

Received
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Code
Compliance
PLANS EXAMINER

- | | | | |
|--------|-------------------|-----------|---------------------------|
| A.B.v. | Anchor Bolt | F.B.C. | Florida Bldg. Code |
| Ab.v. | Above | Fin. Fir. | Finished Floor |
| A.C. | Air-Conditioning | F.G. | Fixed Glass |
| A/D | Adjustable | Fir. | Fire |
| A.F.F. | As Finished Floor | Fir. | Finished Floor |
| A.H.U. | Air Handler Unit | Fin. Sys. | Fire System |
| ALT. | Alternate | F.P.I. | Fireplace |
| B.C. | Base Cabinet | FL | Foot / Feet |
| B.F. | Bifold Door | Flg. | Footing |
| Bk.Sh. | Book Shelf | Flv. | Flush |
| B.N. | Beam | Galv. | Galvanized |
| BOT. | Bottom | G.C. | General Contractor |
| B.P. | Bypass door | G.F.I. | Ground Fault Interrupter |
| Brg. | Bearing | G.I.T. | Girdir Ties |
| Cr. | Circle | Hdr. | Header |
| Cgl. | Ceiling | Hgt. | Height |
| Col. | Column | HB | Hose Bibb |
| Comp. | A/C Compressor | Int. | Interior |
| C.T. | Ceramic Tile | K.Wall | Kneewall |
| D. | Decorative | K.S. | Knee Space |
| Dec. | Decorative | Laun. | Laundry |
| Dec. | Dedicated Outlet | Lav. | Lavatory |
| Disl. | Double | L.F. | Linear Ft. |
| Dia. | Diameter | L.T. | Laundry Tub |
| Disp. | Disposal | Mas. | Maximum |
| Dist. | Distance | Max | Maximum |
| D.S. | Drawer Stack | M.C. | Medicine Cabinet |
| D.V. | Drayer Vent | MDP | Master Distribution Panel |
| D.V. | Drainage | Mfg. | Manufacturer |
| E. | Each | Micro. | Microwave |
| E.W. | Each Way | Min | Minimum |
| Elev. | Electrical | M.L. | Microam |
| Ent. | Entire | Mirr. | Mirror |
| Exp. | Exterior | Mono | Monolithic |
| Exp. | Expansion | N.T.S. | Not to Scale |

FOUNDATIONS

SOIL TO BE COMPACTED TO AT LEAST 95%
OF MAX. DRY DENSITY AS DETERMINED BY
ASTM - 1557 (MODIFIED PROCTOR)

FOUNDATION INSPECTIONS

A FOUNDATION SURVEY SHALL BE PERFORMED AND A COPY OF THE SURVEY SHALL BE ON SITE FOR THE BUILDING INSPECTORS USE, OR ALL PROPERTY MARKERS SHALL BE EXPOSED AND A STRING STRECHED FROM MARKER TO MARKER TO VERIFY REQUIRED SETBACKS.

CAST IN PLACE CONCRETE

1. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 3000 PSI. A SLUMP OF 6" PLUS OR MINUS 1". AND HAVE 2 TO 5% AIR ENTRAINMENT, AND A MAXIMUM WATER/CEMENT RATIO OF 0.63
2. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A-815 GRADE 60.
3. WIRE FABRIC SHALL CONFORM TO ASTM A-185. WWF SHALL BE LAPPED AT LEAST 6" AND CONTAIN AT LEAST ONE CROSS WIRE WITHIN THE 6".
4. HOOKS SHALL BE PROVIDED AT DISCONTINUOUS ENDS OF ALL TOP BARS OF BEAMS.
5. HORIZONTAL FOOTING BARS SHALL BE BENT 1"0" AROUND CORNERS OR CORNER BARS WITH A 2"0" LAP PROVIDED.
6. MINIMUM LAP SPACES ON ALL REINFORCING BAR SPLICES SHALL BE 40x BAR DIAMETERS TYP.
7. CONCRETE COVER MIN. 3" WHEN EXPOSED TO EARTH OR 1 1/2" TO FORM

MASONRY WALL CONST.

1. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2, CONFORMING TO ASTM C90, WITH A MINIMUM NET COMPRESSIVE STRENGTH OF 1900 PSI ($f_m = 1350$ PSI)
2. MORTAR SHALL BE TYPE "M" OR "S", CONFORMING TO ASTM C270.
3. COURSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI SLUMP 8" TO 11".
4. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS WITH THE CELLS FILLED WITH COARSE GROUT.
5. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM OF THE CELL AT A MAXIMUM SPACING OF 192 BAR DIAMETERS. REINFORCEMENT SHALL BE PLACED IN THE CENTER OF THE MASONRY CELL TYPICAL UNLESS OTHERWISE NOTED.
6. REINFORCING STEEL SHALL BE APPLIED A MINIMUM OF 40 BAR DIAMETERS, UNLESS OTHERWISE NOTED ON THE DRAWINGS
7. GROUT STOPS SHALL BE PROVIDED BELOW BOND BEAM, PLASTIC SHEETING, METAL LATH STRIP OR CAVITY CAPS MAY BE USED TO PREVENT FLOW GROUT INTO THE CELLS BELOW, THE USE OF FELPAPER AS A STOP IS PROHIBITED.

WOOD CONSTRUCTION

1. WOOD CONSTRUCTION SHALL CONFORM TO THE NFPA "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", LATEST EDITION.
2. ALL EXTERIOR WOOD STUD WALLS, BEARING WALLS, SHEAR WALLS AND CONC. STRUCTURAL WOOD FRAMING MEMBERS, (I.E. JACKS OR CABLE END BRACING) SHALL BE EITHER SOUTHERN PINE, OR S.P.F. NUMBER 2 GRADE SHALL BE USED REGARDLESS OF SPECIES.
3. ANY WOOD FRAME INTERIOR BEARING WALL STUDS THAT HAVE HOLES IN THE CENTER OF THE STUD UP TO 1" DIA. SHALL HAVE STUD PROTECTION
FOR STUDS FOR ALL HOLES EXCEEDING 1" DIA.
FOR PLUMBING LINES, STUDS SHALL BE REPAIRED WITH SIMPSON HSS2 STUD SHOES, TYP., U.N.O.

WOOD FRAMING INSPECTION

ALL PLUMBING, ELECTRICAL, AND MECHANICAL ROUGH-INS MUST BE COMPLETE, INSPECTED AND APPROVED BEFORE REQUESTING FRAMING INSPECTION.

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PREFABRICATED WOOD TRUSSES

1. ALL PREFABRICATED WOOD TRUSSES SHALL BE SECURELY FASTENED TO THEIR SUPPORTING WALLS OR BEAMS WITH HURRICANE CLIPS OR ANCHORS.
2. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENERS" RECOMMENDED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.
3. TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPORTIONED (WITH A MAXIMUM ALLOWABLE STRESS INCREASE FOR LOAD DURATION OF 25%) TO WITHSTAND THE LIVE LOADS GIVEN IN THE NOTES AND TOTAL DEAD LOAD.
4. BRIDGING FOR PRE-ENGINEERED TRUSSES SHALL BE AS REQUIRED BY THE TRUSS MANUFACTURER UNLESS NOTED ON THE PLANS.
5. TRUSS ELEVATIONS AND SECTIONS ARE FOR GENERAL CONFIGURATION OF TRUSSES ONLY. WEB MEMBERS ARE TO BE SHOWN, BUT SHALL BE DESIGNED BY THE TRUSS MANUFACTURER IN ACCORDANCE WITH THE FOLLOWING DESIGN LAOS:
6. DESIGN SPECIFICATIONS FOR LIGHT WEIGHT METAL PLATE CONNECTED WOOD TRUSSES PER THE TRUSS PLATE INSTITUTE TPI LATEST EDITION.
7. PRE-ENGINEERED WOOD TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH SPECIFIED LOADS AND GOVERNING CODES. SUBMITTALS SHALL INCLUDE TRUSS FRAMING PLANS AND DETAILS SHOWING MEMBER SIZES, BRACING, JOINT CONNECTIONS, MEMBER LOCATIONS, AND ANCHORING, PERMANENT BRACING AND/OR BRIDGING AS REQUIRED FOR ERECTION AND FOR THE PERMANENT STRUCTURE. EACH SUBMITTAL SHALL BE SIGNED AND SEALED BY A FLORIDA REGISTERED STRUCTURAL ENGINEER. SUBMIT 3 COPIES FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.
8. THE TRUSS MANUFACTURER SHALL DETERMINE ALL SPANS, WORKING POINTS, BEARING POINTS, AND SIMILAR CONDITIONS. TRUSS SHOP DRAWINGS SHALL SHOW ALL TRUSSES, ALL BRACING MEMBERS, AND ALL TRUSS TO TRUSS HANGERS.

UPLIFT CONNECTORS

1. UPLIFT CONNECTORS SUCH AS HURRICANE CLIPS, TRUSS ANCHORS AND ANCHOR BOLTS ARE ONLY REQUIRED ON MEMBERS IN WALLS THAT ARE EXPOSED TO UPLIFT FORCES. INTERIOR LOAD BEARING WALLS ARE NOT ALWAYS EXPOSED TO UPLIFT FORCES. THE MEMBERS OF THESE WALLS WOULD NOT NEED TO HAVE CONNECTORS APPLIED. PLEASE CONSULT THE TRUSS ENGINEERING FOR THE LOCATION OF THESE WALLS.

FIELD REPAIR NOTES

1. MISSED LINTEL STRAPS FOR MASONRY CONSTRUCTION MAY BE SUBSTITUTED W/ (1) "SIMPSON MTSMT16 TWIST STRAP W/ (4) 1/4" X 2 1/4" DIA. TIE BARS TO THE BOWS BEAM BLOCK ANCHOR 108 TO THE JOISTS FOR UPLIFTS OF 1000 LBS. OR LESS. USE (2) FOR 2000 LBS. OR LESS. OTHERS MAY BE SUBSTITUTED ON A CASE BY CASE BASIS.
2. MISSED "J" BOLTS FOR WOOD BEARING WALLS MAY BE SUBSTITUTED W/ 1/2" DIA. ANCHOR BOLTS SET IN 3/4" DIA. X 6" DEEP UNITEK "PROPOXY" 300 ADHESIVE BINDER FOLLOWING ALL MANUFACTURERS RECOMMENDATIONS (OR 1/2" X 6" RAWL STUD EXPANSION ANCHORS.)

3. REGARDING MISSED REPAIR IN VERTICAL FILLED CELLS:
DRILL A 3/4" DIAMETER HOLE 6" DEEP AT THE LOCATION OF THE OMITTED REPAIR, AND INSTALL A 32" LONG #5 BAR INTO THE EPOXY FILLED HOLE. USE A TWO PART EPOXY EMBEDMENT EPOXY (SIMPSON "EPOXY THE SET", OR HILTI "2 PART" EMBEDMENT EPOXY), MIXED PER MANUFACTURER'S INSTRUCTIONS. ASSURE THAT ALL DUST AND DEBRIS FROM DRILLING ARE REMOVED FROM THE HOLE BY BRUSHING AND USING COMPRESSED AIR. THEN APPLY THE EPOXY. AFTER THE EPOXY TO CURE TO MANUFACTURER'S SPECIFICATIONS, THEN FILL THE CELL IN THE NORMAL WAY DURING BOND BEAM POUR.
4. HURRICANE STRAPS MAY BE SUBSTITUTED WITH A STRAP OF GREATER HOLD-DOWN VALUE, OR GREATER UP-LIFT VALUE IN THE FIELD. VERIFY, PROVIDED ALL MANUFACTURER'S INSTALLATION INSTRUCTIONS ARE FOLLOWED.
5. FOR MORTAR JOINTS LESS THAN 1/4" PROVIDE (1) #5 VERT. IN CONC. FILLED CELL EACH SIDE OF THE JOINT (BAR DOES NOT HAVE TO BE CONT. TO FOOTING)

CODES:	FLORIDA BUILDING CODE, 2010 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318-05) SPECIFICATIONS FOR STRUCTURAL CONCRETE BUILDINGS (ACI 301-05) BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530-05) NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, 2001 EDITION APA PLYWOOD DESIGN SPECIFICATION		
LIVE LOADS:	ROOF RESIDENTIAL FLOOR, UNLESS OTHERWISE INDICATED BALCONIES STAIRS LIGHT PARTITIONS (DEAD LOAD), U.N.O.	20 PSF (REDUCIBLE) 40 PSF 40 PSF 40 PSF 20 PSF	
WIND LOADS: (F.B.C.)	WIND LOADS BASED ON FBC, SECTION 1609 WIND VELOCITY: 125 M.P.H., USE FACTOR: 1.0		
CONCRETE STRENGTH @ 28 DAYS	ALL CONCRETE UNLESS OTHERWISE INDICATED PEA GRAVEL CONCRETE FOR MASONRY CELLS ONLY (DO NOT USE FOR CONCRETE COLUMNS OR TIE BEAMS)	3000 PSI 3000 PSI	
REINFORCING:	WELDED WIRE FABRIC SHALL CONFORM TO ALL REINFORCING BARS ALL STIRRUPS AND TIES	ASTM A185 ASTM A615-40 40,000 PSI ASTM A615-40 40,000 PSI	
CONCRETE MASONRY UNITS:	ASTM C90-99b, STANDARD WEIGHT UNITS, fm=1500 PSI MORTAR TYPE "S" 1800 PSI CONCRETE GROUT: 3000 PSI CONTINUOUS MASONRY INSPECTION IS REQUIRED DURING CONSTRUCTION		
STRUCTURAL STEEL:	ALL STRUCTURAL AND MISCELLANEOUS STEEL A36 36,000 PSI, U.N.O SHOP AND FIELD WELDS: E70XX ELECTRODES ALL BOLTS CAST IN CONCRETE: ASTM A36 OR ASTM A-307		
WOOD FRAMING:	BEAMS, RAFTERS, JOIST, PLATES, ETC. U.N.O. NO. 2 SOUTHERN YELLOW PINE (19% M.C.) ROOF DECK: PLYWOOD C-C/C-D, EXTERIOR, OR OSB FLOOR SHEATHING: T&G A-C GROUP 1 APA RATED (48/24) WALL SHEATHING: PLYWOOD C-C/C-D, EXTERIOR OR OSB VERSA LAM BEAM Fb = 2900 PSI (2.0E) WOOD COLS. PARALLAM 2.0E U.N.O.		
WOOD ROOF TRUSSES:	DESIGN LOADS: TOP CHORD LIVE AND DEAD LOAD: BOTTOM CHORD DEAD LOAD: TOTAL:	30 PSF 10 PSF 40 PSF	
	SEE DRAWINGS FOR SPECIAL CONCENTRATED LOADS. DESIGN FOR NEW WIND UPLIFT AS PER SPECIFIED CODES, DEDUCTING A MAXIMUM OF 5 P.S.F. DEAD LOAD, BUT NOT EXCEEDING ACTUAL DEAD LOAD.		
WOOD FLOOR TRUSSES:	DESIGN LOADS: DEAD LOAD: LIVE LOAD: TOTAL:	15 PSF 40 PSF 55 PSF	
SOIL BEARING VALUE:	ASSUMED ALLOWABLE SOIL BEARING PRESSURE AFTER COMPACTION: 2,000 PSF SEE SOILS REPORT AND SPECIFICATIONS FOR COMPACTION REQUIREMENTS IF SOIL CONDITIONS IN THE PROJECT DO NOT MEET OR EXCEED THE CAPACITY THE GENERAL CONTRACTOR SHALL CONTACT THE ENGINEER PRIOR TO FOUNDATION POUR FOR VERIFICATION OF FOUNDATION DESIGN.		

