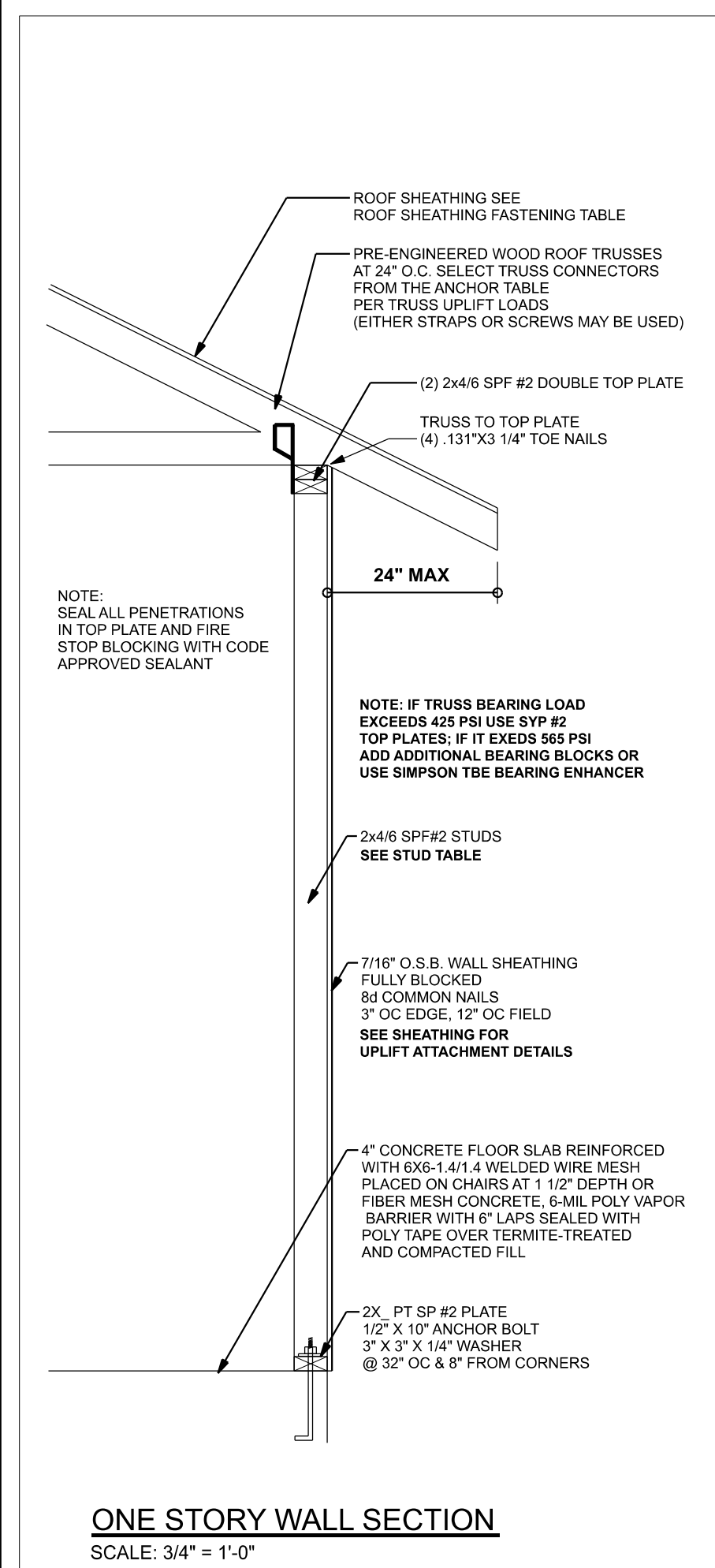
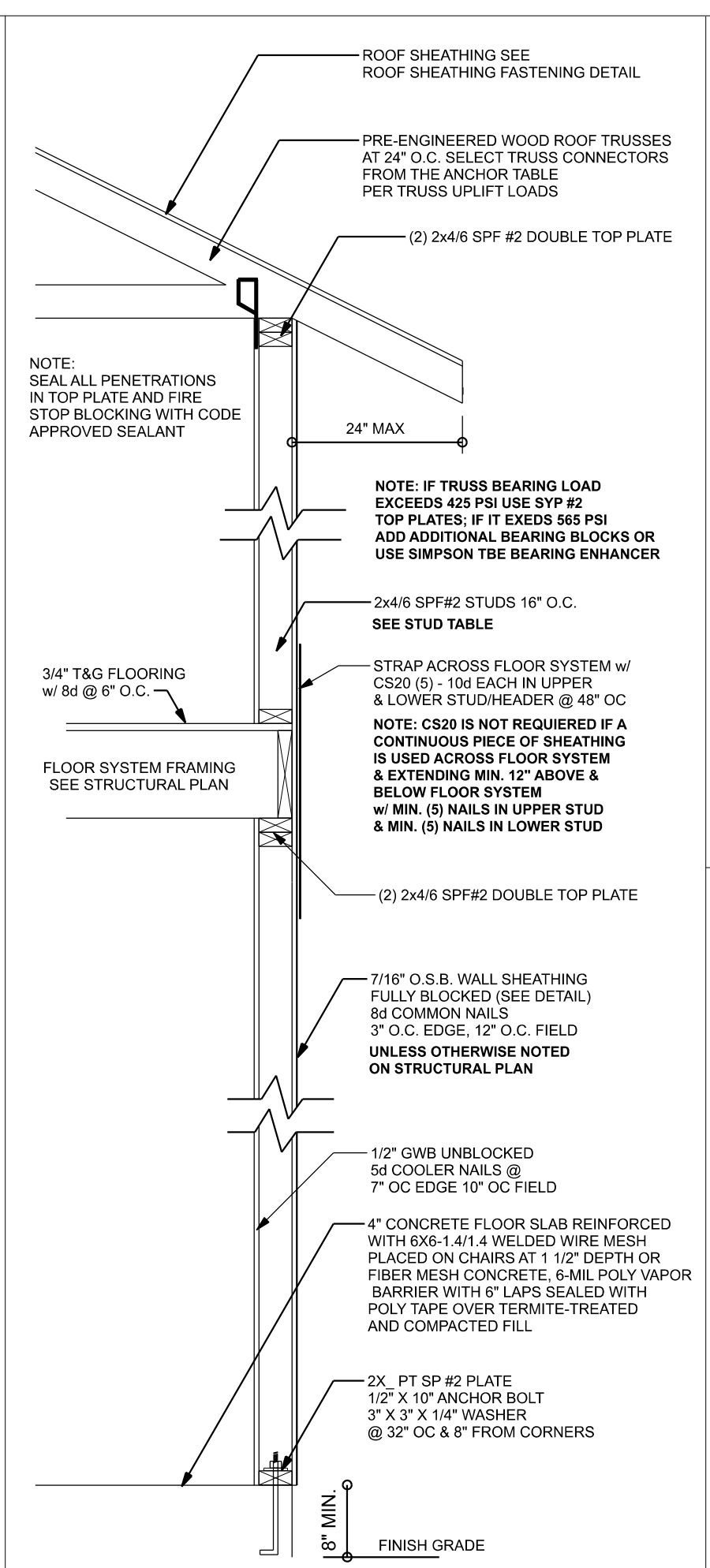
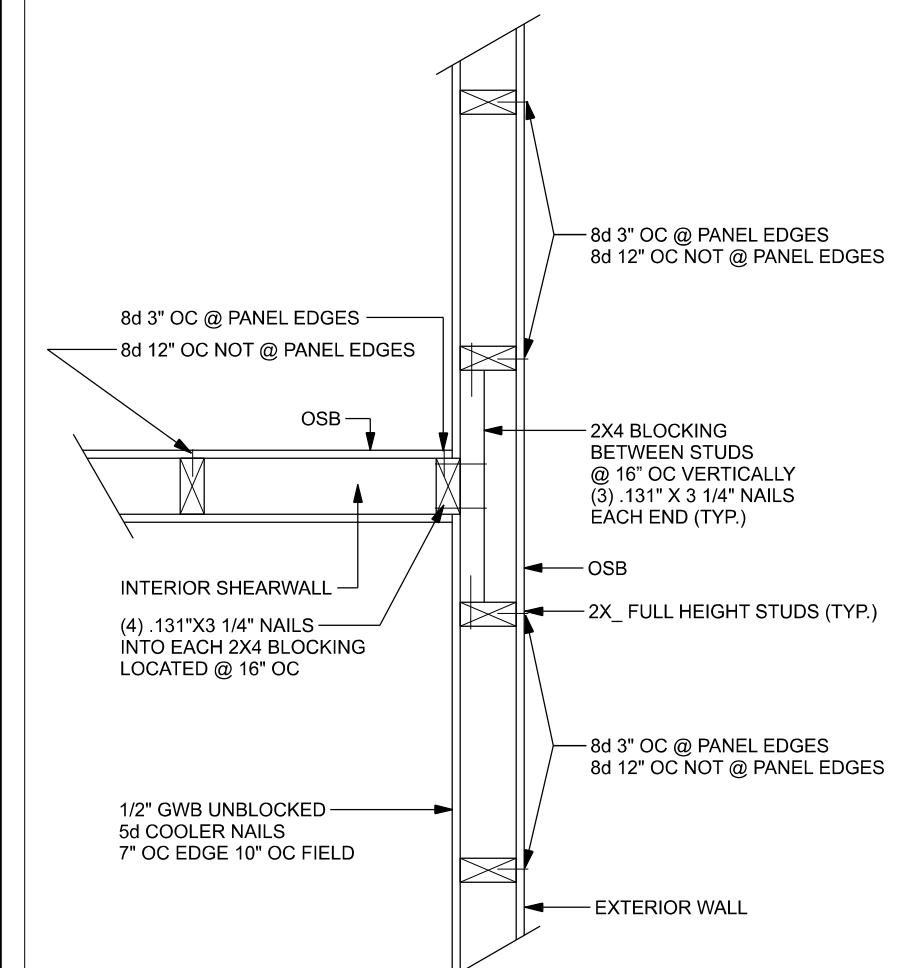




