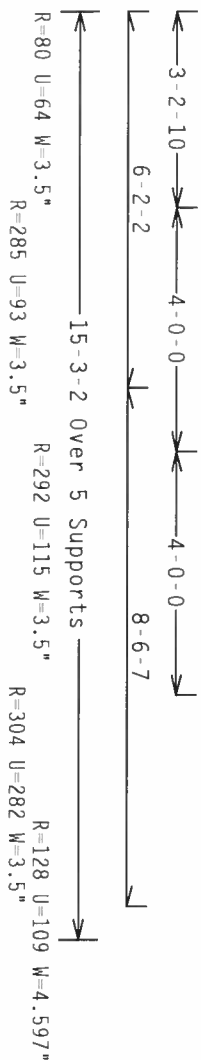
LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 66 PLF at 0.00 to 66 PLF at 6.18
TC - From 68 PLF at 6.18 to 68 PLF at 15.26
BC - From 4 PLF at 0.00 to 4 PLF at 15.26

Wind reactions based on MMFRS pressures.

(A) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

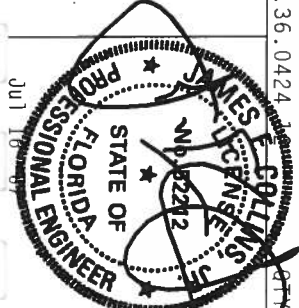
Bottom chord checked for 20.00 psf non-concurrent live load.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.



Scale = .3125"/Ft.

ITW Building Components Group, Inc.
Haines City, FL 33844
Telephone: 800/368-7272



TC LL	20.0 PSF	REF	R8228- 81258
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197073
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEQN-	2103
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1T938228Z01

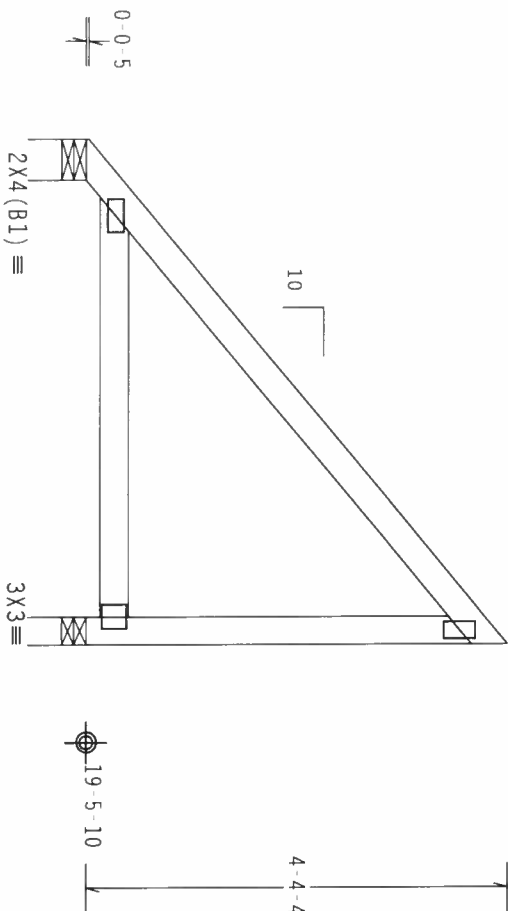
110 mph wind, 21.66 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, $I_w=1.00$ gcpi(+/-)=0.18

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

2X4 III



SPECIAL LOADS

-----	(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC From	66 PLF at -0.62 to 66 PLF at 4.57
BC From	4 PLF at -0.62 to 4 PLF at 4.57

Wind reactions based on MAFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 20.00 psf non-concurrent live load.

PLT TYP. Wave

Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)$

 $Cq/RT=1.00(1.25)/10(0)$

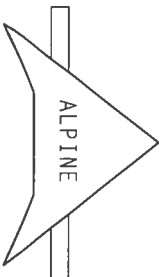
7.36.0424

QTY:9

FL/-/4/-/-/R/-

Scale = .5"/Ft.

5-2-5 Over 2 Supports \rightarrow
 $R=193$ W=5.077" $R=171$ U=113 W=3.5"

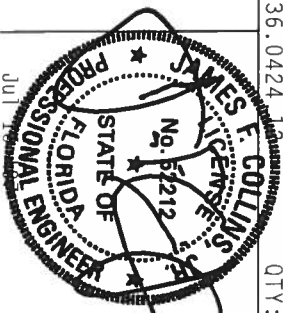


ITW Building Components Group, Inc.
Haines City, FL 33844
E1 Certificate of Authorization # 667

****WARNING**** THIS IS A BUILDING EXERCISE CASE FOR INFORMATION. HANDLING, SHIPPIING, INSTALLING AND BRACING REFER TO BC01 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY IPT (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD TRUSS COMPANY OF AMERICA), 63000 ENTERPRISE LANE, HUNTSVILLE, AL 35893 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** DISTRIBUTE A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, THE BGC, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AREA) AND TPI (TIB RECORD DRAWINGS).
CONNECTION PLATES ARE MADE OF 20/18/16GA (#4/HSS/27) ASH ASSI GABOE 40/60 (K=1.55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AREA X AS OF TPI 1/2002 SEC.3. A SEAL ON 160A-2 DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENTS OF THE DESIGN SHOWS THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2



TC LL	20.0 PSF	REF	R8228- 81259
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197075
BC LL	0.0 PSF	HC-ENG	TCE/WMK
TOT.LD.	40.0 PSF	SEQN-	1843
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1T938228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 25.18 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $1w=1.00 \text{ Gcpi}(+/-)=0.18$

End verticals not exposed to wind pressure.

3x4=

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

2x3=

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

SPECIAL LOADS

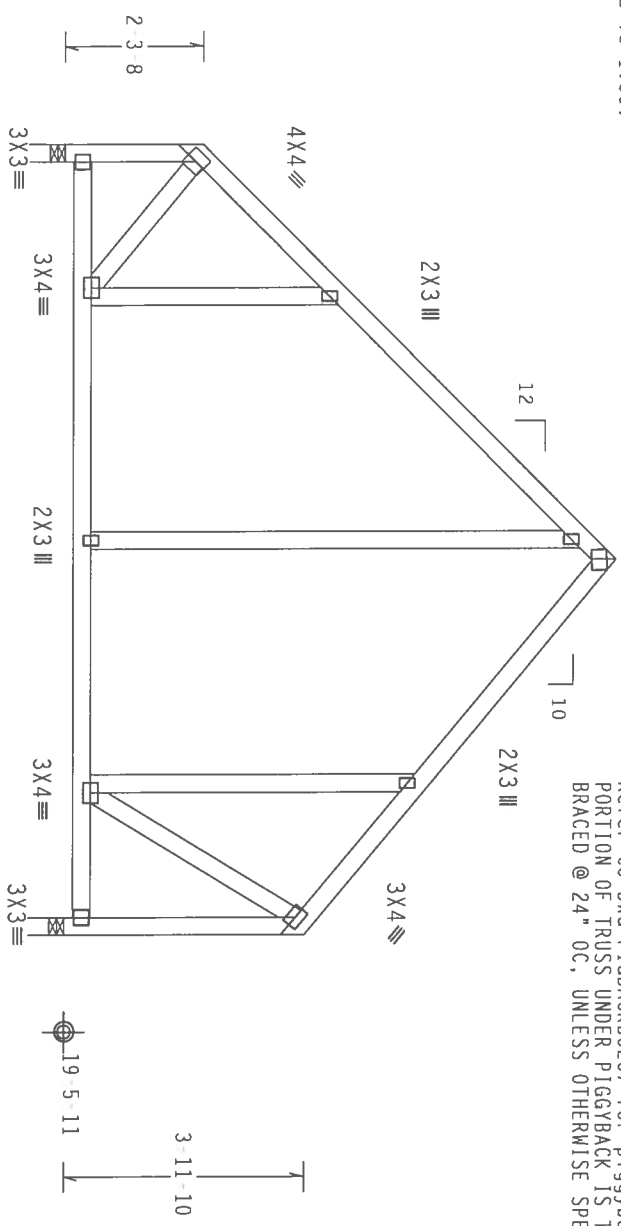
(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC From 68 PLF at 0.00 to 68 PLF at 6.82
TC From 66 PLF at 6.82 to 66 PLF at 13.00
BC From 4 PLF at 0.00 to 4 PLF at 13.00

Wind reactions based on MWFRS pressures.

Calculated horizontal deflection is 0.17" due to live load and 0.16" due to dead load.

Bottom chord checked for 20.00 psf non-concurrent live load.

Refer to DWG PIGBACK80207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.



6-9-14 6-2-2
13-0-0 Over 2 Supports
R=467 U=133 W=3.5"
R=459 U=170 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

7.36.0424.12

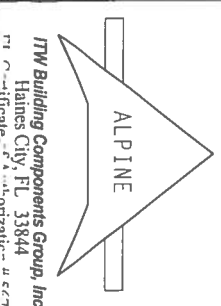
QTY:2 FL/-/4/-/R/-

Scale=.3125"/Ft.

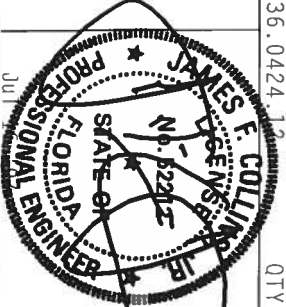
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND MICA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. THE BCG PROJECT NO. 040101/0102 (P. 01/03/01) ASH 1053 GRADE 40/60 (R. 4/11/01) GALV. STEEL. APPLY TO FACTORS AND LOADS. THE TRUSS IS DESIGNED FOR THIS DESIGN. POSITION PER DRAWING 160A.2. ANY INSPECTION OF PLATES FOLLOWED BY A QUALIFIED ENGINEER SHALL BE REQUIRED FOR THE TRUSS. THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS. THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group, Inc.
Haines City, FL 33844
Office: 888.677.2727



TC LL	20.0 PSF	REF	R8228-81260
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197077
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEON	2113
DUR.FAC.	1.25		
SPACING	24.0"	JREF	1T938228Z01

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

End verticals not exposed to wind pressure.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC

 $3 \times 4 =$

Deflection meets L/240 live and L/180 total load. Creep increase in deflection for dead load is 1.50.

SPECIAL LOADS
-----LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 68 PLF at 0.00 to 68 PLF at 6.70
TC - From 66 PLF at 6.70 to 66 PLF at 12.87
BC - From 4 PLF at 0.00 to 4 PLF at 12.87

Wind reactions based on MWFRS pressures.

(A) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

Bottom chord checked for 20.00 psf non-concurrent live load.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

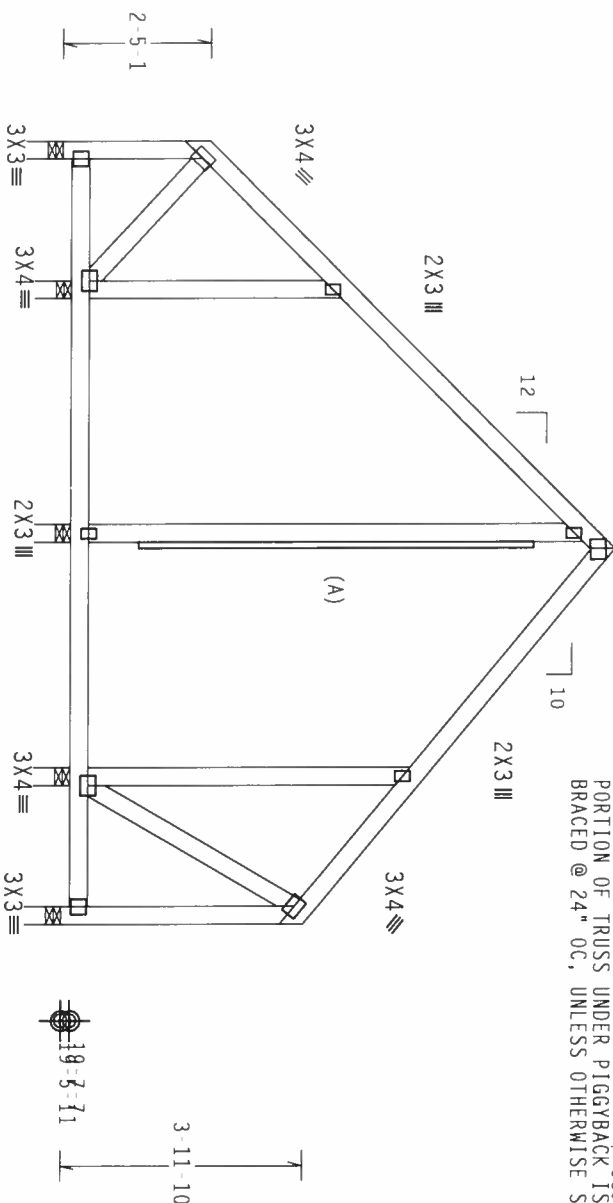


Diagram illustrating the layout of a 12' 10" x 8' over 5 supports beam. The beam is divided into segments with dimensions and support locations indicated by arrows.

Dimensions and Support Locations:

- Segment 1: 2'-5.4" (Support at end)
- Segment 2: 6'-8.6" (Support at end)
- Segment 3: 4'-0.0" (Support at end)
- Segment 4: 4'-0.0" (Support at end)
- Segment 5: 6'-2.2" (Support at end)

Overall dimensions and support locations:

- Overall length: 12'-10.8" over 5 supports
- Support 1: R=116 U=250 W=3.5"
- Support 2: R=229 U=400 W=3.5"
- Support 3: R=243 U=45 W=3.5"
- Support 4: R=125 U=50 W=3.5"
- Support 5: R=203 U=314 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/10(0)$

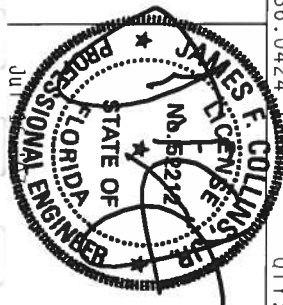
QTY:1 FL/-/4/-/-/R/-

Scale = .3125"/ft.

*****WARNING***** FIRE'S RICHMOND EXTERIOR CEMENT, IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND PRACTICE
REFER TO GC-1 (BOLDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (FIRMS) PLANT INSTITUTE, 218
MORTON LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND MICA (MICA) TRUSS COMPANY OF AMERICA, 65000
ENTERPRISE LANE, ANDOVER, MA 01810 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
PROPERLY ATTACHED RIGID CELLING.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
Certificate of Authorization # 567



TC LL	20.0 PSF	REF	R8228- 81261
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197088
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT. LD.	40.0 PSF	SEQN-	2165
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1T938228Z01

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11/11/2011



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JREF- 1T938228Z01

BC - From 4 PLF at -0.55 to
Wind reactions based on MFRS pr

Right end vertical not exposed to

24" OC.