ALL WIND LOADS ARE IN ACCORDANCE WITH SECTION 1609, FLORIDA BUILDING CODE, 2010												
BASIC WIND SPEED		125 MPH										
IMPORTANCE FACTOR		1.00										
BUILDING CATEGORY		II										
EXPOSURE		B										
INTERNAL PRESSURE COEFFICIENT		+/- 0.18										
TYPE OF STRUCTURE		ENCLOSED										
MWFRS PER ASCE 7 DESIGN WIND PRESSURES WORST CASE		Zone 1 - Windward Wall						+26.5 psf				
		Zone 2 and 3 - Windward and Leeward Roof						-29.1 psf				
		Zone 2 - Sloped Windward Roof						-29.1 psf				
		Zone										
		3 - Leeward Roof						-29.1 psf				
		4 - Leeward Wall						-18.6 psf				
		5 & 6 Sidewalls						-23.9 psf				
		Zone 7 - Overhang						+20.9 psf				
COMPONENTS AND CLADDING PER ASCE 7 DESIGN WIND PRESSURES WORST CASE (PSF)		Roof	10 sf		20 sf		50 sf		100 sf			
			pos. neg.		pos. neg.		pos. neg.		pos. neg.			
			Zone 1		18.06 -28.70		16.50 -27.88		14.34 -26.84		12.78 -30.16	
			Zone 2		18.06 -49.96		16.50 -53.12		14.34 -46.96		12.78 -44.27	
			Zone 3		18.06 -73.9		16.50 -69.14		14.34 -62.74		12.78 -66.88	
		Wall	Zone 4		31.38 -34.04		29.94 -32.62		28.08 -30.76		26.72 -29.32	
			Zone 5		31.38 -42.00		29.94 -39.20		28.08 -35.40		26.72 -32.62	

a: 10% of least horizontal dim. or 0.4h, whichever is smaller, but not less than either 4% of least horizontal dimension or 3 ft.
h: mean roof height, in feet.

ELIZABETH WILLIAMSON RESIDENCE

GENERAL NOTES SHEET

P.O. BOX 860125
ST. AUGUSTINE, FL. 32086
(904) 429-7536
C.O.A. # 00008701



COASTAL
ENGINEERING
AND TESTING, INC.

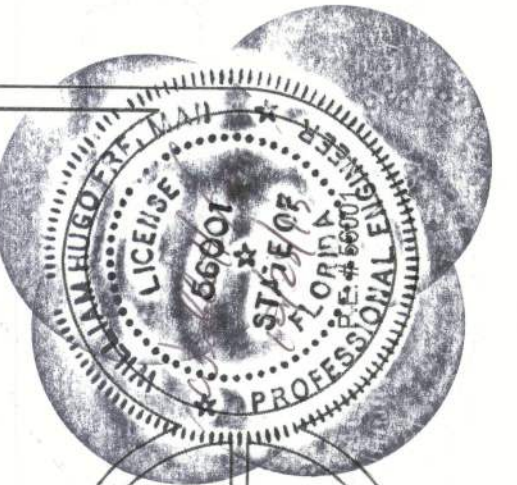
DATE 10/27/13	DRAWN BY W.H.F.
	APPROVED W.H.F.

REVISIONS

SHEET A-1

OF 7

PROJECT NO.
13.R021



ELIZABETH WILLIAMSON RESIDENCE

FLOOR PLAN AND ELEVATIONS

P.O. BOX 860125
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(904) 429-7536
COA #0008701



DATE
10/27/13

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W.H.F.

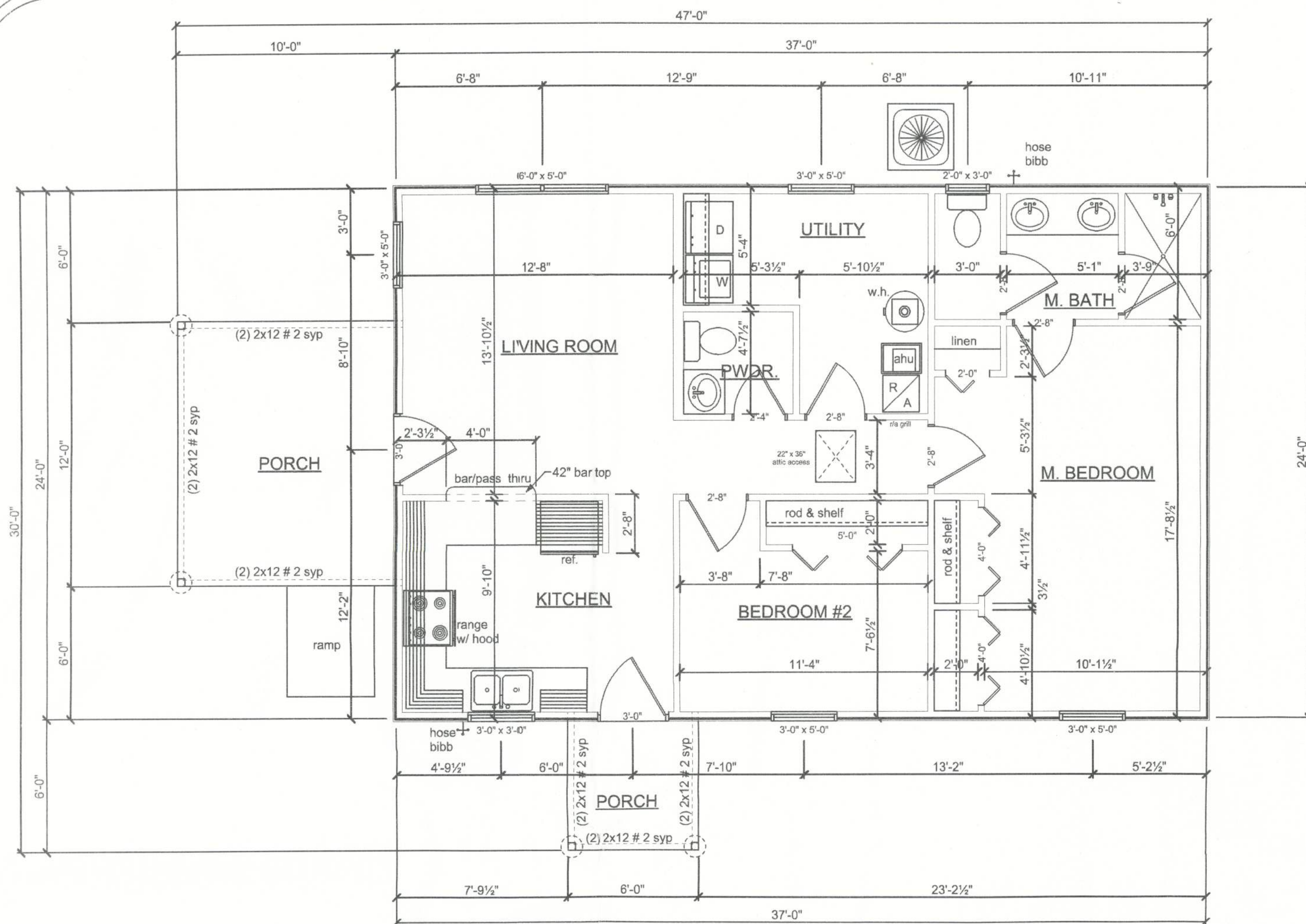
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REVISIONS

SHEET
A-2

OF
7

PROJECT NO.
13.R021



FLOOR PLAN
SCALE: 1/4" = 1'-0"

AREA SUMMARY

CONDITIONED LIVING	888 SF
PORCHES	156 SF
TOTAL	1,044 SF

CONSTRUCTION DOCUMENTS:

THE CUSTOMER IS RESPONSIBLE FOR DELIVERING THE REQUIRED SETS OF CONSTRUCTION DOCUMENTS TO THE PERMIT ISSUING AUTHORITY FOR THE ISSUANCE OF CONSTRUCTION PERMITS. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR REVIEWING THE PLANS AND VERIFYING ALL EXISTING CONDITIONS, ELEVATIONS, AND DIMENSIONS PRIOR TO COMMENCING CONSTRUCTION INCLUDING FABRICATION. ALL DISCREPANCIES SHALL BE REPORTED TO THE ARCHITECT/ENGINEER FOR RESOLUTION.

DO NOT SCALE THESE PLANS:

AMPLE DIMENSIONS ARE SHOWN ON THE PLANS TO LOCATE ALL ITEMS. SIMPLE ARITHMETIC MAY BE USED TO DETERMINE THE LOCATION OF THOSE ITEMS NOT DIMENSIONED.

CHANGES TO PLAN SETS:

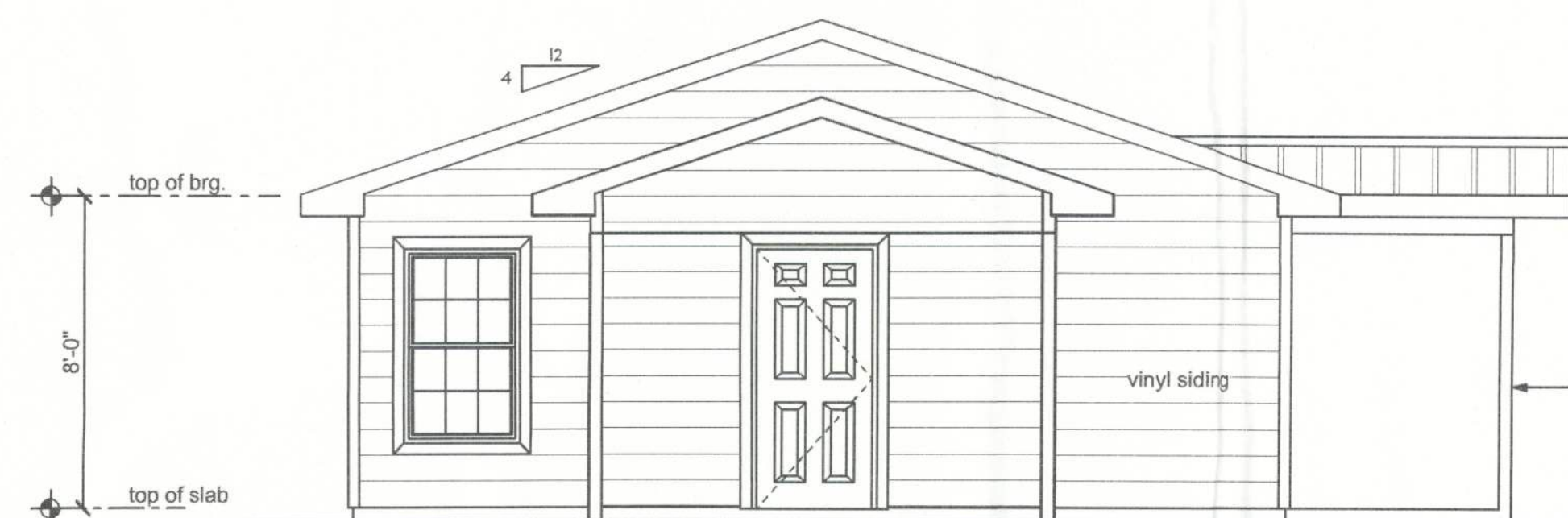
PLEASE DO NOT MAKE ANY STRUCTURAL CHANGES TO THESE PLANS WITHOUT CONSULTING WITH THE ARCHITECT/ENGINEER. THE OWNER SHALL ASSUME ANY AND ALL LIABILITY FOR STRUCTURAL DAMAGE RESULTING FROM CHANGES MADE TO THE PLANS OR BY SUBSTITUTION OF MATERIALS DIFFERENT FROM SPECIFICATIONS ON THE PLANS.



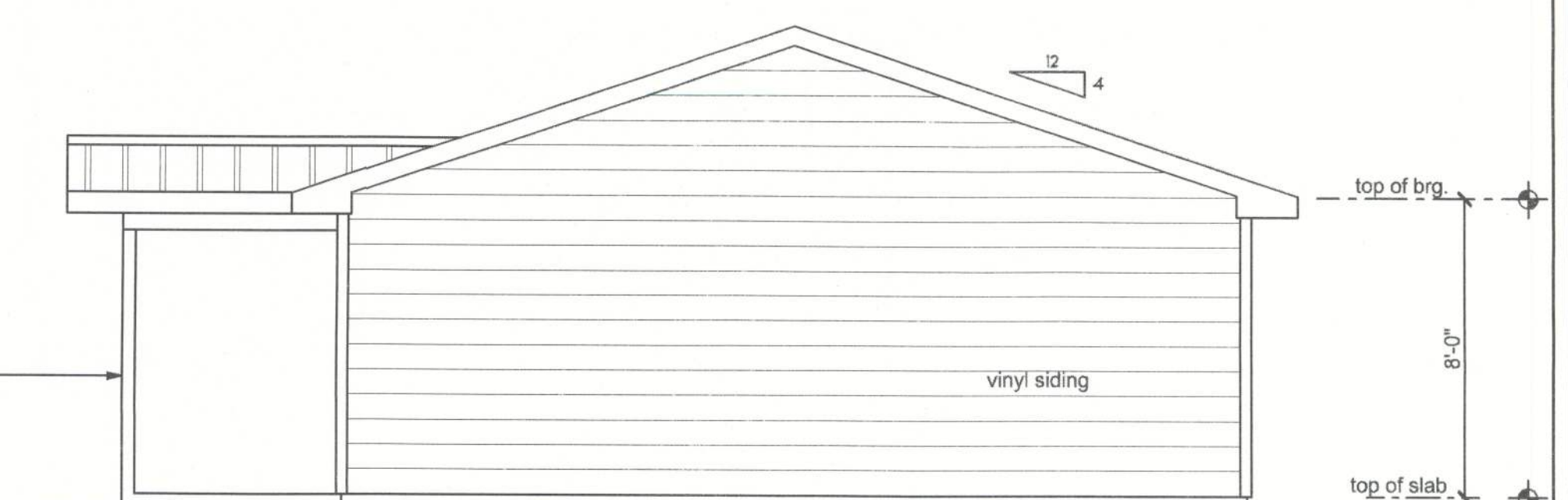
FRONT ELEVATION
SCALE: 1/4" = 1'-0"



