DATE 12/07/2005

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000023939

APPLICANT HUGO ESCALANTE PHONE 386-288-8666

ADDRESS 6210 SW CR 18 FORT WHITE FL 32038

OWNER HBM CONSTRUCTION PHONE 813-209-0363

ADDRESS 177 SW GERALD CONNER DR LAKE CITY FL 32024

CONTRACTOR HUGO ESCALANTE PHONE 386-288-8666

LOCATION OF PROPERTY 47 S, R 242, R SW CANNON CREEK DR, R SW GERALD CONNER DR, 3RD ON RIGHT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 96600.00

HEATED FLOOR AREA 1932.00 TOTAL AREA 2640.00 HEIGHT 20.60 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 24-4S-16-03114-103 SUBDIVISION CANNON CREEK PLACE

LOT 3 BLOCK PHASE UNIT TOTAL ACRES .50

000000911 (Culvert Permit No.) | (Culvert Waiver) | CRC1326967 (Contractor's License Number) | Hugo Escalante (Applicant/Owner/Contractor)

PERMIT (Driveway Connection) | 05-1113-N (Septic Tank Number) | BK (LU & Zoning checked by) | JH (Approved for Issuance) | N (New Resident)

COMMENTS: 1ST FLOOR ELEVATION TO BE 102.0FT, ELEVATION LETTER REQUIRED

BEFORE SLAB

NOC ON FILE Check # or Cash 3495

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power ____ date/app. by | Foundation ____ date/app. by | Monolithic ____ date/app. by

Under slab rough-in plumbing ____ date/app. by | Slab ____ date/app. by | Sheathing/Nailing ____ date/app. by

Framing ____ date/app. by | Rough-in plumbing above slab and below wood floor ____ date/app. by

Electrical rough-in ____ date/app. by | Heat & Air Duct ____ date/app. by | Peri. beam (Lintel) ____ date/app. by

Permanent power ____ date/app. by | C.O. Final ____ date/app. by | Culvert ____ date/app. by

M/H tie downs, blocking, electricity and plumbing ____ date/app. by | Pool ____ date/app. by

Reconnection ____ date/app. by | Pump pole ____ date/app. by | Utility Pole ____ date/app. by

M/H Pole ____ date/app. by | Travel Trailer ____ date/app. by | Re-roof ____ date/app. by

BUILDING PERMIT FEE $ 485.00 CERTIFICATION FEE $ 13.20 SURCHARGE FEE $ 13.20

MISC. FEES $ .00 ZONING CERT. FEE $ 50.00 FIRE FEE $ .00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 611.40

INSPECTORS OFFICE L. Hodges CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application 611.40

Revised 9-23-04

For Office Use Only Application # 0511-102 Date Received 11/29/05 By G Permit # 911/23939

Application Approved by - Zoning Official BLK Date 1.12.05 Plans Examiner OK JTH Date 12-1-05

Flood Zone X per PLAT Development Permit N/A Zoning RSF-2 Land Use Plan Map Category RES. Low Dev.

Comments 1st Floor Elevation to be 102.0 ft Elevation letter Required

Applicants Name Hugo Escalante Phone 386-288-8666

Address 6210 S.W. CR 18, Fort White, FL 32038

Owners Name HBM Construction Phone 813-209-0363

911 Address 177 S.W. Gerald Corner DR; lake City, FL

Contractors Name Hugo Escalante, EWPL INC Phone 386-288-8666

Address 6210 S.W. CR 18 FT White, FL 32038

Fee Simple Owner Name & Address None

Bonding Co. Name & Address None

Architect/Engineer Name & Address Daniel Shaheen, Lake City, FL 365-1892

Mortgage Lenders Name & Address

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progressive Energy

Property ID Number 34-4S-16-03114-103 Estimated Cost of Construction $135,000

Subdivision Name Canoe Creek Cannon Creek Place Lot 3 Block ___ Unit ___ Phase ___

Driving Directions 47 S, R 242, R SW Cannon Creek Dr., (R) SW Gerald Conner Dr. 3rd on (R)

Type of Construction New Single Family Dwelling Number of Existing Dwellings on Property ___

Total Acreage .5 Acre Lot Size 1/2 Acre Do you need a - (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 50' Side 45' Side 45' Rear 80'

Total Building Height 20'-6" Number of Stories 1 Heated Floor Area 1932 SF Roof Pitch 6-12

Porches 227 Garage = 481 Living 1932 TOTAL 2640

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

Hugo Escalante
Contractor Signature

Contractors License Number CRC1326967

Competency Card Number ___

NOTARY STAMP/SEAL

CARRIE L. REVELL
MY COMMISSION # DD 181630
EXPIRES: February 11, 2007
Bonded Thru Notary Public Underwriters

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me

this 29th day of November 2005.

Personally known X or Produced Identification ___

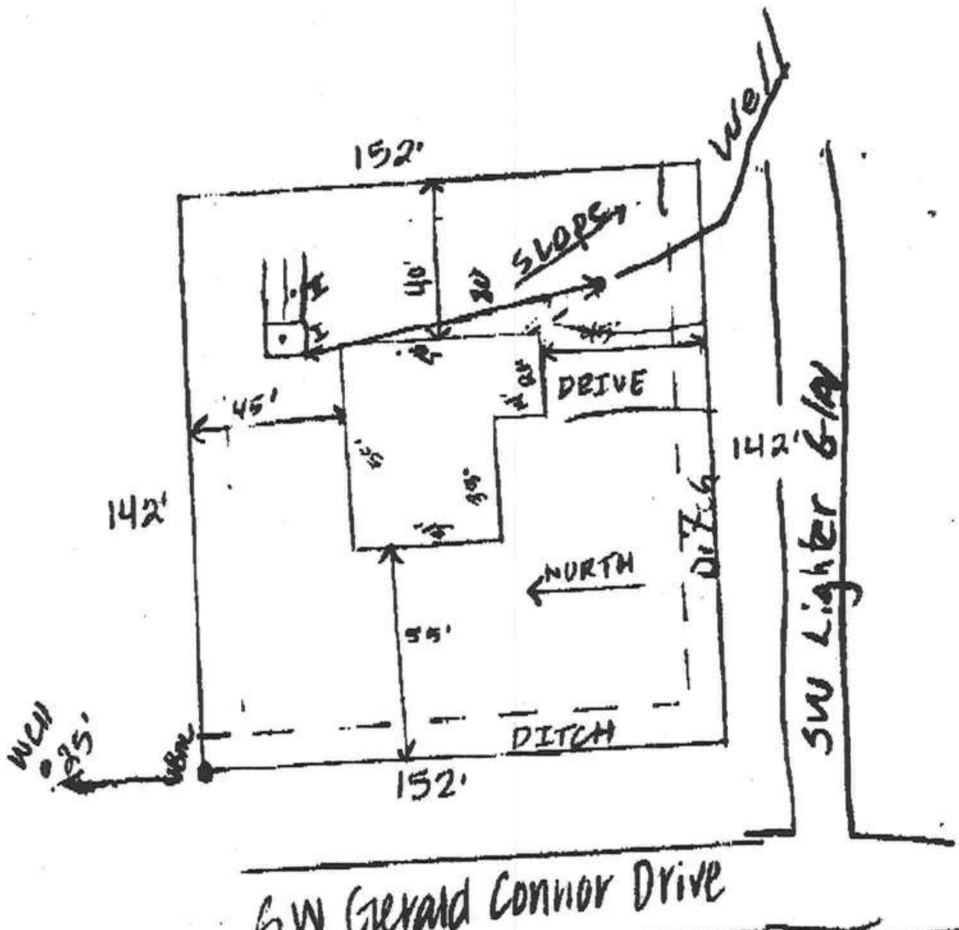
Carrie L. Revell
Notary Signature

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 05-1113N

----- PART II - SITEPLAN -----

: 1 inch = 50 feet.



tes: ____

s Plan submitted by: ________ **MASTER CONTRACTOR**

Not Approved ________ Date 10.31.05

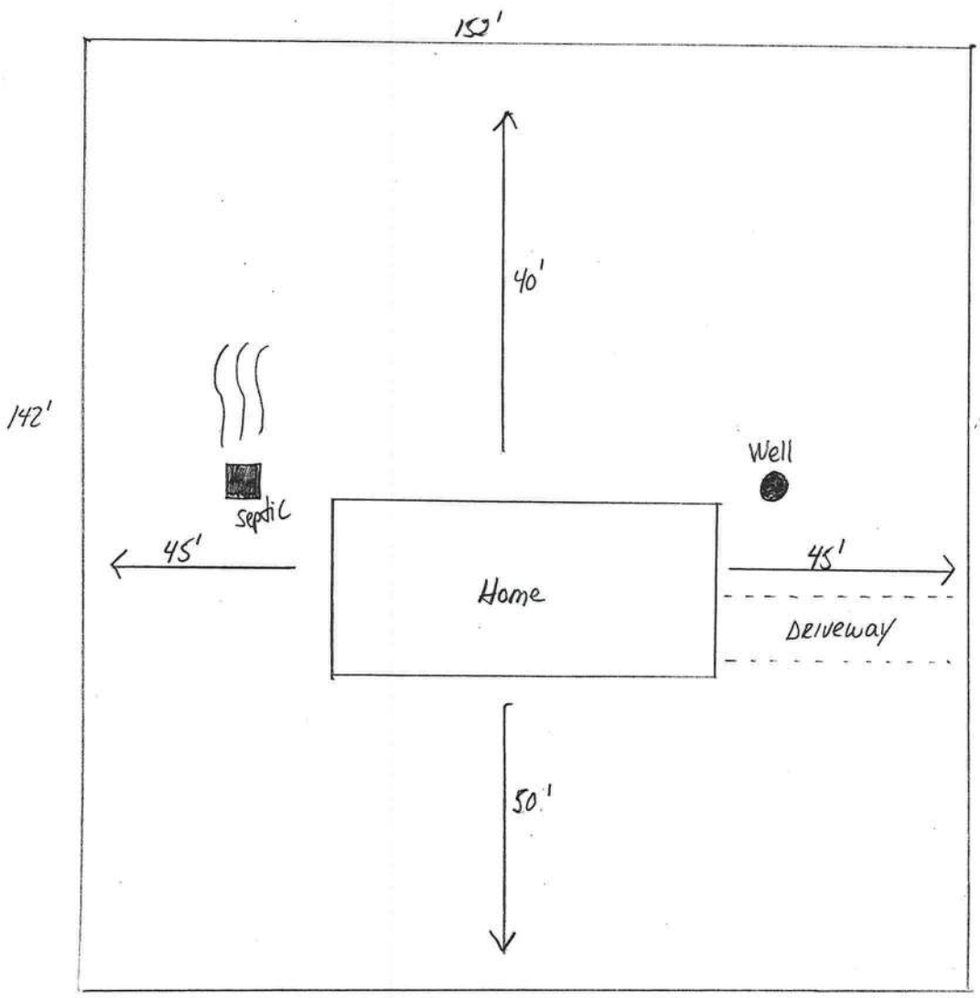
n Approved X Sallie Gaddy · ESI COLUMBIA ________ County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

(0/13, 10/96 (Replaces HRS-H Form 4015 which may be used)
ck Number: 5744-002-4015-6)

COL. CO. HEALTH DEPT. ID:386-758-2187 OCT 28'05 14:55 No.002 P.01

Lot 3 Canoe Creek S/D.
W/D 1056-2031
Parcel # 24-4$-16-03114-103



SW Gerald Conner Drive

Columbia County Property Appraiser

DB Last Updated: 10/21/2005

Parcel: 24-4S-16-03114-103

2006 Proposed Values

Tax Record | Property Card | Interactive GIS Map | Print

Search Result: 1 of 14 Next >>

Owner & Property Info

Owner's Name	H & M CONSTRUCTION CORP.
Site Address	
Mailing Address	10155 COLLINS AVE. STE. 1004 BAL HARBOUR, FL 33154
Brief Legal	LOT 3 CANNON CREEK PLACE S/D. WD 1056-2031.

Use Desc. (code)	VACANT (000000)
Neighborhood	24416.00
Tax District	2
UD Codes	MKTA06
Market Area	06
Total Land Area	0.510 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	$36,000.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$36,000.00

Just Value	$36,000.00
Class Value	$0.00
Assessed Value	$36,000.00
Exempt Value	$0.00
Total Taxable Value	$36,000.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
8/22/2005	1056/2031	WD	V	Q		$468,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1.000 LT - (.510AC)	1.00/1.00/1.00/1.00	$36,000.00	$36,000.00

Columbia County Property Appraiser

DB Last Updated: 10/21/2005

1 of 14 Next >>

Disclaimer

This information was derived from data which was compiled by the Columbia County Property Appraiser's Office solely for the government purpose of property assessment. The information shown is a **work in progress** and should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's Office. The assessed values are **NOT CERTIFIED** values and therefore are subject to change before finalized for ad-valorem assessment purposes.

Columbia County 9-1-1 Addressing / GIS Department
Address Assignment Data
Cannon Creek Place Subdivision, Section 24, Township 4 South, Range 16 East
Columbia County, Florida

LOT#:	ADDRESS ASSIGNED
1	123 SW GERALD CONNER DR
2	149 SW GERALD CONNER DR
3*	177 SW GERALD CONNER DR
3*	121 SW LIGHTER GLN
4	147 SW LIGHTER GLN
5	161 SW LIGHTER GLN
6	160 SW LIGHTER GLN
7	146 SW LIGHTER GLN
8*	120 SW LIGHTER GLN
8*	217 SW GERALD CONNER DR *
9*	243 SW GERALD CONNER DR
9*	119 SW ARROW GLN
10	143 SW ARROW GLN
11	171 SW ARROW GLN
12	176 SW ARROW GLN
13	156 SW ARROW GLN
14*	122 SW ARROW GLN
14*	281 SW GERALD CONNER DR
15*	307 SW GERALD CONNER DR
15*	119 SW ARROWBEND DR
16	143 SW ARROWBEND DR
17	161 SW ARROWBEND DR
18*	179 SW ARROWBEND DR *
18*	123 SW HAVER HILL GLN
19	139 SW HAVER HILL GLN
20	138 SW HAVER HILL GLN
21	130 SW HAVER HILL GLN
22*	114 SW HAVER HILL GLN
22*	225 SW ARROWBEND DR
23	247 SW ARROWBEND DR
24	261 SW ARROWBEND DR

LOT#:	ADDRESS ASSIGNED
25	275 SW ARROWBEND DR
26	293 SW ARROWBEND DR
27	315 SW ARROWBEND DR
28	335 SW ARROWBEND DR
29	351 SW ARROWBEND DR
30	350 SW ARROWBEND DR
31	334 SW ARROWBEND DR
32	314 SW ARROWBEND DR
33	292 SW ARROWBEND DR
34	262 SW ARROWBEND DR
35	228 SW ARROWBEND DR
36	204 SW ARROWBEND DR
37	176 SW ARROWBEND DR
38	142 SW ARROWBEND DR
39*	116 SW ARROWBEND DR
39*	353 SW GERALD CONNER DR
40	364 SW GERALD CONNER DR
41	332 SW GERALD CONNER DR
42	306 SW GERALD CONNER DR
43	280 SW GERALD CONNER DR
44	254 SW GERALD CONNER DR
45	228 SW GERALD CONNER DR
46	200 SW GERALD CONNER DR
47	176 SW GERALD CONNER DR
48	148 SW GERALD CONNER DR
49	122 SW GERALD CONNER DR

(NOTE: * IDENTIFIES CORNER LOTS. CONTACT THE 9-1-1 ADDRESSING DEPARTMENT FOR CORRECT ADDRESS.)

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 24-4S-16-03114-103

1. **Description of property: (legal description of the property and street address or 911 address)**
   Lot 3 Cannon Creek Place S/D. WD 1056-2031
   177 S.W. Gerald Corner DR.
2. **General description of improvement:** New Single Family Residence
3. **Owner Name & Address** H&M Construction Corp. 10155 Collins Ave. STE 1004
   Bal Harbour, FL 33154 **Interest in Property** 100%
4. **Name & Address of Fee Simple Owner (if other than owner):** N/A
5. **Contractor Name** Hugo Escalante **Phone Number** 386-288-8666
   Address 6210 S.W. CR 18 Ford White, FL 32038
6. **Surety Holders Name** N/A **Phone Number**
   Address N/A
   Amount of Bond N/A
   Inst:2005029416 Date:11/29/2005 Time:11:19 MK DC,P.DeWitt Cason,Columbia County B:1066 P:778
7. **Lender Name**
   Address
8. **Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:**
   Name Hugo Escalante **Phone Number** 386-288-8666
   Address P.O. BOX 280, Ford White, FL 32038
9. **In addition to himself/herself the owner designates** Hugo Escalante **of** Ford White, FL **to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) – (a) 7. Phone Number of the designee** 386-288-8666
10. **Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified)**

NOTICE AS PER CHAPTER 713, Florida Statutes:
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Hugo Escalante
Signature of Owner

Sworn to (or affirmed) and subscribed before day of 29, **20** 05

CARRIE L. REVELLE
MY COMMISSION # DD 181697
EXPIRES: February 11, 2007
Bonded Thru Notary Public Underwriters

NOTARY STAMP/SEAL

Carrie L. Revelle
Signature of Notary

LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave
Lake City, FL. 32025
Phone 386-752-6677
Fax 386-752-1477

Cannon Creek Place Lot 3

Building Permit # _____ Owner's Name EWPH, Inc

Well Depth _____ Ft. Casing Depth _____ Ft. Water Level _____ Ft.

Casing Size 4 inch Steel Pump Installation: Deep Well Submersible

Pump Make Red Jacket Pump Model FCF2118 HP 1

System Pressure (PSI) _____ On 30 Off 50 Average Pressure 40

Pumping System GPM at average pressure and pumping level 20 (GPM)

Tank Installation: Precharged Bladder Make Challenger Model PC244 Size 81

Tank Draw-down per cycle at system pressure 25.1 gallons

I HEREBY VERTIFY THAT THIS WATER WELL SYSTEM HAS BEEN INSTALLED AS PER THE ABOVE INFORMATION.

Linda Newcomb
Signature

Linda Newcomb
Print Name

2609
License Number

11-29-05
Date

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**THE NATHAN 4-BED**	Builder:	**EWPL INC.**
Address:	**Lot: 3, Sub: Cannon Creek, Plat:**	Permitting Office:	Columbia
City, State:	**Lake City, FL 32024-**	Permit Number:	23939
Owner:	**EWPL INC**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1.	New construction or existing	New		12.	Cooling systems		
2.	Single family or multi-family	Single family		a.	Central Unit	Cap: 36.0 kBtu/hr	
3.	Number of units, if multi-family	1				SEER: 12.00	
4.	Number of Bedrooms	4		b.	N/A		
5.	Is this a worst case?	No					
6.	Conditioned floor area (ft²)	1932 ft²		c.	N/A		
7.	Glass area & type						
a.	Clear - single pane	0.0 ft²		13.	Heating systems		
b.	Clear - double pane	339.0 ft²		a.	Electric Heat Pump	Cap: 36.0 kBtu/hr	
c.	Tint/other SHGC - single pane	0.0 ft²				HSPF: 6.80	
d.	Tint/other SHGC - double pane	0.0 ft²		b.	N/A		
8.	Floor types						
a.	Slab-On-Grade Edge Insulation	R=0.0, 204.0(p) ft		c.	N/A		
b.	N/A						
c.	N/A			14.	Hot water systems		
9.	Wall types			a.	Electric Resistance	Cap: 50.0 gallons	
a.	Frame, Wood, Adjacent	R=13.0, 232.0 ft²				EF: 0.88	
b.	Frame, Wood, Exterior	R=13.0, 1660.0 ft²		b.	N/A		
c.	N/A						
d.	N/A			c.	Conservation credits		
e.	N/A				(HR-Heat recovery, Solar		
10.	Ceiling types				DHP-Dedicated heat pump)		
a.	Under Attic	R=30.0, 1932.0 ft²		15.	HVAC credits		
b.	N/A				(CF-Ceiling fan, CV-Cross ventilation,		
c.	N/A				HF-Whole house fan,		
11.	Ducts				PT-Programmable Thermostat,		
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 125.0 ft			MZ-C-Multizone cooling,		
b.	N/A				MZ-H-Multizone heating)		

Glass/Floor Area: 0.18

Total as-built points: 29571
Total base points: 32701

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 10-11-05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA
IN GOD WE TRUST

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 3, Sub: Cannon Creek, Plat: , Lake City, FL, 32024-** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 3, Sub: Cannon Creek, Plat: , Lake City, FL, 32024-** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
4		2746.00	10984.0	50.0	0.88	4	1.00	2746.00	1.00	10984.0	
				As-Built Total:						**10984.0**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
11044	**10673**	**10984**	**32701**	**8291**	**10296**	**10984**	**29571**

PASS

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 3, Sub: Cannon Creek, Plat: , Lake City, FL, 32024-** PERMIT #:

BASE			AS-BUILT						
Winter Base Points:		**17012.0**	**Winter As-Built Points:**						**17666.1**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
17012.0	**0.6274**	**10673.3**	17666.1 **17666.1**	1.000 **1.00**	(1.069 x 1.169 x 0.93) **1.162**	0.501 **0.501**	1.000 **1.000**	10295.8 **10295.8**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 3, Sub: Cannon Creek, Plat: , Lake City, FL, 32024-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM	X WOF	= Points	
.18	1932.0	12.74	4430.5	Double, Clear	N	1.5	7.5	84.0	14.30	1.00	1202.9	
				Double, Clear	N	6.0	3.0	12.5	14.30	1.03	183.3	
				Double, Clear	E	1.5	5.5	30.0	9.09	1.04	284.0	
				Double, Clear	S	1.5	7.0	30.0	4.03	1.07	129.9	
				Double, Clear	SW	8.0	7.5	21.0	7.17	1.64	246.6	
				Double, Clear	S	8.0	8.0	70.0	4.03	2.73	770.7	
				Double, Clear	N	1.5	6.0	16.0	14.30	1.00	229.4	
				Double, Clear	W	1.5	7.5	21.0	10.77	1.01	229.1	
				Double, Clear	N	1.5	3.0	12.5	14.30	1.01	180.4	
				Double, Clear	S	1.5	8.0	42.0	4.03	1.04	176.3	
				As-Built Total:				**339.0**			**3632.6**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	232.0	3.60	835.2	Frame, Wood, Adjacent			13.0	232.0	3.30		765.6	
Exterior	1660.0	3.70	6142.0	Frame, Wood, Exterior			13.0	1660.0	3.40		5644.0	
Base Total:	**1892.0**		**6977.2**	**As-Built Total:**				**1892.0**			**6409.6**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	20.0	11.50	230.0	Exterior Wood				20.0	12.30		246.0	
Exterior	60.0	12.30	738.0	Adjacent Wood				20.0	11.50		230.0	
				Exterior Wood				40.0	12.30		492.0	
Base Total:	**80.0**		**968.0**	**As-Built Total:**				**80.0**			**968.0**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X WCM =		Points	
Under Attic	1932.0	2.05	3960.6	Under Attic			30.0	1932.0	2.05 X 1.00		3960.6	
Base Total:	**1932.0**		**3960.6**	**As-Built Total:**				**1932.0**			**3960.6**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	204.0(p)	8.9	1815.6	Slab-On-Grade Edge Insulation			0.0	204.0(p	18.80		3835.2	
Raised	0.0	0.00	0.0									
Base Total:			**1815.6**	**As-Built Total:**				**204.0**			**3835.2**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	1932.0	-0.59	-1139.9					1932.0	-0.59		-1139.9	

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 3, Sub: Cannon Creek, Plat: , Lake City, FL, 32024-** PERMIT #:

BASE			AS-BUILT					
Summer Base Points:		**25887.6**	**Summer As-Built Points:**					**25621.7**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
25887.6	**0.4266**	**11043.6**	25621.7 **25621.7**	1.000 **1.00**	(1.090 x 1.147 x 0.91) **1.138**	0.284 **0.284**	1.000 **1.000**	8290.7 **8290.7**

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 3, Sub: Cannon Creek, Plat: , Lake City, FL, 32024-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1932.0	20.04	6969.1	Double, Clear	N	1.5	7.5	84.0	19.22	0.96	1552.3	
				Double, Clear	N	6.0	3.0	12.5	19.22	0.62	149.7	
				Double, Clear	E	1.5	5.5	30.0	40.22	0.90	1081.5	
				Double, Clear	S	1.5	7.0	30.0	34.50	0.89	925.8	
				Double, Clear	SW	8.0	7.5	21.0	38.46	0.48	385.5	
				Double, Clear	S	8.0	8.0	70.0	34.50	0.52	1257.1	
				Double, Clear	N	1.5	6.0	16.0	19.22	0.94	288.6	
				Double, Clear	W	1.5	7.5	21.0	36.99	0.95	737.2	
				Double, Clear	N	1.5	3.0	12.5	19.22	0.83	199.7	
				Double, Clear	S	1.5	8.0	42.0	34.50	0.92	1337.8	
				As-Built Total:				**339.0**			**7915.2**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	232.0	0.70	162.4	Frame, Wood, Adjacent			13.0	232.0	0.60		139.2	
Exterior	1660.0	1.70	2822.0	Frame, Wood, Exterior			13.0	1660.0	1.50		2490.0	
Base Total:	**1892.0**		**2984.4**	**As-Built Total:**				**1892.0**			**2629.2**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	20.0	2.40	48.0	Exterior Wood				20.0	6.10		122.0	
Exterior	60.0	6.10	366.0	Adjacent Wood				20.0	2.40		48.0	
				Exterior Wood				40.0	6.10		244.0	
Base Total:	**80.0**		**414.0**	**As-Built Total:**				**80.0**			**414.0**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1932.0	1.73	3342.4	Under Attic			30.0	1932.0	1.73 X 1.00		3342.4	
Base Total:	**1932.0**		**3342.4**	**As-Built Total:**				**1932.0**			**3342.4**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	204.0(p)	-37.0	-7548.0	Slab-On-Grade Edge Insulation			0.0	204.0(p	-41.20		-8404.8	
Raised	0.0	0.00	0.0									
Base Total:			**-7548.0**	**As-Built Total:**				**204.0**			**-8404.8**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1932.0	10.21	19725.7					1932.0	10.21		19725.7	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.3

The higher the score, the more efficient the home.

EWPL INC, Lot: 3, Sub: Cannon Creek, Plat: , Lake City, FL, 32024-

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family	a.	Central Unit	Cap: 36.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 12.00
4.	Number of Bedrooms	4	b.	N/A	
5.	Is this a worst case?	No			
6.	Conditioned floor area (ft²)	1932 ft²	c.	N/A	
7.	Glass area & type				
a.	Clear - single pane	0.0 ft²	13.	Heating systems	
b.	Clear - double pane	339.0 ft²	a.	Electric Heat Pump	Cap: 36.0 kBtu/hr
c.	Tint/other SHGC - single pane	0.0 ft²			HSPF: 6.80
d.	Tint/other SHGC - double pane	0.0 ft²	b.	N/A	
8.	Floor types				
a.	Slab-On-Grade Edge Insulation	R=0.0, 204.0(p) ft	c.	N/A	
b.	N/A				
c.	N/A		14.	Hot water systems	
9.	Wall types		a.	Electric Resistance	Cap: 50.0 gallons
a.	Frame, Wood, Adjacent	R=13.0, 232.0 ft²			EF: 0.88
b.	Frame, Wood, Exterior	R=13.0, 1660.0 ft²	b.	N/A	
c.	N/A				
d.	N/A		c.	Conservation credits	
e.	N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
a.	Under Attic	R=30.0, 1932.0 ft²	15.	HVAC credits	
b.	N/A			(CF-Ceiling fan, CV-Cross ventilation,	
c.	N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 125.0 ft		RB-Attic radiant barrier,	
b.	N/A			MZ-C-Multizone cooling,	
				MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

EnergyGauge® (Version: FLRCPB v3.2)

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000000911

DATE 12/07/2005 PARCEL ID # 24-4S-16-03114-103

APPLICANT HUGO ESCALANTE PHONE 386-288-8666

ADDRESS 6210 SW CR 18 FORT WHITE FL 32038

OWNER HBM CONSTRUCTION PHONE 813-209-0363

ADDRESS 177 SW GERALD CORNER DR LAKE CITY FL 32024

CONTRACTOR HUGO ESCALANTE PHONE 386-288-8666

LOCATION OF PROPERTY 47 S, R 242, R SW CANNON CREEK DR, R SW GERALD CONNER DR,
3RD ON RIGHT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT CANNON CREEK PLACE 3

SIGNATURE Hugo Escalante

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

Donald F. Lee & Associates, Inc.

Surveyors & Engineers

D.F.L.

140 NW Ridgewood Avenue
Lake City, Florida 32055
(386) 755-6166
Fax (386) 755-6167
dfla@suwanneevalley.net

Wednesday, February 01, 2006

TO: Columbia County Building & Zoning Department

FROM: Tim Delbene, PLS – Donald F. Lee & Associates, Inc.

RE: Floor (Stemwall) Elevation check – Lot 3, Cannon Creek Place

CC: EWPL – Hugo Escalante

Elevations were obtained at the above referenced Lot using local subdivision benchmarks. The results are as follows:

FLOOR ELEVATION: 105.14

LOWEST ADJACENT GRADE: 102.62

HIGHEST ADJACENT GRADE: 104.02

According to the record plat of Cannon Creek Place, the subdivision's engineer has set a minimum floor elevation for this lot at 102.00.

