

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID: ITV18228Z0330151203

Building Permit #
28035



Seal Date: 09/30/2009

-Truss Design Engineer-

James F. Collins Jr.

Florida License Number: 52212

1950 Marley Drive

Haines City, FL 33844

Truss Fabricator: Anderson Truss Company

Job Identification: 9-136--Doug Edgley Tuwuana Rossin -- , **

Truss Count: 69

Model Code: Florida Building Code 2007 and 2009 Supplement

Truss Criteria: FBC2007Res/TPI-2002(STD)

Engineering Software: Alpine Software, Version 9.02.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-05 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: BRCLBSUB-A140GC020109-A140GS020109-HIPFRAME-

#	Ref	Description	Drawing#	Date
1	29471--H7A		09273020	09/30/09
2	29472--H9A		09273007	09/30/09
3	29473--H11A		09273038	09/30/09
4	29474--A19		09273008	09/30/09
5	29475--H13A		09273046	09/30/09
6	29476--A1		09273009	09/30/09
7	29477--A2		09273010	09/30/09
8	29478--A3		09273011	09/30/09
9	29479--A4		09273012	09/30/09
10	29480--A5		09273013	09/30/09
11	29481--A6		09273016	09/30/09
12	29482--A7		09273018	09/30/09
13	29483--A8		09273020	09/30/09
14	29484--A9		09273021	09/30/09
15	29485--A10		09273022	09/30/09
16	29486--A11		09273023	09/30/09
17	29487--A12		09273024	09/30/09
18	29488--A13		09273025	09/30/09
19	29489--A14		09273026	09/30/09
20	29490--A15		09273027	09/30/09
21	29491--A16		09273028	09/30/09
22	29492--A17		09273029	09/30/09
23	29493--A18		09273030	09/30/09
24	29494--H7B		09273030	09/30/09
25	29495--H9B		09273011	09/30/09
26	29496--H11B		09273014	09/30/09
27	29497--H13B		09273015	09/30/09
28	29498--H15B		09273033	09/30/09
29	29499--B1		09273035	09/30/09
30	29500--B2		09273038	09/30/09
31	29501--B3		09273054	09/30/09
32	29502--H7C		09273001	09/30/09
33	29503--H9C		09273006	09/30/09
34	29504--H11C		09273013	09/30/09
35	29505--C1		09273023	09/30/09
36	29506--C2		09273033	09/30/09

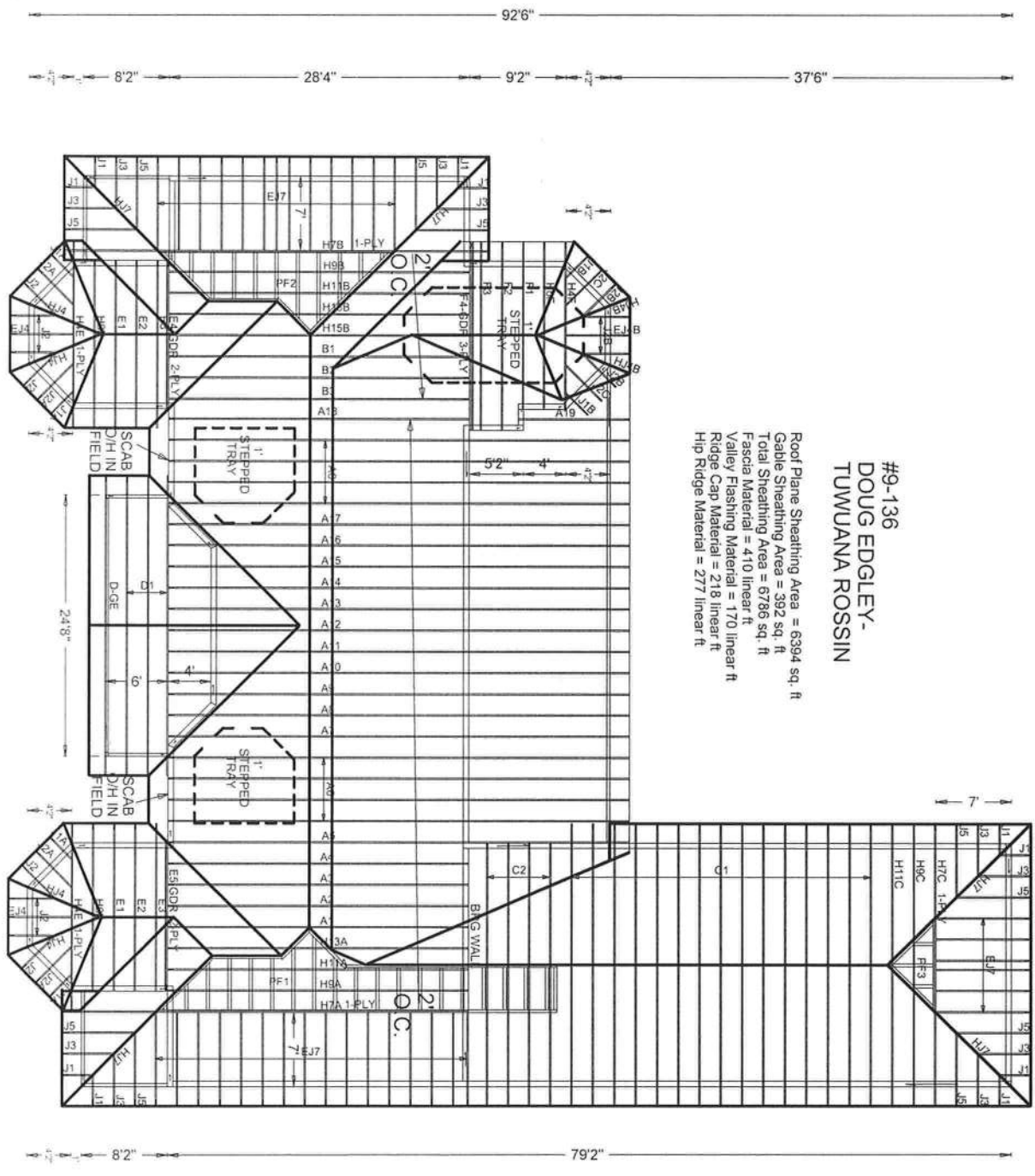
#	Ref	Description	Drawing#	Date
37	29507--D1		09273007	09/30/09
38	29508--D-GE		09273009	09/30/09
39	29509--H4E		09273003	09/30/09
40	29510--H6E		09273004	09/30/09
41	29511--E1		09273036	09/30/09
42	29512--E2		09273037	09/30/09
43	29513--E3		09273005	09/30/09
44	29514--E4-GDR		09273045	09/30/09
45	29515--E5-GDR		09273052	09/30/09
46	29516--H4F		09273035	09/30/09
47	29517--H6F		09273010	09/30/09
48	29518--F4-GDR		09273053	09/30/09
49	29519--F1		09273049	09/30/09
50	29520--F2		09273051	09/30/09
51	29521--F3		09273048	09/30/09
52	29522--HJ7		09273008	09/30/09
53	29523--J1		09273012	09/30/09
54	29524--J3		09273016	09/30/09
55	29525--J5		09273025	09/30/09
56	29526--EJ7		09273026	09/30/09
57	29527--J1A		09273022	09/30/09
58	29528--J2A		09273024	09/30/09
59	29529--J2		09273018	09/30/09
60	29530--HJ4		09273017	09/30/09
61	29531--EJ4		09273019	09/30/09
62	29532--J1B		09273031	09/30/09
63	29533--J2C		09273034	09/30/09
64	29534--J2B		09273028	09/30/09
65	29535--HJ4B		09273027	09/30/09
66	29536--EJ4B		09273029	09/30/09
67	29537--PF2		09273032	09/30/09
68	29538--PF3		09273002	09/30/09
69	29539--PF1		09273021	09/30/09





#9-136
DOUG EDGLEY-
TUWUANA ROSSIN

Roof Plane Sheathing Area = 6394 sq. ft
Gable Sheathing Area = 392 sq. ft
Total Sheathing Area = 6786 sq. ft
Fascia Material = 410 linear ft
Valley Flashing Material = 170 linear ft
Ridge Cap Material = 218 linear ft
Hip Ridge Material = 277 linear ft



JOB DESCRIPTION:: Doug Edgley
/: Tuwuana Rossin

JOB NO:
9-136

PAGE NO:
1 OF 1

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Page 1 of 1 Document ID:1TVI8228Z0330151203

Truss Fabricator: Anderson Truss Company
Job Identification: 9-136--Doug Edgley Tuwuana Rossin -- , **
Truss Count: 1
Model Code: Florida Building Code 2007 and 2009 Supplement
Truss Criteria: FBC2007Res/TPI-2002(STD)
Engineering Software: Alpine Software, Version 9.02.
Structural Engineer of Record:
Address:
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-05 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Seal Date: 09/30/2009

-Truss Design Engineer-
James F. Collins Jr.
Florida License Number: 52212
1950 Marley Drive
Haines City, FL 33844

Revised Trusses

#	Ref	Description	Drawing#	Date
1	29501--B3		09273054	09/30/09

ALPINE



Negative reaction(s) of -372# MAX. (See below) from a non-wind load case requires uplift connection.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

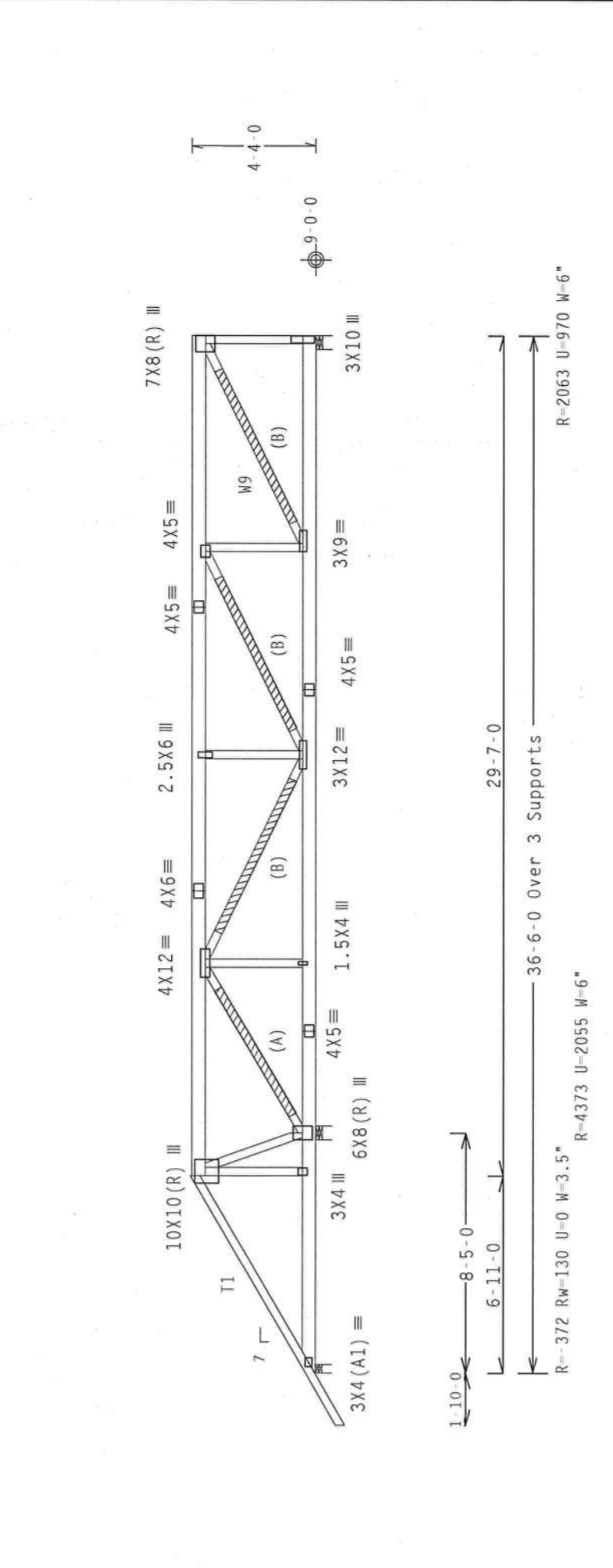
Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

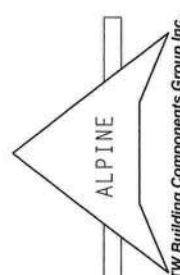
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Left side jacks have 7-0-0 setback with 0-0-0 cant and 1-10-0 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 1-10-0 overhang. Right side jacks have 0-0-0 setback with 0-0-0 cant and 0-0-0 overhang.

Top chord 2x6 SP #2 :T1 2x4 SP #2 Dense:
Bot chord 2x6 SP #2
Webs 2x4 SP #3 :W9 2x4 SP #2 Dense:
Roof overhang supports 2.00 psf soffit load.
(B) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.
(A) (2) #3 or better scab braces. Same size & 80% length of web member. Attach one to each face w/10d Box or Gun (0.128"x3", min.) nails @ 6" OC.
#1 hip supports 7-0-0 jacks with no webs.
Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .1875"/Ft.

TC LL	20.0 PSF	REF	R8228- 29471
TC DL	10.0 PSF	DATE	09/30/09
BC DL	10.0 PSF	DRW	HCUSR8228 09273020
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	46650
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TVI8228Z03

QTY:1

FL/-/4/-/-/R/-

Scale = .1875"/Ft.

SEP 30 2009

JAMES P. QUINN, JR.
LICENSED PROFESSIONAL ENGINEER
No. 52212
STATE OF FLORIDA

SEP 30 2009

1TVI8228Z03

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. TRUSSES ARE NOT TO BE USED FOR ANY PURPOSES OTHER THAN THAT FOR WHICH THEY WERE DESIGNED. THE USER OF THIS TRUSS ASSUMES ALL RESPONSIBILITY FOR THE TRUSS COMPONENT DESIGNER'S DESIGN. THE USER OF THIS TRUSS ASSUMES ALL RESPONSIBILITY FOR THE TRUSS COMPONENT DESIGNER'S DESIGN. THE USER OF THIS TRUSS ASSUMES ALL RESPONSIBILITY FOR THE TRUSS COMPONENT DESIGNER'S DESIGN.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

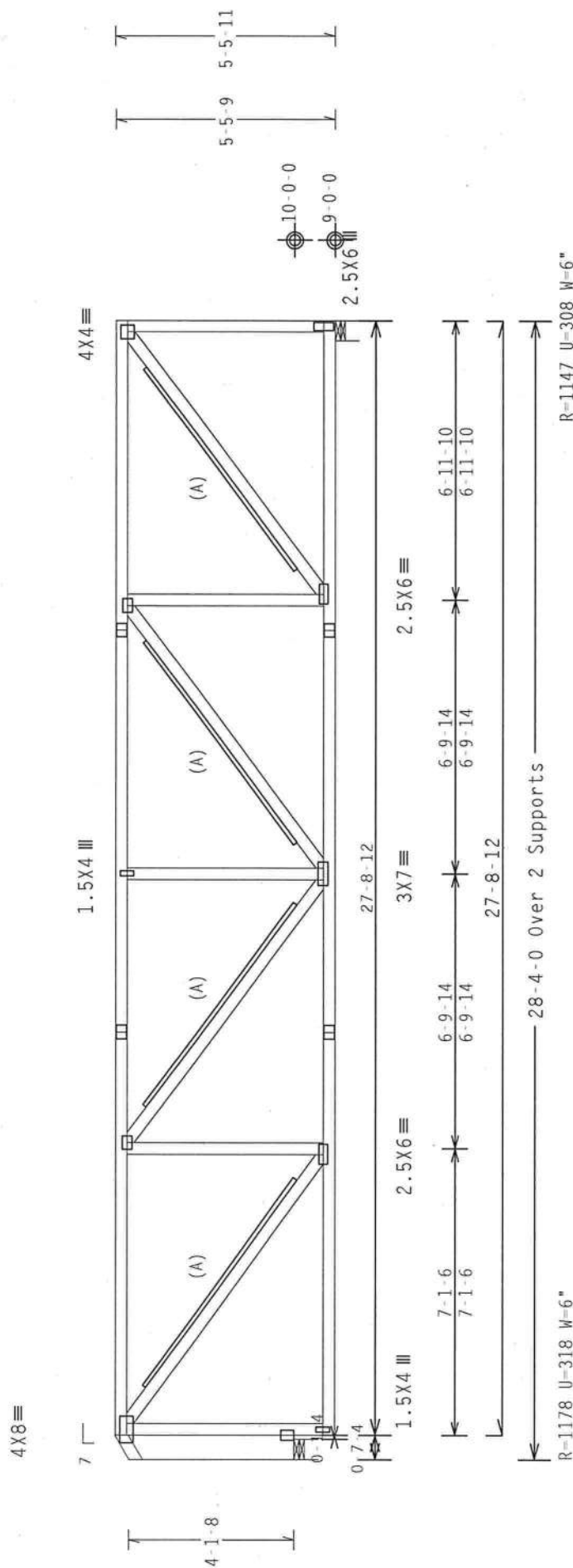
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

(A) 1x4 #3SRB SPF-S or better "T" brace, 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

9.02.00

QTY:1 FL/-/4/-/-/R/-/

Scale = .25"/Ft.

TC LL	20.0	PSF	REF	R8228- 29472
TC DL	10.0	PSF	DATE	09/30/09
BC DL	10.0	PSF	DRW	HCUSR8228 09273007
BC LL	0.0	PSF	HC-ENG	KD/AP
TOT.LD.	40.0	PSF	SEQN-	48761
DUR.FAC.	1.25		FROM	AH
SPACING	24.0"		JREF-	1TVI8228Z03

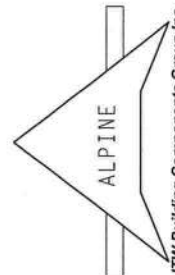


Sep 30 09

****WARNING**** THUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESIG'S FOLLOWING COMPONENT SAFETY INFORMATION, PUBLISHED BY API (CRUSS PLATE INSTITUTE, 210 ENTERPRISE LANE, 46015-0041, 53719), FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CHORD.

****IMPORTANT*****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TROSS IN CONFORMANCE WITH THIS OR FABRICATING, HANDLING, SHIPPING, INSTALLING, & BRACING OF TROSSES.

THIS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AISC) AND TPI, (DESIGN OF STEEL BUILDINGS) (NATIONAL SPECIFICATION FOR BRACING OF TRUSSES).
CONNECTOR PLATES ARE MADE OF 20/18/1668 (N/155/K) ASTM A663 GRADE 40/60 (N, K/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. A SEAL ON THIS INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABRAS 43 OF TPI-2002 SEC. 2.
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUBMITAL AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISC/TPI 1 SEC. 2.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

10 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ Gcpi (+/-)=0.18

blind reactions based on MWFRS pressures.

right end vertical not exposed to wind pressure.

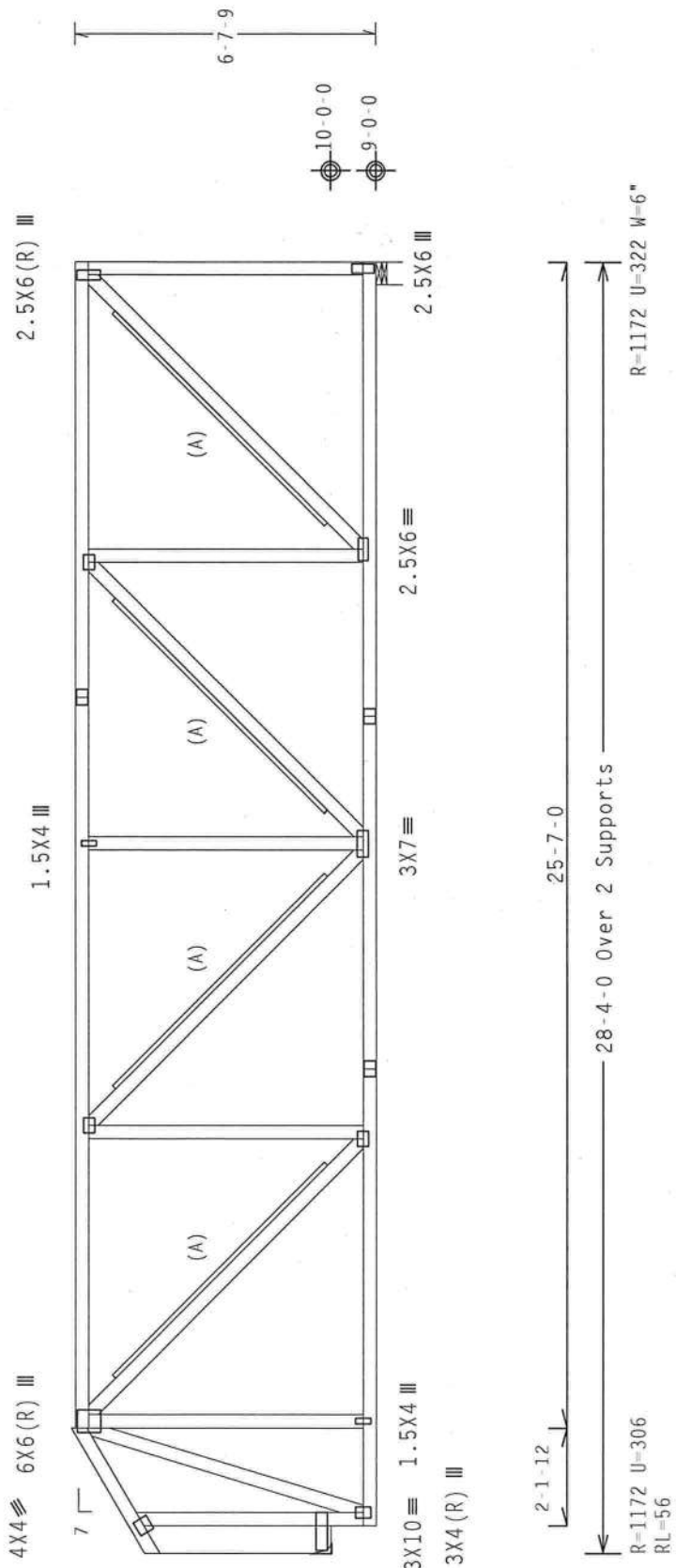
In lieu of structural panels use purlins to brace all flat TC @ 4" OC.