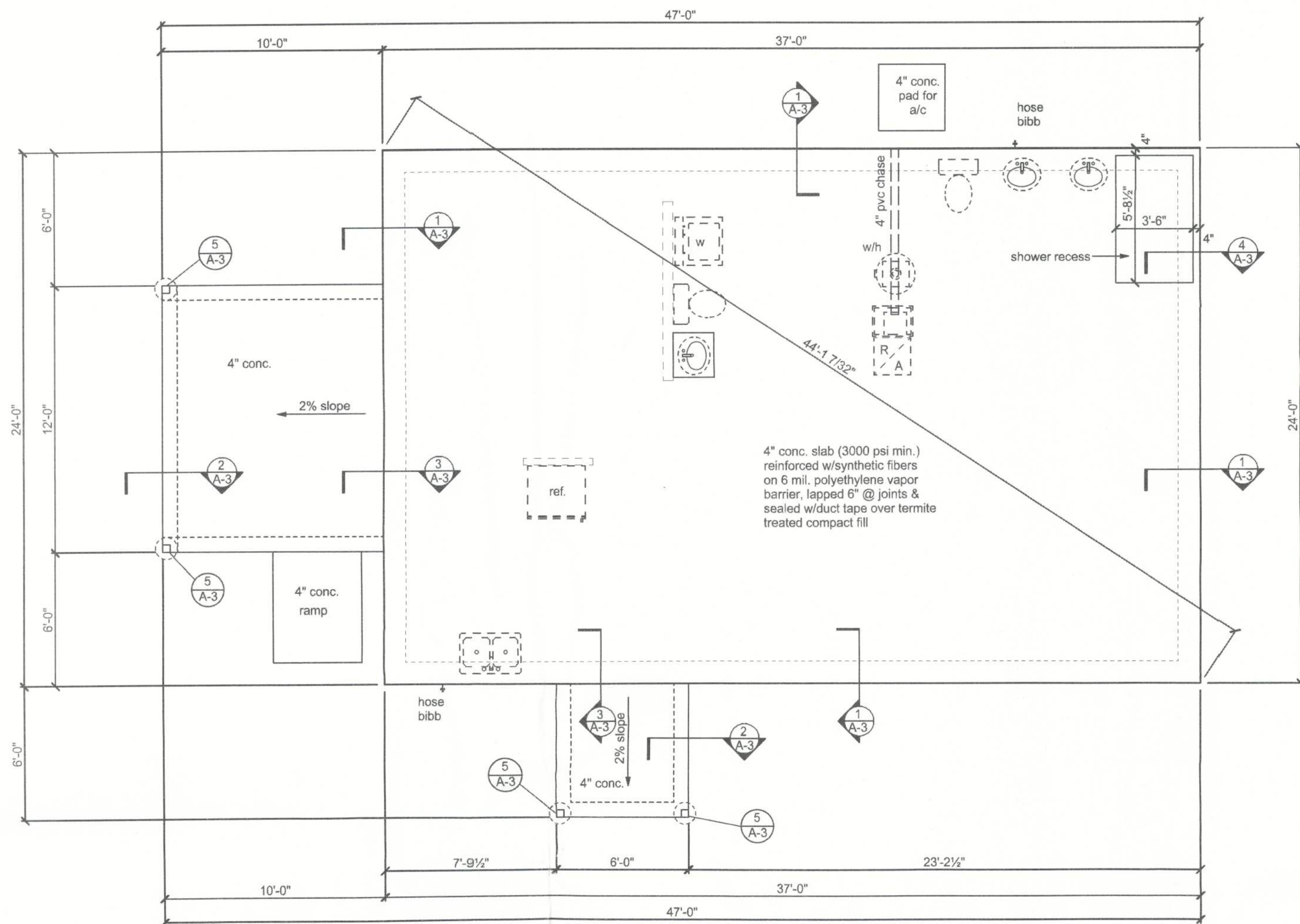
REAR ELEVATION
SCALE: 1/4" = 1'-0"



LEFT ELEVATION
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



FOUNDATION PLAN
SCALE: 1/4"=1'-0"

FOUNDATION NOTES:

CONCRETE:
CONCRETE SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS.

GALVANIZATION:
METAL ACCESSORIES FOR USE IN EXTERIOR WALL CONSTRUCTION AND NOT DIRECTLY EXPOSED TO THE WEATHER SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A 153, CLASS B-2. METAL PLATE CONNECTORS, SCREWS, BOLTS AND NAILS EXPOSED DIRECTLY TO THE WEATHER SHALL BE STAINLESS STEEL OR HOT DIPPED GALVANIZED.

REINFORCING STEEL:
THE REINFORCING STEEL SHALL BE MINIMUM GRADE 60

REINFORCEMENT MAY BE BENT IN THE SHOP OR THE FIELD PROVIDED:

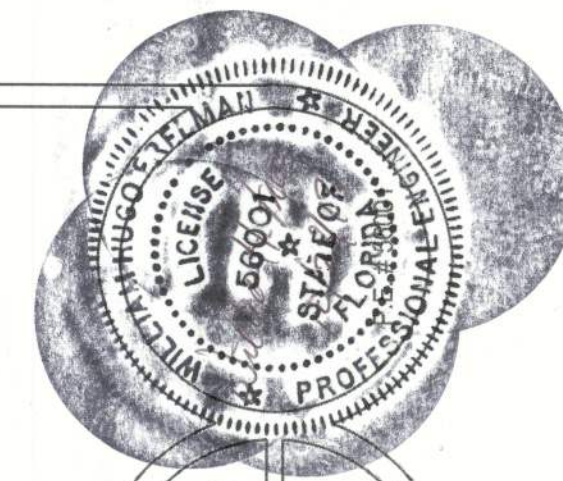
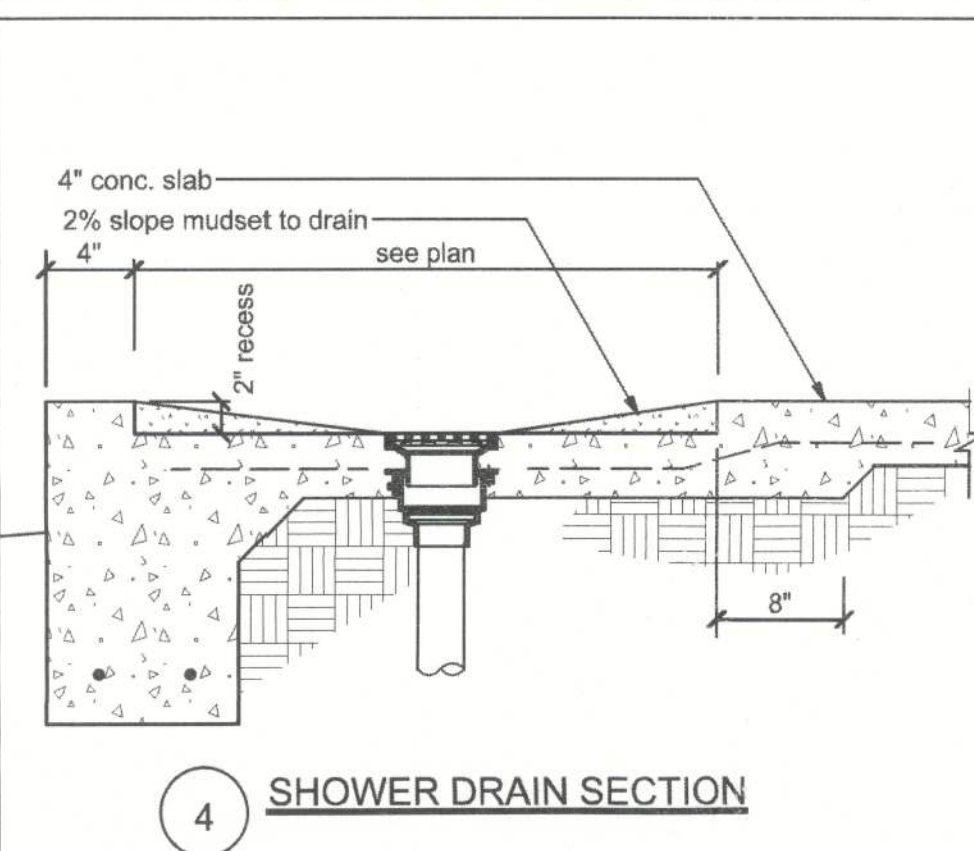
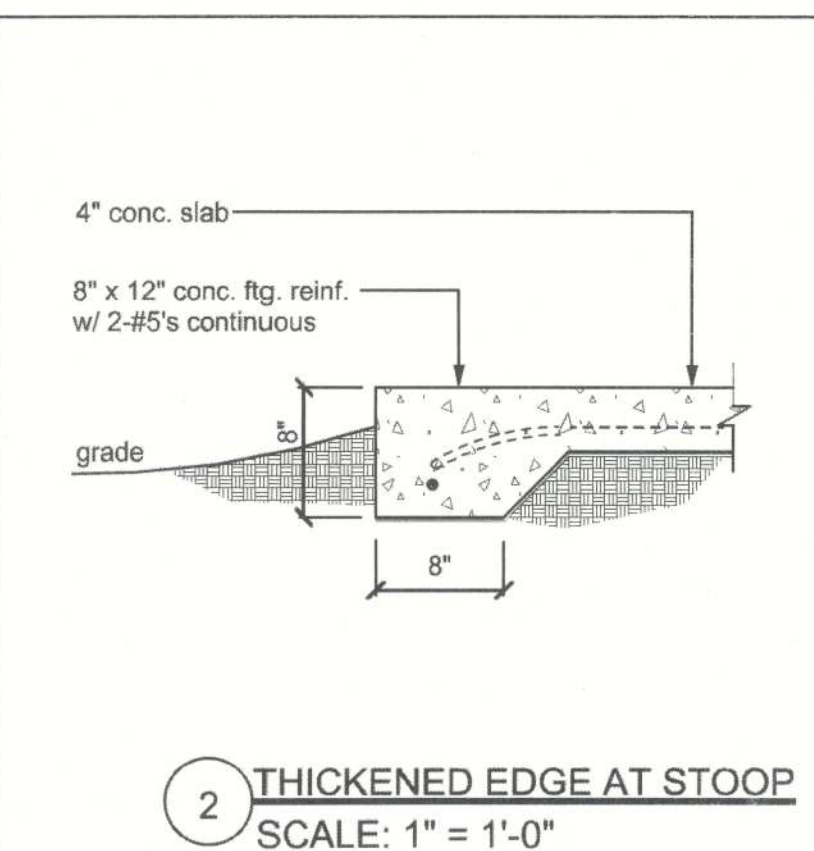
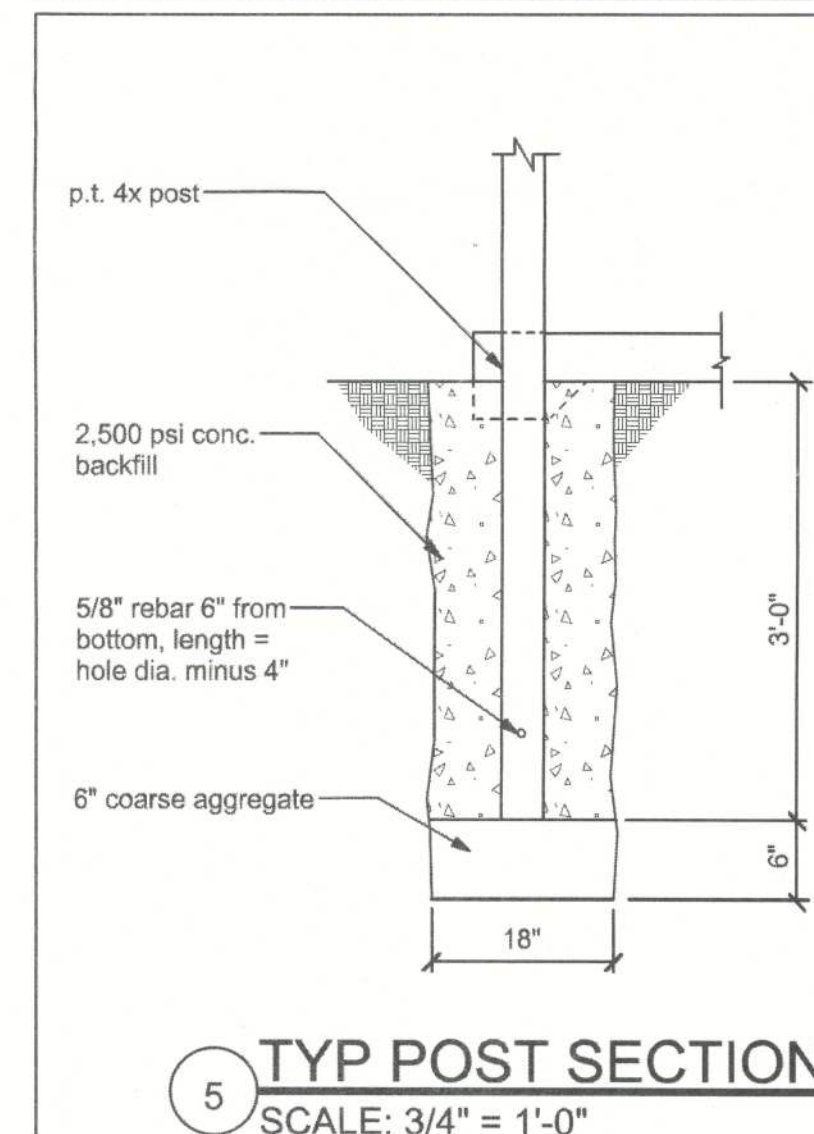
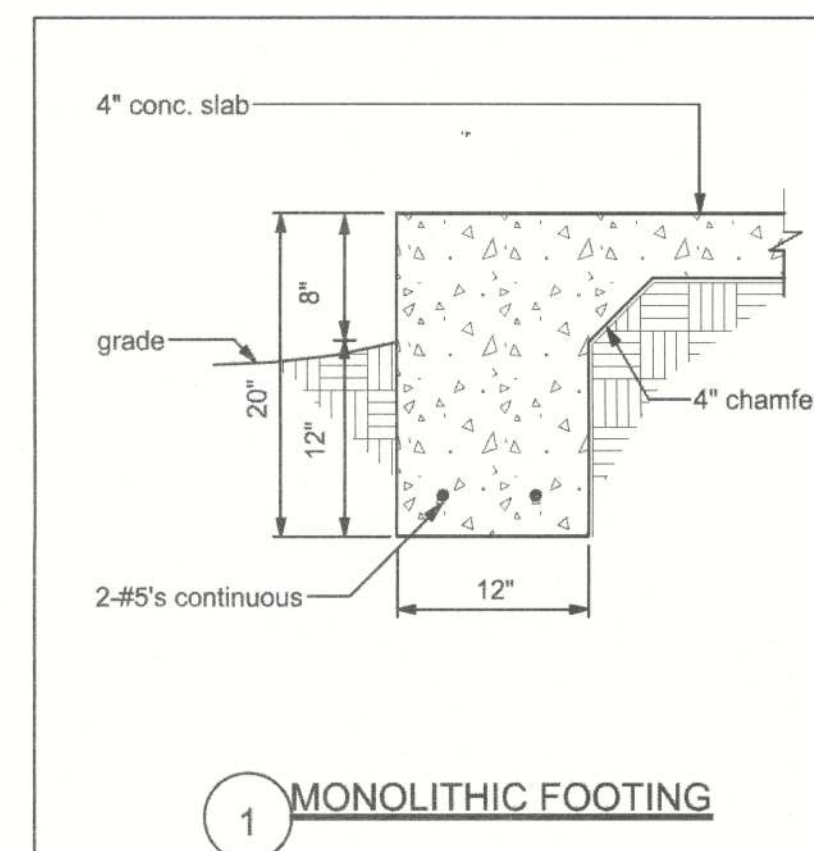
1. ALL REINFORCEMENT IS BENT COLD.
2. THE DIAMETER OF THE BEND, MEASURED ON THE INSIDE OF THE BAR, IS NOT LESS THAN SIX-BAR DIAMETERS AND
3. REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT.

EXCEPTION: WHERE BENDING IS NECESSARY TO ALIGN DOWEL BARS WITH A VERTICAL CELL, BARS PARTIALLY EMBEDDED IN CONCRETE SHALL BE PERMITTED TO BE BENT AT A SLOPE OF NOT MORE THAN 1 INCH OF HORIZONTAL DISPLACEMENT TO 6 INCHES OF VERTICAL BAR LENGTH.