SIGNED: _____
Timothy A. Delbene, P.L.S.
Registration No. LS 5594

DATE: 2/1/2006

23939

Project Information for:	L139892		
Builder:	HUGO ESCALANTE	Date:	11/15/2005
Lot:	LOT 3 CANNON CREEK	Start Number:	1973
Subdivision:	N/A		
County or City:	COLUMBIA COUNTY	Refer to Master:	
Truss Page Count:	40		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	120		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ESCALANTE, HUGO CRC 1326967
Address: P.O. BOX 280
FORT WHITE, FL. 32038 Designer: 29

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1115051973	11/15/2005				
2	CJ3	1115051974	11/15/2005				
3	CJ5	1115051975	11/15/2005				
4	EJ7	1115051976	11/15/2005				
5	EJ7A	1115051977	11/15/2005				
6	EJ7B	1115051978	11/15/2005				
7	EJ7G	1115051979	11/15/2005				
8	EJ7GA	1115051980	11/15/2005				
9	HJ9	1115051981	11/15/2005				
10	T01	1115051982	11/15/2005				
11	T01G	1115051983	11/15/2005				
12	T02	1115051984	11/15/2005				
13	T03	1115051985	11/15/2005				
14	T04	1115051986	11/15/2005				
15	T05	1115051987	11/15/2005				
16	T06	1115051988	11/15/2005				
17	T07	1115051989	11/15/2005				
18	T08	1115051990	11/15/2005				
19	T09	1115051991	11/15/2005				
20	T10	1115051992	11/15/2005				
21	T11	1115051993	11/15/2005				
22	T12	1115051994	11/15/2005				
23	T13	1115051995	11/15/2005				
24	T14	1115051996	11/15/2005				
25	T15	1115051997	11/15/2005				
26	T16	1115051998	11/15/2005				
27	T17	1115051999	11/15/2005				
28	T18	1115052000	11/15/2005				
29	T19	1115052001	11/15/2005				
30	T20	1115052002	11/15/2005				
31	T21	1115052003	11/15/2005				
32	T22	1115052004	11/15/2005				
33	T23	1115052005	11/15/2005				
34	T24	1115052006	11/15/2005				
35	T25	1115052007	11/15/2005				
36	T26	1115052008	11/15/2005				
37	T27	1115052009	11/15/2005				
38	T28	1115052010	11/15/2005				
39	T29	1115052011	11/15/2005				
40	T29G	1115052012	11/15/2005				

Dwg.#1115051972

02:00:39 PM 10/6/2004

Licensee Details

Licensee Information

Name:	**ESCALANTE, HUGO (Primary Name)**
	EWPL INC (DBA Name)
Main Address:	**P.O. BOX 280** **FORT WHITE, Florida 32038**

License Information

License Type:	**Certified Residential Contractor**
Rank:	**Cert Residental**
License Number:	**CRC1326967**
Status:	**Current, Active**
Licensure Date:	**11/24/2003**
Expires:	**08/31/2006**

Special Qualifications	Effective Date
Qualified Business License Required	11/24/2003

View Related License Information

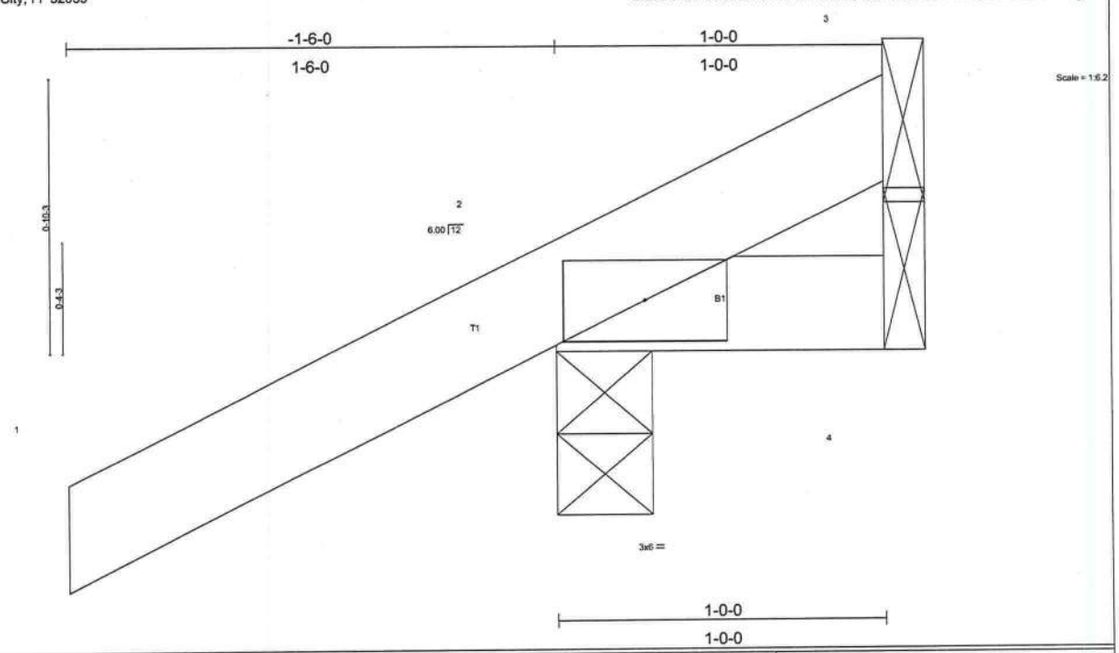
View License Complaint

New Search Back

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#I115051973
L139892	CJ1	MONO TRUSS	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 15:21:10 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=189/0-3-8, 4=14/Mechanical, 3=-40/Mechanical
Max Horz 2=84(load case 5)
Max Uplift2=-220(load case 5), 3=-40(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=73(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/41
BOT CHORD 2-4=0/0

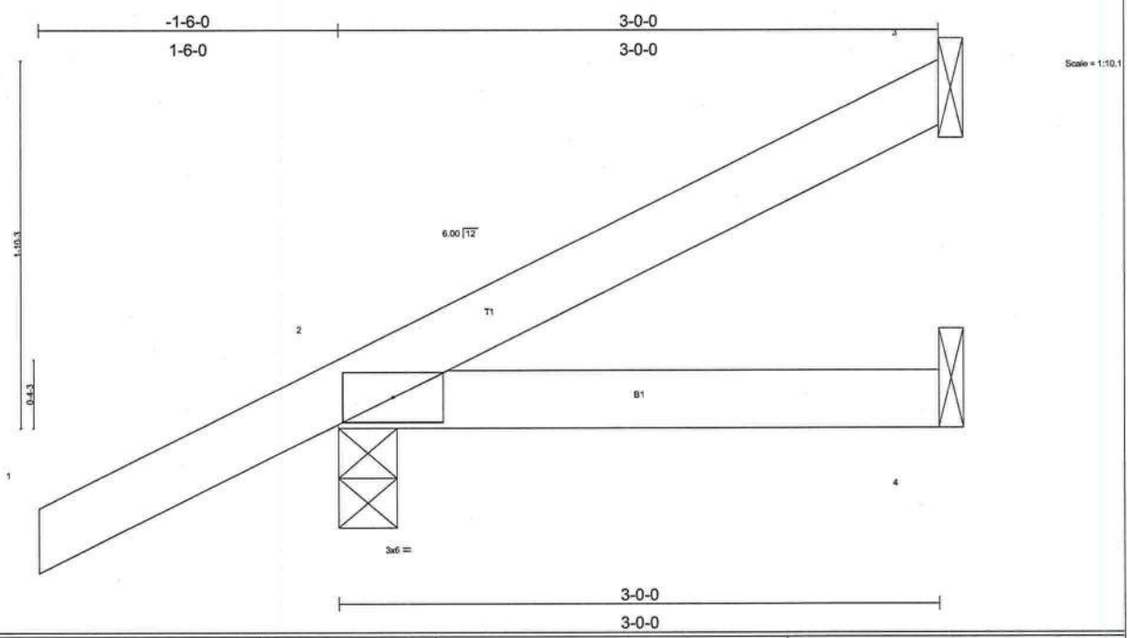
NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 220 lb uplift at joint 2 and 40 lb uplift at joint 3.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#I15051974
L139892	CJ3	MONO TRUSS	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 15:21:11 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical
Max Horz 2=137(load case 5)
Max Uplift3=-47(load case 5), 2=-187(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-52/16
BOT CHORD 2-4=0/0

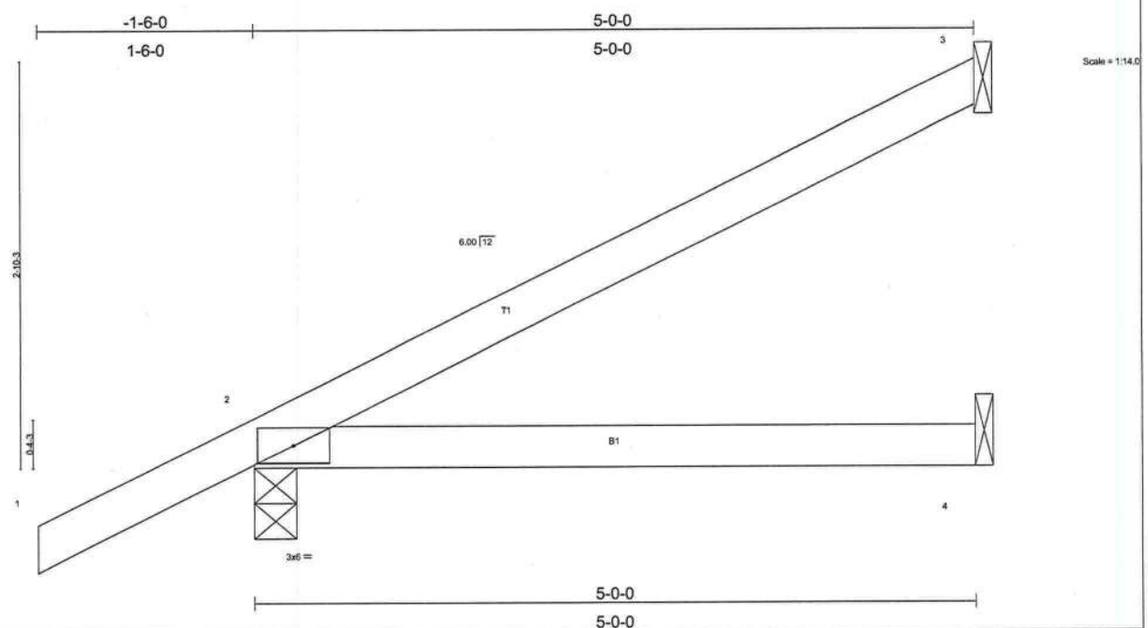
NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 3 and 187 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#115051975
L139892	CJ5	MONO TRUSS	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 15:21:12 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=192(load case 5)
Max Uplift3=-124(load case 5), 2=-197(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-112/41
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 3 and 197 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#I15051976
L139892	EJ7	MONO TRUSS	29	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 15:21:12 2005 Page 1

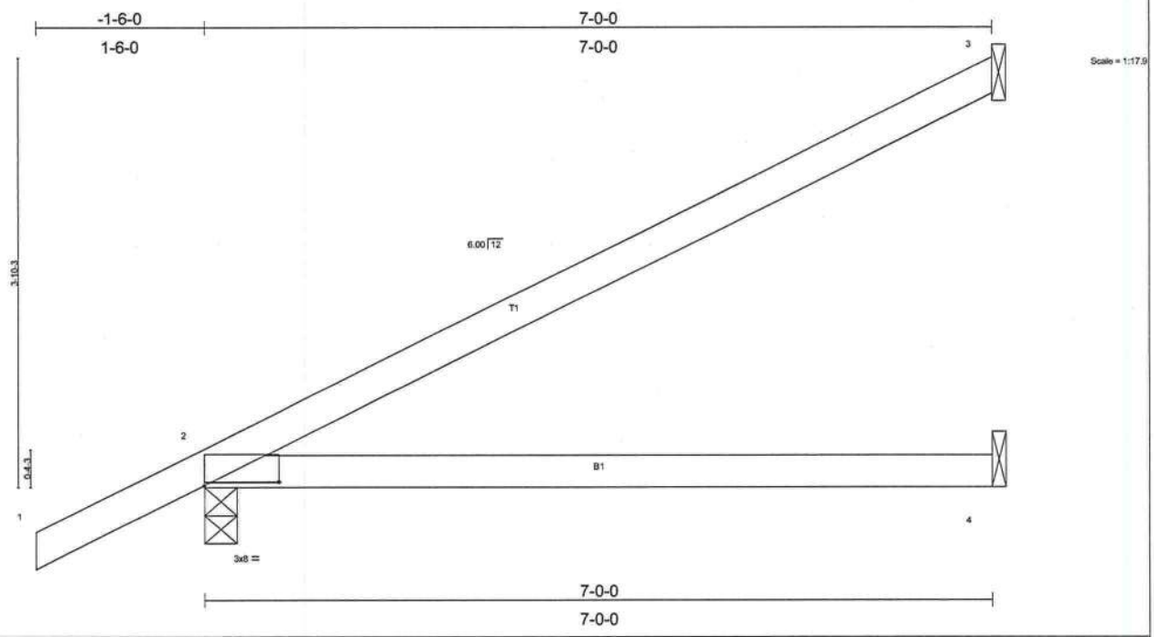


Plate Offsets (X,Y): [2:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.13	2-4	>606	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.22	2-4	>365	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=166/Mechanical, 2=385/0-3-8, 4=108/Mechanical
Max Horz 2=247(load case 5)
Max Uplift3=-170(load case 5), 2=-217(load case 5), 4=-1(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-142/59
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 3, 217 lb uplift at joint 2 and 1 lb uplift at joint 4.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK
L139892	EJ7A	MONO HIP	1	1	Job Reference (optional)

Dwg.#I15051977

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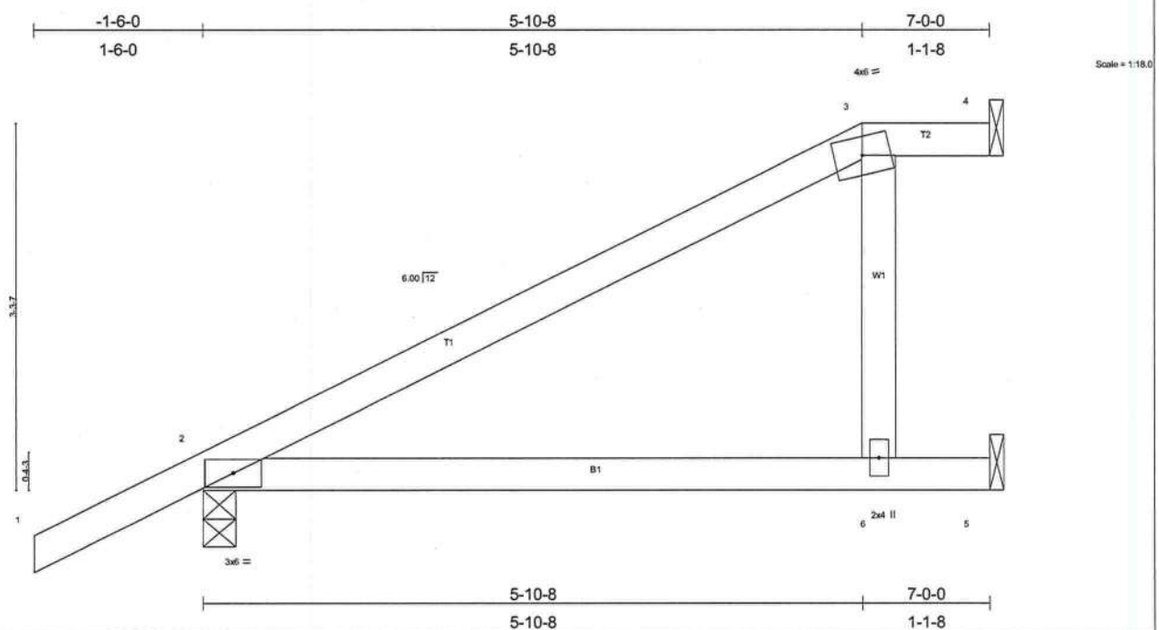

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.20	2-6	>404	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.25	2-6	>323	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.06	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=77/Mechanical, 2=385/0-3-8, 5=197/Mechanical
Max Horz 2=219(load case 5)
Max Uplift4=-13(load case 3), 2=-229(load case 5), 5=-160(load case 5)
Max Grav 4=94(load case 10), 2=385(load case 1), 5=197(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-113/42, 3-4=-0/1
BOT CHORD 2-6=-15/5, 5-6=0/0
WEBS 3-6=-112/302

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 4, 229 lb uplift at joint 2 and 160 lb uplift at joint 5.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#115051978
L139892	EJ7B	MONO HIP	1	1	Job Reference (optional)

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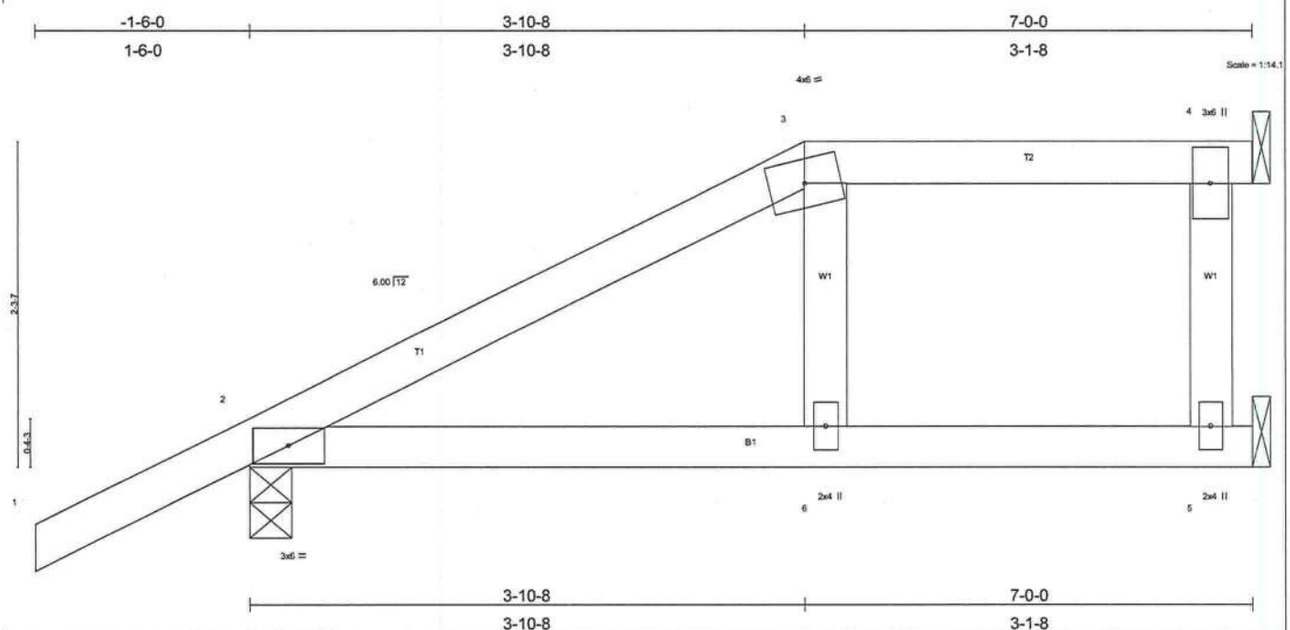

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.19	2-6	>422	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.22	2-6	>355	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.11	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=376/0-3-8, 4=138/Mechanical, 5=127/Mechanical
Max Horz 2=164(load case 5)
Max Uplift2=-236(load case 5), 4=-83(load case 4), 5=-36(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-80/6, 3-4=-0/0
BOT CHORD 2-6=-12/4, 5-6=0/0
WEBS 3-6=-55/167, 4-5=0/0

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 236 lb uplift at joint 2, 83 lb uplift at joint 4 and 36 lb uplift at joint 5.
6) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK
L139892	EJ7G	MONO TRUSS	1	1	Job Reference (optional)

Dwg.#I15051979

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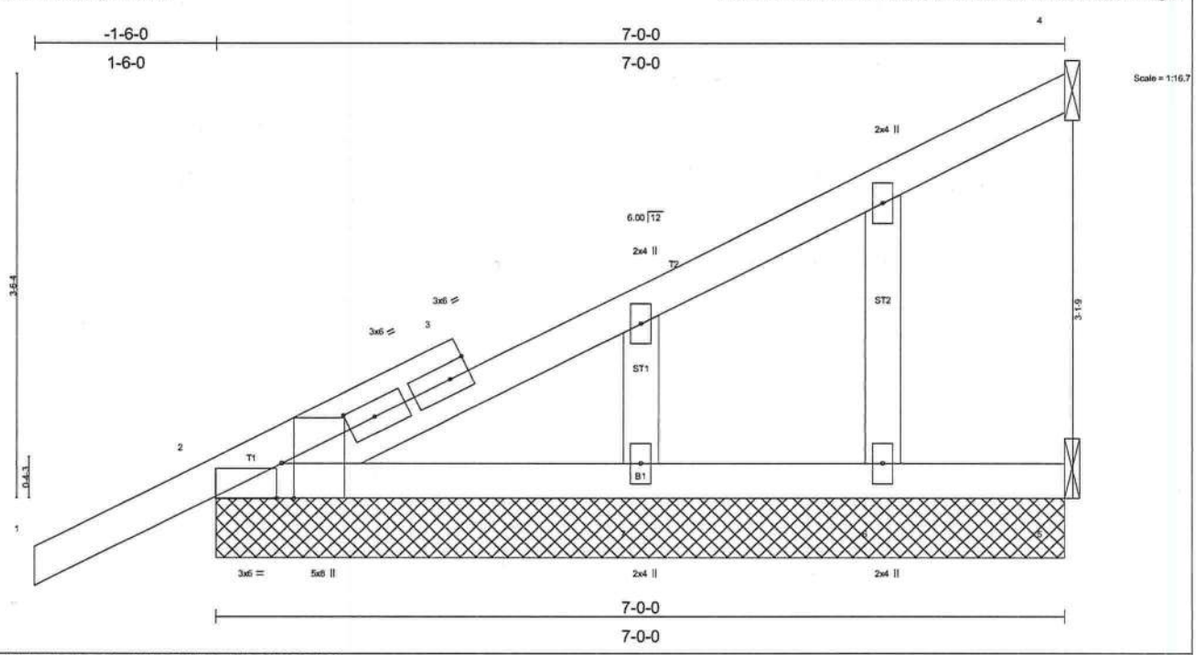

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [3:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.04	2-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	0.03	2-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except*
T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/7-0-0, 4=146/Mechanical, 5=35/Mechanical, 7=243/7-0-0, 6=-27/7-0-0
Max Horz 2=229(load case 5)
Max Uplift2=-175(load case 5), 4=-154(load case 5), 5=-12(load case 5), 7=-116(load case 5), 6=-27(load case 1)
Max Grav 2=266(load case 1), 4=146(load case 1), 5=35(load case 1), 7=243(load case 1), 6=73(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-146/0, 3-4=-112/49
BOT CHORD 2-7=0/0, 6-7=0/0, 5-6=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Gable studs spaced at 2-0-0 oc.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 175 lb uplift at joint 2, 154 lb uplift at joint 4, 12 lb uplift at joint 5, 116 lb uplift at joint 7 and 27 lb uplift at joint 6.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#115051980
L139892	EJ7GA	MONO HIP	1	1	Job Reference (optional)

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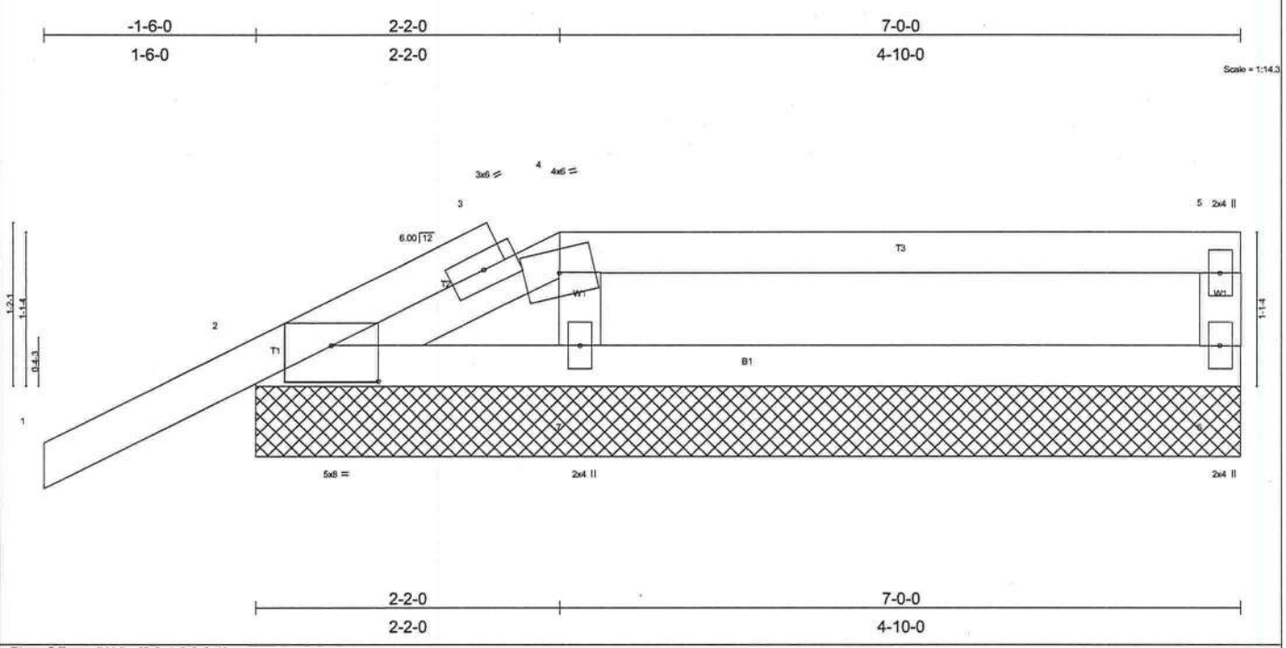

Plate Offsets (X,Y): [2:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.01	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.10	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 27 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=198/7-0-0, 6=204/7-0-0, 7=464/7-0-0
Max Horz 2=99(load case 5)
Max Uplift2=-196(load case 5), 6=-100(load case 3), 7=-206(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8/51, 2-3=-77/28, 3-4=-10/10, 4-5=-0/0
BOT CHORD 2-7=-70/60, 6-7=0/0
WEBS 4-7=-338/393, 5-6=-148/173

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 196 lb uplift at joint 2, 100 lb uplift at joint 6 and 206 lb uplift at joint 7.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-79(F=-25), 4-5=-79(F=-25), 2-6=-30

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANTILEVER GABLE
L139892	HJ9	MONO TRUSS	4	1	Job Reference (optional)

Dwg.#I115051981

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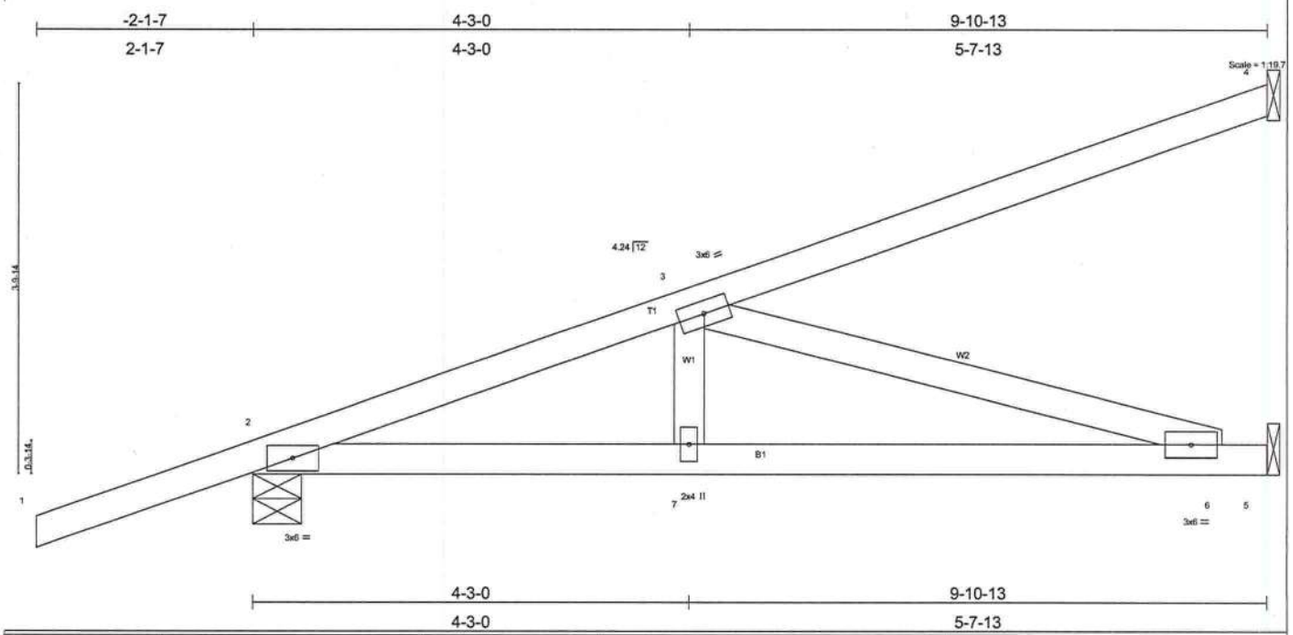

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.18	6-7	>626	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.50	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-6-11 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=486/0-5-11, 5=386/Mechanical
Max Horz 2=303(load case 2)
Max Uplift4=-289(load case 2), 2=-285(load case 2), 5=-107(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-934/246, 3-4=-124/65
BOT CHORD 2-7=-475/869, 6-7=-475/869, 5-6=0/0
WEBS 3-7=0/208, 3-6=-906/495

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 289 lb uplift at joint 4, 285 lb uplift at joint 2 and 107 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#I115051982
L139892	T01	COMMON	2	1	Job Reference (optional)

