

HARDI PANEL & BRICK. ALL OTHER WALL SHALL BE CONSIDERED BRITTLE FINISH.

STUCCO FINISH, -MIN. 7/16", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, FASTENED W/ 8D @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL ORIENTED WITH THE LONG DIMENSION PERPENDICULAR TO THE STUDS. CONTRACTOR MAY USE 3/16 STRUCTURAL 1 GRADE SHEATHING OR 15/32 OSB SHEATHING AND ORIENT THE PANELS VERTICALLY.

MASONRY HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 530-05, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI530.1-05. GROUT SHALL BE IN ACCORDANCE WITH ASTM C476 WITH A MINIMUM OF 28 DAY COMPRESSIVE STRENGTH OF 2000 psi PER ASTM C1019, GROUT SHALL HAVE A MAXIMUM COURSE AGGREGATE SIZE OF $rak{3}$ PLACED AT AN 8" TO 11" SLUMP. MORTAR SHALL CONFORM TO ASTM C270 AND TYPE N MORTAR MAY BE USED IN BRICK VENEER. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND

CONCRETE MASONRY UNITS (CMU):

CMU SHALL BE IN ACCORDANCE WITH ASTM C90-75, HOLLOW LOAD-BEARING (CMU), TYPE 1, GRADE N-1, NORMAL WEIGHT, WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 psi (f'm=1500 psi). GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT IN 5'-0" MAXIMUM LIFTS PROVIDE CLEANOUTS PER ACI 530.1-02 IN THE BOTTOM OF COURSE OF MASONRY WHEN THE WALL HEIGHT EXCEEDS 5'-0".

MASONRY STEMWALLS: ALL CONCRETE MASONRY UNITS SHALL BE COMPOSED OF ASTM C90E, E GRADE N-1 HOLLOW CONCRETE MASONRY UNITS WITH TYPE 'S' MORTAR. WALL COURSING SHALL BE RUNNING BONDS, STACK BOND SHALL NOT BE USED. GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT WITH 3000 PSI PEA ROCK CONCRETE GROUT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS. ALL EXTERIOR WALLS SHALL BE REINFORCED FULL HEIGHT WITH - #4 @ 4'-0" O.C. MAX. AND AT EACH CORNER WALL END, AND WALL INTERSECTIONS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR MASONRY ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. AT STEMWALL CONSTRUCTED OF 5 OR MORE COURSES, PROVIDE HORIZONTAL JOINT REINFORCEMENT AT 16" O.C. VERTICALLY, (EVERY OTHER COURSE), AND VERTICAL REINF. SHALL BE INCREASED AS NOTED ON 1/S1.0. UNLESS NOTED OTHERWISE. LAP JOINT REINFORCING SHALL BE A MINIMUM OF 6".

ALL CONCRETE HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 318-08, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 301. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS CONCRETE AT GARAGE AND PORCH SLABS SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI.

GENERAL NOTES:

FOOTINGS AND FOUNDATIONS SHALL BE IN ACCORDANCE WITH LOCAL BUILDING CODES. FOOTING HAVE BEEN DESIGNED WITH A SOIL BEARING (DESIGN MAXIMUM) OF 2000 PSF. A SOILS INVESTIGATION REPORT IS RECOMMENDED TO VERIFY SUITABLE SUBSURFACE CONDITIONS. IF THE FOOTING ELEVATIONS SHOWN OCCUR IN A DISTURBED OR UNSTABLE SOIL, THE ENGINEER SHALL BE NOTIFIED. SOIL SHALL BE FREE OF ORGANIC MATERIAL AND COHESIVE (CLAY) SOILS. SOIL COMPACTION AND FILL SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557

FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFORMATION, FOR GENERAL FEATURES, CONDUITS, ELECTRICAL EMBEDS, STEP HEIGHTS, ETC., SEE ARCHITECTURAL PLANS, DO NOT SCALE FOOTING DIMENSIONS AND LOCATION FROM THE FOUNDATION PLAN SHOWN ON \$1.0. DO NOT DETERMINE FOOTING LOCATION BASED ON EITHER THE ARCHITECTURAL PLAN OR FRAMING PLAN, BUT BY DIMENSIONS PROVIDED ON FOUNDATION PLAN. IF FOOTING SIZE OR LOCATION IS NOT DETERMINED ON PLAN THEN CONTACT ENGINEER OF RECORD (EOR) UNLESS OTHERWISE NOTED ON DRAWINGS, MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE 3" IN FOOTINGS AND MESH SHALL BE CENTERED IN SLAB ON GRADE. IN ALL CONTINUOUS FOOTINGS PROVIDE #3 @ 48" O.C. OR ROD CHAIRS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR CONCRETE ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS

SHALL BE INSTALLED OVER MINIMUM 6 MIL POLYETHYLENE VAPOR RETARDER WITH JOINTS LAPPED 6" AND SEALED OVER CLEAN, COMPACTED EARTH OR FILL WITH APPROVED CHEMICAL SOIL TREATMENT FOR PREVENTION OF SUBTERRANEAN TERMITES. SAWCUTS: FOR CONTROLLED CRACKING CUT A 1" SAWCUT INTO SLAB IN A 12'x12' GRID WITHIN 12 HOURS OF CONCRETE PLACEMENT, PROVIDE SAWCUTS THROUGH OUT SLAB CALL EOR FOR ALTERNATIVE METHODS.

ALL WOOD FRAMING HAS BEEN DESIGNED IN ACCORDANCE WITH NATIONAL DESIGN SPECIFICATIONS (NDS) FOR WOOD CONSTRUCTION, LATEST EDITION. ALL WOOD MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH MASONRY, CONCRETE OR SOIL SHALL BE PRESSURE-TREATED. IF, ACQ OR NON-DOT BORATE PRESERVATIVE TREATMENT IS USED, ALL ATTACHED FASTENERS SHALL BE STAINLESS STEEL.

