LIVE LOADS:

WOOD ROOF

TRUSSES:

CAST IN PLACE CONCRETE

- 1. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 2500 PSI, A SLUMP OF 3" FOR FOOTINGS/FOUNDATIONS AND 4" FOR SLABS
- 2. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A-615 GRADE 40.
- 3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185. WWF SHALL BE LAPPED AT LEAST 6" AND CONTAIN AT LEAST ONE CROSS WIRE WITHIN THE 6".
- 4. HOOKS SHALL BE PROVIDED AT DISCONTINUOUS ENDS OF ALL TOP BARS
- OF BEAMS. 5. HORIZONTAL FOOTING BARS SHALL HAVE 1'-0" HOOK LENGTH OR
- CORNER BARS WITH A 2'-1" LAP PROVIDED 6. MINIMUM LAP SPLICES ON ALL REINFORCING BAR SPLICES SHALL BE 40 BAR DIAMETERS TYP.
- 7. CONCRETE COVER MIN. 3" WHEN EXPOSED TO EARTH OR 1 1/2" TO

REINFORCING STEEL

ALL REINFORCING STEEL SHALL BE NEW DEFORMED BARS FREE FROM RUST, SCALE & OIL & SHALL MEET ASTM A-615 REINFORCING FOR FOOTING SHALL BE SUPPORTED ON PRE-CAST CONCRETE PADS, TOP REINFORCING SHALL BE POSITIVELY SUPPORTED BY TEMPORARY STRINGERS. DOWELS FOR COLUMNS & FILLED CELLS SHALL BE SECURED IN PLACE BY USING ADDITIONAL CROSS-REINFORCING TIED TO FOOTING REINFORCING. SPLICES IN REINFORCING WHERE PERMITTED SHALL BE THE FOLLOWING MINIMUM, UNLESS OTHERWISE INDICATED ON THE DRAWINGS:

FTGS, WALLS, COLUMNS, BEAMS, SLABS: 36 DIA. OR 2'-0" MIN. FILLED CELL REINFORCING: 40 DIA. OR 2'-1" MIN. 20 DIA. OR 1'-0" MIN. TEMPERATURE REINFORCING: WELDED WIRE MESH: 8" LAP

MASONRY WALL CONST.

- 1. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2. CONFORMING TO ASTM C90. WITH A MINIMUM NET COMPRESSIVE STRENGTH OF 1900 PSI (f'm = 1500 PSI)
- 2. MORTAR SHALL BE TYPE "S", CONFORMING TO ASTM C270. 3. COARSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH
- AT 28 DAYS OF 3000 PSI SLUMP 8" TO 11". 4. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS WITH THE CELLS FILLED WITH COARSE GROUT
- 5. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 192 BAR DIAMETERS. REINFORCEMENT SHALL BE PLACED IN THE CENTER OF THE MASONRY CELL TYPICAL UNLESS OTHERWISE NOTED
- 6. REINFORCING STEEL SHALL BE LAPPED A MINIMUM OF 40 BAR

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DIAMETERS, UNLESS OTHERWISE NOTED ON THE DRAWINGS. 7. GROUT STOPS SHALL BE PROVIDED BELOW BOND BEAM. PLASTIC SCREEN, METAL LATH STRIP OR CAVITY CAPS MAY BE USED TO PREVENT THE FLOW GROUT INTO CELLS BELOW. THE USE OF FELT PAPER AS A STOP IS PROHIBITED.

WOOD CONSTRUCTION

ANCHORS.

- 1. WOOD CONSTRUCTION SHALL CONFORM TO THE NDS "NATIONAL DESIGN
- SPECIFICATION FOR WOOD CONSTRUCTION", 2018 EDITION. 2. ALL EXTERIOR WOOD STUD WALLS, BEARING WALLS, SHEAR WALLS AND MISC. STRUCTURAL WOOD FRAMING MEMBERS, (I.E. BLOCKING OR GABLE END BRACING) SHALL BE EITHER SOUTHERN PINE, OR S.P.F. NUMBER
- 3. ANY WOOD FRAME INTERIOR BEARING WALL STUDS THAT HAVE HOLES IN THE CENTER OF THE STUD UP TO 1" DIA. SHALL HAVE STUD PROTECTION SHIELDS FOR ALL HOLES OVER 1" IN DIA. FOR PLUMBING LINES, ETC. SHALL BE REPAIRED WITH SIMPSON HSS2 STUD SHOES, TYP., U.N.O.
- 4. FASTENERS FOR PRESSURE PRESERVATIVE AND FIRE-RETARDANT-TREATED WOOD SHALL BE OF HOT-DIPPED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE OR COPPER.

PREFABRICATED WOOD TRUSSES

- 1. ALL PREFABRICATED WOOD TRUSSES SHALL BE SECURELY FASTENED TO THEIR SUPPORTING WALLS OR BEAMS WITH HURRICANE CLIPS OR
- 2. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENERS" AS RECOMMENDED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.
- TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPORTIONED (WITH A MAXIMUM ALLOWABLE STRESS INCREASE FOR LOAD DURATION OF 25%) TO WITHSTAND THE LIVE LOADS GIVEN IN THE NOTES AND TOTAL
- DEAD LOAD. 4. BRIDGING FOR PRE-ENGINEERED TRUSSES SHALL BE AS REQUIRED BY THE TRUSS MANUFACTURER UNLESS NOTED ON THE PLANS.
- TRUSS ELEVATIONS AND SECTIONS ARE FOR GENERAL CONFIGURATION OF TRUSSES ONLY. WEB MEMBERS ARE NOT SHOWN, BUT SHALL BE DESIGNED BY THE TRUSS MANUFACTURER IN ACCORDANCE WITH THE FOLLOWING DESIGN LOADS:
- 6. DESIGN SPECIFICATIONS FOR LIGHT WEIGHT METAL PLATE CONNECTED WOOD TRUSSES PER THE TRUSS PLATE INSTITUTE TPI LATEST EDITION.
- 7. PRE-ENGINEERED WOOD TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH SPECIFIED LOADS AND GOVERNING CODES . SUBMITTALS SHALL INCLUDE TRUSS FRAMING PLANS AND DETAILS SHOWING MEMBER SIZES, BRACING, ANCHORAGE. CONNECTIONS, TRUSS LOCATIONS, AND PERMANENT BRACING AND/OR BRIDGING AS REQUIRED FOR ERECTION AND FOR THE PERMANENT STRUCTURE. EACH SUBMITTAL SHALL BE SIGNED AND SEALED BY A FLORIDA REGISTERED STRUCTURAL ENGINEER. SUBMIT 2 COPIES FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.
- . THE TRUSS MANUFACTURER SHALL DETERMINE ALL SPANS WORKING POINTS, BEARING POINTS, AND SIMILAR CONDITIONS. TRUSS SHOP DRAWINGS SHALL SHOW ALL TRUSSES, ALL BRACING MEMBERS, AND ALL TRUSS TO TRUSS HANGERS.