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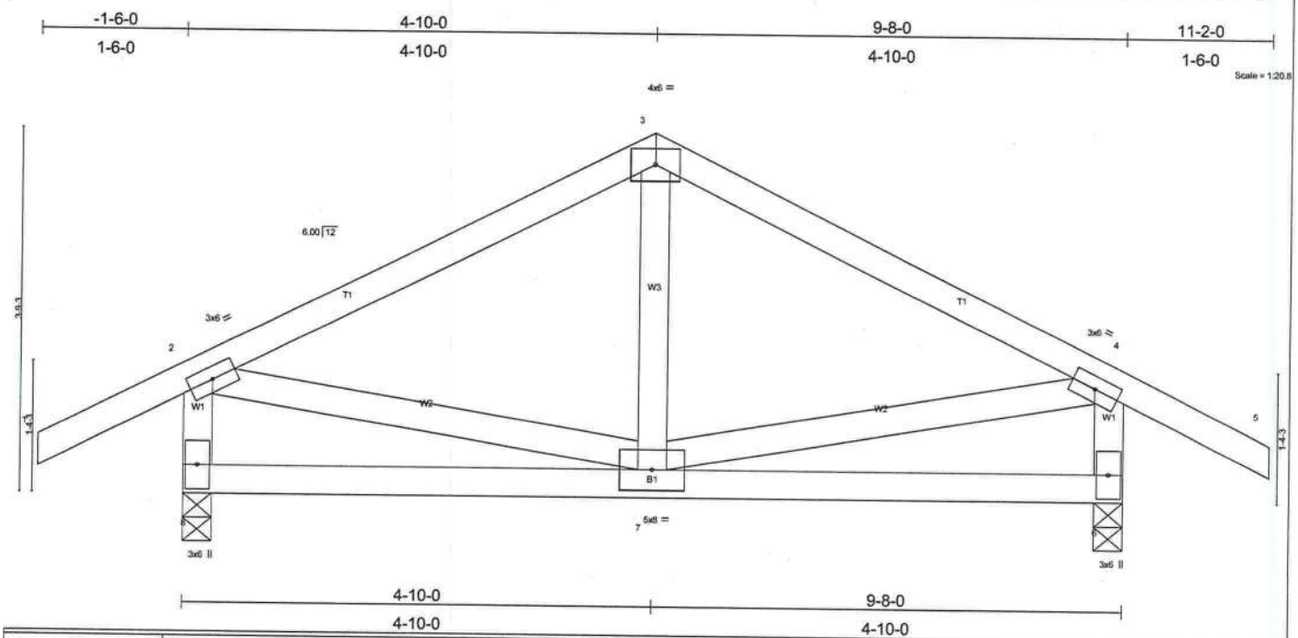

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.03	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	0.03	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 56 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=483/0-3-8, 6=483/0-3-8
Max Horz 8=87(load case 4)
Max Uplift8=-417(load case 5), 6=-417(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-380/578, 3-4=-380/578, 4-5=0/40, 2-8=-413/604, 4-6=-413/604
BOT CHORD 7-8=-134/83, 6-7=-71/83
WEBS 3-7=-223/67, 2-7=-219/222, 4-7=-219/222

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 417 lb uplift at joint 8 and 417 lb uplift at joint 6.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#I15051983
L139892	T01G	COMMON	1	1	Job Reference (optional)

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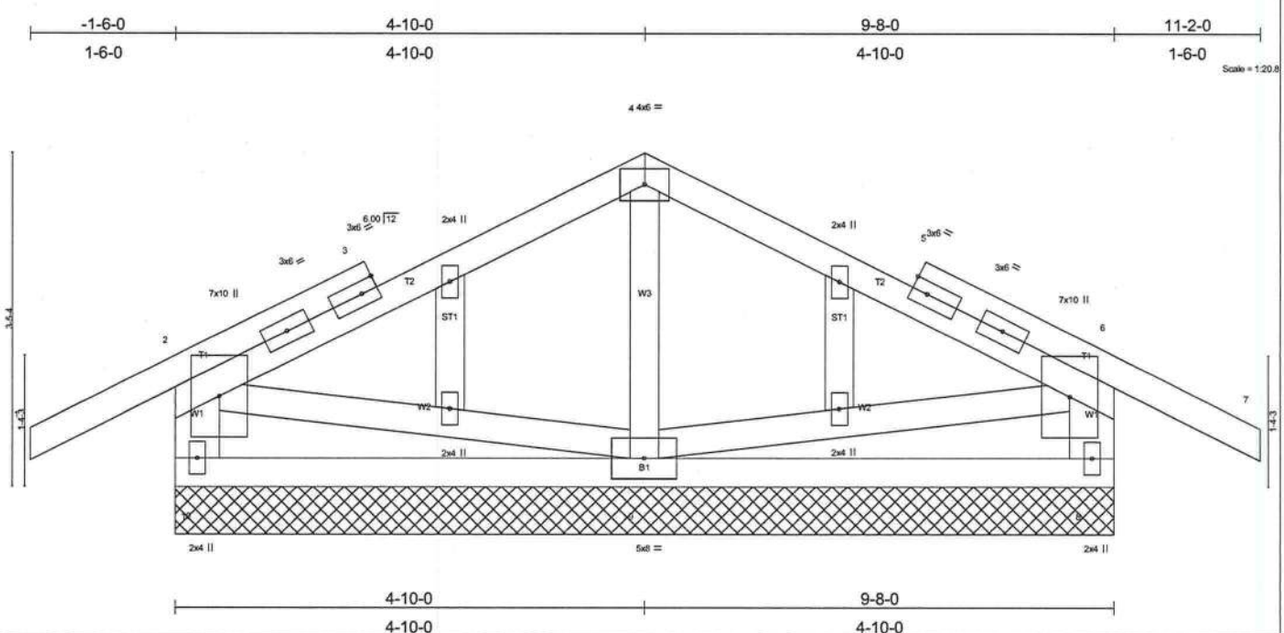

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.01	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.07	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 67 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 6 SYP No.1D, W1 2 X 6 SYP No.1D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 10=401/9-8-0, 8=401/9-8-0, 9=474/9-8-0
Max Horz 10=79(load case 4)
Max Uplift10=-266(load case 5), 8=-269(load case 6), 9=-186(load case 5)
Max Grav 10=405(load case 9), 8=405(load case 10), 9=474(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/51, 2-3=-118/80, 3-4=-41/76, 4-5=-41/76, 5-6=-118/80, 6-7=-6/51, 2-10=-338/386, 6-8=-338/386
BOT CHORD 9-10=-109/151, 8-9=-36/151
WEBS 4-9=-292/263, 2-9=-120/163, 6-9=-120/163

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 10, 269 lb uplift at joint 8 and 186 lb uplift at joint 9.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-79(F=-25), 2-4=-79(F=-25), 4-6=-79(F=-25), 6-7=-79(F=-25), 8-10=-30

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANYON CREEK
L139892	T02	HIP	1	1	Job Reference (optional)

Dwg.#I115051984

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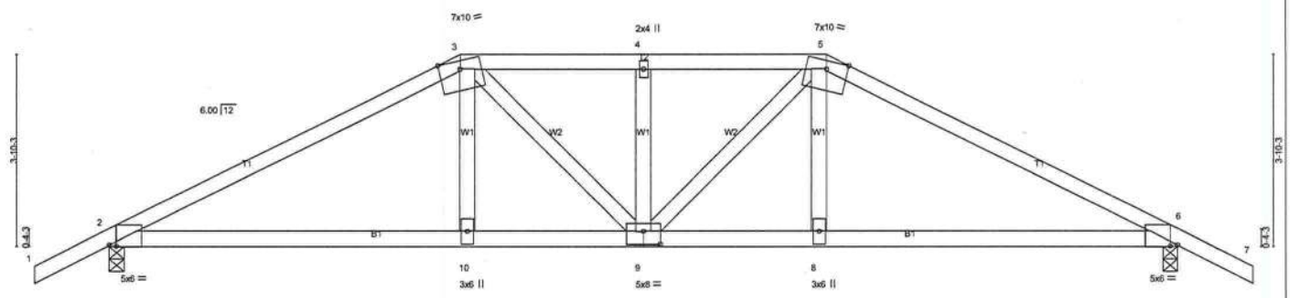
-1-6-0	3-9-4	7-0-0	10-8-0	14-4-0	17-6-12	21-4-0	22-10-0
1-6-0	3-9-4	3-2-12	3-8-0	3-8-0	3-2-12	3-9-4	1-6-0

Scale = 1:40.4



3-9-4	7-0-0	10-8-0	14-4-0	17-6-12	21-4-0
3-9-4	3-2-12	3-8-0	3-8-0	3-2-12	3-9-4

Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-1-11,Edge], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.16	6-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.26	6-8	>955	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 100 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-3-8 oc bracing.

REACTIONS (lb/size) 2=1841/0-3-8, 6=1841/0-3-8
Max Horz 2=-92(load case 5)
Max Uplift2=-978(load case 4), 6=-978(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-3339/1619, 3-4=-3176/1615, 4-5=-3176/1615, 5-6=-3339/1619, 6-7=0/35
BOT CHORD 2-10=-1368/2897, 9-10=-1380/2930, 8-9=-1327/2930, 6-8=-1316/2897
WEBS 3-10=-291/803, 3-9=-315/472, 4-9=-365/397, 5-9=-316/472, 5-8=-291/803

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 978 lb uplift at joint 2 and 978 lb uplift at joint 6.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 321 lb up at 14-4-0, and 539 lb down and 321 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-113(F=-58), 5-7=-54, 2-10=-30, 8-10=-62(F=-33), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-539(F) 8=-539(F)

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANOPY SHEET Dwg.#I115051985
L139892	T03	HIP	1	1	Job Reference (optional)

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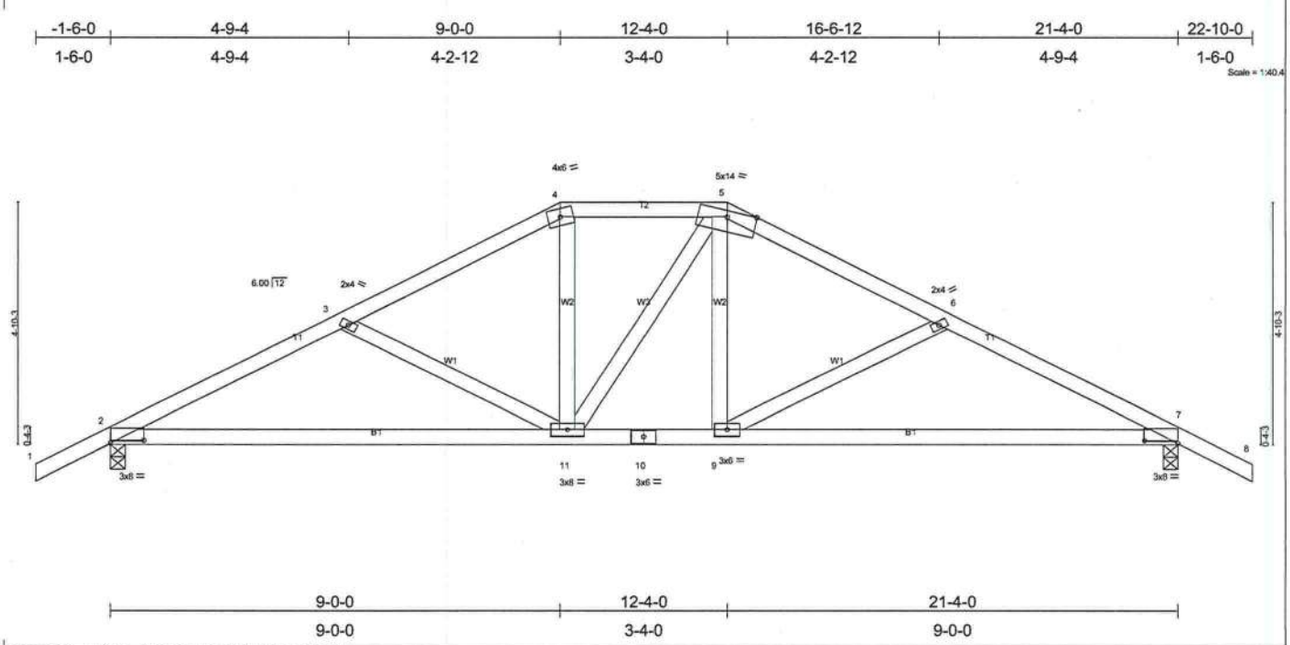

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [7:0-8-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.18	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.30	7-9	>829	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 107 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-7 oc bracing.

REACTIONS (lb/size) 2=973/0-3-8, 7=973/0-3-8
Max Horz 2=-109(load case 6)
Max Uplift2=-475(load case 5), 7=-475(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1468/815, 3-4=-1199/657, 4-5=-1030/648, 5-6=-1198/657, 6-7=-1468/815, 7-8=0/35
BOT CHORD 2-11=-568/1277, 10-11=-304/1028, 9-10=-304/1028, 7-9=-568/1277
WEBS 3-11=-291/300, 4-11=-100/314, 5-11=-102/106, 5-9=-100/315, 6-9=-293/300

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 475 lb uplift at joint 2 and 475 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#115051986
L139892	T04	COMMON	2	1	Job Reference (optional)

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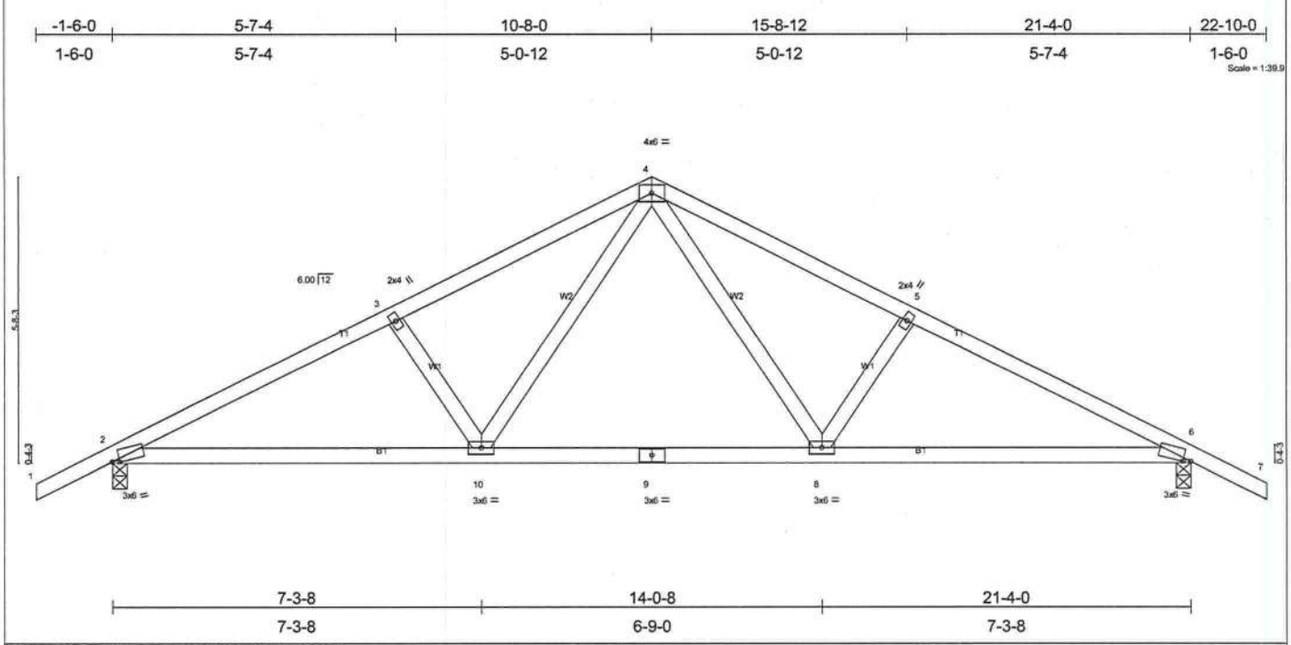

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.19	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.31	8-10	>821	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 100 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-10 oc bracing.

REACTIONS (lb/size) 2=1141/0-3-8, 6=1141/0-3-8
Max Horz 2=-123(load case 6)
Max Uplift2=-568(load case 5), 6=-568(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1888/1014, 3-4=-1732/1007, 4-5=-1732/1007, 5-6=-1888/1014, 6-7=0/35
BOT CHORD 2-10=-732/1619, 9-10=-383/1112, 8-9=-383/1112, 6-8=-732/1619
WEBS 3-10=-250/291, 4-10=-365/734, 4-8=-364/734, 5-8=-250/291

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 568 lb uplift at joint 2 and 568 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#I15051987
L139892	T05	COMMON	4	1	Job Reference (optional)

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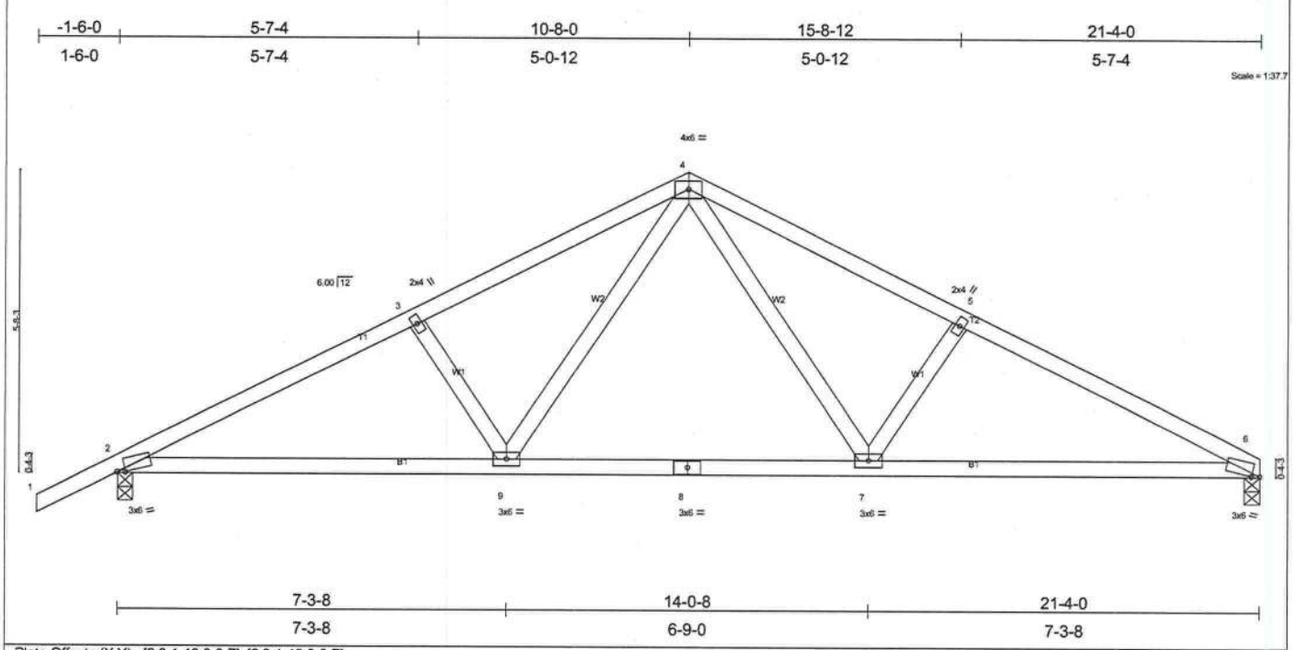

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.19	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.30	7-9	>829	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.29	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 97 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-7-13 oc bracing.

REACTIONS (lb/size) 6=1049/0-3-8, 2=1145/0-3-8
Max Horz 2=144(load case 5)
Max Uplift6=-450(load case 6), 2=-570(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1895/1028, 3-4=-1739/1020, 4-5=-1753/1046, 5-6=-1895/1055
BOT CHORD 2-9=-813/1625, 8-9=-465/1119, 7-8=-465/1119, 6-7=-845/1642
WEBS 3-9=-250/292, 4-9=-361/734, 4-7=-402/753, 5-7=-260/311

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 450 lb uplift at joint 6 and 570 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-9=-30, 7-9=-80(F=-50), 6-7=-30

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#I115051988
L139892	T06	SPECIAL	1	1	Job Reference (optional)

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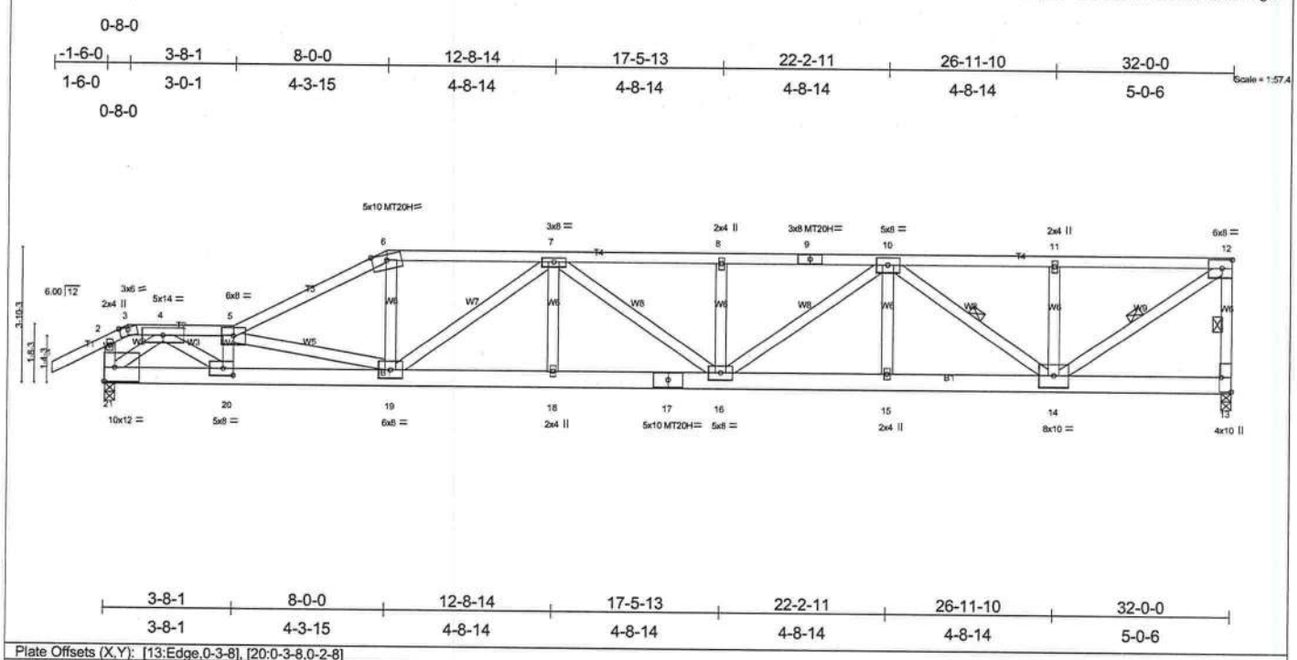

Plate Offsets (X,Y): [13:Edge,0-3-8], [20:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.87	Vert(LL)	0.47	16-18	>814	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.74	16-18	>515	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.84	Horz(TL)	0.14	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 208 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2, W9 2 X 4 SYP No.2, W1 2 X 4 SYP No.1D, W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-4-11 oc bracing.
WEBS 1 Row at midpt 12-13, 10-14, 12-14

REACTIONS (lb/size) 13=2838/0-3-8, 21=2696/0-3-8
Max Horz 21=226(load case 4)
Max Uplift13=-1601(load case 3), 21=-1421(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-258/154, 3-4=-214/142, 4-5=-5976/3114, 5-6=-5829/3177, 6-7=-5292/2935, 7-8=-6576/3693, 8-9=-6576/3693, 9-10=-6576/3693, 10-11=-3449/1939, 11-12=-3449/1939, 12-13=-2651/1587, 2-21=-237/227
BOT CHORD 20-21=-1554/2812, 19-20=-3335/6244, 18-19=-3636/6480, 17-18=-3636/6480, 16-17=-3636/6480, 15-16=-3162/5610, 14-15=-3162/5610, 13-14=-72/100
WEBS 4-20=-1998/3868, 5-20=-2401/1319, 5-19=-1086/592, 6-19=-1072/2149, 7-19=-1489/909, 7-18=0/295, 7-16=-91/119, 8-16=-521/519, 10-16=-659/1198, 10-15=0/335, 10-14=-2678/1516, 11-14=-545/554, 12-14=-2290/4108, 4-21=-3427/1790

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1601 lb uplift at joint 13 and 1421 lb uplift at joint 21.
5) Girder carries hip end with 0-0-0 right side setback, 8-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 616 lb down and 397 lb up at 8-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 3-5=-54, 5-6=-54, 6-12=-113(F=-59), 19-21=-30, 13-19=-62(F=-33)
Concentrated Loads (lb)
Vert: 19=-616(F)

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#115051989
L139892	T07	SPECIAL	1	1	Job Reference (optional)

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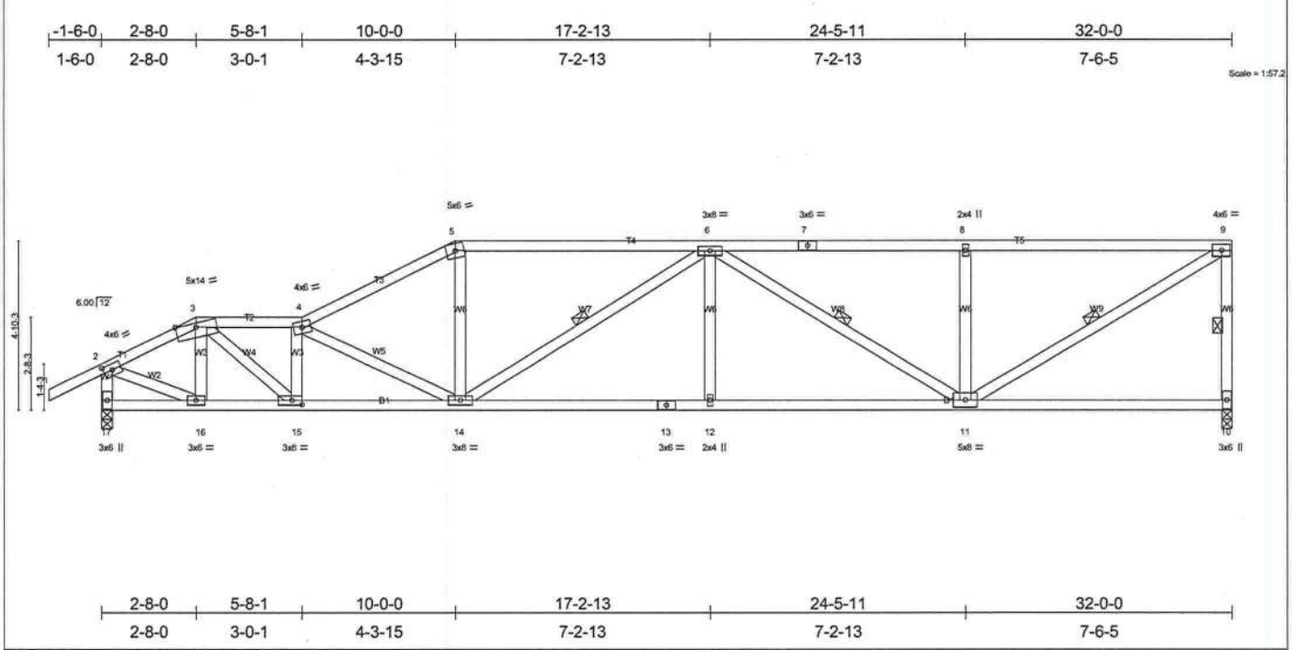

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [15:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.21	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.34	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.08	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 184 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-3-7 oc bracing.
WEBS 1 Row at midpt 9-10, 6-14, 6-11, 9-11

REACTIONS (lb/size) 10=1329/0-3-8, 17=1423/0-3-8
Max Horz 17=283(load case 5)
Max Uplift10=-619(load case 4), 17=-613(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1515/771, 3-4=-2461/1281, 4-5=-2293/1181, 5-6=-2053/1135, 6-7=-1733/901, 7-8=-1733/901, 8-9=-1733/901, 9-10=-1218/698, 2-17=-1373/825
BOT CHORD 16-17=-248/33, 15-16=-829/1292, 14-15=-1445/2516, 13-14=-1257/2373, 12-13=-1257/2373, 11-12=-1257/2373, 10-11=-34/68
WEBS 3-16=-458/255, 3-15=-760/1522, 4-15=-900/520, 4-14=-548/358, 5-14=-214/661, 6-14=-380/269, 6-12=0/217, 6-11=-756/421, 8-11=-406/366, 9-11=-1018/1957, 2-16=-647/1394

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 619 lb uplift at joint 10 and 613 lb uplift at joint 17.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
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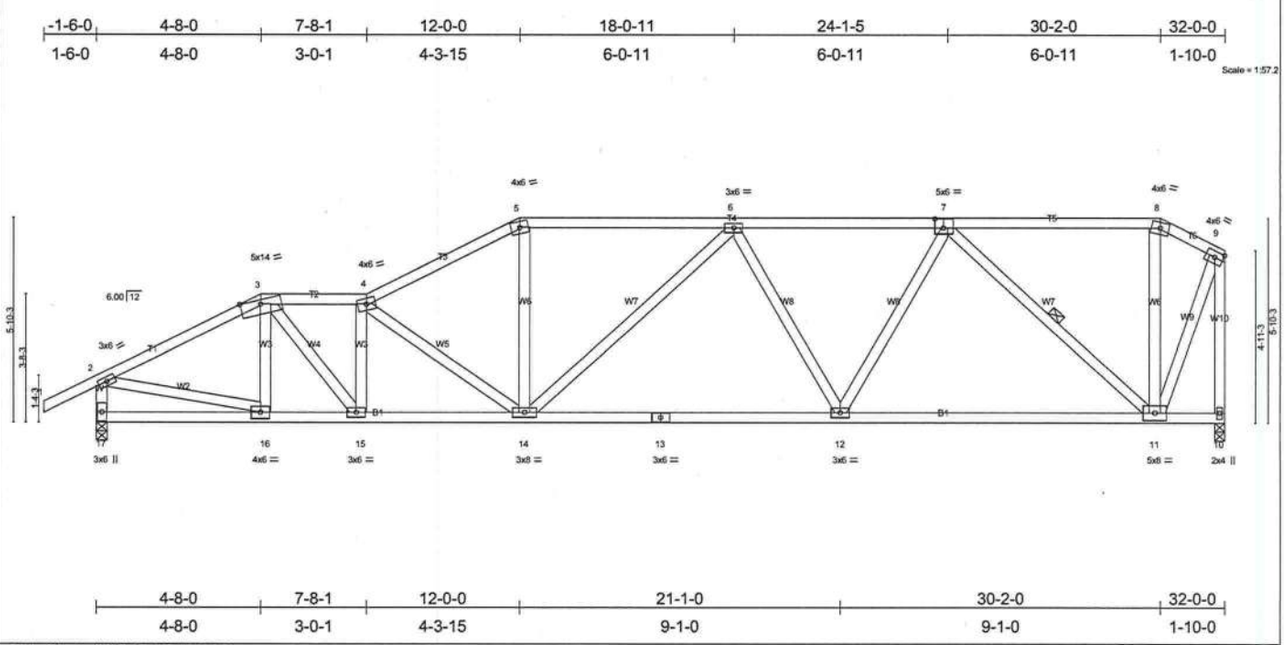
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#I15051990
L139892	T08	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 15:21:23 2005 Page 1