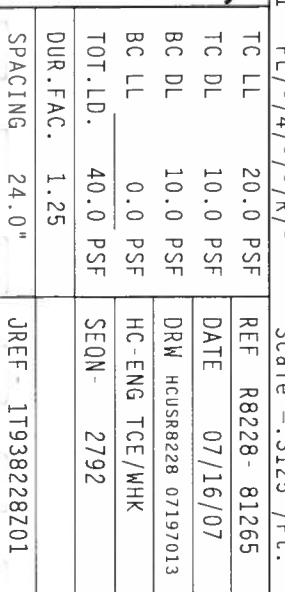
$$4X4 \equiv 2X3 \equiv 2X3 \equiv$$


Cq/RT=1.00(1.25)/10(0) 7.36.0424.12 QTY:



STAFF

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/FP1 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 81265
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197013
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEQN-	2792
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1T938228Z01

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

110 mph wind, 23.46 ft mean hgt, ASCE 7 02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, $I_w=1.00$ Gcp1(+/-)0.18

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

TC	(LUMBER DUR, FAC. = 1.25 / PLATE DUR, FAC. = 1.25
TC	From 68 PLF at 0.55 to 68 PLF at 7.35
TC	From 68 PLF at 7.35 to 68 PLF at 13.08
BC	From 4 PLF at 0.55 to 4 PLF at 13.08

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 20.00 psf non-concurrent live load

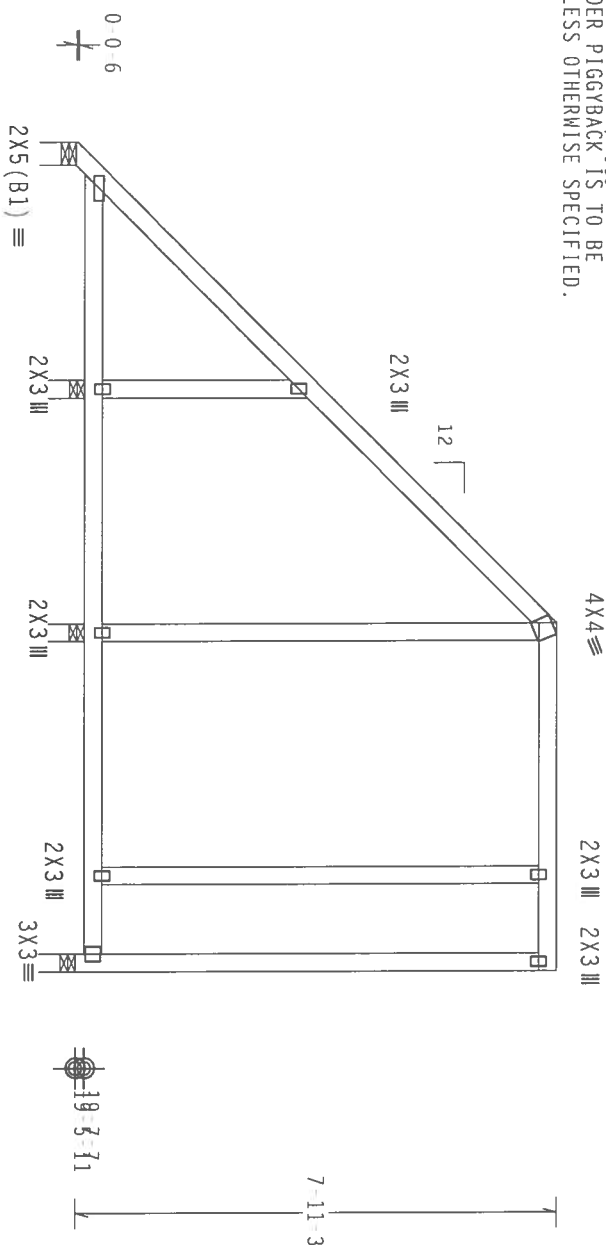


Diagram illustrating the layout of a 13-7-10 Over 4 Supports bridge. The spans are labeled as 3-6-0, 7-4-4, 4-0-0, and 5-8-12. The supports are labeled as R=146 U=41 W=4.596", R=264 U=246 W=3.5", R=398 U=199 W=3.5", and R=177 U=68 W=3.5".

PLT TYP. Wave

Design Critt: TPI -2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/10(0)$

7.36.0424 FL/-/4/-/-/R/ QTY:1

Scale = .3125"/Ft.

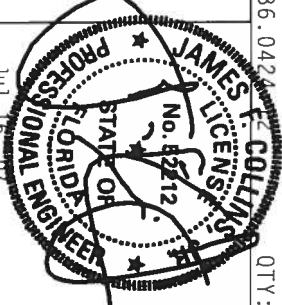
WARNING: *TRUCKS DRIVING EXISTING CAVE IN VARIATION*, HANDLING, SHIPPING, INSTALLING AND PRACTICING THE FOLLOWING INFORMATION IS NOT A SUBSTITUTE FOR THE DESIGN OF THE TRUSS COMPANY. IT IS THE RESPONSIBILITY OF THE USER TO OBTAIN ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AUTHORITIES. THE TRUSS COMPANY ASSUMES NO LIABILITY FOR ANY DAMAGE OR LOSS OF LIFE OR PROPERTY RESULTING FROM THE USE OF THIS INFORMATION.

REFLECT TO DESI (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATING INSTITUTE), 218 E. HARRIS LANE, SUITE 312, ALEXANDRIA, VA, 22314 AND WICK CONSULT OF AMERICA, 65000 N. CENTRAL EXPRESSWAY, SUITE 312, FORT WORTH, TX, 76116. THE TRUSS COMPANY ASSUMES NO LIABILITY FOR ANY DAMAGE OR LOSS OF LIFE OR PROPERTY RESULTING FROM THE USE OF THIS INFORMATION.

DRAWINGS INDICATED THAT CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
F1 Certificate of Authorization # 567



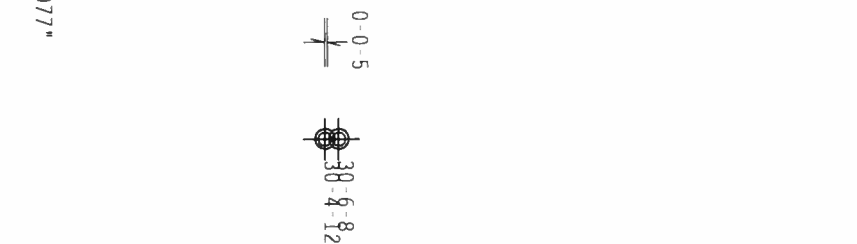
FL/-/4/-/-/R/-	Scale = .3125"/Ft.
TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"
JREF- 1T938228Z01	

[illegible]

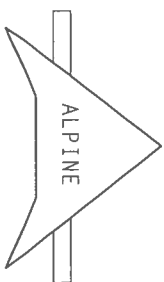
TC	F
BC	F

Wind reactions based on MAFRS pressures.
Bottom chord checked for 20.00 psf non-concurrent live load.

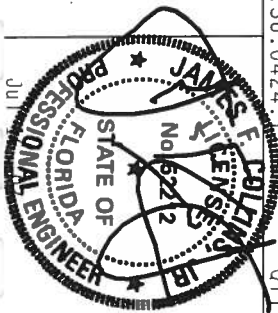
4x4 ≡



Scale = .5" / Ft.



DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE IRSS COMMENCEMENT DESIGN STUDY. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 81267
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197102
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEON-	2646
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1T938228Z01

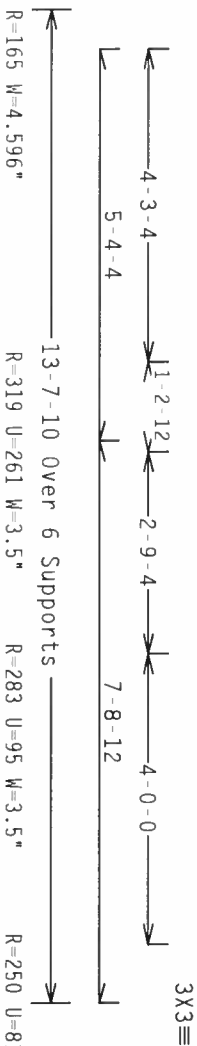
SPECIAL LOADS

(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC From 68 PLF at -0.55 to 68 PLF at 5.35

BC - From 4 PLF at -0.55 to 4 PLF at 13.08
Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

2X3 III



R=250 U=81 W=3.5^m

Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/10(0)$$

7.36.0424 18

QTY:1

FL/-/4/-/-/R/-/

Scale = .375"/Ft.

JAMES F. COLLINS JR.
LICENSED
No. 82512

TC LL	20.0 PSF	REF	R8228- 81268
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197103
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEQN-	1920
DUR.FAC.	1.25		
SPACING	24.0"	JREF -	1T938228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 21.46 ft mean hgt, ASCE 7-02, CLOSED bldg, not
located within 4.50 ft from roof edge, CAT II, EXP B, wind TC
DL=5.0 psf, wind BC DL=2.0 psf, $I_w=1.00$ $G_{CPI}(+/-)=0.18$

In lieu of structural panels or rigid ceiling use purlins to
brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

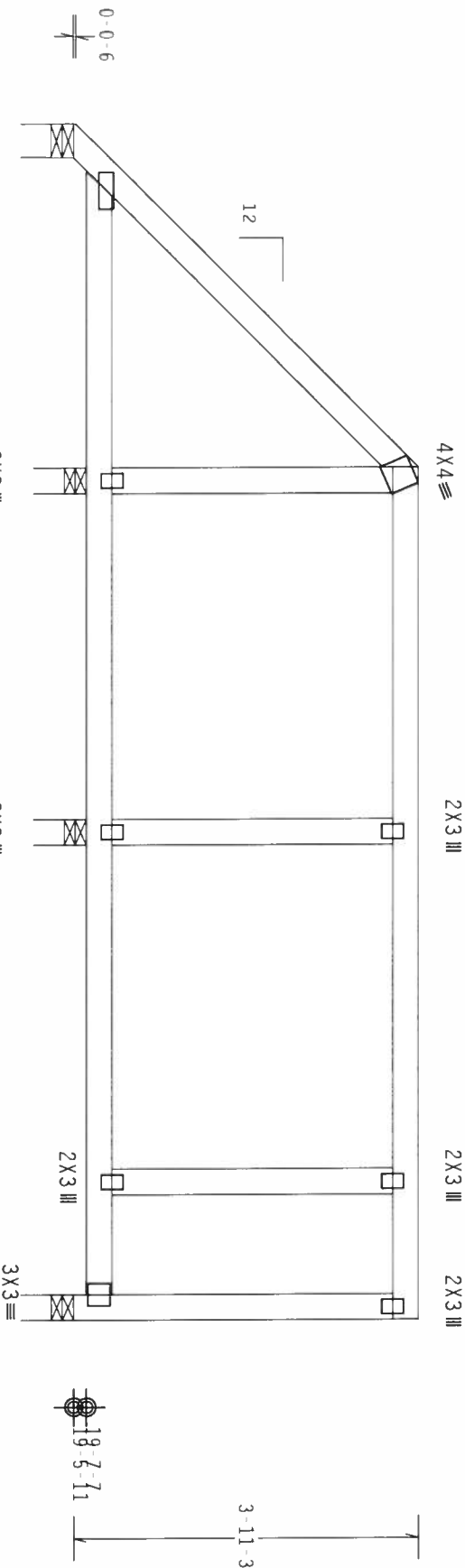
SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 68 PLF at -0.55 to 68 PLF at 3.35
TC - From 68 PLF at 3.35 to 68 PLF at 13.08
BC - From 4 PLF at -0.55 to 4 PLF at 13.08

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 20.00 psf non-concurrent live load.



2X5 (B1) ≡ 2X3 III 2X3 III 2X3 III 2X3 III 3X3 ≡

3-6-0 3-4-4 4-0-0 9-8-12

R=151 W=4.596" R=250 U=147 W=3.5" R=411 U=141 W=3.5" R=174 U=57 W=3.5"

13-7-10 Over 4 Supports

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.36.0424

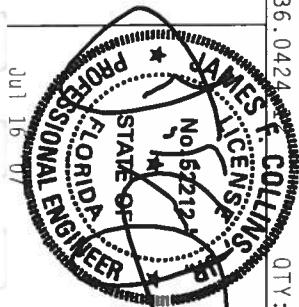
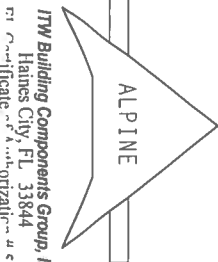
OTV:1

FL/-/4/-/R/-

Scale = .5"/ft.

WARNING TRUSSES REQUIRING EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210
NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300
ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT
BE RESPONSIBLE FOR ANY VIOLATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH
TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ITW BCG
CONNECTION PLATES ARE MADE OF 20/16/10/6 (K/1/5/5/7) ASH 4653 GRADE 40/60 (K/1/5/5) GALV. STEEL. APPLY
ANY INSPECTION OF ALL TRUSSES UNDER THESE SPECIFICATIONS. SPECIFICATIONS FOR PERMANENT DRAWINGS AND THIS
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT
BUILDING DESIGNER PER AISC/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 81269
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197104
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEQN-	1927
DUR.FAC.	1.25		
SPACING	24.0"	JREF -	1T938228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 20.46 ft mean hgt, ASCE 7 02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, $I_w=1.00$ $G C p_i(+/-)=0.18$

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

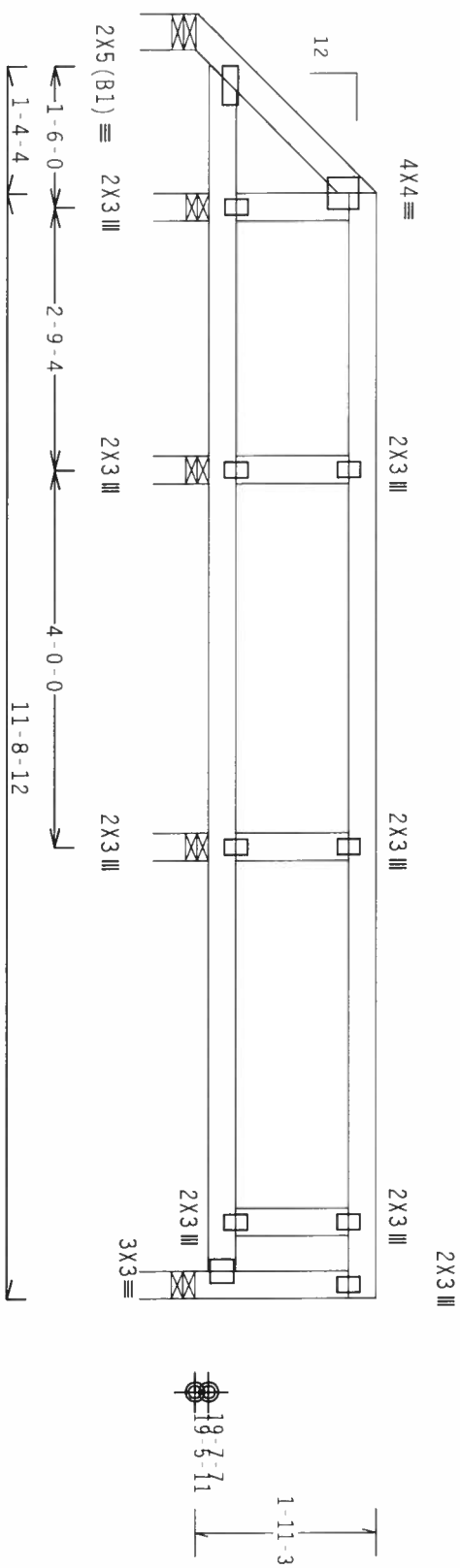
SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 68 PLF at -0.55 to 68 PLF at 1.35
TC - From 68 PLF at 1.35 to 68 PLF at 13.08
BC - From 4 PLF at -0.55 to 4 PLF at 13.08

Wind reactions based on MMFRS pressures.

Bottom chord checked for 20.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=90 W=4.596" R=140 U=63 W=3.5" R=242 U=74 W=3.5" R=364 U=108 W=3.5" R=150 U=44 W=3.5"

Note: All Plates Are 2X3 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

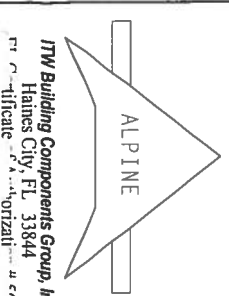
OTV:1 FL/-/4/-/-/R/-

Scale = .5"/ft.

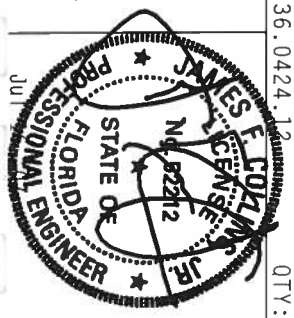
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD TRUSS COUNCIL OF AMERICA), 6900 ENTERPRISE LANE, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

CONNECTIONS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND AISC, THE BCG CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF ALL CONNECTIONS. THE BCG CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF ALL CONNECTIONS. THE BCG CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF ALL CONNECTIONS. THE BCG CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF ALL CONNECTIONS.



ITW Building Components Group, Inc.
Haines City, FL 33844
Tel: 888-444-4444
Fax: 888-444-4444



TC LL	20.0 PSF	REF	R8228 - 81270
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSRB228 07197105
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEQN	1940
DUR.FAC.	1.25		
SPACING	24.0"	JREF	1T938228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Weds 2x4 SP #3

110 mph wind, 21.32 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

Bottom chord checked for 20.00 psf non-concurrent live load.

