			DESIGN SPE	ECIFICA	TIONS					USF
	DESIGN CODE: 2023 FLORIDA BUILDING CODE – DESIGN IS VOID ONE YEAR AFTER TO PLANS HAVE BEEN REVIEWED FOR CO	HE DATE OF THE CODE COMPLIANCE	ORIGINAL PLANS, UNLESS	TRIBUTARY AREA (sf)	COMPONENTS & CL ALLOWABLE DESIGN PI INTERIOR ZONE (PSF)	RESSURES	RIP (PSF): 5'-6"	GARAGE DOOR PRESSURES (PSF) 1 CAR GARAGE DOOR	CONNECTOR	UF SYP 450
	DESIGN LOADS: ACTUAL AND UN	<u>ROOF</u> (cd=1.25)	FLOOR (cd=1.00)	10 50	+25.5 -27.7 +22.9 -25.0		-34.2 -28.8	(8'x7') 2 CAR GARAGE	USP RT7 USP RT8A USP MTW12	585 775 1195
	TOP CHORD LIVE LOAD TOP CHORD DEAD LOAD TOP CHORD DEAD LOAD	20 psf 7 psf (ARCH SH 20 psf (TILE SHI	INGLES) 10 psf	100	+21.8 -23.2		-26.5		USP HTW20 USP MSTA24	1450 1640
2	BOTTOM CHORD LIVE LOAD BOTTOM CHORD DEAD LOAD DEFLECTION CRITERIA:	10 psf 5 psf	0 psf 5 psf	WIND PRESS	S ABOVE ARE ALLOWAE SURES HAVE BEEN RED SIGN METHODOLOGY. NO	UCE BY 0.6	0 AS PERMITI	ED BY THE ALLOWABLE	USP MSTA36 USP LTS20B	2065 1105
	ROOF FRAMING: LIVE LOAD L/240 T FLOOR FRAMING: LIVE LOAD L/360 0.75" MAX ANY CASE				T & CLADDING WALL EL IVE PRESSURES SHOWN			NED FOR BOTH POSITIVE	USP HTT16	1305 4290
- -	WIND LOADING: ASCE 7/22 FOR WIND UPLIFT, TRUSS	SES SHALL BE DE	SIGNED WITH A MIN.		ERPOLATION IS PERMISS RESSURE AND MINUS =				USP HTT22 USP PAU44	5370 2535
-	DEAD LOAD CONDITION OF 5 PSF TO REACTIONS CALCULATED FOR THE BE BE REDUCED. SPECIFICALLY, ATTIC F	EARING POINTS OF LOOR LIVE LOADS	F ROOF TRUSSES SHALL S COMBINED WITH ROOF	DESIGN OF	WINDOWS/DOORS FAST	ENING TO TI			USP PAU66 USP MSTAM24	2535 1545
ן ר	LIVE LOADS SHALL BE MULTIPLIED B BASIC WIND SPEED (ASCE 7-22)				ED POSITIVE AND NEGA					SIMPS
	IMPORTANCE FACTOR — — — — MEAN ROOF HEIGHT — — — —		————— 1.00 ————— 20.0 FT			╡	-		CONNECTOR	UF
ן נ	BUILDING CATEGORY		7/12 II C	, ↓ª ↓ DE	NOTES EDGE STRIP.				A35 H2.5T	450 600
	EXPOSURE CATEGORY ENCLOSURE CLASSIFICATION - INTERNAL PRESSURE COEFFICIENT	·	——————————————————————————————————————	SE SE	E C & C CHART OVE FOR 'a' DIMENSION				HTS16 MTS12	1150 1000
	MATERIAL S		CATIONS						HTS20 MSTA24	1450 1765
	HARDWARE AND ANCHORS:			END ZONE W	DTH 'a' = 5'-6"				MSTA36	2050 3480
	ANCHOR BOLTS & THREADED ROD: S OR ASTM F 1554 GRADE 36. WASHERS: SHALL BE IN ACCORDANCE	E WITH ASTM A50	DO (GRADE B).		SCOPE O	-			НТТ4 НТТ5	5250
	NUTS: SHALL BE IN ACCORDANCE WI METAL CONNECTORS: ALL METAL CON SHALL BE GALVANIZED.	TH ASTM A 563 NNECTORS WHICH	GRÀDE A HEX. ARE EXPOSED TO EXTERIOR	MEANS AND M	ETHODS:				LUS28	930
•	RETROFIT REBAR/ROD INSTALLATION: SHALL BE 12 BAR DIAMETER MINIMUM REBAR SIX AND 1/8" LARGER THAN	I, HOLES SHALL	BE $\frac{1}{4}$ " LARGER THAN	THE STRUCTUR CONSTRUCTION THE ACTS OR	RAL ENGINEER SHALL N MEANS, METHODS, TEC OMISSIONS OF THE CO	CHNIQUES, F NTRACTOR (PROCEDURES, DR ANY OTHEF	OR SEQUENCES; FOR R PERSONS PERFORMING		905
	ANCHORING ADHESIVE: SHALL BE ON CARTRIDGE INSTALLATION ONLY): EPOXY: ITW RED HEAD A7			ACCORDANCE	FOR THE FAILURE FOR WITH THE CONTRACT DO	OCUMENTS.			ABU44	2200
	REINFORCING STEEL: SHALL BE ASTM STRUCTURAL STEEL: SHALL BE ASTM	A992, GRADE 50		THE ITEMS SPE THE FOLLOWING	UCTURAL ENGINEERING ECIFICALLY DESIGNED B' G: CONTINUOUS LOAD F	Y THE STRU PATH FOR V	ICTURAL ENGIN VIND UPLIFT, V	NEER ARE LIMITED TO	ABU66 SET	2300 N/A
	WELDED WIRE FABRIC (WWF): SHALL LAMINATED VENEER LUMBER (LVL): A OR EXCEED THE FOLLOWING DESIGN F	LL LAMINATED VE		SUPPORTING R	WALL FRAMING AND RE OOF FRAMING. ITEMS N JSSES, FLOOR FRAMING	OT DESIGNE	D PRE-ENGINI FICALLY ADDR	EERED WOOD FLOOR ESSED.	LTT20B	1675
	BENDING STRESS (Fb) 2600psi			TRUSS-TO-TR ELECTRICAL SY	USS CONNECTION, AND (STEM.	ANY ARCHI	TECTURAL, ME	CHANICAL OR	LSTA12 CS16	805 1705
			GENE	ERAL N	NOTES & (CON	STRU	CTION SPE	ECIFICAT	IONS
30	FLOOR SHEATHING SPECIFICATIONS 23/32" T&G OSB OR PLYWOOD SHEAT	HING, GLUE AND N	IAIL WITH 10d COMMON @ 6" C).C. EDGE & FIELD)					
01	ROOF SHEATHING SPECIFICATIONS: SHINGLE- MIN. 15/32", 32/16, APA R	ATED OSB OR PLY	WOOD SHEATHING, NAILED w/	0.131x2 <mark>2</mark> " RING SI	HANK NAILS @ 6" O.C. E	EDGE & 6"C).C. FIELD (AT	GABLE ENDS DECREASE EI	DGE NAIL SPACING TO	4" O.C. WITHIN 4
Г	<u>TILE</u> – MIN. 15/32" 32/16, APA RATE		. –			-				
$\overline{\mathbf{N}}$	METAL – MIN. 1/2", 24/16, AP. WALL SHEATHING SPECIFICATIONS:							·		