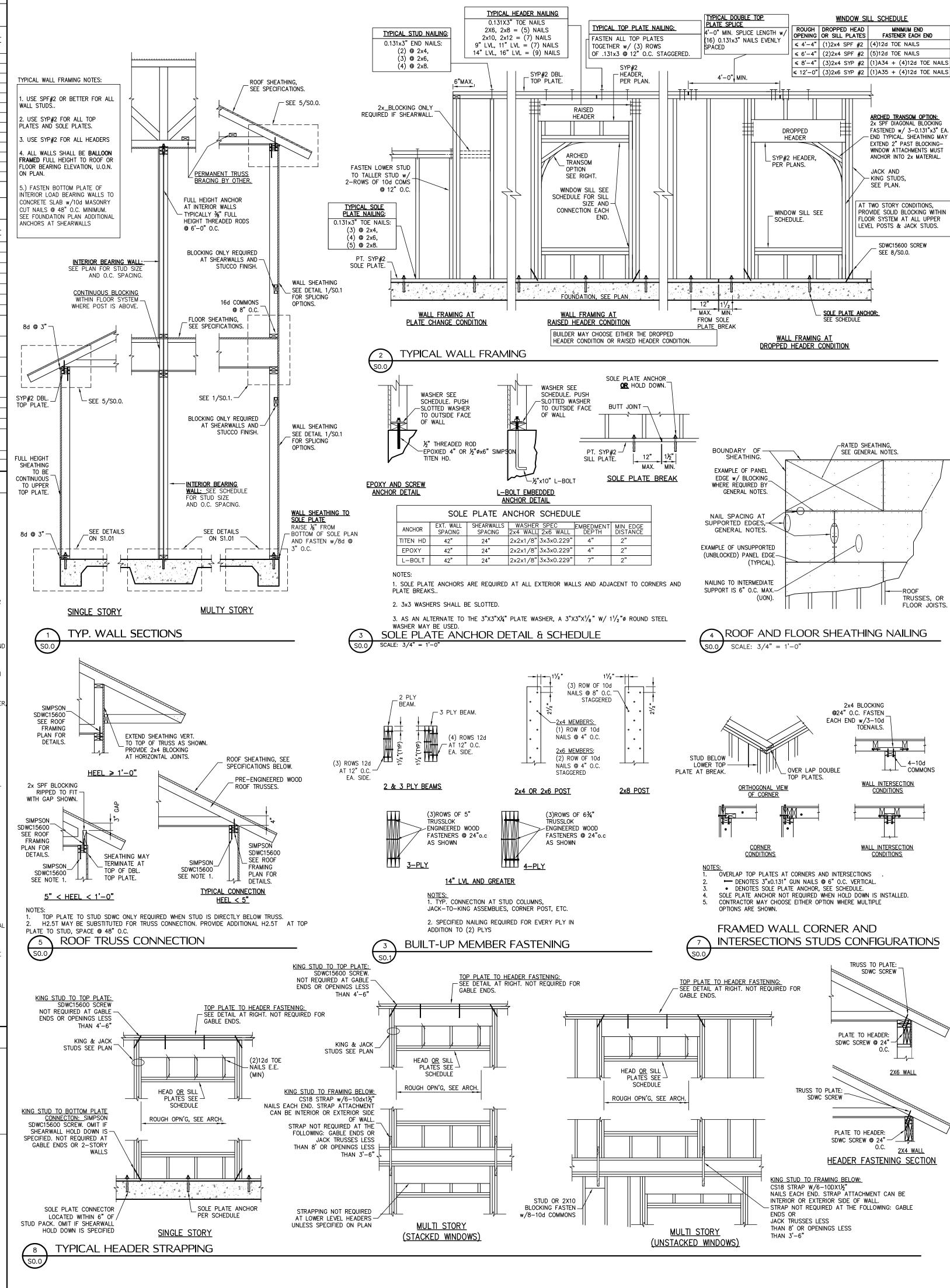
SHALL BEAR THE SEAL OF AN ENGINEER IN THE STATE WHERE PROJECT IS BEING BUILT AND SHALL COMPLY WITH NFPA, TPI, AND AITC 100. CONTRACTOR SHALL VERIFY THAT ADEQUATE TRUSS BEARING IS INSTALLED AT ALL TRUSSES AS INDICATED IN THE TRUSS SHOP

DRAWINGS. ALL TRUSS-TO-TRUSS CONNECTIONS AND TRUSS PROFILES ARE THE RESPONSIBILITY OF THE DELEGATED TRUSS SHALL HAVE TEMPORARY BRACING PER 'COMMENTARY' AND RECOMMENDATION FOR HANDLING, INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES, HIB-91." AT MULTIPLE STRAP CONNECTIONS, SPREAD STRAPS TO AVOID NAILING CONFLICTS THROUGH TRUSS. WHEN USING (2) STRAPS ON SINGLE PLY TRUSSES, PLACE STRAPS DIAGONALLY ACROSS DBL. TOP PLATE FROM EA. OTHER.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF THE ROOF COVERING SYSTEM. ASPHALT SHINGS SHALL COMPLY WITH ASTM D3161 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. CLAY AND TILE ROOFS SHALL BE INSTALLED PER THE "CONCRETE AND CLAY ROOF TILE INSTALLATION MANUAL." AND THE MANUFACTURER'S REQUIREMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL METAL FLASHING AND VALLEY MATERIALS

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN/INSTALLATION OF ALL WATER PROOFING.

WOOD FASTENING SCHEDULE			BRICK NOTES / LINTEL SCHD			PLAN LEGEND AND ABBREVIATIONS	
MEMBERS	CONNECTION TYPE	FASTENER	LINTEL DIMENSION	MIN. BRG.	MAX. SPAN	INTERIOR LOAD BEARING WALL	BUILT-UP POST IN THE WALL
TOP PLATE TO TOP PLATE	FACE NAIL	2-GUN NAILS @ 12" STAG.	L3½x3½x1/4	4"	6'-0"	GABLE X-BRACE, SEE DETAIL 10/S0.1	_
TOP PLATE, LAPS/INTERSECTION	FACE NAIL	(2-16d) 3-GUN NAILS	L4x3 ¹ / ₂ "x ¹ / ₄	6"	8'-0"		(2)2×8-1/2 HEADER SIZE, JACK AND KING STUD QUANTITY.
DBL. TOP PLATE TO STUD	FACE NAIL	(2-16d) 3-GUN NAILS	L5x3 ¹ / ₂ "x ¹ / ₄	6"	10'-0"	DESIGNATES SHEARWALL. THE HIDDEN LINE DESIGNATES SIDE OF WALL THE	KING STUD QUANTITY.
RIM JOIST TO TOP PLATE	TOE NAIL	(8d @ 6") GUN NAIL @ 6"	L6x3 ¹ / ₂ "x ¹ / ₄	6"	12'-0"	SHEARWALL SHEATHING TO BE APPLIED. SHEARWALL SHEATHING TO BE APPLIED. SHEARWALL SHEATHING TO BE APPLIED.	
CEILING JOIST TO TOP PLATE	TOE NAIL	(3-8d) 5-GUN NAILS	L7×3 ¹ / ₂ "x ¹ / ₄	6"	16'-0"	SW 3"/6" 80 @ 3/6 DESIGNATES 80 COMMONS @ 3" O.C. EDGE & 6" O.C. "IN THE FIELD"	
CEILING JOIST, OVER PARTITIONS	FACE NAIL	(3-16d) 4-GUN NAILS	1. STEEL LINTELS TO BE MINIMAL 36" LINTE		10 0	1	
CEILING JOIST TO ROOF RAFTER	FACE NAIL	(6-16d) 8-GUN NAILS	MUST HAVE CORROSION RESISTANT COATING		BRICK	ADJ — ADJACENT	LG - Long
JOIST/TRUSS TO PLATE	TOE NAIL	(2-16d) 3-GUN NAILS	OF EPOXY BASED PAINT.		VENEER	BM — BEAM	MANUF — Manufacture
RAFTER TO PLATE	TOE NAIL	(3-8d) 3-GUN NAILS	2. LINTEL MORE THAN 8'-0". SHOULD BE		WEATHER BARRIER	BOT — BOTTOM BRG — BEARING	MONO — Monolithic OC — On Center
JACK RAFTER TO HIP	TOE NAIL	(3-10d) 4-GUN NAILS	LATERALLY SUPPORTED NOT TO EXCEED 6 FT. O.C. w/ $2-\frac{1}{4} \times 3$ WD. SCREWS INTO			CMU — CONCRETE MASONRY UNIT DBL — DOUBLE	OSB — Oriented Strand Board PERP — Perpendicular
ROOF RAFTER TO 2x_ RIDGE BM.	TOE NAIL	(2-16d) 3-GUN NAILS	HEADER PROVIDE A 1/2" VERTICAL SLOTTED	\ <i>\</i>	LINTEL	DIA — DIAMETER	PRE ENG - Pre Engineered
CONT. HEADER, TWO PIECES	FACE NAIL	16d@ 16" O.C. @ EDGE	HOLE FOR SCREW.		ATTACHMENT	EA — EACH EE — EACH END	PSF — Pounds per Square Foot PSI — Pounds per Square Inch
CONT. HEADER TO STUD	TOE NAIL	(3-16d) 4-GUN NAILS	3. BRICK VENEER ATTACHMENT: HORIZONTA	∟ /\[SEE NOTE 2	EOR — ENGINEER OF RECORD	PT — PRESSURE TREATED
STUD TO SOLE PLATE	TOE NAIL	(3-16d) 4-GUN NAILS	TIES @ 24" O.C., VERT. TIES @ 12" O.C (FOR 110mph WIND-ZONE VERT. TIES @ 16		 	EQ — EQUAL EXT — EXTERIOR	QT — Quick Tie REINF — Reinforce
SOLE PLATE TO JOIST/BLOCKING	FACE NAIL	(16d @ 16") GUN NAIL @ 8"	O.C.). AT ALL OPENINGS SPACE TIES WITHIN	HEADER,_		FBC — FLORIDA BUILDING CODE FDN — FOUNDATION	SF — Square Foot SPF —Spruce Pine Fur
$3"x0.131" \emptyset = GUN NAILS$ $2"x0.113" \emptyset = 6d$ $3"x0.148" \emptyset = 10d$	$2"x0.113" \emptyset = 6d$ $2\frac{1}{2}"x0.131" \emptyset = 8d$		12" OF OPENINGS. PROVIDE $^3/_{16}$ "Ø WEEP HOLES @ 33" O.C. IMMEDIATELY ABOVE FLASHING.		BRICK LINTEL, SEE SCHEDULE SECTION VIEW BRICK LINTEL	FTN — FOUNDATION FT — FOOT FTG — FOOTING HDR — HEADER HORIZ — HORIZONTAL LBS — POUNDS	SYP — Southern Yellow Pine THRU — Through TYP — Typical UON — Unless Otherwise Noted VERT — Vertical WWF — Welded Wire Fabric