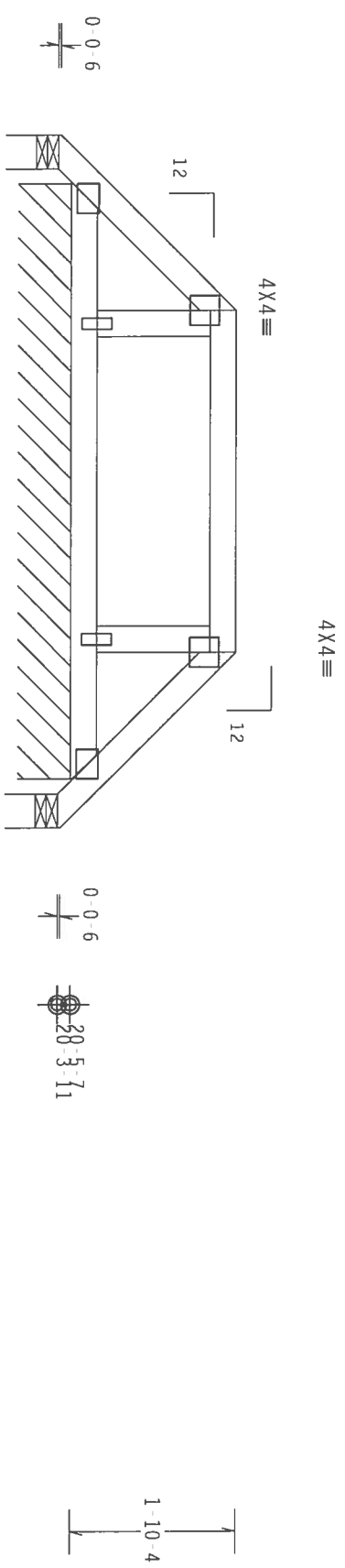
Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS

(LUMBER DUR.FAC.=1.15 / PLATE DUR.FAC.=1.15)
TC - From 68 PLF at -0.55 to 68 PLF at 5.25
TC - From 68 PLF at 5.25 to 68 PLF at 7.22
BC - From 4 PLF at -0.55 to 4 PLF at 7.22

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



3X4 (B1) ≡ 1.5X4 III 1.5X4 III 3X4 (B1) ≡
1-5-1 3-4-0 3-9-15 1-5-1
7-9-4 Over 3 Supports
R=26 U=180 W=4.596"
R=72 PLF U=27 PLF W=6-8-1
R=26 U=180 W=4.596"

PLT TYP. Wave

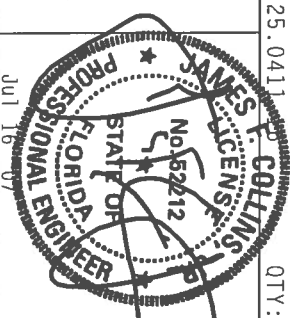
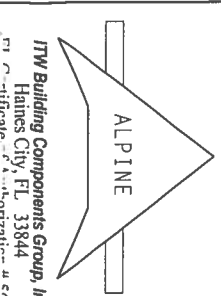
Design Crtt: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

QTY: 1 FL/-/4/-/-/R/-

Scale = .5"/ft.

WARNING TRUSSES REQUIRE EXTERIOR TAPE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD TRUSS COUNCIL OF AMERICA), 6800 ENTERPRISE LANE, MADISON, WI 53719. FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. THE BCG DESIGN CONFORMS WITH AISC 360-10 (AISC 360-10) AND AISC 360-10 (AISC 360-10) AND AISC 360-10 (AISC 360-10). ANY DEVIATION FROM THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE STABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 81271
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197106
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEON-	21605 REV
DUR.FAC.	1.15		
SPACING	24.0"	JREF-	1T938228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 22.67 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, $I_w=1.00$ $G_Cp(+/-)=0.18$

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

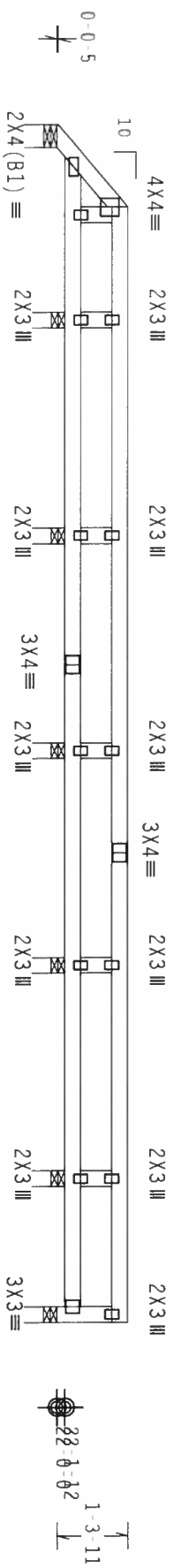
SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 66 PLF at -0.62 to 66 PLF at 0.91
TC - From 66 PLF at 0.91 to 66 PLF at 21.70
BC - From 4 PLF at -0.62 to 4 PLF at 21.70

Wind reactions based on MMFRS pressures.

Bottom chord checked for 20.00 psf non-concurrent live load.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



22-3-14 Over 7 Supports

R=118 U=30 W=5.077" R=276 U=102 W=3.5" R=282 U=99 W=3.5" R=278 U=97 W=3.5" R=287 U=99 W=3.5" R=251 U=87 W=3.5" R=61 U=22 W=3.5"

Note: All Plates Are 2x3 Except As Shown.

PLT TYP. Wave

Design Cmt: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

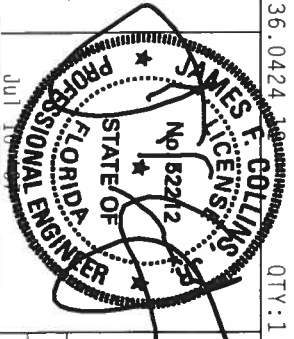
7.36.0424 10 OTY:1 FL/-/4/-/-/R/-

Scale = .3125"/Ft.

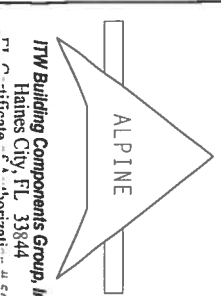
WARNING TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSTI (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE - 210 HORTON LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA - 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

THIS DESIGN IS THE PROPERTY OF BCG, INC. AND IS NOT TO BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, INCLUDING PHOTOCOPYING, RECORDING, OR BY ANY INFORMATION STORAGE AND RETRIEVAL SYSTEM. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.



TC LL	20.0 PSF	REF	R0228 - 81272
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR0228 07197107
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEQN-	1747
DUR.FAC.	1.25		
SPACING	24.0"	JREF -	1T938228201

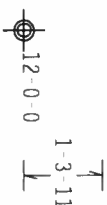


Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Weds 2x4 SP #3 :W1, W2, W9, W10 2x4 SP #2 Dense:

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.97 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi (+/-)=0.18



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

QTY:1

FL/-/4/-/-/R/-

Scale = .3125"/Ft.

WARNING: THESE PRODUCTS REQUIRE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING, AND PRACTICING. REFER TO EACH BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY IPI (TRUSS PRACTICE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 OR AMERICAN WOOD TRUSS COUNCIL OF AMERICA, 65000 14TH ENTERPRISE LANE, HUNTSVILLE, AL 35899 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNDESIGNED ORIENTATIONS INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT

TP1; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES

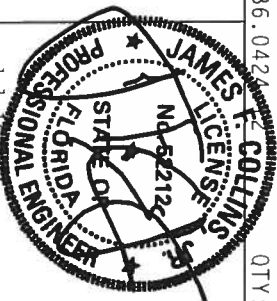
CONNECTOR PLATE SHALL BE 20/18/16GA (W, U, S, K) ASTM A653 GRADE 40/60 (W, K/U, S) GALV. STEEL. APPLY

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

—

ITW Building Components Group, Inc.
Haines City, FL 33844
Certificate of Incorporation 4567



TC LL	20.0 PSF	REF	R8228- 81273
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCU8R8228 07197010
BC LL	0.0 PSF	HC-ENG	JB/WHK
TOT. LD.	40.0 PSF	SEQN-	2014
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1T938228Z01

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
Webbs	2x4	SP	#3	

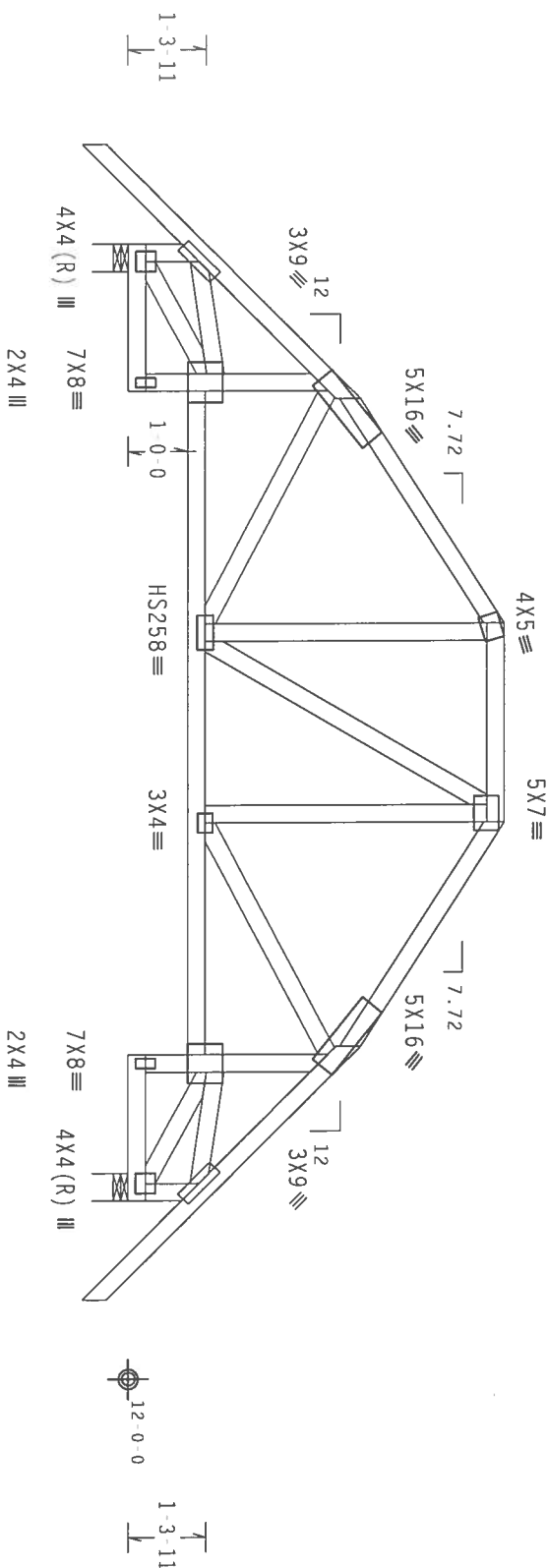
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, Wind BC DL=5.0 psf. $I_w=1.00$ GCpf(+/-)=0.18

Wind reactions based on M/FRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 20.00 psf non-concurrent live load.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



SPECIAL LOADS			
	(LUMBER	DUR.FAC.=1.25 /	PLATE DUR.FAC.=1.25)
TC	From	68 PLF at -1.67 to	68 PLF at 2.59
TC	From	64 PLF at 2.59 to	64 PLF at 6.34
TC	From	64 PLF at 6.34 to	64 PLF at 9.66
TC	From	68 PLF at 9.66 to	64 PLF at 13.41
TC	From	68 PLF at 13.41 to	68 PLF at 17.67
BC	From	6 PLF at -1.67 to	6 PLF at 2.00
BC	From	20 PLF at 0.00 to	20 PLF at 2.46
BC	From	20 PLF at 2.46 to	20 PLF at 13.54
BC	From	20 PLF at 13.54 to	20 PLF at 16.00
BC	From	6 PLF at 16.00 to	6 PLF at 17.67
PLT	1 LB Conc.	Load at (2.59,15,.84)	(13.41,15,.84)
PLT	93 LB Conc.	Load at (2.91,16,.05)	(13.09,16,.05)
PLT	145 LB Conc.	Load at (5.00,17,.40)	(11.00,17,.40)
PLT	115 LB Conc.	Load at (6.34,18,.27)	(9.66,18,.27)
PLT	157 LB Conc.	Load at (6.66,18,.27)	(8.00,18,.27)
PLB	35 LB Conc.	Load at (2.91,13,.04)	(13.41,13,.04)
PLB	75 LB Conc.	Load at (2.59,13,.04)	(13.09,13,.04)
PLB	86 LB Conc.	Load at (5.00,13,.04)	(11.00,13,.04)
PLB	225 LB Conc.	Load at (5.34,13,.04)	(9.66,13,.04)
PLB	96 LB Conc.	Load at (6.66,13,.04)	(8.00,13,.04)

PLT TYP. 20 Gauge HS, Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

7.36.0424

QTY:1

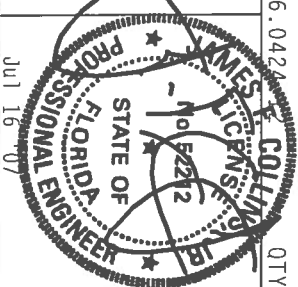
FL/-/4/-/-/R/-/

Scale = .3125"/Ft.

*****WARNING***** FRILES (FRIGID) EXPIRE CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO DC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY IPI (FIBERS PASTE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND TRUSS COMPANY OF AMERICA, 65000 INDUSTRIAL ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES AND PRIOR TO PERFORMING THESE FUNCTIONS. UNDESIRABLE DOWNSIDE FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

IMPORT/ANI FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH ITM OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

ITW Building Components Group, Inc.
Haines City, FL 33844



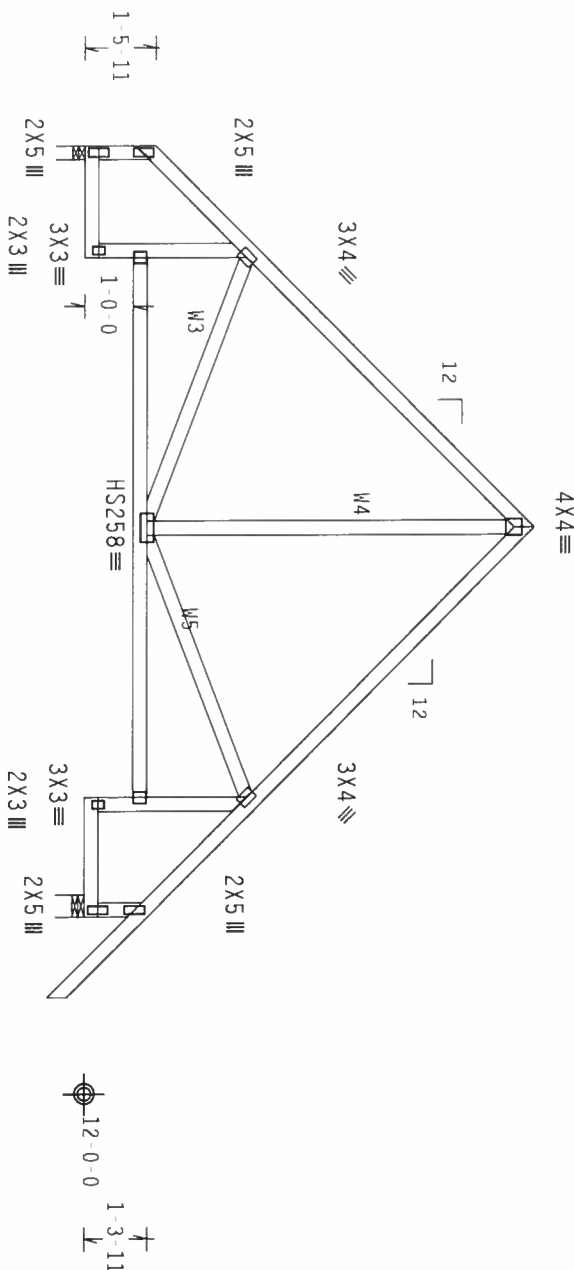
Jul 16 '07

TC LL	20.0 PSF	REF	R8228- 81274
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197032
BC LL	0.0 PSF	HC-ENG	JB/WHK
TOT.LD.	40.0 PSF	SEQN-	2049
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1T938228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #2 Dense :W3, W4, W5 2x4 SP #3:

Bottom chord checked for 20.00 psf non-concurrent live load.

110 mph wind; 16.47 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.18



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

QTY:2 FL/-/4/-/-/R/-

Scale = .25"/Ft.

WARNING THESE BUILDING COMPONENTS CAUSE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO GC51 (BUILDING CONTRACT SPECIFIC INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WPCA (WOOD PRESERVATION COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MANASSAS, VA, 20109) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED FIELD CEILING.