00	FLEXIBLE FINISH-MIN. 7/16", 24/16, AF HARDI PANEL & BRICK. ALL OTHER WA	ALL SHALL BE CON	ISIDERED BRITTLE FINISH.							
2	STUCCO FINISH, -MIN. 7_{16} ", 24/16, AP 15 %2 OSB SHEATHING AND ORIENT THE			ED W/ 8D @ 6" (D.C. EDGE AND 6" O.C. F	FIELD. SHEAT	HING SHALL OF	RIENTED WITH THE LONG D	IMENSION PERPENDICUL	.AR TO THE STUI
0	MASONRY SPECIFICATIONS: MASONRY HAS BEEN DESIGNED IN A	CCORDANCE WITH	ACI 530-05 AND SHALL BE		IN ACCORDANCE WITH	ACI530 1-05	5 GROUT SHA	II BE IN ACCORDANCE V	WITH ASTM C476 WITH	A MINIMUM OF
.::	GROUT SHALL HAVE A MAXIMUM COU INSTALLATION OF ALL FLASHING.	JRSE AGGREGATE	SIZE OF 36 PLACED AT AN 8	8" TO 11" SLUMF	P. MORTAR SHALL CONF	FORM TO AS	STM C270 AND) TYPE M OR S. TYPE N	MORTAR MAY BE USE	ED IN BRICK VEI
Σ	CONCRETE MASONRY UNITS (CMU): CMU SHALL BE IN ACCORDANCE WIT LIFTS PROVIDE CLEANOUTS PER ACI						MINIMUM COM	PRESSIVE STRENGTH OF	1900 psi (f'm=1500	psi). GROUT ALL
	MASONRY STEMWALLS: ALL CONCRE	IE MASONRY UNIT	S SHALL BE COMPOSED OF A	ASTM C90E, E GI	RADE N-1 HOLLOW CON	NCRETE MAS				
F/	CONTAINING VERTICAL REINFORCEMEN WALL END, AND WALL INTERSECTION: COURSES, PROVIDE HORIZONTAL JOIN	S. PROVIDE CONTI	INUITY OF REINFORCING AT IN	ITERSECTIONS OF	PERPENDICULAR MASC	ONRY ELEME	NTS BY INSTA	LLING CORNER BARS, MI	NIMUM OF 40 BAR DIA	AMETERS INTO E
Щ	CONCRETE SPECIFICATIONS: ALL CONCRETE HAS BEEN DESIGNED CONCRETE AT GARAGE AND PORCH				CTED IN ACCORDANCE V	WITH ACI 30	1. ALL CONCR	ETE SHALL HAVE A MINI	MUM COMPRESSIVE ST	RENGTH OF 300
4	GENERAL NOTES:	SLADS SHALL HA	VE A COMPRESSIVE STRENGT	1 OF 3000 F3I.						
	FOOTING AND FOUNDATIONS: FOOTINGS AND FOUNDATIONS SHALL	BE IN ACCORDAN	NCE WITH LOCAL BUILDING CC	DES. FOOTING H	AVE BEEN DESIGNED WI	ITH A SOIL	BEARING (DES	IGN MAXIMUM) OF 2000	PSF. A SOILS INVEST	IGATION REPORT
Z	IF THE FOOTING ELEVATIONS SHOWN PROCTOR IN ACCORDANCE WITH AST		TURBED OR UNSTABLE SOIL,	THE ENGINEER S	HALL BE NOTIFIED. SOIL	l SHALL BE	FREE OF OR	GANIC MATERIAL AND CO	HESIVE (CLAY) SOILS.	SOIL COMPACTI
ш	FOUNDATION PLAN ONLY CONVEYS S DO NOT DETERMINE FOOTING LOCATI									
RR	UNLESS OTHERWISE NOTED ON DRAV REINFORCING AT INTERSECTIONS OF									
5	CONCRETE SLABS ON GRADE: SHALL BE INSTALLED OVER MINIMUM CONTROLLED CRACKING CUT A 1" S/									
U	WOOD FRAMING SPECIFICATIONS: ALL WOOD FRAMING HAS BEEN DESI	GNED IN ACCORDA	ANCE WITH NATIONAL DESIGN	SPECIFICATIONS	(NDS) FOR WOOD CON	STRUCTION,	LATEST EDITIO	DN. ALL WOOD MEMBERS	S EXPOSED TO WEATH	IER OR IN CONT
	PRESSURE-TREATED. IF, ACQ OR NO PRE-ENGINEERED WOOD TRUSSES:									
30	SHALL BEAR THE SEAL OF AN ENGI DRAWINGS. ALL TRUSS-TO-TRUSS C PLATE CONNECTED WOOD TRUSSES,	ONNECTIONS AND	TRUSS PROFILES ARE THE F	RESPONSIBILITY C	OF THE DELEGATED TRU	ISS ENGINEE	R. ALL TRUSS	ES SHALL HAVE TEMPOR	ARY BRACING PER 'CO	OMMENTARY' AN
01	ROOF COVERING SPECIFICATIONS: THE CONTRACTOR SHALL BE RESPO	NSIBLE FOR THE [DESIGN AND INSTALLATION OF	THE ROOF COV	ERING SYSTEM. ASPHAL	_T SHINGS S	SHALL COMPLY	WITH ASTM D3161 AND	BE INSTALLED ACCOF	RDING TO THE M
Б	INSTALLED PER THE "CONCRETE AND CONTRACTOR SHALL BE RESPONSIBL						G SEAM METAL	. ROOFS SHALL COMPLY	WITH ASTM E1514 AN	D BE INSTALLED
2	WATERPROOFING: THE CONTRACTOR SHALL BE RESPON	NSIBLE FOR THE [DESIGN/INSTALLATION OF ALL	WATER PROOFIN	IG.					
00										
\sim	WOOD FAST	ENING	SCHEDULE	BF	RICK NOT	ΈS /	LINTE	EL SCHD	PLAN	LEGEN
0	MEMBERS	CONNECTION TYPE	FASTENER	L	INTEL DIMENSION		MIN. BRG.	MAX. SPAN		ERIOR LOAD BEA
ü	TOP PLATE TO TOP PLATE	FACE NAIL	2-GUN NAILS @ 12" STA (2-16d) 3-GUN NAILS	.G.	$L3^{1}/_{2} \times 3^{1}/_{2} \times 1/_{4}$ L4×3 ¹ / ₂ "× ¹ / ₄		4" 6"	6'-0" 8'-0"	1	BLE X-BRACE, SEE
Σ	DBL. TOP PLATE TO STUD RIM JOIST TO TOP PLATE	FACE NAIL TOE NAIL	(2-16d) 3-GUN NAILS (8d @ 6") GUN NAIL @ 6	5"	L5x3 ¹ / ₂ "x ¹ / ₄		6"	10'-0"	LINE	SIGNATES SHEARWAL E DESIGNATES SIDE EARWALL SHEATHING @ 3/6 DESIGNATES
	CEILING JOIST TO TOP PLATE CEILING JOIST, OVER PARTITIONS	TOE NAIL FACE NAIL	(3-8d) 5-GUN NAILS (3-16d) 4-GUN NAILS		$L6x3\frac{1}{2}x1\frac{4}{4}$ $L7x3\frac{1}{2}x1\frac{4}{4}$	7.07	6" 6"	12'-0" 16'-0"		© 3/6 DESIGNATES O.C. EDGE & 6" O.