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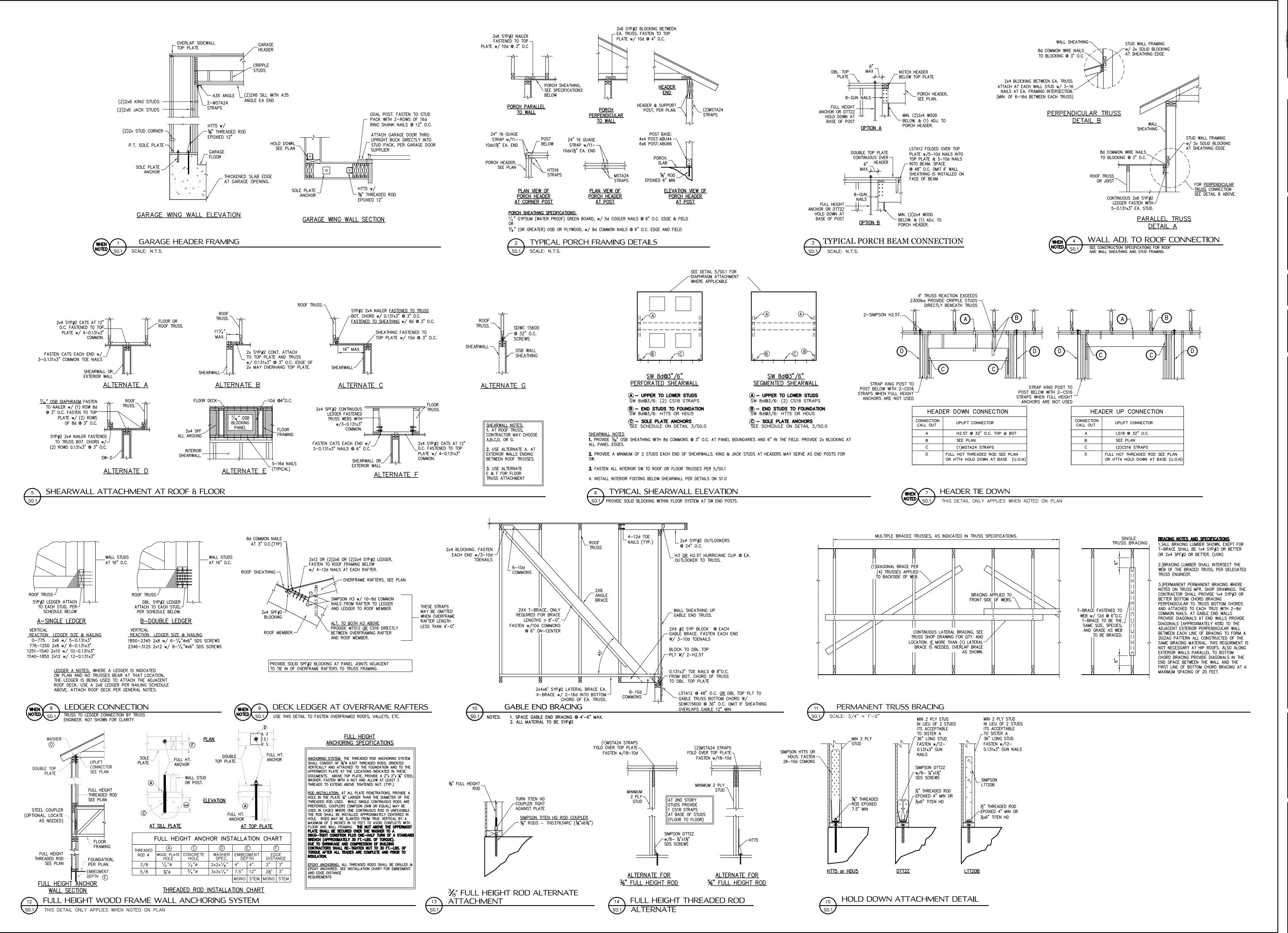
ARCHITECTURAL DRAWINGS OF **DESIGN** CRITERIA

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UNCLEAR REFER TO THE

GENERAL NOTES

SHEET 1 OF 7



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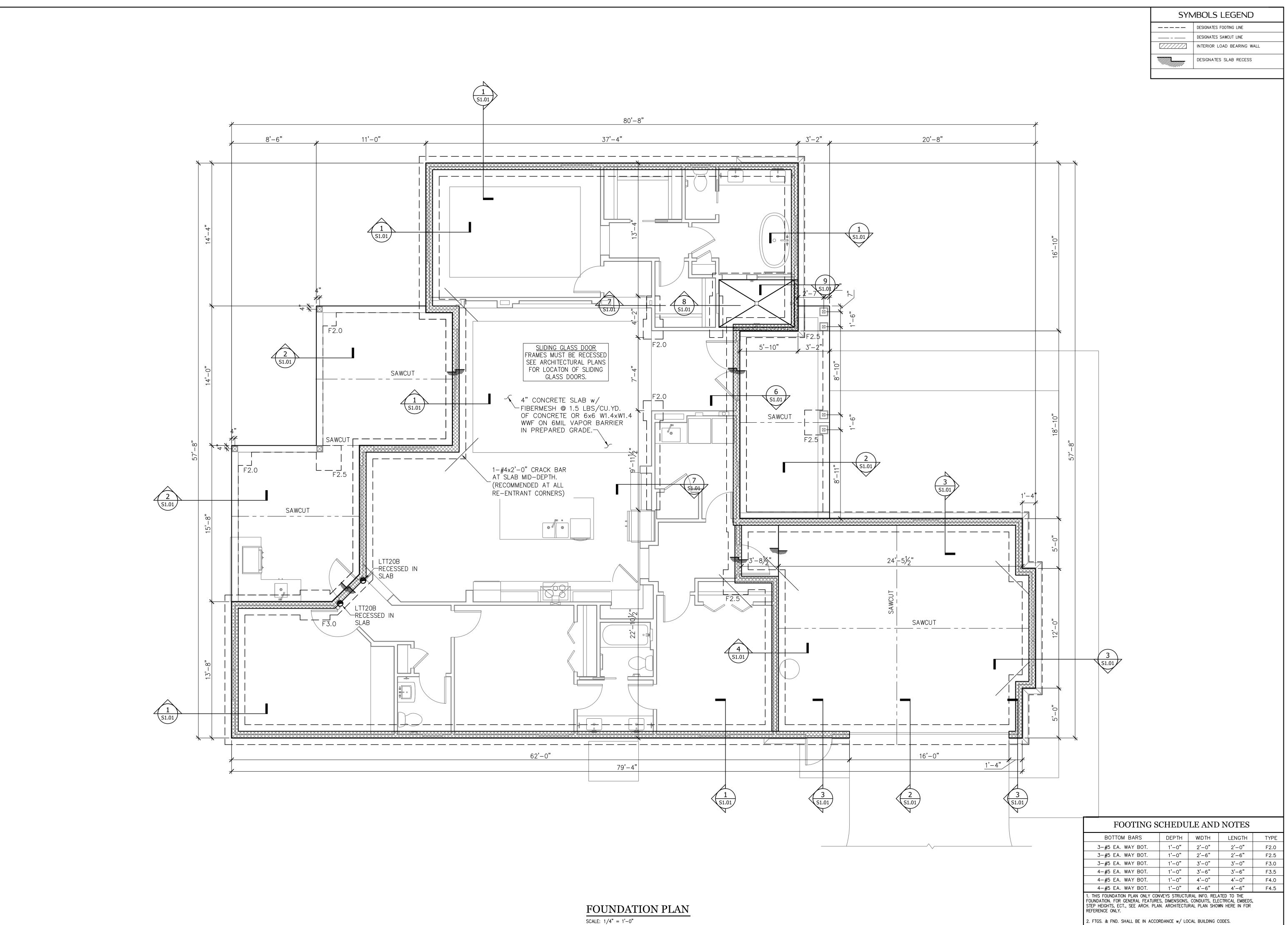
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DESIGN CRITERIA GENERAL