SOIL BEARING VALUE:

ASSUMED ALLOWABLE SOIL BEARING PRESSURE AFTER COMPACTION: 2000 PSF SEE SOILS REPORT AND SPECIFICATIONS FOR COMPACTION REQUIREMENTS IF SOIL CONDITIONS IN THE PROJECT DO NOT MEET OR EXCEED THE CAPACITY THE GENERAL CONTRACTOR SHALL CONTACT THE ENGINEER PRIOR TO FOUNDATION POUR FOR VERIFICATION OF FOUNDATION DESIGN. SOIL TO BE COMPACTED TO AT LEAST 95% OF MAX. DRY DENSITY AS DETERMINED BY ASTM - D1557

L915R50

FIELD REPAIR NOTES

- 1. MISSED "J" BOLTS FOR WOOD BEARING WALLS MAY BE SUBSTITUTED W/ 1/2" DIA. EPOXY ANCHORS WITH 6" EMBEDMENT. SIMPSON "SET" EPOXY ADHESIVE BINDER FOLLOWING ALL MANUFACTURERS RECOMMENDATIONS. SEE PLAN FOR EMBEDMENT DEPTH AT FLOOR STEPS.
- 2. FOR MISSED VERT. DOWELS DRILL A 3/4" DIAMETER HOLE 6" DEEP AT THE LOCATION OF THE OMITTED REBAR, AND INSTALL A 32" LONG #5 BAR INTO THE EPOXY FILLED HOLE. USE A TWO PART EMBEDMENT EPOXY (SIMPSON "SET", EPOXY), MIXED PER MANUFACTURER'S INSTRUCTIONS. ASSURE THAT ALL DUST AND DEBRIS FROM DRILLING ARE REMOVED FROM THE HOLE BY BRUSHING AND AND USING COMPRESSED AIR PRIOR TO APPLYING THE EPOXY. ALLOW THE EPOXY TO CURE TO MANUFACTURER'S SPECIFICATIONS. THEN FILL THE CELL IN THE NORMAL WAY DURING BOND BEAM POUR.
- 3. FOR MORTER JOINTS LESS THAN 1/4", PROVIDE (1) #5 VERT. IN CONC. FILLED CELL EACH SIDE OF THE JOINT (BAR DOES NOT HAVE TO BE CONT. TO FOOTING)
- MISSED LINTEL STRAPS FOR MASONRY CONSTRUCTION MAY BE SUBSTITUTED WITH (1) SIMPSON MTSM16 TWIST STRAP W/ (4) 1/4" X 21/4" TITENS TO MASONRY AND (7)-10d NAILS TO TRUSS FOR UPLIFTS LESS THAN 860 LBS (USE (2) MTSM16 FOR UPLIFTS LESS THAN 1720#). NO MORE THAN 10 STRAPS MAY BE SUBSTITUTED OR NO MORE THAN 3 IN A ROW. IF GIRDER TRUSS CONNECTIONS ARE MISSED CONTACT ENGINEER OF RECORD FOR SUBSTITUTION.

TERMITE SPECIFICATIONS:

SECTION R318 PROTECTION AGAINST TERMITES

TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOIL APPLIED PESTICIDES, BAITING SYSTEMS, AND PESTICIDES APPLIED TO WOOD, OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE A PREVENTIVE TREATMENT TO NEW CONSTRUCTION (SEE SECTION 202, REGISTERED TERMITICIDE). UPON COMPLETION OF THE APPLICATION OF THE TERMITE PROTECTIVE TREATMENT A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. TREATMENT IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES."

RADON:

WHERE PROJECT IS TO BE LOCATED IN KNOWN RADON GAS PREVALENT AREAS, APPENDIX "F" OF THE 2017 FLORIDA RESIDENTIAL BUILDING CODE IS TO BE IMPLEMENTED. CONCRETE STRENGTH IN THESE AREAS ARE TO BE A MINIMUM OF 3000 P.S.I.. THEREFORE, ANY AND ALL NOTES ON THESE PLANS THAT INDICATE 2500 PSI SHALL BE REPLACED WITH 3000 P.S.I. FOR THE CONCRETE STRENGTH.