UWFRS loads based on trusses located at least 7.50 ft. from roof edge.

op chord 2x4 SP #2 Dense
not chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Lt Bearing Leg 2x8 SP #1 Dense:
(A) 1x4 #3SRB SPF-S or better "I" brace. 80% length of web
member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6"
c.

Bottom chord checked for 10.00 psf non-concurrent live load.

Reflection meets L/240 live and L/180 total load.



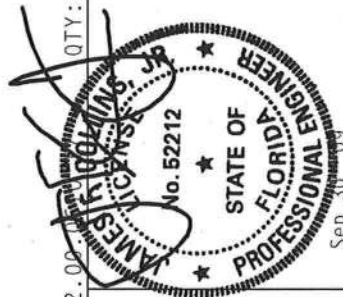
Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

~~9.02.00: 78~~

Scale = .25"/Ft.

TC LL	20.0 PSF	REF	R8228- 29473
TC DL	10.0 PSF	DATE	09/30/09
BC DL	10.0 PSF	DRW	HCUSR8228 09273038
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	46691
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TVI8228Z03

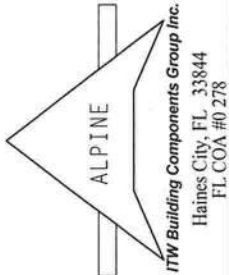


****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST SUITING COMPANY (SAFETY INFORMATION), PUBLISHED BY IPF (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MAUNSLON, NJ 07219) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SIALS HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SIALS HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****-FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. LTV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATIONS FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGNER'S REQUIREMENTS WILL BE AT THE INSTALLATION CONTRACTOR'S RISK.

DESIGNER COMPLIES WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY ACPA) AND TPI-1 (TRUSS INSTITUTIONAL PLATING CODE). ALL CONNECTOR PLATES ARE MADE OF 20/18/16GA (98.4 MINUS/5K GRADE 49/60 (H./K)55 GALV., STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON DESIGN POSITION PER DRAWINGS 160A-2.

IF ANY INSPECTION OF PLATES FOLLOWED BY (1), SHALL BE PERFORMED BY AN AIA OF TPI-1 2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



(9-136--Doug Edgley Tuwana Rossin -- ** - A19)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

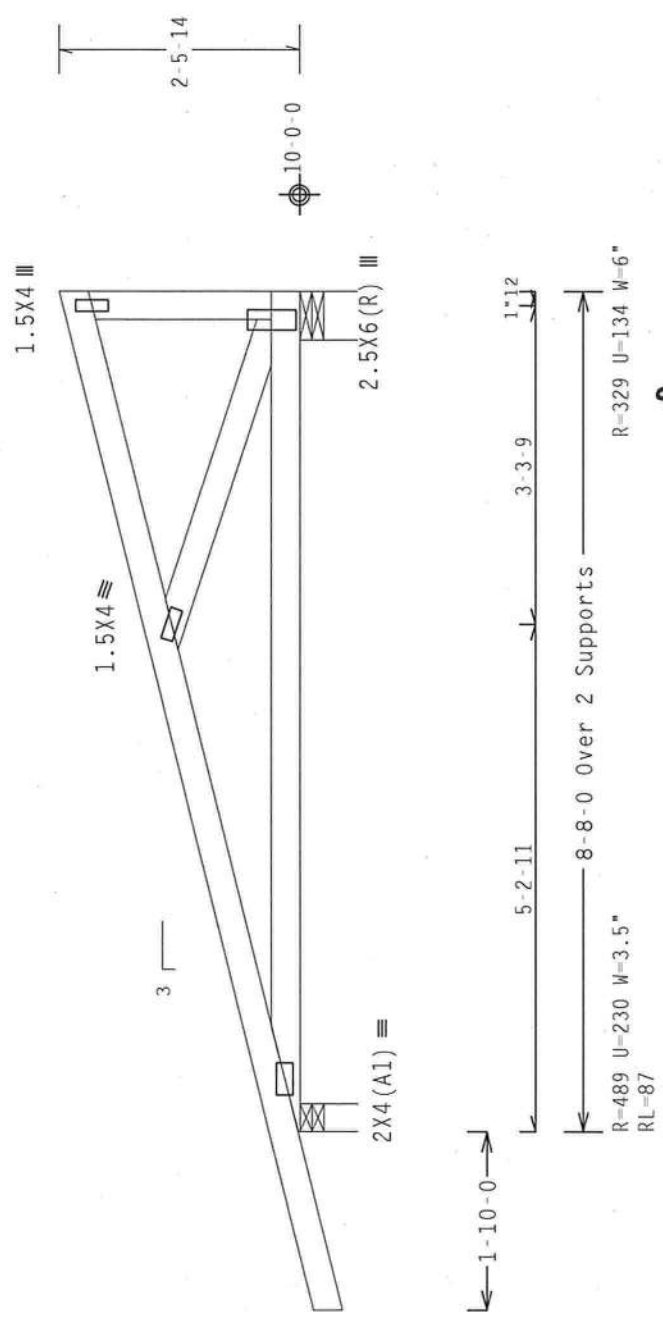
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART-ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind IC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.55

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



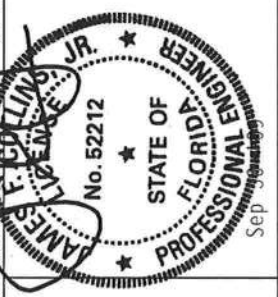
Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10% (0%)/0(0)

Scale = .5"/Ft.

QTY: 1

FL/-/4/-/-/R/-

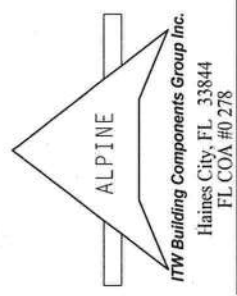
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TC DL	10.0 PSF	DATE	09/30/09
BC DL	10.0 PSF	DRW	HCUSR8228 09273008
BC LL	0.0 PSF	HC-ENG	KD/AP
TOT.LD.	40.0 PSF	SEQN-	48718
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TVI8228203



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI, 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CORROSION WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/106A (4.0/55/51) ASTM A653 GRADE 40/40 (4.0, K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DIMENSIONS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 13.1. A SEAL ON THIS DESIGN SIGNIFIES THE DESIGNER'S RESPONSIBILITY TO SELECT THE TRUSS FOR THE INSTALLATION WHERE THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



110 mph wind, 15.51 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCp1(+/-)=-0.18

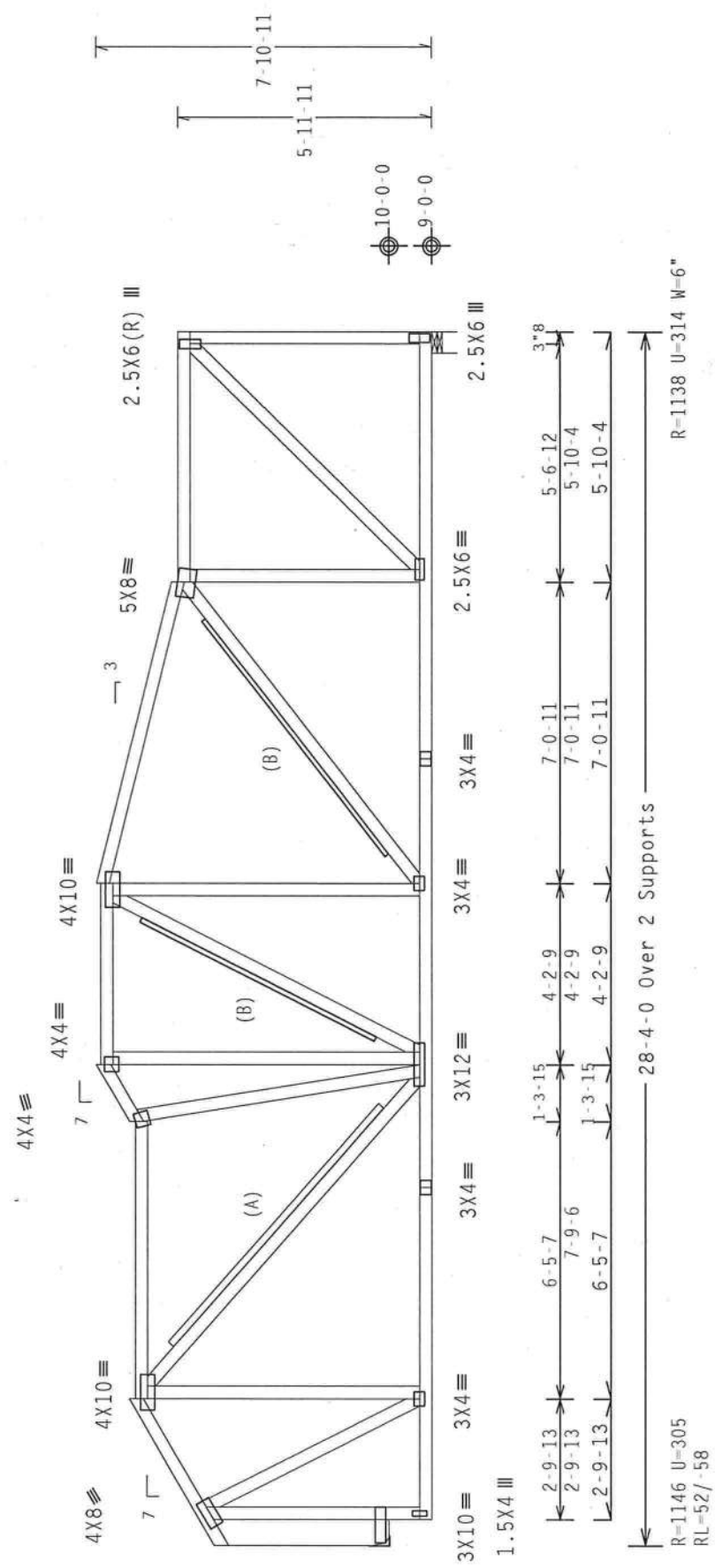
Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

(A) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.75 ft. from roof edge.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .25"/Ft.

QTY:1 FL/-/4/-/-/R/-/-

QTY: 00:00

~~9.02.00.~~

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST SUITING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2310 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WPCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

TC LL	20.0	PSF	REF	R8228-	29475
TC DL	10.0	PSF	DATE	09/30/09	
BC DL	10.0	PSF	DRW	HCUSR8228	0927304
BC LL	0.0	PSF	HC-ENG	KD/AP	
TOT.LD.	40.0	PSF	SEQN-	48895	
DUR.FAC.	1.25		FROM	AH	
SPACING	24.0"		JREF-	1TVI8228Z03	

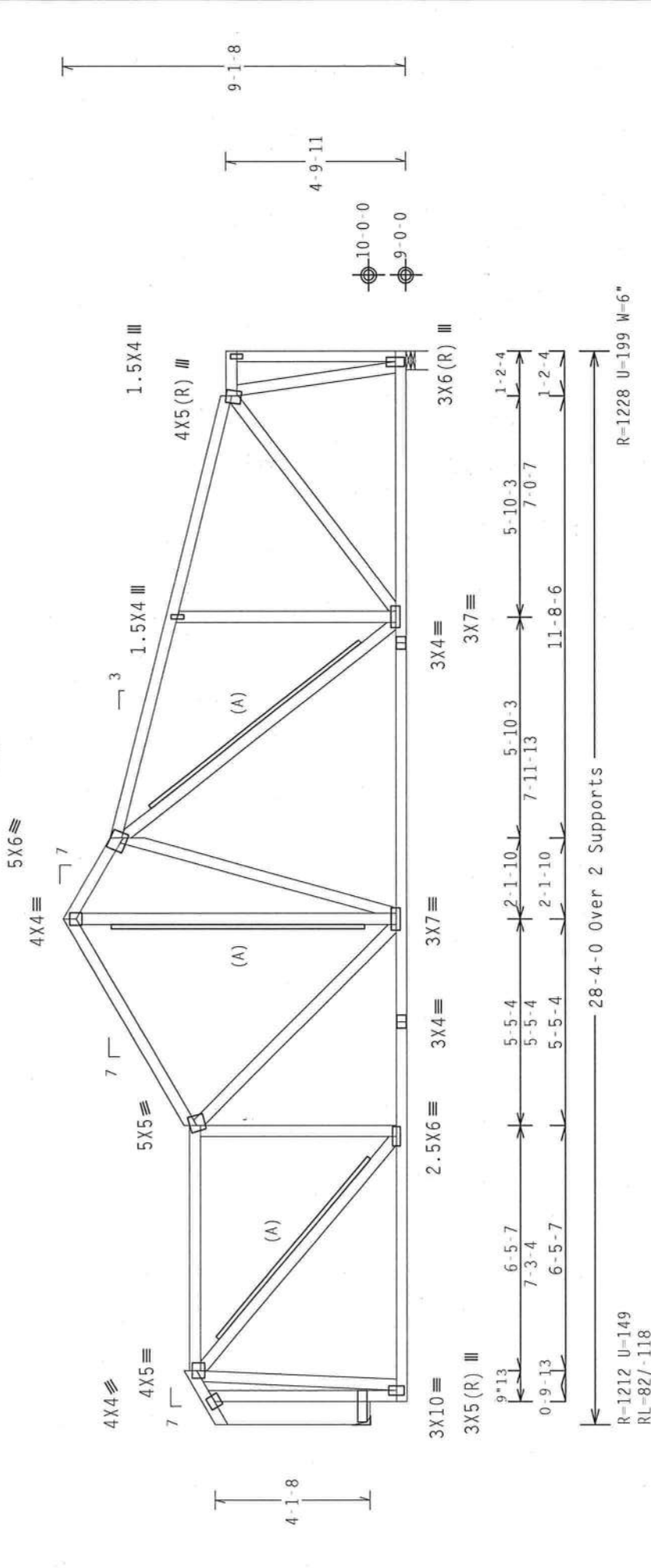


ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Bearing Leg 2x8 SP #1 Dense:

(A) 1x4 #3SRB SPF-S or better "I" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5".min.)nails @ 6" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.
Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .25"/Ft.

QTY:1

FL/-4/-/-R/-

TC LL	20.0 PSF	REF	R8228- 29476
TC DL	10.0 PSF	DATE	09/30/09
BC DL	10.0 PSF	DRW	HCUSR8228 09273009
BC LL	0.0 PSF	HC-ENG	KD/AP
TOT.LD.	40.0 PSF	SEQN-	48917
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TVI8228Z03

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, SHIPPING, HANDLING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304, AND WTC (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. TPI BCS CORRELATOR PLATES ARE MADE OF 20/18/16GA (4-H/SS/K) ASTM A653 GRADE 40/60 (4, 8/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 10GA-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED AS OF 1/11/2002 SEC.3. A SEAL OR THIS DESIGN INDICATE THE SUFFICIENCY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

JAMES Y. COLLINS JR.
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Sep 2009

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

(9-136--Doug Edgley Tuwuana Rossin -- ** - A2)

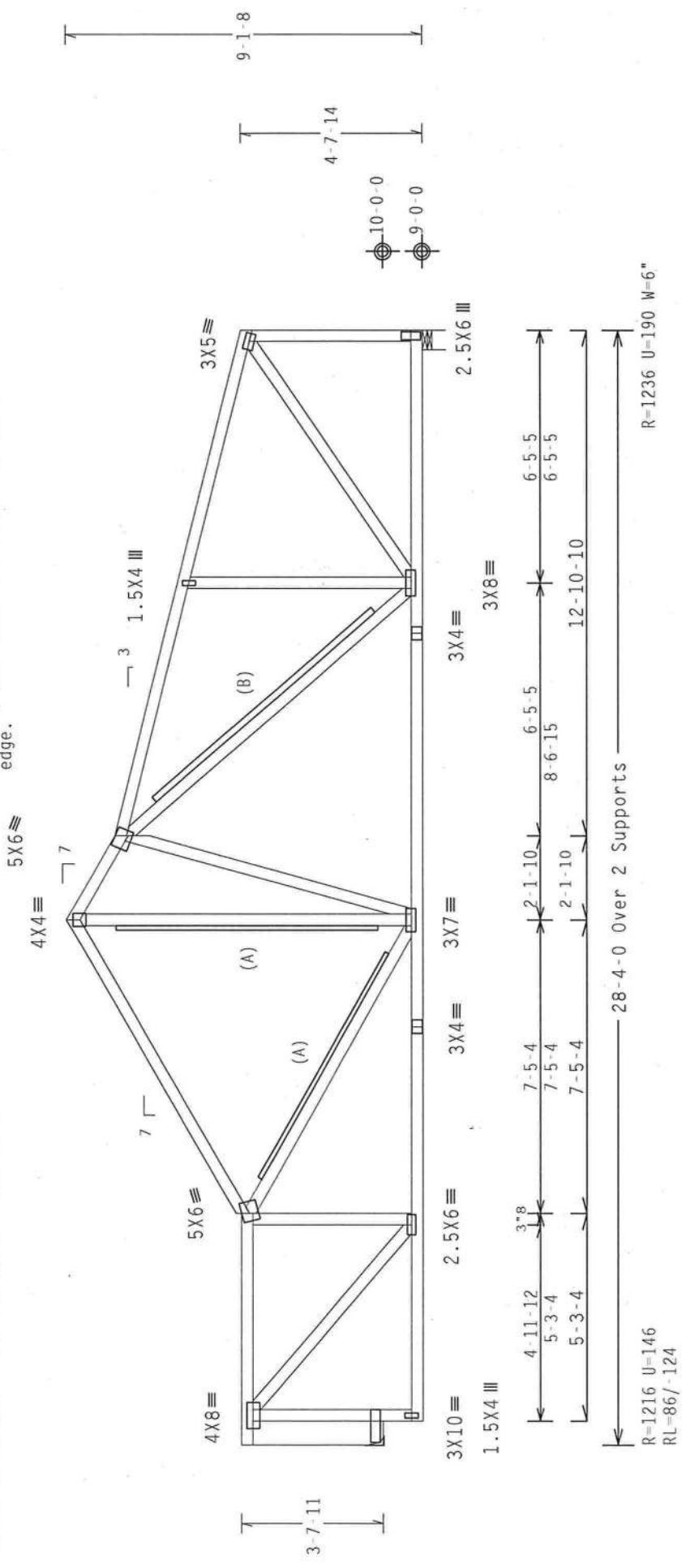
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Bearing Leg 2x8 SP #1 Dense:

(A) 1x4 #3SRB SPF-S or better "I" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.
OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.88 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.18
Wind reactions based on MMFRS pressures.
Right end vertical not exposed to wind pressure.

(B) 2x4 #3 or better "I" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.
MMFRS loads based on trusses located at least 15.88 ft. from roof edge.



PLT TYP. Wave

ALPINE

rtw Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

9.02.00 (0.00)

QTY:1

Scale = .25"/Ft.