4-8-0	7-8-1	12-0-0	21-1-0	30-2-0	32-0-0
4-8-0	3-0-1	4-3-15	9-1-0	9-1-0	1-10-0

Plate Offsets (X,Y): [7:0-2-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.91	Vert(LL)	-0.21	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.36	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.07	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 202 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-14 oc bracing.
WEBS 1 Row at midpt 7-11

REACTIONS (lb/size) 17=1423/0-3-8, 10=1329/0-3-8
Max Horz 17=305(load case 5)
Max Uplift17=-631(load case 5), 10=-545(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1791/910, 3-4=-2210/1195, 4-5=-2058/1090, 5-6=-1831/1048, 6-7=-1724/926, 7-8=-459/274, 8-9=-500/256, 2-17=-1340/832, 9-10=-1376/671
BOT CHORD 16-17=-316/130, 15-16=-940/1535, 14-15=-1292/2242, 13-14=-1018/1878, 12-13=-1018/1878, 11-12=-760/1416, 10-11=-5/4
WEBS 3-16=-225/177, 3-15=-519/1046, 4-15=-739/414, 4-14=-535/379, 5-14=-206/597, 6-14=-202/170, 6-12=-322/302, 7-12=-237/645, 7-11=-1317/739, 8-11=-56/119, 2-16=-649/1455, 9-11=-604/1254

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 631 lb uplift at joint 17 and 545 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#I115051991
L139892	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 15:21:23 2005 Page 1

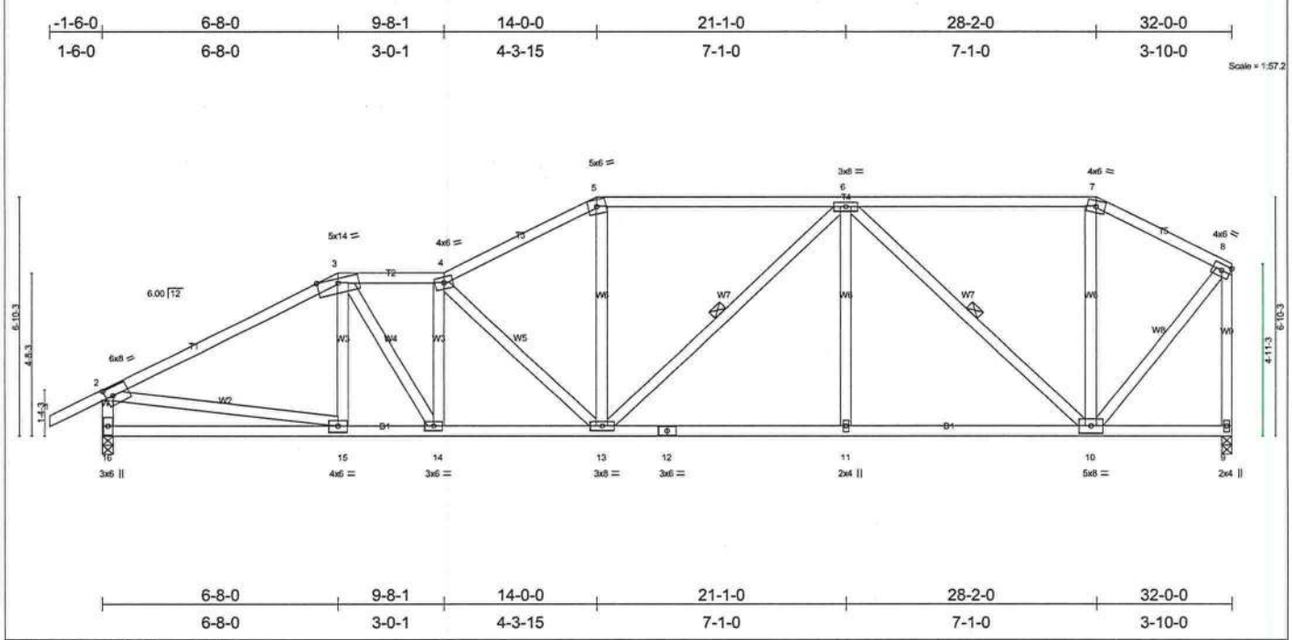


Plate Offsets (X,Y): [2:0-2-7,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.83	Vert(LL)	-0.14	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.22	11-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 210 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-11-4 oc bracing.
WEBS 1 Row at midpt 6-13, 6-10

REACTIONS (lb/size) 16=1423/0-3-8, 9=1329/0-3-8
Max Horz 16=321(load case 5)
Max Uplift16=-648(load case 5), 9=-479(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1887/964, 3-4=-1994/1119, 4-5=-1825/1019, 5-6=-1623/987, 6-7=-719/476, 7-8=-830/462, 2-16=-1310/844, 8-9=-1290/709
BOT CHORD 15-16=-412/271, 14-15=-959/1608, 13-14=-1147/2011, 12-13=-781/1497, 11-12=-781/1497, 10-11=-781/1497, 9-10=-8/7
WEBS 3-15=-73/130, 3-14=-327/706, 4-14=-525/287, 4-13=-559/385, 5-13=-145/463, 6-13=-161/183, 6-11=0/218, 6-10=-1088/575, 7-10=0/93, 2-15=-559/1358, 8-10=-566/1125

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 648 lb uplift at joint 16 and 479 lb uplift at joint 9.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#I15051992
L139892	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 15:21:24 2005 Page 1

-1-6-0	4-5-12	8-8-0	11-8-1	16-0-0	21-1-0	26-2-0	32-0-0	33-6-0
1-6-0	4-5-12	4-2-4	3-0-1	4-3-15	5-1-0	5-1-0	5-10-0	1-6-0

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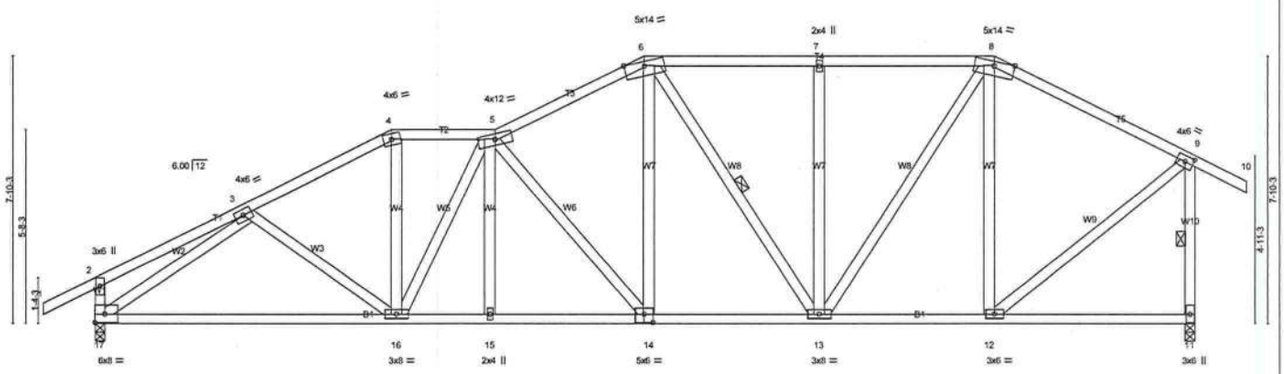

8-8-0	11-8-1	16-0-0	21-1-0	26-2-0	32-0-0
8-8-0	3-0-1	4-3-15	5-1-0	5-1-0	5-10-0

Plate Offsets (X,Y): [9:0-2-15,0-2-0], [14:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.13	16-17	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.23	16-17	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.06	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 229 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-1-2 oc bracing.
WEBS 1 Row at midpt 6-13, 9-11

REACTIONS (lb/size) 17=1421/0-3-8, 11=1421/0-3-8
Max Horz 17=297(load case 4)
Max Uplift17=-674(load case 5), 11=-565(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-335/151, 3-4=-1802/1023, 4-5=-1583/979, 5-6=-1611/1004, 6-7=-1264/887, 7-8=-1264/887, 8-9=-1024/684, 9-10=0/40, 2-17=-348/327, 9-11=-1335/865
BOT CHORD 16-17=-715/1467, 15-16=-786/1806, 14-15=-786/1805, 13-14=-558/1415, 12-13=-333/844, 11-12=-46/98
WEBS 3-16=-58/162, 4-16=-262/561, 5-16=-476/290, 5-15=0/46, 5-14=-629/443, 6-14=-312/645, 6-13=-288/181, 7-13=-281/247, 8-13=-376/793, 8-12=-495/280, 3-17=-1538/913, 9-12=-410/1060

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 674 lb uplift at joint 17 and 565 lb uplift at joint 11.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANYON CREEK Dwg.#115051993
L139892	T11	SPECIAL	1	1	Job Reference (optional)

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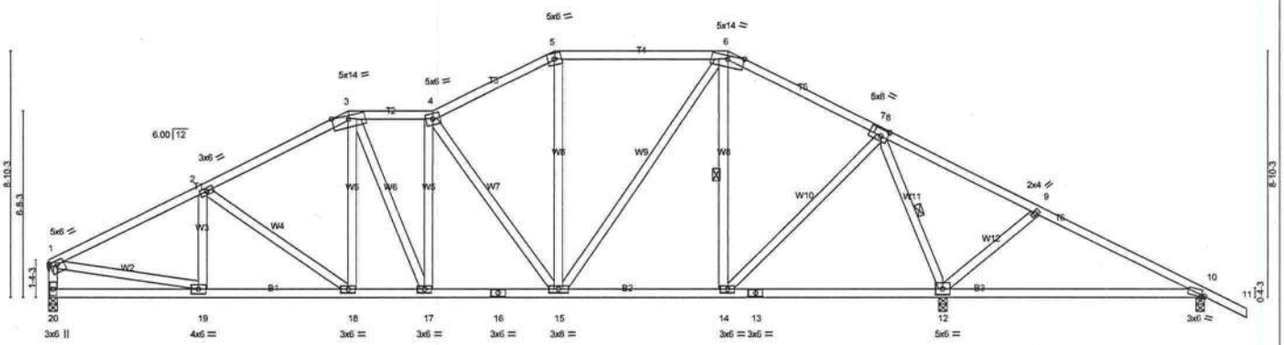
5-5-12	10-8-0	13-8-1	18-0-0	24-2-0	29-7-13	35-1-11	41-2-0	42-8-0
5-5-12	5-2-4	3-0-1	4-3-15	6-2-0	5-5-13	5-5-13	6-0-5	1-6-0

Scale = 1:72.0



5-5-12	10-8-0	13-8-1	18-0-0	24-2-0	31-10-4	41-2-0
5-5-12	5-2-4	3-0-1	4-3-15	6-2-0	7-8-4	9-3-12

Plate Offsets (X,Y): [1:Edge,0-1-12], [7:0-2-12,0-3-0], [10:0-1-5,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	0.41	10-12	>266	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	0.35	10-12	>312	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.04	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 260 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-14, 8-12

REACTIONS (lb/size) 20=1256/0-3-8, 12=2045/0-3-8, 10=221/0-3-8
Max Horz 20=-209(load case 3)
Max Uplift20=-539(load case 5), 12=-913(load case 6), 10=-345(load case 6)
Max Grav 20=1256(load case 1), 12=2045(load case 1), 10=282(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1744/943, 2-3=-1589/943, 3-4=-1461/954, 4-5=-1238/836, 5-6=-1076/816, 6-7=-882/612, 7-8=-886/592, 8-9=-177/587, 9-10=-79/357, 10-11=0/35, 1-20=-1167/676
BOT CHORD 19-20=-195/213, 18-19=-652/1501, 17-18=-494/1368, 16-17=-511/1464, 15-16=-511/1464, 14-15=-114/728, 13-14=0/174, 12-13=0/174, 10-12=-287/160
WEBS 2-19=-118/170, 2-18=-177/204, 3-18=-104/235, 3-17=-111/228, 4-17=-121/128, 4-15=-681/476, 5-15=-97/225, 6-15=-346/648, 6-14=-449/309, 8-14=-301/898, 8-12=-1633/892, 9-12=-351/439, 1-19=-600/1315

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 539 lb uplift at joint 20, 913 lb uplift at joint 12 and 345 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANYON CREEK Dwg.#115051994
L139892	T12	SPECIAL	1	1	Job Reference (optional)

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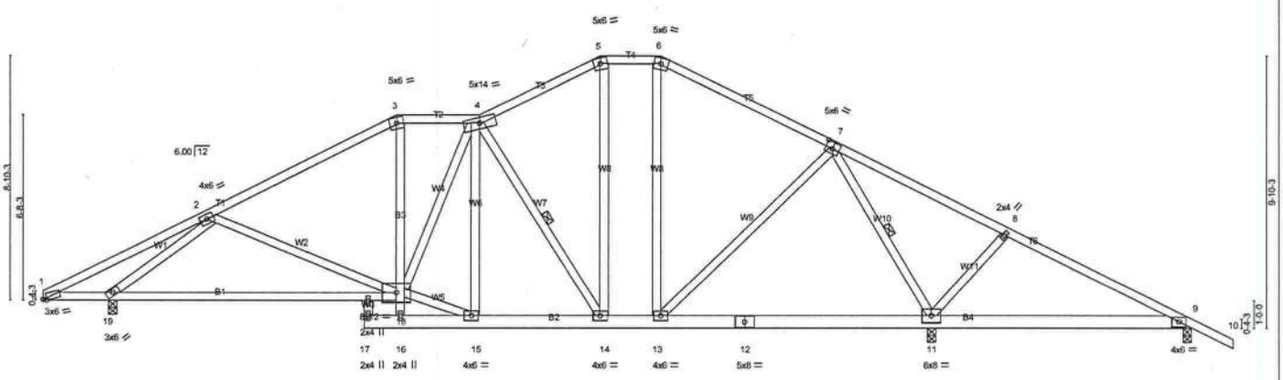
6-0-4	11-6-0	12-8-0	15-8-1	20-0-0	22-2-0	28-3-13	34-5-11	41-2-0	42-8-0
6-0-4	5-5-12	1-2-0	3-0-1	4-3-15	2-2-0	6-1-13	6-1-13	6-8-5	1-6-0

Scale = 1:72.4



2-5-12	11-6-0	12-8-0	15-8-1	20-0-0	22-2-0	31-10-4	41-2-0
2-5-12	9-0-4	1-2-0	3-0-1	4-3-15	2-2-0	9-8-4	9-3-12

Plate Offsets (X,Y): [1:0-1-8,0-0-7], [7:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.14	9-11	>768	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	0.12	9-11	>888	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 280 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B1 2 X 4 SYP No.2, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-14, 7-11

REACTIONS (lb/size) 11=1806/0-3-8, 9=343/0-3-8, 19=1424/0-3-8
Max Horz 19=-254(load case 6)
Max Uplift11=-872(load case 6), 9=-373(load case 6), 19=-686(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-303/85, 2-3=-1566/836, 3-4=-1310/825, 4-5=-977/724, 5-6=-827/705, 6-7=-998/697, 7-8=-14/329, 8-9=-56/103, 9-10=0/39
BOT CHORD 1-19=-10/341, 18-19=-575/1252, 16-18=0/59, 3-18=-99/383, 16-17=0/0, 15-16=-231/0, 14-15=-380/1204, 13-14=-135/827, 12-13=-69/454, 11-12=-69/454, 9-11=-57/93
WEBS 2-18=-16/179, 15-18=-256/1446, 4-18=-46/282, 4-15=-230/28, 4-14=-758/515, 5-14=-238/318, 6-13=-56/215, 7-13=-92/529, 7-11=-1409/760, 8-11=-335/419, 2-19=-1640/1232

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 872 lb uplift at joint 11, 373 lb uplift at joint 9 and 686 lb uplift at joint 19.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#I115051995
L139892	T13	SPECIAL	1	1	Job Reference (optional)

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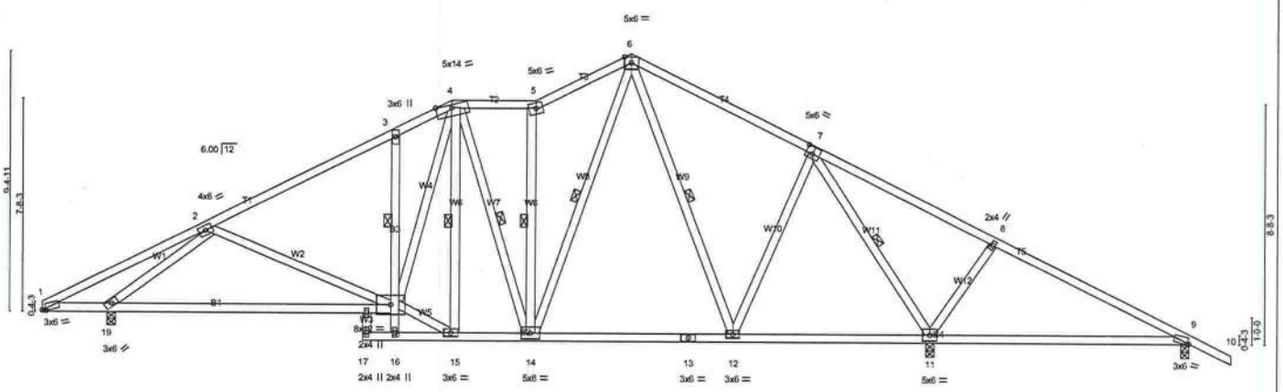
6-0-4	11-6-0	14-8-0	17-8-1	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
6-0-4	5-5-12	3-2-0	3-0-1	3-4-15	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:72.4



14-8-0						
2-5-12	11-6-0	12-6-0	17-8-1	24-9-2	31-10-4	41-2-0
2-5-12	9-0-4	1-0-0 2-2-0	3-0-1	7-1-2	7-1-2	9-3-12

Plate Offsets (X,Y): [1:0-1-8,0-0-7], [7:0-3-0,0-3-0], [9:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.45	9-11	>243	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	0.38	9-11	>290	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 273 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 3-18
WEBS 1 Row at midpt 4-15, 4-14, 5-14, 6-14, 6-12, 7-11

REACTIONS (lb/size) 11=1892/0-3-8, 9=277/0-3-8, 19=1400/0-3-8
Max Horz 19=-261(load case 6)
Max Uplift11=-918(load case 6), 9=-345(load case 6), 19=-669(load case 5)
Max Grav 11=1892(load case 1), 9=331(load case 10), 19=1400(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-301/91, 2-3=-1523/808, 3-4=-1435/955, 4-5=-994/747, 5-6=-1131/880, 6-7=-798/622, 7-8=-106/477, 8-9=-98/274, 9-10=0/35
BOT CHORD 1-19=-14/339, 18-19=-562/1225, 16-18=0/64, 3-18=-239/341, 16-17=0/0, 15-16=-184/0, 14-15=-267/991, 13-14=-97/737, 12-13=-97/737, 11-12=-5/453, 9-11=-211/208
WEBS 2-18=0/186, 15-18=-167/1241, 4-18=-420/976, 4-15=-472/84, 4-14=-46/71, 5-14=-593/487, 6-14=-511/850, 6-12=-286/169, 7-12=-88/513, 7-11=-1535/789, 8-11=-373/466, 2-19=-1609/1204

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 918 lb uplift at joint 11, 345 lb uplift at joint 9 and 669 lb uplift at joint 19.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANTILEVER GABLE
L139892	T14	SPECIAL	1	1	Job Reference (optional)

Dwg.#115051996

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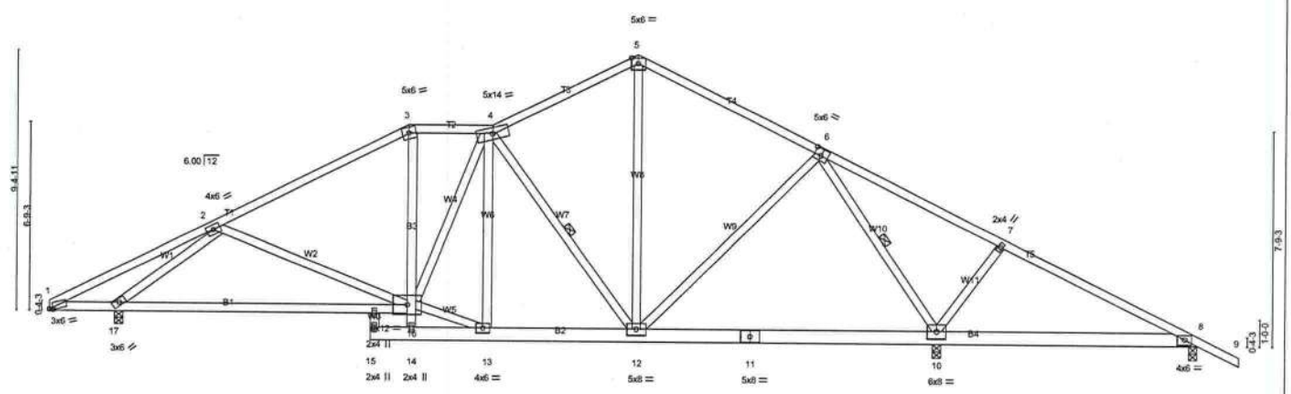
6-0-4	11-6-0	12-10-0	15-10-1	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
6-0-4	5-5-12	1-4-0	3-0-1	5-2-15	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:72.4



2-5-12	11-6-0	12-10-0	15-10-1	23-10-2	31-10-4	41-2-0
2-5-12	9-0-4	1-4-0	3-0-1	8-0-2	8-0-2	9-3-12

Plate Offsets (X,Y): [1:0-1-8,0-0-7], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.14	8-10	>778	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	0.13	8-10	>877	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 270 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B1 2 X 4 SYP No.2, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-12, 6-10

REACTIONS (lb/size) 10=1903/0-3-8, 8=271/0-3-8, 17=1405/0-3-8
Max Horz 17=-263(load case 6)
Max Uplift10=-914(load case 6), 8=-348(load case 6), 17=-668(load case 5)
Max Grav 10=1903(load case 1), 8=329(load case 10), 17=1405(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-306/78, 2-3=-1527/801, 3-4=-1272/796, 4-5=-916/666, 5-6=-943/661, 6-7=-116/469, 7-8=-107/287, 8-9=0/39
BOT CHORD 1-17=-3/343, 16-17=-565/1231, 14-16=0/98, 3-16=-71/354, 14-15=0/0, 13-14=-239/0, 12-13=-344/1140, 11-12=-18/441, 10-11=-18/441, 8-10=-233/234
WEBS 2-16=-4/195, 13-16=-205/1367, 4-16=-73/364, 4-13=-360/77, 4-12=-657/484, 5-12=-253/433, 6-12=-105/510, 6-10=-1482/831, 7-10=-345/433, 2-17=-1602/1211

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 914 lb uplift at joint 10, 348 lb uplift at joint 8 and 668 lb uplift at joint 17.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK
L139892	T15	SPECIAL	1	1	Job Reference (optional)

Dwg.#115051997

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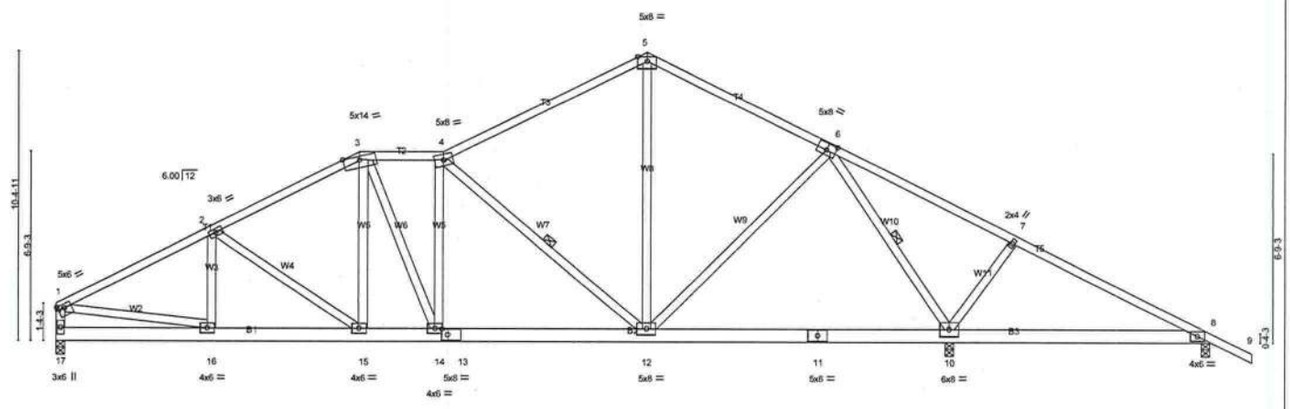
5-6-12	10-10-0	13-10-1	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
5-6-12	5-3-4	3-0-1	7-2-15	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:72.0



5-6-12	10-10-0	13-10-1	21-1-0	31-10-4	41-2-0
5-6-12	5-3-4	3-0-1	7-2-15	10-9-4	9-3-12

Plate Offsets (X,Y): [1:Edge,0-1-12], [6:0-4-0,0-3-0], [13:0-2-7,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.14	8-10	>781	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.12	8-10	>882	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.03	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 276 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-12, 6-10

REACTIONS (lb/size) 17=1264/0-3-8, 10=2012/0-3-8, 8=247/0-3-8
Max Horz 17=-239(load case 6)
Max Uplift17=-549(load case 5), 10=-964(load case 6), 8=-328(load case 6)
Max Grav 17=1264(load case 1), 10=2012(load case 1), 8=315(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1782/963, 2-3=-1613/956, 3-4=-1504/993, 4-5=-1033/702, 5-6=-1024/712, 6-7=-166/524, 7-8=-140/334, 8-9=0/39, 1-17=-1153/666
BOT CHORD 16-17=-274/293, 15-16=-700/1534, 14-15=-533/1387, 13-14=-580/1507, 12-13=-580/1507, 11-12=-28/460, 10-11=-28/460, 8-10=-275/280
WEBS 2-16=-115/173, 2-15=-207/216, 3-15=-111/258, 3-14=-176/287, 4-14=-141/173, 4-12=-885/648, 5-12=-241/456, 6-12=-164/591, 6-10=-1612/929, 7-10=-349/438, 1-16=-549/1263

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 549 lb uplift at joint 17, 964 lb uplift at joint 10 and 328 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#I115051998
L139892	T16	SPECIAL	1	1	Job Reference (optional)

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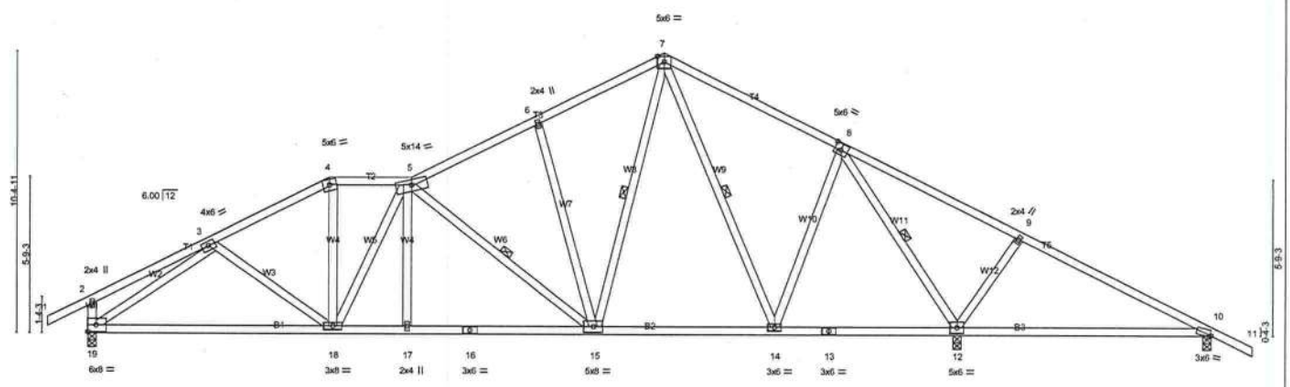
1-6-0	4-6-12	8-10-0	11-10-1	16-5-8	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	4-6-12	4-3-4	3-0-1	4-7-8	4-7-8	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:74.0



8-10-0	11-10-1	18-6-2	25-2-3	31-10-4	41-2-0
8-10-0	3-0-1	6-8-1	6-8-1	6-8-1	9-3-12

Plate Offsets (X,Y): [8:0-3-0,0-3-0], [10:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	0.45	10-12	>246	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	0.37	10-12	>294	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.06	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 258 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-15, 7-15, 7-14, 8-12