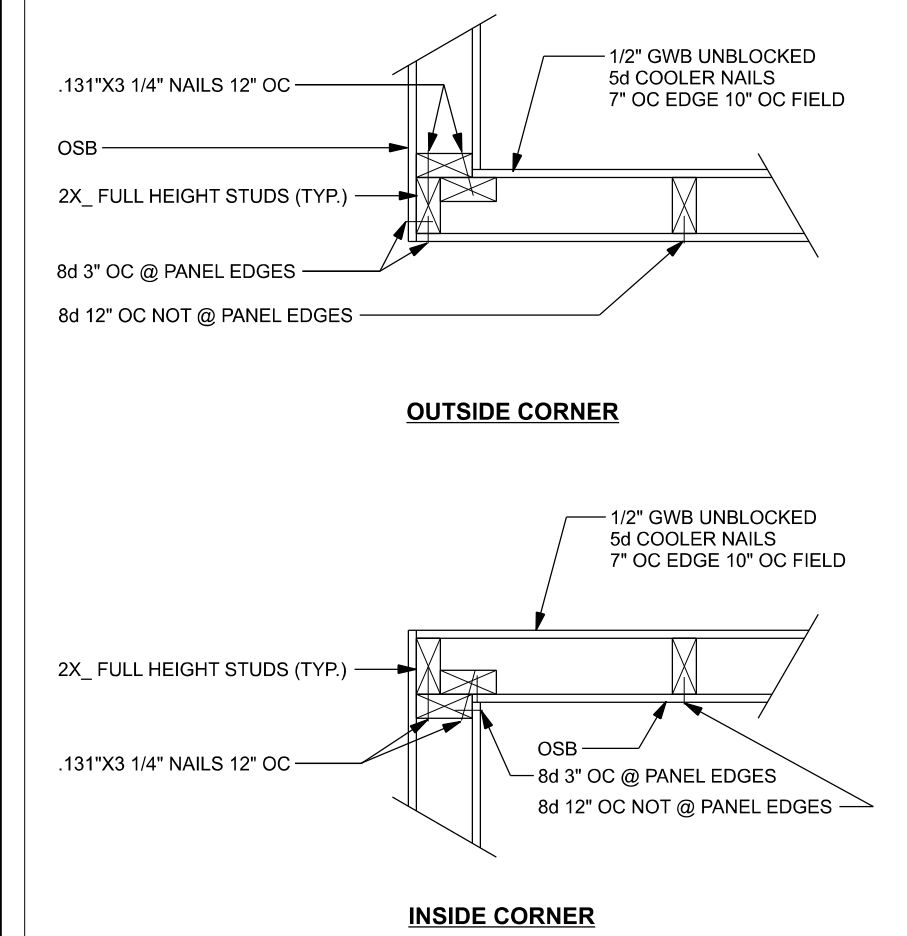
ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



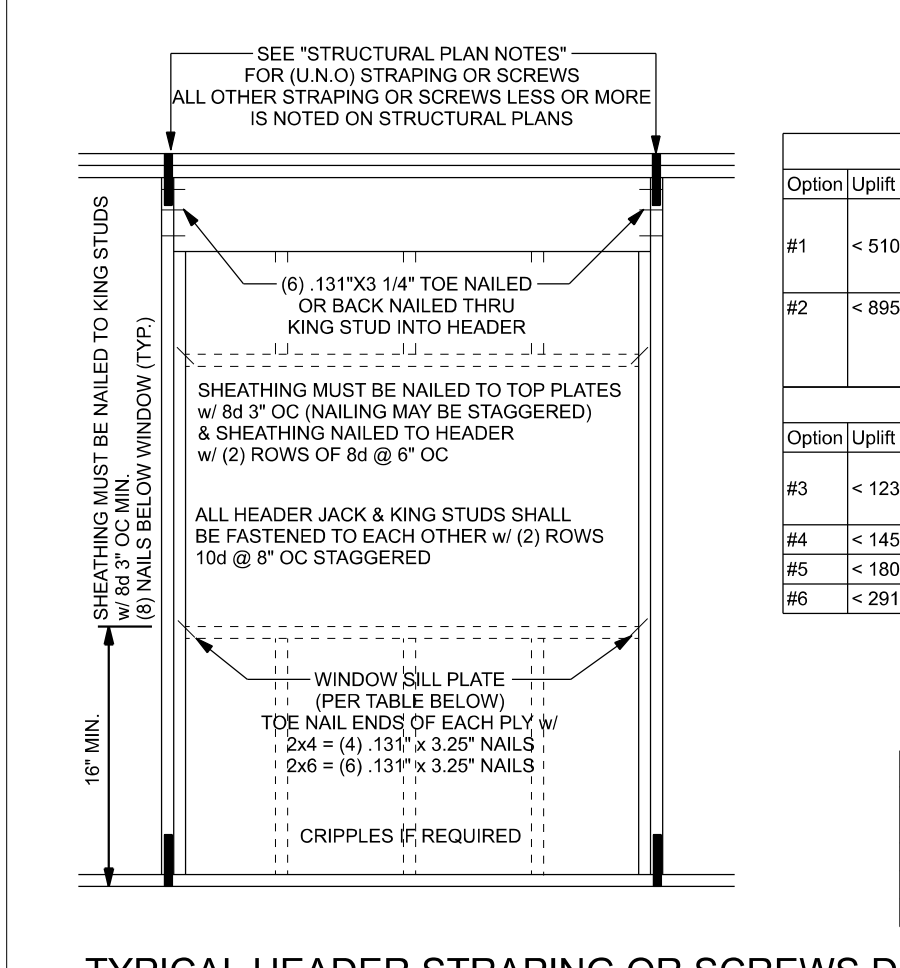
TWO STORY WALL SECTION
SCALE: 3/4" = 1'-0"