NOTES

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SHEET 2 OF 7



STATE OF STORIOS STORI

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STRUCTURAL ENGINEERING FOR THE LANCASTER 1752F-L MODEL AT WOOBOROUGH

FIELD ALTERATION

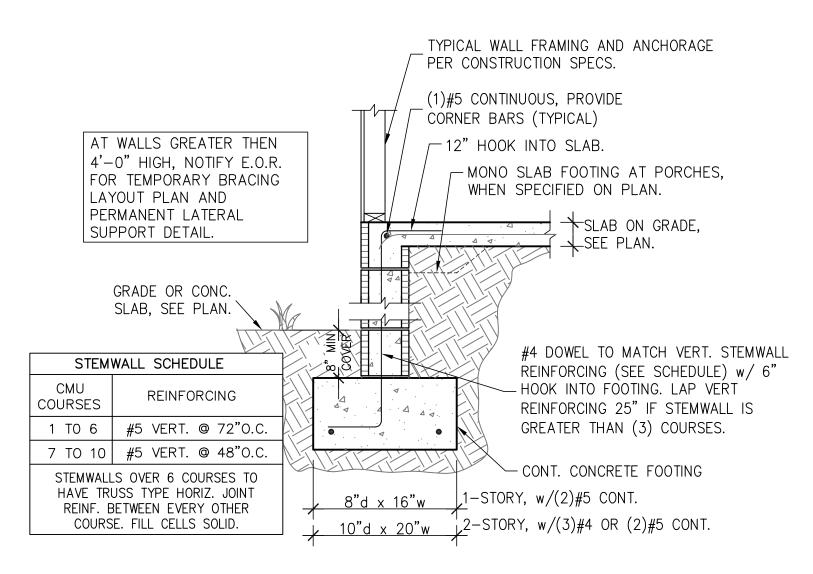
CONTRACTOR SHALL CONTACT SABO
STRUCTURAL ENGINEERING PRIOR TO
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ABOURIN MAY RESULT IN ADDITIONAL
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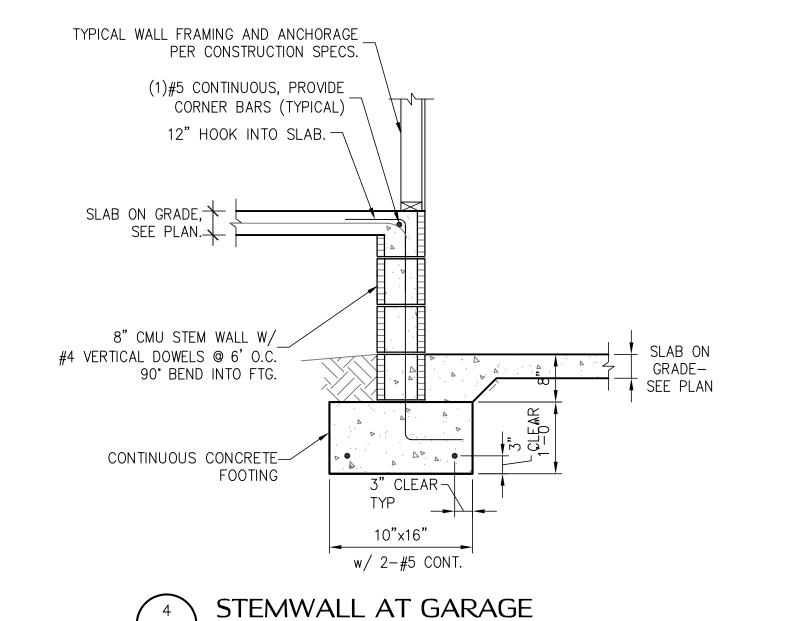
FOUNDATION PLAN

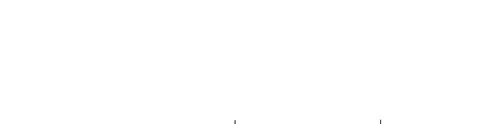
SHEET S OF 7

3. SOIL COMPACTION AND FILL SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.





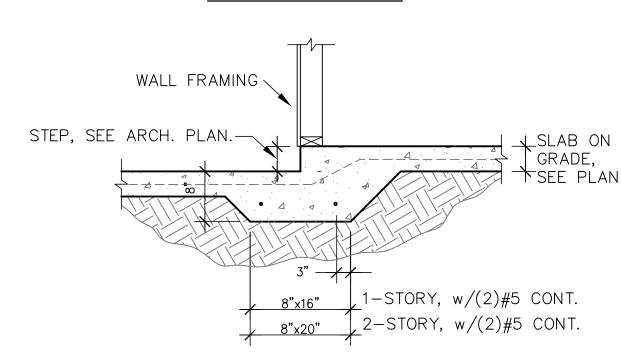




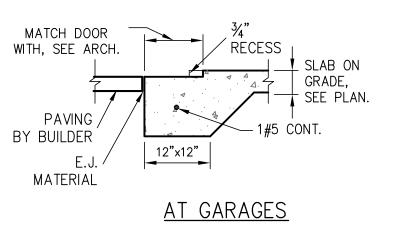
SCALE: 3/4" = 1'-0"

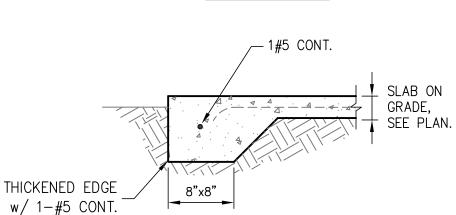
(4" MAX.)