TC LL	20.0 PSF	REF	R8228- 29477
TC DL	10.0 PSF	DATE	09/30/09
BC DL	10.0 PSF	DRW	HCUSR228 09273010
BC LL	0.0 PSF	HC-ENG	KD/AP
TOT.LD.	40.0 PSF	SEQN-	48931
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TVI8228203

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSS (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BCS (NATIONAL DESIGN SPEC. FOR ALPINE) AND TPI. ITH BCG CONNECTOR PLATES ARE MADE OF 2018/166A (4-H/SS/4) ASTM A653 GRADE 40/60 (H, K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-Z. INSPECTION OF PLATES IS REQUIRED FOR ALL TRUSSES. SMALL WEATHERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

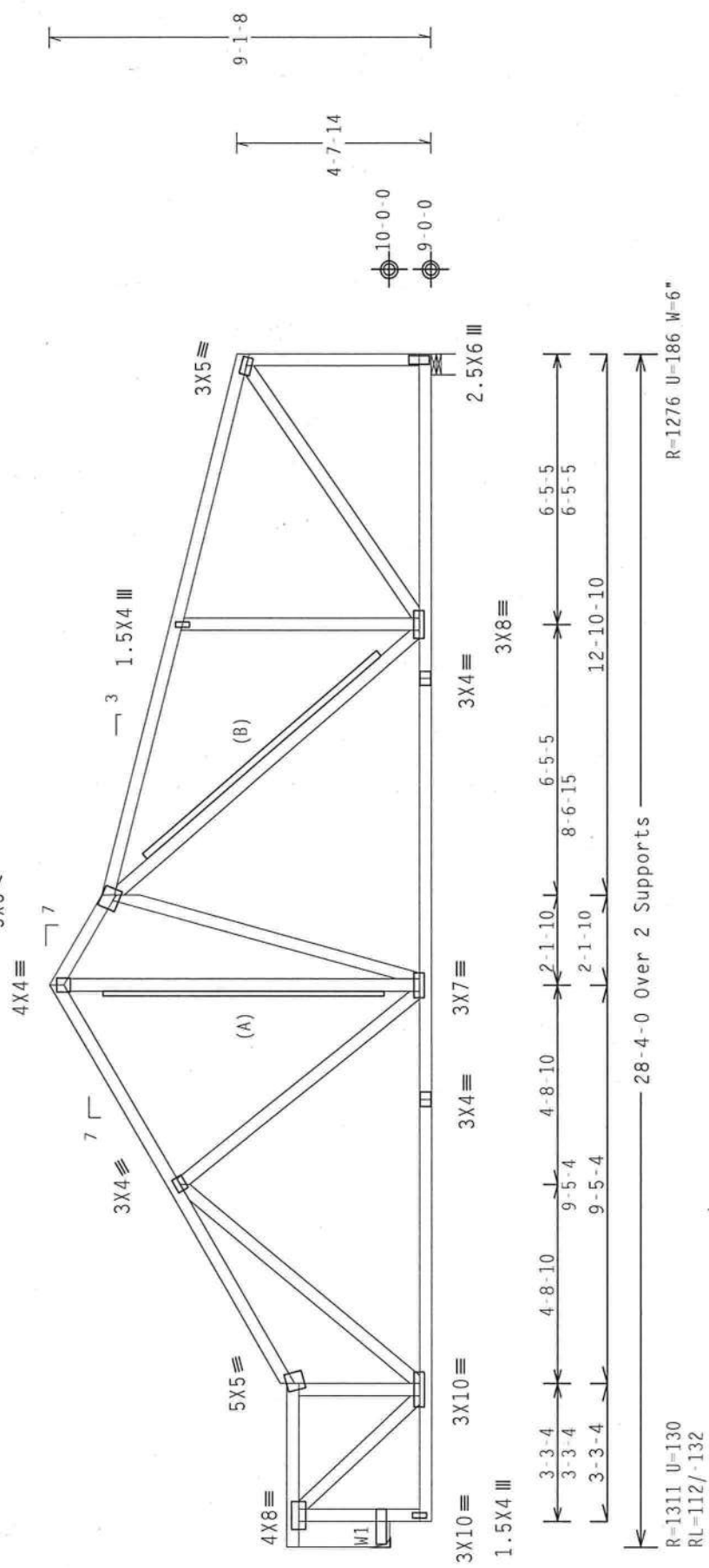
JAMES E. SULLIVAN, JR.
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Sep 30 2009

(9-136--Doug Edgley Tuwuana Rossin -- ** - A3)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W1 2x4 SP #2 Dense:
:Lt Bearing Leg 2x8 SP #1 Dense:

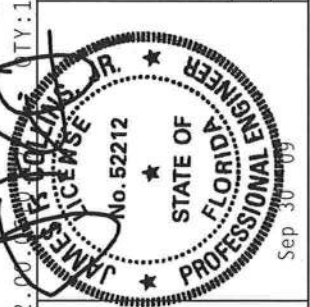
(A) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.30 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpl(+/-)=0.18
Wind reactions based on MMFRS pressures.
Right end vertical not exposed to wind pressure.
(B) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.
MMFRS loads based on trusses located at least 15.30 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave	Scale = .25" / Ft.
TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"
REF	R8228- 29478
DATE	09/30/09
DRW	HCSR8228 09273011
HC-ENG	KD/AP
SEQN	48944
FROM	AH
JREF	1TVI8228Z03



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REPLY TO: BUILDING COMPONENTS GROUP INC., 11700 BAYVIEW BLVD., SUITE 312, ALEXANDRIA, VA, 22304, AND NCEA TRUSS COUNCIL OF AMERICA, 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304. FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF RDS (NATIONAL DESIGN SPEC. BY ACPA) AND TPI. ITM BCG CONNECTOR PLATES ARE MADE OF 2017/16GA (4-H/SS/A) ASTM A653 GRADE 40/60 (4, K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX A3 OF TPI-2002 SEC.3. A SEAL OR THIS DESIGNER'S SIGNATURE SHALL BE REQUIRED FOR THE BOTTOM CHORD. THE BOTTOM CHORD SHALL BE THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AWS/TPI 1 SEC. 2.

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ Gcpi (+/-)=0.18

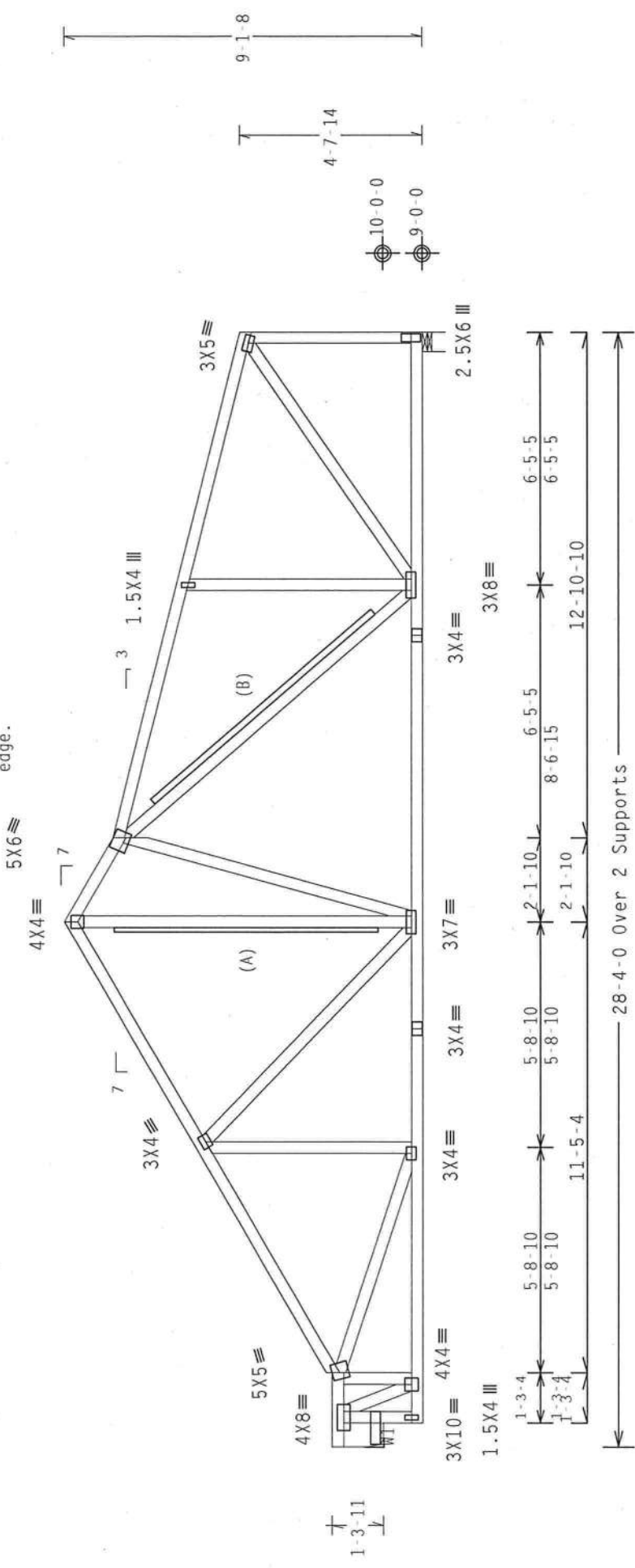
Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

(B) 2x4 #3 or better "T" brace. 80% Length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

MMFERS loads based on trusses located at least 15.00 ft. from roof edge.



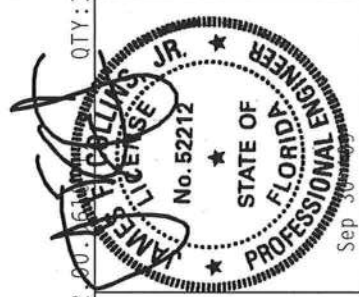
R=1237 U=184 W=6"

R=1225 U=129
RL=139/-135

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .25"/Ft.

TC LL	20.0 PSF	REF	R8228- 29479
TC DL	10.0 PSF	DATE	09/30/09
BC DL	10.0 PSF	DRW	HCUSR8228 09273012
BC LL	0.0 PSF	HC-ENG	KD/AP
TOT.LD.	40.0 PSF	SEQN -	48958
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF -	1TVI8228Z03



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BEG, INC. SHALL NOT BE RESPONSIBLE FOR INSTALLATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN COORDINATES, HANDLING, SHIPPING, INSTALLATION & BRACING OF TRUSSES. THE BEG DESIGN COORDINATES WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AISC) AND TPI. CONNECTOR PLATES ARE MADE OF 2018/18/16GA. (41H/55/PS) ASTM A663 GRADE 40/60 (4, 8/11-55) GALV., STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, IF NECESSARY, LOCATED ON THE POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX 4-3 OF TPI-2002 4-3-3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE INCLUDING DESIGNER PER AMS/TP-1 AND 2.



ALPINE

ITW Building Components Group Inc.
 Haines City, FL 33844
 FL COA #0 278

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

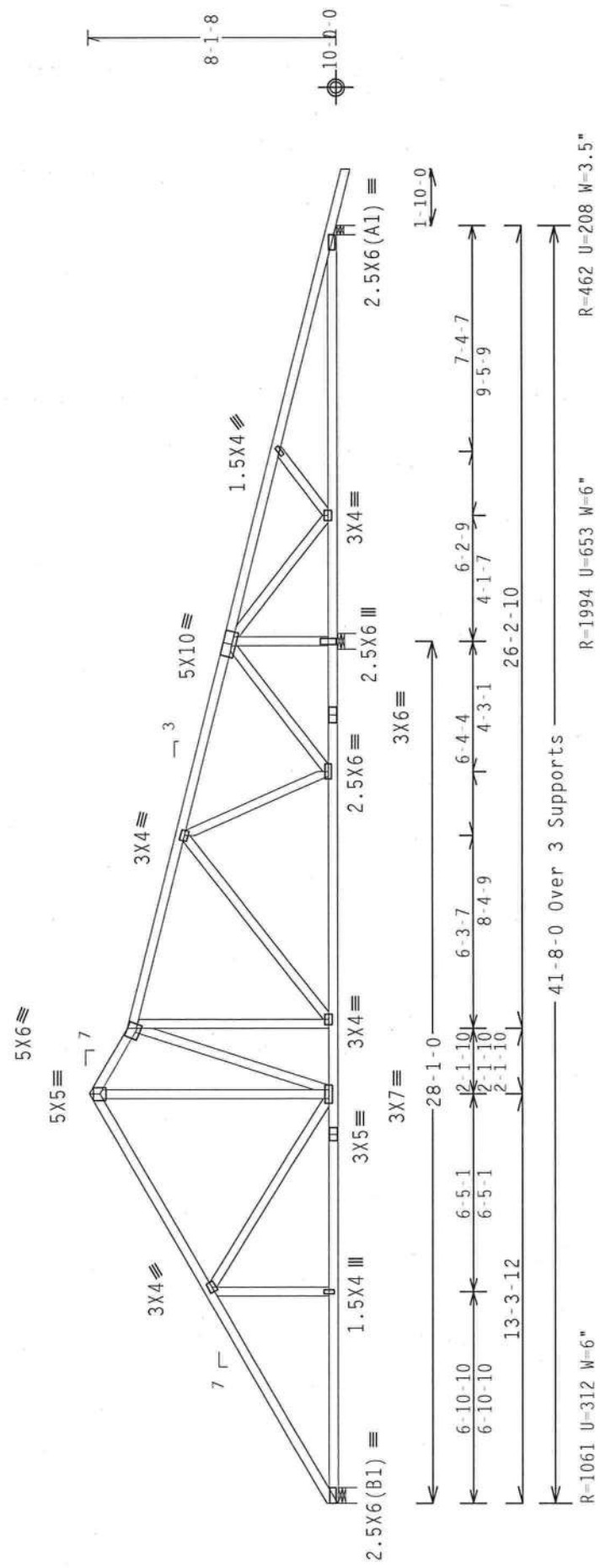
Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART-ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCPI (+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .1875"/Ft.

PLT TYP. Wave	QTY:1	FL/-/4/-/R/-	TC LL	20.0 PSF	REF	R8228 - 29480
			TC DL	10.0 PSF	DATE	09/30/09
			BC DL	10.0 PSF	DRW	HCUSR8228 09273013
			BC LL	0.0 PSF	HC-ENG	KD/AP
			TOT.LD.	40.0 PSF	SEQN	48965
			DUR.FAC.	1.25	FROM	AH
			SPACING	24.0"	JREF	1TVI8228203

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

JAMES E. HAYES, JR.
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Sep 30 09

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO THE TRUSS MANUFACTURER'S INSTRUCTIONS FOR THE TRUSS. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE TRUSS. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE TRUSS. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE TRUSS.

****IMPORTANT**** THROUGH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, ITW BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE TRUSS. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE TRUSS. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE TRUSS.

Top	chord	2x4	SP	#2	Dense
30t	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART_ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpf(+/-)-0.55

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

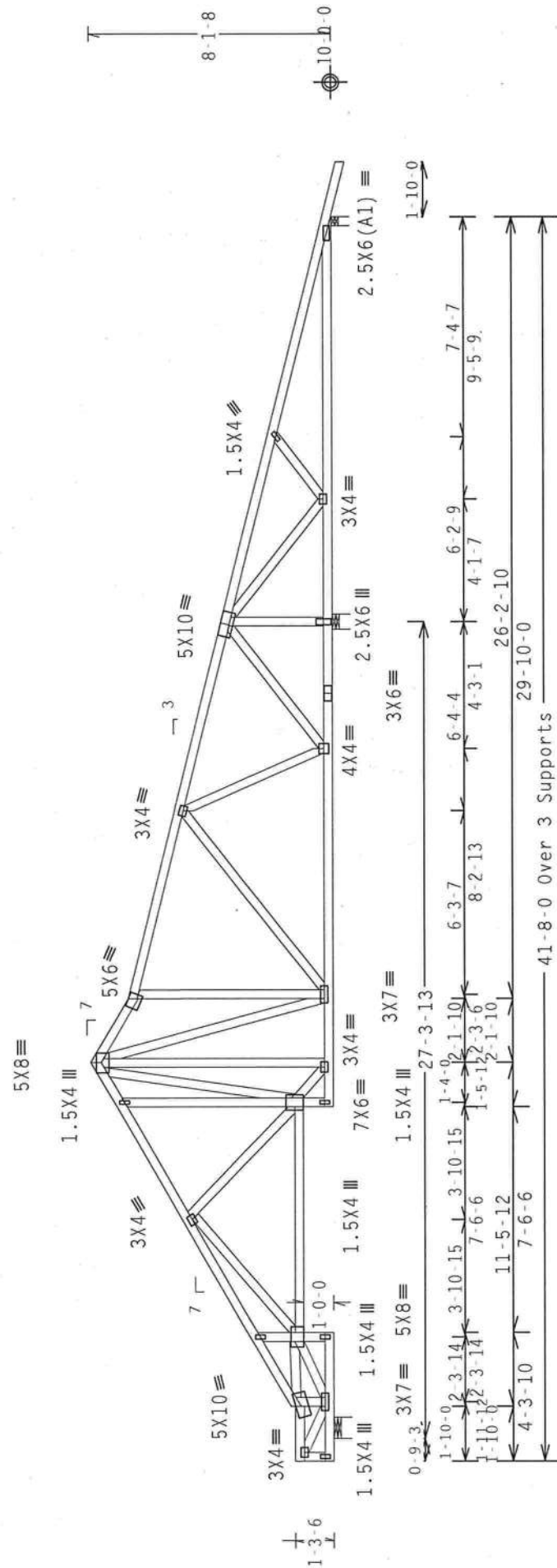
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

24°C.

Deflection meets L/240 live and L/180 total load.

WFRS loads based on trusses located at least 30.00 ft. from roof edge.



R=1064 U=278 W=8.485"
RL=130/-213

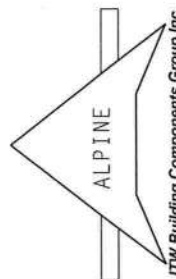
R=2048 U=661 W=6" R=400 U=194 W=3.5"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

~~000006~~ QTY: 1 FL/-/4/-/-/R/-

Scale = .1875"/Ft.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

[illegible]

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH ITPI-1 OR TABRICATED JOINTS, INCLUDING THE PROVISIONS OF AWS (ASTM) DESIGN SPEC., BY AEPWA AND TPTI. [ITW BCG] CORROSION RESISTANT COATING SHALL BE PROVIDED BY THE FABRICATOR. PROVIDER OF ROPS (ASTM) DESIGN SPEC., BY AEPWA AND TPTI. CONNECTOR PLATES ARE MADE OF 2018/16GSA (Q/H/45/SP) ASTM A563 GRADE 90/60 (H, K/J/S5) GALV. STEEL, APPLIED TO EACH FACE OF TRUSS AND/OR OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. IF AN INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX 4 OF TP11-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 29482
TC DL	10.0 PSF	DATE 09/30/09
BC DL	10.0 PSF	DRW HCUR8228 09273018
BC LL	0.0 PSF	HC-ENG KD/AP
TOT.LD.	40.0 PSF	SEQN- 49003
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TVI8228Z03

(9-136--Doug Edgley Tuwuana Rossin --, ** - A8)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

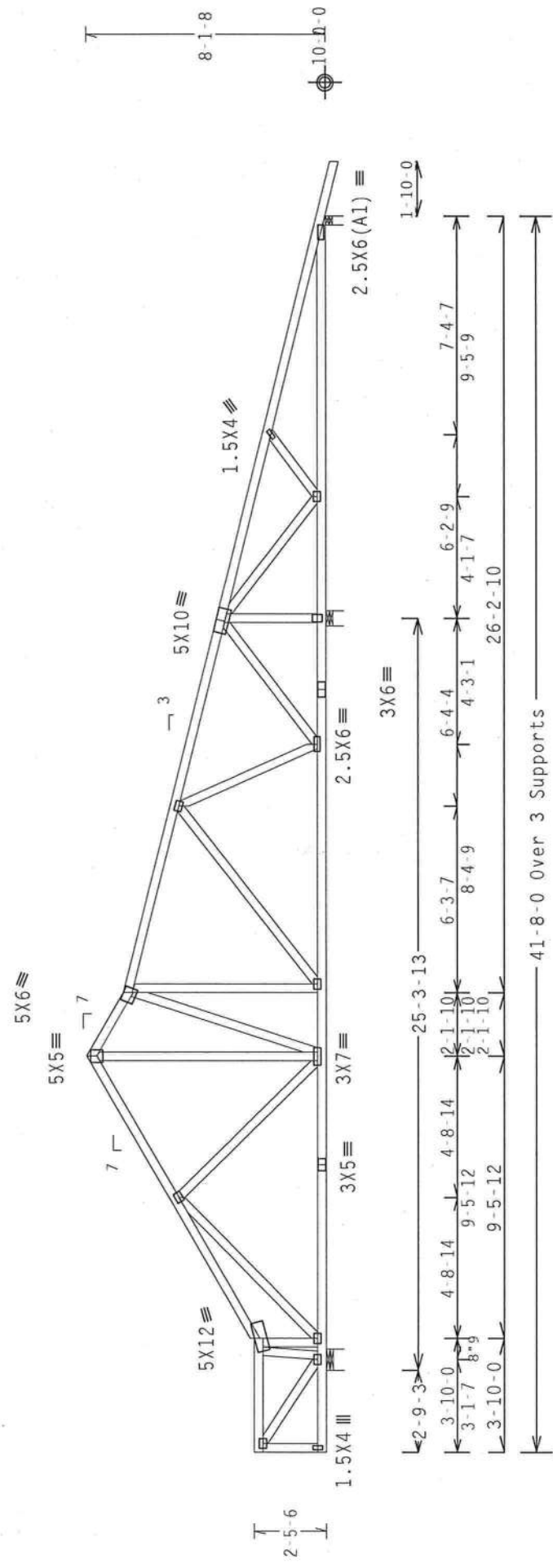
MWFRS loads based on trusses located at least 30.00 ft. from roof edge.