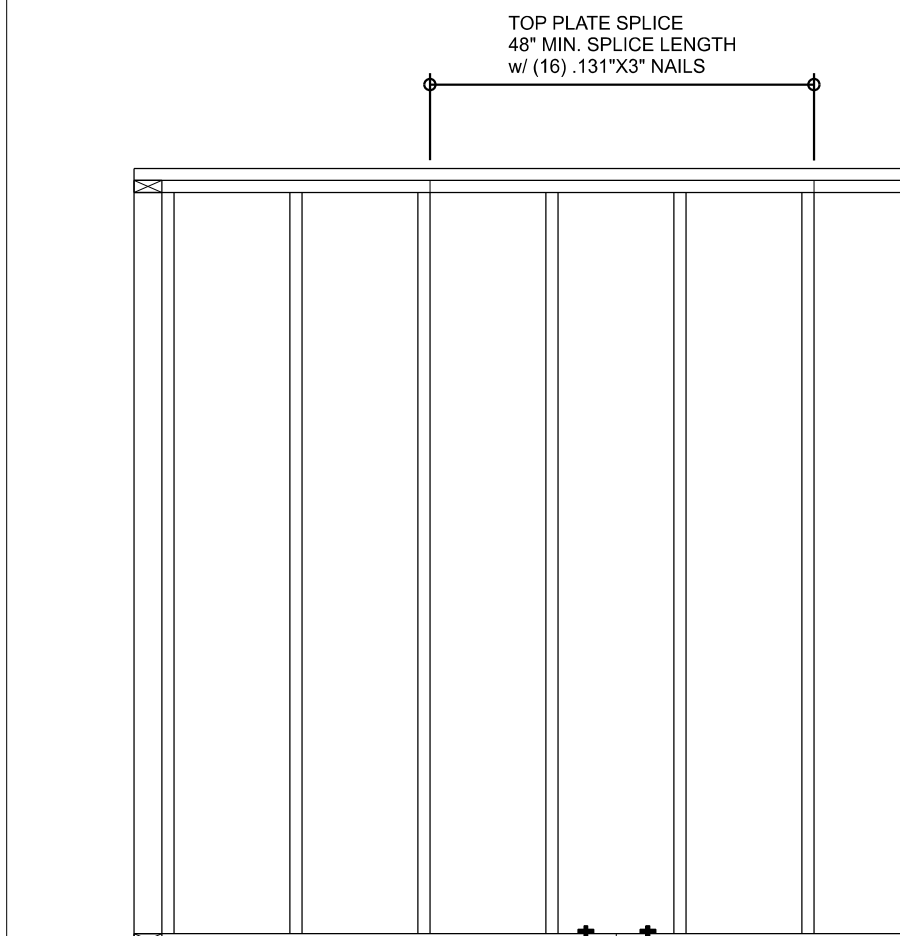
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



(TYP.) CORNER FRAMING
WOOD FRAME



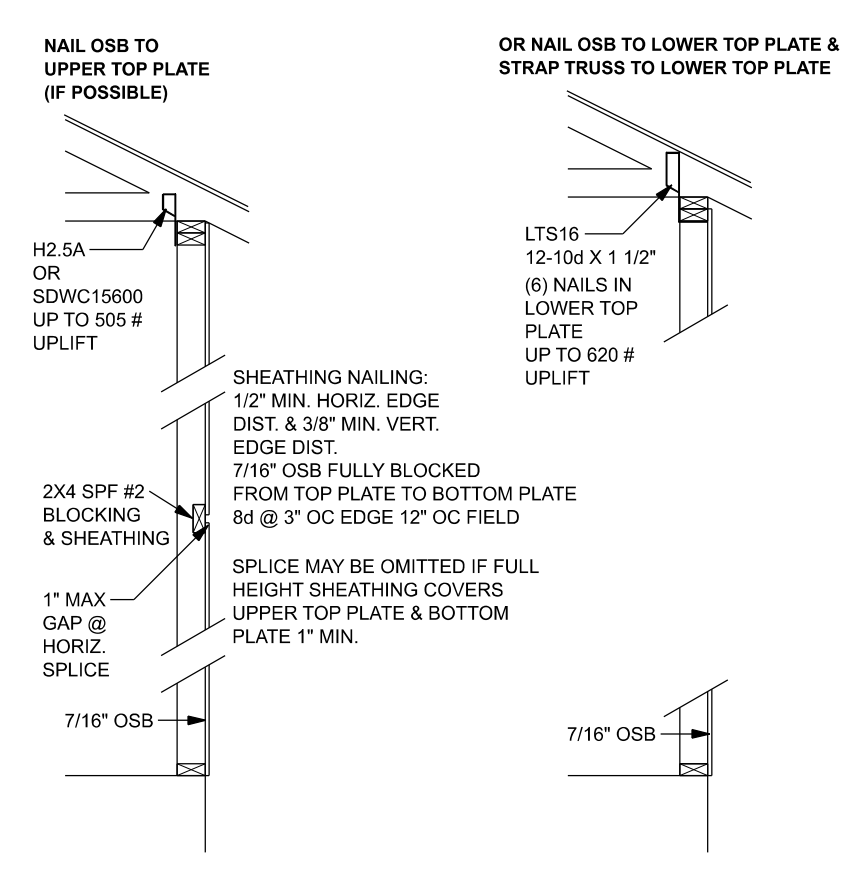
TYPICAL HEADER STRAPING OR SCREWS DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME

ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)					
Wind Speed	Sheathing Thickness Plywood OR OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field	
120 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	12" oc	
120 mph Exp. C	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc	
120 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	
130 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc	
130 mph Exp. C	15/32"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc	
130 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc	
140 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	
150 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	
150 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	4" oc	4" oc	

Note: For sheathing located a minimum of 1 foot from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.