COVER SHEET FLOOR PLAN ACI 318-19 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE **ELEVATIONS** ACI 530-19 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES FLOOR FRAMING PLAN J-BOLT LAYOUT APA PLYWOOD DESIGN SPECIFICATION FOUNDATION PLAN ASCE/SEI 7-16 AMERICAN SOCIETY OF CIVIL ENGINEERS ELECTRICAL PLAN 20 PSF (REDUCIBLE) 40 PSF RESIDENTIAL FLOOR, UNLESS OTHERWISE INDICATED 60 PSF BALCONIES

40 PSF

ASTM A615-40 40,000 PSI

ASTM A615-40 40,000 PSI

MINIMUM 1.5 LBS. OF

FIBERS PER CUBIC YARD

20 PSF LIGHT PARTITIONS (DEAD LOAD), U.N.O. 10 PSF ATTIC L.L. 2500 PSI CONCRETE ALL CONCRETE UNLESS OTHERWISE INDICATED 3000 PSI STRENGTH PEA GRAVEL CONCRETE FOR MASONRY CELLS ONLY (DO NOT USE FOR CONCRETE COLUMNS OR TIE BEAMS) @ 28 DAYS ASTM A1064/A1064M WELDED WIRE FABRIC SHALL CONFORM TO **REINFORCING:**

POLYPROPYLENE FIBERS FOR SLABS ON GRADE CONCRETE ASTM C90-01, STANDARD WEIGHT UNITS, fm=1500 PSI MORTAR TYPE "S" 1800 PSI **MASONRY**

ALL REINFORCING BARS

ALL STIRRUPS AND TIES

STAIRS

CONCRETE GROUT 3000 PSI UNITS: CONTINUOUS MASONRY INSPECTION IS REQUIRED DURING CONSTRUCTION ALL STRUCTURAL AND MISCELLANEOUS STEEL A36 36,000 PSI, U.N.O STRUCTURAL SHOP AND FIELD WELDS: E70XX ELECTRODES STEEL:

WOOD FRAMING: BEAMS, RAFTERS, JOIST, PLATES, ETC. U.N.O. NO. 2 SOUTHERN YELLOW PINE (19% M.C.) ROOF DECK: PLYWOOD C-C/C-D, EXTERIOR, or OSB FLOOR SHEATHING: T&G A-C GROUP 1 APA RATED (48/24) WALL SHEATHING: PLYWOOD C-C/C-D, EXTERIOR OR OSB VERSA LAM BEAM Fb = 2900 PSI (2.0E)

ALL BOLTS CAST IN CONCRETE: ASTM A36 OR ASTM A-307

WOOD COLS. PARALLAM 2.0E U.N.O. **DESIGN LOADS:** SHINGLE ROOF: 20 PSF TOP CHORD LIVE LOAD: 10 PSF TOP CHORD DEAD LOAD: BOTTOM CHORD DEAD LOAD:

BOTTOM CHORD ATTIC LIVE LOAD: 10 PSF SEE DRAWINGS FOR SPECIAL CONCENTRATED LOADS. DESIGN FOR NEW WIND UPLIFT AS PER SPECIFIED CODES, DEDUCTING A MAXIMUM OF 5 P.S.F. DEAD LOAD, BUT NOT EXCEEDING ACTUAL DEAD LOAD.

TRUSS LAYOUT S-1 DETAILS

INDEX OF DRAWINGS

TITLE

SHT NO:

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sign

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Randolp'

STATE OF

Wiggins 2022.09.

M

WIND LOADING CRITERIA

WIND SPEED (ULTIMATE) 130 MPH WIND SPEED (ALLOWABLE) 101 MPH EXPOSURE CATEGORY BUILDING CATEGORY BUILDING TYPE ENCLOSURE CLASSIFICATION ENCLOSED INTERNAL PRESSURE COEFFICIENT +/- 0.18

NOTICE TO BUILDER

THIS DRAWING AND DESIGN IS VALID FOR 12 MONTHS AFTER THE DATE IT IS SIGNED AND SEALED OR WHILE **CURRENT CODE IS VALID**

IT IS THE INTENT OF THIS DESIGNER THAT THESE PLANS ARE ACCURATE AND ARE CLEAR ENOUGH FOR THE LICENSED PROFESS-ONAL TO CONSTRUCT THIS PROJECT. IN THE EVENT THAT SOMETHING IS UNCLEAR OR NEEDS CLARIFICATION..STOP..AND CALL THE DESIGNER LISTED IN THIS TITLE PAGE. IT IS THE RESPONSIBILITY OF THE LICENSED PROFESSIONAL THAT IS CONSTRUCTING THIS PROJECT TO FULLY REVIEW THESE DOCUMENTS BEFORE CONSTRUCTION BEGINS AND ANY AND ALL CORRECTIONS, IF NEEDED, TO BE MADE BEFORE ANY WORK IS DONE. *DO NOT SCALE DRAWINGS FOR CRITICAL DIMENSIONS