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)-0.55



R=1198 U=340 W=8.485"
RL=103/-202

R=1809 U=601 W=6"

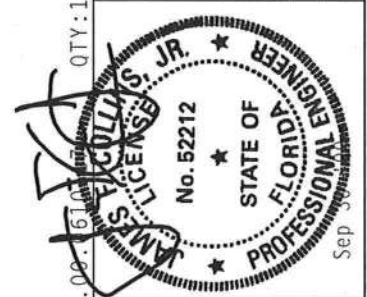
R=501 U=219 W=3.5"

Note: All Plates Are 3x4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .1875"/Ft.

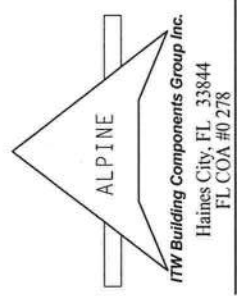
TC LL	20.0 PSF	FL/-/4/-/R/-	QTY:1	REF	R8228- 29483
TC DL	10.0 PSF			DATE	09/30/09
BC DL	10.0 PSF			DRW	HCUSR228 09273020
BC LL	0.0 PSF			HC-ENG	KD/AP
TOT.LD.	40.0 PSF			SEQN-	49011
DUR.FAC.	1.25			FROM	AH
SPACING	24.0"			JREF-	1TVI8228203



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCS1 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ACPA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/30/16GA (4-H/55/74) ASPH A653 GRADE 40/60 (4, 8/2H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 300A-2. DRAWING SPECIFIC PLACEMENT OF PLATES SHALL BE DETERMINED BY THE INSTALLATION CONTRACTOR. THE INSTALLATION CONTRACTOR SHALL BE RESPONSIBLE FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

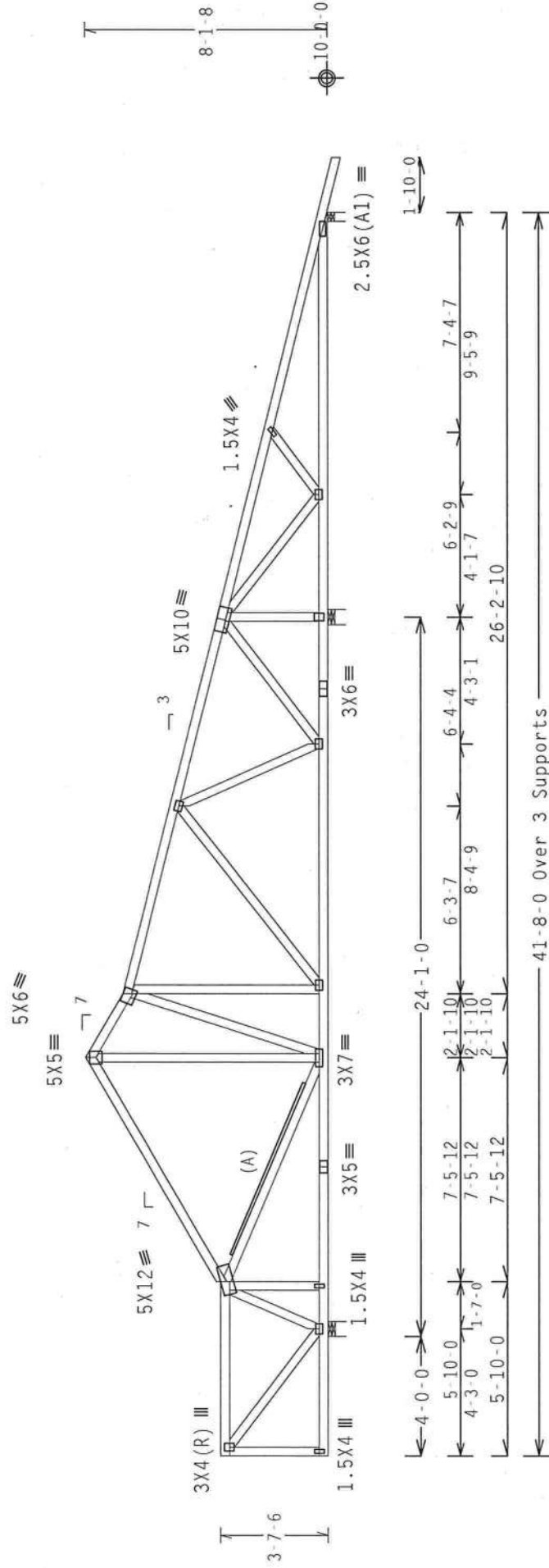


110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCpi (+/-)-0.55

Wind reactions based on MWFRS pressures.

(A) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.
 MWFRS loads based on trusses located at least 30.00 ft. from roof edge.



R=1731 U=581 W=6" R=514 U=223 W=3.5"

RL-76/-190

Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

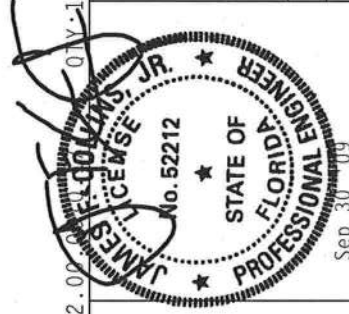
PLT TYP. Wave

FT/RT=10%(0%)/0(0) 9.02.00

FL/-/4/-/-/R/-

Scale = .1875" / Ft.

TC LL	20.0	PSF	REF	R8228 - 29484
TC DL	10.0	PSF	DATE	09/30/09
BC DL	10.0	PSF	DRW	HCUSR8228 09273021
BC LL	0.0	PSF	HC-ENG	KD/AP
TOT.LD.	40.0	PSF	SEQN -	49023
DUR.FAC.	1.25		FROM	AH
SPACING	24.0"		JREF -	1TVI8228Z03

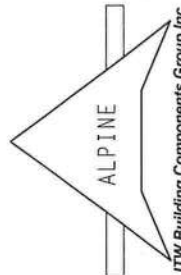


Sep 30 09

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO NCST (BUILDING COMPONENT SAFETY INFORMATION), AND PUBLISHED BY TP1 (TRUSS PLATE INSTITUTE), 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

**** IMPORTANT **** TURN SUB A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR FOR USE BY THE TRUSS INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN COORDINATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC. BY AISC) AND IPTI.
CONNECTIONS WITH TRUSSES SHALL BE MADE IN ACCORDANCE WITH THE FOLLOWING:
CONNECTOR PLATES ARE MADE OF 2X18/16GA (44/H/S&P) ASTM A653 GRADE 40/60 (44 K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2.
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF IPTI-2002 SEC.3.
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/IPTI 160A-2.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

(9-136--Doug Edgley Tuwana Rossin -- ** - A12)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

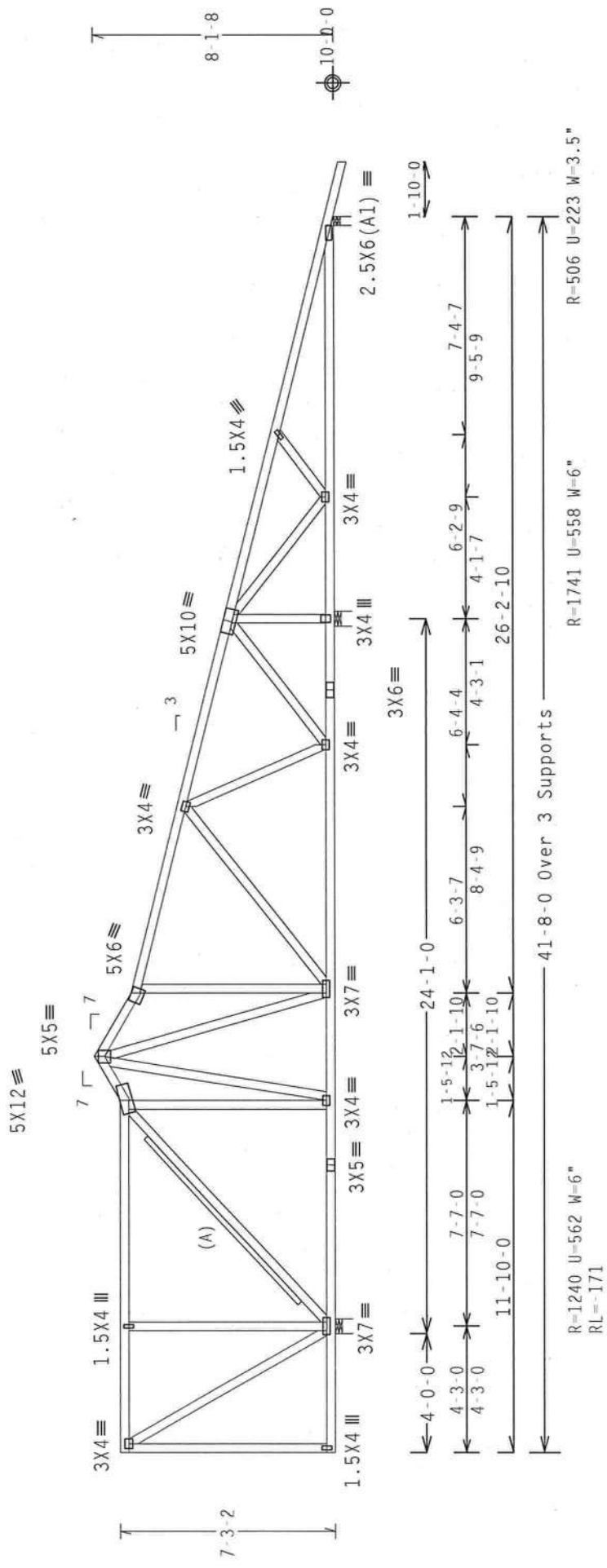
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCpl(+/-)=0.55

Wind reactions based on MMFRS pressures.

(A) 2x4 #3 or better "T" brace, 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 30.00 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .1875" / Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R8228- 29487
DATE	09/30/09
DRW	HCUSR228 09273024
HC-ENG	KD/AP
SEQN	49047
FROM	AH
JREF	1TVI8228203

WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. THE TRUSSES (INCLUDING COMPONENTS) SHOWN ON THIS DRAWING ARE THE PROPERTY OF ALPINE BUILDING COMPONENTS GROUP, INC. AND ARE TO BE USED ONLY FOR THE PROJECT AND LOCATION SPECIFIED HEREON. ANY REUSE OR MODIFICATION OF THESE TRUSSES WITHOUT THE WRITTEN CONSENT OF ALPINE BUILDING COMPONENTS GROUP, INC. IS PROHIBITED. THE USER SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AGENCIES. THE USER SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AGENCIES. THE USER SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AGENCIES.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PA) AND TPI. ITM BCG CORRELATOR PLATES ARE MADE OF 20/18/16GA (4-H/55/S) ASTM A653 GRADE 40/60 (IN. K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

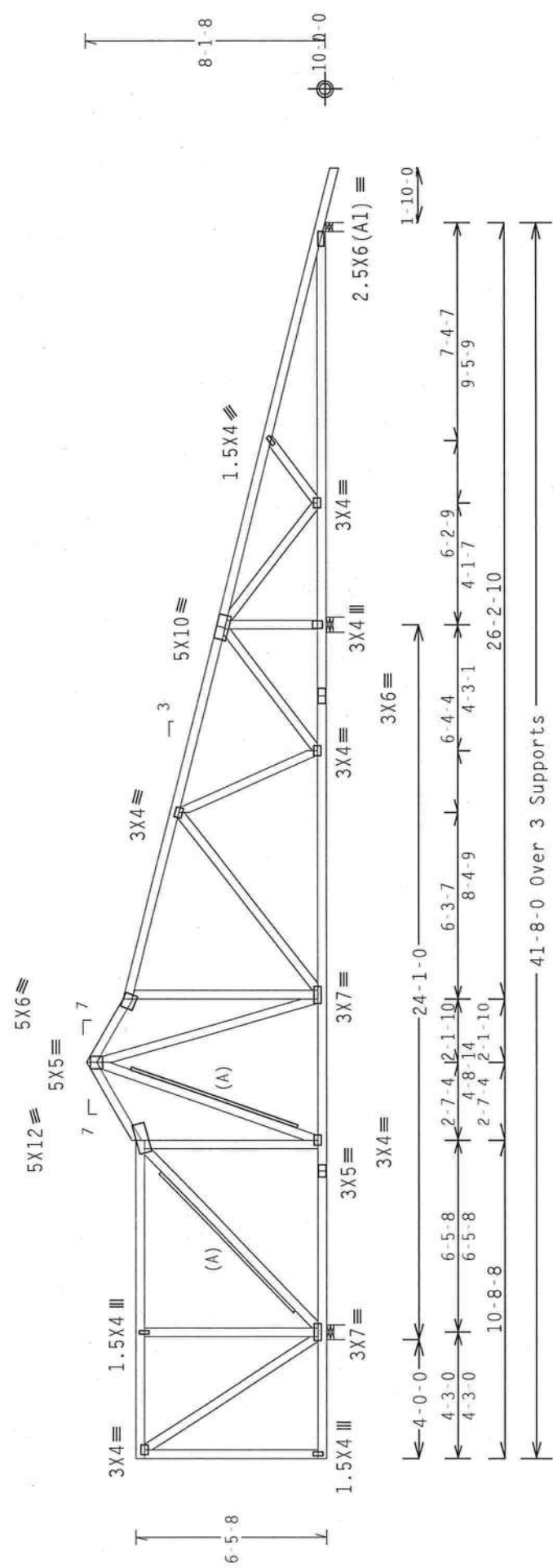
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $1w=1.00$ Gcpi (+/-)=0.55

Wind reactions based on MWFRS pressures.

(A) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5" min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 30.00 ft. from roof edge.

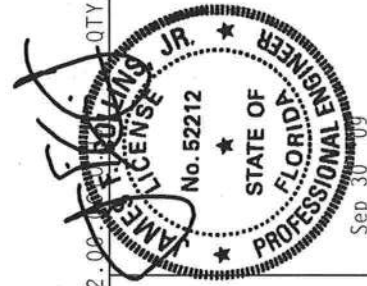


R=1742 U=559 W=6" R=506 U=223 W=3.5"

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

Scale = .1875"/Ft.

	TC LL	20.0	PSF	REF	R8228- 29488
	TC DL	10.0	PSF	DATE	09/30/09
	BC DL	10.0	PSF	DRW	HCUSR8228 09273025
	BC LL	0.0	PSF	HC-ENG	KD/AP
	TOT.LD.	40.0	PSF	SEQN -	49055
	DUR.FAC.	1.25		FROM	AH
SPACING			24.0"	JREF -	1TVI8228Z03



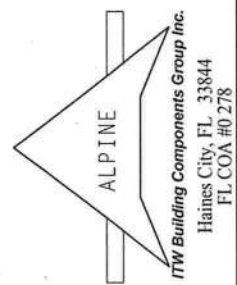
Sep 30 09

****WARNING**** TRUSSES REQUIRING EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. TRUSS DESIGNER SHALL FURNISH BUILDING COMPONENT SAFETY INFORMATION PUBLISHED BY ITW TRUSS PLATE INSTITUTE, 234 NORTH LEE STREET, SUITE 302, ALEXANDRIA, VA, 22314) AND WCA HOBAS TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, WADSWORTH, IL, 60157) TO TRUSS PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGNER COMPLIES WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., 4TH EDITION) AND TPI. ITW BCG CONNECTION PLATES ARE MADE TO 2610/1000/40/55/25/40/ASTM A572 GR50S. THIS DESIGN POSITION PER DRAWINGS 1600-2. ALL CONNECTIONS SHALL BE WELDED UNLESS OTHERWISE SPECIFIED.

AN INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER A3 OF TPI-2002 SEC. 2. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMS/TPI 1 SEC. 2.



1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART_ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC D1=5.0 psf, wind BC D1=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

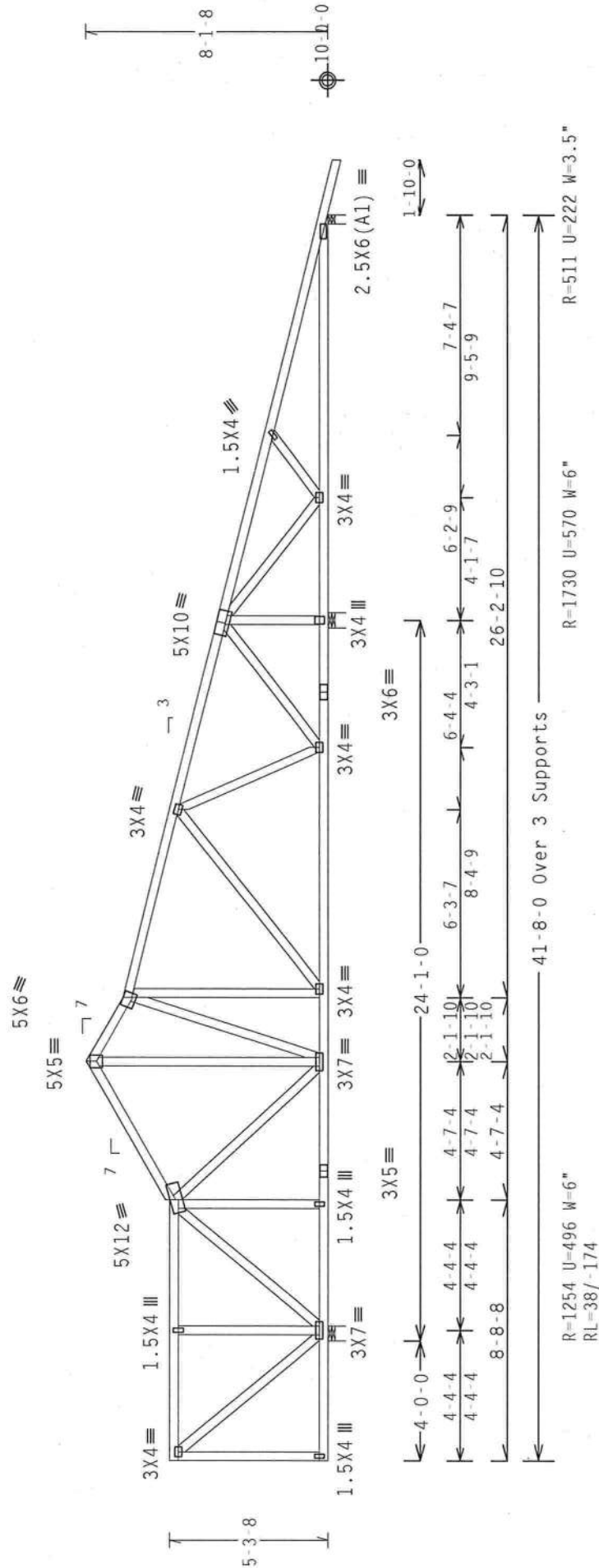
Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 30.00 ft. from roof edge.



Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=10%(0%)/0(0)

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RES3I (BUILDING COMPONENT SPECIFICATIONS), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 2300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND AISC (4000 TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE SPECIFIED, ALL TRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED ROOFING CLIPS.

Design Crit: FBC2007Res/TPI-2002 (STD)
 $FT/RT=10\%(0\%)/0(0)$

9.02.2017

OTY:1

FL1-141-1-1R1-

Scale = .1875"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE COMPANION SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 219 E. CHURCH STREET, SUITE 200, CHICAGO, ILL. 60601-2220, OR TPI (800) 392-7643, FOR TRUSS SAFETY INFORMATION. TPI EMPHASIZES THAT, UNLESS OTHERWISE SPECIFIED, ALL TRUSSES ARE DESIGNED TO PERFORM ONLY THE FUNCTIONS LISTED. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

[illegible]

TC LL	20.0	PSF
TC DL	10.0	PSF
BC DL	10.0	PSF
BC LL	0.0	PSF
TOT. LD.	40.0	PSF
DUR. FAC.	1.25	
SPACING	24.0"	

REF	R8228-	29489
DATE	09/30/09	
DRW	HCUSR8228	09273021
HC-ENG	KD/AP	
SEQN-	49062	
FROM	AH	
DATE-	11VI8228703	

Sep 30 09

JREF- 1TVI8228703

1110 mph wind, 15.00 ft mean hgt., ASCE 7-05, PART-ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, wind BC DL=5.0 GCPI(+/-)=0.55

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

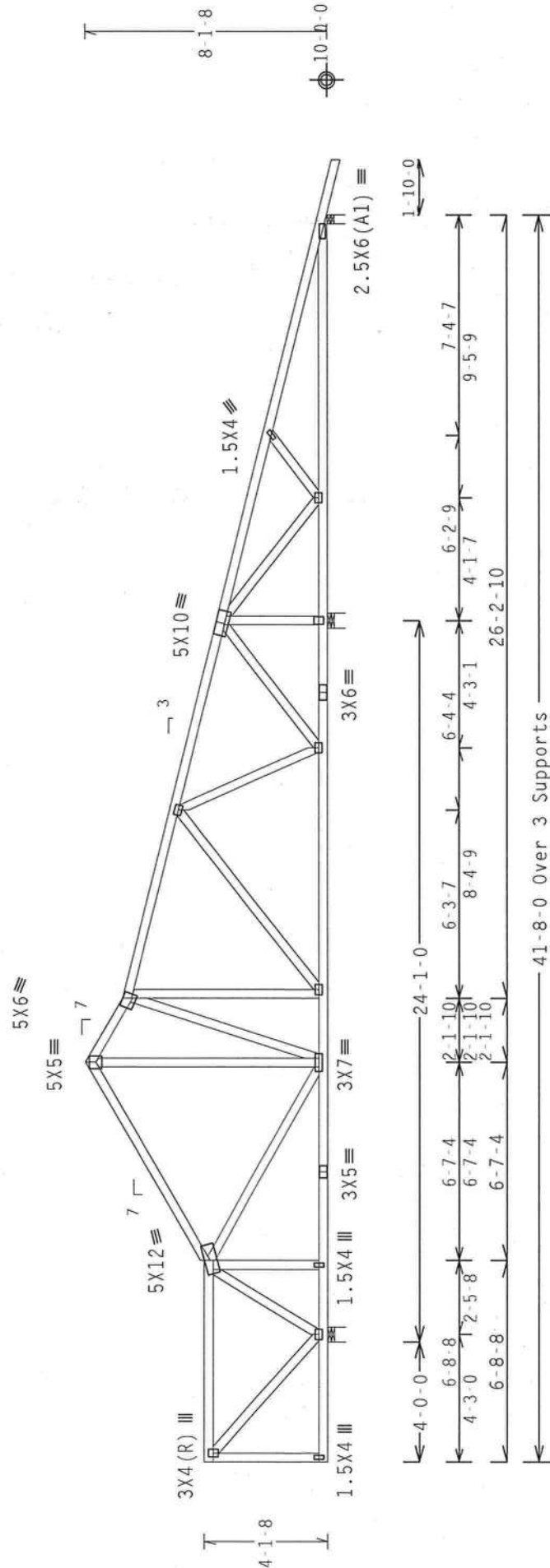
Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 30.00 ft. from roof edge.