JAMES F. COLLINS
LICENSE
No. B212
JP

TC LL	20.0 PSF
TC DL	10.0 PSF

REF	R8228 - 81275
DATE	07/16/07

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
F1 Certificate of Authorization # 5627

[illegible]

QTY 86, 0424

JAMES F. COLLINS
No. E22124
FLORIDA
STATE OF
PROFESSIONAL ENGINEER

Jul 10 07

BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

HC-ENG	JB/WHK
SEQN-	2011
JREF-	1T938228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 : W1 2x8 SP #2:

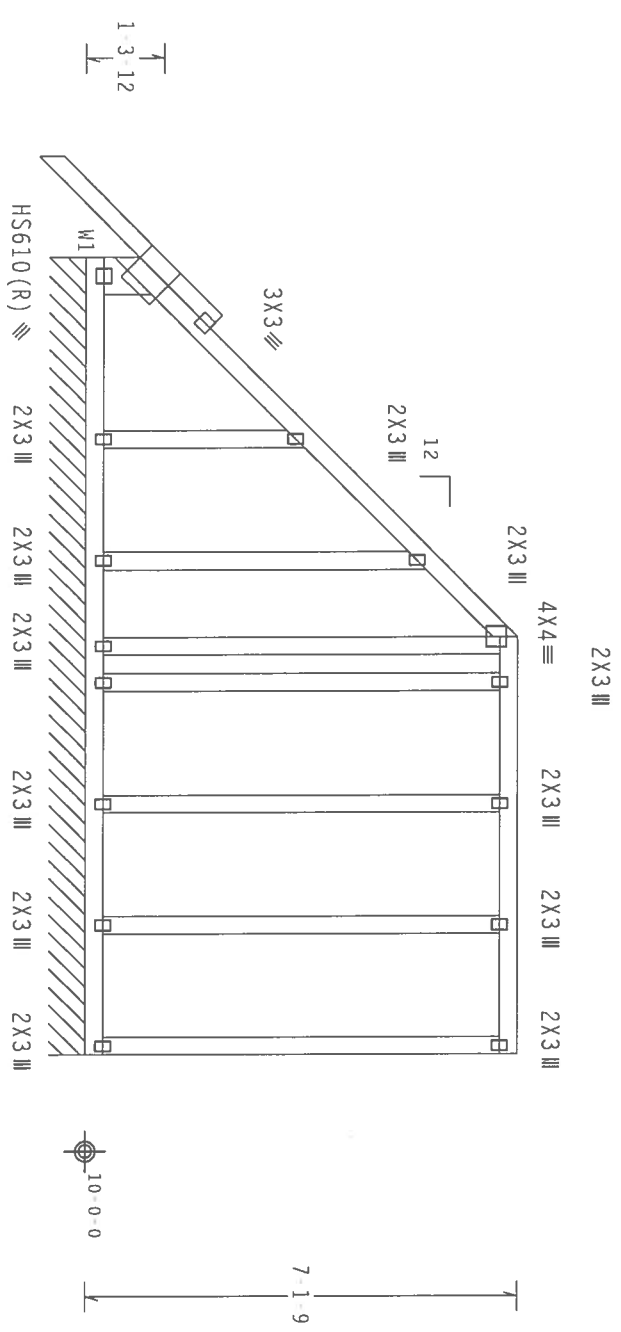
SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 110 PLF at -1.67 to 110 PLF at 0.96
TC - From 110 PLF at 0.96 to 110 PLF at 6.23
TC - From 110 PLF at 6.23 to 110 PLF at 13.13
BC - From 6 PLF at -1.67 to 6 PLF at 0.00
BC - From 20 PLF at 0.00 to 20 PLF at 13.13

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

See DWGS A1101SEC0207 & 6BLLETIN0207 for more requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, IW=1.00 gcpl (+/-)=0.18
Wind reactions based on MMFRS pressures.
Right end vertical not exposed to wind pressure.
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Bottom chord checked for 20.00 psf non-concurrent live load.
Fasten rated sheathing to one face of this frame.



13'-1-8 Over Continuous Support

PLT TYP. 20 Gauge HS,Wave

Design Cmt: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.36.0424

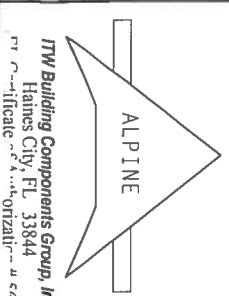
QTY: 1 FL/-/4/-/ -/R/-

Scale = .3125"/Ft.

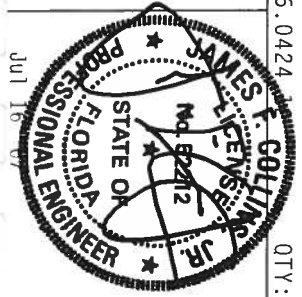
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304, AND WCA (WOOD TRUSS COUNCIL OF AMERICA), 6800 ENTERPRISE LANE, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF 2002 (OPTIONAL DESIGN SPEC., BY AREA) AND TPI. ITW BCG CONDUCTS FACTORY INSPECTIONS OF 20/10/10/10 (W/15/5/5) ASH K653 GRADE 40/60 (K, R/H/55) GALV. STEEL. APPLY TO EACH FACTORY INSPECTION. THE TRUSS SHALL BE DELIVERED AS OF 12/15/07. SECTION PER DRAWING 10/15/07. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE DELIVERED AS OF 12/15/07. SECTION PER DRAWING 10/15/07. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group, Inc.
Haines City, FL 33844
Tel: 888-444-4444



TC LL	20.0 PSF	REF	R8228 - 81276
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197016
BC LL	0.0 PSF	HC-ENG	JB/WHK
TOT.LD.	40.0 PSF	SEQN-	2327
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1T938228Z01

Top chord 2x4 SP #2 Dense :T2 2x6 SP #1 Dense:

:T3 2x6 SP #2: SP SS :B1 2x8 SP #2:

:B3 2x4 SP #2 Dense:

Weds 2x4 SP #3

Calculated horizontal deflection is 0.25" due to live load and 0.46" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Collar tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 Gcpl(+/-)=0.18

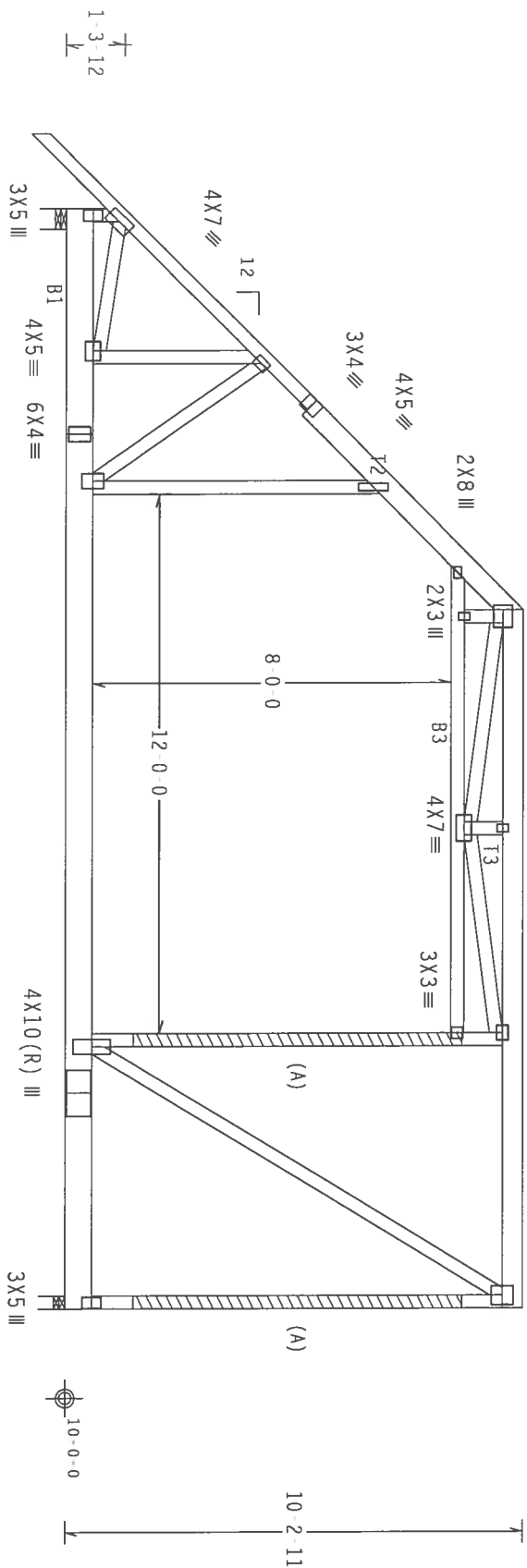
Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d box or gun (0.128"x3" min.) nails @ 6" OC.

Bottom chord checked for 20.00 psf non-concurrent live load.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-4-0 to 18-4-0.



PLT TYP. 18 Gauge HS.Wave

Design Crit: TP1-2002 (STD) /FBC

Cq/RT=1.00(1.25)/10(0)

7.36.0424

QTY:1

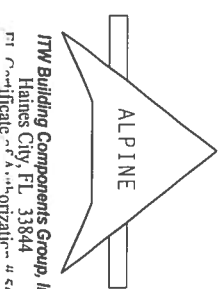
FL/-/4/-/-/R/-

Scale = .25" /Ft.

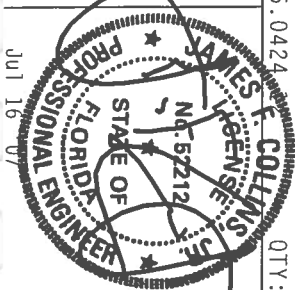
****WARNING**** TRUSSES REQUIRE EXTERIOR CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE COMPONENT SAFETY INFORMATION. PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WEA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WEA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 (SEC. 3). DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SIGN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TP11 SEC. 2.



ITW Building Components Group, Inc.
Haines City, FL 33844
Tel: 888.444.4444



TC LL	20.0 PSF	REF R8228-81277
TC DL	10.0 PSF	DATE 07/16/07
BC DL	10.0 PSF	DRW HCUSR8228 07197041
BC LL	0.0 PSF	HC-ENG JB/WHK
TOT. LD.	40.0 PSF	SEQN- 1726
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1T938228Z01

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #1 Dense:

:T3 2x6 SP #2:

Bot chord 2x8 SP #2 :B3 2x4 SP #2 Dense:

Webs 2x4 SP #3

Calculated horizontal deflection is 0.11" due to live load and 0.20" due to dead load.

Bottom chord checked for 20.00 psf non-concurrent live load.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-4-0 to 18-4-0.

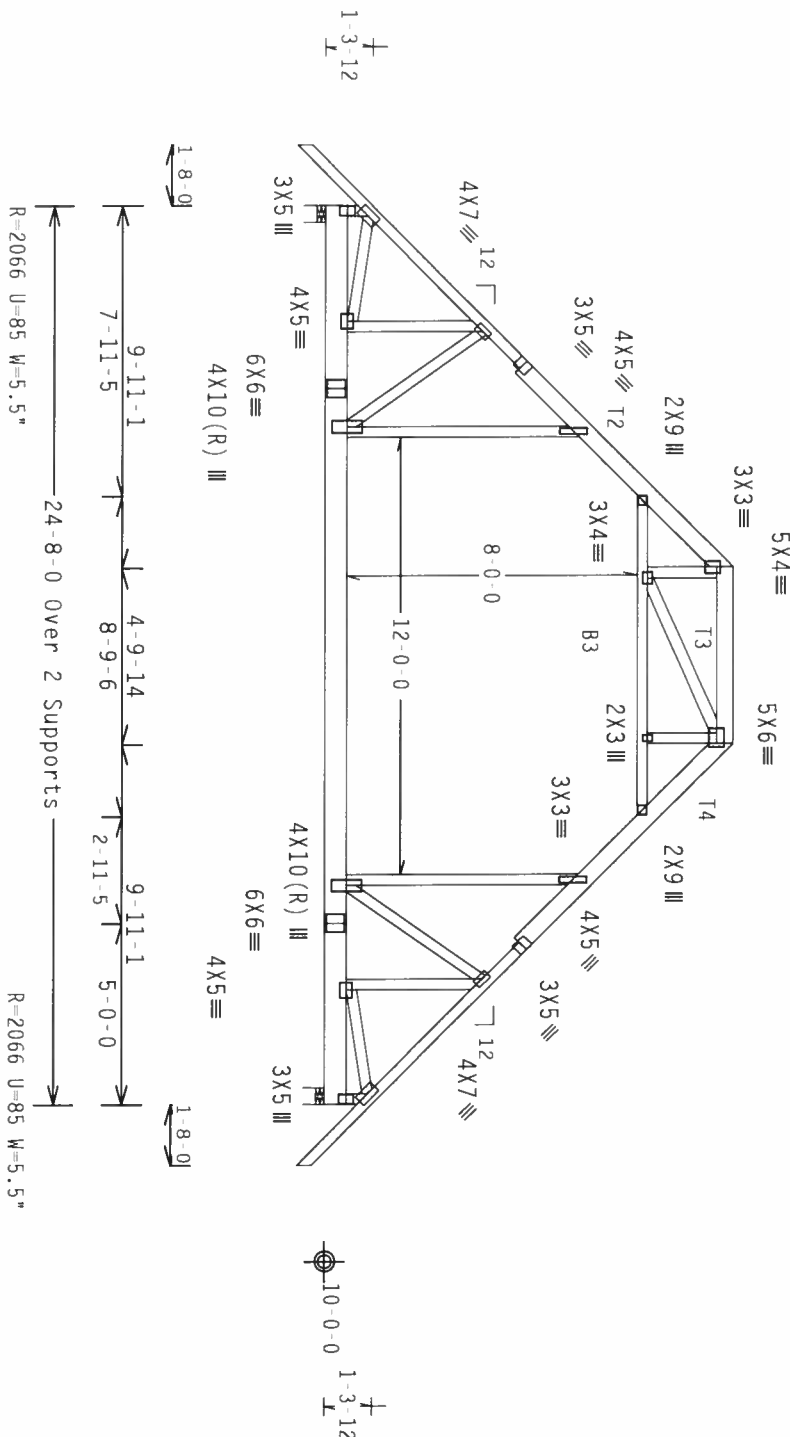
110 mph wind, 15.44 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 gcpi (+/-)-0.18

Wind reactions based on MMFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

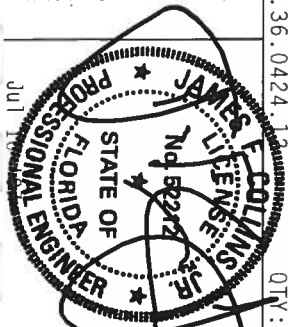
QTY: 7 FL/-/4/-/-/R/-

Scale = .1875"/Ft.

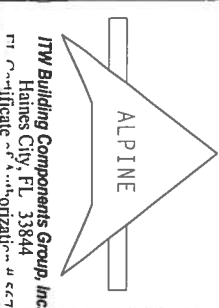
WARNING TRUSSES RIGIDLY EXTERIOR CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. THE BCG DESIGNER CERTIFIES THAT THE TRUSS HAS BEEN DESIGNED TO MEET THE DESIGN CONDITIONS AND THE TRUSS SHALL BE CONSIDERED TO BE A PROPERLY ATTACHED RIGID CEILING. ANY INSPECTION OF TRUSSES AND/OR JOINTS SHALL BE PERFORMED BY A QUALIFIED PERSONNEL PER AISC 308.1.5. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISC/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 81278
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197059
BC LL	0.0 PSF	HC-ENG	JB/WHK
TOT.LD.	40.0 PSF	SEQN-	1700
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1T938228Z01



[illegible]

110 mph wind, 15.44 ft mean hgt, ASCE 7-02, CLOSED bldg, not

DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.18

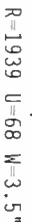
[illegible]

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Collar-tie braced with continuous lateral bracing at 24" OC. or

Deflection meets L/240 live and L/180 total load. Creep increase



Scale = .1875"/Ft.