REACTIONS (lb/size) 12=2020/0-3-8, 19=1353/0-3-8, 10=239/0-3-8
Max Horz 19=-210(load case 6)
Max Uplift12=-964(load case 6), 19=-661(load case 5), 10=-327(load case 6)
Max Grav 12=2020(load case 1), 19=1353(load case 1), 10=309(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-342/148, 3-4=-1676/956, 4-5=-1466/919, 5-6=-1267/817, 6-7=-1199/887, 7-8=-807/640, 8-9=-155/559, 9-10=-142/356, 10-11=0/35, 2-19=-351/326
BOT CHORD 18-19=-679/1384, 17-18=-659/1659, 16-17=-659/1657, 15-16=-659/1657, 14-15=-135/802, 13-14=-9/458, 12-13=-9/458, 10-12=-275/257
WEBS 3-18=-11/119, 4-18=-240/514, 5-18=-416/241, 5-17=0/103, 5-15=-734/483, 6-15=-247/273, 7-15=-558/946, 7-14=-414/217, 8-14=-133/589, 8-12=-1684/885, 9-12=-374/467, 3-19=-1423/860

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 964 lb uplift at joint 12, 661 lb uplift at joint 19 and 327 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#I115051999
L139892	T17	SPECIAL	1	1	Job Reference (optional)

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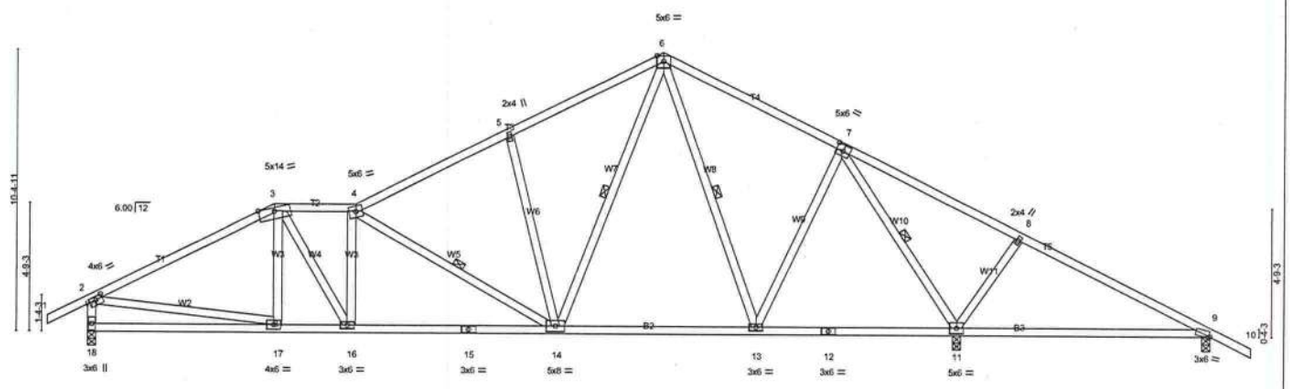
1-6-0	6-10-0	9-10-1	15-5-8	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	6-10-0	3-0-1	5-7-8	5-7-8	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:74.0



6-10-0	9-10-1	17-2-2	24-6-3	31-10-4	41-2-0
6-10-0	3-0-1	7-4-1	7-4-1	7-4-1	9-3-12

Plate Offsets (X,Y): [2:0-3-0,0-1-8], [7:0-3-0,0-3-0], [9:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	0.45	9-11	>242	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	0.38	9-11	>287	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 248 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-14, 6-14, 6-13, 7-11

REACTIONS (lb/size) 18=1344/0-3-8, 11=2058/0-3-8, 9=209/0-3-8
Max Horz 18=-210(load case 6)
Max Uplift18=-659(load case 5), 11=-974(load case 6), 9=-319(load case 6)
Max Grav 18=1344(load case 1), 11=2058(load case 1), 9=290(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1753/944, 3-4=-1843/1098, 4-5=-1405/866, 5-6=-1334/965, 6-7=-823/642, 7-8=-181/622, 8-9=-158/419, 9-10=0/35, 2-18=-1227/833
BOT CHORD 17-18=-268/290, 16-17=-654/1486, 15-16=-782/1856, 14-15=-782/1856, 13-14=-133/783, 12-13=-3/421, 11-12=-3/421, 9-11=-328/280
WEBS 3-17=-77/113, 3-16=-324/663, 4-16=-404/308, 4-14=-764/492, 5-14=-307/357, 6-14=-610/992, 6-13=-375/219, 7-13=-144/607, 7-11=-1721/911, 8-11=-376/469, 2-17=-513/1214

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 659 lb uplift at joint 18, 974 lb uplift at joint 11 and 319 lb uplift at joint 9.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK
L139892	T18	HIP	1	1	Job Reference (optional)

Dwg.#I115052000

Builders FirstSource, Lake City, Fl 32055

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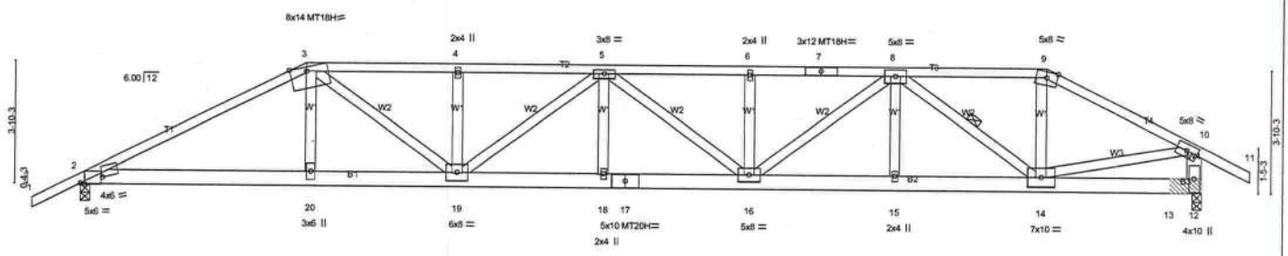
-1-6-0	3-9-4	7-0-0	11-8-4	16-2-12	20-9-4	25-3-12	30-0-0	34-10-0	36-4-0
1-6-0	3-9-4	3-2-12	4-8-4	4-6-8	4-6-8	4-6-8	4-8-4	4-10-0	1-6-0

Scale = 1:62.7



3-9-4	7-0-0	11-8-4	16-2-12	20-9-4	25-3-12	30-0-0	34-10-0
3-9-4	3-2-12	4-8-4	4-6-8	4-6-8	4-6-8	4-8-4	4-10-0

Plate Offsets (X,Y): [2:0-8-10,0-0-7], [2:0-1-10,Edge], [3:0-6-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	0.56	16-18	>736	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.89	16-18	>464	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.74	Horz(TL)	0.17	12	n/a	n/a	MT18H	244/190
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 218 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 6 SYP No.1D, W3 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-9-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-1-3 oc bracing.
WEBS 1 Row at midpt 8-14

REACTIONS (lb/size) 2=2968/0-3-8, 12=3155/0-3-12 (0-3-8 + bearing block)
Max Horz 2=-241(load case 5)
Max Uplift2=-1564(load case 4), 12=-1636(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-5883/3187, 3-4=-6969/3936, 4-5=-6968/3937, 5-6=-7366/4173, 6-7=-7366/4173, 7-8=-7366/4173, 8-9=-3634/2132, 9-10=-4128/2241, 10-11=0/42, 10-12=-2918/1614
BOT CHORD 2-20=-2678/5179, 19-20=-2691/5214, 18-19=-4160/7733, 17-18=-4160/7733, 16-17=-4160/7733, 15-16=-3233/6063, 14-15=-3233/6063, 13-14=-268/477, 12-13=-268/477
WEBS 3-20=-321/838, 3-19=-1356/2280, 4-19=-518/529, 5-19=-991/587, 5-18=0/338, 5-16=-480/258, 6-16=-494/497, 8-16=-909/1655, 8-15=0/337, 8-14=-3088/1740, 9-14=-546/1325, 10-14=-1636/3193

NOTES
1) 2 X 6 SYP No.1D bearing block 12" long at jt. 12 attached to front face with 3 rows of 0.131"x3" Nails spaced 3" o.c. 12 Total fasteners. Bearing is assumed to be SYP.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1564 lb uplift at joint 2 and 1636 lb uplift at joint 12.
7) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 348 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-9=-113(F=-58), 9-10=-112(F=-58), 10-11=-54, 2-20=-30, 12-20=-62(F=-33)
Concentrated Loads (lb)
Vert: 20=-539(F)

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#1115052001
L139892	T19	HIP	1	1	Job Reference (optional)

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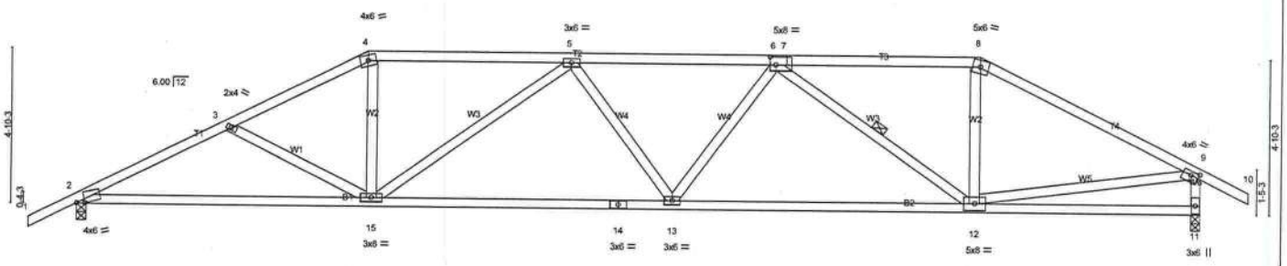
-1-6-0	4-9-4	9-0-0	15-4-0	21-8-0	28-0-0	34-10-0	36-4-0
1-6-0	4-9-4	4-2-12	6-4-0	6-4-0	6-4-0	6-10-0	1-6-0

Scale = 1:62.7



9-0-0	18-6-0	28-0-0	34-10-0
9-0-0	9-6-0	9-6-0	6-10-0

Plate Offsets (X,Y): [2:0-3-0,0-0-11], [7:0-2-0,0-3-0], [9:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.30	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.50	12-13	>836	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.12	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 181 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-9-8 oc bracing.
WEBS 1 Row at midpt 6-12

REACTIONS (lb/size) 2=1540/0-3-8, 11=1540/0-3-8
Max Horz 2=118(load case 5)
Max Uplift2=-650(load case 5), 11=-620(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2681/1409, 3-4=-2457/1276, 8-9=-2081/1066, 9-10=0/40, 9-11=-1444/909, 4-5=-2180/1216, 5-6=-2748/1477, 6-7=-1803/1044, 7-8=-1803/1044
BOT CHORD 2-15=-1100/2343, 14-15=-1194/2715, 13-14=-1194/2715, 12-13=-1127/2582, 11-12=-85/217
WEBS 3-15=-212/258, 4-15=-290/779, 5-15=-749/451, 5-13=0/109, 6-13=-25/315, 6-12=-1035/549, 8-12=-161/598, 9-12=-646/1592

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 650 lb uplift at joint 2 and 620 lb uplift at joint 11.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

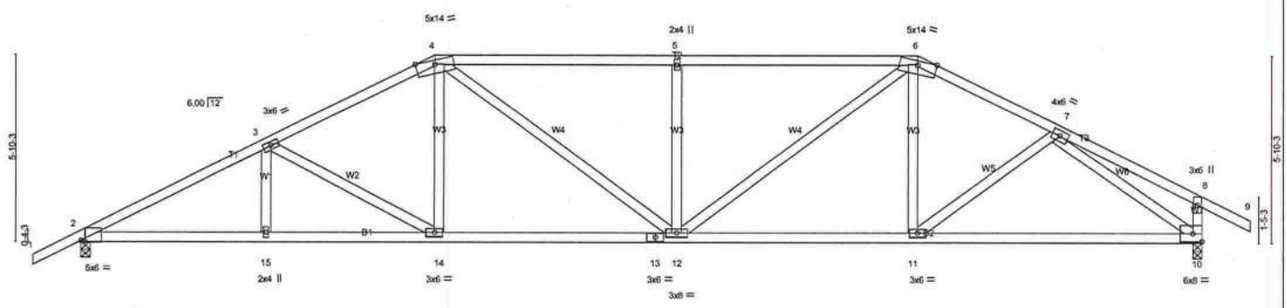
Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK
L139892	T20	HIP	1	1	Dwg.#115052002

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 15:21:33 2005 Page 1

-1-6-0	5-9-4	11-0-0	18-6-0	26-0-0	30-3-4	34-10-0	36-4-0
1-6-0	5-9-4	5-2-12	7-6-0	7-6-0	4-3-4	4-6-12	1-6-0

Scale = 1:62.7



5-9-4	11-0-0	18-6-0	26-0-0	34-10-0
5-9-4	5-2-12	7-6-0	7-6-0	8-10-0

Plate Offsets (X,Y): [2:0-1-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.21	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.35	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.11	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 192 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-1-5 oc bracing.

REACTIONS (lb/size) 2=1540/0-3-8, 10=1540/0-3-8
Max Horz 2=135(load case 5)
Max Uplift2=-672(load case 5), 10=-644(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2733/1390, 3-4=-2270/1241, 4-5=-2303/1341, 5-6=-2303/1341, 6-7=-1994/1106, 7-8=-336/172, 8-9=0/40, 8-10=-358/345
BOT CHORD 2-15=-1073/2365, 14-15=-1073/2365, 13-14=-791/1990, 12-13=-791/1990, 11-12=-662/1754, 10-11=-695/1582
WEBS 3-15=0/165, 3-14=-442/325, 4-14=-119/410, 4-12=-291/513, 5-12=-429/375, 6-12=-377/762, 6-11=-10/135, 7-11=-133/317, 7-10=-1700/954

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 672 lb uplift at joint 2 and 644 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

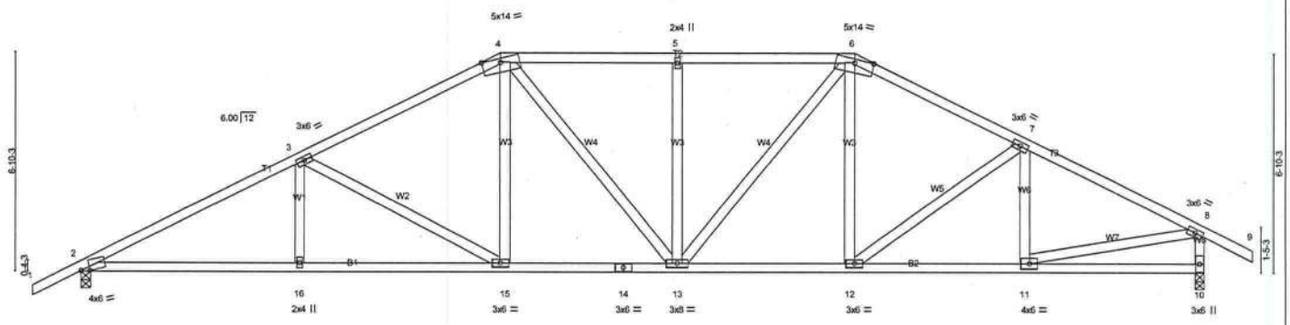
Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK
L139892	T21	HIP	1	1	Dwg.#1115052003

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 15:21:34 2005 Page 1

-1-6-0	6-9-4	13-0-0	18-6-0	24-0-0	29-3-4	34-10-0	36-4-0
1-6-0	6-9-4	6-2-12	5-6-0	5-6-0	5-3-4	5-6-12	1-6-0

Scale = 1:62.7



6-9-4	13-0-0	18-6-0	24-0-0	29-3-4	34-10-0
6-9-4	6-2-12	5-6-0	5-6-0	5-3-4	5-6-12

Plate Offsets (X,Y): [2:0-3-4,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.16	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.27	15-16	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.10	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 204 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-8 oc bracing.

REACTIONS (lb/size) 2=1540/0-3-8, 10=1540/0-3-8
Max Horz 2=151(load case 5)
Max Uplift2=-691(load case 5), 10=-665(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2705/1394, 3-4=-2097/1185, 4-5=-1920/1187, 5-6=-1920/1187, 6-7=-1911/1107, 7-8=-1990/1068, 8-9=0/40, 8-10=-1448/924
BOT CHORD 2-16=-1066/2337, 15-16=-1066/2337, 14-15=-687/1814, 13-14=-687/1814, 12-13=-606/1659, 11-12=-730/1717, 10-11=-34/162
WEBS 3-16=0/220, 3-15=-608/435, 4-15=-172/462, 4-13=-189/307, 5-13=-303/269, 6-13=-244/505, 6-12=-78/233, 7-12=-109/183, 7-11=-196/203, 8-11=-742/1592

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 691 lb uplift at joint 2 and 665 lb uplift at joint 10.

LOAD CASE(S) Standard

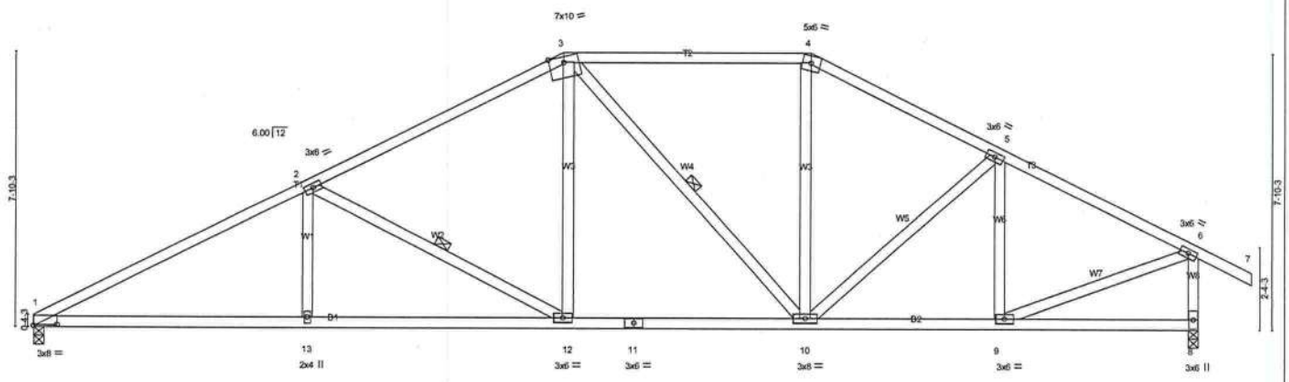
NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg.#115052004
L139892	T22	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 15:21:35 2005 Page 1

7-9-4	15-0-0	22-0-0	27-4-4	33-0-0	34-6-0
7-9-4	7-2-12	7-0-0	5-4-4	5-7-12	1-6-0

Scale = 1:57.2



7-9-4	15-0-0	22-0-0	27-4-4	33-0-0
7-9-4	7-2-12	7-0-0	5-4-4	5-7-12

Plate Offsets (X,Y): [1:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.20	1-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.32	1-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 187 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-9-5 oc bracing.
WEBS 1 Row at midpt 2-12, 3-10

REACTIONS (lb/size) 1=1372/0-3-8, 8=1465/0-3-8
Max Horz 1=181(load case 4)
Max Uplift1=-566(load case 5), 8=-640(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2511/1342, 2-3=-1777/1055, 3-4=-1356/950, 4-5=-1568/977, 5-6=-1539/883, 6-7=0/40, 6-8=-1378/897
BOT CHORD 1-13=-1025/2160, 12-13=-1025/2160, 11-12=-537/1518, 10-11=-537/1518, 9-10=-523/1314, 8-9=-18/77
WEBS 2-13=0/267, 2-12=-740/558, 3-12=-211/552, 3-10=-346/175, 4-10=-135/335, 5-10=-98/168, 5-9=-322/252, 6-9=-597/1325

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 566 lb uplift at joint 1 and 640 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

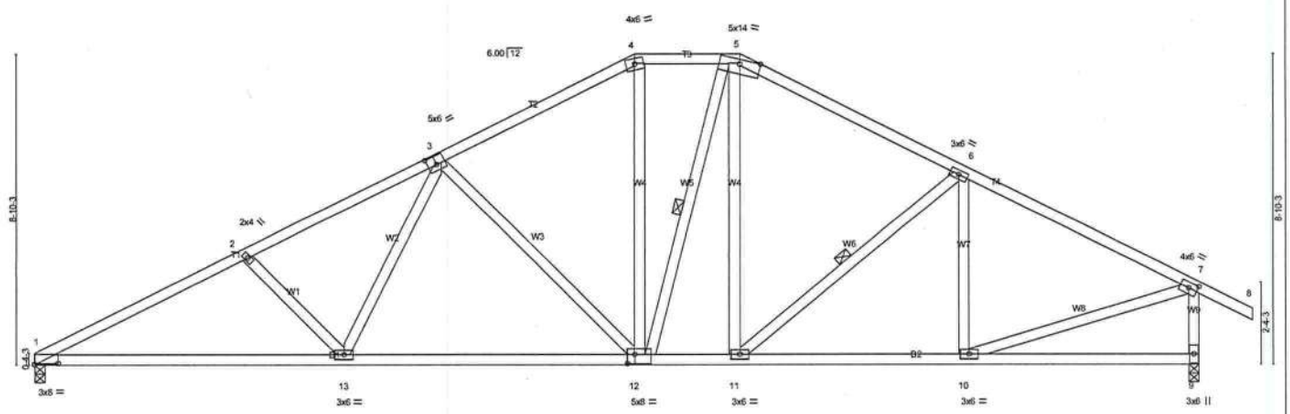
Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK
L139892	T23	HIP	1	1	Job Reference (optional)

Dwg.#115052005

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 15:21:35 2005 Page 1

6-0-5	11-6-3	17-0-0	20-0-0	26-4-4	33-0-0	34-6-0
6-0-5	5-5-13	5-5-13	3-0-0	6-4-4	6-7-12	1-6-0

Scale = 1:57.2



8-9-4	17-0-0	20-0-0	26-4-4	33-0-0
8-9-4	8-2-12	3-0-0	6-4-4	6-7-12

Plate Offsets (X,Y): [1:0-8-0,0-0-6], [3:0-3-0,0-3-0], [7:0-2-15,0-2-0], [12:0-2-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.20	1-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.34	1-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.08	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 202 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-9-10 oc bracing.
WEBS 1 Row at midpt 5-12, 6-11

REACTIONS (lb/size) 1=1372/0-3-8, 9=1465/0-3-8
Max Horz 1=198(load case 4)
Max Uplift1=-601(load case 5), 9=-657(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2520/1409, 2-3=-2309/1330, 3-4=-1560/1016, 4-5=-1332/978, 5-6=-1499/974, 6-7=-1620/934, 7-8=0/40, 7-9=-1368/908
BOT CHORD 1-13=-1111/2200, 12-13=-768/1768, 11-12=-389/1270, 10-11=-555/1379, 9-10=-13/102
WEBS 2-13=-291/352, 3-13=-236/551, 3-12=-634/513, 4-12=-259/452, 5-12=-151/328, 5-11=-130/267, 6-11=-241/236, 6-10=-213/233, 7-10=-598/1343

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 601 lb uplift at joint 1 and 657 lb uplift at joint 9.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK
L139892	T24	COMMON	2	1	Job Reference (optional)

Dwg.#115052006

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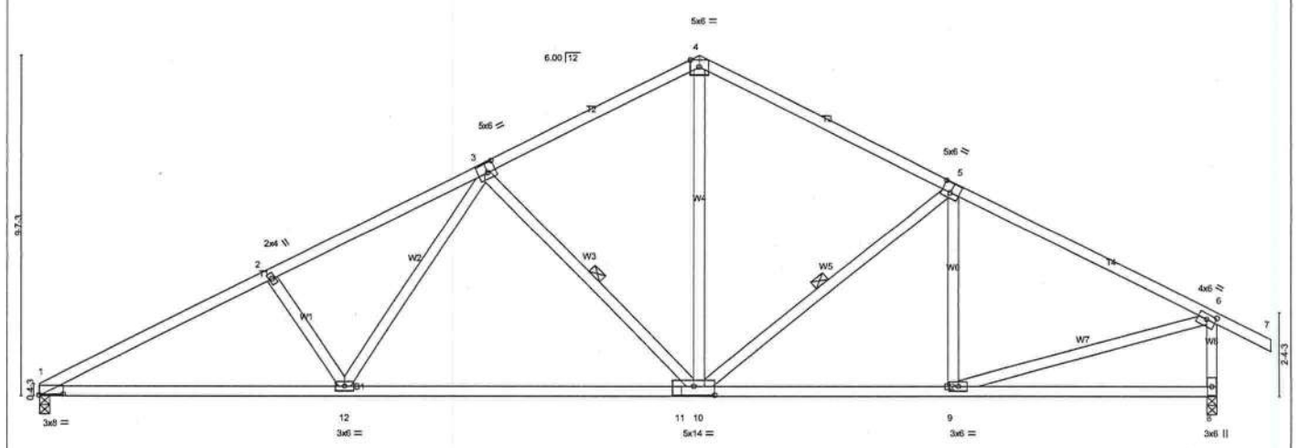
6-6-5	12-6-3	18-6-0	25-7-4	33-0-0	34-6-0
6-6-5	5-11-13	5-11-13	7-1-4	7-4-12	1-6-0

Scale = 1:56.6



8-7-0	18-6-0	25-7-4	33-0-0
8-7-0	9-11-0	7-1-4	7-4-12

Plate Offsets (X,Y): [1:0-8-0,0-0-6], [3:0-2-12,0-3-0], [5:0-3-0,0-3-4], [6:0-3-0,0-1-12], [11:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.26	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.44	10-12	>886	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 183 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-13 oc bracing.
WEBS 1 Row at midpt 3-10, 5-10

REACTIONS (lb/size) 1=1372/0-3-8, 8=1465/0-3-8
Max Horz 1=211(load case 5)
Max Uplift1=-585(load case 5), 8=-670(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2520/1398, 2-3=-2346/1379, 3-4=-1442/968, 4-5=-1462/960, 5-6=-1651/947, 6-7=0/40, 6-8=-1350/921
BOT CHORD 1-12=-1188/2195, 11-12=-807/1676, 10-11=-807/1676, 9-10=-648/1399, 8-9=-56/141
WEBS 2-12=-304/381, 3-12=-324/666, 3-10=-669/537, 4-10=-507/869, 5-10=-303/288, 5-9=-171/219, 6-9=-616/1309

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 585 lb uplift at joint 1 and 670 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

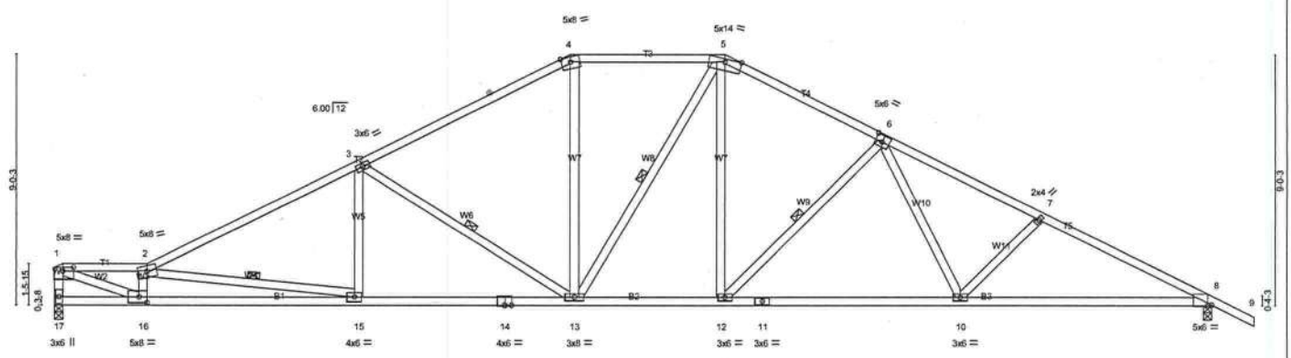
Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNONGATE II
L139892	T25	SPECIAL	1	1	Job Reference (optional)

Dwg#115052007

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 15:21:37 2005 Page 1

0-3-8	3-3-9	10-9-12	18-4-0	23-10-0	29-5-3	35-0-5	41-2-0	42-8-0
0-3-8	3-0-1	7-6-4	7-6-4	5-6-0	5-7-3	5-7-3	6-1-11	1-6-0

Scale = 1:72.0



3-3-9	10-9-12	18-4-0	23-10-0	32-2-12	41-2-0
3-3-9	7-6-4	7-6-4	5-6-0	8-4-12	8-11-4

Plate Offsets (X,Y): [1:0-4-8,0-1-12], [6:0-3-0,0-3-0], [8:0-1-11,Edge], [16:0-3-8,0-2-8]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.35	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.57	15-16	>866	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.16	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 236 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D *Except*
B2 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-5-11 oc bracing.
WEBS 1 Row at midpt 2-15, 3-13, 5-13, 6-12

REACTIONS (lb/size) 17=1715/0-3-8, 8=1807/0-3-8
Max Horz 17=-246(load case 6)
Max Uplift17=-560(load case 6), 8=-812(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4004/2182, 2-3=-3243/1755, 3-4=-2343/1393, 4-5=-2018/1343, 5-6=-2278/1388, 6-7=-3030/1682, 7-8=-3239/1756, 8-9=0/35, 1-17=-1616/867
BOT CHORD 16-17=-109/191, 15-16=-2155/4182, 14-15=-1314/2833, 13-14=-1314/2833, 12-13=-720/1986, 11-12=-1068/2417, 10-11=-1068/2417, 8-10=-1386/2833
WEBS 1-16=-2263/4124, 2-16=-1468/950, 2-15=-1366/851, 3-15=-99/488, 3-13=-983/682, 4-13=-288/656, 5-13=-156/242, 5-12=-325/654, 6-12=-633/506, 6-10=-206/544, 7-10=-278/334

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 560 lb uplift at joint 17 and 812 lb uplift at joint 8.