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=1254 U=444 W=6"

RL=65/-185

Note: All Plates Are 3X4 Except As Shown.

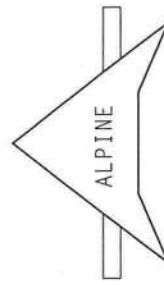
Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

PLT TYP. Wave

9.02.00

QTY:1 FL/-/4/-/-/R/-

Scale = .1875"/Ft.

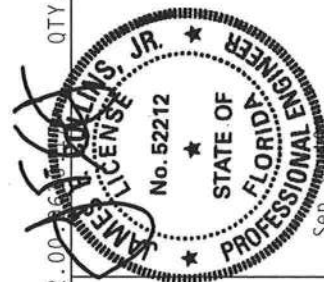


ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE FASTENING, 2300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 5300 OLD FARM ROAD, PITTSBURGH, PA, 15222). THESE PRACTICES ARE THE MINIMUM REQUIREMENTS. OTHERS MAY BE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID ENDS.

*****IMPORTANT***** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TYPE OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

CONNECTION PLATES ARE MADE OF 2019-T3 ALUMINUM (PER AISC 308) WITH THE FOLLOWING DIMENSIONS: 17U BCG CONNECTOR PLATES ARE MADE OF 2019-T3 ALUMINUM (PER AISC 308) WITH THE FOLLOWING DIMENSIONS: 17U BCG PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE NOTED ON THIS DESIGN. POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AISC A3. OF TP11-2002 SEC 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISC/TP11 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 29490
TC DL	10.0 PSF	DATE	09/30/09
BC DL	10.0 PSF	DRW	HCUSR8228 09273027
BC LL	0.0 PSF	HC-ENG	KD/AP
TOT.LD.	40.0 PSF	SEQN-	49072
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TVI8228Z03

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART-ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind ic DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

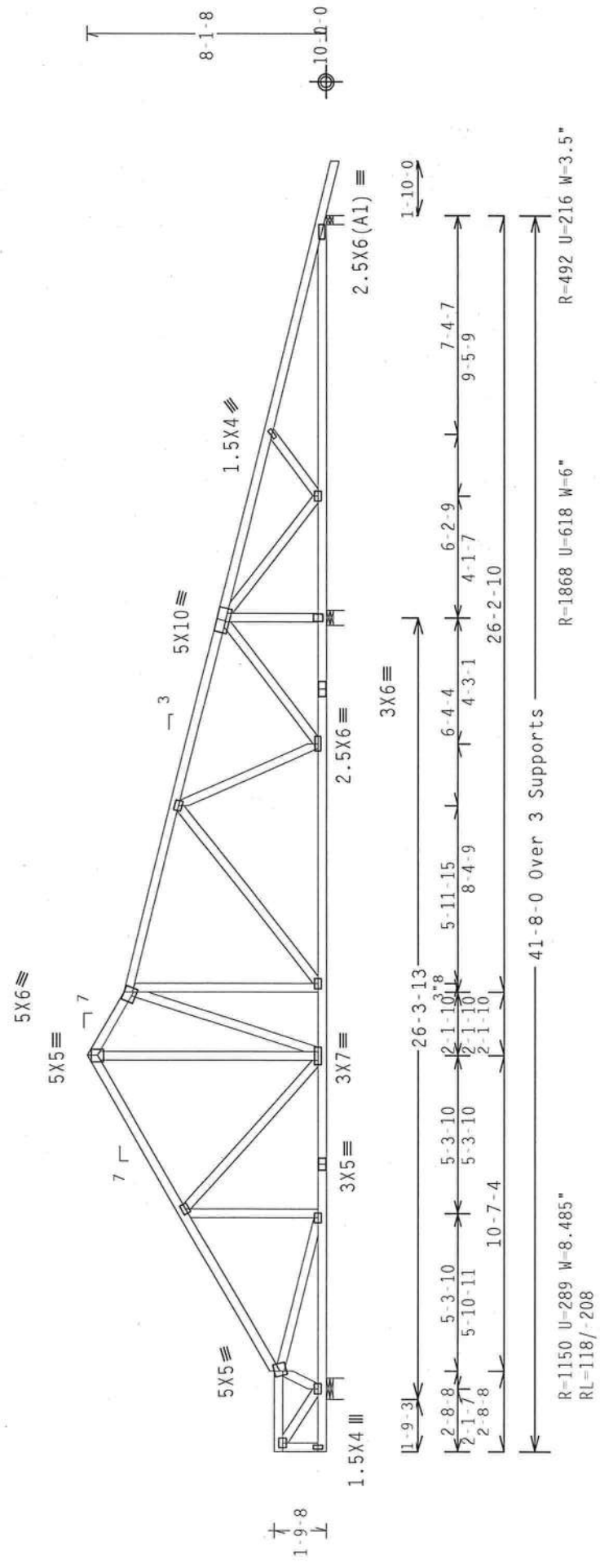
Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 30.00 ft. from roof edge.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

Scale = .1875"/Ft.

QTY:1	FL/-/4/-/-/R/-	TC LL	20.0 PSF	REF	R8228- 29492
		TC DL	10.0 PSF	DATE	09/30/09
		BC DL	10.0 PSF	DRW	HCUSR228 09273029
		BC LL	0.0 PSF	HC-ENG	KD/AP
		TOT.LD.	40.0 PSF	SEQN-	49089
		DUR.FAC.	1.25	FROM	AH
		SPACING	24.0"	JREF-	1TVI8228203

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

James F. Collins, Jr.
Professional Engineer
No. 52212
State of Florida
Sep 2009

WARNING** TRUSSES BEARING EXTREME GARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

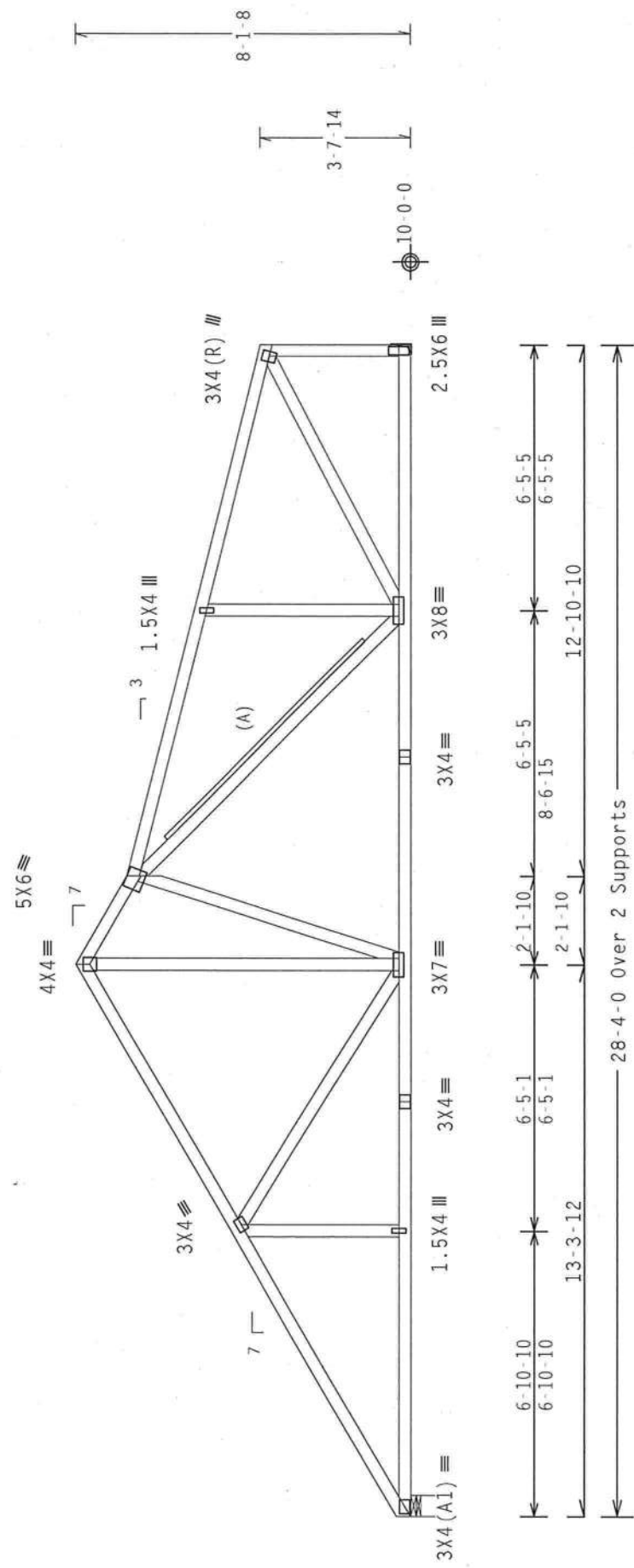
IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CORRELATOR PLATES ARE MADE OF 20/18/16GA (4.0/5.5/5.8) ASH A653 GRADE 40/60 (N. K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 300A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) DESIGN GUIDE 9. THE SUFFICIENCY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Top chord	2x4	SP #2	Dense	110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18
Bot chord	2x4	SP #2	Dense	
Webbs	2x4	SP #3		

(J) hanger connection not found in inventory file for this condition. Provide connection.

(A) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.) nails @ 6" c/c.

WVFRS loads based on trusses located at least 15.00 ft. from roof edge.



R=1177 U=126 W=6"
RL=163/-144

R=1147 U=184 H=Simpson LUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder is (3)2X0 min. (H)

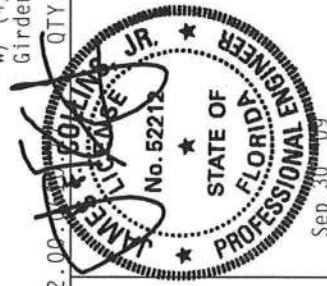
Scale = .25"/Ft.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

*****WARNING***** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, HANOVER, MI 483219). FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

*** IMPORTANT *** TENDER A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR-ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH ITPI, OR FABRICATING, WELDING, SHIPPING, INSTALLING OR BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/PSA) AND TPI1. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/25%CS) ASTM A653 GRADE 40/60 (N, K/H-55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE NOTED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANNEX A/TPI1 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 29493
TC DL	10.0 PSF	DATE	09/30/09
BC DL	10.0 PSF	DRW	HCUSR8228 09273030
BC LL	0.0 PSF	HC-ENG	KD/AP
TOT.LD.	40.0 PSF	SEQN-	49097
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TVI8228Z03

Negative reaction(s) of -385# MAX. (See below) from a non-wind load case requires uplift connection.

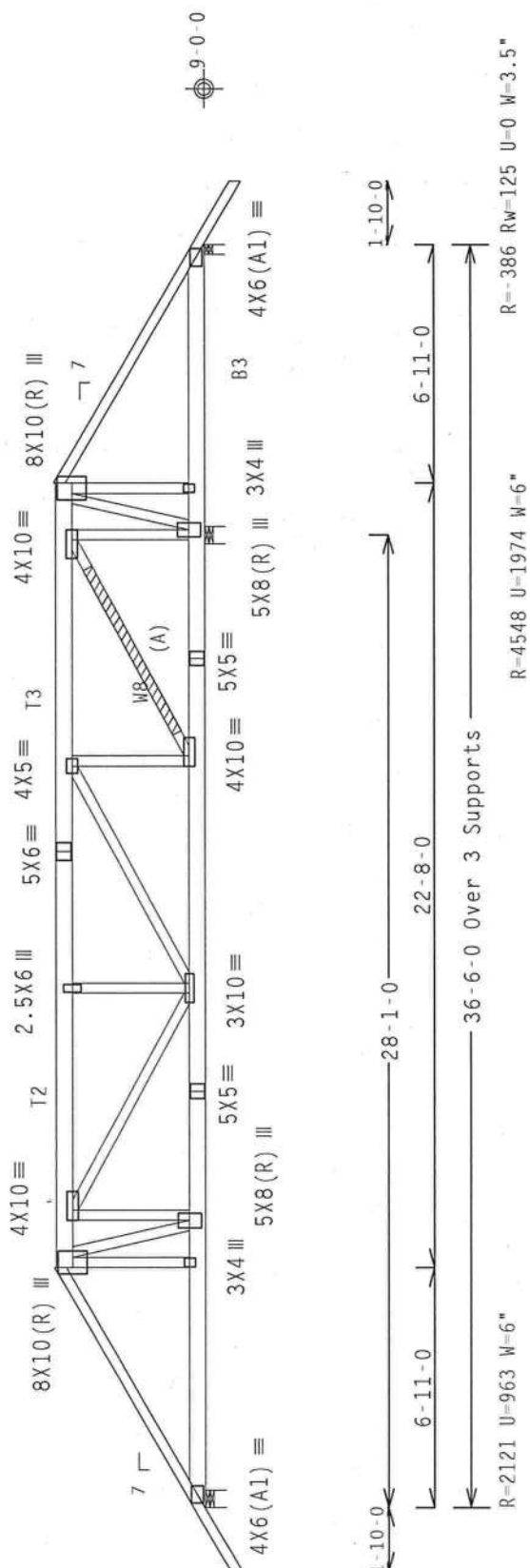
Top chord 2x4 SP #2 Dense :12, T3 2x6 SP #1 Dense:
 Bot chord 2x6 SP #2 :B3 2x6 SP #1 Dense:
 Webs 2x4 SP #3 :W8 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

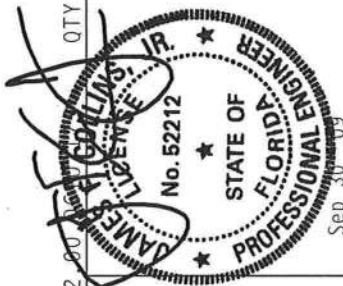
Deflection meets L/240 live and L/180 total load.



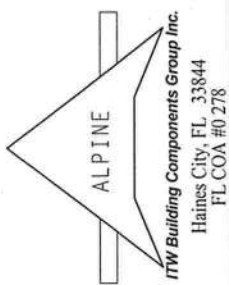
Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

QTY:1 FL/-/4/-/-/R/-
Scale =.1875"/Ft.

TC LL	20.0	PSF	REF	R8228 - 29494
TC DL	10.0	PSF	DATE	09/30/09
BC DL	10.0	PSF	DRW	HCUSR8228 09273030
BC LL	0.0	PSF	HC-ENG	JB/DF
TOT.LD.	40.0	PSF	SEQN-	46965
DUR.FAC.	1.25		FROM	AH
SPACING	24.0"		JREF-	1TVI8228Z03



*****IMPORTANT***** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN, IF ANY FAILURE TO BUILD THE TROSS IN CONFORMANCE WITH THE DRAWINGS, WITHIN THE SPECIFIC PROVISIONS OF THIS NATIONAL DESIGN SPEC., BY A/FRPA AND TP1. ITW BCG CORRELATOR PLATES ARE MADE OF 2018/166GA. (4.45/55)PC ASTM A653 GRADE 40/60 (K. 40/55) GALV. STEEL, APPLY PLATES TO EACH FACE OF TROSS AND, OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX 43 OF 1911-2002 SEC.2. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TROSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AM51/TP1 1 SEC. 2.



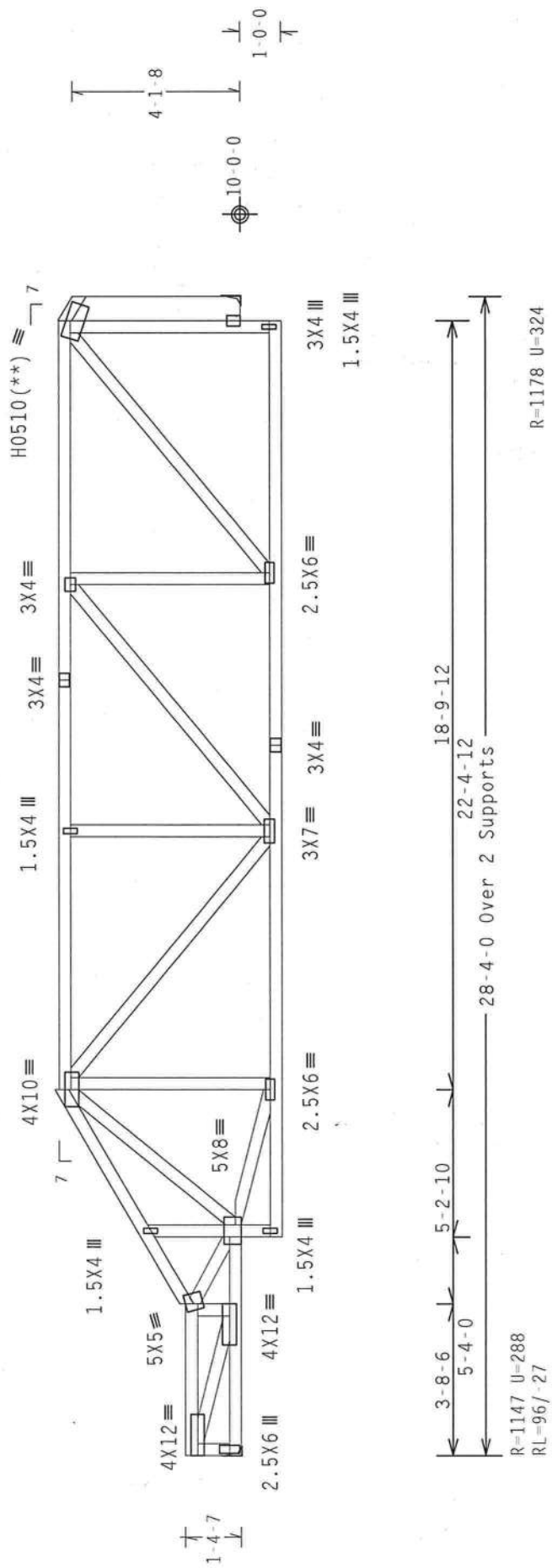
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gpsi (+/-)=0.18

blind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



ULT TYP. 20 Gauge HS.Wave
Design Crit: FBC2007Res/TPI-2002 (STD)
FT/R=10%(0%) / 0(0)

Scale = .25"/Ft.

TC LL	20.0 PSF	REF R8228 - 29495
TC DL	10.0 PSF	DATE 09/30/09
BC DL	10.0 PSF	DRW HCUSR8228 09273011
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN - 47119
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF - 1TVI8228Z03

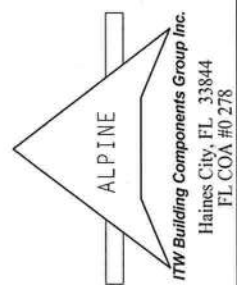


****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY ITW TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, HOUSTON, TX 77061, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-1 OR TABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION) CODES AND 16/19/20 U.S. NATIONAL DESIGN SPEC. BY AISC AND TPI-1.