INSTEAD CALL THE DESIGNER LISTED IN TITLE PAGE*

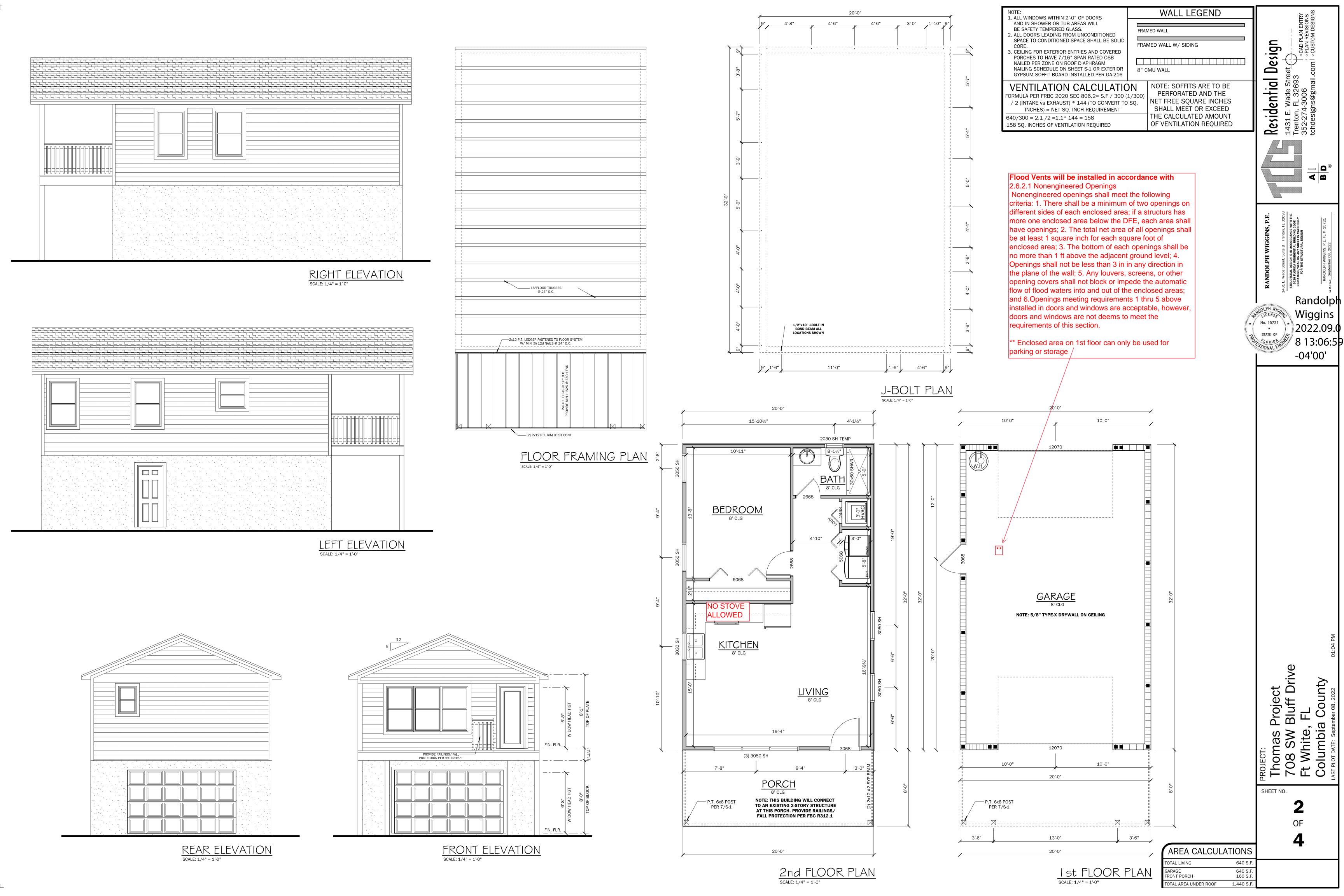
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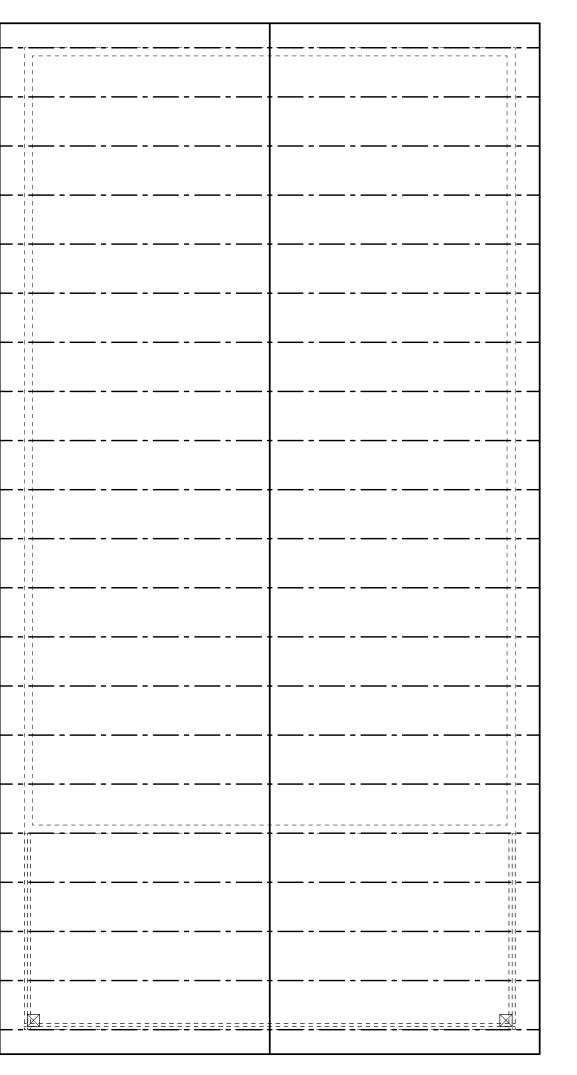
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Thomas | 708 SW | Ft White, Columbia hom:

SHEET NO.

USP AND SIMPSON CONNECTOR CROSS REFERENCE CHART





MARK	HOLD DOWN ANALYSIS	UPLIFT
	UNLESS NOTED OTHERWISE PROVIDE APPROPRIATE CONNECTOR PER CHART AND PROVIDED TRUSS ENGINEERING WOOD CONNECTIONS = H2.5 W/ 10-8d NAILS H1 W/ 10-8d NAILS H10 W/ 16-8d NAILS OR	365# 400# 850#
	MTS12 W/ 14-10dX1-1/2" NAILS HCP (HIP TRUSS) 12-10dX1-1/2" NAILS	645#
$\langle A \rangle$	2 - MTS12 W/ 14 10dX1 1/2" NAILS	1720 #U
$\langle B \rangle$	2 - HTS20 W/ 20 - 10d	2900 #U
$\langle C \rangle$	HCP2 W/ 12-10d X 1 1/2" NAILS	520 #U
$\langle D \rangle$	LGT2 W/30-16d SINKERS	1785 #U

NOTE: TYP. TRUSS TO FRAME CONNECTORS

TO BE SIMPSON H10 TYP. U.N.O.