T/	CEILING JOIST TO ROOF RAFTER JOIST/TRUSS TO PLATE	FACE NAIL	(6-16d) 8-GUN NAILS (2-16d) 3-GUN NAILS	MUST HAV	INTELS TO BE MINIMAL 3 E CORROSION RESISTANT BASED PAINT.		т	BRICK	ADJ — , BM — B	ADJACENT BEAM
H H	RAFTER TO PLATE	TOE NAIL	(3-8d) 3-GUN NAILS (3-10d) 4-GUN NAILS	LATERALLY	MORE THAN 8'-0". SHOU SUPPORTED NOT TO EX	XCEED 6		WEATHER BARRIER	BOT — BRG —	
4	JACK RAFTER TO HIP ROOF RAFTER TO 2x_ RIDGE BM.	TOE NAIL	(2-16d) 3-GUN NAILS	FT. O.C. w	$1/2-1/_4 \times 3^{"}$ WD. SCREWS ROVIDE A $1/_2^{"}$ VERTICAL	S INTO		LINTEL	DBL – I	DOUBLE DIAMETER
	CONT. HEADER, TWO PIECES CONT. HEADER TO STUD	TOE NAIL	16d@ 16" O.C. @ EDGE (3-16d) 4-GUN NAILS	3. BRICK	VENEER ATTACHMENT: H " O.C., VERT. TIES @ 12		I M	ATTACHMENT	EE - E/	ACH END ENGINEER OF RE
	STUD TO SOLE PLATE SOLE PLATE TO JOIST/BLOCKING	TOE NAIL FACE NAIL	(3-16d) 4-GUN NAILS (16d @ 16") GUN NAIL @	<pre> (FOR 110m 8" 0.C.). AT</pre>	nph WIND-ZONE VERT. TI ALL OPENINGS SPACE TI	ies @ 16" Es within	HEADER,	-	EXT - E FBC - I	EXTERIOR FLORIDA BUILDIN FOUNDATION
AR	3"x0.131"ø = GUN NAILS		113"ø = RINK SHANK		PENINGS. PROVIDE ³ / ₁₆ "ø v 33" O.C. IMMEDIATELY AE		SEE PLAN	BRICK LINTEL, SEE SCHEDULE	FT - F(OOT FOOTING
	$2^{"}x0.113"\phi = 6d$ $3^{"}x0.148"\phi = 10d$ $1^{1}/_{2}"x0.148"\phi = 10dx1^{1}/_{2}"$		$"\phi = 8d$ $(0.162"\phi = 16d)$ $"\phi = 8dx1^{1}/_{2}"$					<u>Section view</u> Brick Lintel		 HORIZONTAL
S	2		-				•			

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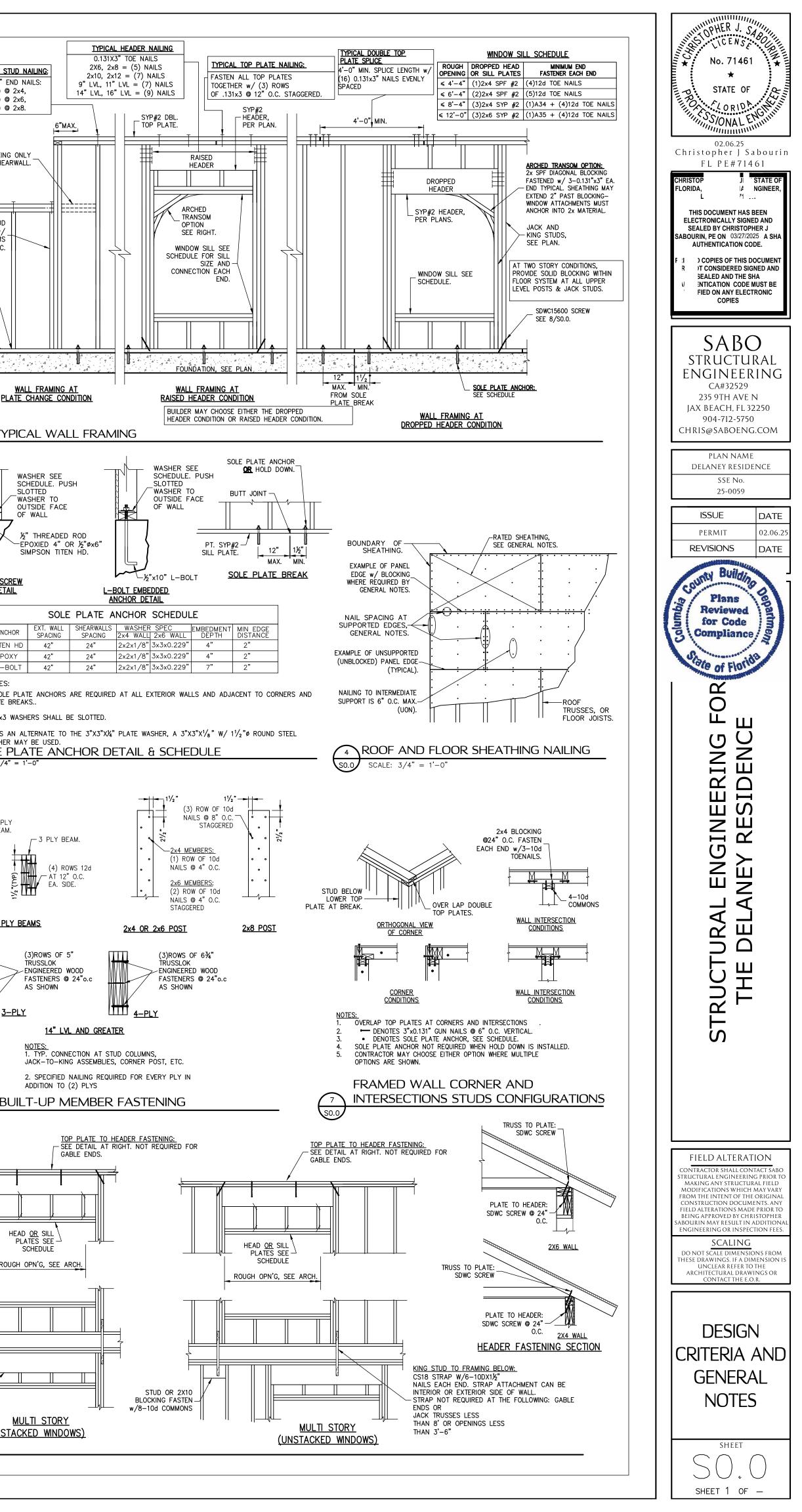