CONNECTIONS WITH PLATES ARE TO BE MADE IN ACCORDANCE WITH THE FOLLOWING POSITION PER FIGURES 160A-2, 160A-3, 160A-4, 160A-5, 160A-6, 160A-7, 160A-8, 160A-9, 160A-10, 160A-11, 160A-12, 160A-13, 160A-14, 160A-15, 160A-16, 160A-17, 160A-18, 160A-19, 160A-20, 160A-21, 160A-22, 160A-23, 160A-24, 160A-25, 160A-26, 160A-27, 160A-28, 160A-29, 160A-30, 160A-31, 160A-32, 160A-33, 160A-34, 160A-35, 160A-36, 160A-37, 160A-38, 160A-39, 160A-40, 160A-41, 160A-42, 160A-43, 160A-44, 160A-45, 160A-46, 160A-47, 160A-48, 160A-49, 160A-50, 160A-51, 160A-52, 160A-53, 160A-54, 160A-55, 160A-56, 160A-57, 160A-58, 160A-59, 160A-60, 160A-61, 160A-62, 160A-63, 160A-64, 160A-65, 160A-66, 160A-67, 160A-68, 160A-69, 160A-70, 160A-71, 160A-72, 160A-73, 160A-74, 160A-75, 160A-76, 160A-77, 160A-78, 160A-79, 160A-80, 160A-81, 160A-82, 160A-83, 160A-84, 160A-85, 160A-86, 160A-87, 160A-88, 160A-89, 160A-90, 160A-91, 160A-92, 160A-93, 160A-94, 160A-95, 160A-96, 160A-97, 160A-98, 160A-99, 160A-100, 160A-101, 160A-102, 160A-103, 160A-104, 160A-105, 160A-106, 160A-107, 160A-108, 160A-109, 160A-110, 160A-111, 160A-112, 160A-113, 160A-114, 160A-115, 160A-116, 160A-117, 160A-118, 160A-119, 160A-120, 160A-121, 160A-122, 160A-123, 160A-124, 160A-125, 160A-126, 160A-127, 160A-128, 160A-129, 160A-130, 160A-131, 160A-132, 160A-133, 160A-134, 160A-135, 160A-136, 160A-137, 160A-138, 160A-139, 160A-140, 160A-141, 160A-142, 160A-143, 160A-144, 160A-145, 160A-146, 160A-147, 160A-148, 160A-149, 160A-150, 160A-151, 160A-152, 160A-153, 160A-154, 160A-155, 160A-156, 160A-157, 160A-158, 160A-159, 160A-160, 160A-161, 160A-162, 160A-163, 160A-164, 160A-165, 160A-166, 160A-167, 160A-168, 160A-169, 160A-170, 160A-171, 160A-172, 160A-173, 160A-174, 160A-175, 160A-176, 160A-177, 160A-178, 160A-179, 160A-180, 160A-181, 160A-182, 160A-183, 160A-184, 160A-185, 160A-186, 160A-187, 160A-188, 160A-189, 160A-190, 160A-191, 160A-192, 160A-193, 160A-194, 160A-195, 160A-196, 160A-197, 160A-198, 160A-199, 160A-200, 160A-201, 160A-202, 160A-203, 160A-204, 160A-205, 160A-206, 160A-207, 160A-208, 160A-209, 160A-210, 160A-211, 160A-212, 160A-213, 160A-214, 160A-215, 160A-216, 160A-217, 160A-218, 160A-219, 160A-220, 160A-221, 160A-222, 160A-223, 160A-224, 160A-225, 160A-226, 160A-227, 160A-228, 160A-229, 160A-230, 160A-231, 160A-232, 160A-233, 160A-234, 160A-235, 160A-236, 160A-237, 160A-238, 160A-239, 160A-240, 160A-241, 160A-242, 160A-243, 160A-244, 160A-245, 160A-246, 160A-247, 160A-248, 160A-249, 160A-250, 160A-251, 160A-252, 160A-253, 160A-254, 160A-255, 160A-256, 160A-257, 160A-258, 160A-259, 160A-260, 160A-261, 160A-262, 160A-263, 160A-264, 160A-265, 160A-266, 160A-267, 160A-268, 160A-269, 160A-270, 160A-271, 160A-272, 160A-273, 160A-274, 160A-275, 160A-276, 160A-277, 160A-278, 160A-279, 160A-280, 160A-281, 160A-282, 160A-283, 160A-284, 160A-285, 160A-286, 160A-287, 160A-288, 160A-289, 160A-290, 160A-291, 160A-292, 160A-293, 160A-294, 160A-295, 160A-296, 160A-297, 160A-298, 160A-299, 160A-300, 160A-301, 160A-302, 160A-303, 160A-304, 160A-305, 160A-306, 160A-307, 160A-308, 160A-309, 160A-310, 160A-311, 160A-312, 160A-313, 160A-314, 160A-315, 160A-316, 160A-317, 160A-318, 160A-319, 160A-320, 160A-321, 160A-322, 160A-323, 160A-324, 160A-325, 160A-326, 160A-327, 160A-328, 160A-329, 160A-330, 160A-331, 160A-332, 160A-333, 160A-334, 160A-335, 160A-336, 160A-337, 160A-338, 160A-339, 160A-340, 160A-341, 160A-342, 160A-343, 160A-344, 160A-345, 160A-346, 160A-347, 160A-348, 160A-349, 160A-350, 160A-351, 160A-352, 160A-353, 160A-354, 160A-355, 160A-356, 160A-357, 160A-358, 160A-359, 160A-360, 160A-361, 160A-362, 160A-363, 160A-364, 160A-365, 160A-366, 160A-367, 160A-368, 160A-369, 160A-370, 160A-371, 160A-372, 160A-373, 160A-374, 160A-375, 160A-376, 160A-377, 160A-378, 160A-379, 160A-380, 160A-381, 160A-382, 160A-383, 160A-384, 160A-385, 160A-386, 160A-387



(1) - plates so marked were sized using a Fabrication Tolerance of 0% and a Rotational Tolerance of 0 degrees.

**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

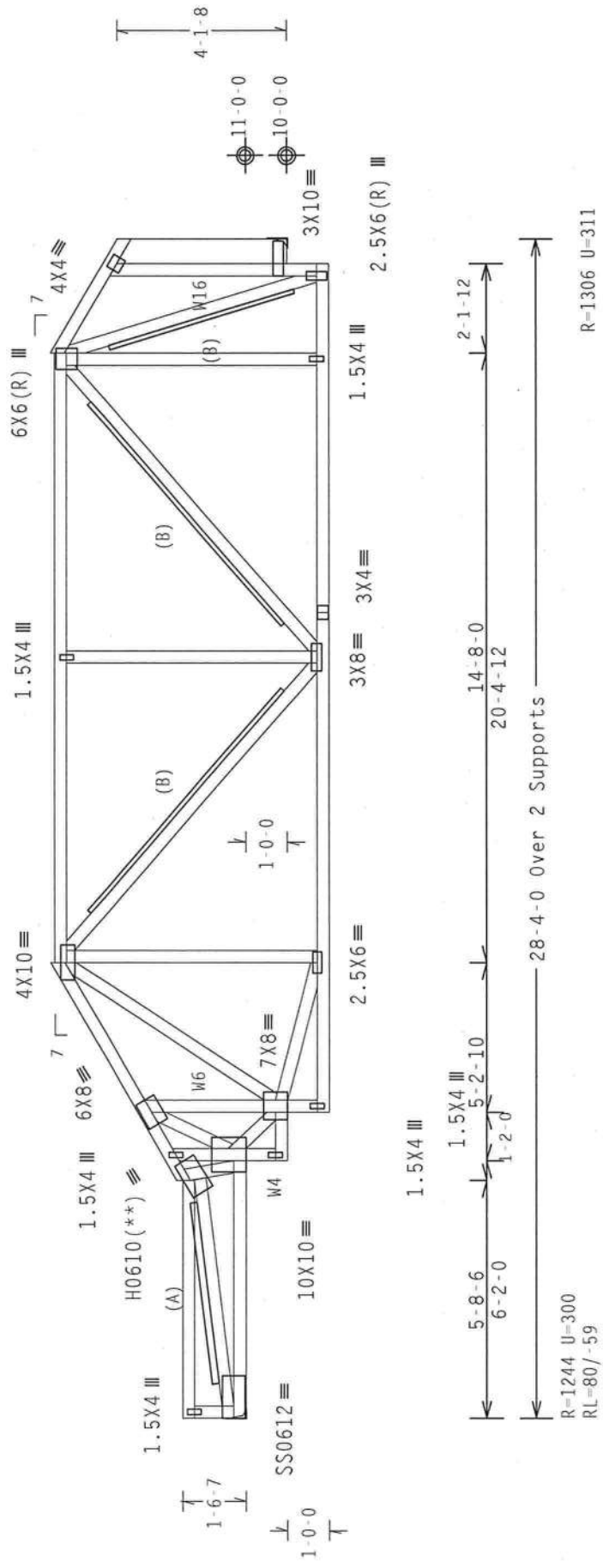
10 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi(+/-)=0.18

n lieu of structural panels use purlins to brace all flat TC @ 4" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Reflection meets L/240 live and L/180 total load.

UNIFORMS loads based on trusses located at least 7.50 ft. from roof edge.



PLT TYP. 20 Gauge HS, 18 Gauge HS, Wave
Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

Scale = .25"/Ft.

QTY:1 FL/-/4/-/-/R/-/

~~00.06.2000~~

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST SOUTHLING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2100 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND NFCA (NATIONAL TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, HANCOCK, MI 483219) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

TC LL	20.0 PSF
-------	----------

REF R8228- 29496

DATE 09/30/09

CO/OC/EO

JKW HCUSR8228 0

HC-ENG JB/DF

SEQN- 4

FROM AH

SPACING 24 0"

1BFF- 1TVI8228703



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

(9-136--Doug Edgley Tuwana Rossin -- , ** - H13B)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W18 2x4 SP #2 Dense:
:Rt Bearing Leg 2x8 SP #1 Dense:

Left end vertical not exposed to wind pressure.

(A) 1x4 #3SRB SPF-S or better "I" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

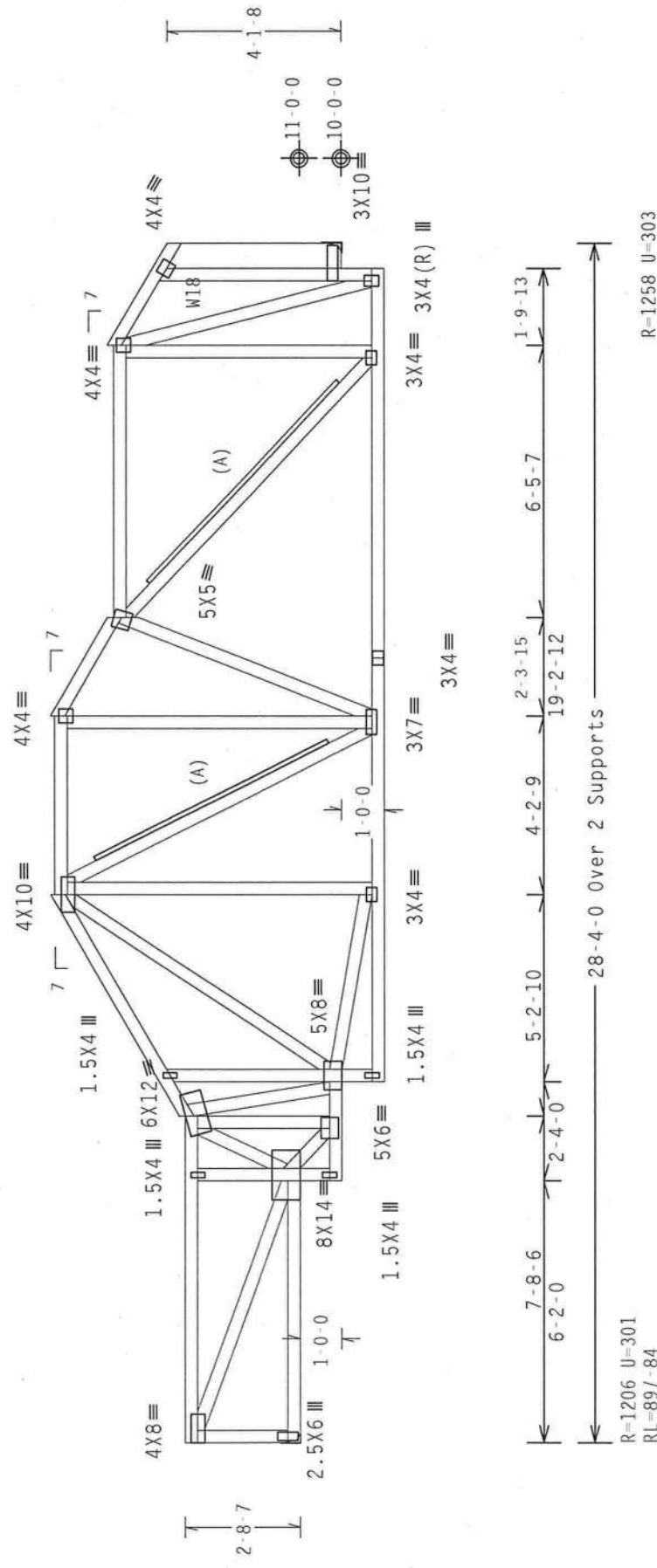
Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.65 ft. from roof edge.

Wind reactions based on MWFRS pressures.

Truss passed check for 20 psf additional bottom chord live load in areas with 42" high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .25"/Ft.

QTY:1

FL/-4/-/-R/-

TC LL	20.0 PSF	REF	R8228- 29497
TC DL	10.0 PSF	DATE	09/30/09
BC DL	10.0 PSF	DRW	HCUSR8228 09273015
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	47193
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TVI8228Z03

WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSS (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITM BCG CONTRACTOR PLATES ARE MADE OF 20/10/10GA (0.41/0.55/7X) ASTM A653 GRADE 40/60 (40, 42H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF EACH TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING. THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

JAMES F. DOLANSKY JR.
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Sep 2009

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

(9-136--Doug Edgley Tuwana Rossin -- ** - H15B)

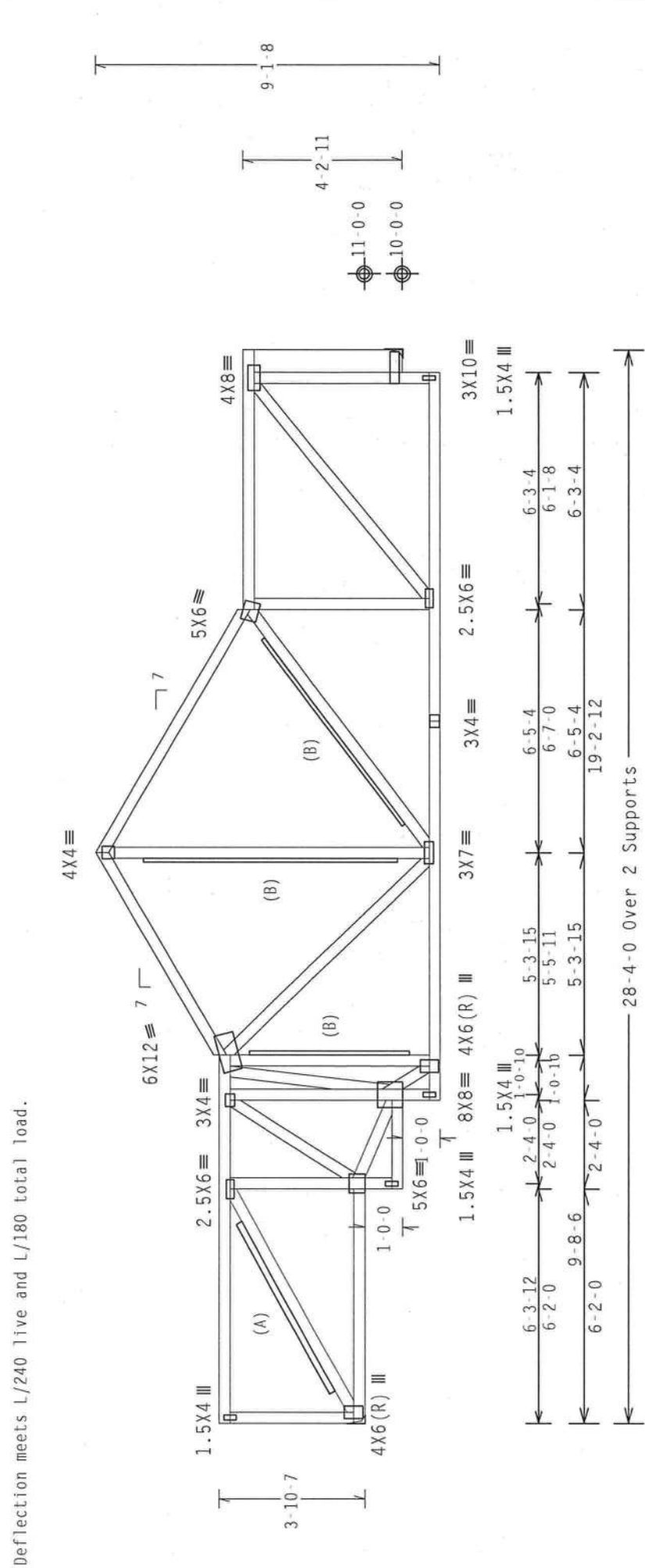
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Rt Bearing Leg 2x8 SP #1 Dense:

Left end vertical not exposed to wind pressure.

(B) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R-1172 U=214
RL=100/-110 H-Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Truss

PLT TYP. WaveGirder is (3)2X0 min. (H)

Scale = .25" / Ft.

QTY: 1

FL / - / 4 / - / R / -

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF R8228- 29498
DATE 09/30/09
DRW HCUSR8228 09273033
HC-ENG KD/AP
SEQN- 48783
FROM AH
JREF- 1TVI8228203

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

SEP 2009
JAMES E. COLLINS JR.
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCGI BUILDING COMPONENT GROUP SAFETY INFORMATION, PUBLISHED BY ITW TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WPCA (4000) TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 48219. FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT: FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCGI CONNECTION PLATES ARE MADE OF 2018/16GA (0.0156") ASP 6053 GRADE 40/60 (0.4% K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DIAGRAMS 100A-2. ITW BCGI CONNECTION PLATES ARE MADE OF 2018/16GA (0.0156") ASP 6053 GRADE 40/60 (0.4% K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DIAGRAMS 100A-2. ITW BCGI CONNECTION PLATES ARE MADE OF 2018/16GA (0.0156") ASP 6053 GRADE 40/60 (0.4% K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DIAGRAMS 100A-2. ITW BCGI CONNECTION PLATES ARE MADE OF 2018/16GA (0.0156") ASP 6053 GRADE 40/60 (0.4% K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DIAGRAMS 100A-2. ITW BCGI CONNECTION PLATES ARE MADE OF 2018/16GA (0.0156") ASP 6053 GRADE 40/60 (0.4% K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DIAGRAMS 100A-2.

(9-136--Doug Edgley Tuwana Rossin -- ** - B1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Rt Bearing Leg 2x8 SP #1 Dense:

Left end vertical not exposed to wind pressure.

(B) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

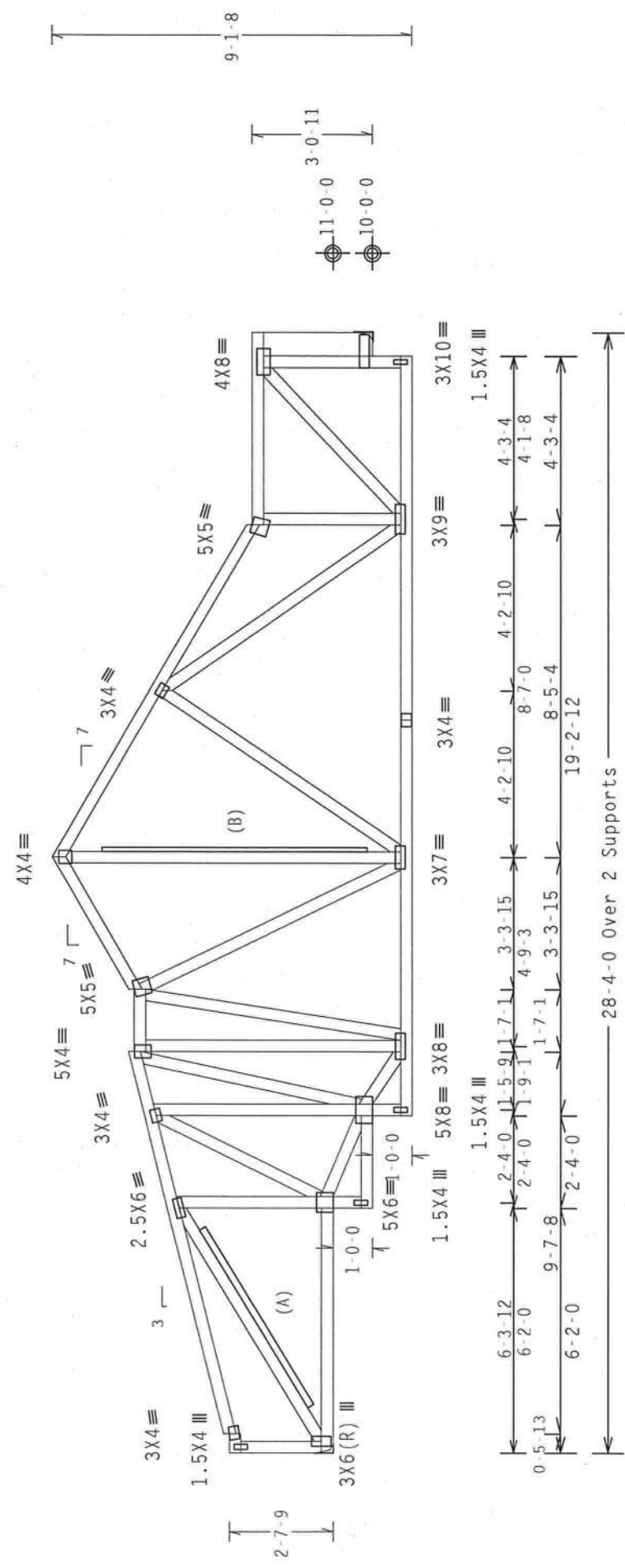
110 mph wind, 15.59 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpl(+/-)-0.18

Wind reactions based on MMFRS pressures.