36.0424

QTY:

JAMES F COLLINS
LICENSE
No. F2212

ALPINE

ITTW Building Components Group,

224
QTY :
JUL 16 07
JAMES F. COLLINS
PROFESSIONAL ENGINEER
FLORIDA
No. F2212
STATE OF FLORIDA
EXPIRATION DATE 07/16/2011

TC LL	20.0 PSF	REF	R8228- 81279
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCSR8228 07197065
BC LL	0.0 PSF	HC-ENG	JB/WHK
TOT.LD.	40.0 PSF	SEQN-	1715
DUR.FAC.	1.25		
SPACING	24.0"	JREF	- 1T938228Z01

Top chord	2x4	SP	#2	Dense
Bot chord	2x6	SP	#2	
Webbs	2x4	SP	#3	

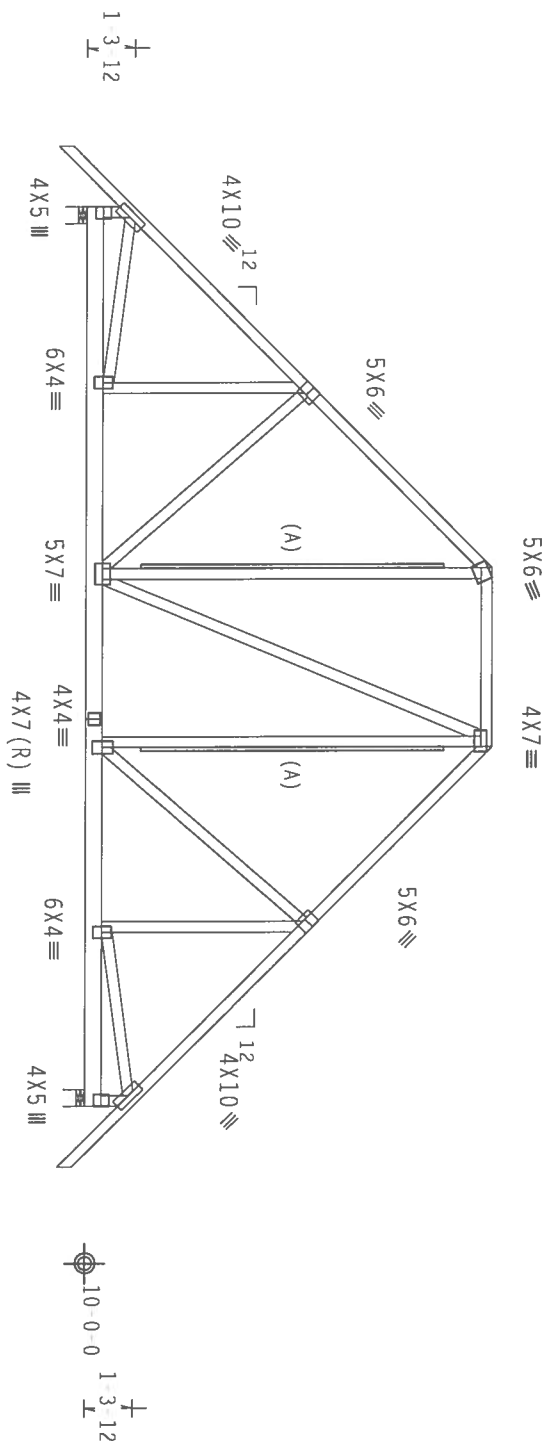
Wind reactions based on MWFRS pressures.

Bottom chord checked for 20.00 psf non concurrent live load.

---(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/10(0)$

7.36.0424

QTY:1 FL/-/4/-/-/R/-

Scale = .1875"/Ft.

WARNING: THESE STRUCTURAL EXISTENCE, CARE IN FABRICATION, HANDLING, UNLOADING, INSTALLING AND BRACING REQUIREMENTS ARE BASED ON THE ASSUMPTIONS OF THE FOLLOWING INFORMATION: PUBLISHED BY THE TRUSS PRACTICE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICKIWOOD TRUSS COMPANY, 63000 ENTERPRISE LANE, ANDOVER, MA 01810 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

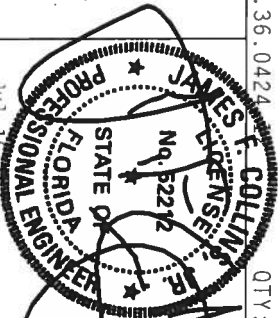
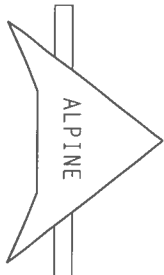
BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH IPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

CONNECTION PLATES ARE MADE OF 20/18/1664 (H, W/55/K) ASTM A653 GRADE 40/60 (H, K/H, 55) GALV. STEEL. PLATES TO EACH FAULT OF BRISSES AND UNLESS OTHERWISE NOTICED ON THIS DESIGN, POSITION OF PLATES SHALL BE AS SHOWN IN THE DRAWINGS.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANEXA A3 OF TP11 2002 SEC.3.

DESIGN SHOW. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ITW Building Components Group, Inc.
Haines City, FL 33844
F1 Certificate of Authorization # 667



TC LL	20.0 PSF	REF	R8228 - 81280
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCSR8228 07197091
BC LL	0.0 PSF	HC-ENG	JB/WHK
TOT. LD.	40.0 PSF	SEON -	1684
DUR. FAC.	1.25		
SPACING	24.0 "	JREF -	1T938228Z01

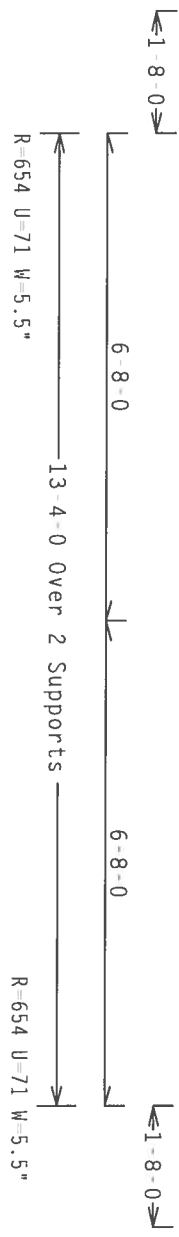
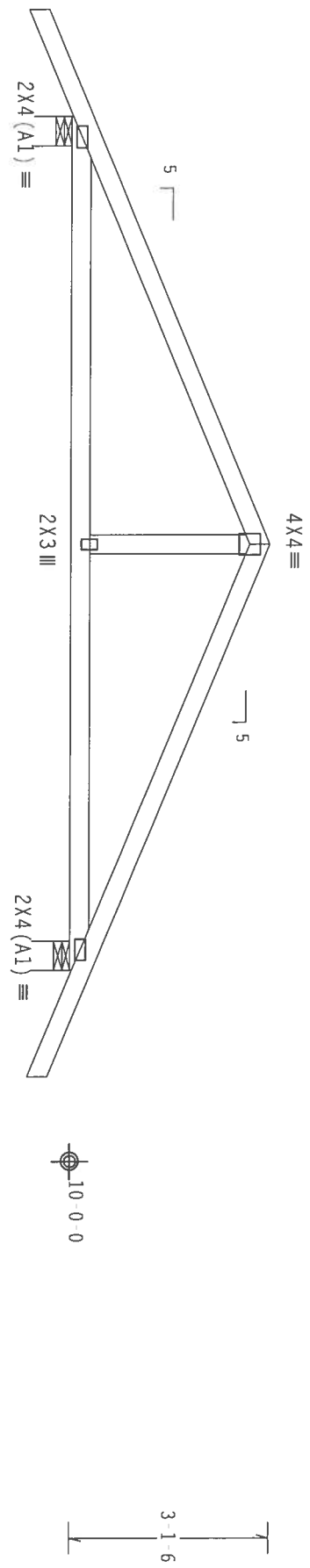
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Weds 2x4 SP #3

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI(+/-) 0.18

Bottom chord checked for 20.00 psf non concurrent live load.

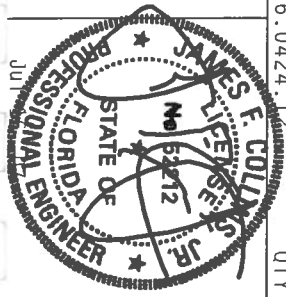


PLT TYP. Wave Design Crit: TP1-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 7.36.0424.12 QTY:1 FL/-/4/-/-/R/- Scale = .375"/Ft.

WARNING INSTRUCTIONS REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
The suitability and use of this component for any building is the responsibility of the building designer per ANSI/PTI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 81281
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCSR8228 07197018
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT. LD.	40.0 PSF	SEQN-	2888
DUR. FAC.	1.25		
SPACING	24.0"	JREF -	1T938228Z01

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCPI (+/-) 0.18

Wind reactions based on MFRS pressures.

Stacked top chord must NOT be notched or cut in area (NNL).

interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



13-4-0 Over Continuous Support

Design Crit: TPI-2002(STD)/FBC

 $Cq/RT=1.00(1.25)/10(0)$

7.36.0424 1 QTY:1

QTY:1

FL/-/4/-/-/R/-

Scale = .375"/Ft.

JAMES F. COLLINS
LICENSE
No. 63275
JR.

STATE OF
MISSISSIPPI
JAMES F. COLLINS
J.R.
no 62212
LICENSE

Professional Engineer Seal for the State of Florida, License No. 52212, signed by J.R. Williams, P.E.

TC LL	20.0 PSF	REF	R8228- 81282
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197014
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEQN-	2898
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1T938228Z01

JREF - 1T938228Z01

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

Bottom chord checked for 20.00 psf non-concurrent live load.



Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)$

 $Cq/RT=1.00(1.25)/10(0)$

7.36.0424 QTY:

QTY:2 FL/-/4/-/-/R/-

Scale = .5"/Ft.

WARNING FRAMES INCLUDING EXISTING GABLE END FAMILICATION, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO NC31 (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICK HOOD TRUSS COUNCIL OF AMERICA, 63000 CREEPER LAKE, MADISON, WI 53719 FOR SAFETY PRACTICES AND TIPS TO PREVENTING THESE CONDITIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT**

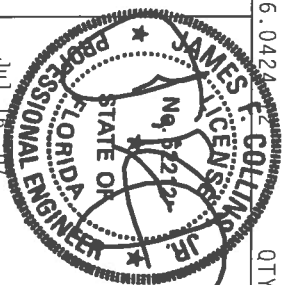
TP1; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

CONNECTOR PLATES ARE MADE OF 20/18/18mm (H, H/55/K) WITH A653 GRADE 40/60 (H, K/H.55) GALV. STEEL. APPLY PLATES TO EACH FACE OF JOINTS AND BOLTS ARE LOCATED ON THE DESIGN POSITION FOR BOLTING (SEE 2

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ITW Building Components Group, Inc.
Haines City, FL 33844
Certificate of Incorporation



FL/-/4/-/-/R/-		Scale =.5"/ft.	
TC LL	20.0 PSF	REF	R8228- 81283
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197015
BC LL	0.0 PSF	HC-ENG TCE/WHK	*
TOT.LD.	40.0 PSF	SEQN-	2936
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1T938228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Stack chord SC1 2x4 SP #2 Dense:
Stack chord SC2 2x4 SP #2 Dense:

See DWGS A11030EE0207 & GBLLETIN0207 for more requirements.

In lieu of structural panels use purlins to brace TC @ 24" OC.

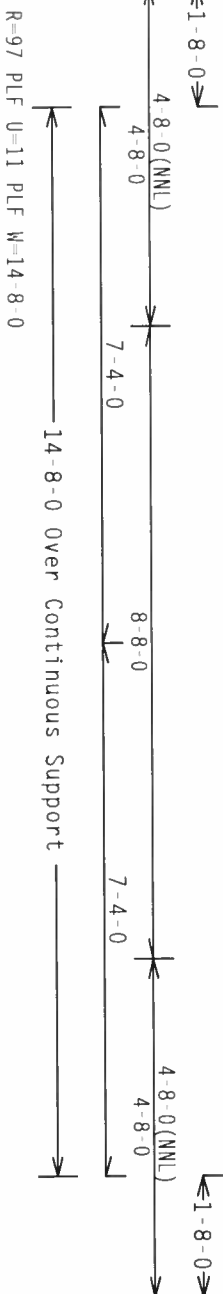
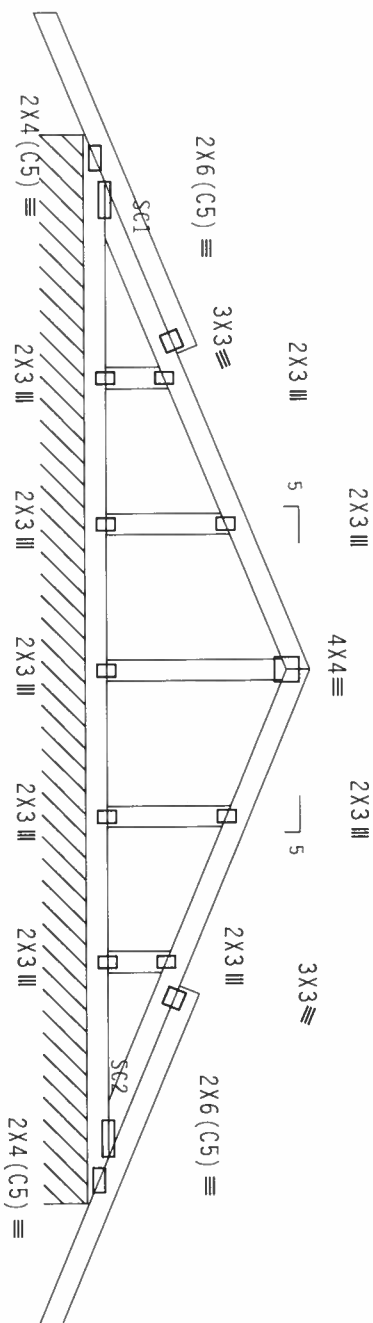
Bottom chord checked for 20.00 psf non concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 23.08 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-) = -0.18

Wind reactions based on MMFRS pressures.

Stacked top chord must NOT be notched or cut in area (NML).
Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.



Note: All Plates Are 2x3 Except As Shown.

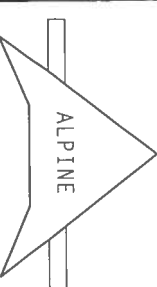
PLT TYP. Wave

Design Cr't: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

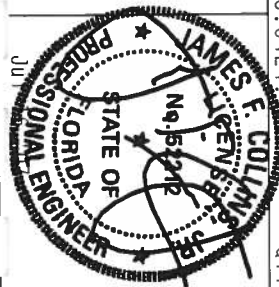
QTY: 1 FL/-/4/-/-/R/-

Scale = .375"/ft.

WARNING THIS IS A PRELIMINARY DESIGN. THE TRUSS IS TO BE CONSTRUCTED WITH TYPICAL OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BOULDER COUNTY STRUCTURAL INSTITUTE) PUBLISHED BY TPI (TRUSS PATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS COMPANY, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL DIMENSIONS SHALL BE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY AT ACHD RIGID CEILING.



TW Building Components Group, Inc.
Haines City, FL 33844
Tel: 888-444-4444



TC LL	20.0 PSF	REF	R8228 - 81285
TC DL	10.0 PSF	DATE	07/16/07
BC DL	10.0 PSF	DRW	HCUSR8228 07197017
BC LL	0.0 PSF	HC-ENG	TCE/WHK
TOT.LD.	40.0 PSF	SEON-	2942
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1T938228201

PIGGYBACK DETAIL

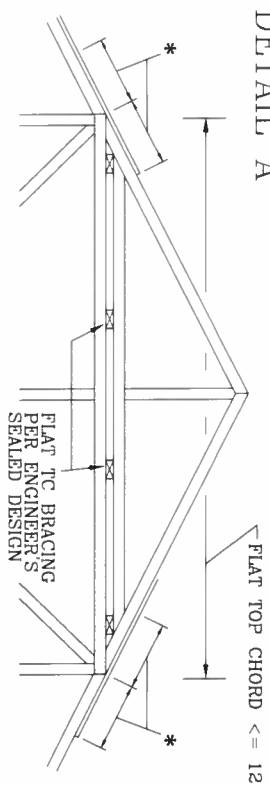
100 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

80 MPH WIND, 30.00 FT MEAN HGT, SBC, ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

100 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

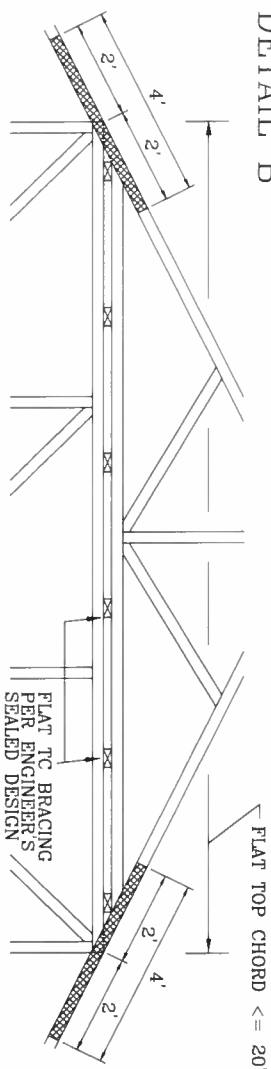
NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATELY BRACED BY SHEATHING OR PURLINS. PROVIDE DIAGONAL BRACING OR OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS.