8B @ SLAB RECESS

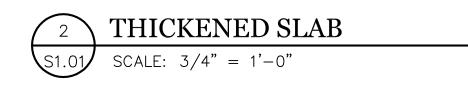


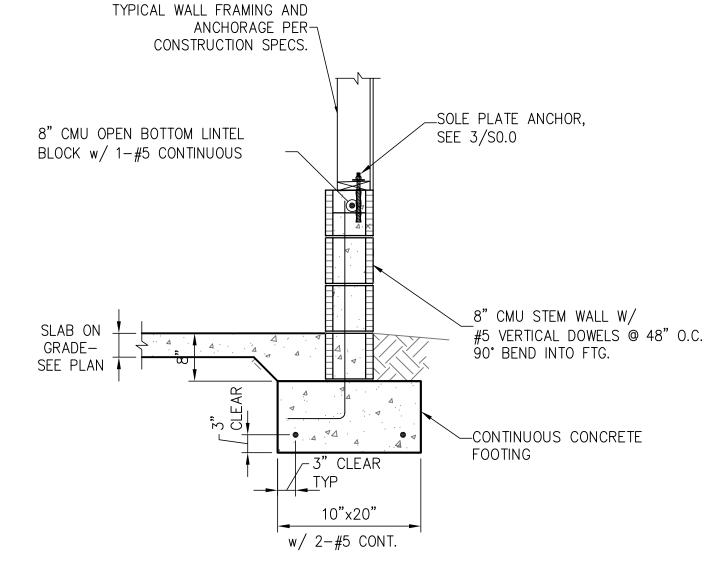
MONO. FOOTING AT STEP-DOWN SCALE: 3/4" = 1'-0"

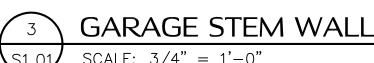


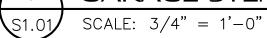


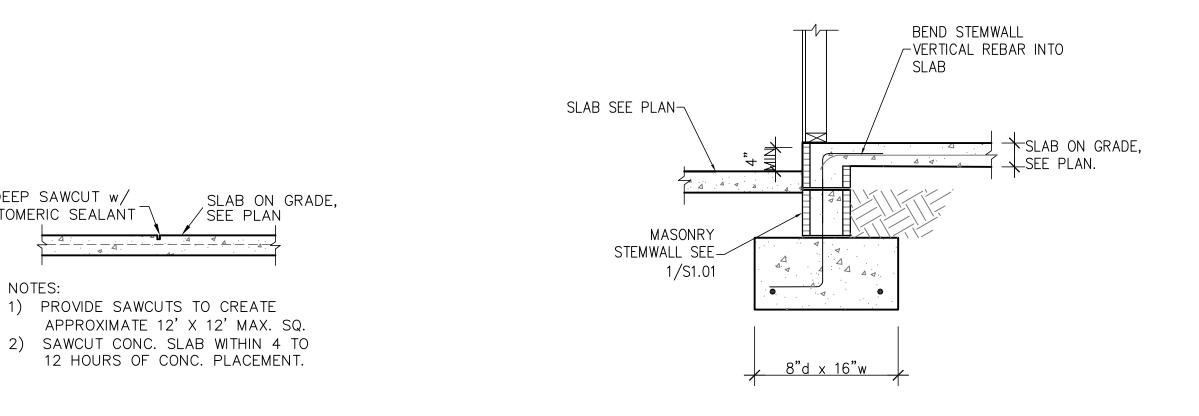
AT PORCHES

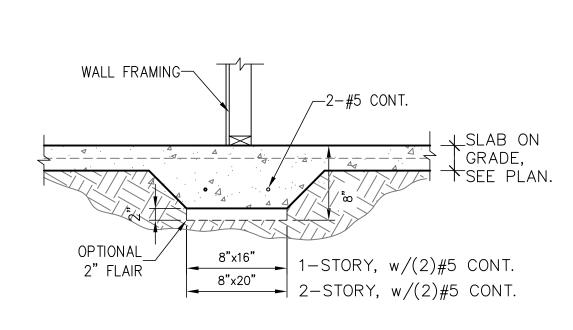


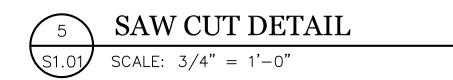










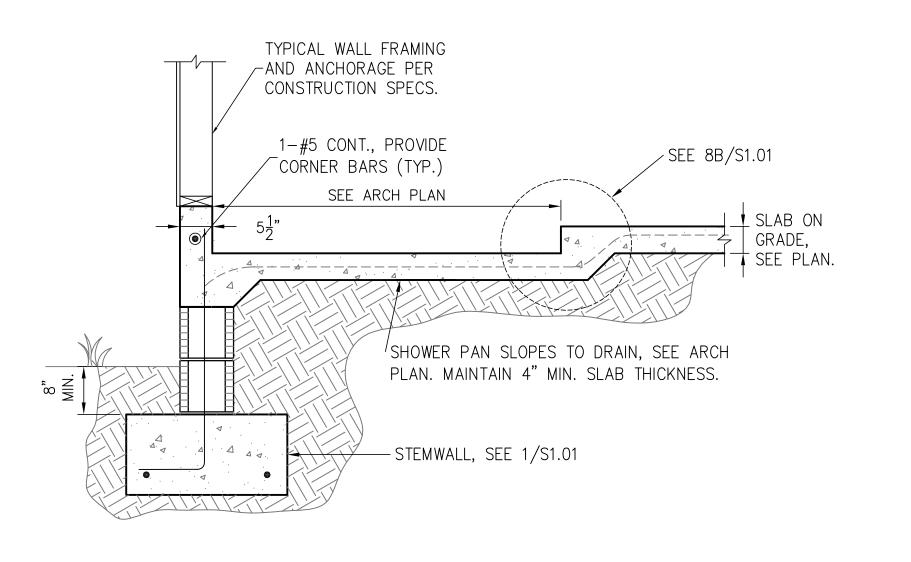


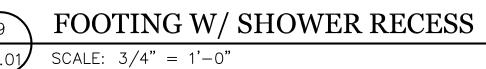
1" DEEP SAWCUT w/

ELASTOMERIC SEALANT









07.17.20

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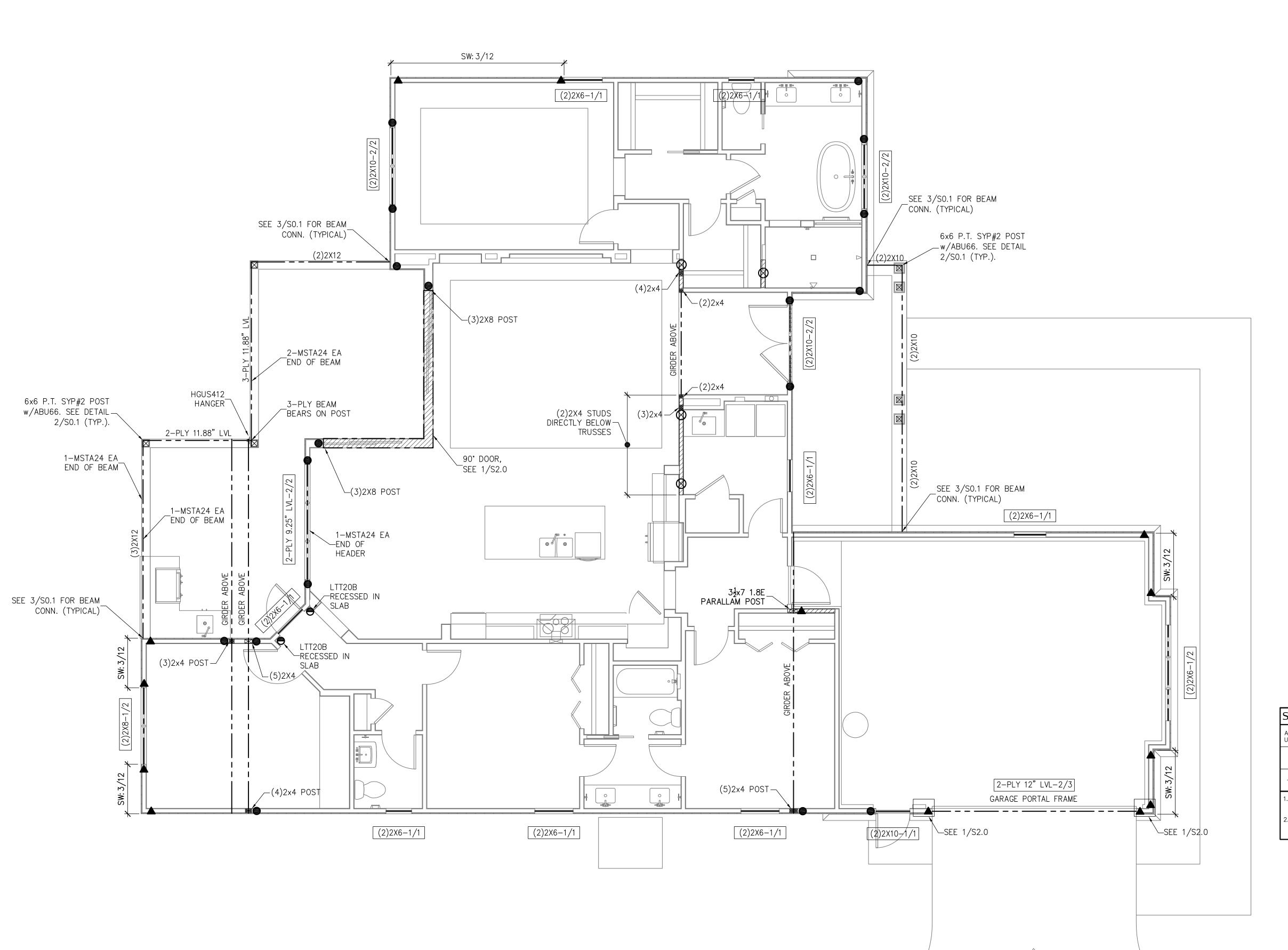
> ENGINEEF ASTER 17 WOOBOR STRUCTURA THE LAI MODEL