LOAD CASE(S) Standard

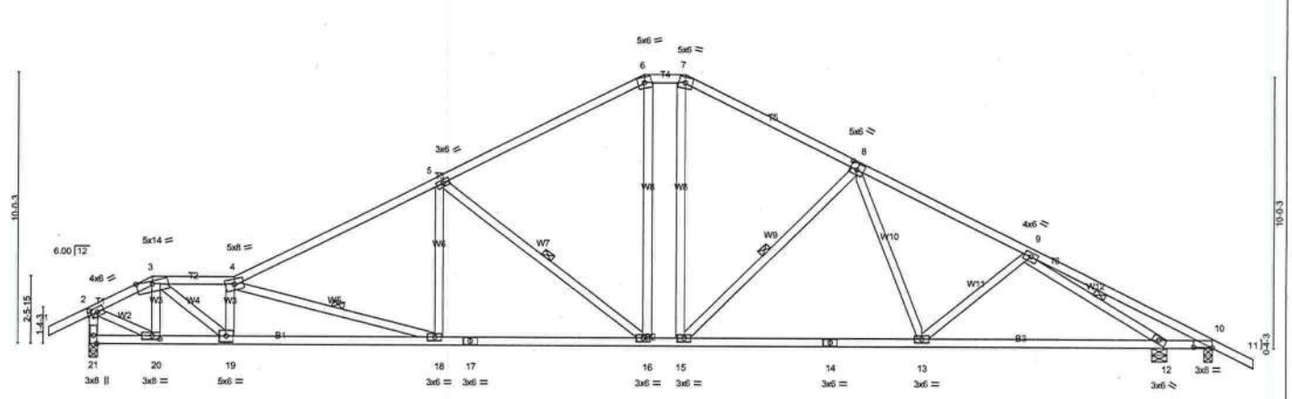
NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON GREEN Dwg#115052008
L139892	T26	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 15:21:38 2005 Page 1

1-6-0	2-3-8	5-3-9	12-9-12	20-4-0	21-10-0	28-1-3	34-4-5	41-2-0	42-8-0
1-6-0	2-3-8	3-0-1	7-6-4	7-6-4	1-6-0	6-3-3	6-3-3	6-9-11	1-6-0

Scale = 1:74.0



2-3-8	5-3-9	12-9-12	20-4-0	21-10-0	30-6-6	39-2-12	41-2-0
2-3-8	3-0-1	7-6-4	7-6-4	1-6-0	8-8-6	8-8-6	1-11-4

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [8:0-3-0,0-3-0], [10:0-8-0,0-0-6], [20:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.31	16-18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.49	16-18	>959	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.74	Horz(TL)	0.14	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 251 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-9-3 oc bracing.
WEBS 1 Row at midpt 4-18, 5-16, 8-15, 9-12

REACTIONS (lb/size) 21=1739/0-3-8, 12=1760/0-6-7, 10=113/0-3-8
Max Horz 21=-204(load case 6)
Max Uplift21=-814(load case 5), 12=-627(load case 6), 10=-269(load case 6)
Max Grav 21=1739(load case 1), 12=1760(load case 1), 10=118(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1763/976, 3-4=-3327/1878, 4-5=-2818/1590, 5-6=-1970/1251, 6-7=-1682/1217, 7-8=-1948/1262, 8-9=-2450/1437, 9-10=-242/163, 10-11=0/35, 2-21=-1682/1052
BOT CHORD 20-21=-139/209, 19-20=-688/1488, 18-19=-1733/3410, 17-18=-1103/2449, 16-17=-1103/2449, 15-16=-560/1682, 14-15=-877/2032, 13-14=-877/2032, 12-13=-1076/2109, 10-12=-1/138
WEBS 3-20=-687/349, 3-19=-1294/2318, 4-19=-1256/851, 4-18=-1001/656, 5-18=-149/547, 5-16=-1000/694, 6-16=-328/609, 7-15=-376/635, 8-15=-574/463, 8-13=-96/315, 9-13=-27/230, 9-12=-2382/1347, 2-20=-829/1643

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 814 lb uplift at joint 21, 627 lb uplift at joint 12 and 269 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANOPY GLEN
L139892	T27	SPECIAL	1	1	Job Reference (optional)

Dwg #1115052009

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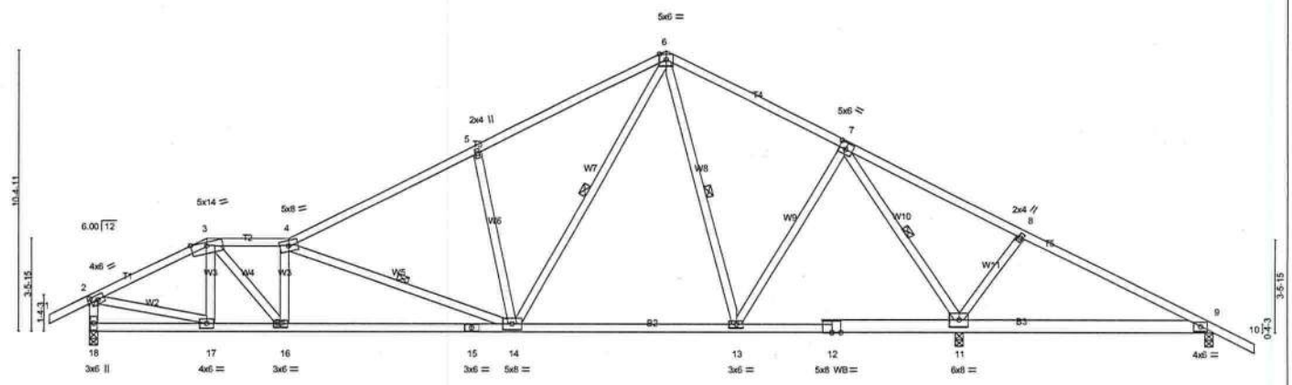
1-6-0	4-3-8	7-3-9	14-2-4	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	4-3-8	3-0-1	6-10-12	6-10-12	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:74.0



4-3-8	7-3-9	15-5-13	23-8-0	31-10-4	41-2-0
4-3-8	3-0-1	8-2-4	8-2-4	8-2-4	9-3-12

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [7:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.14	9-11	>789	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	0.12	9-11	>918	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 251 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-14, 6-14, 6-13, 7-11

REACTIONS (lb/size) 18=1332/0-3-8, 11=2113/0-3-8, 9=166/0-3-8
Max Horz 18=-212(load case 6)
Max Uplift18=-651(load case 5), 11=-995(load case 6), 9=-303(load case 6)
Max Grav 18=1332(load case 1), 11=2113(load case 1), 9=268(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1610/869, 3-4=-2127/1211, 4-5=-1585/920, 5-6=-1513/1062, 6-7=-836/636, 7-8=-266/715, 8-9=-234/510, 9-10=0/39, 2-18=-1247/817
BOT CHORD 17-18=-166/215, 16-17=-637/1371, 15-16=-968/2157, 14-15=-968/2157, 13-14=-121/761, 12-13=0/374, 11-12=0/376, 9-11=-426/370
WEBS 3-17=-283/164, 3-16=-596/1133, 4-16=-648/493, 4-14=-865/557, 5-14=-377/449, 6-14=-684/1077, 6-13=-342/233, 7-13=-174/633, 7-11=-1771/987, 8-11=-349/436, 2-17=-613/1305

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 651 lb uplift at joint 18, 995 lb uplift at joint 11 and 303 lb uplift at joint 9.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANOPY GREEN Dwg#115052010
L139892	T28	SPECIAL	1	1	Job Reference (optional)

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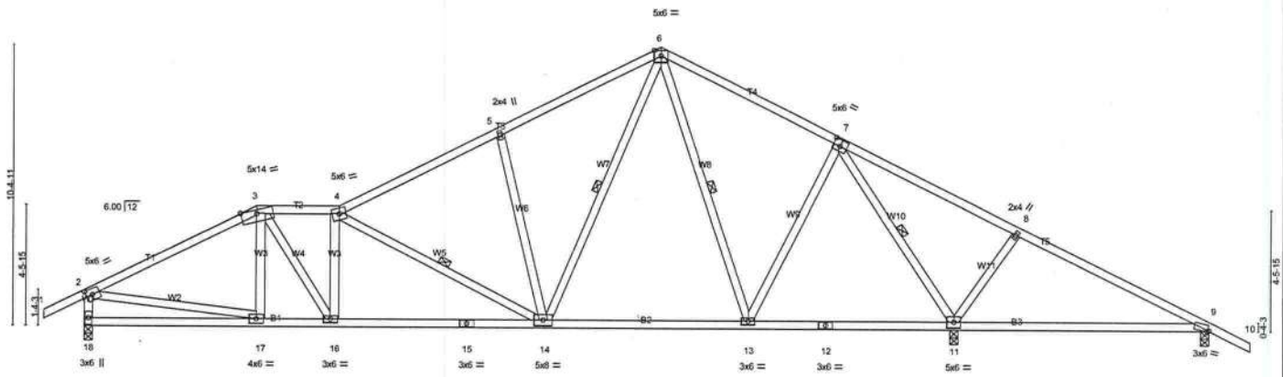
1-6-0	6-3-8	9-3-9	15-2-4	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	6-3-8	3-0-1	5-10-12	5-10-12	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:74.0



6-3-8	9-3-9	16-9-13	24-4-0	31-10-4	41-2-0
6-3-8	3-0-1	7-6-4	7-6-4	7-6-4	9-3-12

Plate Offsets (X,Y): [2:0-2-11,0-2-8], [7:0-3-0,0-3-0], [9:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.46	9-11	>241	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	0.39	9-11	>286	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 246 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-14, 6-14, 6-13, 7-11

REACTIONS (lb/size) 18=1342/0-3-8, 11=2068/0-3-8, 9=202/0-3-8
Max Horz 18=-210(load case 6)
Max Uplift18=-658(load case 5), 11=-976(load case 6), 9=-318(load case 6)
Max Grav 18=1342(load case 1), 11=2068(load case 1), 9=286(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1737/937, 3-4=-1900/1121, 4-5=-1444/880, 5-6=-1372/987, 6-7=-827/642, 7-8=-189/637, 8-9=-165/435, 9-10=0/35, 2-18=-1232/830
BOT CHORD 17-18=-242/248, 16-17=-657/1475, 15-16=-818/1916, 14-15=-818/1916, 13-14=-131/779, 12-13=0/411, 11-12=0/411, 9-11=-342/287
WEBS 3-17=-117/121, 3-16=-378/754, 4-16=-458/351, 4-14=-779/502, 5-14=-322/376, 6-14=-625/1007, 6-13=-365/221, 7-13=-149/613, 7-11=-1730/919, 8-11=-376/470, 2-17=-546/1251

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 658 lb uplift at joint 18, 976 lb uplift at joint 11 and 318 lb uplift at joint 9.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg#1115052011
L139892	T29	MONO TRUSS	1	1	Job Reference (optional)

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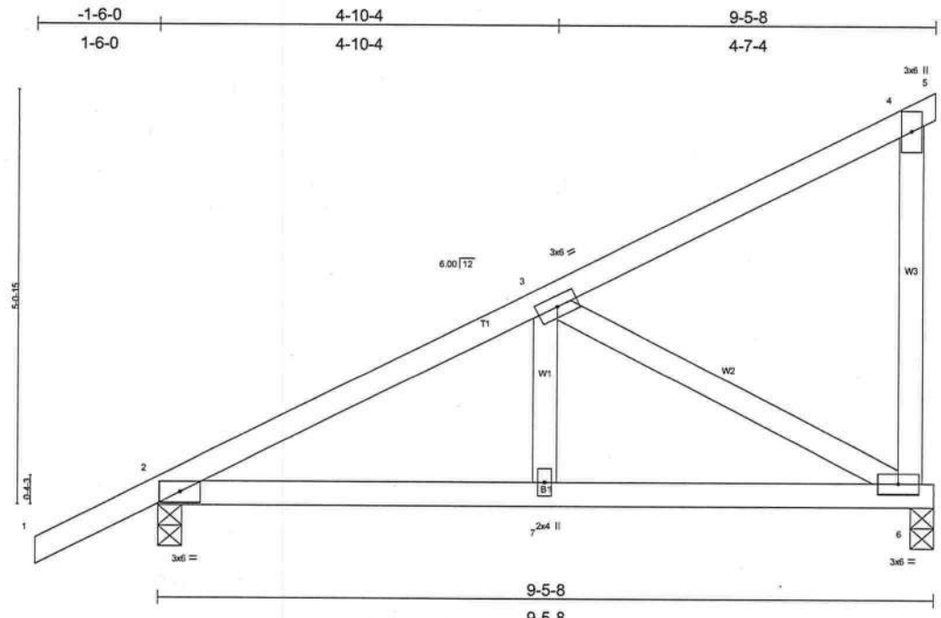

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	0.04	2-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	0.04	2-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.20	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-3-6 oc bracing.

REACTIONS (lb/size) 6=375/0-3-8, 2=476/0-3-8
Max Horz 2=315(load case 5)
Max Uplift6=-385(load case 5), 2=-374(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-512/540, 3-4=-81/27, 4-5=-2/0, 4-6=-104/147
BOT CHORD 2-7=-723/407, 6-7=-723/407
WEBS 3-6=-439/781, 3-7=-324/147

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 385 lb uplift at joint 6 and 374 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 3 CANNON CREEK Dwg#115052012
L139892	T29G	MONO TRUSS	1	1	Job Reference (optional)

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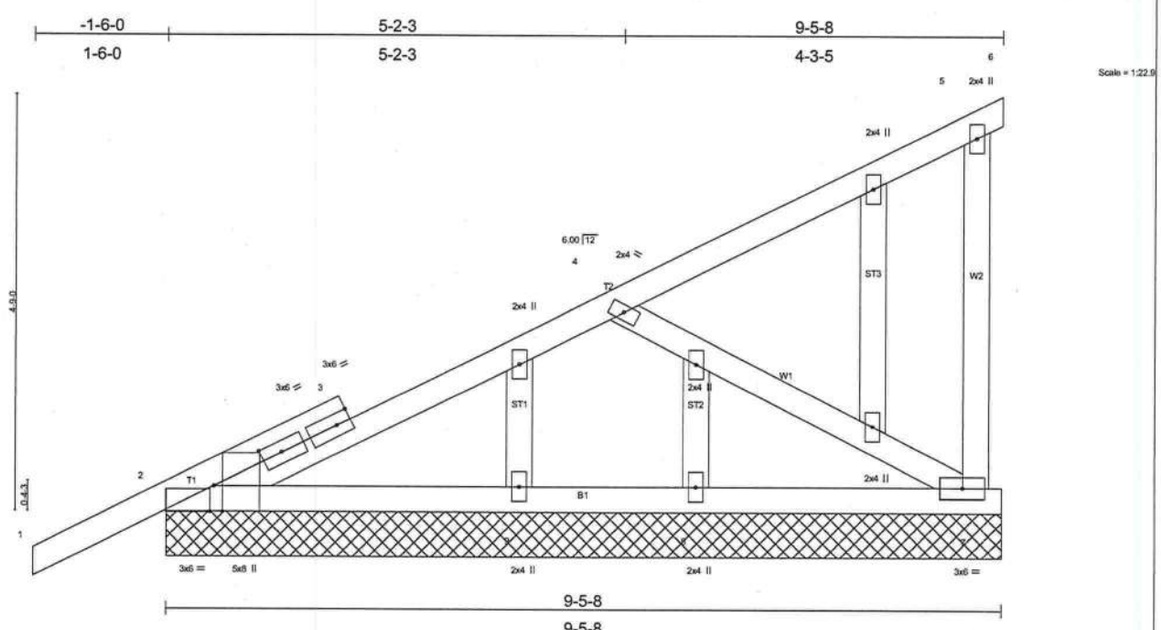

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [3:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.02	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	0.03	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.18	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 56 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-0-2 oc bracing.

REACTIONS (lb/size) 2=522/9-5-8, 6=-161/9-5-8, 7=582/9-5-8, 9=147/9-5-8, 8=52/9-5-8
Max Horz 2=298(load case 5)
Max Uplift2=-313(load case 5), 6=-161(load case 1), 7=-486(load case 5), 9=-12(load case 5)
Max Grav 2=522(load case 1), 6=161(load case 5), 7=582(load case 1), 9=147(load case 1), 8=52(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/51, 2-3=-506/240, 3-4=-450/238, 4-5=-92/54, 5-6=-82/100, 5-7=-323/433
BOT CHORD 2-9=-462/402, 8-9=-462/402, 7-8=-462/402
WEBS 4-7=-447/515

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Gable requires continuous bottom chord bearing.
4) Gable studs spaced at 2-0-0 oc.
5) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 2, 161 lb uplift at joint 6, 486 lb uplift at joint 7 and 12 lb uplift at joint 9.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-79(F=-25), 5-6=-79(F=-25), 2-7=-30

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
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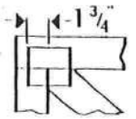
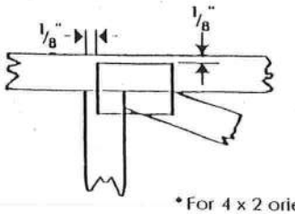
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

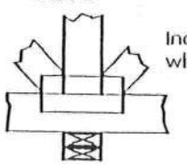
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

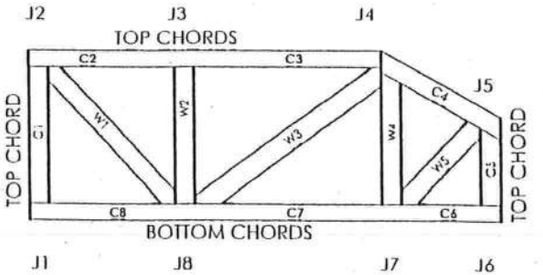
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

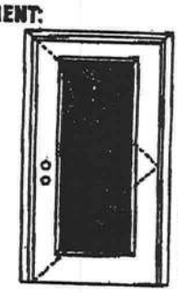
1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

X
Glazed Inswing Unit

COP-WL EN4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Test Data Review Certificate #20264470 and COP/Test Report Validation Matrix #30264470-001 provides additional information - available from the ITS/WH website (www.steeldoor.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+50.5/-50.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series

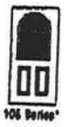

133, 135 Series


135 Series


680 Series


822 Series

1/2 GLASS:


106 Series*


108, 109 Series*


120 Series*


200 Series*


12 R/L, 33 R/L, 34 R/L Series*


197 Series*


106 Series


324 Series

*This glass kit may also be used in the following door styles: 6-panel; 8-panel with scroll; Eyebrow 6-panel; Eyebrow 8-panel with scroll.

Entergy
Entry Systems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Glazed Inswing Unit

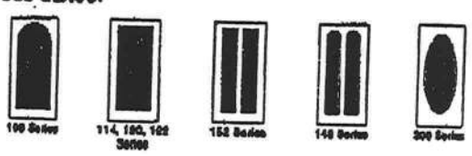
COP WI FN4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:

FULL GLASS:



CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Test Data Review Certificate #0025447C and COP/Test Report Validation Matrix #0025447C-001 provide additional information - available from the ITS/WH website (www.ntsemko.com). The Masonite website (www.masonite.com) or the Masonite technical center.

Entergy
Entry Systems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product data subject to change without notice.

FREMDOOR Collection
Premium Quality Doors

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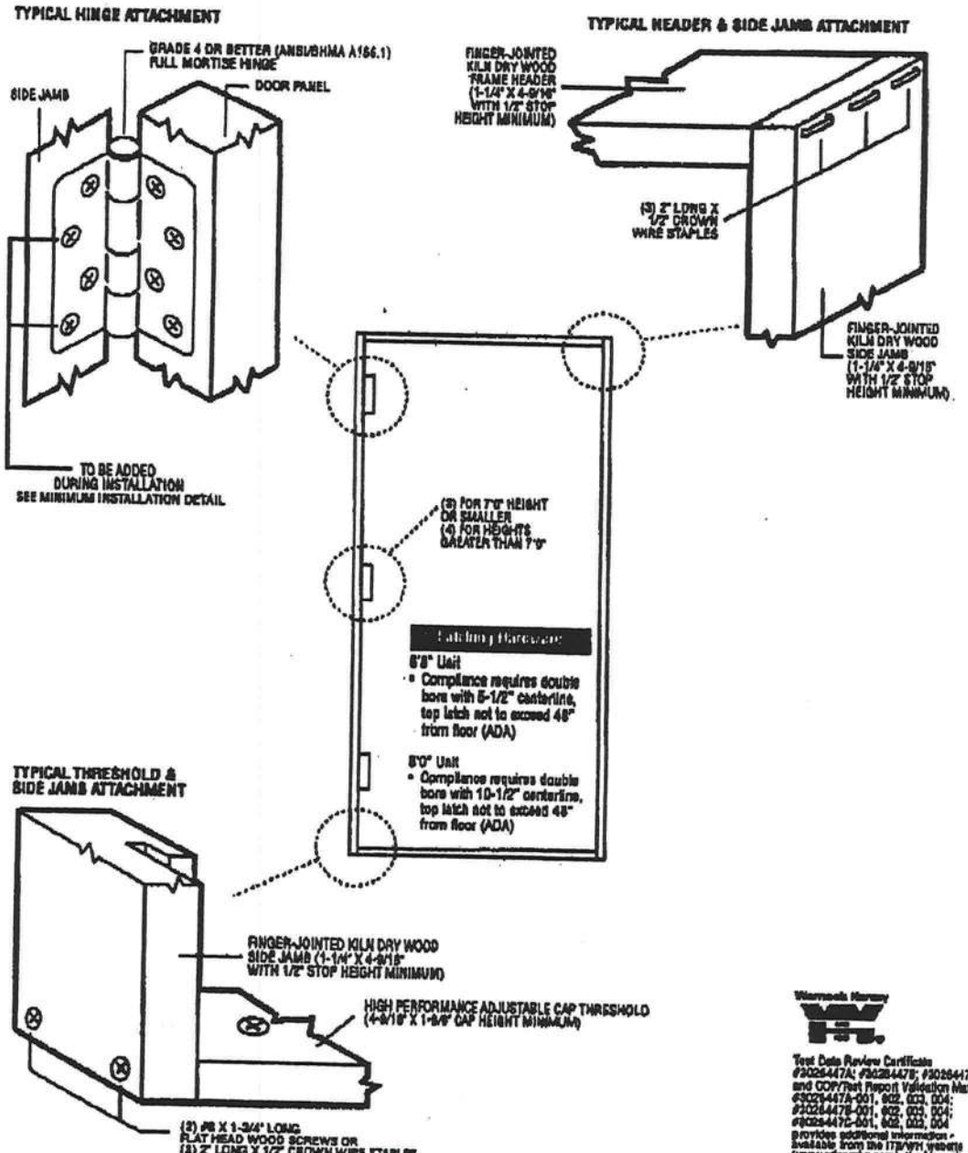
Masonite International Corporation

X
Unit

MAD-WI-MA0001-02

INSWING UNIT WITH SINGLE DOOR

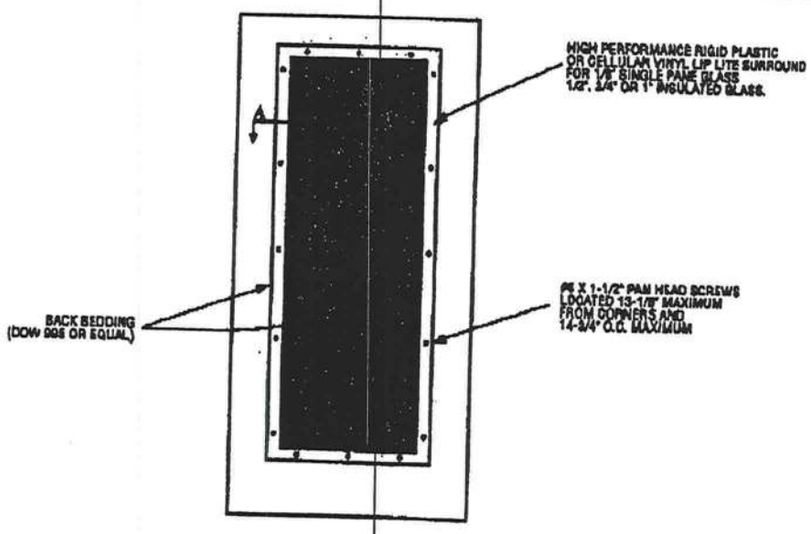


Test Data Review Certificate #30254478; #30254478; #30254470 and CCF/Test Report Validation Matrix #30254478-001, 002, 003, 004; #30254478-001, 002, 003, 004; #30254470-001, 002, 003, 004 provides additional information available from the ITS/WDI website (www.wdiasnap.com), the Masonite website (www.masonite.com) or the Masonite technical center.

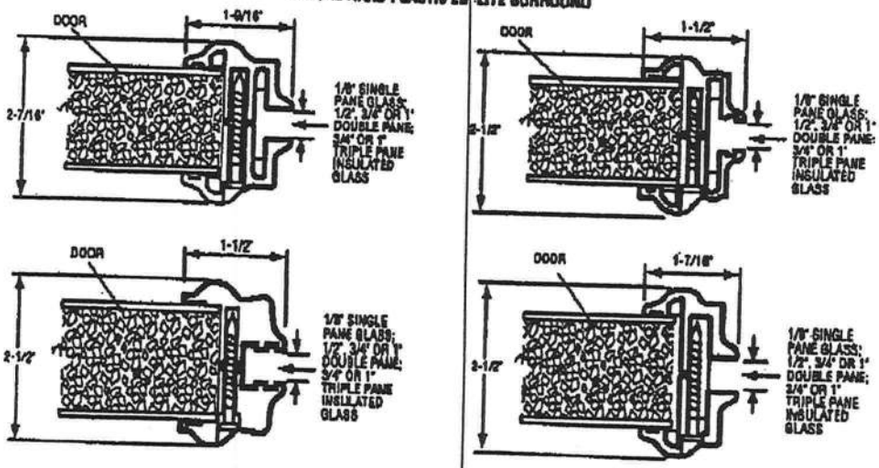
Masonite

MAD-WI-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and CDF/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOOR Collection Prefinished Quality Doors

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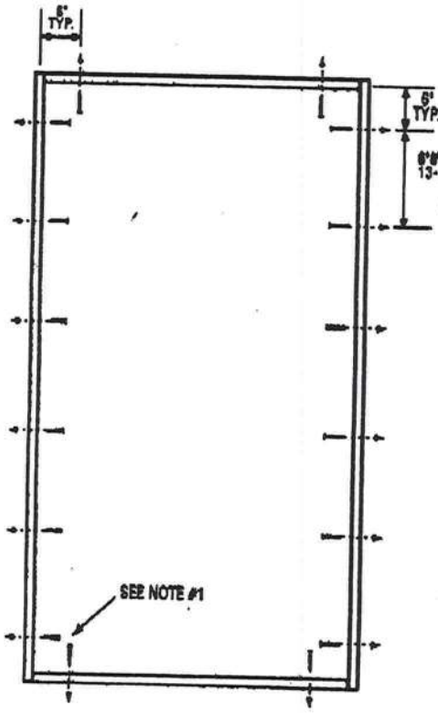
Masonite International Corporation

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate #30264474; #30264478; #30264470 and COP/Test Report Validation Matrix #9026-447A-001, 002, 003, 004; #9026-447B-001, 002, 003, 004; #9026-470-001, 002, 003, 004 provides additional information - available from the ITE/WH website (www.etiamerica.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 9246*, 9256*, 9261*, 9248, 9251* or 9258**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

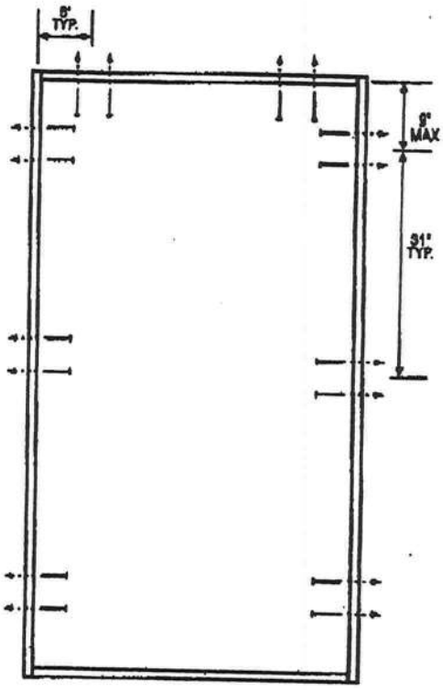
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 8 per vertical framing member for 7'0" height and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 4 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey Test Data Review Certificate #30284478; #30284478, #30284479 and COP/Test Report Validation Matrix #30284478-001, 003, 003, 004; #30284478 001, 002, 003, 004; #30284479-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.wherdata.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0248*, 8285*, 3241*, 3248, 3291* or 3286**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

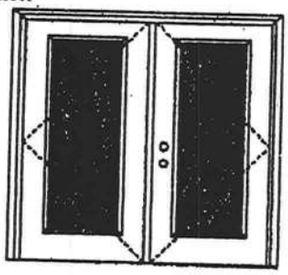
Masonite®

XX
Glazed Outswing Unit

COP-WL-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Wisconsin Testing

This Data Review Confirms #302564470 and COP/Test Report Validation Matrix #302564470-001 provides additional information - available from the ITS/WI website (www.otsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+50.5/-50.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:
1/4 GLASS:


100 Series


133, 135 Series


136 Series


660 Series


622 Series

1/2 GLASS:


105 Series*


108, 160 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


167 Series*


106 Series


304 Series

*This glass kit may also be used in the following door styles: 6-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 6-panel with scroll.

Entergy
Entry Systems

June 17, 2003
Our continuing program of product improvements makes specifications, design and product detail subject to change without notice.

FREMDOOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

XX
Glazed Outswing Unit

COP-WI-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:

FULL GLASS:

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.032" steel. Bottom and rails constructed of 0.082" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Test Data Review Certificate #30284417C and COP/Test Report Validation Matrix #30284417C-001 provide additional information - available from the ITS/WH website (www.qlaminia.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Entergy
Entry Systems

June 17, 2002
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Premium Quality Doors

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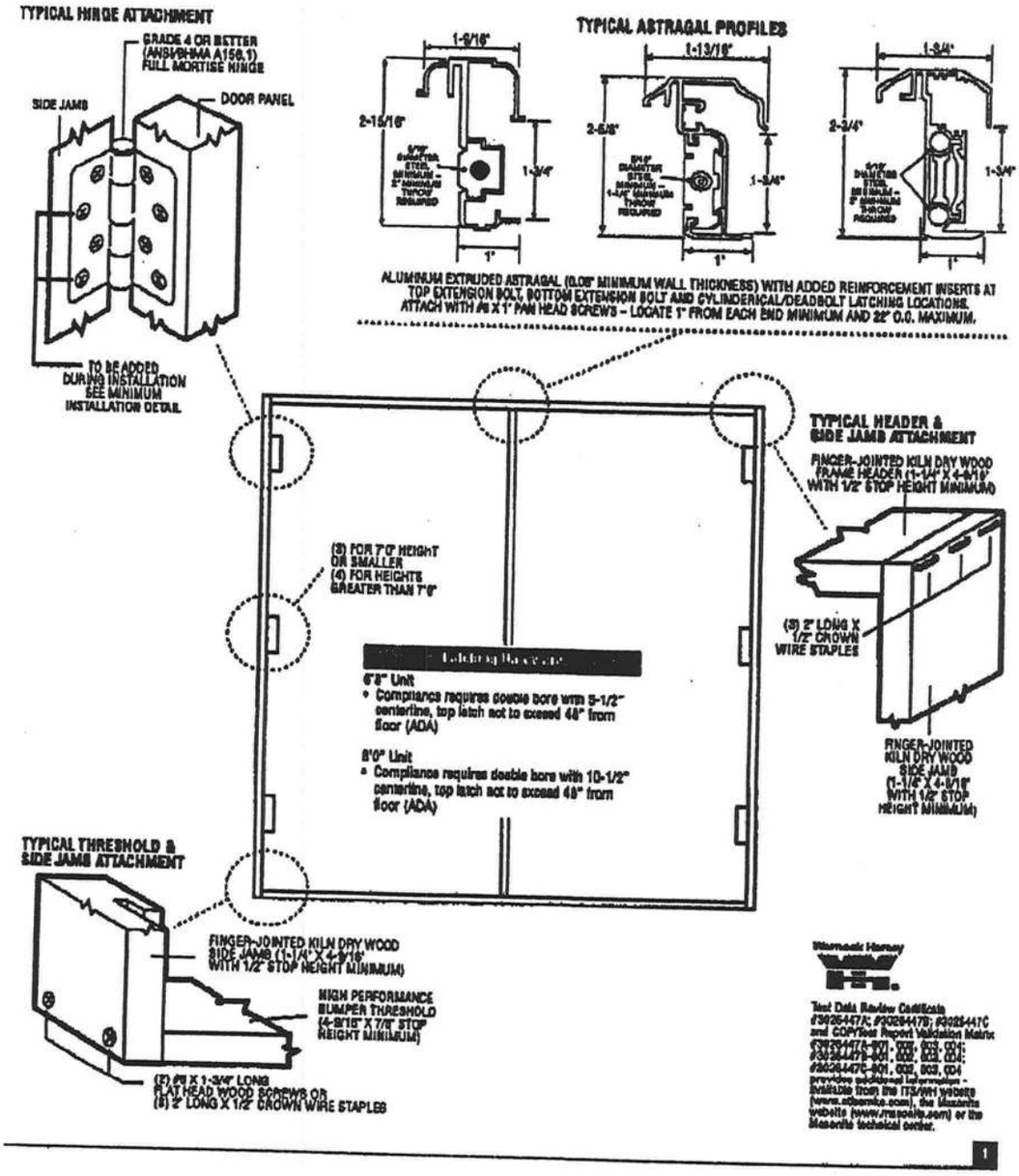
Masonite International Corporation

XX
Unit

MAD WL-MA0012-02

OUTSWING UNITS WITH DOUBLE DOOR



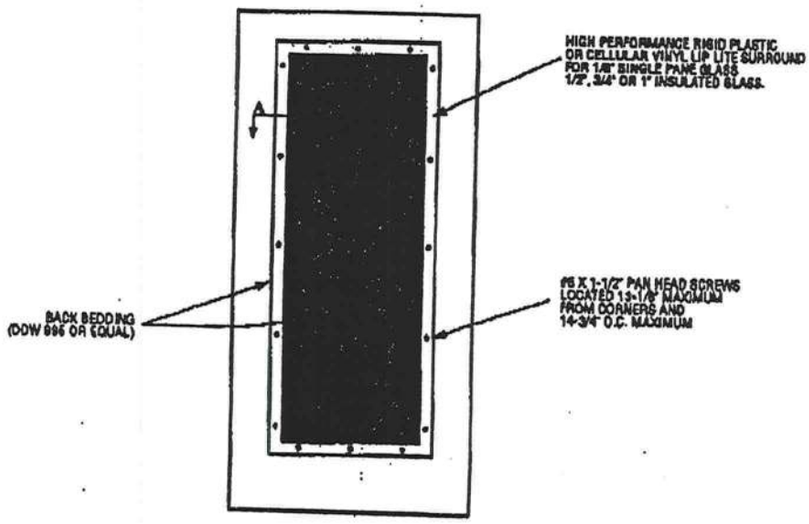
Test Data Review Certificate #30264474; #30264475; #30264476 and COPYtest Report Validation Matrix #30264474-001, 002, 003, 004; #30264475-001, 002, 003, 004; #30264476-001, 002, 003, 004 previous validated information available from the ITS/WH website (www.wdma.com), the Masonite website (www.masonite.com) or the Masonite technical center.

October 14, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

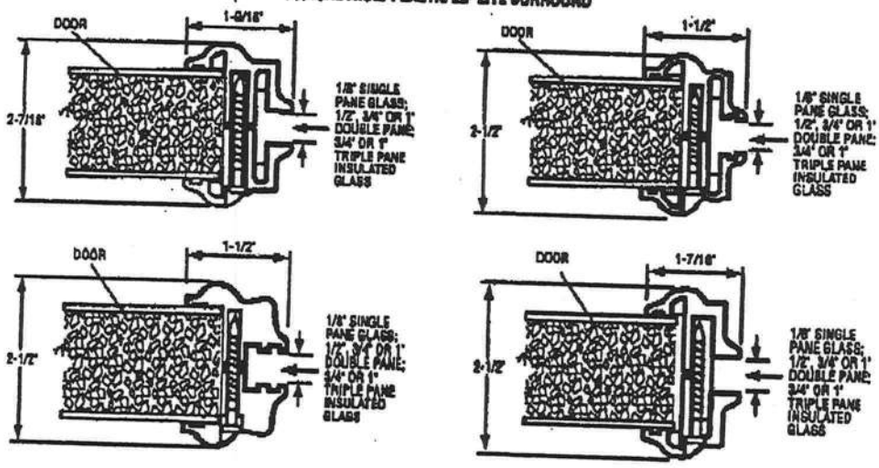
Masonite

MAD-WI-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

Test Data Review Certificate #30284478; #30284478; #30284478 and CCP/Test Report Validation Matrix #30284478-001, 002, 003; #30284478-001, 002, 003; #30284478-001, 002, 003 Provides additional information - available from the ITEANA website (www.ntsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

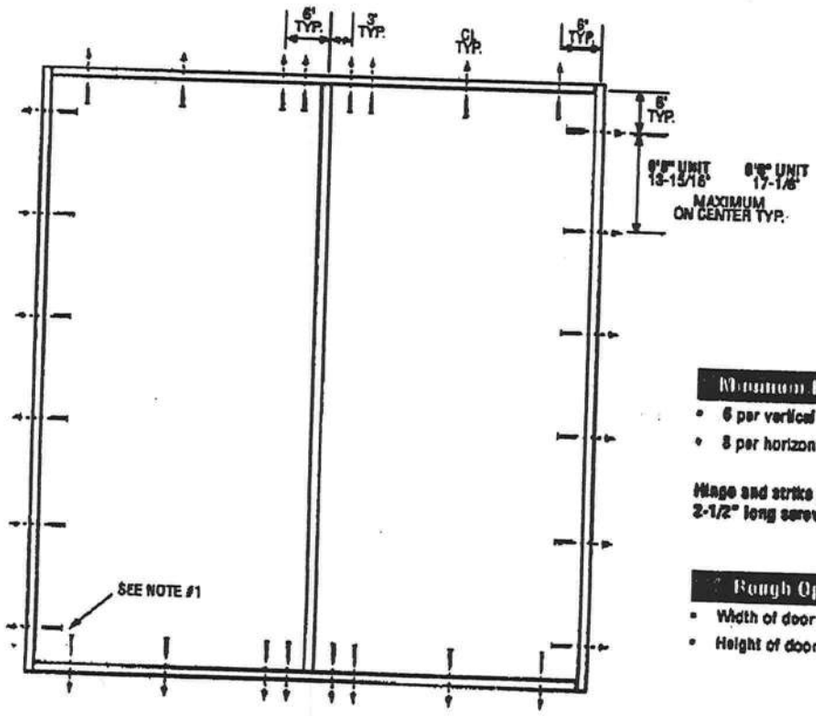
PREMDOR by Masonite Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

XX
Unit

MID-WI-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4" .

Warnock Hersey Test Data Review Certificate #30254447A; #30254447B; #30254447C and CCP/Test Report Validation Matrix #30254447A-001, 002, 003, 004; #30254478-001, 002, 003, 004; #30254447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY CCP DOCUMENT 8247*, 8287*, 3242*, 3247, 3282* or 3267**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see CCP sheet for details.

Notes:

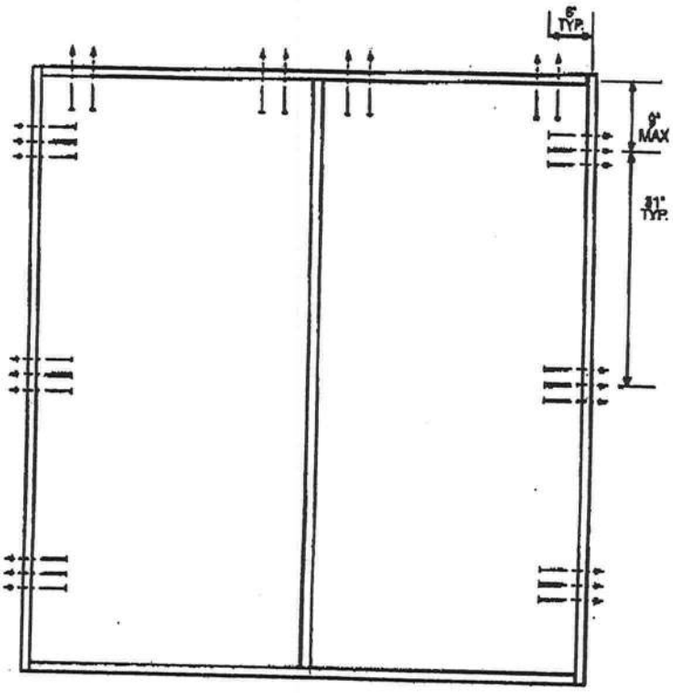
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite®

XX
Unit

MID-WL-MA0002 02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0" heights and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate #30254478; #30254478; #30254470 and COP/Test Report Validation Matrix #30254478-001, 002, 003, 004; #30254478-001, 002, 003, 004; #30254470-001, 002, 003, 004 provides additional information - available from the ITS/Intertek website (www.etsamic.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0247*, 0267*, 3242*, 3247, 3262* or 3267**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 wood screws and 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw and common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite

MI Home Products, Inc.
650 West Market St.
P.O. Box 370
Gratz, PA 17030-0370

(717) 365-3300
(717) 362-7025 Fax

740/744 Single Hung (Fin & Flange)
165 Single Hung (Fin & Flange)
BB165/740/744 Fixed (Fin & Flange)

- Test Reports
  - 165 Single Hung
    - #CTLA-787W (Fin)
    - #CTLA-787W-1 (Flange)
  - 740/744 Single Hung
    - #01-40351.03 (Fin)
    - #01-40351.04 (Flange)
  - 165/740/744 Fixed
    - #NCTL-310-0005-2.1 (Fin)
    - # NCTL-310-0005-5.1 (Flange)
    - #01-40486.03 (2-Panel Fixed)
- Installation Instructions
- Sample 110/120/140 MPH Labels

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 740/744
TYPE: Aluminum Single Hung Window with Nail Fin

Title of Test	Results
Rating	H R45 52 x 72
Overall Design Pressure	45 psf
Operating Force	24 lb max.
Air Infiltration	0.10 cfm/ft^2
Water Resistance	6.75 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-40351.03 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:baw

ALLEN N. REEVES
CERTIFICATE
NO. 1935
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
15 FEBRUARY 2002

THIS FENESTRATION PRODUCT COMPLIES* WITH THE

NEW FLORIDA BUILDING CODE

FOR RESIDENTIAL BUILDINGS WITH A MEAN ROOF HEIGHT OF 30 FT. OR LESS, *EXPOSURE "B"* (WHICH IS INLAND OF A LINE THAT IS 1500 FT. FROM THE COAST), AND *WALL ZONE "5"* (INSTALLED NEAR THE CORNER OF THE BUILDING).

PER *ASTM E1300*, THE CORRECT GLASS THICKNESS, BASED ON THE *NEGATIVE* DESIGN PRESSURE (DP) LISTED BELOW, HAS BEEN INSTALLED IN THIS UNIT. THE GLASS THICKNESS IS BASED ON ITS' WIDTH, HEIGHT, AND ASPECT RATIO.

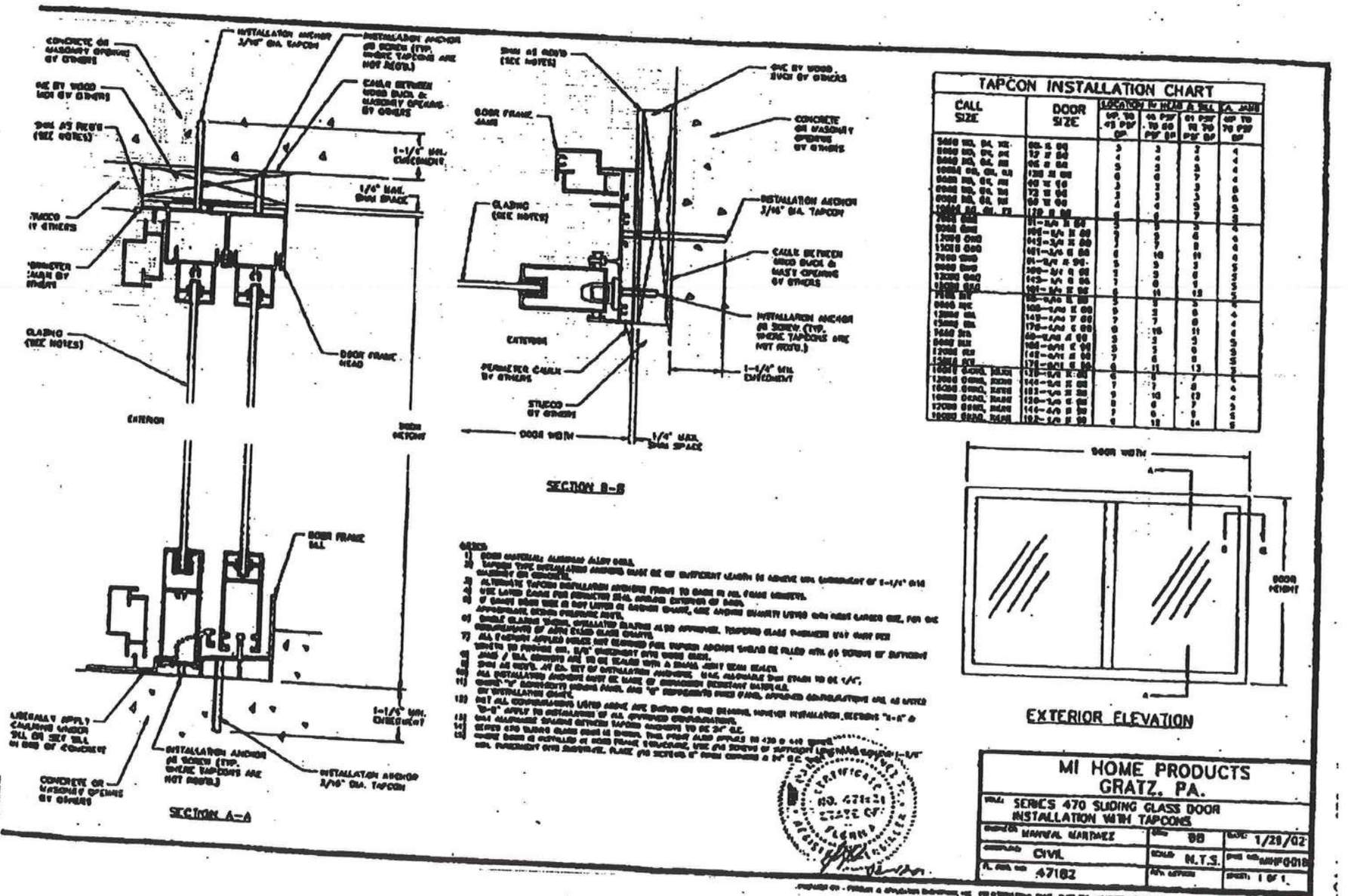
Series 470HP SLIDING GLASS DOOR – all 6'- 8" High Panels

- **2'- 6" WIDE** DP + 40.0 / - 55.4
- **3'- 0" WIDE** DP + 40.0 / - 48.5
- **4'- 0" WIDE** DP + 40.0 / - 40.3

THIS PRODUCT MEETS THE REQUIREMENTS FOR STRUCTURAL LOADS, WATER AND AIR INFILTRATION PER ATTACHED *AAMA* PERFORMANCE LABEL. BE ADVISED THAT IF LOADS ARE PLACED UP TO OR EXCEEDING THE TESTED LEVELS, THIS PRODUCT MAY BE ALTERED IN SUCH A WAY THAT FUTURE PERFORMANCE WILL BE REDUCED.

* COMPLIANCE MUST INCLUDE INSTALLATION ACCORDING TO MANUFACTURER'S INSTRUCTIONS AND FLORIDA CODE REQUIREMENTS.

MIP-686


SECTION A-A
SECTION B-B
EXTERIOR ELEVATION
TAPCON INSTALLATION CHART
MI HOME PRODUCTS
GRATZ, PA.
SERIES 470 SLIDING GLASS DOOR
INSTALLATION WITH TAPCONS
MANUAL MARKUP
CIVIL
47182
N.T.S.
1/29/02

DOCUMENT CONTROL ADDENDUM #01-40351.00

Current Issue Date: 02/15/02

Report No.: 01-40351.01

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 744 aluminum single hung window with flange.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Report No.: 01-40351.02

Requested by: William Emley, MI Home Products, Inc.
Purpose: Change of glass type.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories.

Report No.: 01-40351.03

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 740/744 aluminum single hung window with nail fin.
Issued Date: 02/15/02
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Allen N. Reeves
15 FEBRUARY 2002

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meting rail) (Loads were held for 52 seconds)		
	@ 45.0 psf (positive)	0.91"*	0.29" max.
	@ 45.0 psf (negative)	0.97"*	0.29" max.
4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads held for 10 seconds)		
	@ 67.5 psf (positive)	0.14"	0.20" max.
	@ 67.5 psf (negative)	0.19"	0.20" max.
4.4.2	@ 70.8 psf (negative)	0.20"	0.20" max.

** Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:baw
01-40351.03

Allen N. Reeves, P.E.
Director - Engineering Services
15 FEBRUARY 2002

ALLEN N. REEVES CERTIFICATE 52119354 STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into the #2 2 x 8 Spruce-Pine-Fir wood buck with 1" galvanized roofing nails through the nail fin every 8" on center. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	24 lbs	30 lbs max.
2.1.2	Air Infiltration (ASTM E 283) @ 1.57 psf (25 mph)	0.10 cfm/ft^2	0.30 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-96) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.86"*	0.29" max.
	@ 15.0 psf (negative)	0.81"*	0.29" max.

*Note: * Exceeds L/175 for deflection, but meets all other test requirements.*

2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.20" max.
	@ 22.5 psf (negative)	<0.01"	0.20" max.
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction at 70 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.03"/6%	0.50"/100%
	Right stile	0.03"/6%	0.50"/100%

Allen N. Reeves
15 FEBRUARY 2002

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.330" high by 0.187" backed polypile with center fin	1 Row	Fixed meeting rail interlock
0.170" high by 0.187" backed polypile with center fin	1 Row	Fixed lite, stiles and top rail
3/8" diameter hollow bulb gasket	1 Row	Bottom rail
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash stiles
0.150" high by 0.187" wide polypile	1 Row	Active sash stiles

Frame Construction: All frame members were constructed of extruded aluminum with coped, butted and sealed corners fastened with two screws each. Fixed meeting rail was secured utilizing one screw in each end directly through exterior face into jamb. Silicone was utilized around exterior meeting rail/jamb joinery.

Sash Construction: All sash members were constructed of extruded aluminum with coped and butted corners fastened with one screw each.

Screen Construction: The screen frame was constructed from roll-formed aluminum members with plastic keyed corners. The screening consisted of a fiberglass mesh and was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Plastic tilt latch	2	One each end of the interior Meeting rail
Metal sweep lock	2	13" from meeting rail ends
Balance assembly	2	One per jamb
Screen tension spring	2	One per end of screen stile
Tilt pin	2	One each end of bottom rail

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
NO. 19356
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-40351.03
Test Dates: 10/22/01
And: 10/23/01
Report Date: 02/15/02
Expiration Date: 10/23/05

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness performance testing on a Series/Model 740/744, aluminum single hung window at MI Home Products, Inc.'s test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R45 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 740/744

Type: Aluminum Single Hung Window With Nail Fin

Overall Size: 4' 4-1/8" wide by 5' 11-5/8" high

Active Sash Size: 4' 2-3/4" wide by 2' 11-5/8" high

Fixed Daylight Opening Size: 4' 1-1/8" wide by 2' 9" high

Screen Size: 4' 1-7/8" wide by 2' 11-5/16" high

Finish: All aluminum was polished.

Glazing Details: The active sash and fixed lite were glazed with one sheet of 1/8" thick clear tempered glass. Each sash was channel glazed using a flexible vinyl gasket.

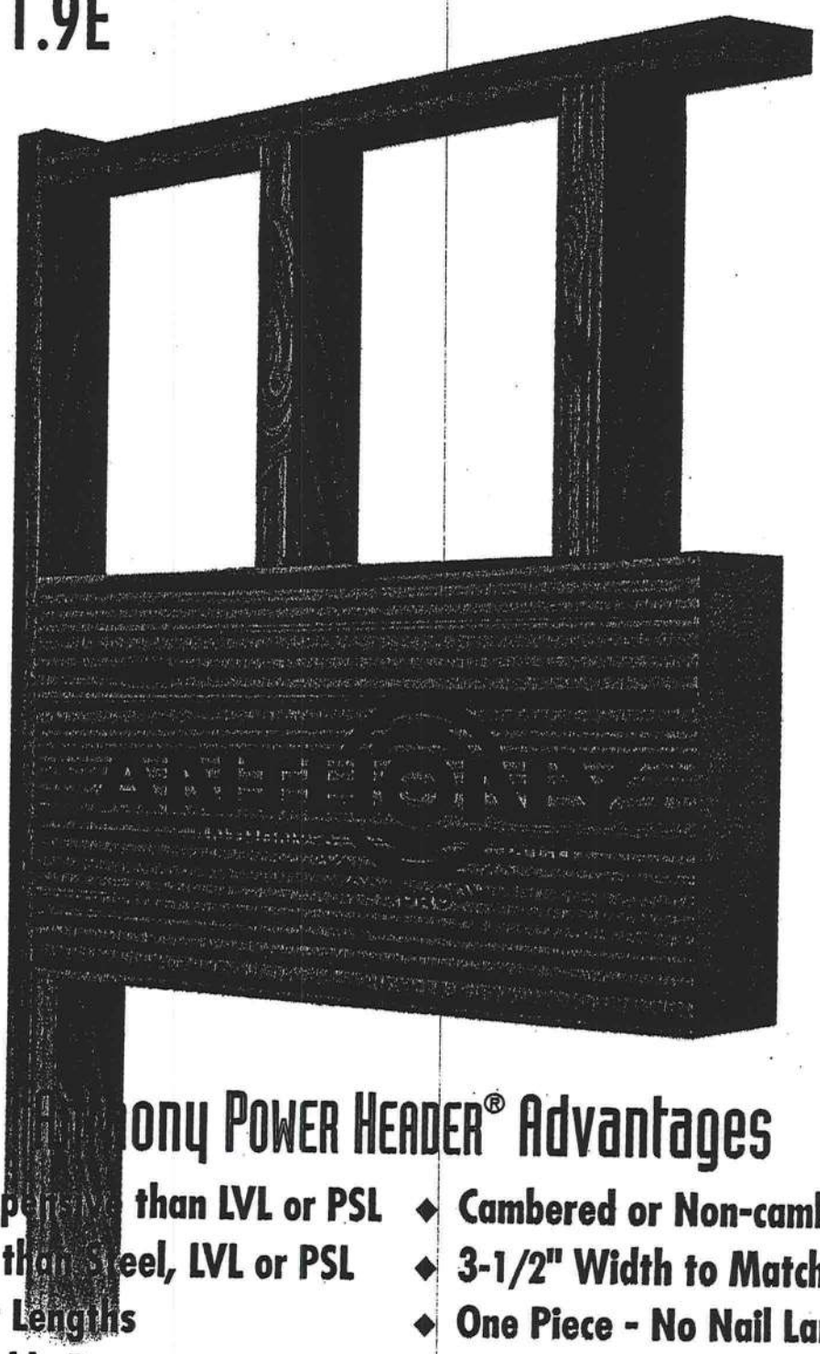
ALLEN M. REEVES
CERTIFICATE
NO. 1935
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen M. Reeves

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.testatl.com

Anthony POWER HEADER®

2600F_b - 1.9E



Anthony POWER HEADER® Advantages

- Less Expensive than LVL or PSL
- Lighter than Steel, LVL or PSL
- Pre-Cut Lengths
- Renewable Resource
- Cambered or Non-cambered
- 3-1/2" Width to Match Framing
- One Piece - No Nail Laminating
- Lifetime Warranty

Garage Header Sizing Tables

ANTHONY®
ANTHONY FOREST PRODUCTS CO.

Anthony POWER HEADER®

$26F_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER APPLICATION - SINGLE STORY

HEADER SUPPORTING: 1/2 ROOF SPAN

9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	16-3/4
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	
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8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	16-3/4	9-3/4	15-3/8		9-3/4		
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8-3/8	14	15-3/8	8-3/8	15-3/8		8-3/8	15-3/8		9-3/4			9-3/4			11-1/4		
8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4			11-1/4		

9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14
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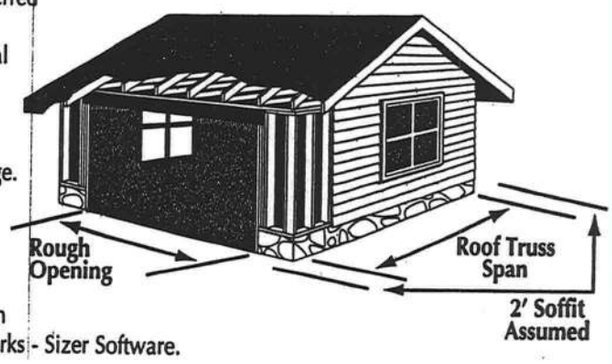
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	

NOTES:

1. Table assumes a simple span header supporting a uniform load transferred from 1/2 the roof span plus a 2' soffit.
2. Roof live and dead loads shown are applied vertically to the horizontal projection. No reductions in roof live loads or snow loads were considered. The header weight is accounted for in the table.
3. Deflection is limited to L/240 for live load and L/180 for total load.
4. Headers are assumed to have continuous lateral support along top edge.
5. Bearing length based on full width bearing is indicated as follows:
   Non-shaded sizes require two trimmers (3" bearing).
   Shaded sizes require three trimmers (4.5" bearing).
   Shaded & outlined sizes require four trimmers (6" bearing).
6. ** Applications where load carrying capacity of 16-3/4" depth has been exceeded. See AFP $30F_b$ POWER BEAM® literature or AFP's WoodWorks - Sizer Software.



Anthony Power Header®

26F$_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

844	896		1216		1573						
161	207	254	330	390	510	552	669	752	824		
114	145	180	231	277	359	391	510	534	653	707	789

844	975		1322								
161	207	254	330	390	510	552	724	752	897		
114	145	180	231	277	359	391	510	534	699	693	

562	778	888	1056	1363	1367		1582						
107	153	169	245	260	380	368	540	501	715	664	864	840	
76	107	120	171	185	267	261	380	356	521	471	684	609	813

NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

Anthony Power Header®

$26F_b$ - 1.9E

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS (F_b) =	2600
COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) =	740
HORIZONTAL SHEAR (F_v) =	225
MODULUS OF ELASTICITY (MOE) =	1.9×10^6

	7.7	9.0	10.4	11.7	12.9	14.2	15.5
	326	514	789	1115	1521	2014	2604
	8865	12015	15996	20145	24772	29877	35460
	3908	4550	5250	5892	6533	7175	7817

NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b, shall be modified by the Volume Factor, C_v, as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1997.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for Power Header®.