COVER OVER REINFORCING STEEL
FOR FOUNDATIONS, MINIMUM CONCRETE COVER OVER REINFORCING BARS SHALL BE:
3 INCHES IN FOUNDATIONS WHERE THE CONCRETE IS CAST AGAINST AND PERMANENTLY IN CONTACT WITH THE EARTH OR EXPOSED TO THE EARTH OR WEATHER AND 1 1/2 INCHES ELSEWHERE. REINFORCING BARS EMBEDDED IN GROUTED CELLS SHALL HAVE A MINIMUM CLEAR DISTANCE OF 1/4 INCH FOR FINE GROUT OR 1/2 INCH FOR COARSE GROUT BETWEEN REINFORCING BARS AND ANY FACE OF A CELL. REINFORCING BARS USED IN MASONRY WALLS SHALL HAVE A MASONRY COVER (INCLUDING GROUT) OF NOT LESS THAN 2 INCHES FOR MASONRY UNITS WITH FACE EXPOSED TO EARTH OR WEATHER 1 1/2 INCHES FOR MASONRY UNITS NOT EXPOSED TO EARTH OR WEATHER

NOTE:

CONCRETE SLABS, WALKS, DRIVES AND PATIOS CAN DEVELOP HAIRLINE CRACKS THAT WILL NOT AFFECT THE STRUCTURAL INTEGRITY OF THE BUILDING. THERE IS NO KNOWN METHOD OF ELIMINATING THIS CONDITION, WHICH IS CAUSED BY THE CHARACTERISTICS OF EXPANSION AND CONTRACTION THAT OCCURS IN ALL CONCRETE APPLICATIONS. IT DOES NOT AFFECT THE STRENGTH OF THE BUILDING, AND IT IS NOT A CONDITION COVERED BY ANY WARRANTY.



ELIZABETH WILLIAMSON RESIDENCE FOUNDATION PLAN

P.O. BOX 860125
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C.O.A.# 00088701



DATE
10/27/13

DRAWN BY
W.H.F.

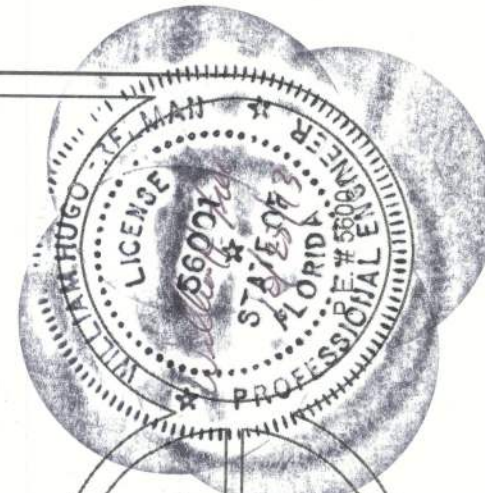
APPROVED
W.H.F.

REVISIONS

SHEET
A-3

OF
7

PROJECT NO.
13.R021



ELIZABETH WILLIAMSON RESIDENCE
ROOF PLAN

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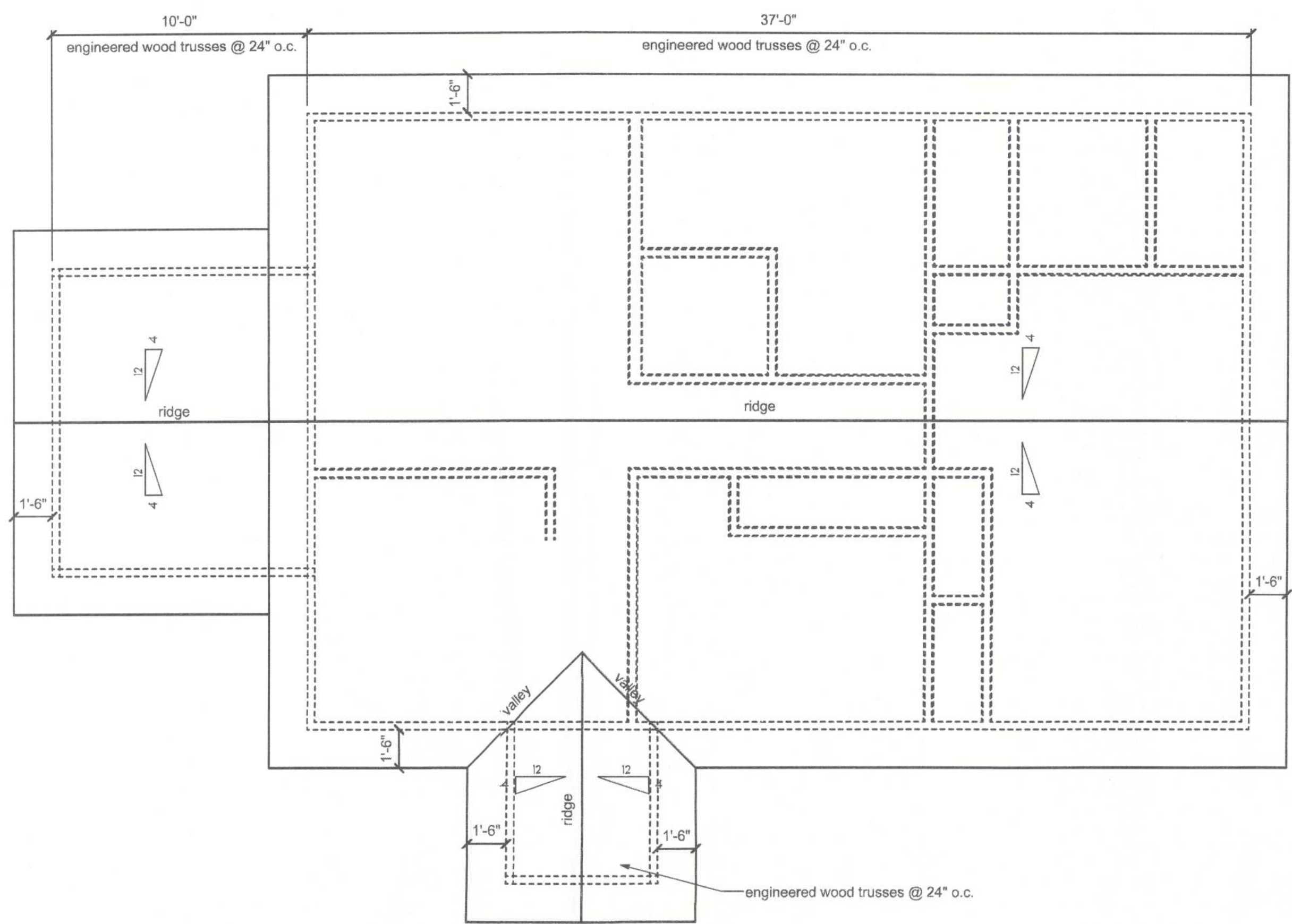
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REVISIONS

SHEET A-4

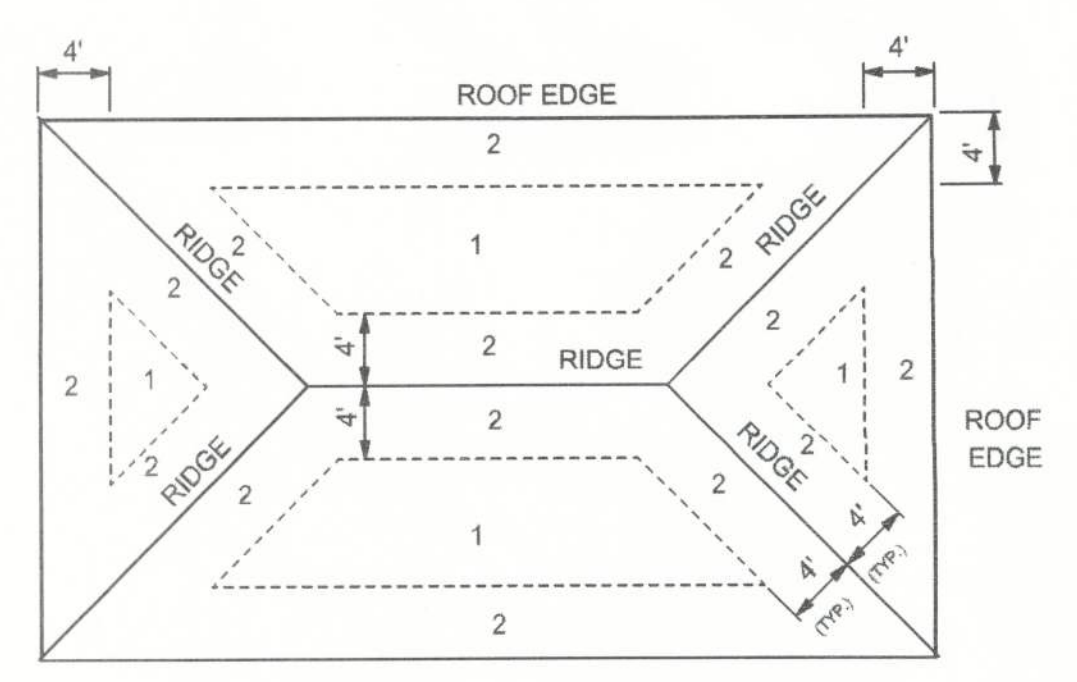
OF 7

PROJECT NO. 13.R021

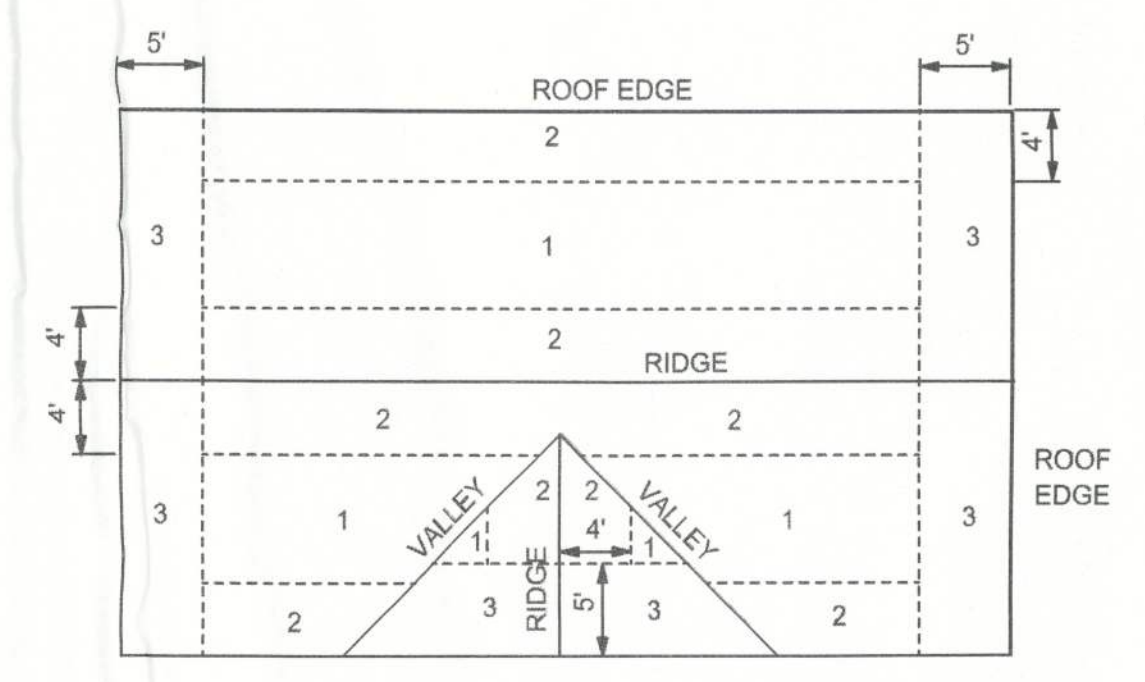


ROOF PLAN
SCALE: 1/4" = 1'-0"

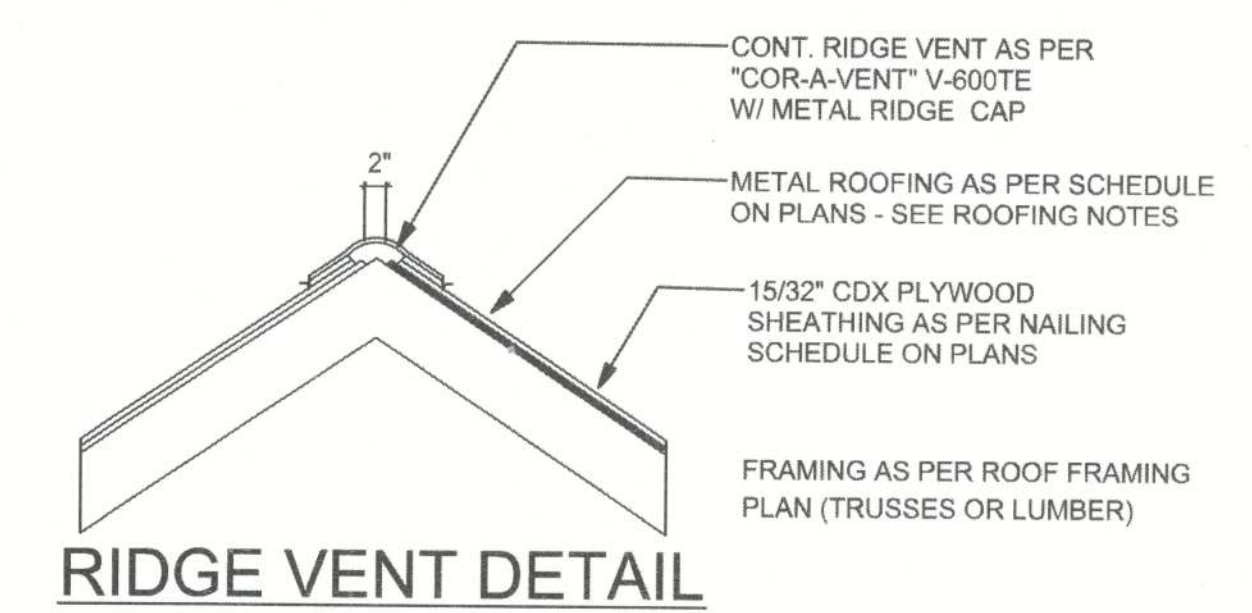
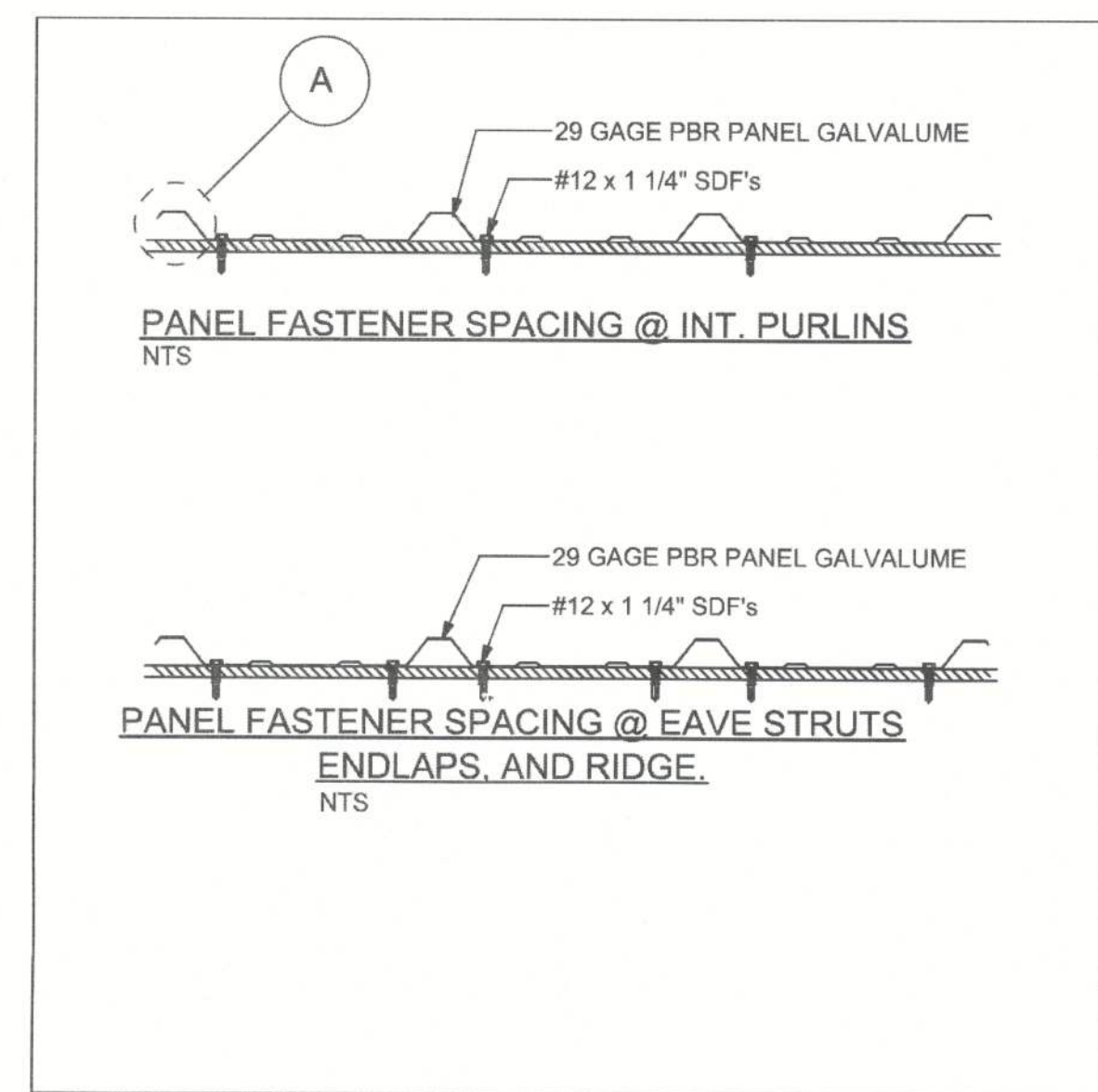
ROOF SHEATHING FASTENINGS			
NAILING ZONE	SHEATHING TYPE	FASTENER	SPACING
1	15/32" cdx	18d ring shank hot dipped galvanized	6 in. o.c. EDGE
2			12 in. o.c. FIELD
3			6 in. o.c. EDGE 6 in. o.c. FIELD