(A) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 15.59 ft. from roof edge.



R-1147 U=183
RL=128/-110 H-Simpson LUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Truss

PLT TYP. Wave Girder is (3)2X0 min. (H) FT/RT=10%(0%)/0(0) 9.02.00

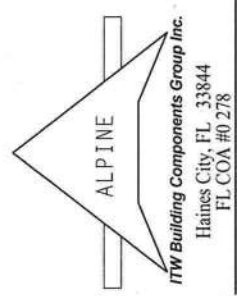
R-1155 U=117

Scale = .25" / Ft.		REF	R8228 - 29499
QTY: 1		DATE	09/30/09
FL / - / 4 / - / R / -		DRW	HCUSR8228 09273035
		HC-ENG	KD/AP
		SEQN	48811
		FROM	AH
		JREF	1TVI8228203
		TC LL	20.0 PSF
		TC DL	10.0 PSF
		BC DL	10.0 PSF
		BC LL	0.0 PSF
		TOT.LD.	40.0 PSF
		DUR.FAC.	1.25
		SPACING	24.0"



WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND METAL TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF RDS, NATIONAL DESIGN SPEC. BY ACPA AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/16GA (4-H/SS/A) ASTM A653 GRADE 40/60 (4, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS T60A 2. PLATE LOCATIONS PLATE LOCATIONS SHALL BE IDENTIFIED BY THE TRUSS DESIGNER. ITW BCG SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



110 mph wind, 15.01 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf Iw=1.00 GCpi(+/-)=0.18

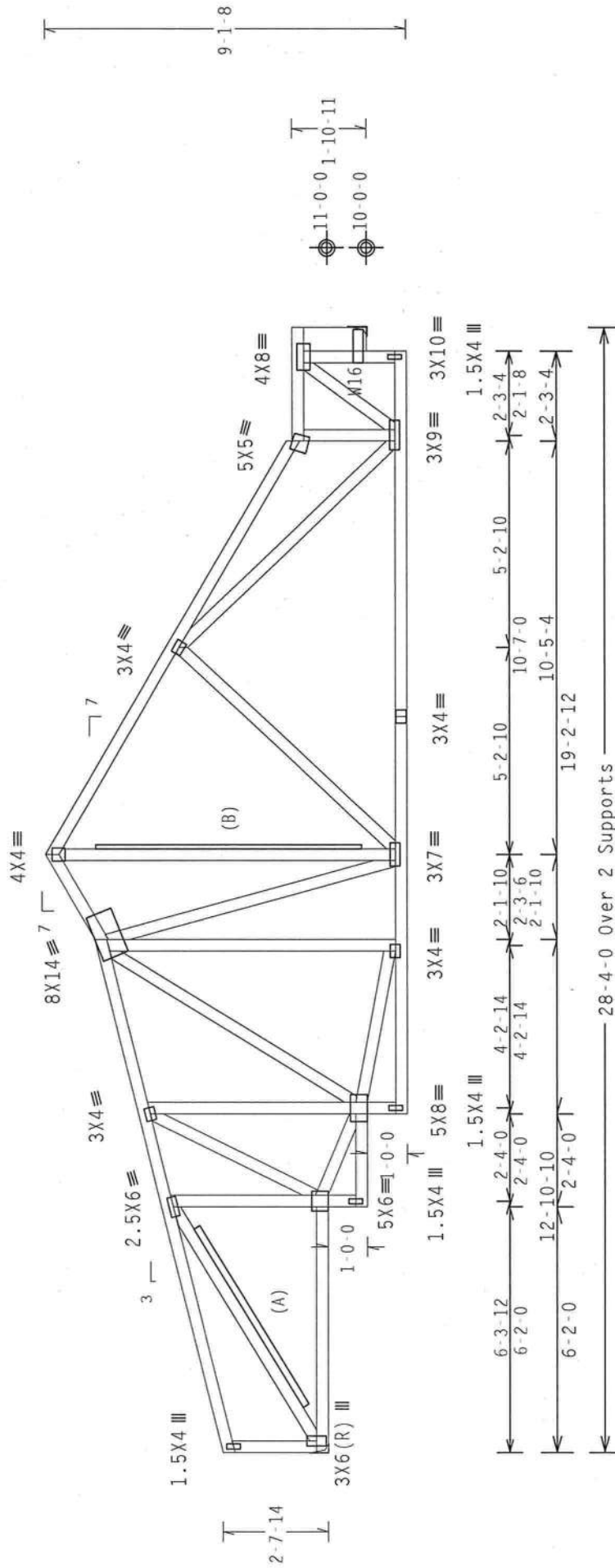
Wind reactions based on MWFRS pressures.

(A) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 15.01 ft. from roof edge.

Deflection meets L/240 live and L/180 total load.



R=1146 U=180

RL=136/-125 H=Simpson LUS26

w/ (4) 10d Common. 0.148"x3.0" nails in Truss

PIT TYP WaveGirder is (3)2X0 min. (H)
 w/ (4) 10d Common, 0.148"x3" Des^gign I & F: F6C2007Res/TPI-2002 (STD)
 FT/RT=10%(0%)/0(0)

9.02.00. ~~Standard~~ OTY: 1 FL / - / 4 / - / - / R / - Scale = .25" / Ft.

R=1158 U=124

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RETURN TO BESTI BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 500 N. MICHIGAN, SUITE 1300, CHICAGO, IL 60610-4000) FOR THE LATEST DESIGN PRACTICES. ALL TRUSSES MUST HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE CORROSION RESISTANT PLATING.

****IMPORTANT*****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH DETAIL OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGNER SHALL COMPLY WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., IV. A5.2.1) AND TPI. TPI OR FABRICATOR, HANDLING, SHIPPING, INSTALLING, AND BRACING OF TRUSSES.

CONNECTION PLATES ARE MADE OF 70/18/16GA (N/5/SS/K) ASTM A653 GRADE 40/60 (N, K/11/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. A SEAL ON THIS AN INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER 43 OF TPI-1/2002 SEC. 3.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN.

THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISC/TPI 1 SEC. 2.



TC LL	20.0	PSF	REF	R8228	29500
TC DL	10.0	PSF	DATE	09/30/09	
BC DL	10.0	PSF	DRW	HCHSR8228	09273038
BC LL	0.0	PSF	HC-ENG	KD/AP	
TOT.LD.	40.0	PSF	SEON-	48825	
DUR.FAC.	1.25		FROM	AH	
SPACING	24.0"		JREF-	1TV18228203	

	Top	chord	2x4	SP	#2	Dense	:T3	2x8	SP	#1	Dense:
Bot	chord	2x4	SP	#2	Dense						
	Web	2x4	SP	#3							

Left end vertical not exposed to wind pressure.

(B) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5".min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

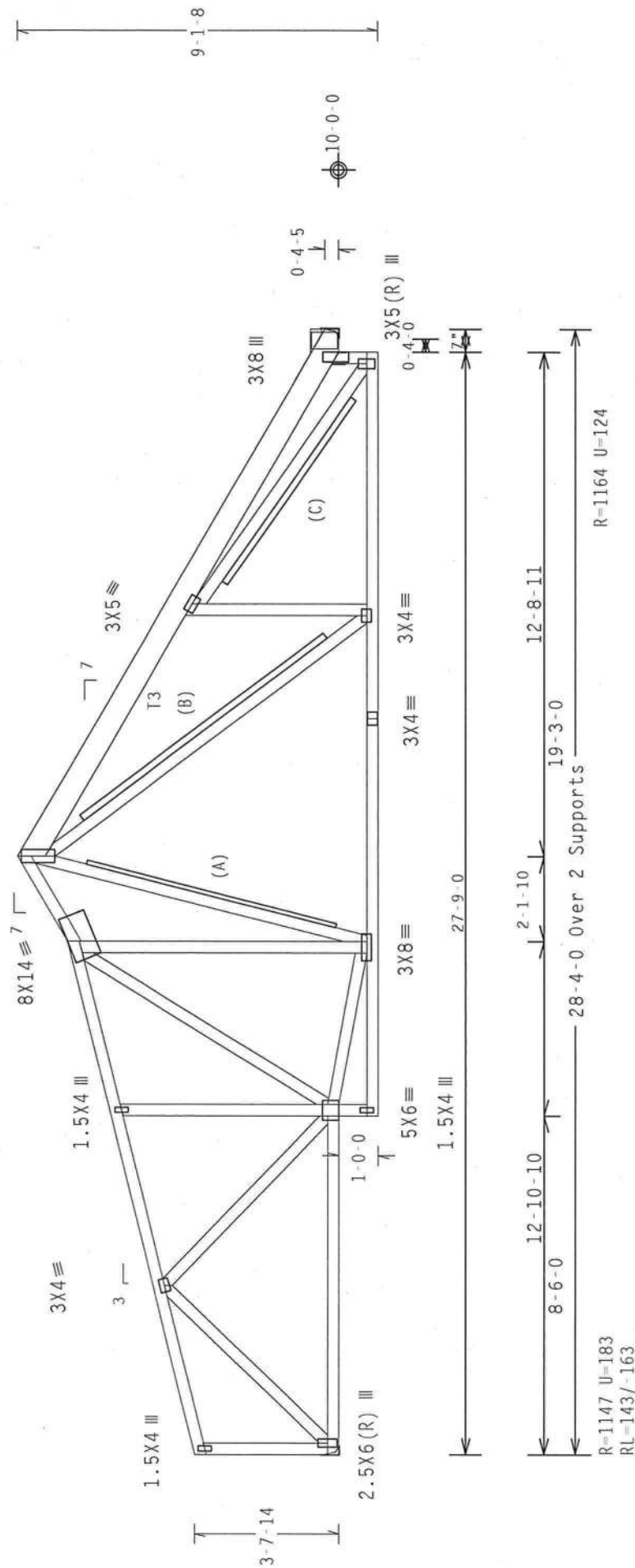
Wind reactions based on MWFRS pressures.

(A) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5".min.)nails @ 6" OC.

(C) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

4X10(R) III



R=1147 U=183
RL=143/-163

Design Crit: FBC2007Res/TPI - 2002(STD)
FT/RT=10%(0%) / 0(0)

PLT TYP. Wave

FL/-14/-1-1R/-

Scale = .25"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE COMPONENT INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION), INC., 500 N. MICHIGAN, CHICAGO, ILL. 60610, FOR THE LATEST RECOMMENDED PRACTICES FOR PERFORMING THESE OPERATIONS. UNLESS PROPERLY ATTACHED, CHORDS SHALL HAVE PROPERLY ALIGNED SYMMETRICAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED, BUILT-UP CHORDS.

TC LL	20.0 PSF	REF	R8228 - 29501
TC DL	10.0 PSF	DATE	09/30/09

****IMPORTANT****—FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR FOR ITR REG. INFO. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TYPE OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN COMPATIBLE WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AREA) AND TPI-1. ITR REG. DESIGN COMPATIBLE WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AREA) AND TPI-1. ITR REG. PLATES TO EACH FACE OF TRUSSES AND UNLESS OTHERWISE LOCATED ON THIS POSITION PER DRAWINGS 160A-2. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT OF THIS BUILDING. DESIGNER PER AREA 43 OF TPI-1 2002 SEC. 2. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT OF THIS BUILDING. DESIGNER PER AREA 43 OF TPI-1 2002 SEC. 2.

DRW	HC03R02620	0527306
HC-ENG	DR/AP	
BC LL	0.0	PSF
TOT.LD.	40.0	PSF
DUR.FAC.	1.25	
FROM	AH	
SPACING	24	0"
REF	11VI8228202	

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL-5.0 psf, wind BC DL-5.0 psf. Iw=1.00 GCpi(+/-)-0.18

Roof overhang supports 2.00 psf soffit load.

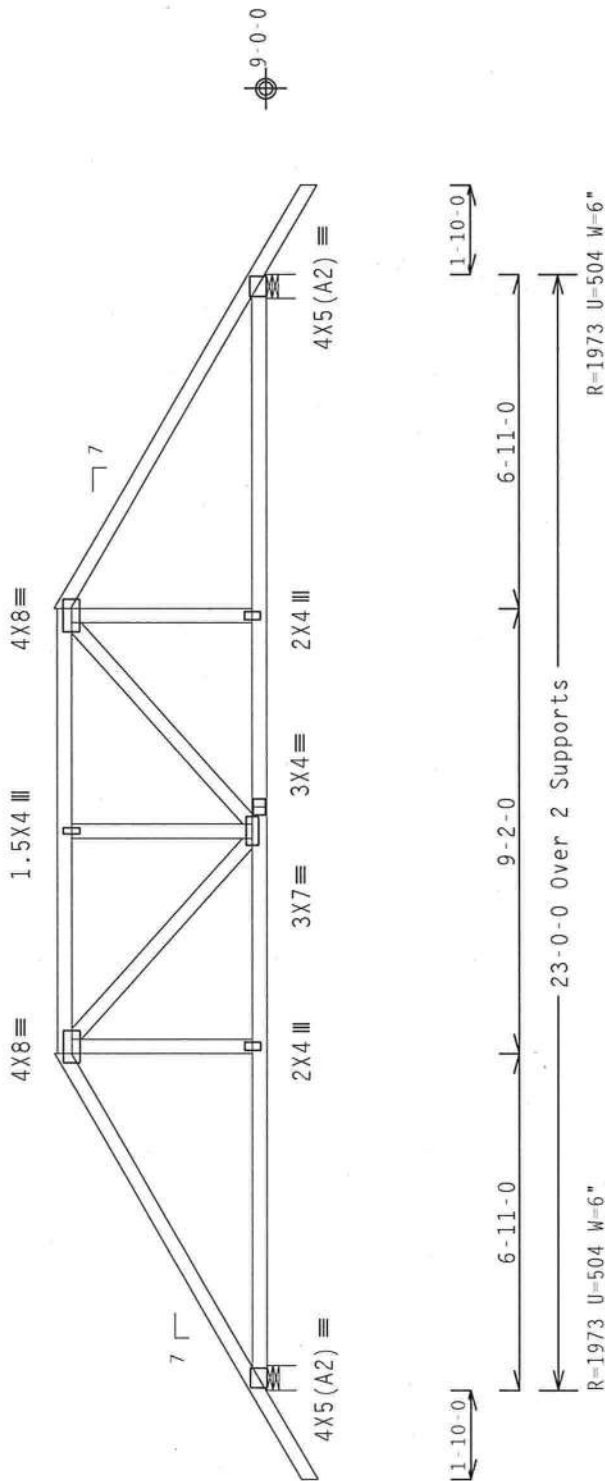
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Wind reactions based on MWFRS pressures.

#1 hip supports 7-0-0 jacks with no webs.

Left side jacks have 7-0-0 setback with 0-0-0 cant and 1-10-0 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 1-10-0 overhang. Right side jacks have 7-0-0 setback with 0-0-0 cant and 1-10-0 overhang.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

Scale = .25"/Ft.

WARNING: TRUSSES ROUTING EXTERIOR CORE IN ADDITION, HANDLING, SHIPPING, INSTALLING AND BRACING TRUSSES MUST BE DONE BY A QUALIFIED PERSON. COMPONENT SAFETY INFORMATION PUBLISHED BY THE MANUFACTURER MUST BE READ AND UNDERSTOOD. TRUSS COMPANY, INC., 10000 W. 10TH AVENUE, SUITE 312, ALEXANDRIA, VA 22314, AND AISC, TRUSS COUNCIL OF AMERICA, 6300 NORTH LEE STREET, MADISON, WI 53719. TRUSS PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

REF R8228- 29502

DATE 09/30/09

0111 02/00/00
0011 00110020 0027300

ORW HCU3K0228 0527300

HC-ENG JB/DF

46345 SEQN-

FROM AH

REF- 1TVI8228Z03

Sep 30 '09

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0370

	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

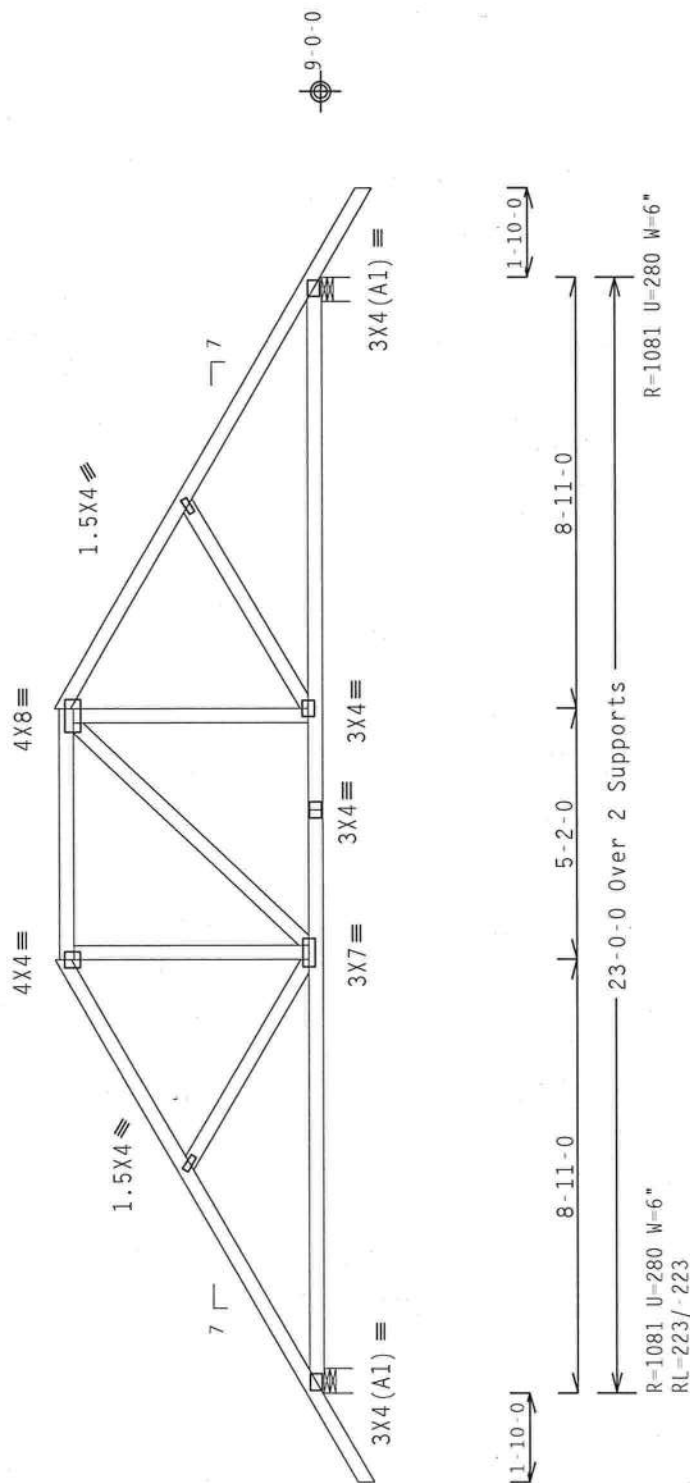
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

24" OC.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

Scale = .25"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND DRAGING. REFER TO BCSJ (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 219 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