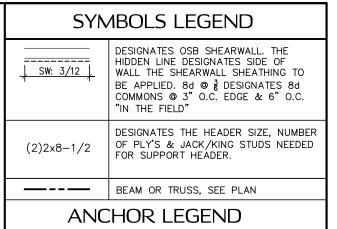
FIELD ALTERATION MAKING ANY STRUCTURAL FIELD MODIFICATIONS WHICH MAY VARY FROM THE INTENT OF THE ORIGINA CONSTRUCTION DOCUMENTS. ANY FIELD ALTERATIONS MADE PRIOR T ABOURIN MAY RESULT IN ADDITION ENGINEERING OR INSPECTION FE

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FOUNDATION **DETAILS**

> S1.01 SHEET 4 OF 7





%" A307 DIAMETER FULL HEIGHT THREADED ROD, SEE DETAIL 12/S0.1 %" A307 DIAMETER FULL HEIGHT THREADED ROD, SEE DETAIL 12/S0.1

%" A307 DIAMETER THREADED ROD TERMINATES AT FIRST FLOOR TOP PLATE, SEE DETAIL 12/S0.1

%" A307 DIAMETER THREADED ROD TERMINATES AT FIRST FLOOR TOP PLATE, SEE DETAIL 12/SO.1

SIMPSON HTT5 SEE DETAIL 15/S0.1 SIMPSON DTT2Z SEE DETAIL 15/S0.1

SIMPSON LTT20B SEE DETAIL 15/S0.1

WALL STUD SCHEDULE PLATE HEIGHT LOCATION 2x4 SPF#2 @ 16" O.C. EXTERIOR 2x6 SPF#2 @ 16" O.C. <u>or</u> 2x4 SPF#2 @ 12" O.C. EXTERIOR EXTERIOR 2x6 SPF#2 @ 16" O.C. INTERIOR 2x4 SPF#2 @ 16" O.C. 2x6 SPF#2 @ 16" O.C. <u>or</u> 2x4 SPF#2 @ 12" O.C. INTERIOR

STUD NOTES: .) WALL STUDS SPECIFIED ON PLAN SUPERSEDE THIS TABLE 2.) MINIMUM STUD SIZE AND SPACING ARE SHOWN. CONTRACTOR MAY INCREASE STUD SIZE TO MEET

4.) USE SYP#2 FOR ALL TOP PLATES AND SOLE PLATES.

ARCHITECTURAL REQUIREMENTS.

5.) FASTEN BOTTOM PLATE OF INTERIOR LOAD BEARING WALLS TO CONCRETE SLAB w/16d MASONRY CUT NAILS @ 16" O.C. MINIMUM. SEE 3/SO.0 FOR ADDITIONAL ANCHORS AT SHEARWALLS

COMBINED USE PANEL NOTES

I. EXTERIOR WALL SHEATHING SHALL BE CONTINUOUS FROM BOTTOM PLATE TO UPPER MOST TOP PLATE. SEE DETAIL 1/SO.1 FOR SHEATHING SPLICE LOCATIONS FOR MULTI STORY CONDITIONS 2. SEE SHEET SO.O FOR WALL SHEATHING

SPECIFICATIONS. 3. UPPER MOST TOP PLATE SUPPORTING ROOF MEMBERS SHALL BE STRAPPED AS SHOWN IN DETAIL 1/SO.0

4. INSTALL SOLE PLATE ANCHORS PER DETAIL

GENERAL NOTES 1. SEE DETAIL 2/SO.O FOR WALL FRAMING DETAIL. SEE WALL STUD SCHEDULE THIS SHEET FOR STUD SIZES AND SPACING. AT GIRDERS AND BEAMS,

PROVIDE STUDS BELOW TO MATCH BEAM/GIRDER

2. SEE SHEET SO.O FOR ROOF AND FLOOR SHEATHING SPECIFICATIONS.

3. WHERE FRAMING MEMBERS CONSIST OF MULTIPLE PLIES (BEAMS, HEADER, AND STUDS) FASTEN PLIES TOGETHER PER DETAIL 6/SO.0 4. INSTALL SOLE PLATE ANCHORS PER DETAIL

5. AT SHEARWALLS, PROVIDE DIAPHRAGM ATTACHMENT PER DETAIL 5/S0.1

6. FOR ATTACHMENT OF EXTERIOR WALLS THAT TERMINATE BETWEEN TRUSSES, SEE 5A/S0.1

7. AT PORCHES, SEE DETAIL 2/SO.1 FOR FRAMING AND HOLD DOWNS

SOLE PLATE ANCHOR SPACING SCHD ALL EXTERIOR WALL 42" O.C. UNLESS OTHER NOTED SHEARWALLS

SOLE PLT @ # WHEN NOTED ON PLAN SEE NOTE 2 INSTALL SOLE PLATE ANCHORS PER DETAIL

(SW 8d@3"/6")