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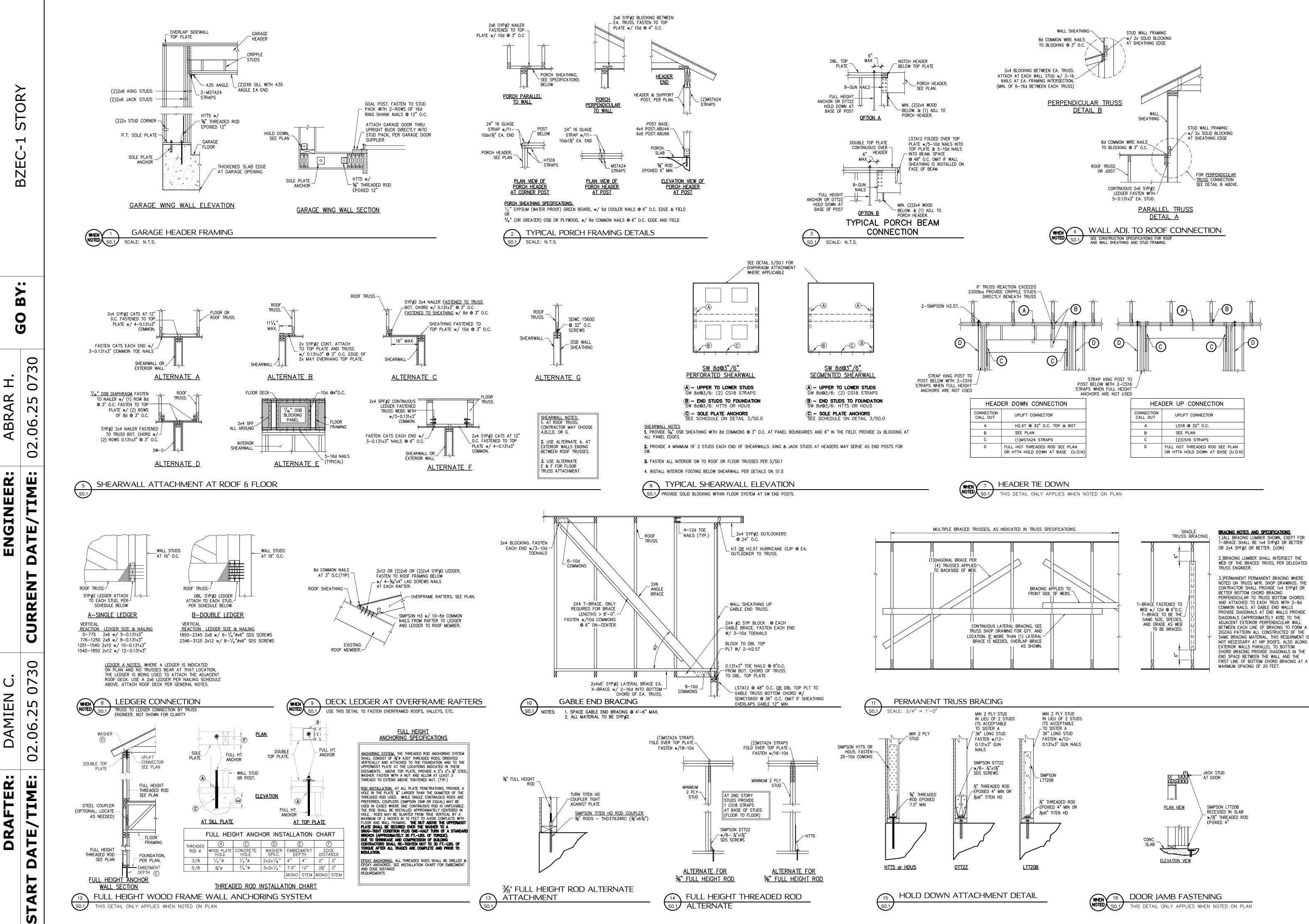
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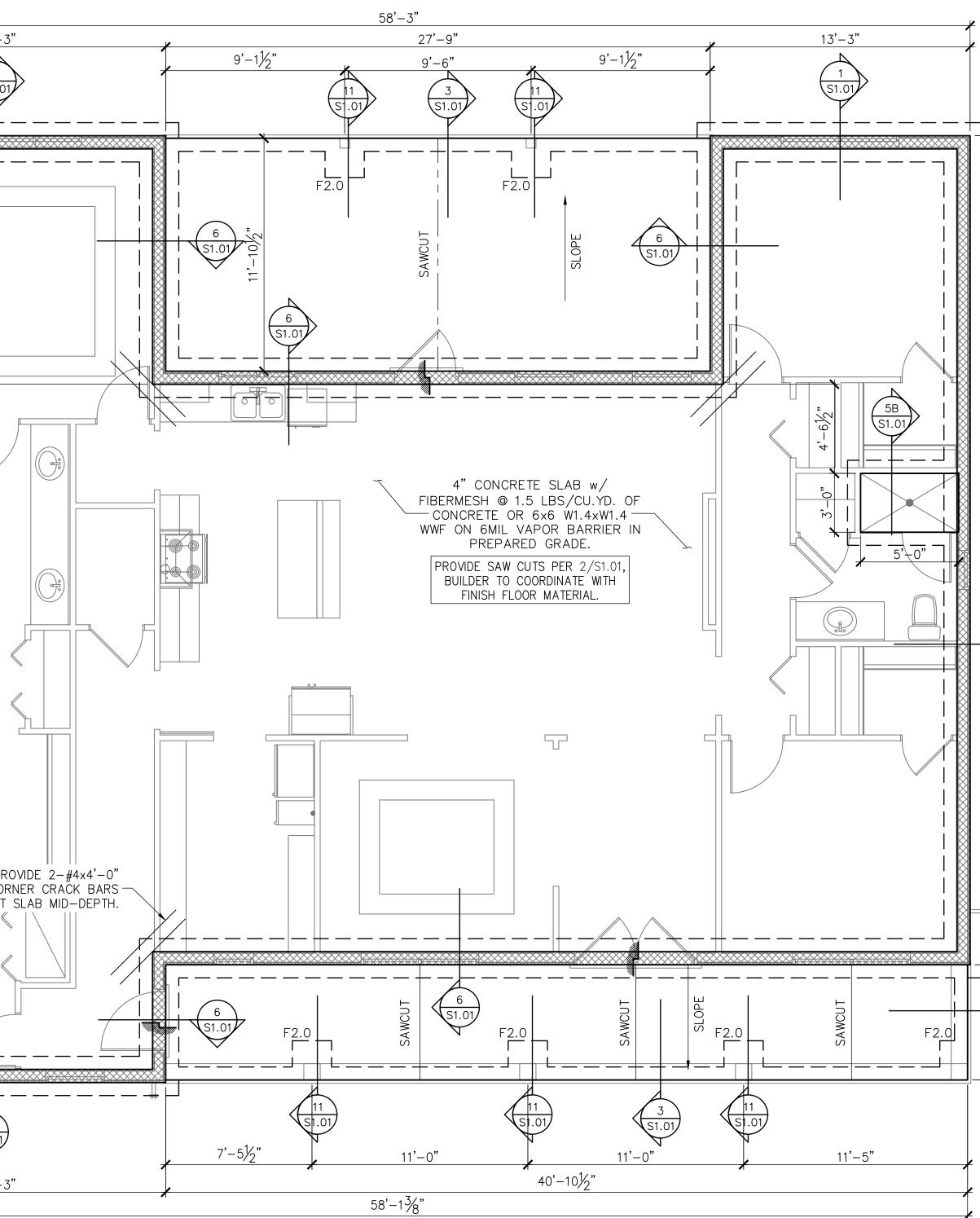
USI		NECTORS		
				Christopher Sabourin c-usation c-usa
SYP	SPF	FASTENERS	FL# CODE	0.131x3" END NAILS: (2) @ 2x4,
450 585	450 495	(9)10dx1 ¹ / ₂ " (5)8d EA. END		(3) @ 2x6, (4) @ 2x8.
775 1195	650 860	(5)10dx1 ¹ / ₂ " EA. END (7)10dx1 ¹ / ₂ " EA. END		TYPICAL WALL FRAMING NOTES:
1450 1640	1245 1455	(12)10dx1 ¹ / ₂ " EA. END (9)10d EA. END		1. USE SPF#2 OR BETTER FOR ALL STUDS
2065 1105	2065 1105	(13)10d EA. END $1/_2$ "Ø ROD TO FTG.		2. USE SYP#2 FOR ALL TOP PLATES AND SOLE PLATES.
1305 4290	1305 4290	(6)10d TO HEADER		3. USE SYP#2 FOR ALL HEADERS
5370	5370	5/8 "Ø ROD TO FTG. 5/8 "Ø ROD TO FTG.		4. ALL WALLS SHALL BE BALLOON
2535 2535		⁵ ∕ ₈ "ø ROD w/ (12)16d ⁵ ∕ ₈ "ø ROD w/ (12)16d		FLOOR BEARING ELEVATION, U.O.N. Image: Constraint of the second seco
1545	1455	(5) ¹ / ₄ "x2- ¹ / ₄ TAPCONS		5.) FASTEN BOTTOM PLATE OF INTERIOR LOAD BEARING WALLS TO
IMPS	SON CO	ONNECTORS		CUT NAILS @ 48" O.C. MINIMUM. SEE CUINDALION PLAN ADDITIONAL
U SYP	PLIFT SPF	FASTENERS	FL# CODE	ANCHORS AT SHEARWALLS HEIGHT IHREADED RODS 0.131x3" TOE NAILS: (3) @ 2x4, (3)
450	450	12-8dx1 ¹ / ₂ "	10446.4	(4) @ 2x6, (5) @ 2x8.
600 1150	520 1085	5-8d EA. END 16-10d EA. END	11478.3 10456.6	INTERIOR BEARING WALL:
1000 1450	860 1245	$7-10dx1\frac{1}{2}$ " EA. END 24-10dx1 $\frac{1}{2}$ " EA. END	10456.3 13872.3	AND O.C. SPACING.