Flood Vents will be installed in accordance with 2.6.2.1 Nonengineered Openings Nonengineered openings shall meet the following 1. There shall be a minimum of two openings on different sides of each enclosed area; if a structurs has more one enclosed area below the DFE, each area shall have openings; 2. The total net area of all openings shall be at least 1 square inch for each square foot of enclosed area; 3. The bottom of each openings shall be no more than 1 ft above the adjacent ground level; 4. Openings shall not be less than 3 in in any direction in the plane of the wall; 5. Any louvers, screens, or other opening covers

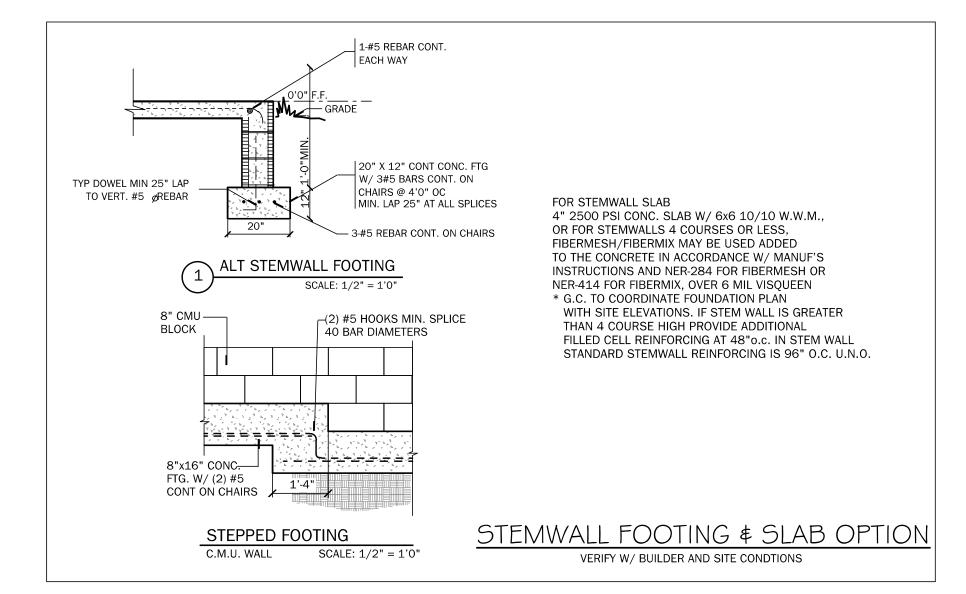
shall not block or impede the automatic flow of flood

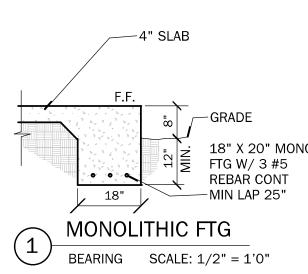
however, doors and windows are not deems to meet

waters into and out of the enclosed areas; and 6.Openings meeting requirements 1 thru 5 above

installed in doors and windows are acceptable,

the requirements of this section.





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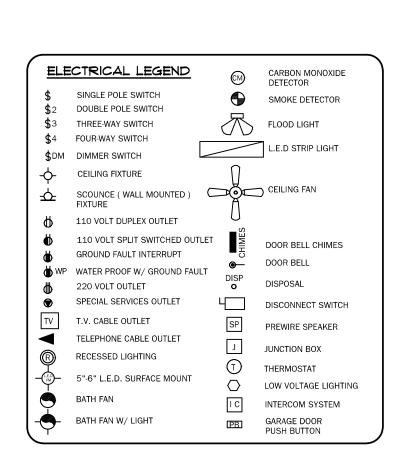
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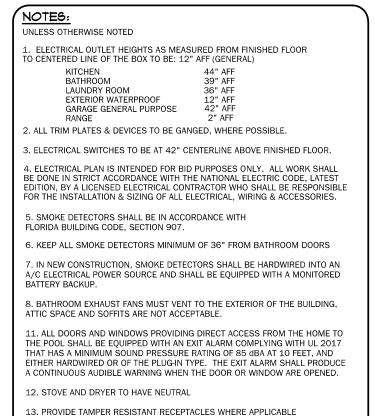
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Wiggins

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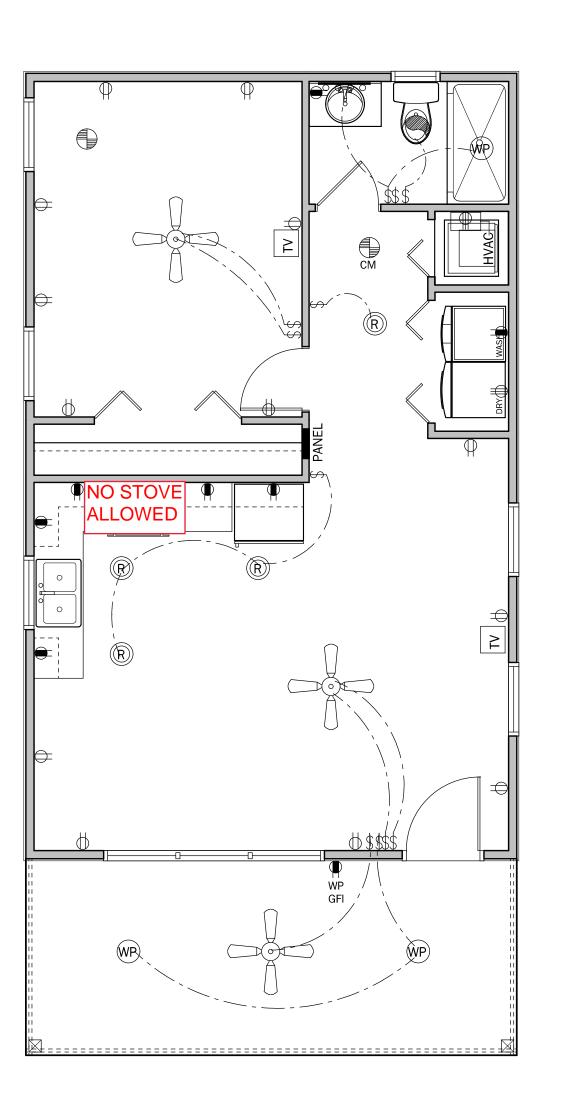
18" X 20" MONO