ROOF SHEATHING NAILING ZONES
(HIP ROOF)

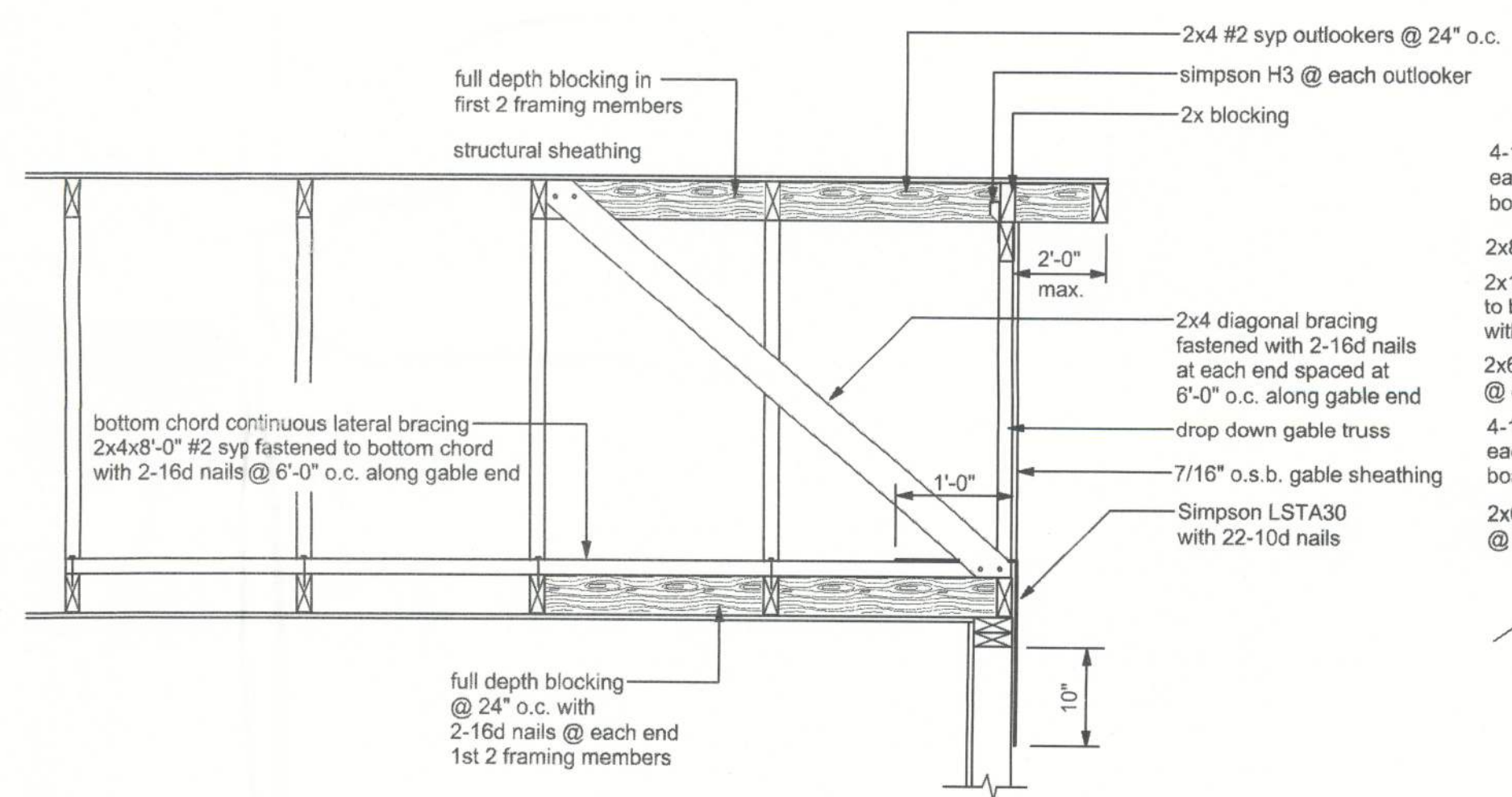


ROOF SHEATHING NAILING ZONES
(GABLE ROOF)



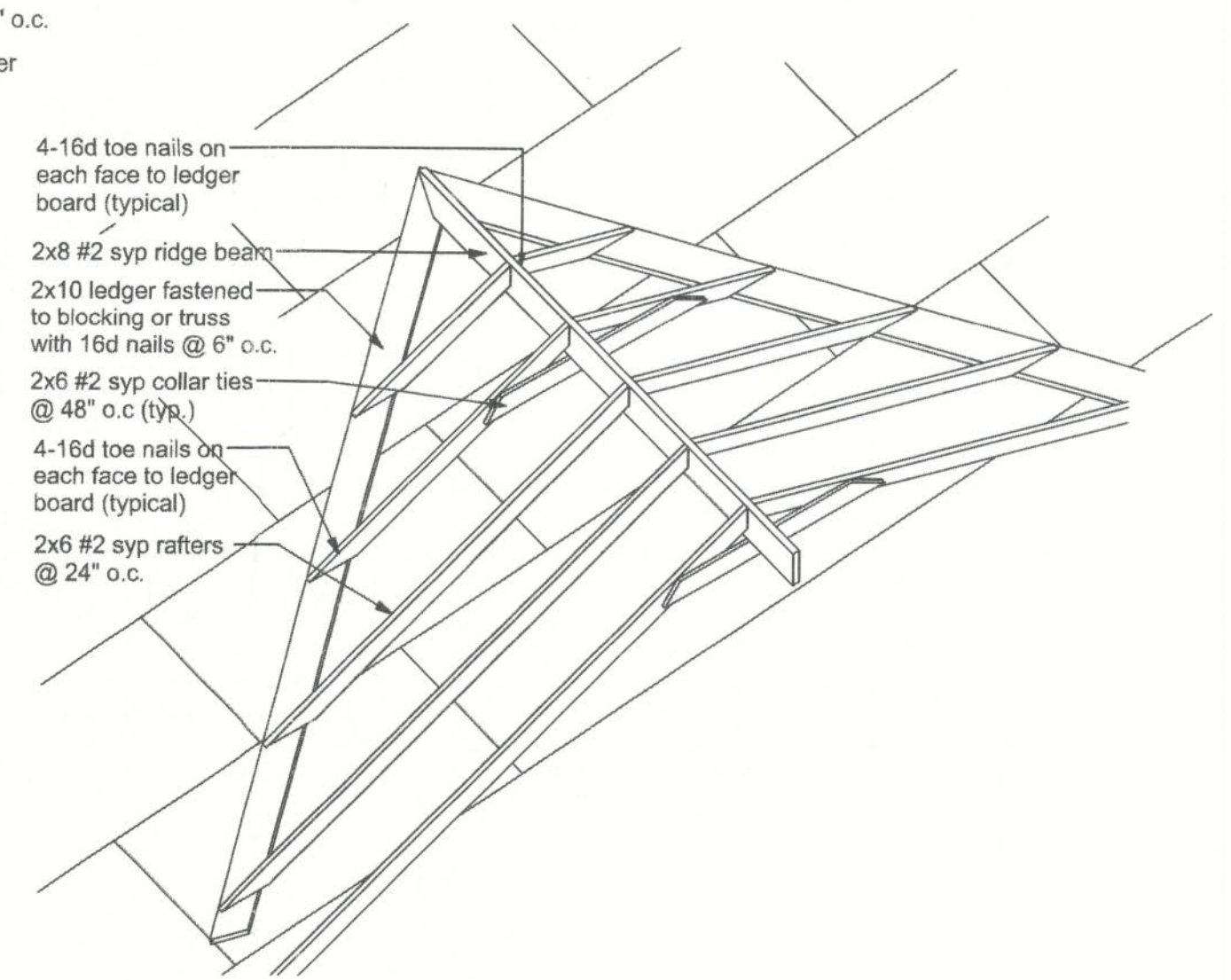
VENTILATION REQUIREMENTS

Total Attic Square Footage	Recommended Length of Cobra Rigid Vent II (Feet)	Minimum Intake Ventilation (Net Free Area in Sq. In.)
1600	21	384
1900	25	456
2200	29	528
2500	33	600
2800	41	744
3100	41	820
3400	45	816



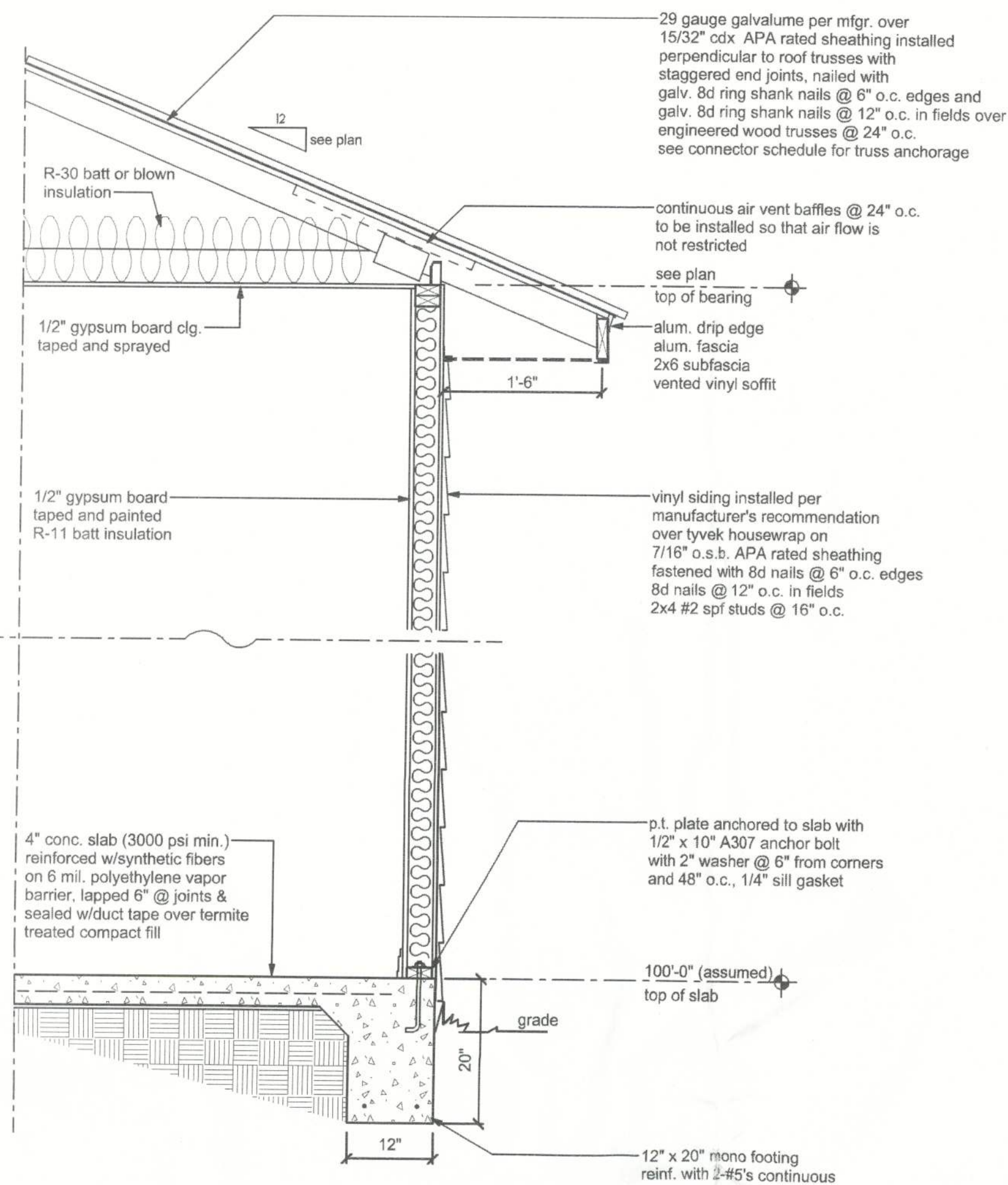
END WALL BRACING FOR CEILING DIAPHRAGM

NTS
NOTE: ALL WOOD TO BE NUMBER 2 GRADE SOUTHERN YELLOW PINE



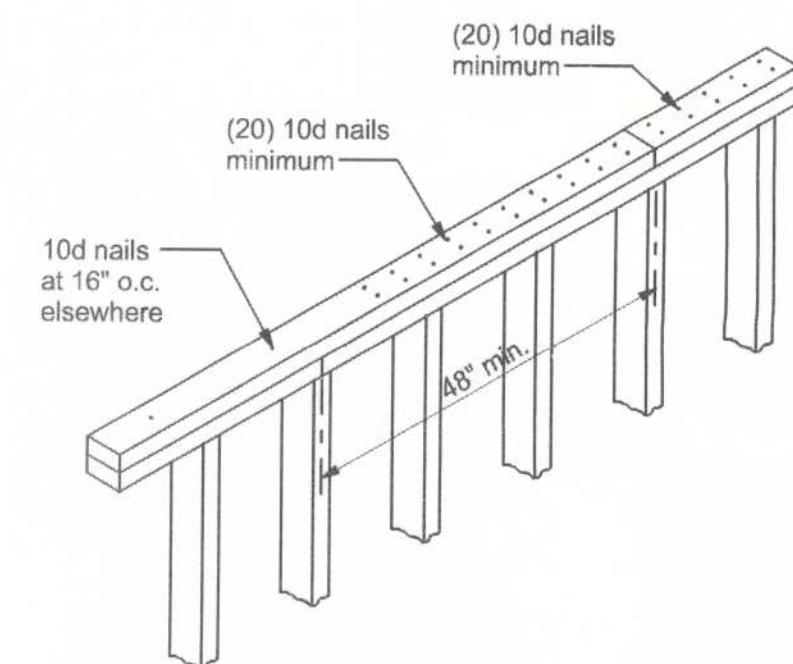
ROOF INTERSECTION DETAIL

CONNECTOR SCHEDULE FOR TRUSS ANCHORAGE				
CONNECTOR	TRUSS	TOP PLATE	UPLIFT PROVIDED	MANUFACTURER
H2.5A	5-8d NAILS	5-8d NAILS	600 LBS	SIMPSON
H10A	9-10d NAILS	9-10d NAILS	1140 LBS	SIMPSON
HTS16	8-10d NAILS	8-10d NAILS	1,260 LBS	SIMPSON
H16	2-10d NAILS	10-10d NAILS	1,470 LBS	SIMPSON
(2)HTS20	10-10d NAILS	10-10d NAILS	2 x 1,450 = 2,900 LBS	SIMPSON