ANCHOR SPACING SHALL BE AS NOTED. FOR EXAMPLE - SOLE PLT @ 36" = 36" ON-CENTER SPACING

SONAL 07.17.20

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ENGINEER ASTER 17: WOOBOR

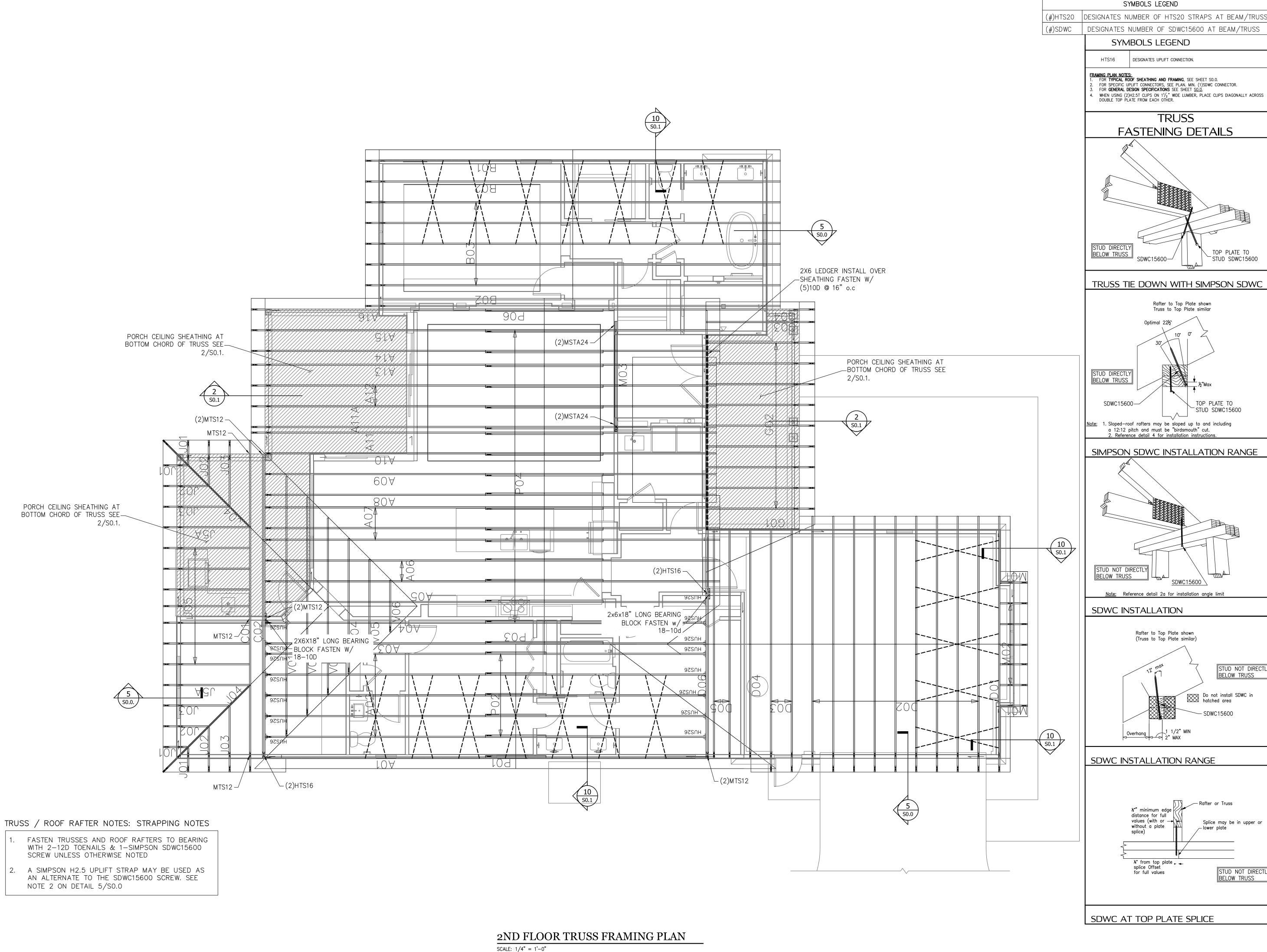
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FIRST LEVEL WALL FRAMING PLAN

SHEET 5 OF 7

FIRST LEVEL WALL FRAMING PLAN SCALE: 1/4" = 1'-0"



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REVISIONS

ENGINEEF ASTER 17 WOOBOR

THE LANCA MODEL AT

Rafter to Top Plate shown Truss to Top Plate similar

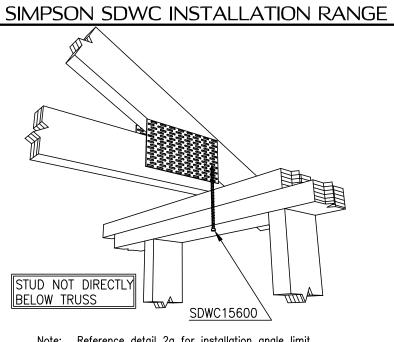
TRUSS

TOP PLATE TO

TOP PLATE TO

STUD SDWC15600

STUD SDWC15600



Note: Reference detail 2a for installation angle limit

Rafter to Top Plate shown (Truss to Top Plate similar)

STUD NOT DIRECTL BELOW TRUSS Do not install SDWC in hatched area

> MAKING ANY STRUCTURAL FIELD MODIFICATIONS WHICH MAY VARY FROM THE INTENT OF THE ORIGINAL CONSTRUCTION DOCUMENTS. ANY FIELD ALTERATIONS MADE PRIOR TO BEING APPROVED BY CHRISTOPHER ABOURIN MAY RESULT IN ADDITION Engineering or inspection fees Rafter or Truss Splice may be in upper or