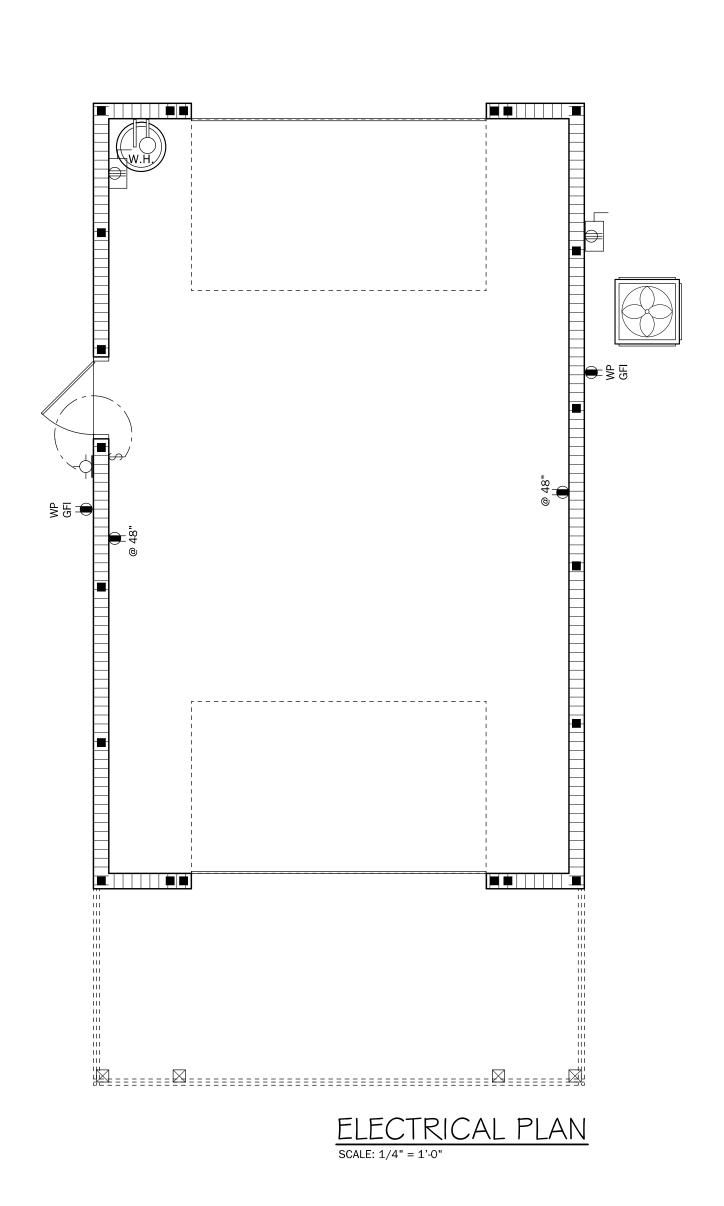


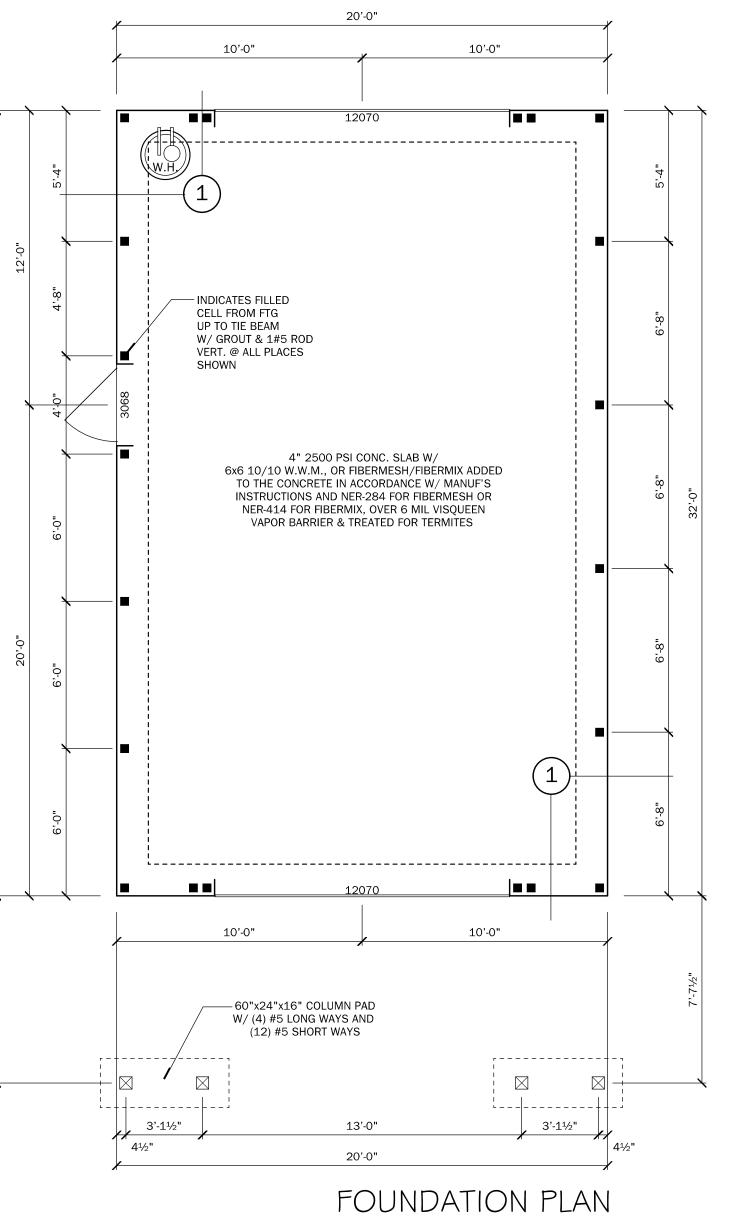


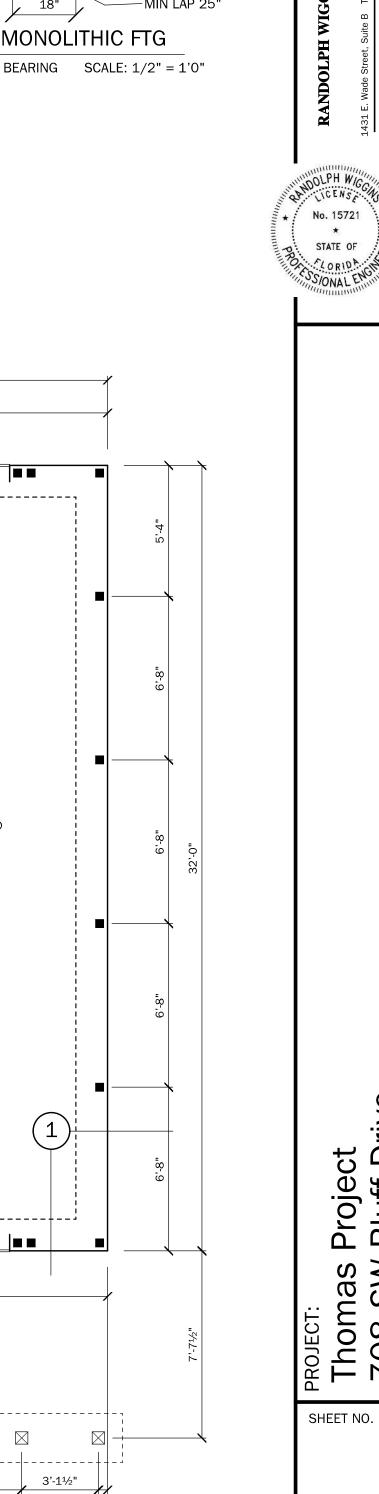
14. PROVIDE ARC-FAULT RECEPTACLES ON ALL 15 & 20 AMP NON