DETAIL A



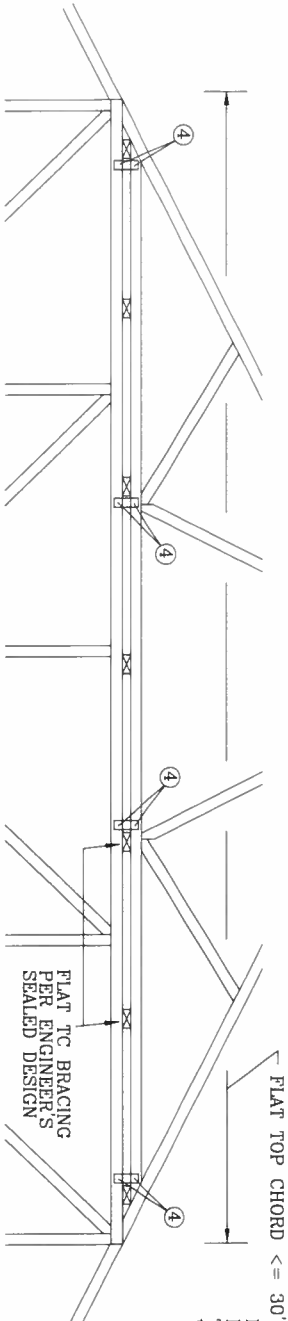
PIGGYBACK CAP TRUSS TOENAILLED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"x3") NAILS.
* 12" MIN RIGID SHEATHING OVERLAP WITH 8d COMMON (0.131"x2.5") OR GUN NAILS IN OVERLAP ZONE SPACED AT 4" O.C.

DETAIL B



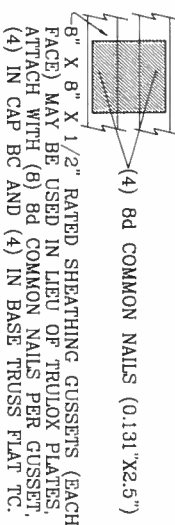
PIGGYBACK CAP TRUSS TOENAILLED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"x3") NAILS AND SECURED WITH 2x4 #3 GRADE SCAB (1 SIDE ONLY) ATTACHED WITH 10d COMMON NAILS AT 4" O.C.

DETAIL C



CAP TRUSS TOENAILLED TO TOP CHORD BRACING AND SECURED WITH 3X8 TRULOX PLATES (EACH FACE) AT EACH END AND AT 1/3 POINTS. CIRCLED NUMBER INDICATES REQUIRED NUMBER OF 0.120" X 1.375" NAILS PER FACE. SEE DRAWING 160TL FOR TRULOX INFORMATION.

IN LIEU OF TRULOX CONNECTORS, ALPINE 62PB SPECIAL PIGGYBACK CONNECTORS MAY BE USED. SHOP APPLY TOOTHED PORTION, FIELD ATTACH TO MATING TRUSS WITH (4) 0.120" X 0.375" NAILS MINIMUM EACH FACE.

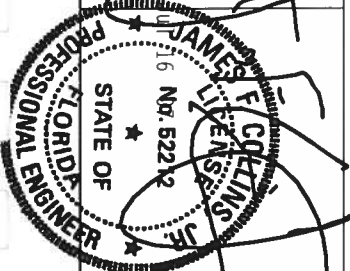


THIS DRAWING REPLACES DRAWINGS 581.670 & 961.860



ITV BUILDING COMPONENTS GROUP, INC.
FOURFAND BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 288 N. DEER ST., SUITE 312, ALEXANDRIA, VA 22304 AND A CROWN TRUSS COMPANY, 1000 W. 10TH ST., SUITE 100, FORT WORTH, TX 76102. SAFETY INFORMATION IS PROVIDED FOR INFORMATION ONLY. IT IS THE USER'S RESPONSIBILITY TO OBTAIN THE LATEST VERSIONS OF THE TRUSS PLATE INSTITUTE'S SAFETY INFORMATION. TRUSS PLATE INSTITUTE'S SAFETY INFORMATION IS PROVIDED FOR INFORMATION ONLY. IT IS THE USER'S RESPONSIBILITY TO OBTAIN THE LATEST VERSIONS OF THE TRUSS PLATE INSTITUTE'S SAFETY INFORMATION. TRUSS PLATE INSTITUTE'S SAFETY INFORMATION IS PROVIDED FOR INFORMATION ONLY. IT IS THE USER'S RESPONSIBILITY TO OBTAIN THE LATEST VERSIONS OF THE TRUSS PLATE INSTITUTE'S SAFETY INFORMATION.



TC LL	PSF	REF	PIGGYBACK
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	PIGGYBACKA0207
BC LL	PSF	-ENG	DLJ/KAR
TOT. LD.	MAX 60 PSF		
DUR. FAC.	1.15		
SPACING	24.0"		

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

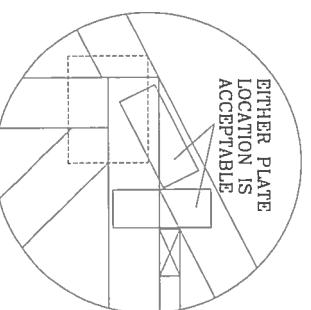
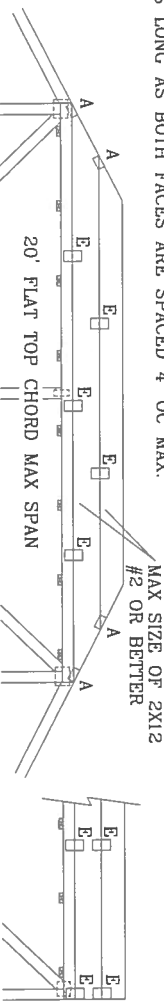
ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

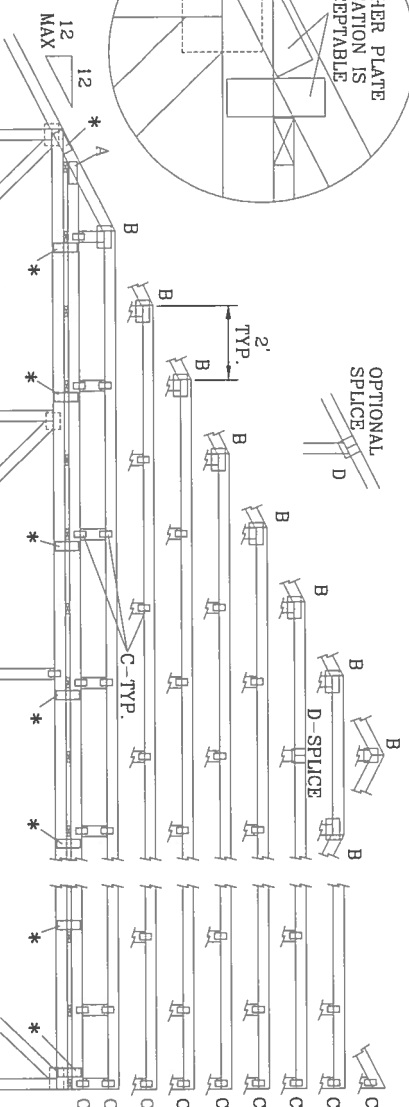
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

- 130 MPH WIND, 30' MEAN HGT, ASCE 7-98, ASCE 7-02 OR ASCE 7-05, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF
- 110 MPH WIND, 30' MEAN HGT, SRC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

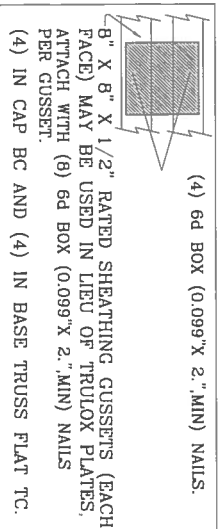
FRONT FACE (E*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.



EITHER PLATE LOCATION IS ACCEPTABLE



*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.



(4) 6d BOX (0.099" X 2" MIN) NAILS.
(4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

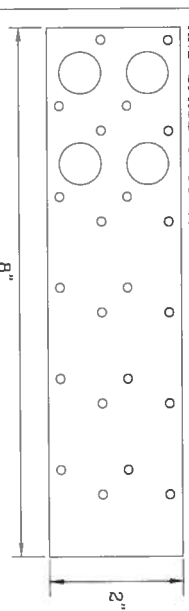
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (B) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB LENGTH	WEB BRACING CHART
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113" X 2.5" MIN) NAILS AT 4' OC.
10' TO 14'	2x4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135" X 3.5" MIN) NAILS AT 4' OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



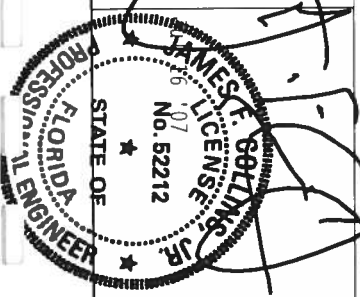
THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 847.045

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMEROY BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 210 NORTH LEE STR, SUITE 312, ALEXANDRIA, VA 22304 AND VTCR (WOOD TRUSS CONDUCTOR) AMERICA, 6900 ENTERPRISE LN, MADISON, WI 53719 FOR SAFETY INFORMATION. SAFETY INFORMATION FOR THESE FUNCTIONS, UNLESS OTHERWISE INDICATED, SHALL BE THE RESPONSIBILITY OF THE USER. THESE FUNCTIONS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES PER DESIGN CONDUCTED WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES PER TPI, BCG CONNECTOR PLATES TO EACH FACE OF TRUSSES AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1606-2, ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



MAX LOADING	REF	PIGGYBACK
55 PSF AT	DATE	2/23/07
1.33 DUR. FAC.	DRWG	PIGGYBACK0207
50 PSF AT	-ENG	DLJ/KAR
1.25 DUR. FAC.		
47 PSF AT		
1.15 DUR. FAC.		
SPACING		24.0"

MAX GABLE VERTICAL LENGTH														
CABLE VERTICAL SPACING	2x4 SPECIES	BRACE	NO BRACES	(1) 1x4 "L" BRACE •		(1) 2x4 "L" BRACE •		(2) 2x4 "L" BRACE **		(1) 2x6 "L" BRACE •		(2) 2x6 "L" BRACE •		
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B			
24" O.C.	SPF	#1 / #2	#1	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
			#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		Hf	STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
			STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"
			#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
	DfL	#2	#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
			STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"	
			#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#1	STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
			STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		Hf	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	DfL	STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"	
		#1 / #2	4' 11"	8' 5"	8' 5"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	
		STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	
SP	STANDARD	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	
	STUD	#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
		#1	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	

CABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS $L/240$.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER-
CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0"

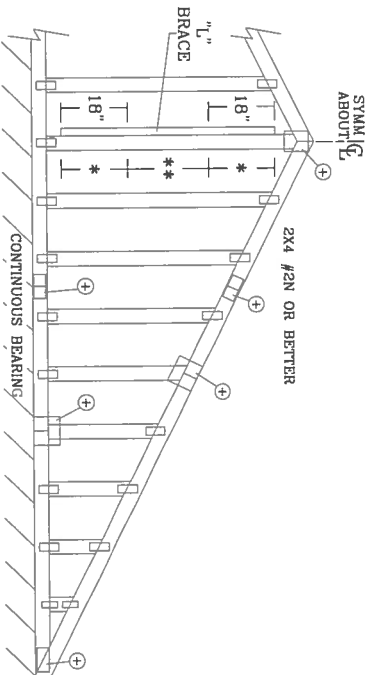
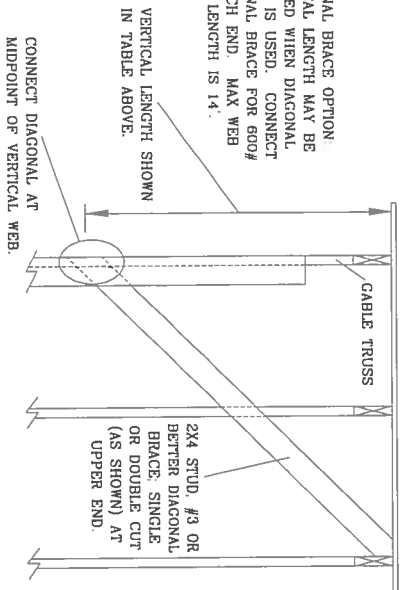
PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES
+ FOR (2) L DIMENS. SPACE MAILED AT 3 O.C.

"1." BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.



REFER TO CHART ABOVE FOR MAX CABLE VERTICAL LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1x4 OR 2x3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2x4
GREATER THAN 11' 6"	2.5x4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

UNREPAIRABLE. TRUSSES REQUIRING EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI DRAWING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 212, ALEXANDRIA, VA 22304 AND VITA QVADRO TRUSS COUNCIL, INC., 6300 ENTERPRISE LN., MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING JOIST FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

UNREPAIRABLE. FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. THIS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NSI NATIONAL DESIGN SPEC. BY AREA AND THE FOLLOWING: 1. TRUSS CONNECTOR PLATES ARE MADE OF 2018/18624 (A/HS/SS) ASTM A653 GRADE 50 (A/HS/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF INSPECTION OF PLATES FOLLOWED BY (D) SHALL BE PER 2018/18624 (A/HS/SS) GALV. STEEL. 2. THE DESIGN OF PLATES FOLLOWED BY (D) SHALL BE PER 2018/18624 (A/HS/SS) GALV. STEEL. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL DESIGNER'S RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER NSI/TP1 1 SEC. 2.

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-02-CAB1015

DATE 2/23/07

DRWG A11015EE0207

—ENG

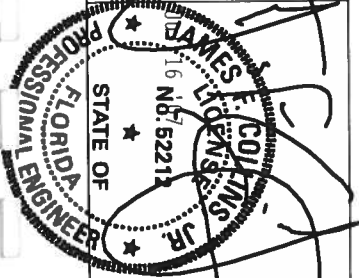


Diagram illustrating connections for uplift on a cable-stayed bridge. The diagram shows a central cable and stay cables connecting to the deck. Connections are marked with circles containing a plus sign (+). A dimension line indicates the cable vertical length (TYP.).

Labels in the diagram include:

- SYM. ABOUT
- CABLE VERTICAL LENGTH TYP.

Table for connections:

+	REFER TO ENGINEER
+	SPLICE, WEB AND
+	* IF CABLE VERTICAL
+	SINGLE PLATE TO

EXAMPLE:

LESS THAN 4' 0"

GREATER THAN 4'

LESS THAN 11' 6"

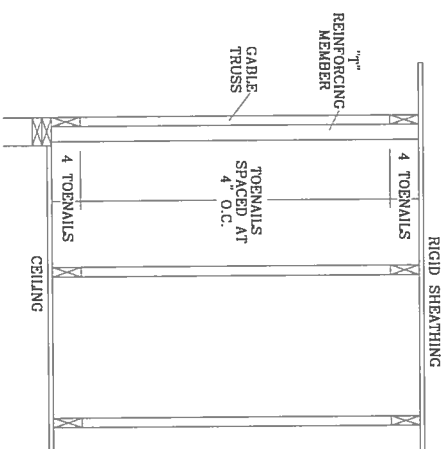
GREATER THAN 11'

PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON 1

VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X6
BETWEEN 4' 0" BUT LESS THAN 11' 6"	2X4	2X6
GREATER THAN 11' 6"	2.5X4	2.5X6

2X4 2X4 2X8

SEE APPROPRIATE ALPINE CABLE DETAIL (ASCE OR SBCS)
WIND LOAD) FOR MAXIMUM UNREINFORCED CABLE
VERTICAL LENGTH.



ALPINE

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POMPANO BEACH, FLORIDA

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IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. TPI, BCO, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR APPLICATION, HANDLING, SHIPPING, INSTALLING, OR BRACING OF TRUSSES. THIS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC.) BY NDS-600 (WOODS DESIGN), BEG CONNECTOR PLATES ARE MADE OF 2010-ALUMINUM AND SHALL BE IDENTIFIED ON THIS TRUSS. POSITION PER DRAWINGS 160A-2. AN INSPECTION OF PLATES FOLLOWED BY CD SHALL BE PER AXES A3 OF TPI-1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER ANSI/TPI-1 SEC. 2.