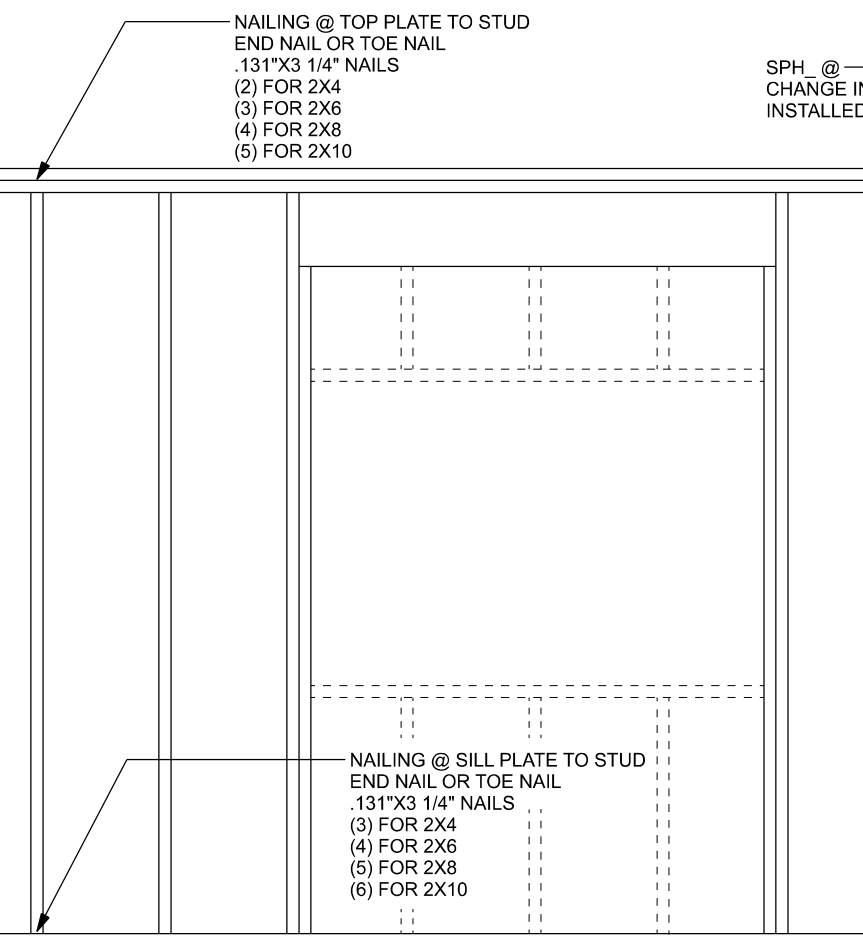


SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME

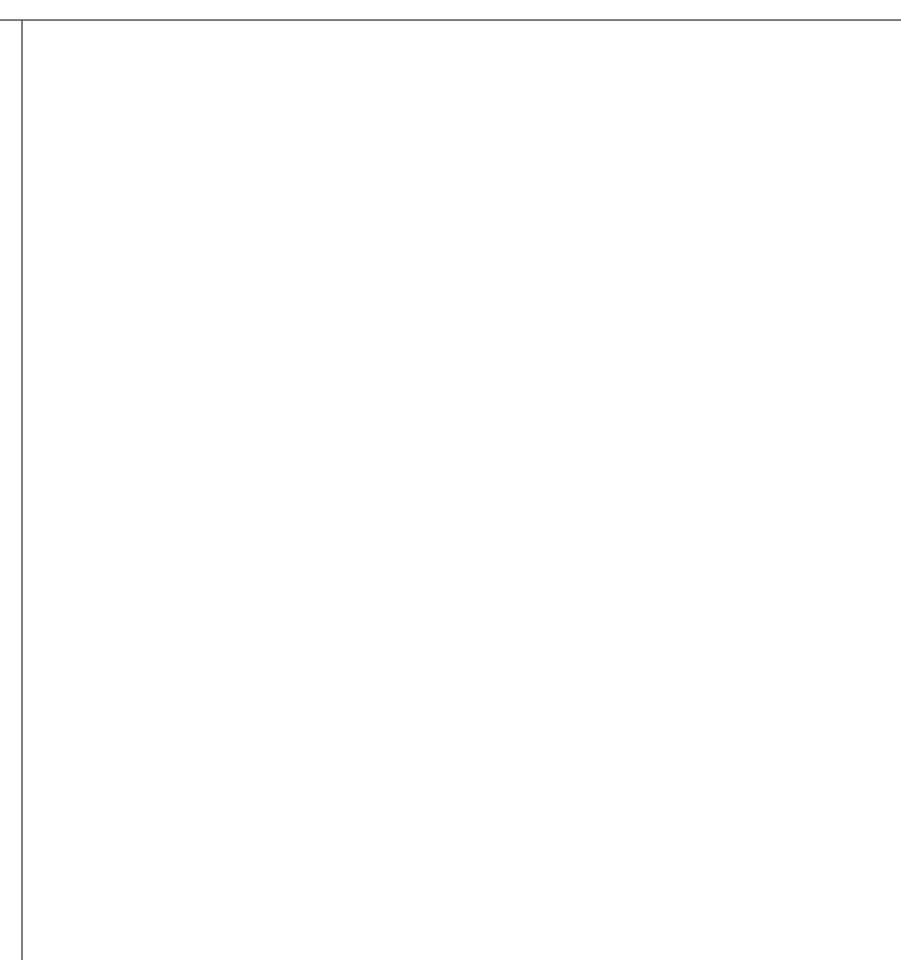
HEADER SCREWS TABLE			
Option	Uplift	Top Connection	Bottom Connection
#1	< 510	Attach king stud to top plate w/ (1) Simpson SDWC15600	Attach king stud to bottom plate w/ (2) Simpson SDWC15450
#2	< 895	Attach king stud to top plate w/ (2) Simpson SDWC15600	Attach king stud to bottom plate w/ (3) Simpson SDWC15450
HEADER STRAP TABLE			
Option	Uplift	Top Connection	Bottom Connection
#3	< 1235	LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate
#4	< 1455	MSTA24, 18-10d header to jacks	DTT22
#5	< 1800	(2) MSTA24, 18-10d header to jacks	DTT22
#6	< 2910	(2) MSTA24, 18-10d header to jacks	HTT4

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT			
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFOF TABLE A.3.3.3.3.3.3	FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)
130 MPH EXP. C	5'-2"	7'-9"	7'-7"
	(1) 2x4	(2) 2x4	(1) 2x6
			(2) 2x6

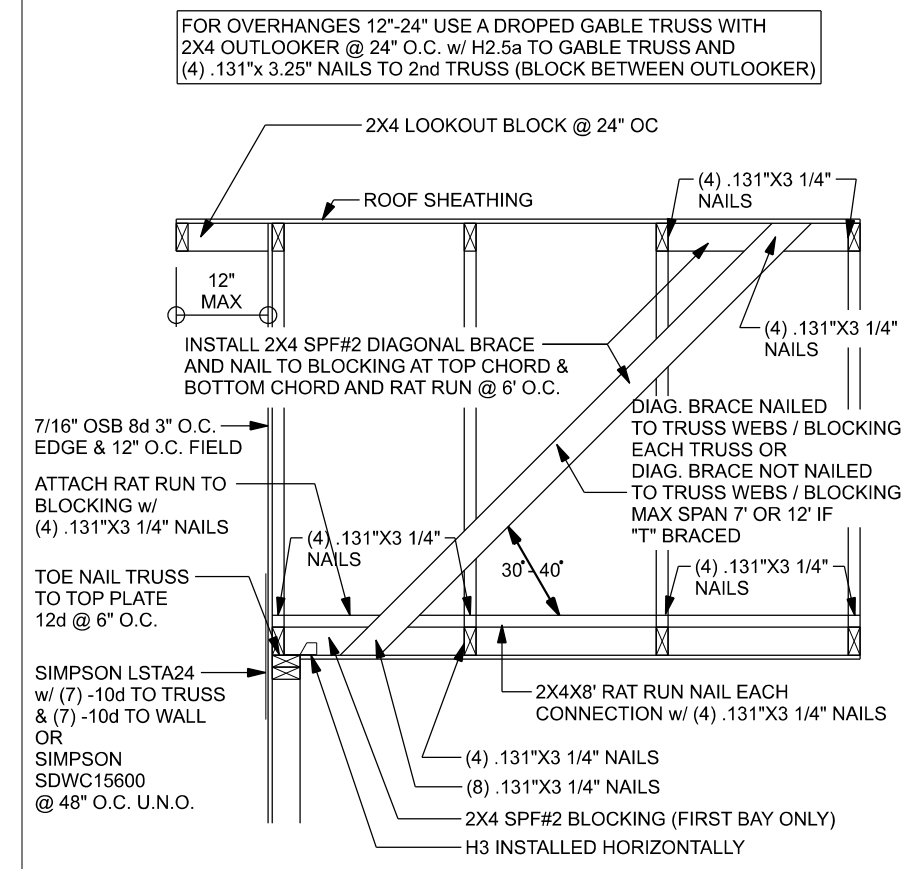
(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



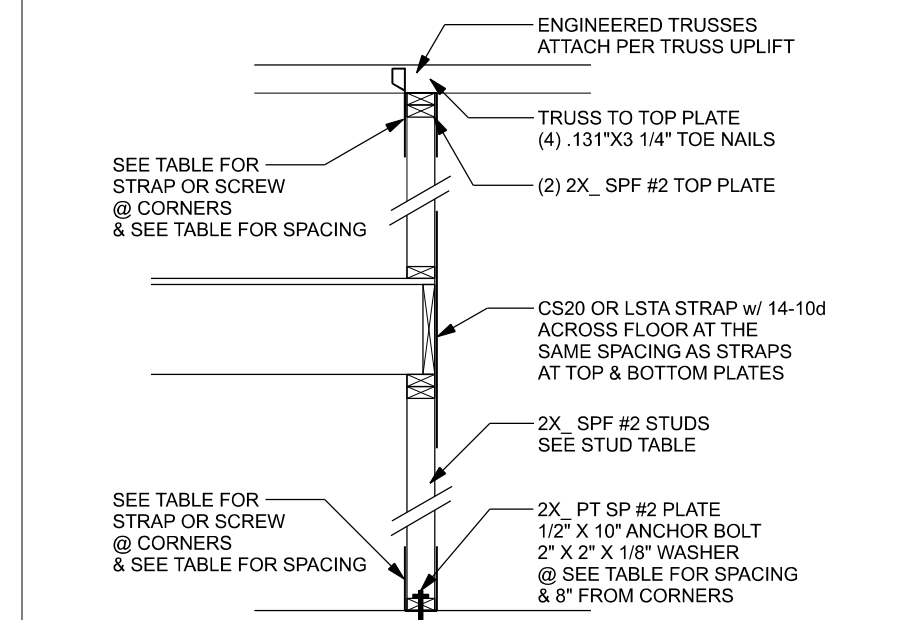
(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS

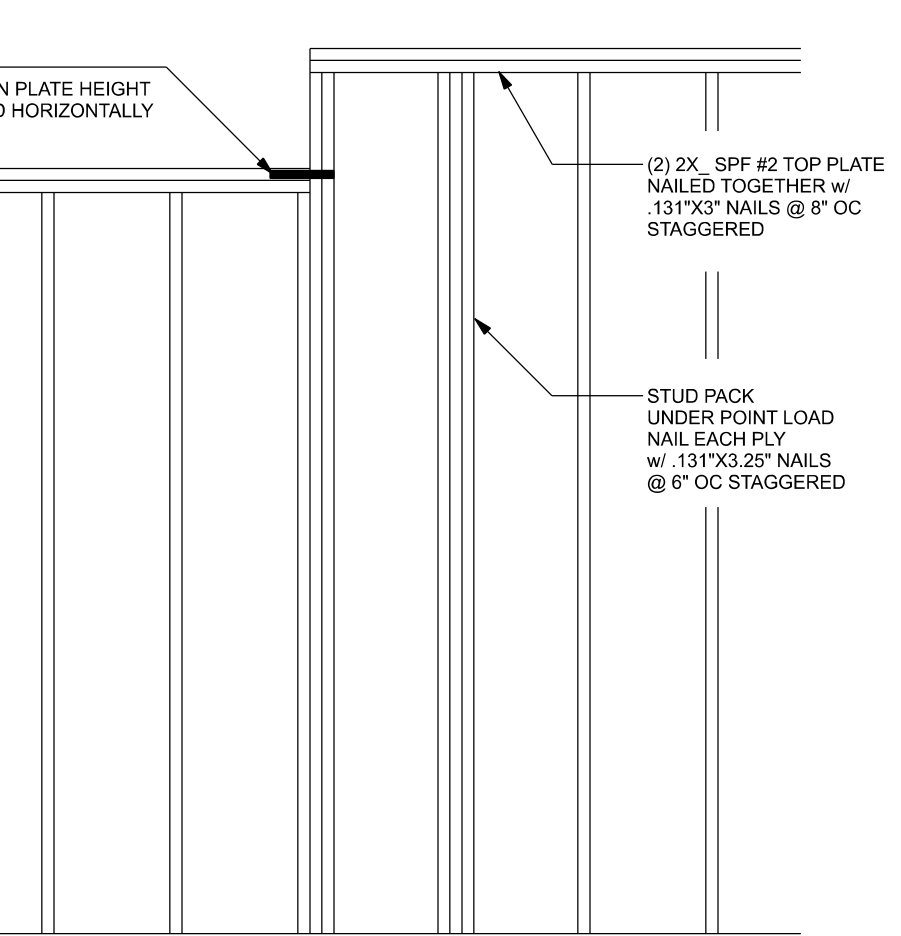


(TYP.) GABLE BRACING DETAIL
WOOD FRAME

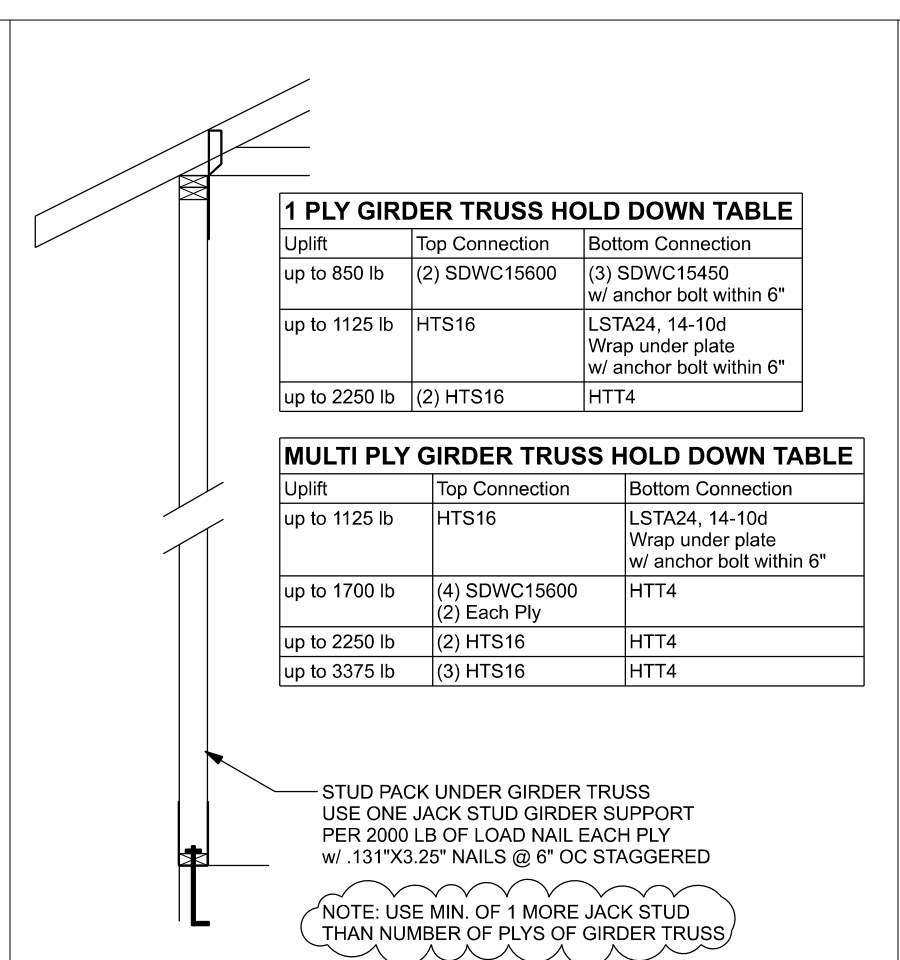


INTERIOR BEARING WALL UPLIFT STRAP OR SCREW TABLE			
Uplift on wall	Top Connection	Bottom Connection	Anchor Bolt Spacing
227 pcf	SP2 @ 32" OC	SP1 @ 32" OC	48" OC
454 pcf	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
681 pcf	(2) SDWC15600 @ 48" OC	(3) SDWC15450 @ 48" OC	48" OC
908 pcf	(2) SDWC15600 @ 32" OC	(3) SDWC15450 @ 32" OC	48" OC
1135 pcf	LSTA24, 14-10d @ 48" OC	LSTA24, 14-10d @ 48" OC	48" OC
1362 pcf	LSTA24, 14-10d @ 32" OC	LSTA24, 14-10d @ 32" OC	32" OC
1589 pcf	LSTA24, 14-10d @ 32" OC	LSTA24, 14-10d @ 32" OC	32" OC

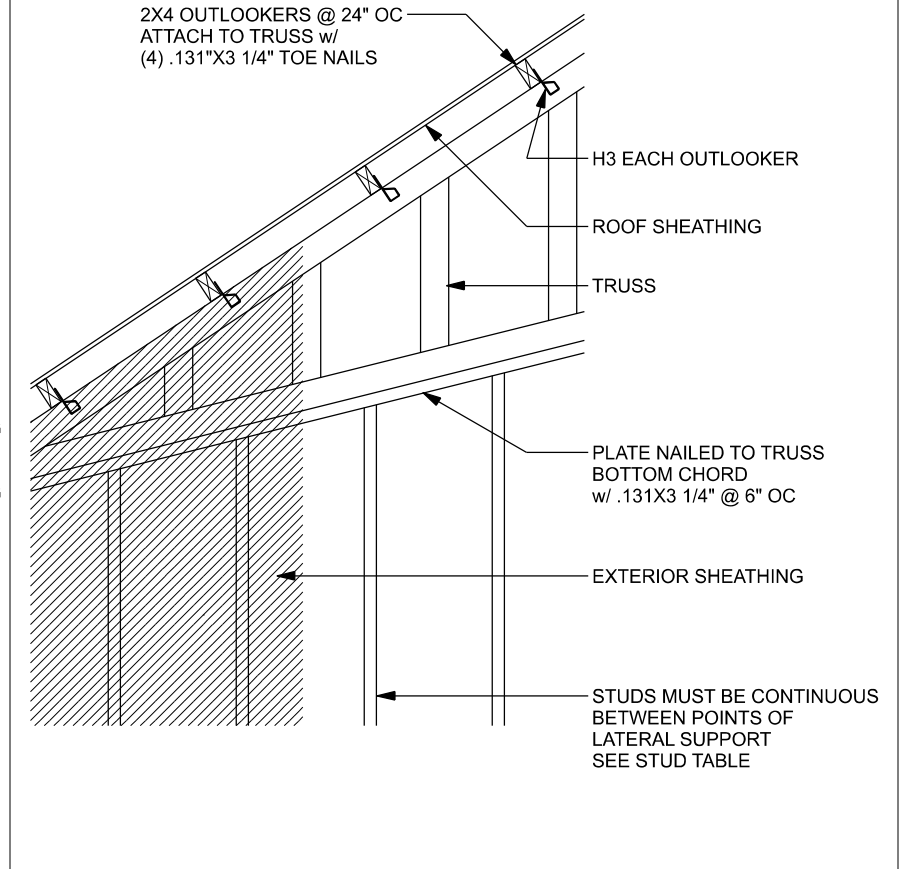
(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS

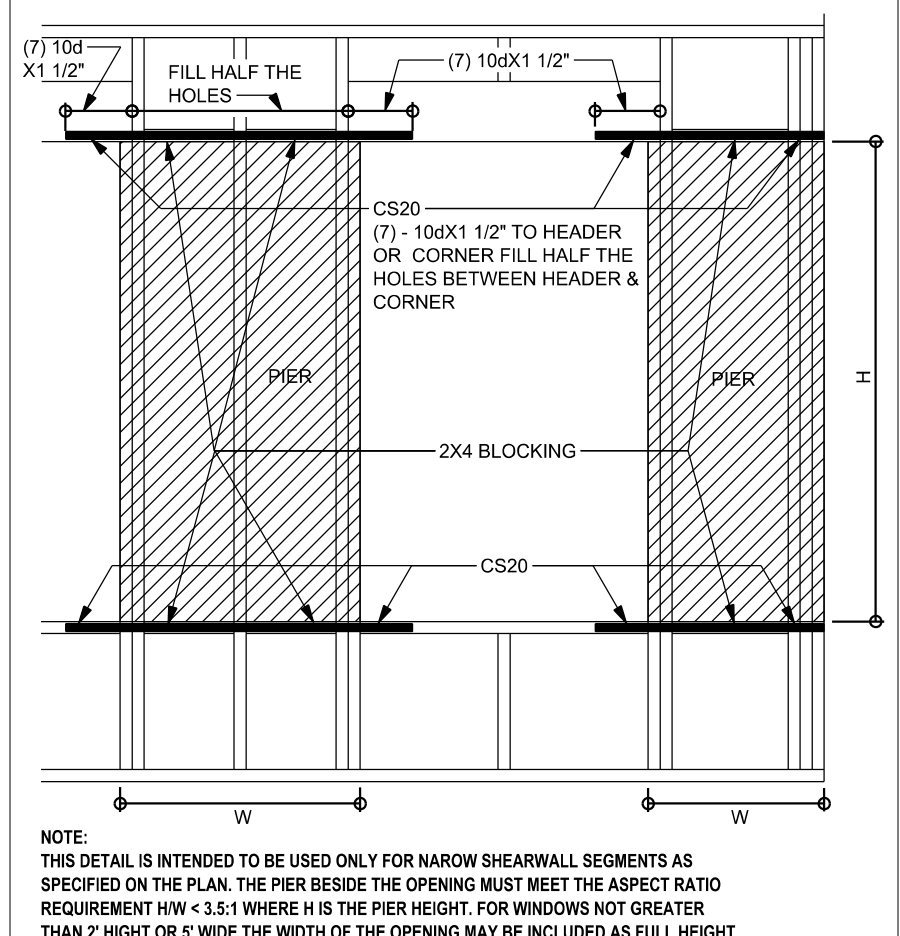


(TYP.) GABLE BRACING DETAIL
WOOD FRAME



INTERIOR BEARING WALL UPLIFT STRAP OR SCREW TABLE			
Uplift on wall	Top Connection	Bottom Connection	Anchor Bolt Spacing
227 pcf	SP2 @ 32" OC	SP1 @ 32" OC	48" OC
454 pcf	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
681 pcf	(2) SDWC15600 @ 48" OC	(3) SDWC15450 @ 48" OC	48" OC
908 pcf	(2) SDWC15600 @ 32" OC	(3) SDWC15450 @ 32" OC	48" OC
1135 pcf	LSTA24, 14-10d @ 48" OC	LSTA24, 14-10d @ 48" OC	48" OC
1362 pcf	LSTA24, 14-10d @ 32" OC	LSTA24, 14-10d @ 32" OC	32" OC
1589 pcf	LSTA24, 14-10d @ 32" OC	LSTA24, 14-10d @ 32" OC	32" OC

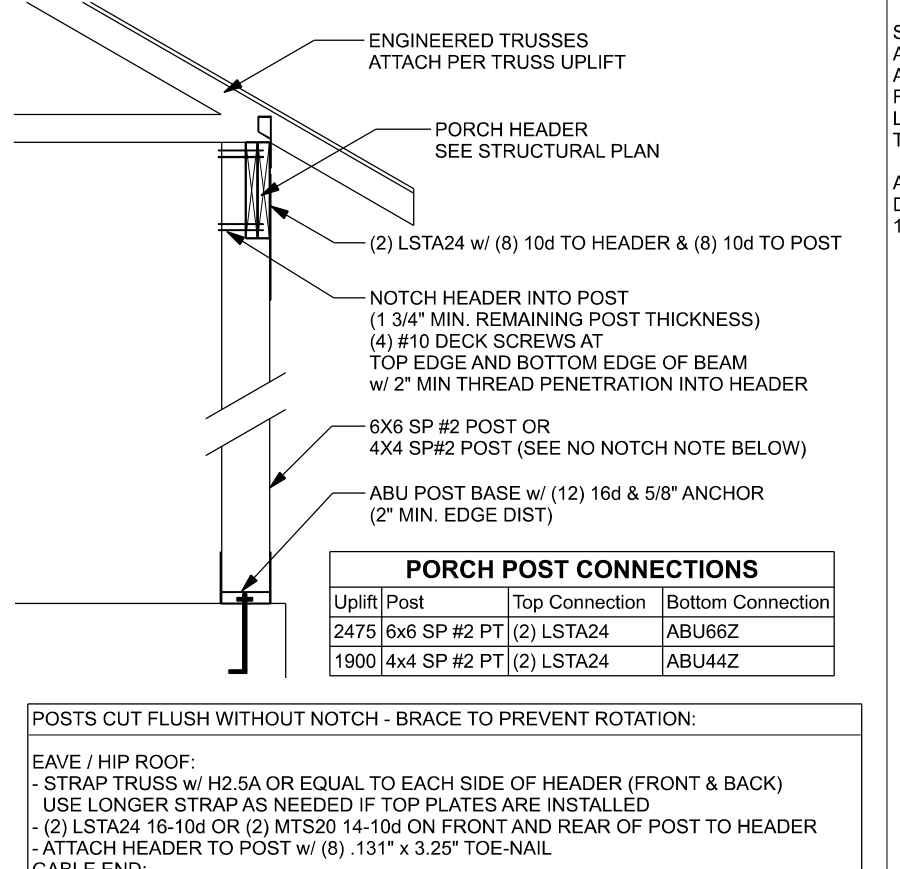
(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

CONNECTOR TABLE					
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter	
805	505	SDWC15600	-	-	
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"	
615	540	H2.5A	5-8d x 1 1/2"	5-8d x 1 1/2"	
1340	1015	H10A	9-10d x 1 1/2"	9-10d x 1 1/2"	
720	620	LTS12-20	6-10d x 1 1/2"	6-10d x 1 1/2"	
1000	860	MTS12-30	7-10d x 1 1/2"	7-10d x 1 1/2"	
1450	1245	HTS20-30	12-10d x 1 1/2"	12-10d x 1 1/2"	
Uplift SP Uplift SPF Strap Ties					
			To One Member	To Other Member	
1235	1235	LSTA21	8-10d	8-10d	
1640	1455	MSTA24	9-10d	9-10d	
1030	1030	CS20	7-10d	7-10d	
Uplift SP Uplift SPF Stud Plate Ties					
			To Stud	To Plate	
585	535	SP1	6-10d	4-10d	
1065	905	SP2	6-10d	6-10d	
771	771	LSTA24	10-10d	wrap under or over plate	
1235	1235	LSTA24	14-10d	wrap under or over plate	
Uplift SP Uplift SPF Holdowns @ Stemwall					
			To Stud / Post	To Anchor	
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD	
4235	3640	HTT4	18-16d x 12"	1/2"x12" Titen HD	
Uplift SP Uplift SPF Holdowns @ Mono					
			To Stud / Post	To Anchor	
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD	
4235	3640	HTT4	18-16d x 12"	1/2"x12" Titen HD	
Uplift SP Uplift SPF Post Bases @ Stemwall					
			To Post	To Anchor	
2400	ABU44Z	ABU44Z	12-16d	5/8"x12" Drill & Epoxy	
1975	ABU66Z	ABU66Z	12-16d	5/8"x12" Drill & Epoxy	
Uplift SP Uplift SPF Post Bases @ Mono					
			To Post	To Anchor	
2400	ABU44Z	ABU44Z	12-16d	5/8"x12" Drill & Epoxy	
1975	ABU66Z	ABU66Z	12-16d	5/8"x12" Drill & Epoxy	