1765	1270	9-10d EA. END	13872.4	WITHIN FLOOR SYSTEM 16d COMMONS FOR SPLICING
2050 3480	3080	13-10d EA. END 18-16d TO TRUSS/BEAM	13872.8	
3400	3080	$1-\frac{5}{8}$ "Ø ROD TO FTG. 32-16d TO TRUSS/BEAM	- 11496.2	8d @ 3"
5250	4670	$1-\frac{5}{8}$ " ϕ ROD TO FTG. 6-10d TO HEADER	- 11496.2	$\begin{array}{c c} \hline \end{array} \\ \hline $ \\ \hline \end{array} \\ \hline \\ \\ \hline \end{array} \\ \hline \end{array} \\ \\ \\ \hline \end{array} \\ \\ \hline \end{array} \\ \\ \\ \hline \end{array} \\ \\ \hline \end{array} \\ \\ \\ \hline \end{array} \\ \\ \\ \\
930	780	4-10d TO JOIST	- 10655.113	
905	785	14-16d TO HEADER 6-16d TO JOIST	10531.36	
2200		$\frac{5}{8}$ " ROD EPOXIED 6" MIN	10849.6	SYP#2 DBL/ SEE 5/SO.0. SEE 1/SO.1/ SEE 5/SO.0. SEE 1/SO.1/ SEE 5/SO.0.
2300 N/A	N/A	5/8"Ø ROD EPOXIED 6" MIN SIMPSON EPOXY-TIE	10849.6 11506.4	BLOCKING ONLY REQUIRED AT SHEARWALLS AND WALL SHEATHING OF WALL
1675	1675	10-16d TO STUD/BEAM/POST $1-\frac{1}{2}$ "ø ROD TO FTG.	- 11496.3	STUCCO FINISH. SEE DETAIL 1/SO.1
805	695	10-10d	13872.5	FULL HEIGHT
1705 DNS	1705	13-8d	10852.1	SHEATHING TO BE CONTINUOUS
				TO UPPER INTERIOR BEARING EPOXY AND SCREW WALL: SEE SCHEDULE FOR STUD SIZE
				AND O.C. SPACING. WALL SHEATHING TO SOLE
O.C. WITHIN	4'-0" OF ROOF I	EDGE).		SOLE PLATE RAISE ½" FROM ANCHOR EXT. WALL
OF ROOF ED		,		8d @ 3" SEE DETAILS BOTTOM OF SOLE PLAN Internet Spacing 0N S1.01 0N S1.01 Internet Spacing Internet Spacing 3" 0.C. EPOXY 42"
E NAIL SP	ACING TO 4" (WICTHIN 4'-0" OF ROOF EDGE).		
TOP PLATE	. FLEXIBLE FINISH	I WALLS INCLUDE: WOOD, CEMENT, OF	R VINYL SIDING,	NOTES: 1. SOLE PLATE ANCHORS /
TO THE STI		R MAY USE ⅔6 STRUCTURAL 1 GRAD	E SHEATHING OR	L PLATE BREAKS
				SINGLE STORY MULTY STORY 2. 3x3 WASHERS SHALL BE 3. AS AN ALTERNATE TO 3
		RESSIVE STRENGTH OF 2000 psi PE	R ASTM C1019	TYP. WALL SECTIONS
		TOR SHALL BE RESPONSIBLE FOR		S0.0 SCALE: $3/4^{"} = 1'-0"$
GROUT AI	L CELLS CONTA	NING VERTICAL REINFORCEMENT IN	5'-0" ΜΑΧΙΜυΜ	
EINFORCED	FULL HEIGHT WI	BOND SHALL NOT BE USED. GROUT TH — #4 @ 4'—0" O.C. MAX. AND / AT STEMWALL CONSTRUCTED OF 5	AT EACH CORNER,	SIMPSON 2 PLY
		ING SHALL BE A MINIMUM OF 6".	OK MORE	SDWC15600 SEE ROOF
IGTH OF 30	00 PSI AT 28 D	AYS		FRAMING PLAN FOR DETAILS. EXTEND SHEATHING VERT. TO TOP OF TRUSS AS SHOWN. (4) ROV
				AT HORIZONTAL JOINTS ROOF SHEATHING, SEE (3) DOWS 124
				HEEL ≥ 1'-0" PRE-ENGINEERED WOOD EA. SIDE.
		DED TO VERIFY SUITABLE SUBSURFA HALL BE COMPACTED TO A MIN. OF		2x SPF BLOCKING RIPPED TO FIT
DIMENSION	S AND LOCATIO	N FROM THE FOUNDATION PLAN SH	OWN ON S1.0.	WITH GAP SHOWN.
		THEN CONTACT ENGINEER OF RECOR	· · /	SIMPSON
	HALL BE 48 BA	C. OR ROD CHAIRS. PROVIDE CONTI R DIAMETERS	NOTT OF	FRAMING PLAN FOR AS SHOWN
TMENT FOR	PREVENTION OF	SUBTERRANEAN TERMITES. SAWCU	TS: _FOR	DETAILS. SIMPSON SIMPSON TERMINATE AT SDWC15600 SIMPSON SDWC15600 SEE ROOF FRAMING SDWC15600 SEE ROOF SDWC15600 SEE ROOF SDWC15600 SEE ROOF
				SDWC15600 TOP OF DBL. SDWC15600 PLAN FOR SEE NOTE 1. TOP PLATE. SEE NOTE 1. DETAILS. <u>14" LVL</u>
		DNRY, CONCRETE OR SOIL SHALL BE BE STAINLESS STEEL.	E	5" < HEEL < 1'-0" TYPICAL CONNECTION NOTES: HEEL < 5"
		ALL TRUSSES AS INDICATED IN THE		NOTES: 1. TOP PLATE TO STUD SDWC ONLY REQUIRED WHEN STUD IS DIRECTLY BELOW TRUSS. 2. H2.5T MAY BE SUBSTITUTED FOR TRUSS CONNECTION. PROVIDE ADDITIONAL H2.5T AT TOP 2. SPECIFIED N
		ATION FOR HANDLING, INSTALLING & ALLY ACROSS DBL. TOP PLATE FRO		PLATE TO STUD, SPACE @ 48" O.C.