~~THIS DRAWING REPLACES~~ DRAWINGS GAB98117 876,719 & HC26294035

TO CONVERT FROM "L" TO "I" REINFORCING MEMBERS, MULTIPLY "I" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE CABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "I" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE $W / "T"$ BRACE

WIND SPEED		"MPH" REINT.		SBCI		ASCE	
AND MRH	MBR. SIZE						
110 MPH	2x4		10 %		10 %		
15 FT	2x6		40 %		50 %		
110 MPH	2x4		10 %		10 %		
30 FT	2x6		50 %		50 %		
100 MPH	2x4		10 %		10 %		
15 FT	2x6		30 %		50 %		
100 MPH	2x4		10 %		10 %		
30 FT	2x6		40 %		40 %		
90 MPH	2x4		20 %		10 %		
15 FT	2x6		20 %		40 %		
90 MPH	2x4		10 %		10 %		
30 FT	2x6		30 %		50 %		
80 MPH	2x4		10 %		20 %		
15 FT	2x6		10 %		30 %		
80 MPH	2x4		20 %		40 %		
30 FT	2x6		0 %		20 %		
70 MPH	2x4		0 %		20 %		
15 FT	2x6		0 %		20 %		
70 MPH	2x4		10 %		30 %		
30 FT	2x6		10 %		30 %		

ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT

GABLE VERTICAL = 24" O.C. SP #3

"T" REINFORCING MEMBER SIZE = 2X4

"J" BRACE INCREASE (FROM ABOVE) = 10% = 1.10

(1) 2×4 L BRACE LENGTH = 6 / MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH

$$1.10 \times 6' 7'' = 7' 3''$$

MAX TOT. LD. 60 PSF

DUR. FAC. ANY

MAX SPACING 24.0'

REF LET-IN VERT

DATE 2/23/07

-ENG DLJ/KAR

A circular professional engineer seal for James F. Collins, Jr. The outer ring contains the text "JAMES F. COLLINS, JR." at the top and "PROFESSIONAL ENGINEER" at the bottom. Inside this ring, the words "STATE OF FLORIDA" are written vertically on the left. In the center, the license number "No. 62212" is printed, with the date "6/07" above it. Two five-pointed stars are positioned on the horizontal center line, one on each side of the license number.

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED
CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB
BRACING.

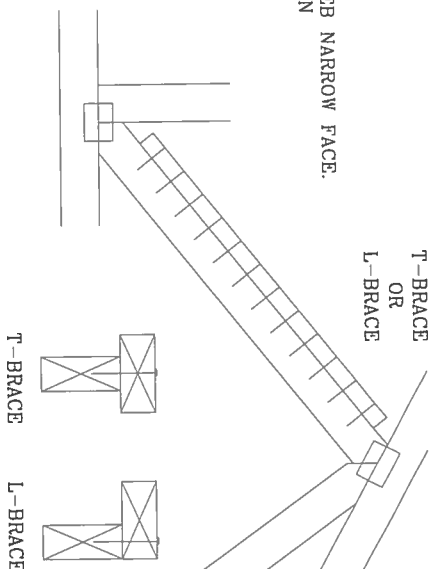
ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE.
FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE
BRACING.

WEB MEMBER SIZE	SPECIFIED CLUB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

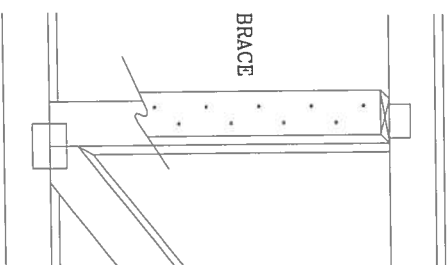
T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

- (*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3" MIN) NAILS.
AT 6" O.C. BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3." MIN) NAILS.
AT 6" O.C. BRACE IS A MINIMUM
50% OF WEB MEMBER LENGTH

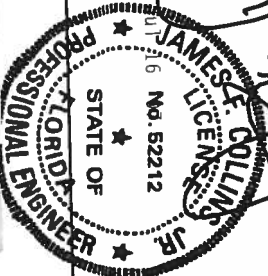


ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

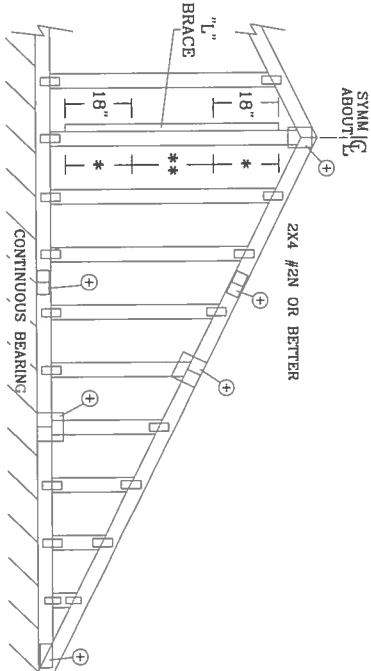
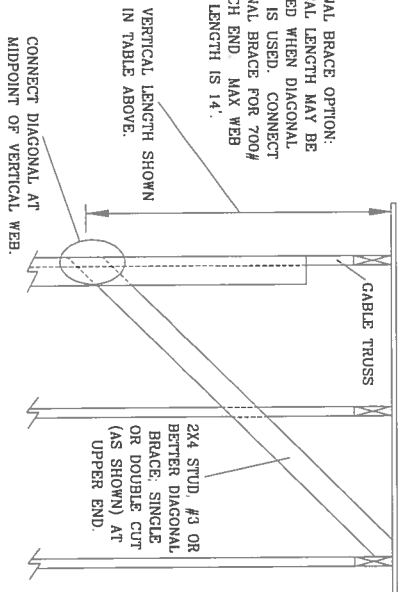
WARNING CROSSES REQUIRE EXTERIOR CAGE FABRICATING, HANDING, SHIPPING, INSTALLING AND BRACING. REFER TO THE FOLLOWING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE MANUFACTURER: 1815 PINEHURST CIR. SUITE 312 ALEXANDRIA, VA 22304 AND VICA (VIA) TRUSS COUNCIL, AMERICA, 6300 ENTERPRISE LN. MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR, TRUSS BCG, SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, AN FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN. HANDING, SHIPPING, INSTALLING, AND BRACING OF TRUSSES DESIGN CONFORMS WITH THE FOLLOWING PROVISIONS OF THE NATIONAL DESIGN SPEC. BY AISC AND THE TRUSS BCG. CONNECTOR PLATES ARE MADE OF 2014-T3 ALUMINUM AS SPECIFIED AND LOCATED ON THIS DRAWING. APPLY PLATES TO EACH FACE OF INSPECTION OF PLATES FOLLOWED BY (D) SHALL BE PER DESIGN, POSITION PER DRAWING. DESIGN, ON THIS DRAWING INDICATES THE SUITABILITY AND ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER UBCS/IFC 1, SEC. 2.



TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	BRCLBSUB0207
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

MAX GABLE VERTICAL LENGTH																
CABLE VERTICAL SPACING	2x4 SPECIES	BRACE GRADE	NO BRACES	(1) 1x4 "L" BRACE *		(1) 2x4 "L" BRACE *		(2) 2x4 "L" BRACE **		(1) 2x6 "L" BRACE *		(2) 2x6 "L" BRACE *		(2) 2x6 "L" BRACE **		
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B			
12" O.C.	SPF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"			
			#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"		
		STUD	3' 7"	5' 5"	5' 5"	7' 1"	7' 1"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"			
			STANDARD	3' 7"	4' 8"	4' 8"	6' 1"	6' 1"	8' 3"	8' 3"	9' 6"	9' 6"	12' 11"	12' 11"		
	HF	#1	4' 0"	6' 4"	6' 10"	7' 6"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"		
			#2	3' 11"	6' 4"	6' 10"	7' 6"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"	
		#3	3' 9"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	8' 11"	9' 5"	11' 5"	11' 5"	14' 0"	14' 0"		
			STUD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"	14' 0"	
	16" O.C.	SPF	#1 / #2	4' 2"	7' 3"	7' 5"	8' 7"	8' 7"	10' 3"	10' 6"	13' 5"	13' 10"	14' 0"	14' 0"		
				#3	4' 1"	6' 8"	6' 8"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"	
STUD			4' 1"	8' 0"	8' 0"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"			
			STANDARD	4' 1"	5' 8"	5' 8"	7' 6"	7' 6"	10' 1"	10' 1"	11' 8"	11' 8"	14' 0"	14' 0"		
HF		#1	4' 7"	7' 3"	7' 9"	8' 7"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"			
			#2	4' 6"	7' 3"	7' 9"	8' 7"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"		
		#3	4' 4"	6' 10"	6' 10"	8' 7"	8' 7"	9' 0"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"			
			STUD	4' 4"	6' 9"	6' 9"	8' 7"	8' 7"	8' 11"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"		
24" O.C.		SPF	#1 / #2	4' 2"	7' 3"	7' 5"	8' 7"	8' 7"	10' 3"	10' 6"	13' 5"	13' 10"	14' 0"	14' 0"		
				#3	4' 1"	6' 8"	6' 8"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"	
	STUD		4' 1"	8' 0"	8' 0"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"			
			STANDARD	4' 1"	5' 8"	5' 8"	7' 6"	7' 6"	10' 1"	10' 1"	11' 8"	11' 8"	14' 0"	14' 0"		
	HF	#1	4' 7"	7' 3"	7' 9"	8' 7"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"			
			#2	4' 6"	7' 3"	7' 9"	8' 7"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"		
		#3	4' 4"	6' 10"	6' 10"	8' 7"	8' 7"	9' 0"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"			
			STUD	4' 2"	6' 9"	6' 9"	8' 7"	8' 7"	8' 11"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"		
	12" O.C.	SPF	#1 / #2	4' 7"	8' 0"	8' 2"	9' 5"	9' 5"	11' 3"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"		
				#3	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
STUD			4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"			
			STANDARD	4' 6"	6' 7"	6' 7"	8' 8"	8' 8"	11' 3"	11' 3"	13' 6"	13' 6"	14' 0"	14' 0"		
HF		#1	5' 1"	8' 0"	8' 7"	9' 5"	9' 5"	10' 2"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"			
			#2	4' 11"	8' 0"	8' 7"	9' 5"	9' 5"	10' 2"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"		
		#3	4' 9"	7' 11"	7' 11"	9' 5"	9' 5"	9' 11"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"			
			STUD	4' 9"	7' 9"	7' 9"	9' 5"	9' 5"	9' 11"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"		
DFL		#1 / #2	4' 7"	6' 9"	6' 9"	8' 10"	8' 10"	11' 3"	11' 7"	13' 10"	13' 10"	14' 0"	14' 0"			
			STANDARD	4' 7"	6' 9"	6' 9"	8' 10"	8' 10"	11' 3"	11' 7"	13' 10"	13' 10"	14' 0"	14' 0"		



REFER TO CHART ABOVE FOR MAX CABLE VERTICAL LENGTH.

DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WHEN DIAGONAL
BRACE IS USED. CONNECT
DIAGONAL BRACE FOR 700#
AT EACH END. MAX WEB
TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN
IN TABLE ABOVE.

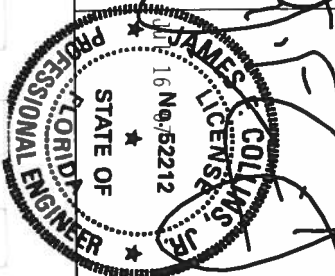
CONNECT DIAGONAL AT
MIDPOINT OF VERTICAL
WEB.



ITV BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI CRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA 22304 AND VITA CADD TRUSS CODE FOR AMERICA, 6900 ENTERPRISE LN, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL DIMENSIONS ARE IN FEET AND INCHES. THESE DIMENSIONS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATING FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA 800 AND ITV), BCG CONNECTOR PLATES ARE MADE OF 20/26GA GALVALUMINUM SHEET. THE CONNECTOR PLATES ARE LOCATED ON THIS TRUSS DESIGN PER DRAWING 1606-27. ANY MODIFICATION TO THIS DESIGN SHALL BE PERMITTED BY PERMITS AUTHORITY AT OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



REF	ASCE7-02 CAB11030
DATE	2/23/07
DRWG	A11030E0207
-ENG	
MAX. TOT. LD.	60 PSF
MAX. SPACING	24.0"

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPICE
LESS THAN 4' 0"	1x4 OR 2x3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2x4
GREATER THAN 11' 6"	2.5x4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

ATTACH EACH "L" BRACE WITH 10d NAILS.

* FOR (1) "L" BRACE: SPACE NAILS AT 2' O.C. IN 18" END ZONES AND 4' O.C. BETWEEN ZONES.

** FOR (2) "L" BRACES: SPACE NAILS AT 3' O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 100 PSF OVER CONTINUOUS BEARING (6 PSF TO DEAD LOAD).

CABLE END SUPPORTS LOAD FROM 4' 0" OUTDOCKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

CABLE TRUSS DETAIL NOTES:

BRACING GROUP SPECIES AND GRADES:			
GROUP A:		GROUP B:	
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STUD	#2	STUD
#3	STUD	#3	STUD
DOUGLAS FIR-LARCH		DOUGLAS FIR-LARCH	
#3	STUD	#3	STUD
STANDARD		STANDARD	

2X4 GABLE VERTICAL		BRACE		NO		(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
SPACING	SPECIES	GRADE	BRACES	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
24" O.C.	SPF	#1 / #2	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	3' 10"	4' 5"	4' 5"	7' 10"	7' 10"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS $L/240$.

PROVIDE UPLIFT CONNECTIONS FOR 80 PSF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

* FOR (1) "L" BRACE: SPACE NAILS AT 2' 0" O.C. IN 16" END ZONES AND 4" O.C. BETWEEN ZONES.

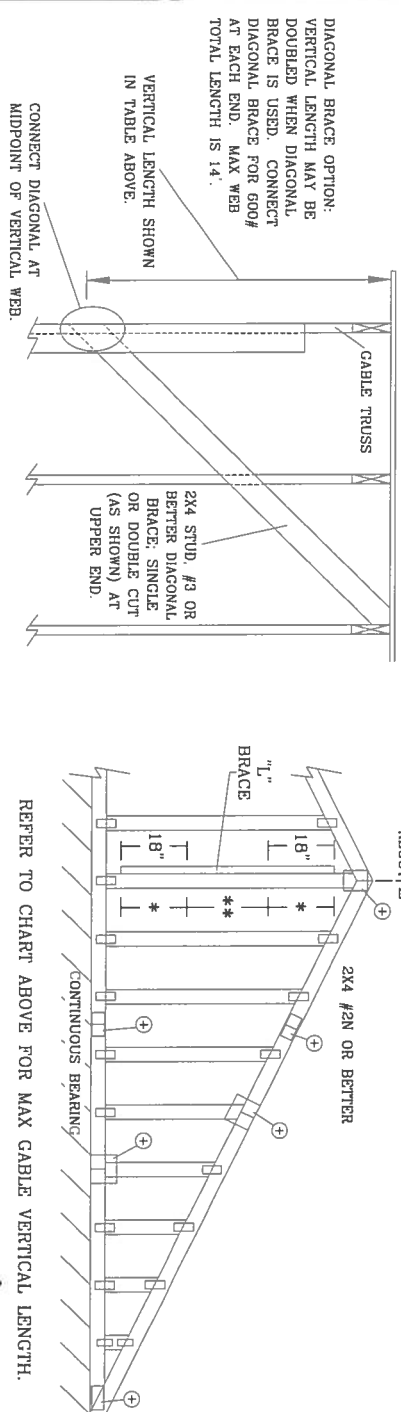
** FOR (2) "L" BRACES: SPACE NAILS AT 3' 0" O.C. IN 16" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

CABLE VERTICAL PLATE SIZES		
VERTICAL LENGTH	NO SPLICE	
LESS THAN 4' 0"	1X4 OR 2X3	
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2X4	
GREATER THAN 11' 6"	2.5X4	

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.



DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 600# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.

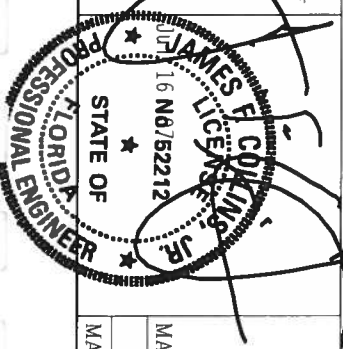
VERTICAL LENGTH SHOWN IN TABLE ABOVE.

CONNECT DIAGONAL AT MIDPOINT OF VERTICAL WEB.