(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS

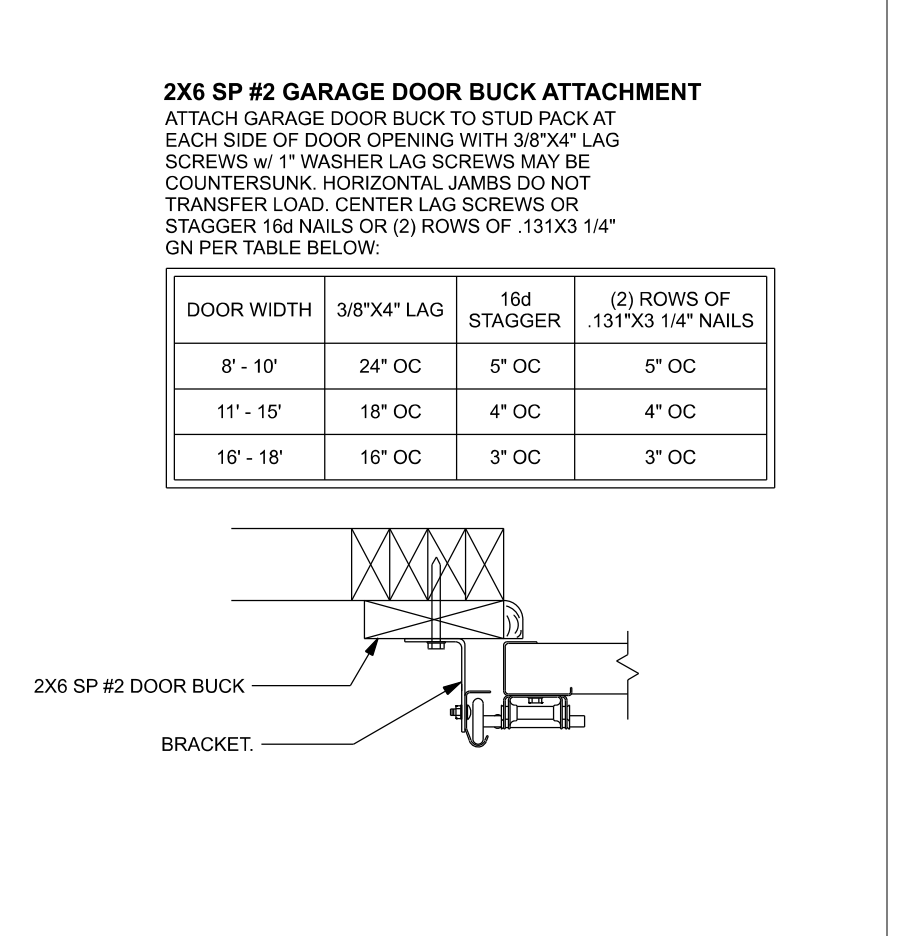


(TYP.) PORCH POST
ONE STORY WOOD



INTERIOR BEARING WALL UPLIFT STRAP OR SCREW TABLE			
Uplift on wall	Top Connection	Bottom Connection	Anchor Bolt Spacing
227 pcf	SP2 @ 32" OC	SP1 @ 32" OC	48" OC
454 pcf	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
681 pcf	(2) SDWC15600 @ 48" OC	(3) SDWC15450 @ 48" OC	48" OC
908 pcf	(2) SDWC15600 @ 32" OC	(3) SDWC15450 @ 32" OC	48" OC
1135 pcf	LSTA24, 14-10d @ 48" OC	LSTA24, 14-10d @ 48" OC	48" OC
1362 pcf	LSTA24, 14-10d @ 32" OC	LSTA24, 14-10d @ 32" OC	32" OC
1589 pcf	LSTA24, 14-10d @ 32" OC	LSTA24, 14-10d @ 32" OC	32" OC

(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCC. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BRACING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S DESIGN AND ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING TRUSSES. THE BUILDER SHALL FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP-2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 2500$ PSI.

WELED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, F8 x 8KSI, WELED WIRE REINFORCEMENT FABRIC W.W.M. CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 12 INCH TO 2 INCHES, DOSAGE AMOUNTS FROM 0.12 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS.

ANCHOR BOLTS: TO COMPLY WITH ASTM C1116, SUPPLIER TO PROVIDE ASTM C1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

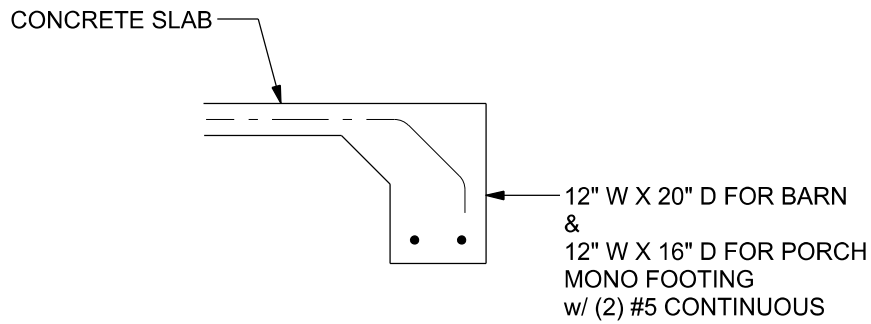
CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND ENGINEER'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI. ALL LAP SPICES 40" DB (25' FOR 45 BARS); UNO. ALL REINFORCEMENT SHALL BE DET

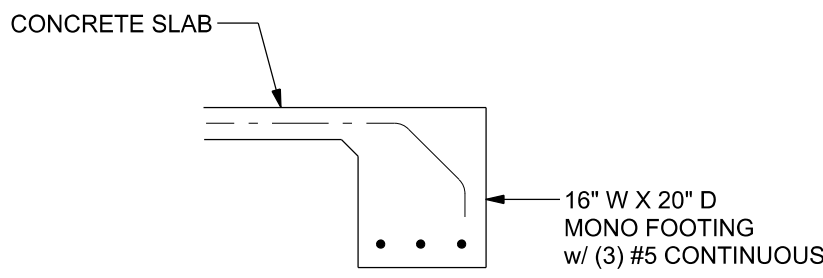
MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476; admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, S.S. 2.75x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM A615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

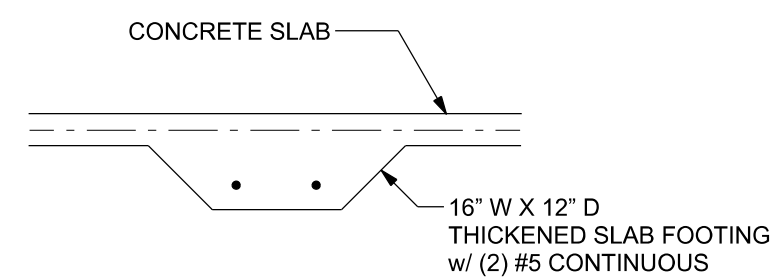
BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