GARAGE HEADER COMPARISONS

	810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
	990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-5/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
	640 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-3/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"*
	765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"*
	750 / 480	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"*
	900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	-----

For more information on Power Header®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

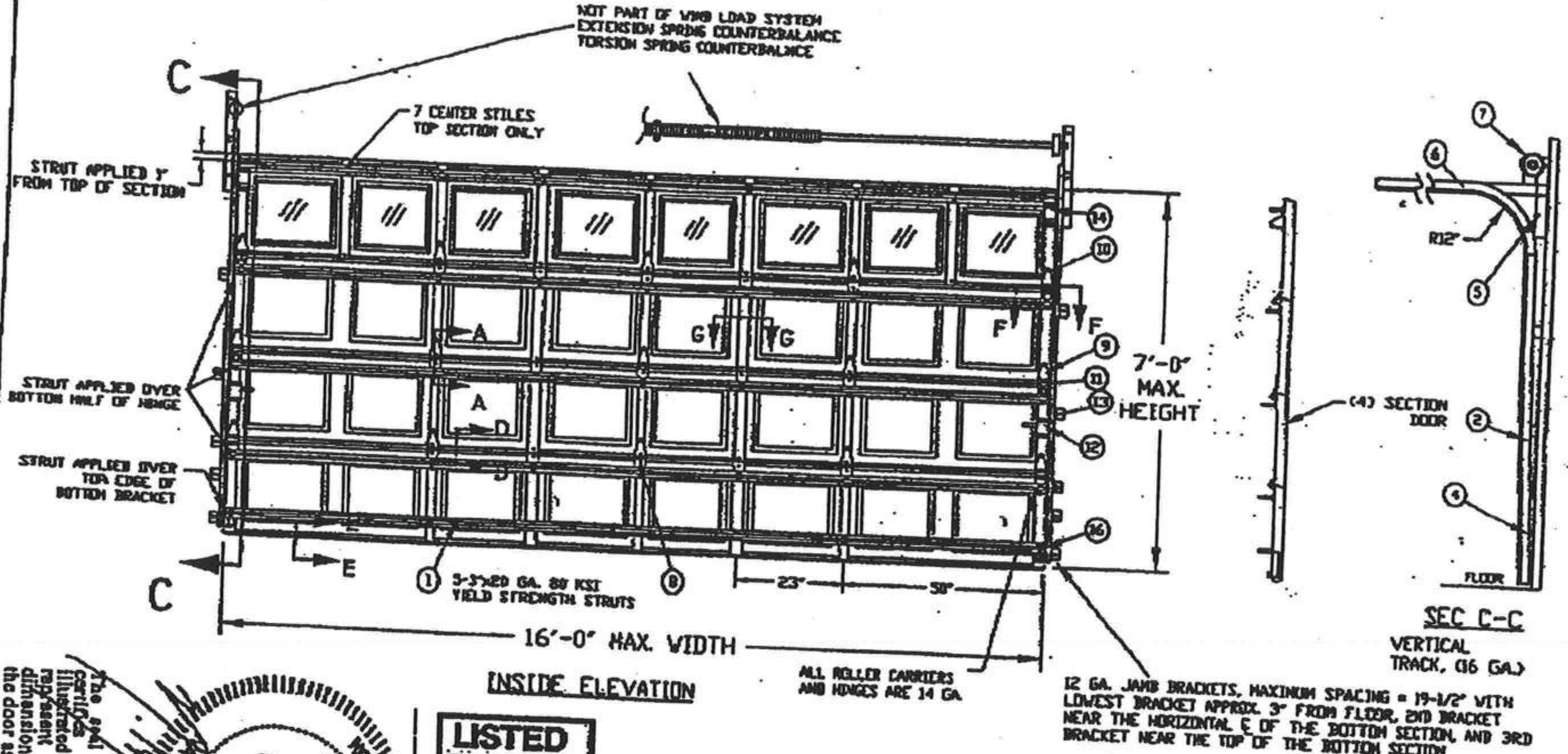
Distributed by:

Anthony Forest Products Company
Post Office Box 1877 • El Dorado, Arkansas 71731
Internet address: http:// www.anthonyforest.com
e-mail: info@anthonyforest.com

REV.	DATE	BY	DESCRIPTION
A–A	11-10-08	DV	SEE E.C.N. 831

NL

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT= 21"
3. SECTION HEIGHTS OF 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION (AS TESTED WITH 1/8" DSB GLASS OR EQUIVALENT) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MINIMUM LENGTH OF ROLLER STEM IS 5½" (7" AS TESTED)
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOWN.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE 0,1, OR 2 AS TESTED.
9. △ DROP IN TYPE OF INSULATION IS OPTIONAL.



INSIDE ELEVATION

NORTH CAROLINA PROFESSIONAL SEAL PE No. 024280 ENGINEER

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.

LISTED SBCCI PST & ESI REPORT No. 2202

TEST REPORTS ON FILE VIDEO 10/19/08 (00293)

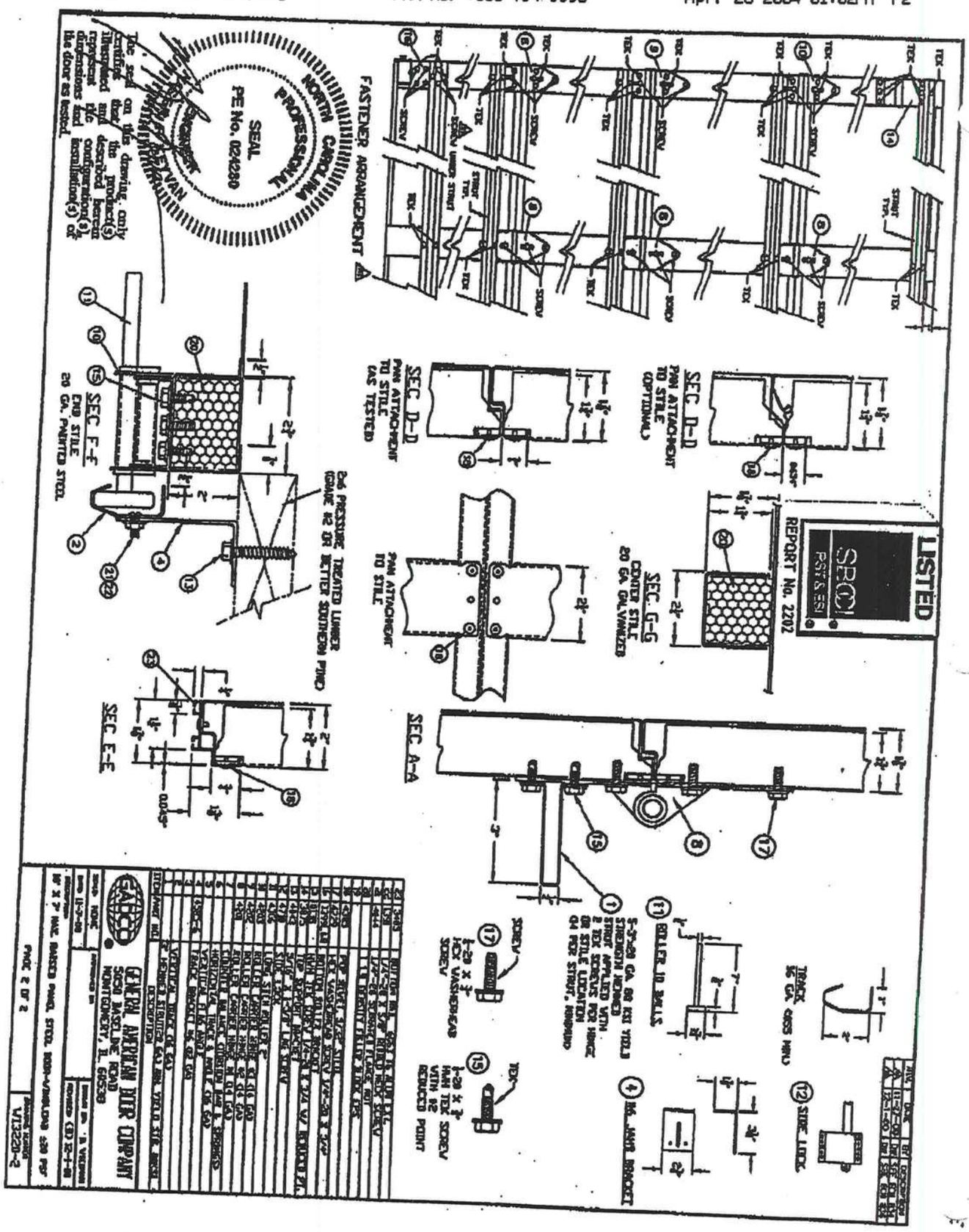
DESIGN LOAD +20.0 PSF & -20.0 PSF
TEST LOAD +30.0 PSF & -30.0 PSF

GADCO DOORS
SERIES 7400, EXTERIOR STEEL =.017 MIN (AS TESTED)
SERIES 7825, EXTERIOR STEEL =.019" MIN △
SERIES 7524, EXTERIOR STEEL =.024" MIN △
(TESTED WITH WINDOWS

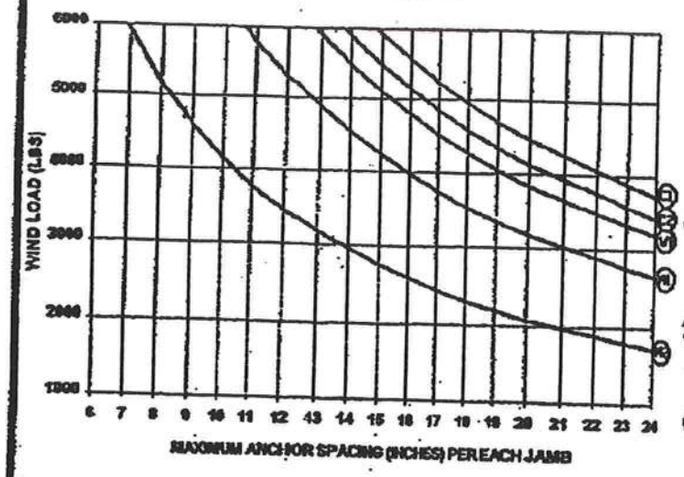
MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CTR. STILE SPACING	STRUTS 80 KSI SIZE	STRUTS 80 KSI QTY.	VERTICAL TRACK
16'	7'	23"	3"	5	2 IN.

GADCO GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: NTS | APPROVED BY: | DRAWN BY: D. VEEMAN
DATE: 10-20-08 | REVISED: (A) 11-10-08
DESCRIPTION: 16' X 7' MAX. RAISED PANEL STEEL DOOR -WINDLOAD ±20 PSF
PART NUMBER: PAGE 1 OF 2 | DRAWING NUMBER: W13220-1


FASTENER ARRANGEMENT
SEC A-A
SEC B-B
SEC C-C
SEC D-D
SEC E-E
SEC F-F
SEC G-G
LISTED
REPORT No. 2202
SEAL
PE No. 024289
NORTH CAROLINA
PROFESSIONAL
GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538
VA13220-2

WIND LOAD vs ANCHOR SPACING



(1) CONCRETE BACKUP HILTI KWIK BOLT II EXPANSION ANCHOR 3/8" DIA. 1-5/8" EMBEDMENT

(2) CONCRETE BACKUP RAWL LOK/BOLT SLEEVE ANCHOR 3/8" DIA. 1-3/8" EMBEDMENT

(3) MASONRY/CMU BACKUP RAWL LOK/BOLT SLEEVE ANCHOR 3/8" DIA. 1-3/8" EMBEDMENT

(4) MASONRY/CMU BACKUP ITW BUILDEX TAPCON MASONRY ANCHOR 1/4" DIA. 1-3/4" EMBEDMENT

(5) WOOD STUD BACKUP LAG SCREWS 5/16" DIA. 1-1/2" EMBEDMENT

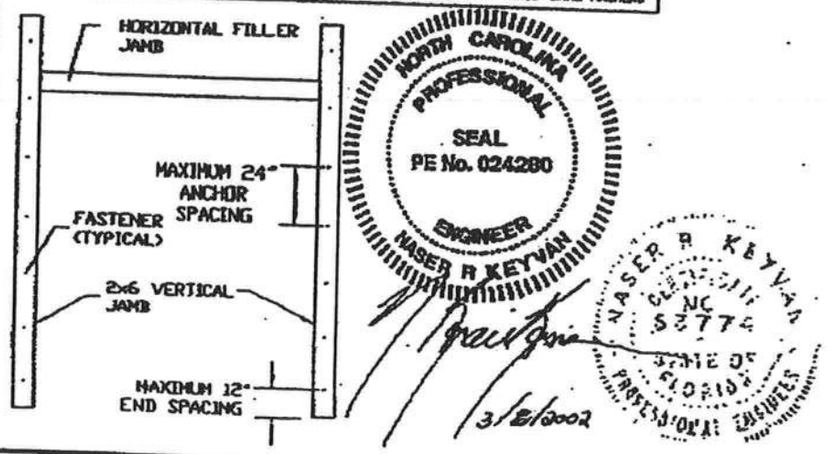
DESIGN LOAD (LBS)/FT² X GARAGE DOOR AREA(WIDTH-FT X HEIGHT-FT) = WIND LOAD(LBS)

EXAMPLE

30 LBS/FT² X 16 FT WIDE X 8 FT HIGH = 3840 LBS

(1) USE 22" SPACING
(2) USE 21" SPACING
(3) USE 19" SPACING
(4) USE 16" SPACING
(5) USE 10" SPACING

SEE NOTE 11 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS



NORTH CAROLINA PROFESSIONAL SEAL PE No. 024280 ENGINEER NASER R KEYVAN

NASER R KEYVAN NC 32774 STATE OF FLORIDA PROFESSIONAL ENGINEER

3/2/2002

2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION" SSTD 10," CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE (#2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.

5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2150 PSI; GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI; REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4"

8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.

9) WASHERS ARE REQUIRED ON ALL FASTENERS.

10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.

11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

GAPCO

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: NONE	APPROVED BY:	DRAWN BY: DJ
DATE: 9-30-99		REVISED
DESCRIPTION: JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS		
	DRAWING NUMBER: A10560	DRAWING NUMBER

ELK

PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE™

Prestique Plus *High Definition* and Prestique Gallery Collection™

Product size	13¼"x 39⅜"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	11

50-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique I *High Definition*

Product size	13¼"x 39⅜"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	14

40-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique *High Definition*

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Raised Profile

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/*FLX*™

Size: 12"x 12"
Exposure: 6½"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors: Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood**, Forest Green, Wedgewood**, Birchwood**, Sandalwood. Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge roofing products contain Elk WindGuard® sealant. WindGuard activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae. Not available in Sablewood.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles meet the latest Metro Dade building code requirements.

***See actual limited warranty for conditions and limitations.**
****Check for product availability.**

SPECIFICATIONS

SCOPE: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color).

MATERIALS: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater: apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For low

warranties are contingent upon the correct installation as shown on the instructions. These instructions are the

Residential System Sizing Calculation

Summary

EWPL INC
Lot 3 Cannon Creek
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

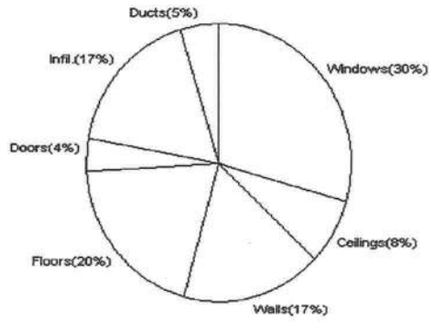
10/11/2005

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**32409**	**Btuh**	**Total cooling load calculation**	**31653**	**Btuh**
Submitted heating capacity	36000	Btuh	Submitted cooling capacity	36000	Btuh
Submitted as % of calculated	111.1	%	Submitted as % of calculated	113.7	%

WINTER CALCULATIONS

Winter Heating Load (for 1932 sqft)

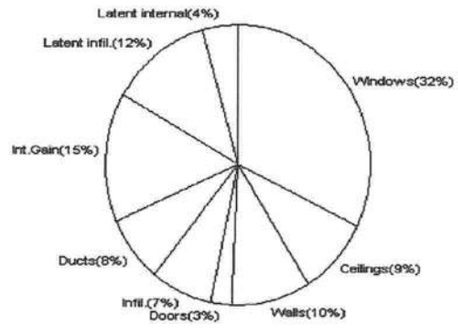
Load component			Load	
Window total	339	sqft	9594	Btuh
Wall total	1892	sqft	5517	Btuh
Door total	80	sqft	1260	Btuh
Ceiling total	1932	sqft	2512	Btuh
Floor total	204	ft	6446	Btuh
Infiltration	129	cfm	5537	Btuh
Subtotal			**30866**	**Btuh**
Duct loss			1543	Btuh
TOTAL HEAT LOSS			**32409**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1932 sqft)

Load component			Load	
Window total	339	sqft	10254	Btuh
Wall total	1892	sqft	3130	Btuh
Door total	80	sqft	798	Btuh
Ceiling total	1932	sqft	2743	Btuh
Floor total			0	Btuh
Infiltration	113	cfm	2236	Btuh
Internal gain			4800	Btuh
Subtotal(sensible)			**23961**	**Btuh**
Duct gain			2396	Btuh
Total sensible gain			**26357**	**Btuh**
Latent gain(infiltration)			3916	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**5296**	**Btuh**
TOTAL HEAT GAIN			**31653**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY:
DATE: 10-11-05

Manual J Winter Calculations

Residential Load - Component Details (continued)

EWPL INC
Lot 3 Cannon Creek
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

10/11/2005

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Winter

Residential Load - Component Details

EWPL INC
Lot 3 Cannon Creek
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

10/11/2005

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	84.0	28.3	2377 Btuh
2	2, Clear, Metal, DEF	N	12.5	28.3	354 Btuh
3	2, Clear, Metal, DEF	E	30.0	28.3	849 Btuh
4	2, Clear, Metal, DEF	S	30.0	28.3	849 Btuh
5	2, Clear, Metal, DEF	SW	21.0	28.3	594 Btuh
6	2, Clear, Metal, DEF	S	70.0	28.3	1981 Btuh
7	2, Clear, Metal, DEF	N	16.0	28.3	453 Btuh
8	2, Clear, Metal, DEF	W	21.0	28.3	594 Btuh
9	2, Clear, Metal, DEF	N	12.5	28.3	354 Btuh
10	2, Clear, Metal, DEF	S	42.0	28.3	1189 Btuh
	Window Total		339		9594 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Adjacent	13.0	232	1.6	371 Btuh
2	Frame - Exterior	13.0	1660	3.1	5146 Btuh
	Wall Total		1892		5517 Btuh
Doors	Type		Area X	HTM=	Load
1	Wood - Exter		20	17.9	359 Btuh
2	Wood - Adjac		20	9.2	184 Btuh
3	Wood - Exter		40	17.9	718 Btuh
	Door Total		80		1260Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1932	1.3	2512 Btuh
	Ceiling Total		1932		2512Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	204.0 ft(p)	31.6	6446 Btuh
	Floor Total		204		6446 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	19320(sqft)	129	5537 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			129	5537 Btuh

Totals for Heating	**Subtotal**	**30866 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1543 Btuh**
	Total Btuh Loss	**32409 Btuh**

System Sizing Calculations - Summer

Residential Load - Component Details

EWPL INC
Lot 3 Cannon Creek
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F 10/11/2005

Window	Type Panes/SHGC/U/InSh/ExSh	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, DEF, N, N	N	1.5	7.5	84.0	0.0	84.0	22	22	1848 Btuh
2	2, Clear, DEF, N, N	N	6	3	12.5	0.0	12.5	22	22	275 Btuh
3	2, Clear, DEF, N, N	E	1.5	5.5	30.0	4.5	25.5	22	72	1936 Btuh
4	2, Clear, DEF, N, N	S	1.5	7	30.0	30.0	0.0	22	37	660 Btuh
5	2, Clear, DEF, N, N	SW	8	7.5	21.0	21.0	0.0	22	62	462 Btuh
6	2, Clear, DEF, N, N	S	8	8	70.0	35.0	35.0	22	37	2065 Btuh
7	2, Clear, DEF, N, N	N	1.5	6	16.0	0.0	16.0	22	22	352 Btuh
8	2, Clear, DEF, N, N	W	1.5	7.5	21.0	1.1	19.9	22	72	1456 Btuh
9	2, Clear, DEF, N, N	N	1.5	3	12.5	0.0	12.5	22	22	275 Btuh
10	2, Clear, DEF, N, N	S	1.5	8	42.0	42.0	0.0	22	37	924 Btuh
	Window Total				339					10254 Btuh

Walls	Type	R-Value	Area	HTM	Load
1	Frame - Adjacent	13.0	232.0	1.0	241 Btuh
2	Frame - Exterior	13.0	1660.0	1.7	2888 Btuh
	Wall Total		1892.0		3130 Btuh

Doors	Type	Area	HTM	Load
1	Wood - Exter	20.0	10.0	200 Btuh
2	Wood - Adjac	20.0	10.0	200 Btuh
3	Wood - Exter	40.0	10.0	399 Btuh
	Door Total	80.0		798 Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load
1	Under Attic/Dark	30.0	1932.0	1.4	2743 Btuh
	Ceiling Total		1932.0		2743 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab-On-Grade Edge Insulation	0.0	204.0 ft(p)	0.0	0 Btuh
	Floor Total		204.0		0 Btuh

Infiltration	Type	ACH	Volume	CFM=	Load
	Natural	0.35	19320	112.9	2236 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			113	2236 Btuh

Internal gain	Occupants	Btuh/occupant			Appliance	Load
	6	X	300	+	3000	4800 Btuh

603 Canoe Creek

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
☑	☐	**Elevations including:**
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation
☑	☐	d) Location, size and height above roof of chimneys
☑	☐	e) Location and size of skylights
☑	☐	f) Building height
☑	☐	e) Number of stories

Floor Plan including:

a) Rooms labeled and dimensioned
b) Shear walls
c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
b) All posts and/or column footing including size and reinforcing
c) Any special support required by soil analysis such as piling
d) Location of any vertical steel

Roof System:

a) Truss package including:
    1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
    2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
b) Conventional Framing Layout including:
    1. Rafter size, species and spacing
    2. Attachment to wall and uplift
    3. Ridge beam sized and valley framing and support details
    4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

a) Masonry wall
    1. All materials making up wall
    2. Block size and mortar type with size and spacing of reinforcement
    3. Lintel, tie-beam sizes and reinforcement
    4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
    5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
    6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
    7. Fire resistant construction (if required)
    8. Fireproofing requirements
    9. Shoe type of termite treatment (termicide or alternative method)
    10. Slab on grade
        a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
        b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
    11. Indicate where pressure treated wood will be placed
    12. Provide insulation R value for the following:
        a. Attic space
        b. Exterior wall cavity
        c. Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
    a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
b) Floor joist size and spacing
c) Girder size and spacing
d) Attachment of joist to girder
e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
b) Ceiling fans
c) Smoke detectors
d) Service panel and sub-panel size and location(s)
e) Meter location with type of service entrance (overhead or underground)
f) Appliances and HVAC equipment
g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

a) Manual J sizing equipment or equivalent computation
b) Exhaust fans in bathroom

Energy Calculations (dimensions shall match plans)

Gas System Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

***Notice Of Commencement Required Before Any Inspections Will Be Done**

Private Potable Water

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued.
   (386) 758-1058 **(Toilet facilities shall be provided for construction workers)**

4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
   A development permit will also be required. Development permit cost is **$50.00**

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (**$25.00**) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (**$50.00**). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

NOTICE:

ADDRESSES BY APPOINTMENT ONLY!

TO OBTAIN A 9-1-1 ADDRESS THE REQUESTER MUST CONTACT THE COLUMBIA COUNTY 9-1-1 ADDRESSING DEPARTMENT AT (386) 752-8787 FOR AN APPOINTMENT TIME AND DATE:

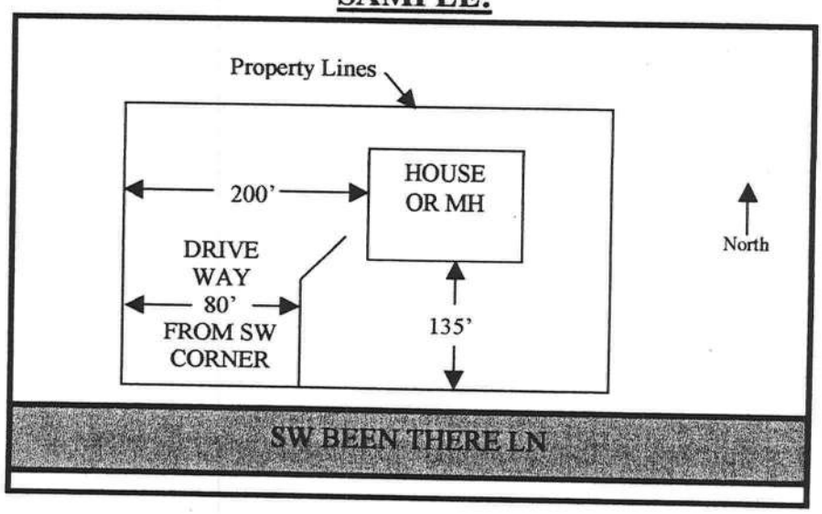
YOU CAN NOT OBTAIN A NEW ADDRESS OVER THE TELEPHONE. MUST MAKE AN APPOINTMENT!

THE ADDRESSING DEPARTMENT IS LOCATED AT 263 NW LAKE CITY AVENUE (OFF OF WEST U.S. HIGHWAY 90 WEST OF INTERSTATE 75 AT THE COLUMBIA COUNTY EMERGENCY OPERATIONS CENTER).

THE REQUESTER WILL NEED THE FOLLOWING:

1. THE PARCEL OR TAX ID NUMBER (SAMPLE: "25-4S-17-12345-123" OR "R12345-123) FOR THE PROPERTY.
2. A PLAT, PLAN, SITE PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
   a. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
   b. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
   c. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



NOTE: 5 TO 7 WORKING DAYS MAY BE REQUIRED IF ADDRESSING DEPARTMENT NEEDS TO CONDUCT AN ON SITE SURVEY.

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 24-4S-16-03114-103 **Building permit No.** 000023939

Use Classification SFD,UTILITY **Fire:** 11.84

Permit Holder HUGO ESCALANTE **Waste:** 24.50

Owner of Building HBM CONSTRUCTION **Total:** 36.34

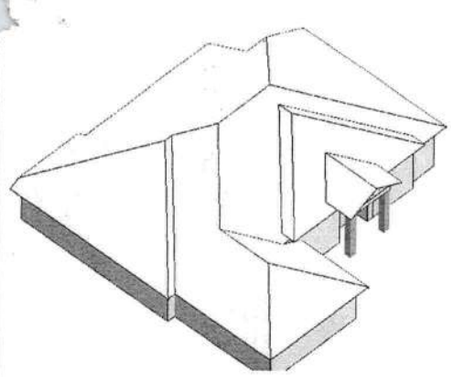
Location: 177 SW GERALD CONNER DRIVE

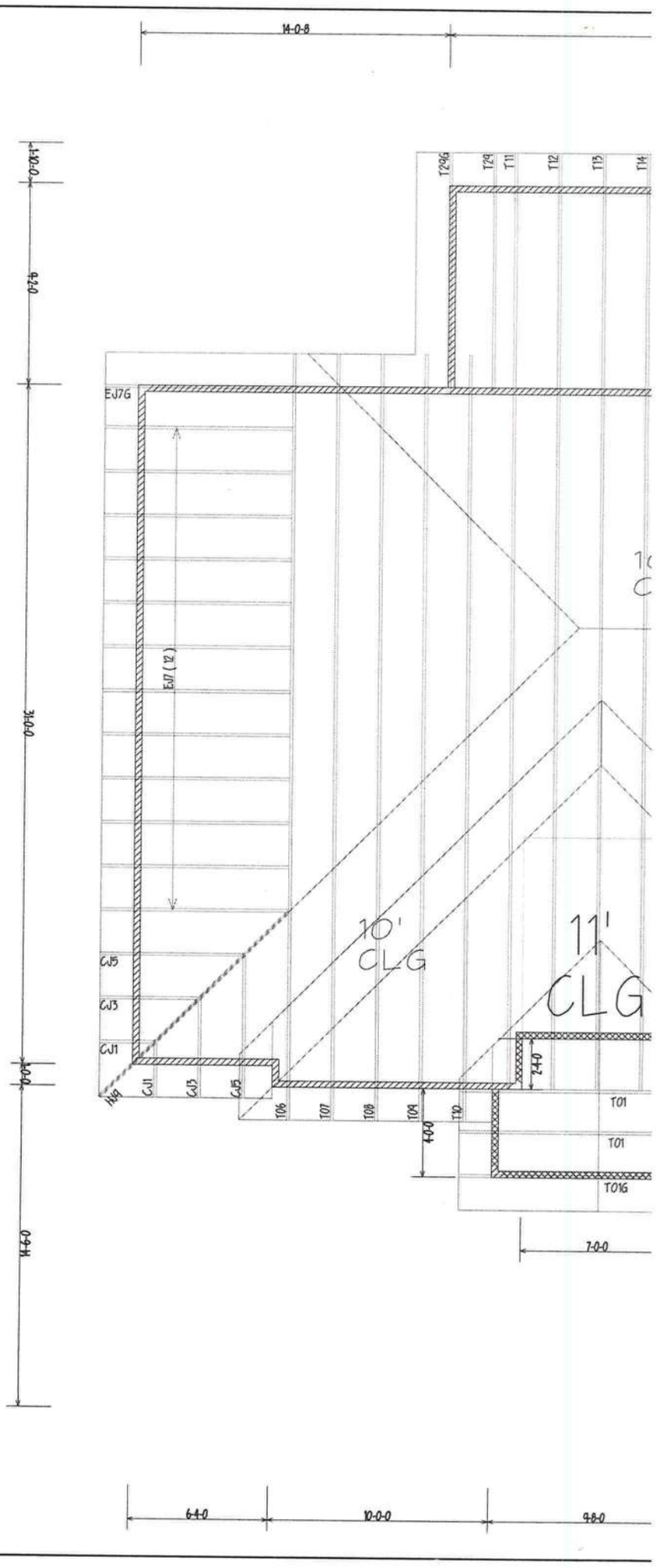
Date: 08/29/2006

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)




14-0-8
0-0-4
9-2-0
34-0-0
0-0-4
14-6-0
EJ7G
EJ7 (12)
CJ5
CJ3
CJ1
INS
CJ1
CJ3
CJ5
T29G
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10'
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11'
CLG
2-4-0
4-0-0
T01
T01
T01G
7-0-0
6-4-0
10-0-0
9-8-0