		S REQUIREMENTS. CLAY AND TILE R		5 ROUF TRUSS CONNECTION 6 BUILT-UP M
E INSTALLE	D ACCORDING TO	D THE MANUFACTURER'S REQUIREME	ENTS. THE	KING STUD TO TOP PLATE:
				NOT REQUIRED AT GABLE
				SDWC15600 SCREW TOP PLATE TO HEADER FASTENING:
				ENDS OR OPENINGS LESS THAN 4'-6"
EGEN	ND AN	D ABBREVIAT	IONS	
OR LOAD BE	ARING WALL	BUILT-UP POS	ST IN THE WALL	(2)12d TOE HEAD OR SILL
(-BRACE, SEE	DETAIL <u>10/S0.1</u>	(2)2×8-1/2 HEADER SIZE,		MILS E.E PLATES SEE (MIN) SCHEDULE
SIGNATES SIDE	LL. THE HIDDEN OF WALL THE G TO BE APPLIED.	(2)2×8-1/2 HEADER SIZE, KING STUD QU	JANTITY.	HEAD <u>OR</u> SILL <u>HEAD OR</u> SILL <u>KING STUD TO FRAMING BELOW:</u> PLATES SEE <u>CS18 STRAP w/6–10dx1½</u>
6 DESIGNATE	S 8d COMMONS @ .C. "IN THE FIELD"			KING STUD TO BOTTOM PLATE SCHEDULE NAILS EACH END. STRAP ATTACHMENT KING STUD TO BOTTOM PLATE CAN BE INTERIOR OR EXTERIOR SIDE
				SDWC15600 SCREW. OMIT IF SHEARWALL HOLD DOWN IS
ACENT 1		LG — Long MANUF — Manufacture MONO — Manalithia		SPECIFIED. NOT REQUIRED AT JACK TRUSSES LESS JALE STORY THAN 8' OR OPENINGS LESS JALE STORY
TOM RING ICRETE MASI	ONRY UNIT	MONO — Monolithic OC — On Center OSB — Oriented Strand Board		WALLS THAN 3'-6"
BLE ETER		PERP — Perpendicular PRE ENG — Pre Engineered PSF — Pounds per Square Foot		
END INEER OF RE	ECORD	PSI — Pounds per Square Inch PT — PRESSURE TREATED		
L ERIOR RIDA BUILDIN		QT — Quick Tie REINF — Reinforce SF — Square Foot		SOLE PLATE CONNECTOR STRAPPING NOT REQUIRED LOCATED WITHIN 6" OF PER SCHEDULE AT LOWER LEVEL HEADERS
NDATION		SPF —Spruce Pine Fur SYP — Southern Yellow Pine		STUD PACK. OMIT IF SHEARWALL HOLD DOWN IS SPECIFIED SINGLE STORY UNLESS SPECIFIED ON PLAN MULTI STORY SINGLE STORY (STACKED WINDO)
TING .DER DRIZONTAL		THRU — Through TYP — Typical UON — Unless Otherwise Noted		TYPICAL HEADER STRAPPING
NDS		VERT - Vertical WWF - Welded Wire Fabric		50.0







ABRAR H.	02.06.25 0730	
ENGINEER:	NT DATE/TIME:	
	CURRENT	PROV CORNE AT SL
DAMIEN C.	02.06.25 0730	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
DRAFTER:	START DATE/TIME:	



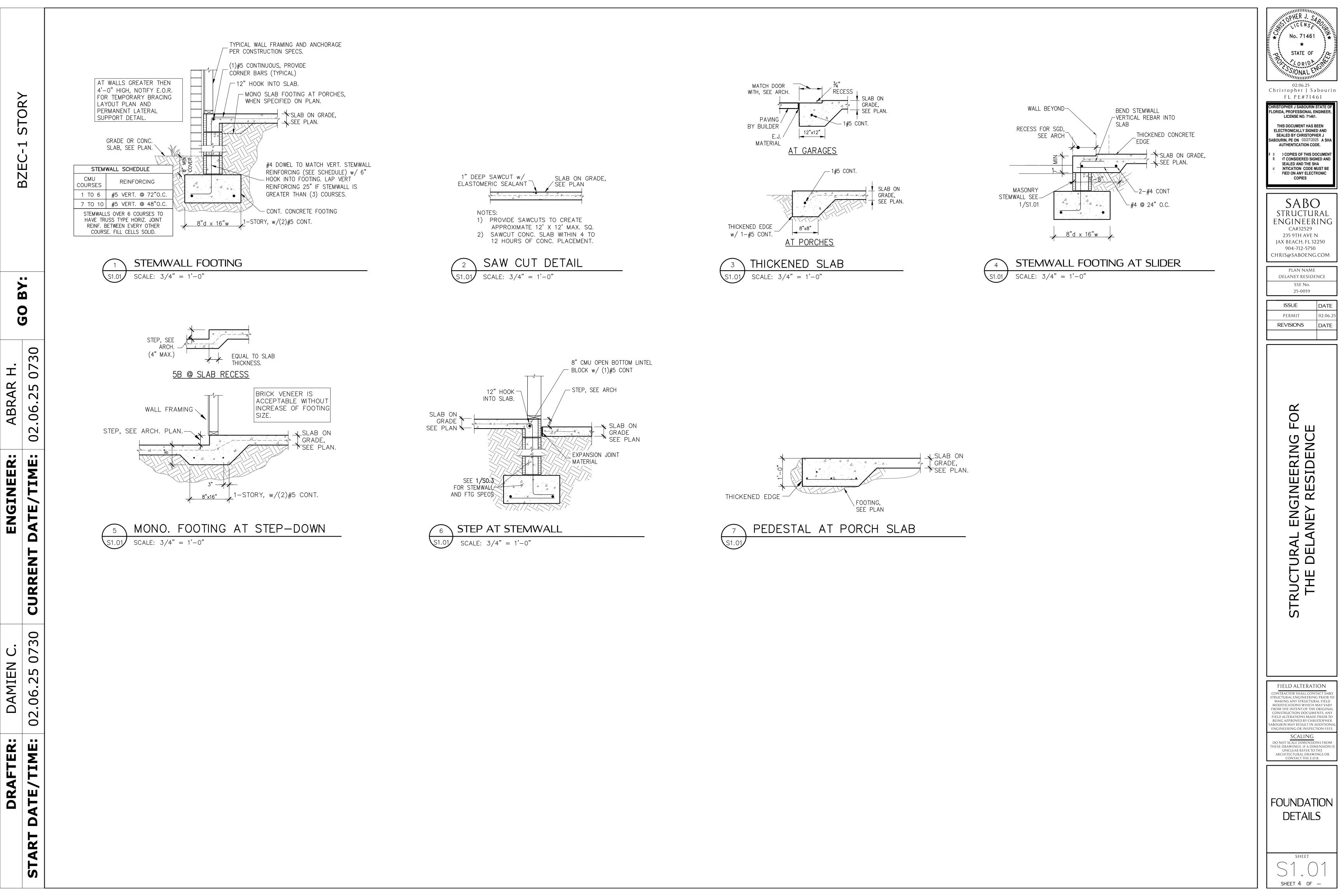
FOUNDATION PLAN SCALE: 1/4" = 1'-0"

	FOUNDATION LEGEND	No. 71461
	DESIGNATES SLAB EDGE LINE DESIGNATES FOOTING LINE	STATE OF
	DESIGNATES SAWCUT LINE	STATE OF CORIDA
	DESIGNATES STEMWALL DESIGNATES SLAB STEP RECESS	02.06.25
	FOOTING SCHEDULE	Christopher J Sabourin FL PE#71461
TYPEF2.0	DEPTH WIDTH BOTTOM BARS 1'-0" 2'-0"x2'-0" (3) #5 EW	CHRISTOPHER J SABOURIN STATE OF FLORIDA, PROFESSIONAL ENGINEER, LICENSE NO. 71461.
F2.5 F3.0	1'-0" 2'-6"x2'-6" (3) #5 EW 1'-0" 3'-0"x3'-0" (3) #5 EW	THIS DOCUMENT HAS BEEN ELECTRONICALLY SIGNED AND SEALED BY CHRISTOPHER J SABOURIN, PE ON 03/27/2025 A SHA
F3.5 F4.0	1'-0"3'-6"x3'-6"(4) #5 EW1'-4"4'-0"x4'-0"(4) #5 EW	AUTHENTICATION CODE.
	GENERAL FOUNDATION NOTES	R DT CONSIDERED SIGNED AND SEALED AND THE SHA M ENTICATION CODE MUST BE
SEE ARCH FO	TION PLAN ONLY CONVEYS STRUCTURAL INFORMATION R DIMENSIONS NOTES AND SPECIFICATIONS ON SO.0 FOR FEATURES	COPIES
	D WITHIN THIS PLAN. D FOUNDATIONS SHALL BE IN ACCORDANCE WITH	SABO
4. SOIL COMPAC	TON AND FILL SHALL BE COMPACTED TO A MIN. OF 95% CTOR IN ACCORDANCE WITH ASTM D 1557.	
		CA#32529 235 9TH AVE N JAX BEACH, FL 32250
CONTRAC	TOR TO VERIFY DIMENSIONS	904-712-5750 CHRIS@SABOENG.COM
		PLAN NAME Delaney residence
		SSE No. 25-0059
		ISSUE DATE
		PERMIT 02.06.25 REVISIONS DATE
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		STRUCTURAL ENGINEE THE DELANEY RESI
		<u>5</u> <u>H</u>
		ST ST
		FIELD ALTERATION
		CONTRACTOR SHALL CONTACT SABO STRUCTURAL ENGINEERING PRIOR TO MAKING ANY STRUCTURAL FIELD MODIFICATIONS WHICH MAY VARY
		FROM THE INTENT OF THE ORIGINAL CONSTRUCTION DOCUMENTS. ANY FIELD ALTERATIONS MADE PRIOR TO BEING APPROVED BY CHRISTOPHER
		SABOURIN MAY RESULT IN ADDITIONAL ENGINEERING OR INSPECTION FEES.
		DO NOT SCALE DIMENSIONS FROM These drawings. IF a dimension is Unclear Refer to the Architectural drawings or Contact the E.O.R.