> > STUD NOT DIRECTI BELOW TRUSS

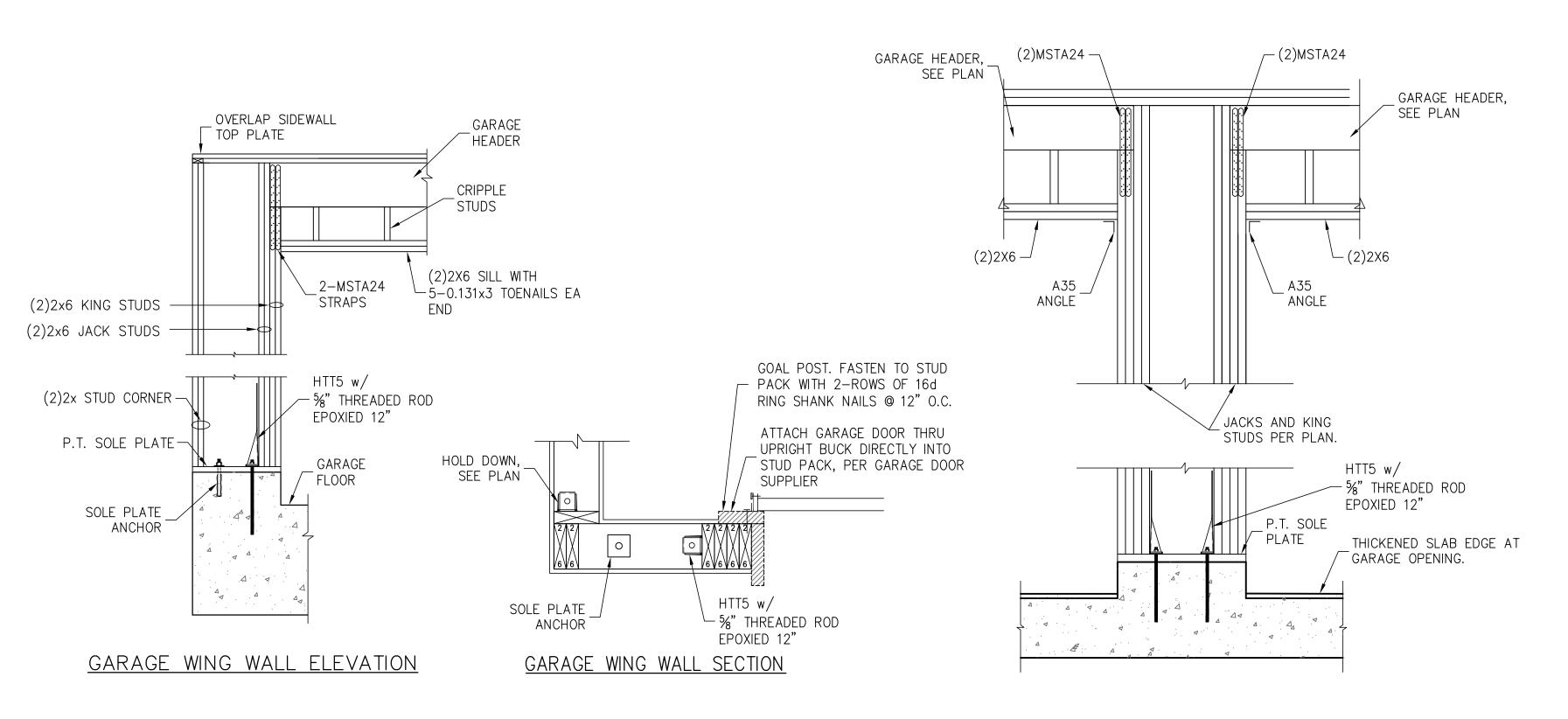
∠lower plate

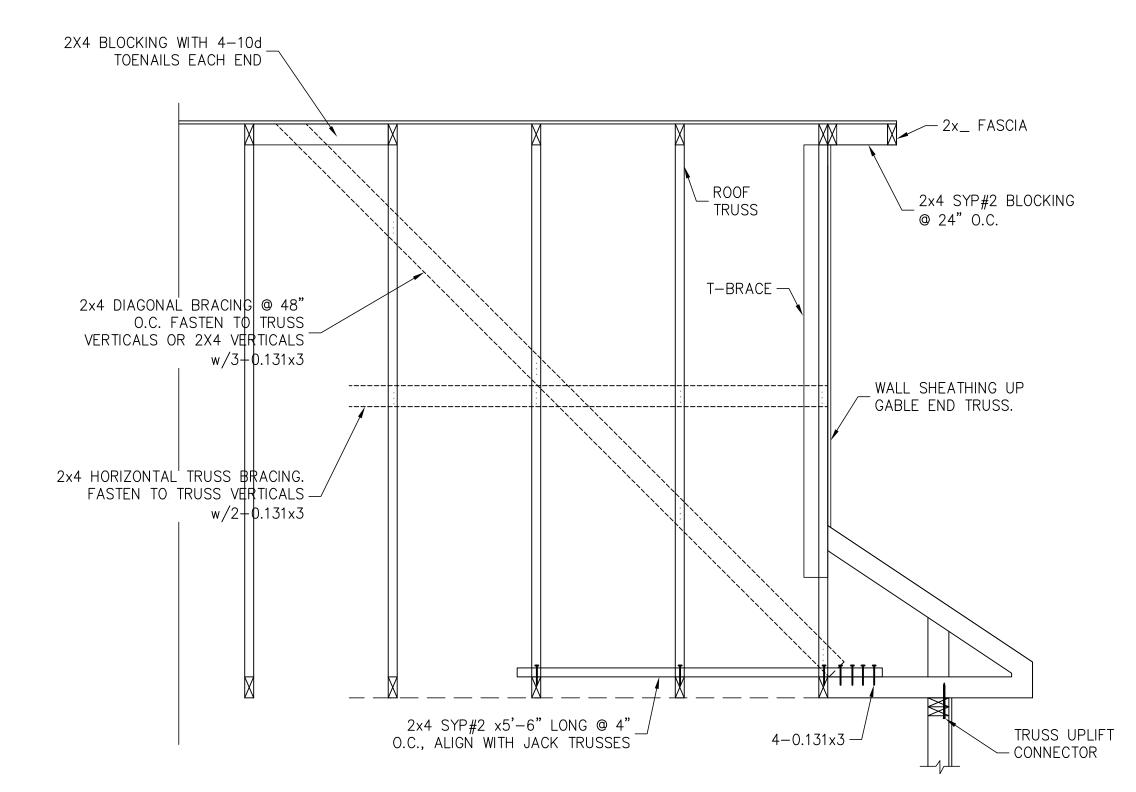
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FIELD ALTERATION

2nd FLOOR TRUSS FRAMING PLAN

SHEET 6 OF 7





TYPICAL GARAGE HEADER/JACK CONNECTION

SCALE: N.T.S.

GARAGE CENTER WALL FRAMING SCALE: NTS

GABLE END BRACE DETAIL

BUILDER TO VERIFY THAT TRUSSES HAVE BEEN DESIGNED FOR 100# POINT LOADS AT MITERED GLASS INTERSECTIONS. PROVIDE NOTCHED AND FITTED (2)2x8 SYP#2 w/PLYWOOD OR OSB SPACER TO SUPPORT HANGING - WALL @ 48" O.C. MAX. ALIGN WITH HANGING ┌¾" Ø THREADED ROD w/ 2×2× WALL WHEN TRUSS PERPENDICULAR TO WALL. WASHER TOP & $1\frac{1}{2}$ "ø WASHER PRE-ENGINEERED WOOD _ SEE SECTION A ¾"ø RODS @ 48" O.C. MIN. ROOF TRUSSES @ 24" O.C.,¬ BOT. RECESSED IN LOWER PLATE. PROVIDE 1 ROD WITHIN 8" OF END -2x2x%" PLATE WASHER. DESIGNED BY OTHERS ROOF TRUSS — OR JOIST. POST. 2-SIMPSON MSTA24 STRAP TIE BENT AT (2)2x8 SYP#2 CORNERS w/22-10d w/ SPACER. H2.5 EA END-12D COMMON NAILS, USE SECTION B 12d COMMON NAILS, USE 8 @ $11\frac{1}{4}$ " FRAMING, 45° <u>OR</u> 22.5° CORNER OF GLASS— 2x_ SYP#2 STUDS AT-8 @ 11¹/₄" FRAMING, $-6 \otimes 7\frac{1}{4}$ " FRAMING, 6 @ $7\frac{1}{4}$ " FRAMING, -AND WALL. 4 @ $5\frac{1}{2}$ " AND 4 @ $5\frac{1}{2}$ " AND - WALL SHEATHING $3\frac{1}{2}$ " FRAMING. $3^{1}/_{2}$ " FRAMING. AT CORNER OVERLAP - EACH PLY. NAIL CORNER WALL SHEATHING-GYPSUM -— 3/8" ROD, SEE ELEVATION w/9-12d IN EACH PLY 1-SIMPSON A35 TOP AND 8" MAX. TO $(3)2X_{TO} MATCH$ BOTTOM w/ (3)2x TO MATCH SEE ELEVATION FOR ATTACHMENT-MITERED GLASS -1st ROD. - WÁLL WIDTH. OVERLAP - WALL WIDTH. OVERLAP $(12) 8dx1\frac{1}{2}$ " 8"MAX. 70 OF LOWER PLATES TO ADJACENT PLIES AT CORNER. FASTEN PLIES AT CORNER. FASTEN 1st ROD/ WALLS (TYPICAL) PLIES WITH 2 ROWS OF PLIES WITH 2 ROWS OF MITERED GLASS 10D COMMONS @ 12" O.C. 10d COMMONS @ 12" O.C. SECTION A SECTION AT CORNER _END POST SEE PLAN **ELEVATION** MITERED WINDOW HEAD FRAMING

07.17.20 Christopher J Sabourin FL PE#71461

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20-0187 DATE ISSUE REVISIONS DATE

ENGINEEF ASTER 17: WOOBOR THE LA MODEL

FIELD ALTERATION MAKING ANY STRUCTURAL FIELD MODIFICATIONS WHICH MAY VARY FROM THE INTENT OF THE ORIGINA CONSTRUCTION DOCUMENTS. ANY FIELD ALTERATIONS MADE PRIOR TO BEING APPROVED BY CHRISTOPHER ABOURIN MAY RESULT IN ADDITION ENGINEERING OR INSPECTION FE

SCALING
DO NOT SCALE DIMENSIONS FROM ESE DRAWINGS. IF A DIMENSION UNCLEAR REFER TO THE ARCHITECTURAL DRAWINGS OR CONTACT THE E.O.R.

MISC. FRAMING **DETAILS**

> S2.0 SHEET 7 OF 7