ITV BUILDING COMPONENTS GROUP, INC.
POMPAHO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY ITV TRUSS PLATE MANUFACTURER, 218 NORTHEAST ST. APT. 312, ALEXANDRIA, VA 22304 AND VICA (VIRGINIA COUNCIL OF ARCHITECTS) FOR ADDITIONAL DESIGN SPECIFICATIONS. ITV TRUSS PLATE MANUFACTURER'S INSTRUCTIONS FOR INSTALLATION AND BRACING OF TRUSSES. GALV. STEEL: APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED BY THIS DESIGN, POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FOLLOWED BY CD SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

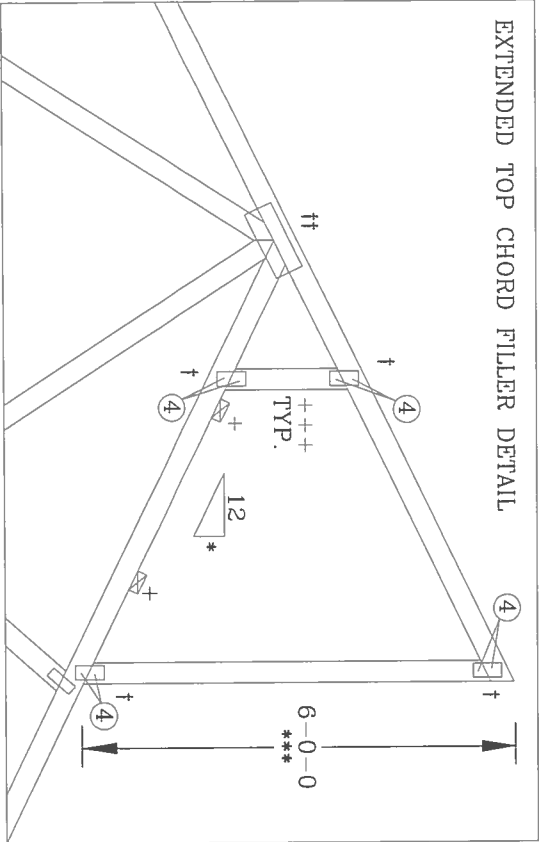


REF	ASCE 7-98-CAB11015
DATE	2/23/07
DRWG	A11015EC0207
ENG	
MAX. TOT. LD.	60 PSF
MAX. SPACING	24' 0"

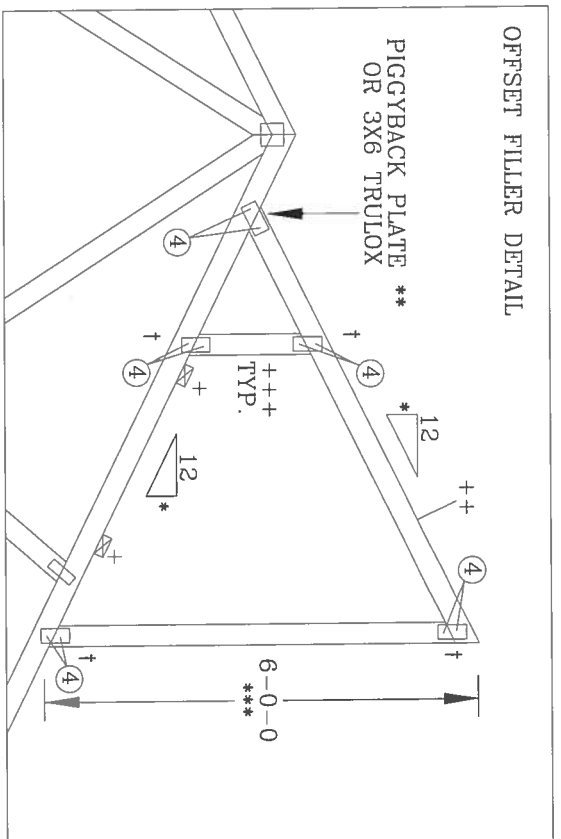
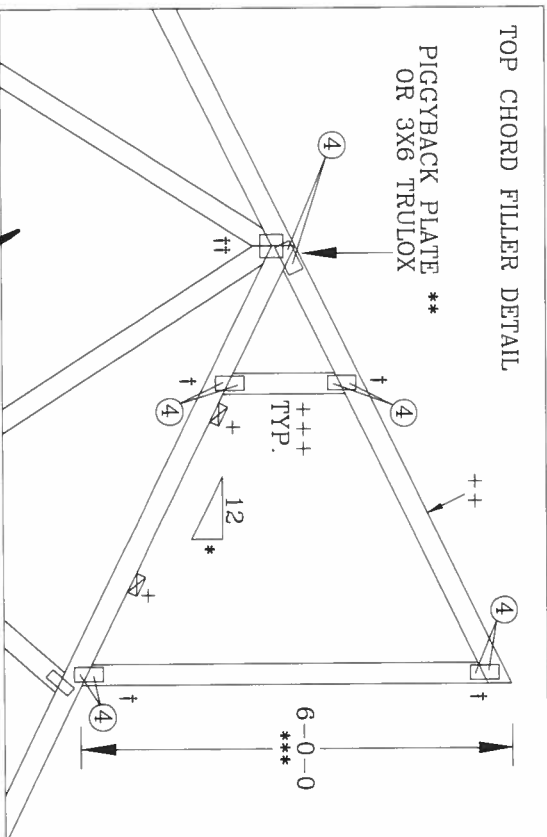
TOP CHORD FILLER DETAIL

- + 2X4 CONTINUOUS LATERAL BRACING AT 24" O.C. MAXIMUM SPACING. ATTACH TO EACH TOP CHORD WITH (2) 16d COMMON (0.162"X 3.5", MIN) NAILS. BRACING MATERIAL TO BE SUPPLIED AND ATTACHED AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.
- ++ 2X4 SO. PINE #2 N OR SPF #1/#2 FILLER TOP CHORD.
- +++ 2X4 SO. PINE #3 OR SPF #1/#2 VERTICAL WEBS SPACED 48" OC MAXIMUM.
- * 8/12 MAXIMUM PITCH.
- ** 2X8/25 PIGGYBACK SPECIAL PLATE. SEE DRAWING PIGGYBACKB0699 FOR PIGGYBACK SPECIAL PLATE INFORMATION.
- *** 6'0" MAXIMUM HEIGHT.
- + W2X4 OR 3X6 TRULOX.
- †† REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.
- 0.120"X 1.375" NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO EACH FACE OF EACH TRUSS PLY. SEE DWG. 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS.

EXTENDED TOP CHORD FILLER DETAIL



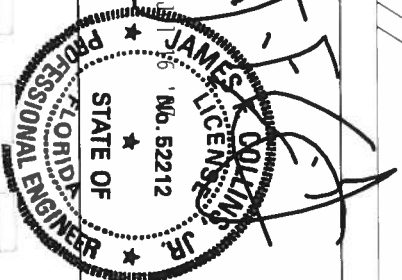
TOP CHORD FILLER DETAIL



ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. TTV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING AT PAPA, TPI DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE AMERICAN 6300 ENTERPRISE, IN HANDLING OF TOP CHORDS. TTV, BCG, CONNECTS PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SAFELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



THIS DRAWING REPLACES DRAWING 884,080			
TC LL	MAX 30 PSF	REF	TC-FILLER
TC DL	MAX 15 PSF	DATE	2/23/07
BC DL	MAX 10 PSF	DRWG	TCFILLER0207
BC LL	0 PSF	-ENG	SJP/KAR
TOT. LD.	MAX 55 PSF		
DUR. FAC.	1.15 OR 1.33		
SPACING	24.0"		

BOTTOM CHORD FILLER DETAIL

* OPTIONAL INTERIOR OR CANTILEVER BEARING. MINIMUM PLATE SIZES (1X3 WAVE) MAY BE USED IF BEARING IS OMITTED. WEDGE OR VERTICAL MEMBER MUST COINCIDE WITH BEARING LOCATION.

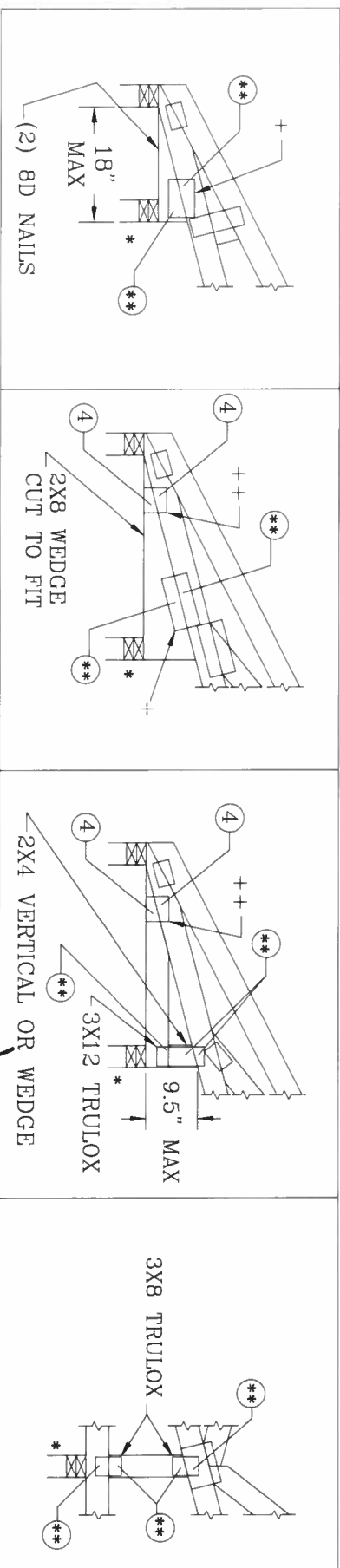
+ 3X4 WAVE OR 4X8 TRULOX
++ 2X4 WAVE OR 3X6 TRULOX

0.120" X 1.375", NAILS, REQUIRED FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO EACH FACE OF THE TRUSS. SEE DWG. 1607L FOR NAILING AND TRULOX PLATE REQUIREMENTS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

ALL TRULOX PLATES SHOWN ARE MINIMUMS. LARGER PLATES MAY BE REQUIRED TO ACCOMMODATE REQUIRED NAILS (**)

FILLER BOTTOM CHORD OR WEDGE SPECIES	MAXIMUM REACTION		MINIMUM BEARING AREA	** REQUIRED NAILS PER FACE WITH TRULOX PLATES					
	DOWNWARD	UPLIFT		1.00 D.O.L.	1.15 D.O.L.	1.25 D.O.L.	1.33 D.O.L.	1.60 D.O.L.	
DOUGLAS FIR-LARCH	3281 #	1656 #	1.5" X 3.5"	12	11	10	9	8	
HEM-FIR	2126 #	1095 #	1.5" X 3.5"	9	8	7	7	6	
SPRUCE-PINE-FIR	2231 #	1192 #	1.5" X 3.5"	10	9	8	8	6	
SOUTHERN PINE DENSE	3465 #	1791 #	1.5" X 3.5"	12	11	10	9	8	
SOUTHERN PINE	2966 #	1492 #	1.5" X 3.5"	10	9	8	8	7	
SOUTHERN PINE NON-DENSE	2520 #	1343 #	1.5" X 3.5"	9	8	7	7	6	



THIS DRAWING REPLACES DRAWINGS A115 A115/R & 884.132

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

JAMES E. COLLINS, JR.
FLORIDA LICENSE NO. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER

TC LL	—	PSF	REF	BC FILLER
TC DL	—	PSF	DATE	2/23/07
BC DL	10.0	PSF	DRWG	BCFILLER0207
BC LL	—	PSF	—	ENG DLJ/KAR
TOT. LD.	—	PSF		
DUR. FAC.	1.0/1.15/1.25/1.33			
SPACING	24.0"			

BOTTOM CHORD FILLER REPAIR

RECOMMENDED REPAIR PROCEDURE

1. MEASURE DISTANCE FOR NEW LENGTH OF FILLER.
2. APPLY NEW 2X4 STUD GRADE OR BETTER VERTICAL SCAB TO BOTTOM CHORD AND FILLER WITH (3) NAILS 0.131" DIA. x 3.0" OR LARGER, (I.E. 10d OR 16d COMMON, SINKER, GUN, OR 16d BOX NAILS) TO EACH END OF VERTICAL.
3. CAREFULLY REMOVE EFFECTED CONNECTOR PLATES. USE CARE NOT TO DAMAGE THE REMAINING CONNECTOR PLATES OR LUMBER IN ANY WAY.
4. TRIM FILLER TO LENGTH, AT EDGE OF NEW VERTICAL SCAB.

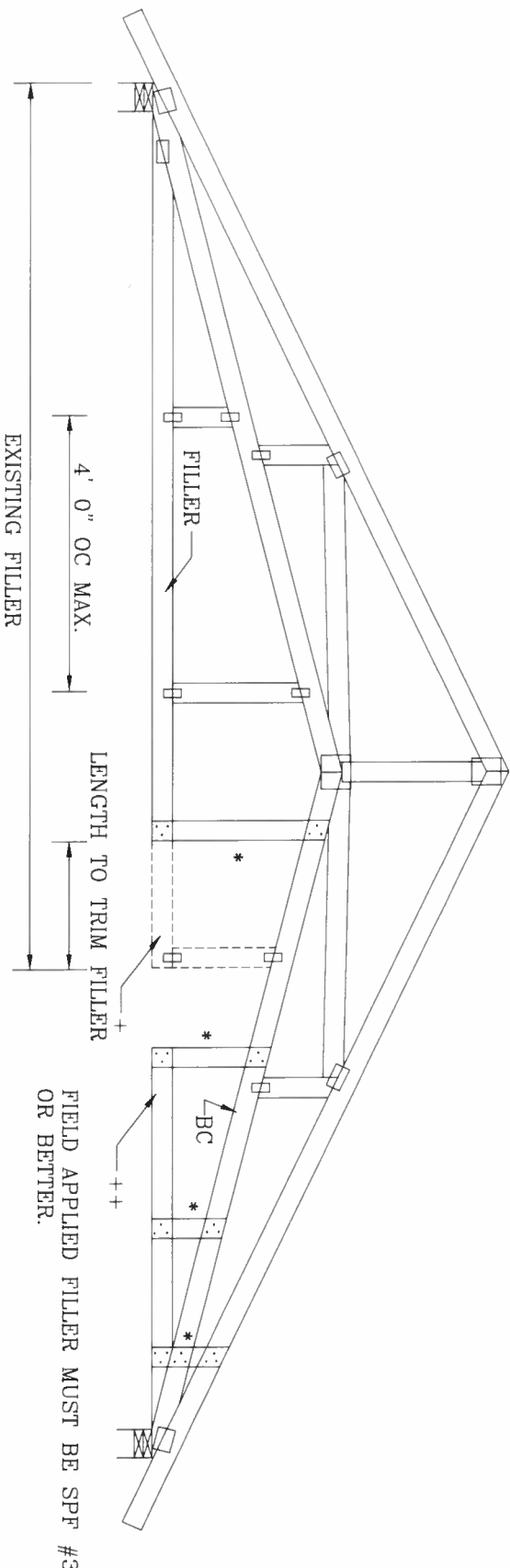
MAXIMUM BOTTOM CHORD LOAD IS 10 PSF.

+ BOTTOM CHORD FILLER TO BE REMOVED. SEE NOTE #3.

++ FIELD APPLIED FILLER.

* 2X4 STUD GRADE OR BETTER VERTICAL SCAB. ATTACH TO BOTTOM CHORD AND FILLER WITH (3) NAILS WITH A MIN. 0.131" DIA. X 3.0" LENGTH.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR ALLOWABLE FILLER DIMENSIONS, PLACEMENT, AND WEBBING.



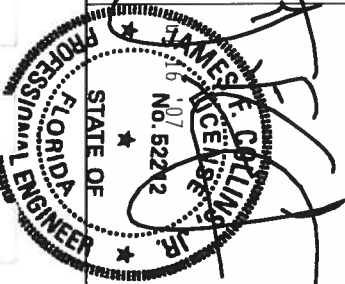
THIS DRAWING REPLACES DRAWING 962.767

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPAHO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 210 NORTH LEE ST., SUITE 312, ALEXANDRIA, VA 22314 AND WCA WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., HANSON, VI 55719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THE FOLLOWING. UNLESS OTHERWISE INDICATED, ALL DIMENSIONS SHALL BE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID GELING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONTRACTORS WITH APPLICABLE PROVISIONS OF NDS QUALITY DESIGN SPEC. BY ACP/PAI AND TPI. ALL BCG CONNECTOR PLATES ARE MADE OF 20/10/16GA CAL/SPLY ASTM A653 GRADE 40/60 (A1033) GALV. COATED STEEL. PLATES ARE EXPOSED TO WEATHER AND SHALL BE PROTECTED BY AN ANTI-RUST DESIGN. POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY CD SHALL BE PER ANNEK A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



REF	BC FILLER REP.
DATE	2/23/07
DRWG	REPBCEFL0207
-ENG	MLH/KAR