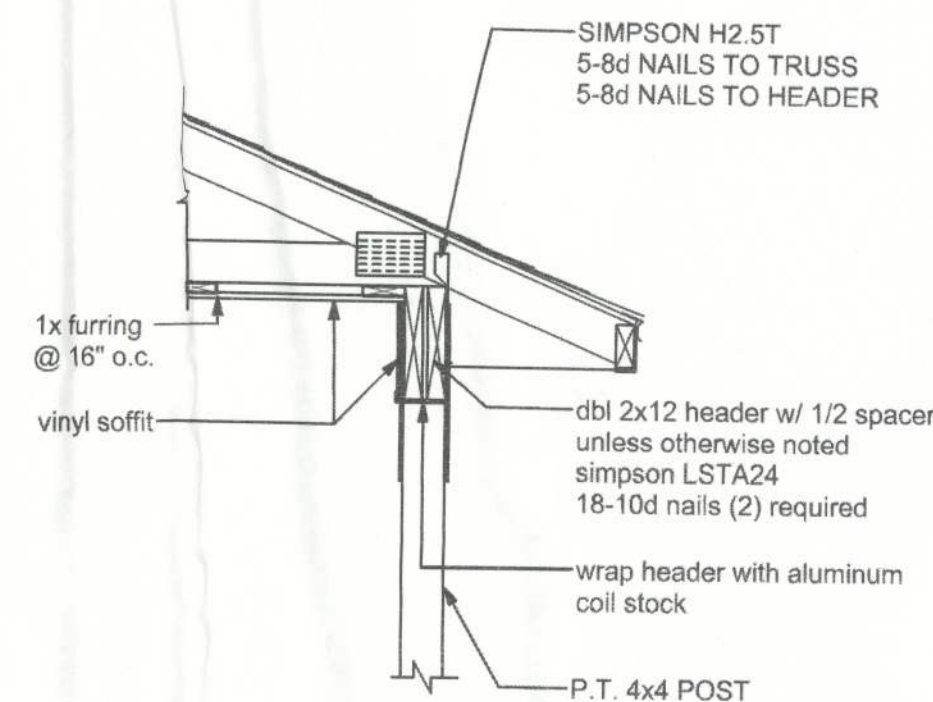
TYPICAL WALL SECTION

SCALE: 3/4" = 1'-0"



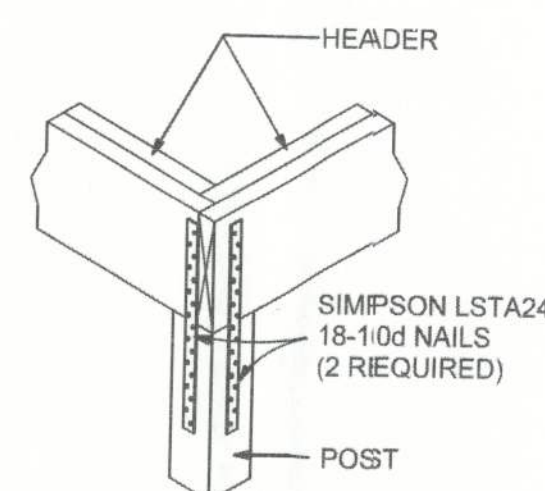
TOP PLATE SPLICE DETAILS

SCALE: 1/2" = 1'-0"



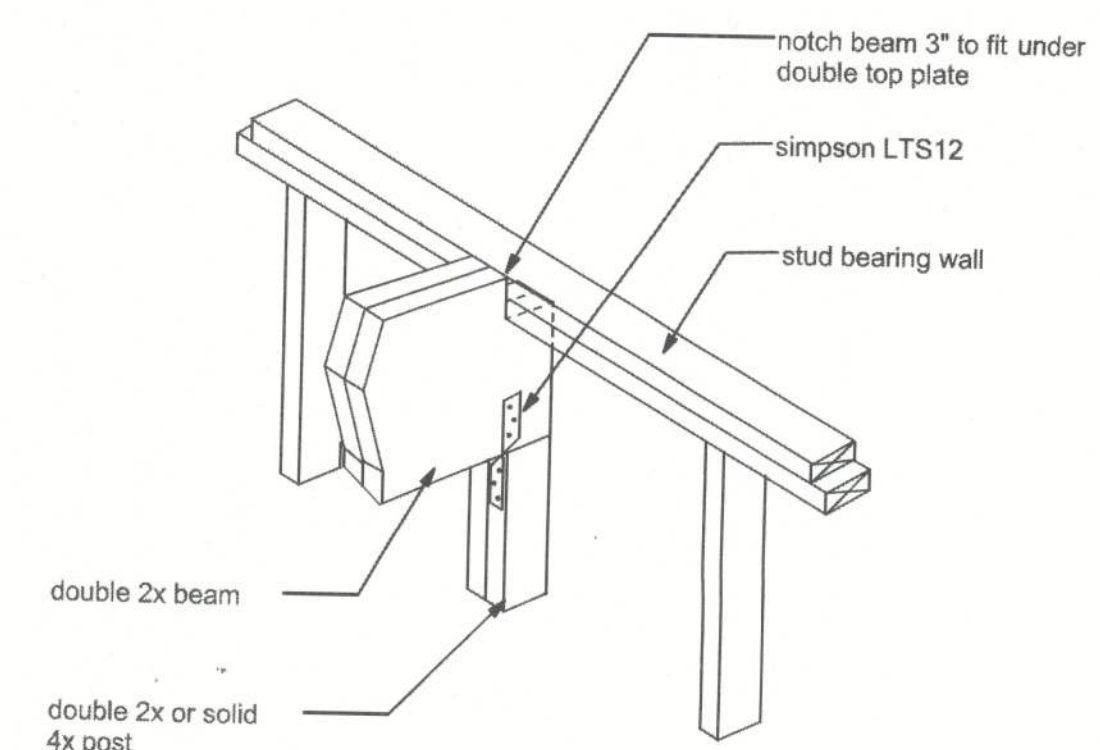
PORCH SECTION

SCALE: 3/4" = 1'-0"



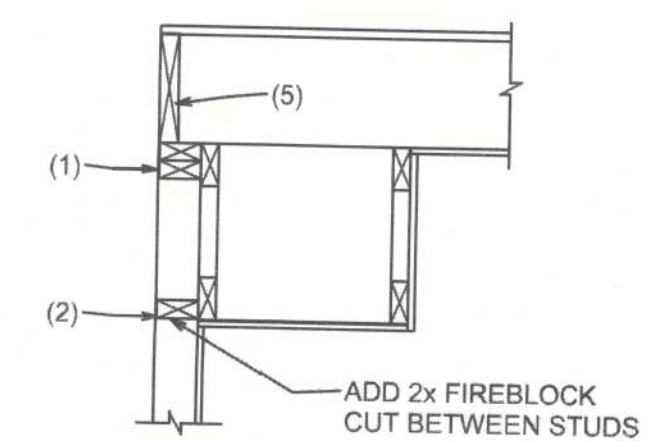
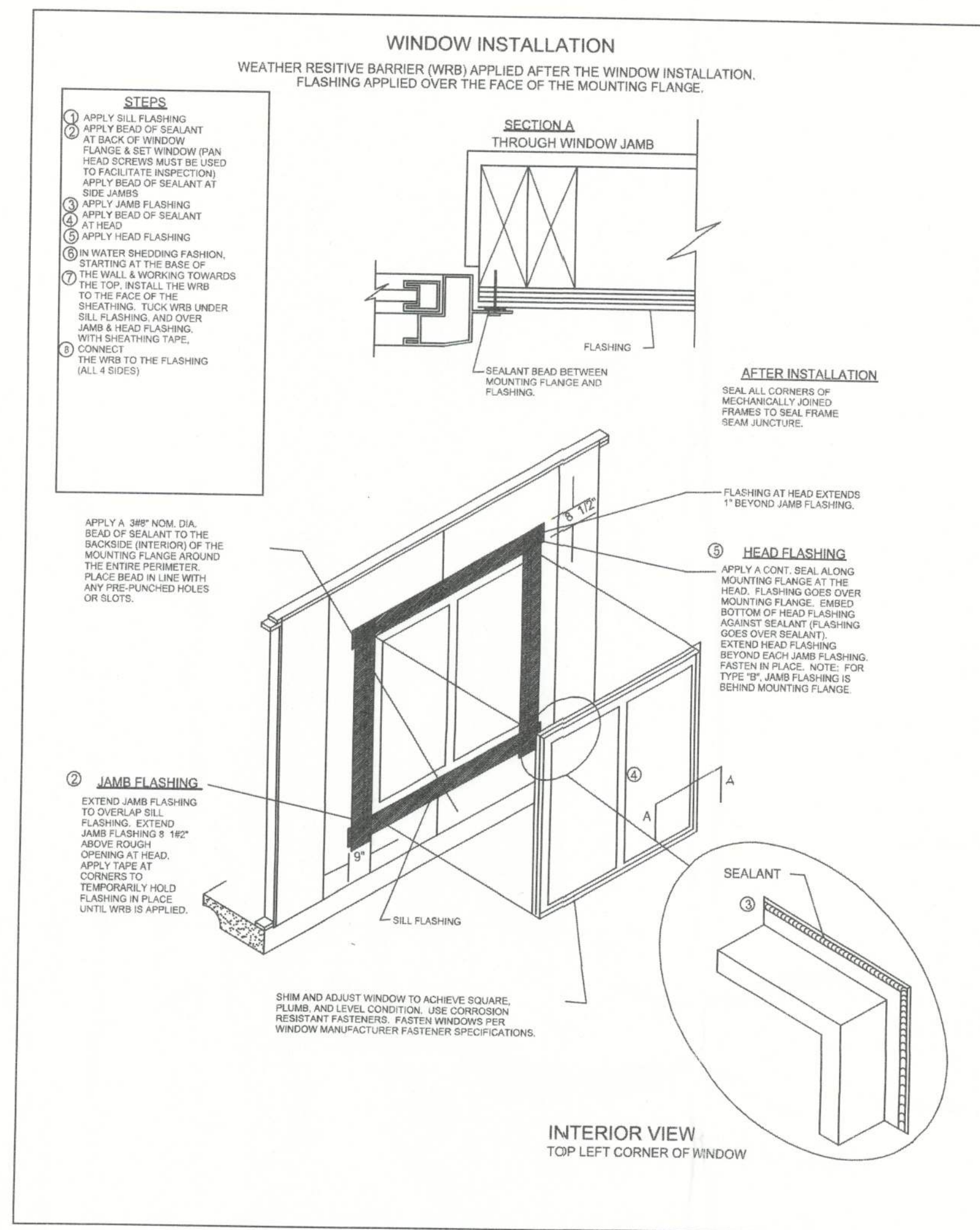
CORNER POST/HEADER DETAIL

NTS



BEAM/WALL CONNECTION

NTS

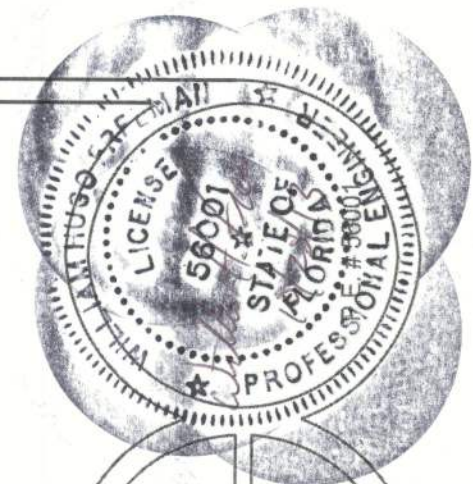


SOFFIT/DROPPED CLG.

FIREBLOCKING NOTES:

FIREBLOCKING SHALL BE INSTALLED IN WOOD FRAME CONSTRUCTION IN THE FOLLOWING LOCATIONS:

1. IN CONCEALED SPACES OF STUD WALLS AND PARTITIONS INCLUDING FULL SPACES AT CEILING AND FLOOR LEVELS.
2. AT ALL INTERCONNECTIONS BETWEEN CONCEALED VERTICAL AND HORIZONTAL SPACES SUCH AS OCCUR AT SOFFITS, DROP CEILINGS, COVE CEILINGS, ETC.
3. IN CONCEALED SPACES BETWEEN STAIR STRINGERS AT THE TOP AND BOTTOM OF THE RUN.
4. AT OPENINGS AROUND VENTS, PIPES, DUCTS, CHIMNEYS AND FIREPLACES AT CEILING AND FLOOR LEVELS WITH PYRO PANEL MULTIFLEX SEALANT.
5. AT ALL INTERCONNECTIONS BETWEEN CONCEALED VERTICAL STUD WALL PARTITION SPACES AND CONCEALED SPACES CREATED BY AN ASSEMBLY OF JOISTS, FIREBLOCKING SHALL BE PROVIDED FOR THE FULL DEPTH OF THE STUDS AND OVER THE SUPPORTS.



ELIZABETH WILLIAMSON RESIDENCE

SHEARWALL DETAILS

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(904) 429-7536
COA E-0000701



DATE
10/27/13

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W.H.F.