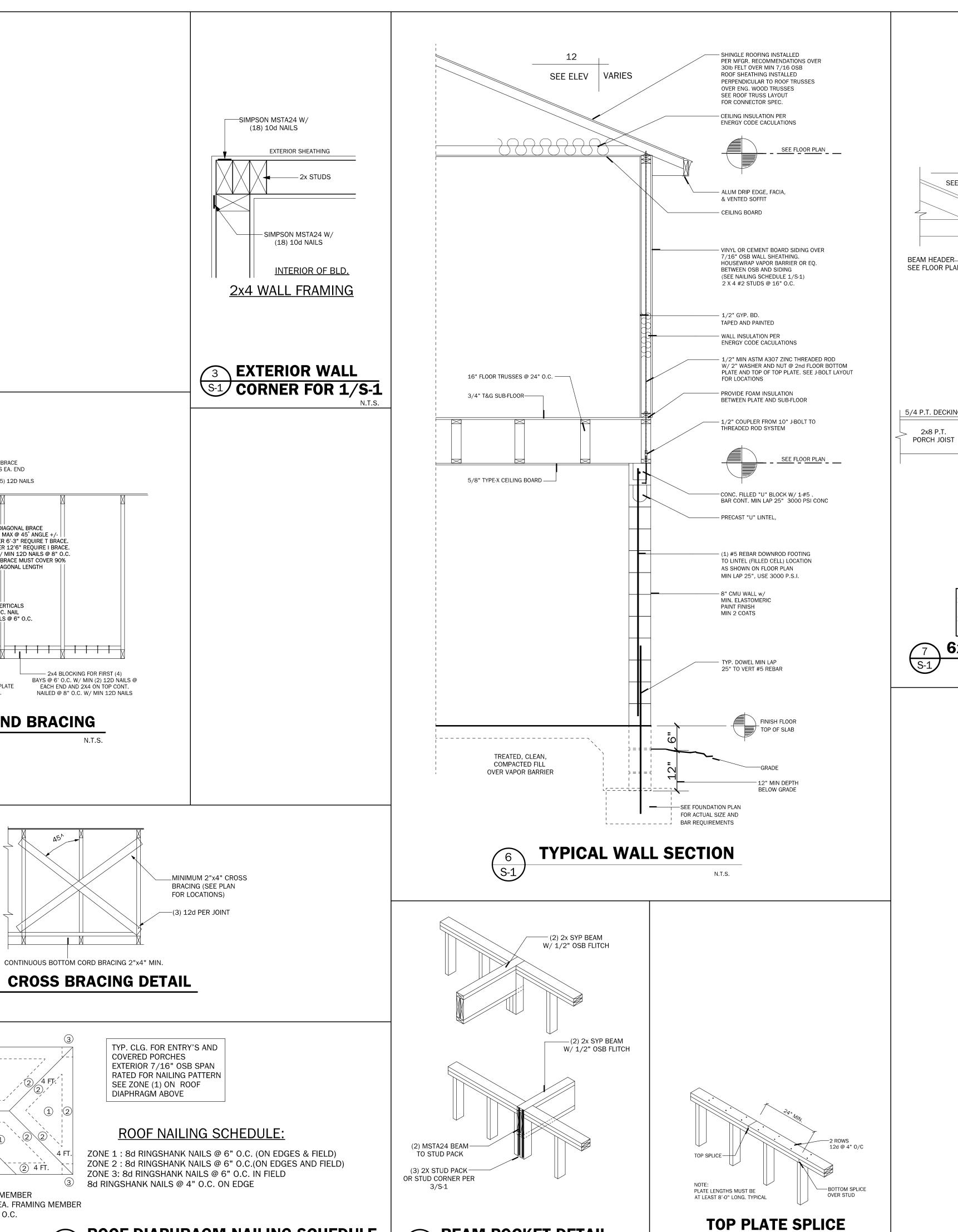
FCI PROTECTED CIRCUITS











14 BEAM POCKET DETAIL

S-1

 2x4 BLOCKING FOR BRACE PROVIDE (2) 12D NAILS EA. END

--- MIN (4) 10D NAILS

LSTA24 @ 48" O.C. — —

5 FT.

4 FT.

1. STAGGER END JOINTS @ FRAMING MEMBER

3. TYP. LOOKOUTS @ EA. JOINT & 24" O.C.

4. TYP. MIN. PLYWOOD WIDTH 12"

2. TYP. H - CLIP, EA. JOINT BETWEEN EA. FRAMING MEMBER

W/ MIN (4) 12D NAILS @ 24" O.C. NAIL

— 2X CONTINUOUS NAILED TO TOP PLATE W/ MIN 12D NAILS @ 8" O.C.

L BRACE TOGETHER W/ MIN 12D NAILS @ 6" O.C.

- MIN (5) 12D NAILS