		FOUNDATION
		PLAN
		SHEET

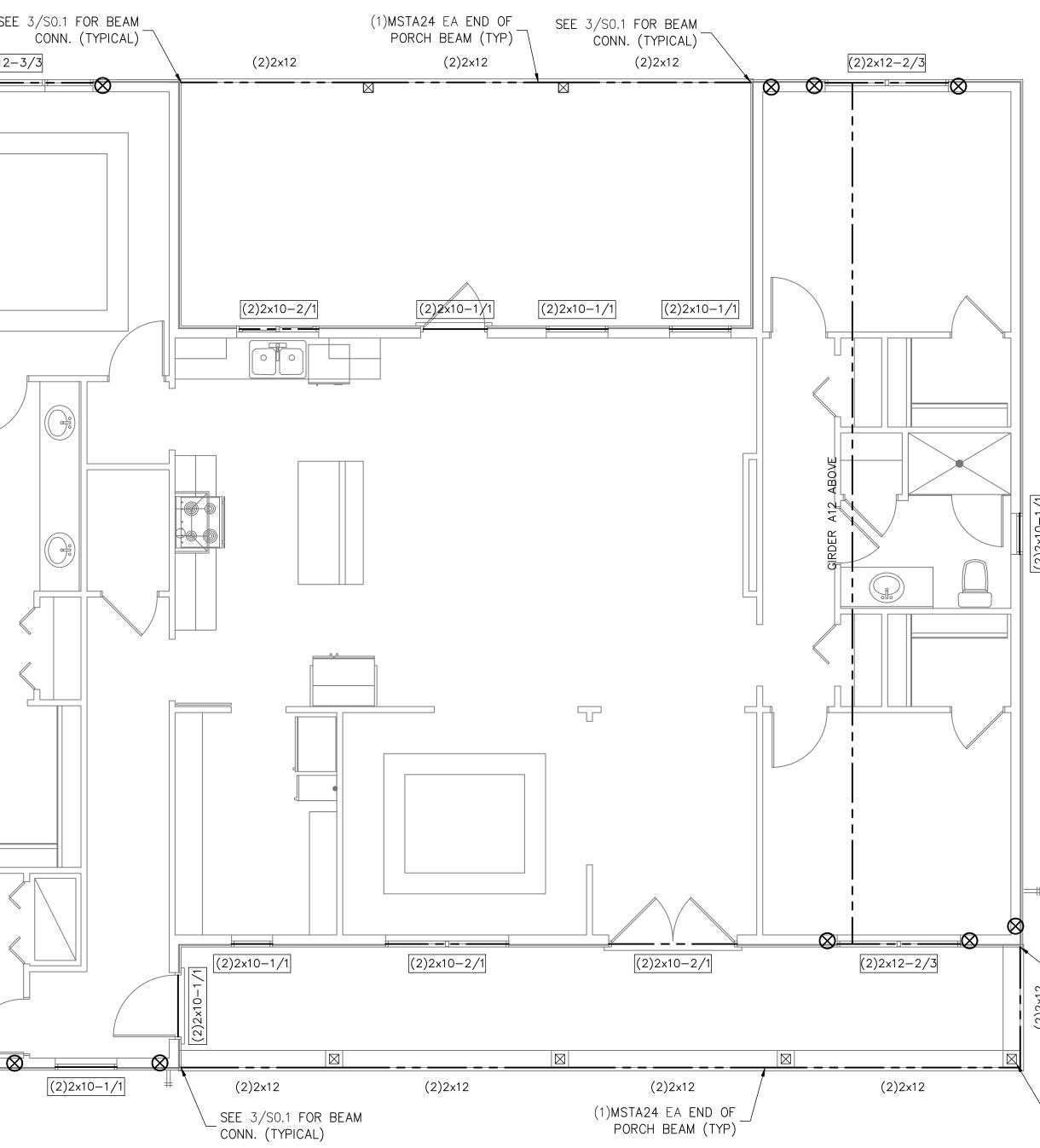
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S1.01

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ENGINEER:	CURRENT DATE/TIME: 0	
DAMIEN C.	02.06.25 0730	(2)2x10-1/1
DRAFTER:	START DATE/TIME:	



FIRST LEVEL WALL FRAMING PLAN

	_	S LEGEND		
SW: 3/12	나 LINE DESI SHEARWAI 로 DESIGNA	DESIGNATES OSB SHEARWALL. THE HIDDEN LINE DESIGNATES SIDE OF WALL THE SHEARWALL SHEATHING TO BE APPLIED. 8d @ § DESIGNATES 8d COMMONS @ 3" O.C. EDGE & 12" O.C. "IN THE FIELD" DESIGNATES THE HEADER SIZE, NUMBER OF PLY'S & JACK/KING STUDS NEEDED FOR SUPPORT HEADER.		
(2)2x8-1/	/2 PLY'S &			
 A		r truss, see plan		
X		AMETER FULL HEIGHT ROD, SEE DETAIL 12/S0.1		
X 5		AMETER FULL HEIGHT ROD, SEE DETAIL 12/S0.1		
\otimes	TERMINATES	IAMETER THREADED ROD AT FIRST FLOOR TOP DETAIL 12/S0.1		
⊗5	TERMINATES	IAMETER THREADED ROD AT FIRST FLOOR TOP DETAIL 12/S0.1		
	SIMPSON H	ITT5 SEE DETAIL 15/S0.1		
	SIMPSON D	TT2Z SEE DETAIL 15/S0.1		
\bigcirc		TT20B SEE DETAIL 15/S0.1		
WAL		SCHEDULE STUD SIZE		
	PLATE HEIGHT 9'-1"	& SPACING		
EXTERIOR	MAX 10'-1	2x4 SPF#2 @ 16" O.C. 2x6 SPF#2 @ 16" O.C. <u>or</u> 2x4 SPF#2 @ 12" O.C.		
EXTERIOR	MAX 10'-1 TO 14'-			
INTERIOR	10'-0" MAX 12'-0"	2x4 SPF#2 @ 16" O.C.		
	12'-0" MAX	2x6 SPF#2 @ 16" O.C. <u>or</u> 2x4 SPF#2 @ 12" O.C.		