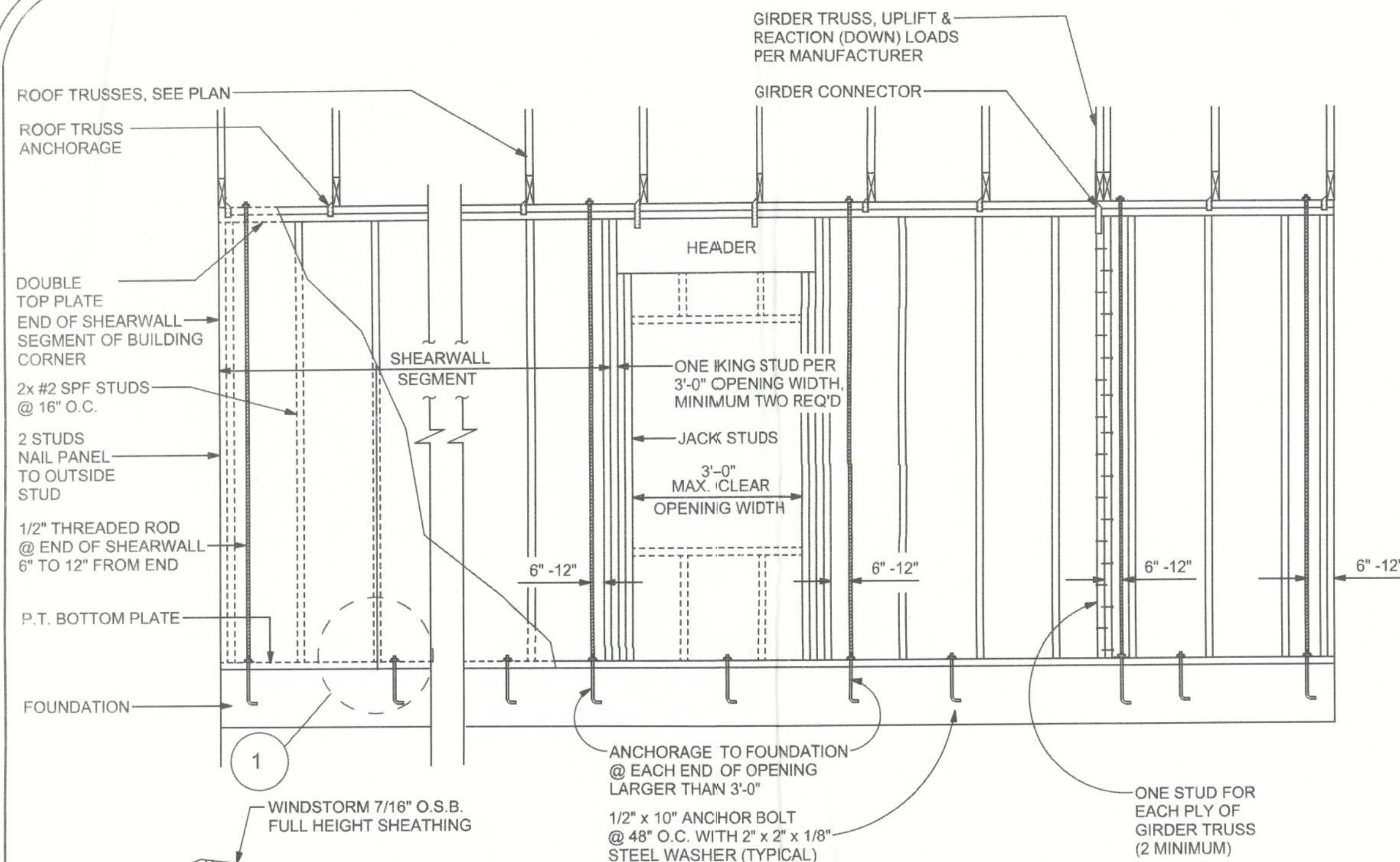
APPROVED
W.H.F.

REVISIONS

SHEET
A-6

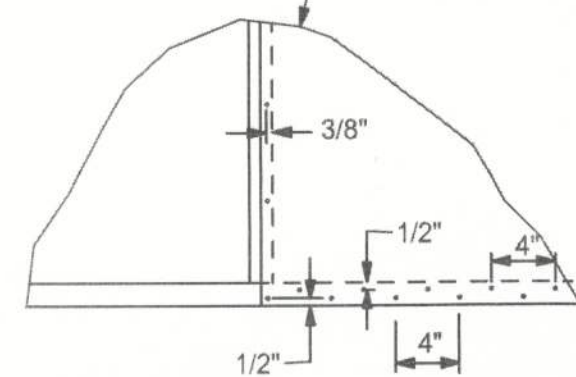
OF
7

PROJECT NO.
13.R021



SHEARWALL DETAILS

SCALE: 1/2" = 1'-0"



DOUBLE NAIL EDGE SPACING TOP AND BOTTOM PLATE

UPLIFT CAPACITY = 474 plf
(TABLE 305S1 SST10-99)

RULES:

1. One all-thread rod at each corner.
2. One all-thread rod at each end of shearwalls.
3. One all-thread rod at each end of opening headers greater than 3'-0"
4. Check sub-sheathing to top plate connection for horizontal transfer capability.
5. If necessary, add all-thread rods to girders individually to exclude the from average uplift plf.
6. Check sole plate to slab connection, additional anchors may be required for lateral and shear load transfer.

ALLOWABLE VALUES	
Connection Type	Allowable Value
Foundation / S.Y.P. Top Plate	3840 lbs.
Foundation / Spruce-Pine-Fir Top Plate	3840 lbs.
Lintel or Bond Beam / S.Y.P. Top Plate	3840 lbs.
Lintel or Bond Beam / Spruce-Pine-Fir Top Plate	3840 lbs.

Placement at slab level:

Corners

When presetting the all-thread rod at a building corner, the rod should be placed 8 to 12 inches away from the corner so it does not set under the corner framing members. When a all-thread rod is specified at a building corner, it may be placed on either side of the corner.

Header ends

When presetting the all-thread rod at a header end, the rod should be placed 8 to 12 inches away from the header end so it does not fall under the stud pack framing members.

Top Connections

Top connections made at corners and header ends shall be made within 2 inches of the framing pack. A nut and 3X3 washer shall be applied to the top plates and tightened securely.

Intermediate Coupler Connections

When using the rod coupler, care should be taken to ensure full and equal thread engagement. This is easily achieved by threading the coupler all the way onto the rod, then standing the two rods end to end, then threading the coupler back over the rod joint so each rod is halfway into the coupler.

Retro-fits

In the case of an all thread rod misplacement, the rod may be epoxied into the concrete.

Sole plate to slab connection:

The slab level sole plate shall be connected to the slab with the connectors specified and at the spacing specified within the design documents. All-thread rods shall be placed as per the design specifications. All-thread rods with a nut and washer at the sole plate will qualify as a sole plate connection but may require other anchors intermediate of the all-thread rod locations to qualify the specified spacing requirements.

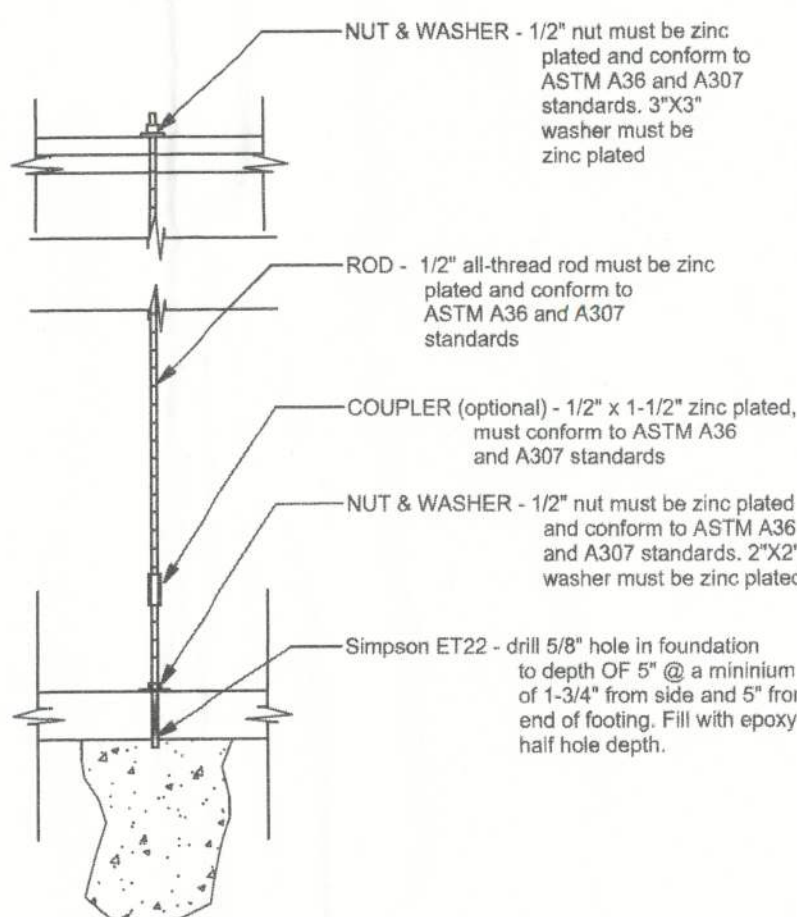
System Tightening

On multiple story applications, the all-thread rod system shall be rechecked for proper tension just before the walls are veneered. This will allow the all-thread rod system to compensate for the buildings dead load compression.

SHEARWALL NOTES:

1. ALL SHEARWALLS SHALL BE TYPE 2 SHEARWALLS AS DEFINED BY STD 10-99 305.4.3.
2. THE WALL SHALL BE ENTIRELY SHEATHED WITH 7/16" O.S.B. INCLUDING AREAS ABOVE AND BELOW OPENINGS.
3. ALL SHEATHING SHALL BE ATTACHED TO FRAMING ALONG ALL FOUR EDGES WITH JOINTS FOR ADJACENT PANELS OCCURING OVER COMMON FRAMING MEMBERS OR ALONG BLOCKING.
4. NAIL SPACING SHALL BE 6" O.C. EDGES AND 12" O.C. IN THE FIELD.
5. TYPE 2 SHEARWALLS ARE DESIGNED FOR THE OPENING IT CONTAINS. MAXIMUM HEIGHT OF OPENING SHALL BE 5/6 TIMES THE WALL HEIGHT. THE MINIMUM DISTANCE BETWEEN OPENINGS SHALL BE THE WALL HEIGHT/3.5 ie. FOR 8'-0" WALLS - (2'-3").

OPENING WIDTH	SILL PLATES	16d TOE NAILS EACH END
UP TO 6'-0"	(1) 2x4 OR (1) 2x6	1
> 6' TO 9'-0"	(3) 2x4 OR (1) 2x6	2
> 9' TO 12'-0"	(5) 2x4 OR (2) 2x6	3

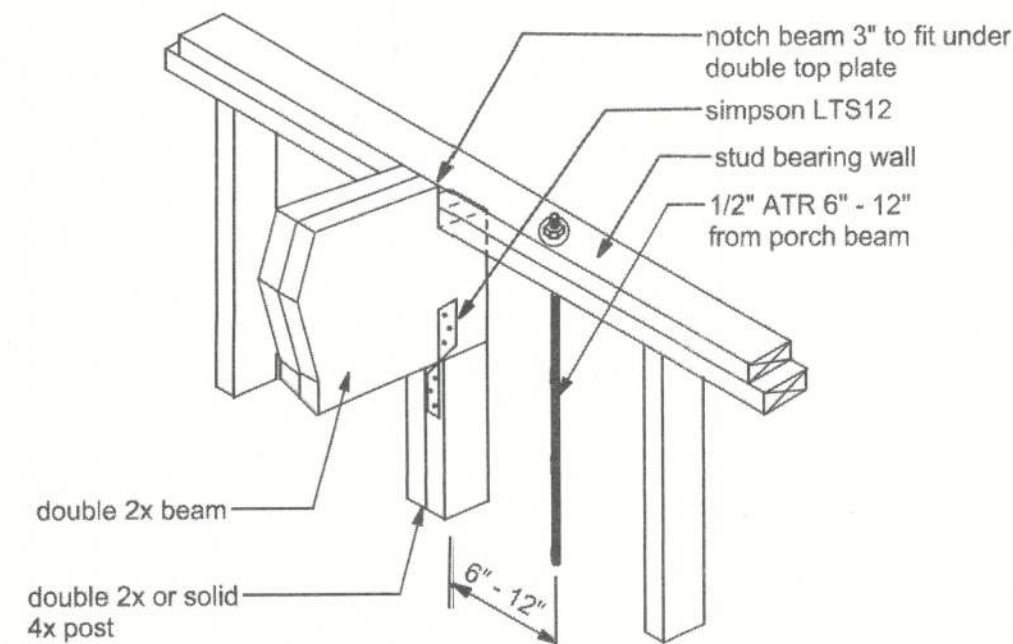


END (TOP OR BOTTOM)

GIRDER COLUMN DETAIL

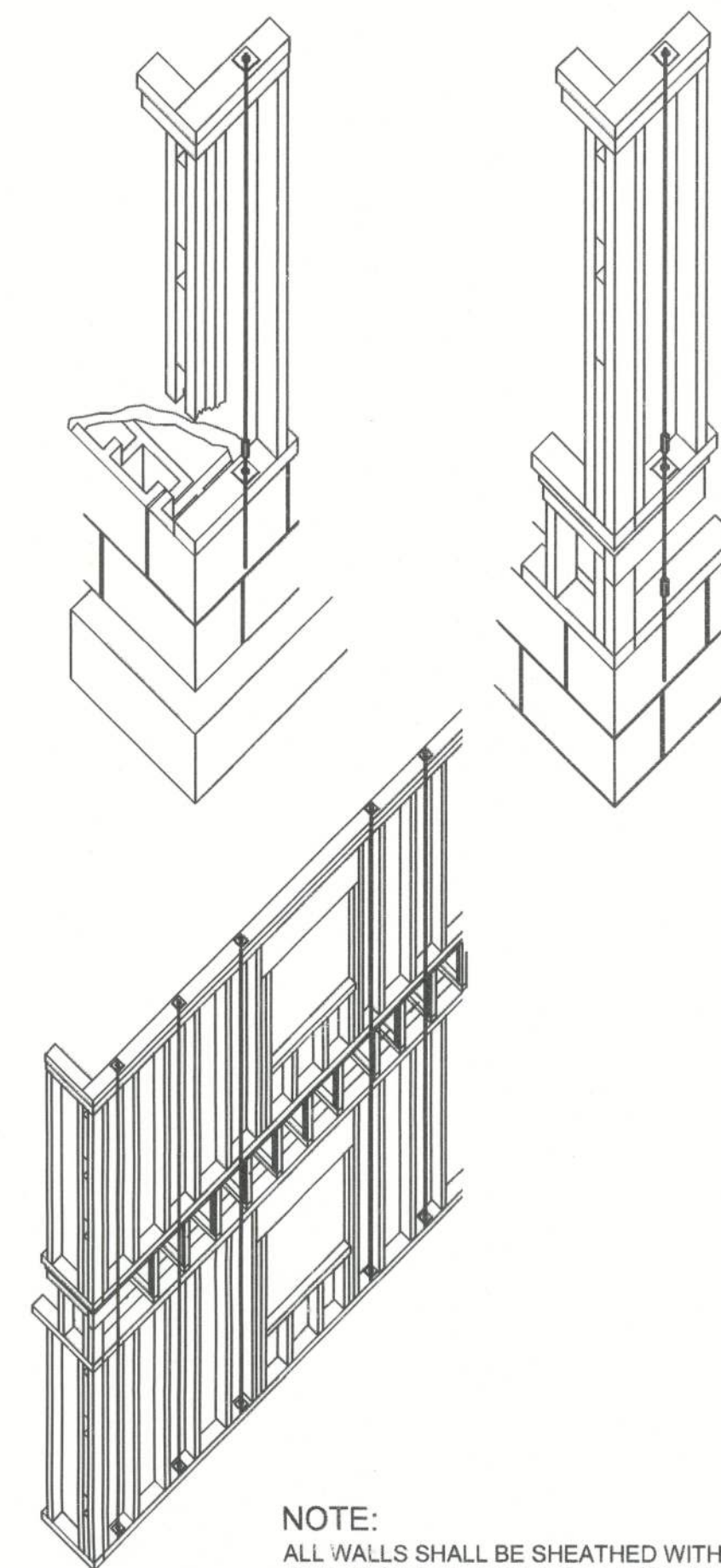
SCALE: 1/2" = 1'-0"