**F1
S-2** MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



**F2
S-2** MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



**F3
S-2** INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"

FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

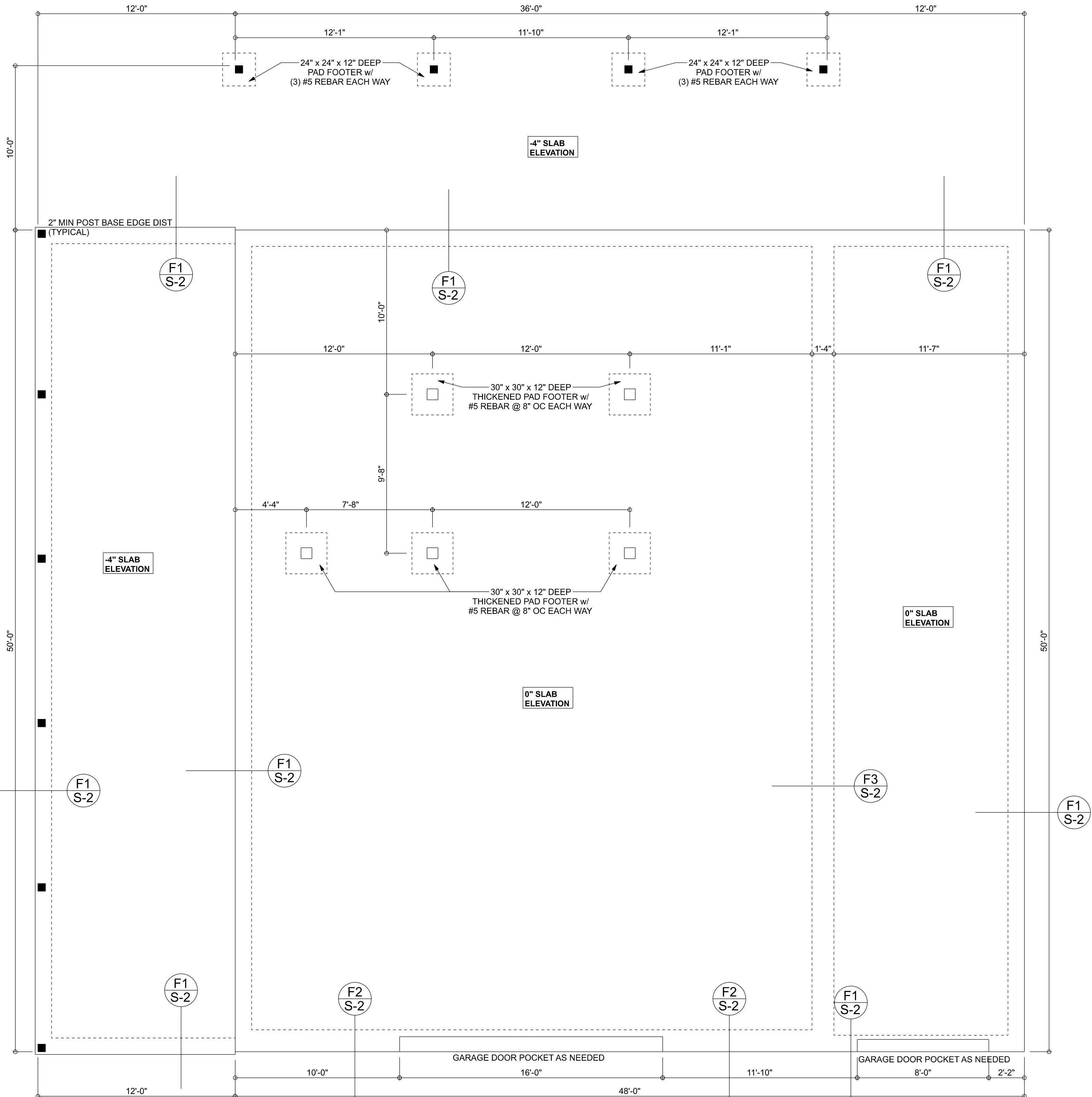
FOUNDATION NOTES

FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN

FN - 2 IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN

FN - 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X8-1 AT 14" WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL. POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITES TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITES TREATMENT METHOD CAN BE USED INSTAD)



Bryan Zecher Construction

Kyle & Jessica Dicks
Barn

PROJECT ADDRESS:
483 SW CR 240
Lake City, Florida 32025

FL PE 53915

This item has been digitally signed and sealed by Mark Disosway PE on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

C=US, O=Florida, dnQualifier=A01410C0000017E97DE07CA000746F0, CN=Mark d Disosway
2023-11-15 17:41:15

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disoswaydesign@gmail.com

JOB NUMBER:
231379

S-2
OF 3 SHEETS

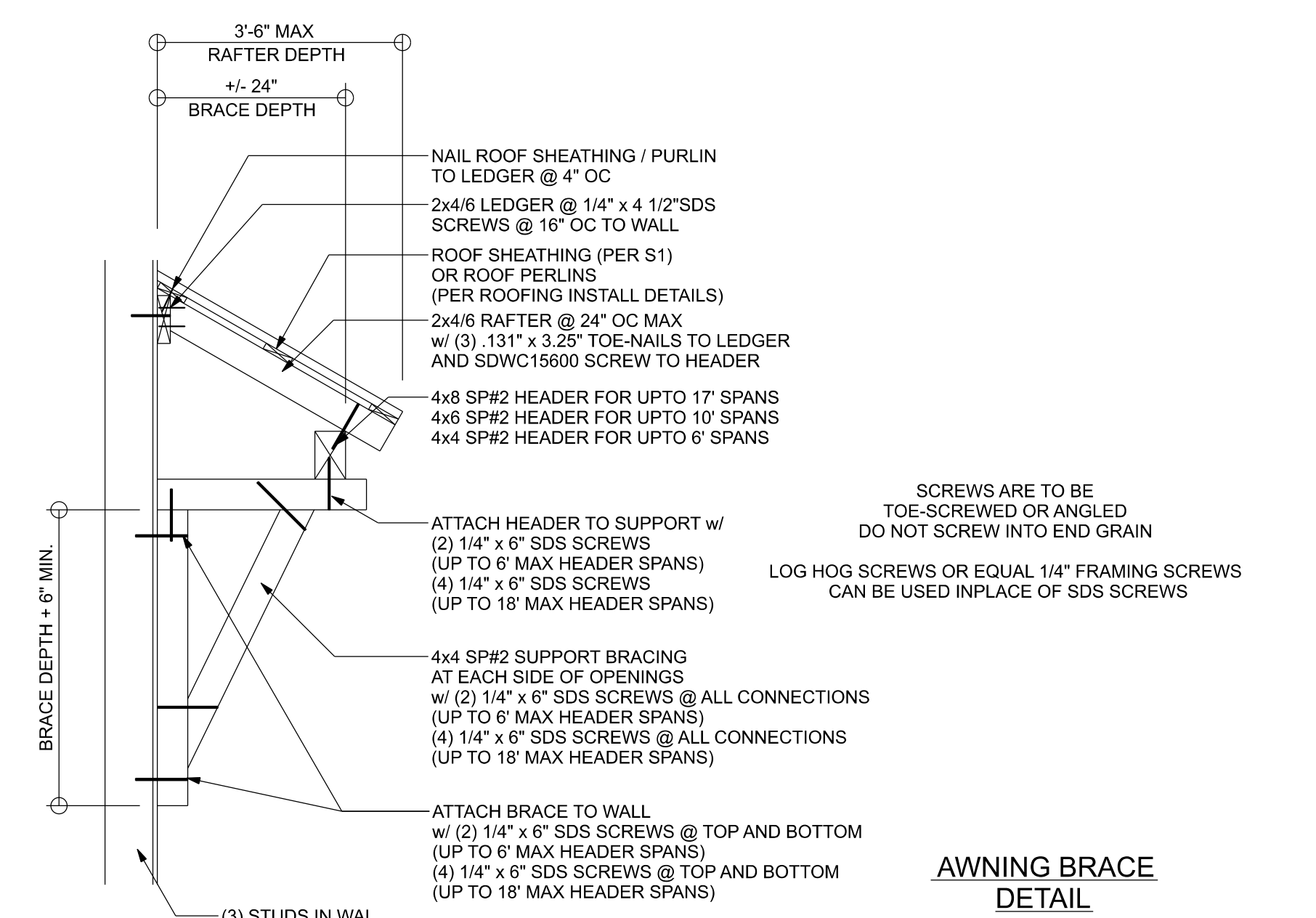


Diagram illustrating the components of a header/beam call-out:

- (2) 2X6X0', 1 J 1K**: The call-out itself, with arrows pointing to its parts:
 - 2**: NUMBER OF KING STUDS EACH SIDE OF BEAM
 - 2X6X0'**: NUMBER OF JACK STUDS EACH SIDE OF BEAM
 - 1 J**: SPAN OF HEADER
 - 1K**: SIZE OF HEADER MATERIAL
- HEADER/BEAM CALL-OUT (U.N.O.)**: The overall label for the call-out.
- NUMBER OF PLIES IN HEADER**: Indicated by an arrow pointing to the '1K' part of the call-out.

PROJECT ADDRESS:
463 SW CR 240
Lake City, Florida 32025

S-3
OF 3 SHEETS