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No. 71461 * STATE OF STATE OF STATE OF 02.06.25 Christopher J Sa FL PE#7140 CHRISTOPHER J SABOURINS FLORIDA, PROFESSIONAL EN LICENSE NO. 71461. THIS DOCUMENT HAS B ELECTRONICALLY SIGNEI SEALED BY CHRISTOPH SABOURIN, PE ON 03/27/202 AUTHENTICATION CODE F. INTECOPIES OF THIS DOC R JT CONSIDERED SIGN SEALED AND THE SH SEALED ON ANY ELECTF COPIES	bourin 61 STATE OF IGINEER, EEN DAND ERJ 5 A SHA E. CUMENT IED AND A UST BE
SABC STRUCTUF ENGINEER CA#32529 235 9TH AVE N JAX BEACH, FL 32 904-712-5750 CHRIS@SABOENG. PLAN NAME DELANEY RESIDE SSE No. 25-0059	RAL ING ¹ 250 COM NCE
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STRUCTURAL ENGINEERING FOR THE DELANEY RESIDENCE	
FIELD ALTERATI CONTRACTOR SHALL CONT STRUCTURAL ENGINEERING MAKING ANY STRUCTUR, MODIFICATIONS WHICH M FROM THE INTENT OF THE CONSTRUCTION DOCUME FIELD ALTERATIONS MADE BEING APPROVED BY CHRI SABOURIN MAY RESULT IN A ENGINEERING OR INSPECT <u>SCALING</u> DO NOT SCALE DIMENSION THESE DRAWINGS. IF A DIM UNCLEAR REFER TO T ARCHITECTURAL DRAWIL	ACT SABO G PRIOR TO AL FIELD AAY VARY DRIGINAL NTS. ANY PRIOR TO STOPHER DDITIONAL ION FEES. NS FROM ENSION IS THE NGS OR
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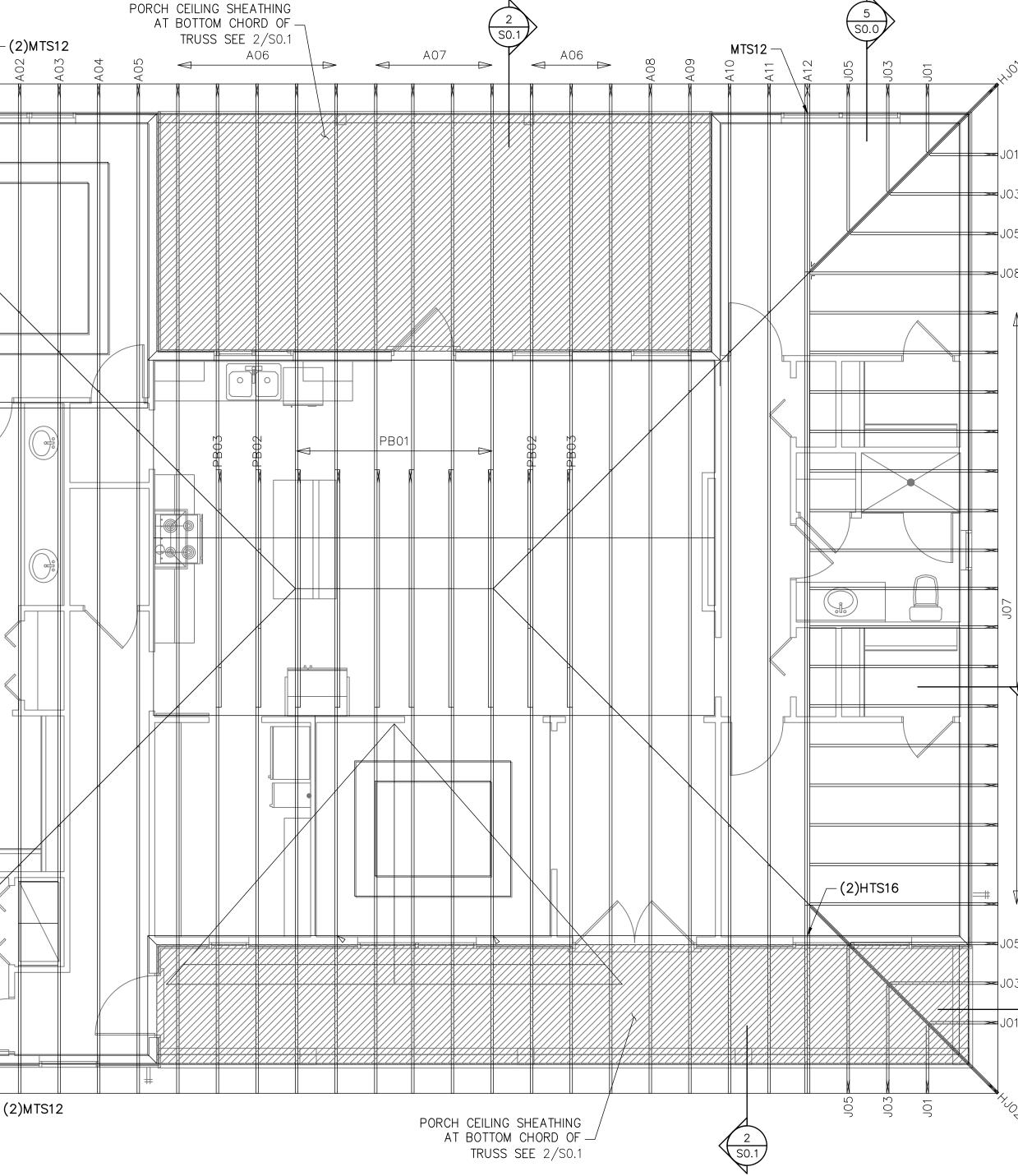
S1,1 Sheet 5 of -

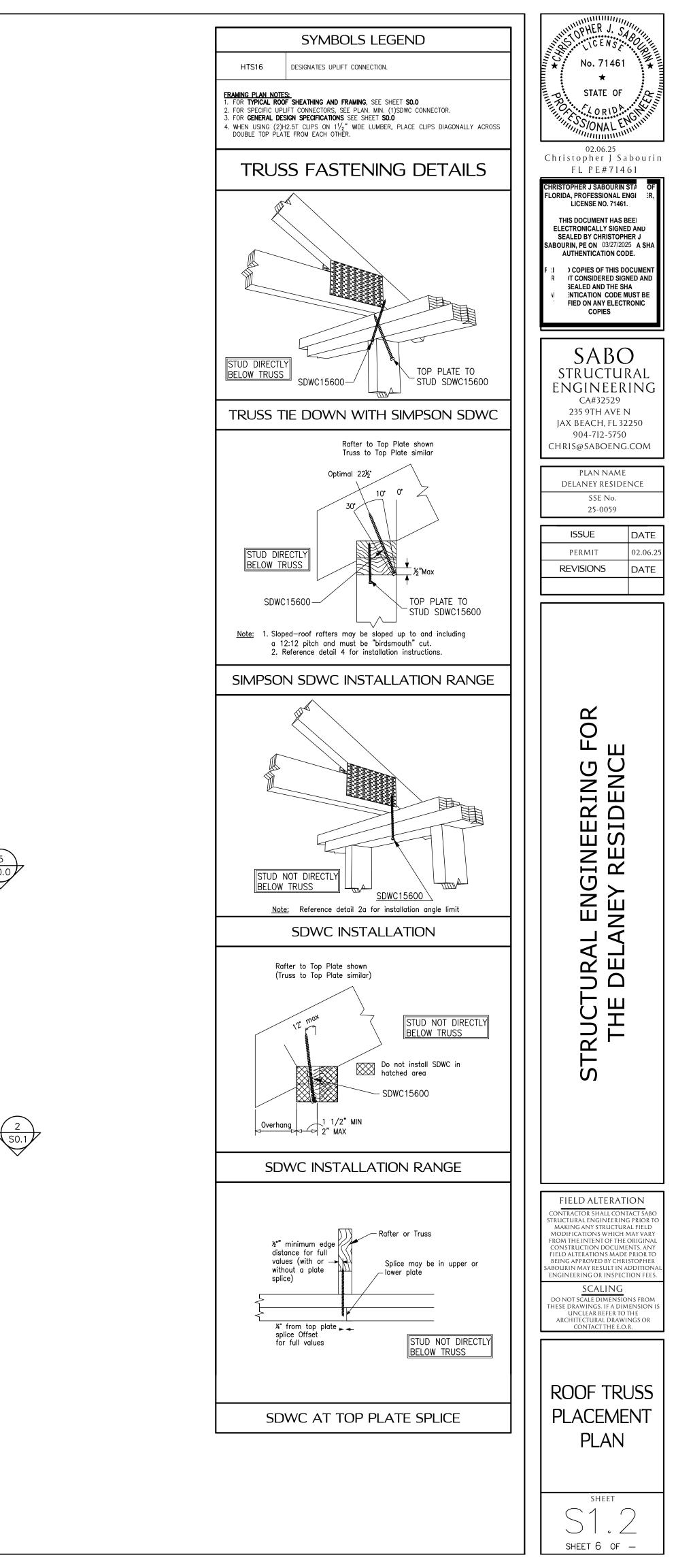
6x6 SYP#2 POST w/ABU66. SEE DETAIL 2/S0.1 (TYPICAL)

ABRAR H.	02.06.25 0730		
ENGINEER:	NT DATE/TIME:		
	CURRENT		
DAMIEN C.	02.06.25 0730		
DRAFTER:	START DATE/TIME:		
	V)		

TRUSS / ROOF RAFTER NOTES: STRAPPING NOTES: STRAP ROOF TRUSSES AND RAFTERS TO BEARING WITH (2)12D TOENAILS & (1)SIMPSON SDWC15600 SCREW UNLESS







_____(0)