OPENING CONNECTION REQUIREMENTS				
CLEAR OPENING WIDTH	HEADER SIZE #2 GRADE OR BETTER	END BEARING	CONNECTOR AT EACH END OF OPENING	ANCHORAGE TO FOUNDATION @ EACH END OF OPENING
0' - 3'	(2) 2x8	1.5"	N/A	N/A
>3' - 6'	(2) 2x10	3"	1/2" ALL THREAD ROD	1/2" ALL THREAD ROD
>6' - 9'	(2) 2x12	3"	1/2" ALL THREAD ROD	1/2" ALL THREAD ROD
>9' - 12'	(2) 1 3/4" x 11 1/4" LVL - 2.0E	3"	1/2" ALL THREAD ROD	1/2" ALL THREAD ROD
>12' - 15'	(2) 1 3/4" x 11 1/4" LVL - 2.0E	3"	1/2" ALL THREAD ROD	1/2" ALL THREAD ROD
>15' - 18'	(2) 1 3/4" x 11 1/4" LVL - 2.0E	4.5"	1/2" ALL THREAD ROD	1/2" ALL THREAD ROD

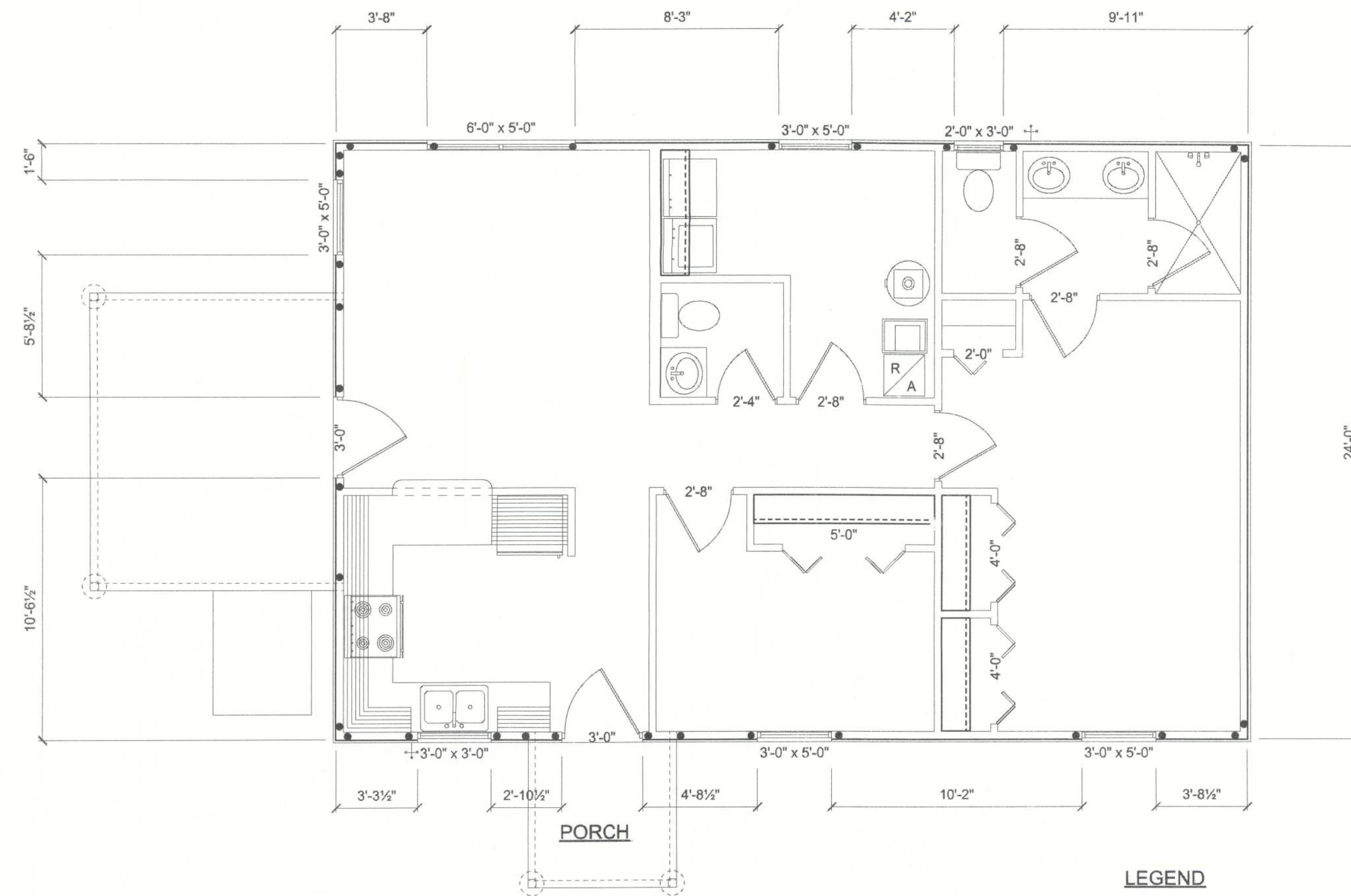


BEAM/WALL CONNECTION

NTS



NOTE:
ALL WALLS SHALL BE SHEATHED WITH WINDSTORM FULL HEIGHT SHEATHING, 8'-1 1/8" TALL.


















LEGEND

- all thread rod location

SHEARWALL LAYOUT

SCALE: 1/8" = 1'-0"

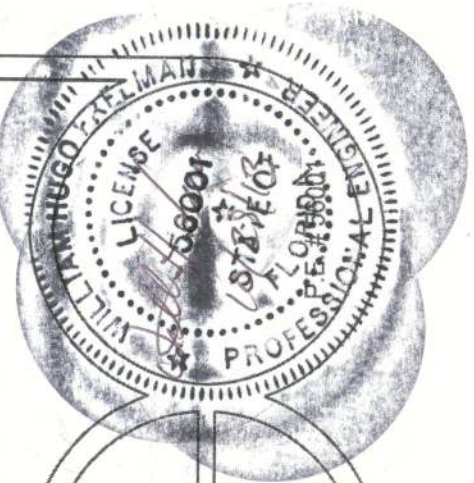
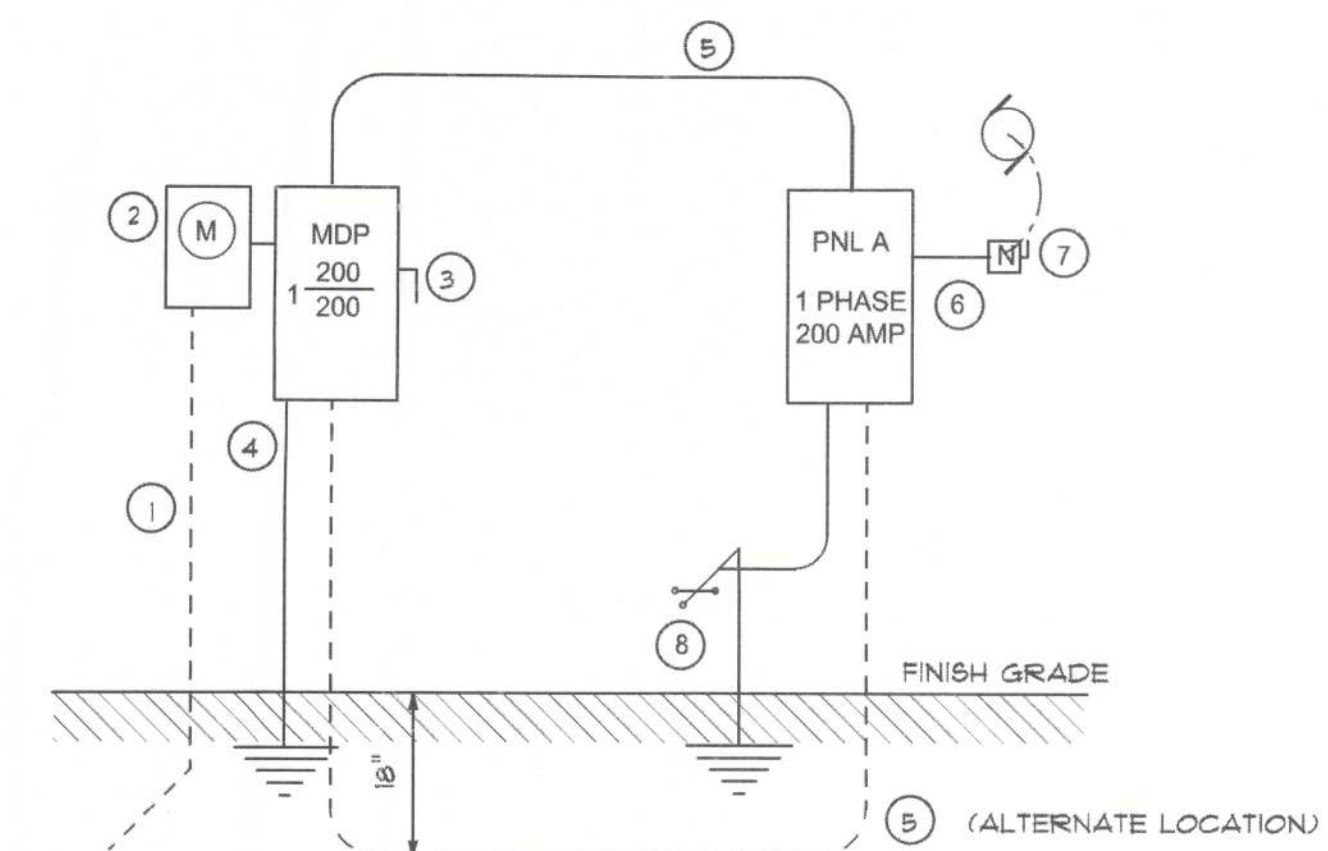


ELECTRICAL	COUNT	SYMBOL
HVAC motor	1	
electrical panel	1	
motor	2	
non fused disconnect	3	
50 cfm exhaust	3	
ELEC METER	1	
WP GFI	3	
carbon monoxide detector	1	
light	12	
outlet	21	
outlet 220v	2	
outlet gfi	11	
smoke detector	3	
switch	9	
switch 3 way	6	

INSTALLATION:

- A. Secure all supports to building structure as specified under raceways. Support horizontal runs of metallic conduit not more than 10 feet apart. Run exposed raceways parallel with or at right angles to walls.
- B. Pass raceways over water, steam or other piping when pull boxes are not required. No raceway within 3 inches of steam or hot water pipes, or appliances, except crossing where the raceway shall be cut at 2 inches from pipe cover.
- C. Cut conduit end square , ream smooth. Paint male threads of field threaded conduit with Graphite based pipe compound. Draw up tight with conduit couplings.
- D. Leave wire sufficiently long to permit making final connections. In raceway over 50 feet in which wiring is not installed, furnish pull wire.
- E. Verify locations of outlets and switches.
- F. Support panel, junction and pull boxes independently to building structure with no weight bearing on conduits.
- G. Connect conduit to motor conduit terminal bases with flexible conduit; minimum 18 inches in length and 50% slack. Do not terminate in or fasten raceways to motor foundation.
- H. This contractor shall provide a temporary electrical distribution system as required; 120/208 volt, 1 phase, 100 amp, for new construction. All temporary work shall be installed in a neat and safe manner.
- I. Contractor to remove and salvage all abandoned electrical equipment.
- J. This contractor shall warrant all labor and materials for one year from date of final written acceptance.

- 1 Service/Feeder Entrance Conductors: 2 1/2" rigid conduit, min 18" deep, w. continuous ground bonding conductor, Service/ Entrance Conductors shall not be spliced except that bolted connections at the Meter, Disconnecting Devices and Panel shall be allowed.
- 2 Existing Meter Enclosure, weatherproof, U.L. Listed.
- 3 Main Disconnect Switch: fused or Main Breaker, weatherproof, U.L. Listed.
- 4 Service entrance ground: 5/8" diameter iron/steel rod x 8'-0" long and/or concrete encased foundation steel rebar x 20'-0" long. Grounding conductor shall be bonded to each piece of Service/Entrance Equipment, and shall be sized per Item #5 below.
- 5 200 Ampere Feeder: 3-2/0-THW-Cu, 1-#2-Cu-GND, 2 1/2" Conduit.
- 6 House Panel (PNL), U.L. Listed, sized per schedule.
- 7 Equipment Disconnect Switch: non-fused, in weather proof enclosure, size according to Panel Schedule loads.
- 8 Provide Ground Bond Wire to metal piping, size in accordance with the Service Ground Conductor.



ELIZABETH WILLIAMSON RESIDENCE

ELECTRICAL PLAN

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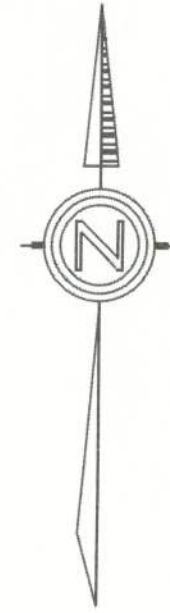
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	APPROVED W.H.F.

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OF 7

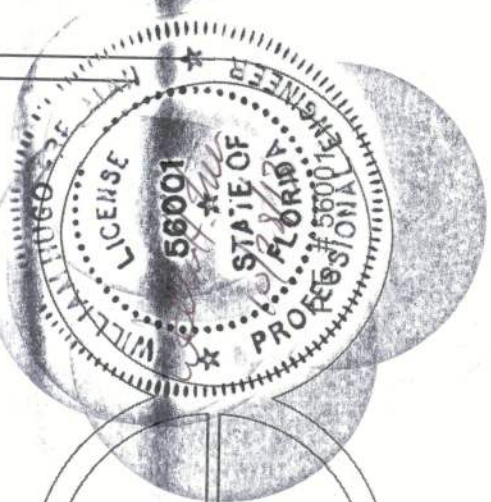
PROJECT NO.
13.R021



DESCRIPTION
ALL OF W1/2 OF SW1/4 LYING E OF CR-245 & N OF CR-252.
SEE SURVEY FOR COMPLETE DESCRIPTION
Section 24-Township 4S- Range 17E



SITE PLAN
SCALE: 1" = 60'-0"



ELIZABETH WILLIAMSON RESIDENCE

SITE PLAN



P.O. BOX 860125
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PROJECT NO. 13.R021	