2x6 DIAGONAL BRACE @ 48" O.C. MAX @ 45° ANGLE +/-

DIAGONALS OVER 6'-3" REQUIRE T BRACE.

DIAGONALS OVER 12'6" REQUIRE I BRACE

NAIL BRACES W/ MIN 12D NAILS @ 8" O.C.

TO DIAGONAL. BRACE MUST COVER 90%

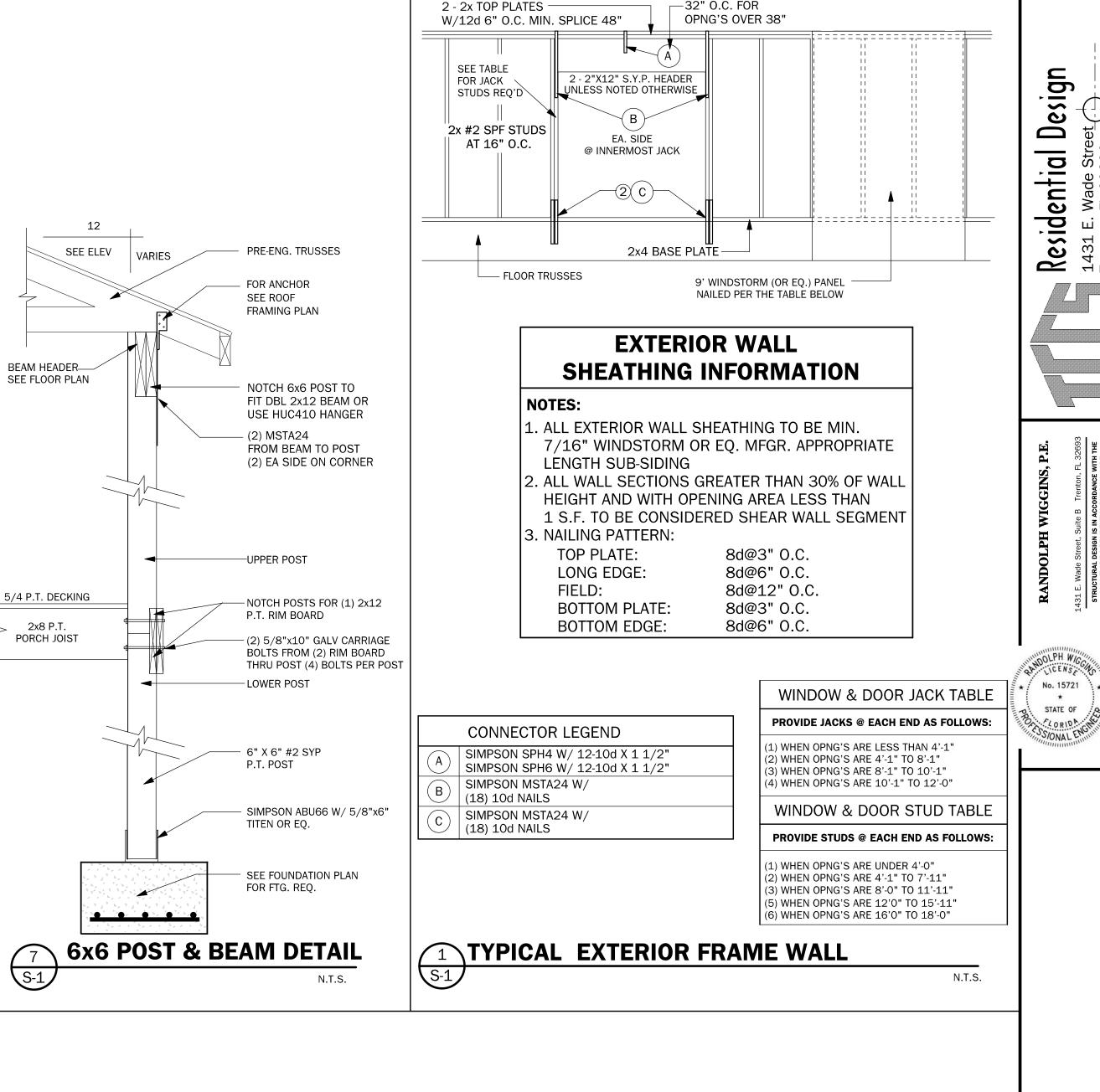
OF DIAGONAL LENGTH

GABLE END BRACING

— 2x4 BLOCKING FOR FIRST (4) BAYS @ 6' O.C. W/ MIN (2) 12D NAILS @ EACH END AND 2X4 ON TOP CONT.

NAILED @ 8" O.C. W/ MIN 12D NAILS

ROOF DIAPHRAGM NAILING SCHEDULE



sign

B

Wiggins

2022.09.D

-04'00'

Thomas P 708 SW B Ft White, F Columbia

SHEET NO.