REF R8228- 29503

DATE 09/30/09

ORW HCUSR8228 0927300

HC-ENG JB/DF

SF0N- 46311

FROM AH

REF- 1TVI8228703

Haines City, FL 33844
FL COA #0 278

Sep

	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x6	SP	#2		
		webs	2x4	SP	#3	

** The maximum horizontal reaction is 269# **

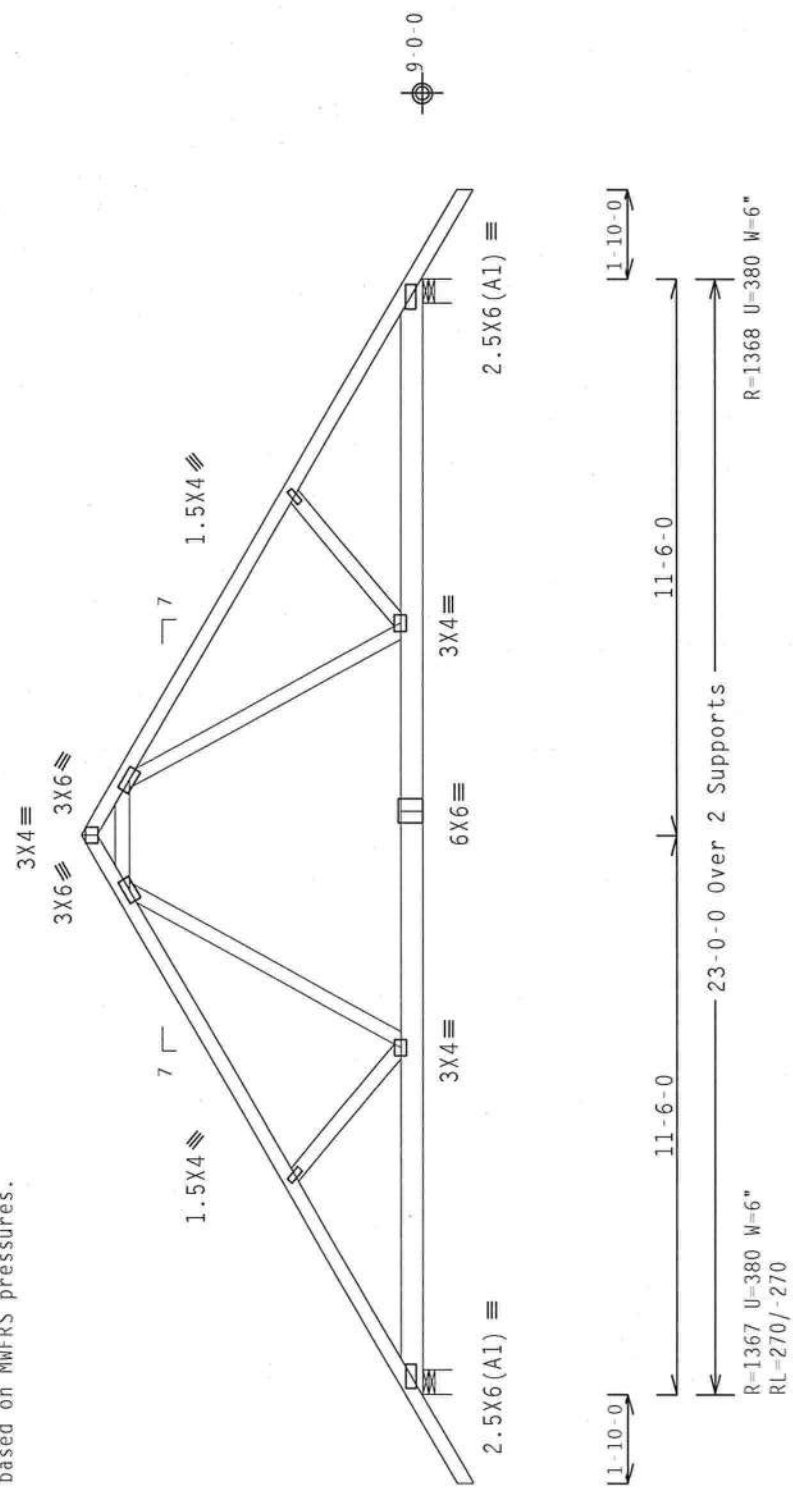
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ Gcpi (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load.

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FABRICATOR TO REVIEW THIS DWG PRIOR TO CUTTING LUMBER TO VERIFY THAT ALL DATA, INCLUDING DIMENSIONS AND LOADS, CONFORM TO THE ARCHITECTURAL PLANS/SPECIFICATIONS AND FABRICATOR'S TRUSS LAYOUT

Wind reactions based on MWFRS pressures.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .25"/ft.

QTY:15 FL/-/4/-/-/R/-

00:00:00

 $9.02.$

FT/RT=10% (0%) /

Design of 12:

YP. Wave

PLT T

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. (SEE NOTES) TRUSSES MUST BE PROPERLY AND CAREFULLY HANDLED BY THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO TRUSSES DUE TO IMPROPER HANDLING, SHIPPING, INSTALLING OR BRACING. TRUSSES ARE TO BE FABRICATED, HANDLING, SHIPPING, INSTALLING AND BRACING TO THE FOLLOWING DESIGN CONDITIONS:

DESIGN CONDITIONS WITH APPLICABLE CODES: (1) AISC 360 (STEEL DESIGN), (2) AISC 358 (WELDED CONNECTIONS), (3) AISC 308 (BOLTED CONNECTIONS), (4) AISC 341 (SEISMIC DESIGN), (5) AISC 343 (STEEL JOINTS), (6) AISC 344 (STEEL JOINTS), (7) AISC 345 (STEEL JOINTS), (8) AISC 346 (STEEL JOINTS), (9) AISC 347 (STEEL JOINTS), (10) AISC 348 (STEEL JOINTS), (11) AISC 349 (STEEL JOINTS), (12) AISC 350 (STEEL JOINTS), (13) AISC 351 (STEEL JOINTS), (14) AISC 352 (STEEL JOINTS), (15) AISC 353 (STEEL JOINTS), (16) AISC 354 (STEEL JOINTS), (17) AISC 355 (STEEL JOINTS), (18) AISC 356 (STEEL JOINTS), (19) AISC 357 (STEEL JOINTS), (20) AISC 358 (STEEL JOINTS), (21) AISC 359 (STEEL JOINTS), (22) AISC 360 (STEEL DESIGN), (23) AISC 361 (STEEL DESIGN), (24) AISC 362 (STEEL DESIGN), (25) AISC 363 (STEEL DESIGN), (26) AISC 364 (STEEL DESIGN), (27) AISC 365 (STEEL DESIGN), (28) AISC 366 (STEEL DESIGN), (29) AISC 367 (STEEL DESIGN), (30) AISC 368 (STEEL DESIGN), (31) AISC 369 (STEEL DESIGN), (32) AISC 370 (STEEL 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	TC LL	20.0	PSF	REF	R8228 - 29505
	TC DL	10.0	PSF	DATE	09/30/09
	BC DL	10.0	PSF	DRW	HCUSR8228 09273023
	BC LL	0.0	PSF	HC-ENG	JB/DF
	TOT.LD.	40.0	PSF	SEGN -	46280
	DUR.FAC.	1.25		FROM	AH
	SPACING	24.0"		JREF -	1TVI8228Z03



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0218

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3

Special loads

TC - From	63 pif at 0.00 to 63 pif at 11.50
TC - From	63 pif at 11.50 to 63 pif at 24.83
BC - From	20 pif at 0.00 to 20 pif at 7.46
BC - From	70 pif at 7.46 to 70 pif at 9.42
BC - From	110 pif at 9.42 to 110 pif at 12.00
BC - From	110 pif at 12.00 to 110 pif at 13.67
BC - From	70 pif at 13.67 to 70 pif at 15.54
BC - From	20 pif at 15.54 to 20 pif at 23.00
BC - From	5 pif at 23.00 to 5 pif at 24.83

Wind reactions based on MWFRS pressures.

** The maximum horizontal reaction is 245#

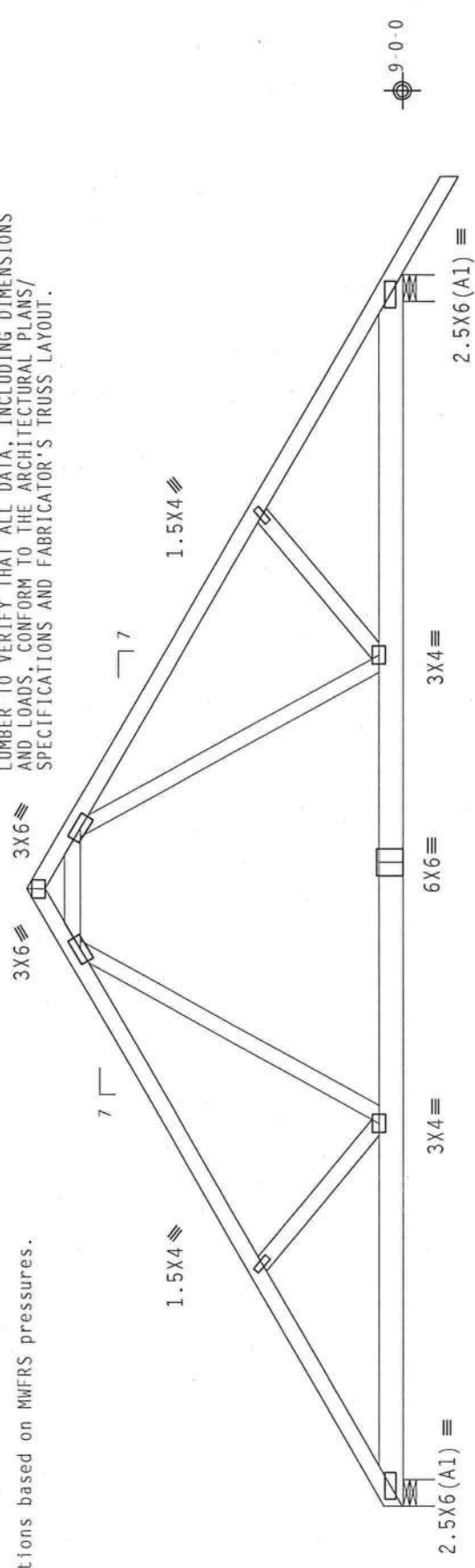
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00 G_{CPI}(+/-)=0.18$

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FABRICATOR TO REVIEW THIS DWG PRIOR TO CUTTING LUMBER TO VERIFY THAT ALL DATA, INCLUDING DIMENSIONS AND LOADS, CONFORM TO THE ARCHITECTURAL PLANS/ SPECIFICATIONS AND FABRICATOR'S TRUSS LAYOUT.



11'-6-0
23'-0-0 Over 2 Supports
R=1237 U=344 W=6"
RL=245/-229
R=1374 U=382 W=6"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .3125"/Ft.

QTY: 4

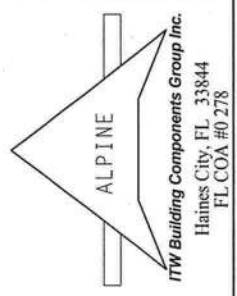
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DESI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/PJ) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2010/16GA (40-H/55/PS) ASTM A653 GRADE 40/60 (4, K/R/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF ALL TRUSS AND, UNLESS OTHERWISE NOTED OR OTHERWISE SPECIFIED, APPLY TO ALL TRUSS MEMBERS. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

JAMES E. KELLY, JR.
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Sep 30 '09

REF R8228- 29506
DATE 09/30/09
DRW HCUSR8228 09273033
HC-ENG JB/DF
SEQN- 46285
FROM AH
JREF- 1TVI8228Z03



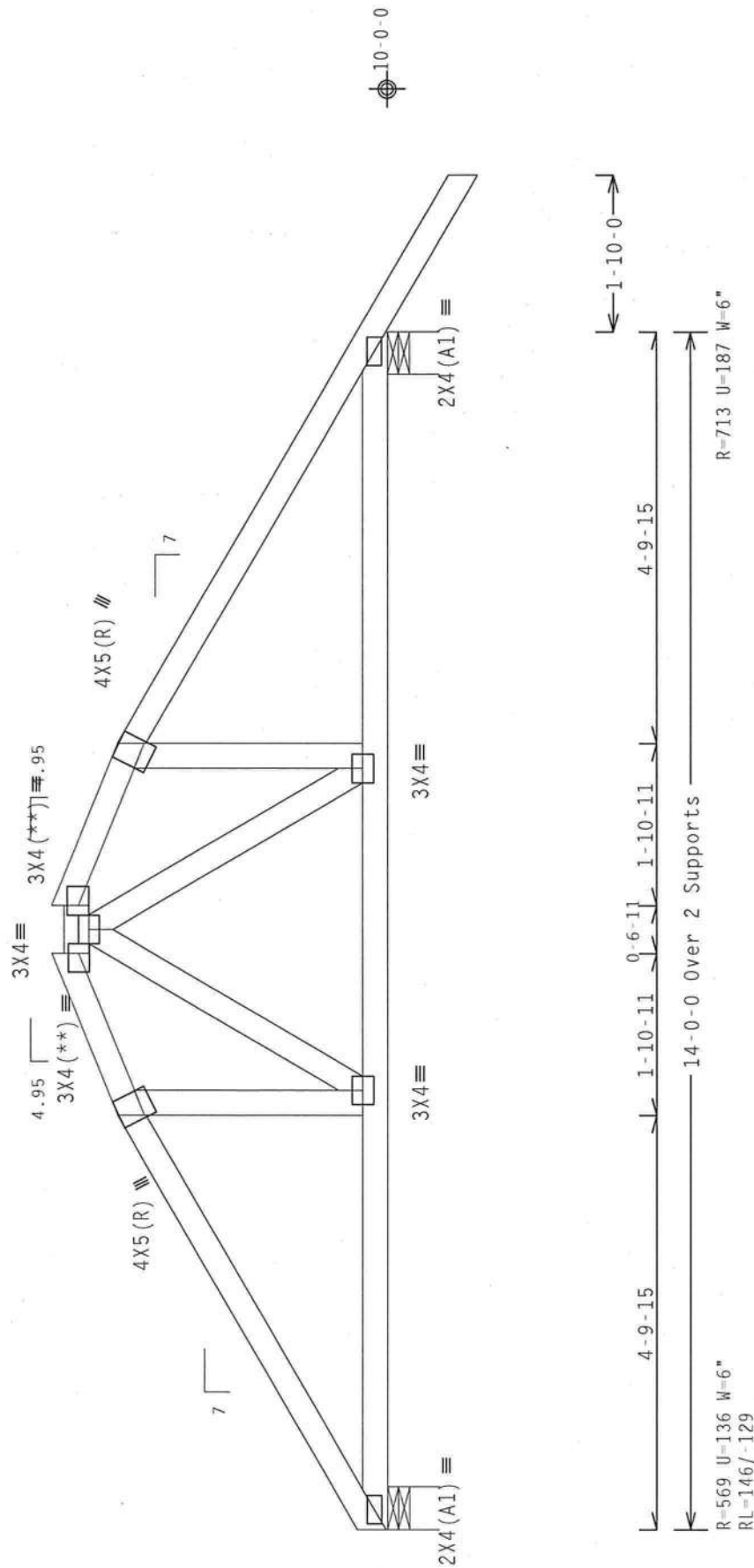
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

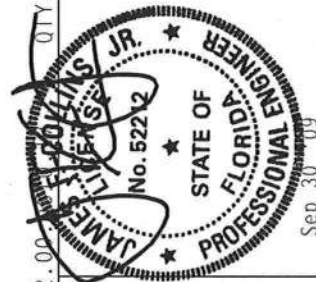
Deflection meets L/240 live and L/180 total load.

MMWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = 5"/Ft.

TC LL	20.0	PSF	REF	R8228-	29510
TC DL	10.0	PSF	DATE	09/30/09	
BC DL	10.0	PSF	DRW	HCUSR8228	09273004
BC LL	0.0	PSF	HC-ENG	JB/DF	
TOT.LD.	40.0	PSF	SEQN-	46381	
DUR.FAC.	1.25		FROM	AH	
SPACING	24.0"		JREF-	1TVI82282703	

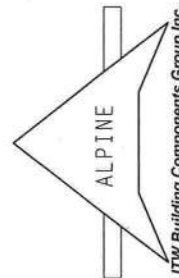


Sep 30 09

*****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENTS INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314), OR TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, PADOSIN, OH, 45715) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE SPECIFIED, ALL TRUSSES MUST HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOLTED CHORDS SHALL HAVE PROPERLY ATTACHED RIGID PURLING.

****IMPORTANT*****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE SPECIFICATIONS WILL BE AT THE RISK OF THE INSTALLER. *ITM BCG, INC. IS NOT RESPONSIBLE FOR ERECTION OF THE TRUSS OR FOR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING, ETC. BRACING OF TRUSSES.
ITEMS CONFORM WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC.) BY AISC/AIA AND TPI.
CONNECTOR PLATES ARE MADE OF 20/30/10GA. (N/155/KIC) ALUM. A653 GRADE 40/60 (N, K/H-SS) GALV. STEEL. APPLY
PLATES TO EACH FACE OF TRUSS AND, OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. A SEAL ON THIS
ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ABEX 43 OF TPI-2002 SEC. 2.
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT
DESIGN SHOWN. THE SUBMITTAL AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE
BUILDING DESIGNER PER AISI/TPI 1 SEC. 2.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

PLT TYP. Wave

110 mph wind, 15.00 ft mean ht, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCpi(+/-)-0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

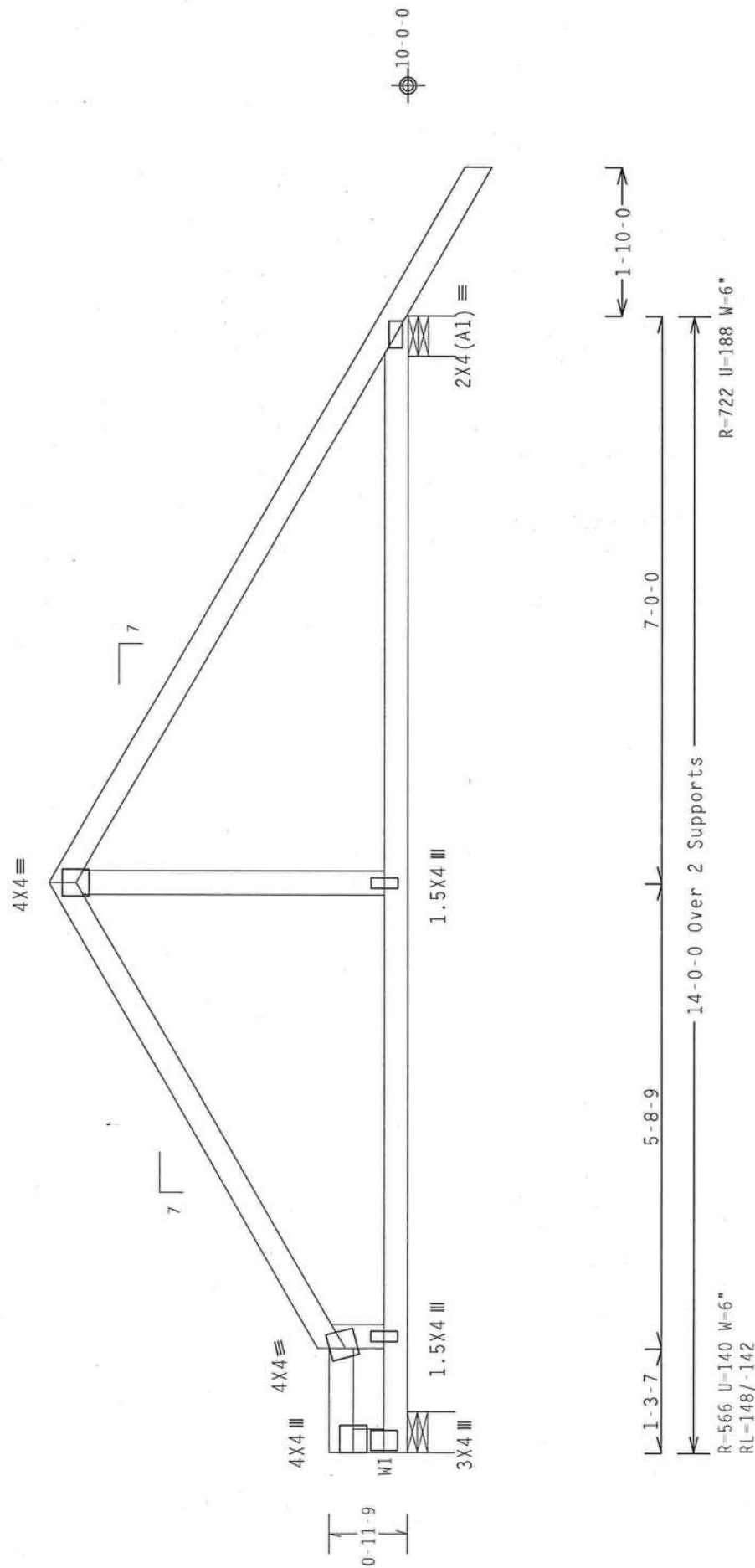
Deflection meets L/240 live and L/180 total load.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W1 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MMFRS loads based on trusses located at least 7.50 ft. from roof edge.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

QTY:2 FL/-/4/-/-/R/-

Scale = .5"/Ft.

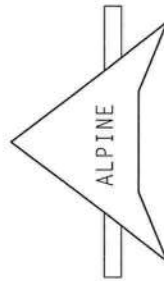
TC LL	20.0 PSF	REF R8228- 29511
TC DL	10.0 PSF	DATE 09/30/09
BC DL	10.0 PSF	DRW HCUSR8228 09273036
BC LL	0.0 PSF	HC-ENG JB/DF *
TOT.LD.	40.0 PSF	SEQN- 46387
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TVI8228Z03



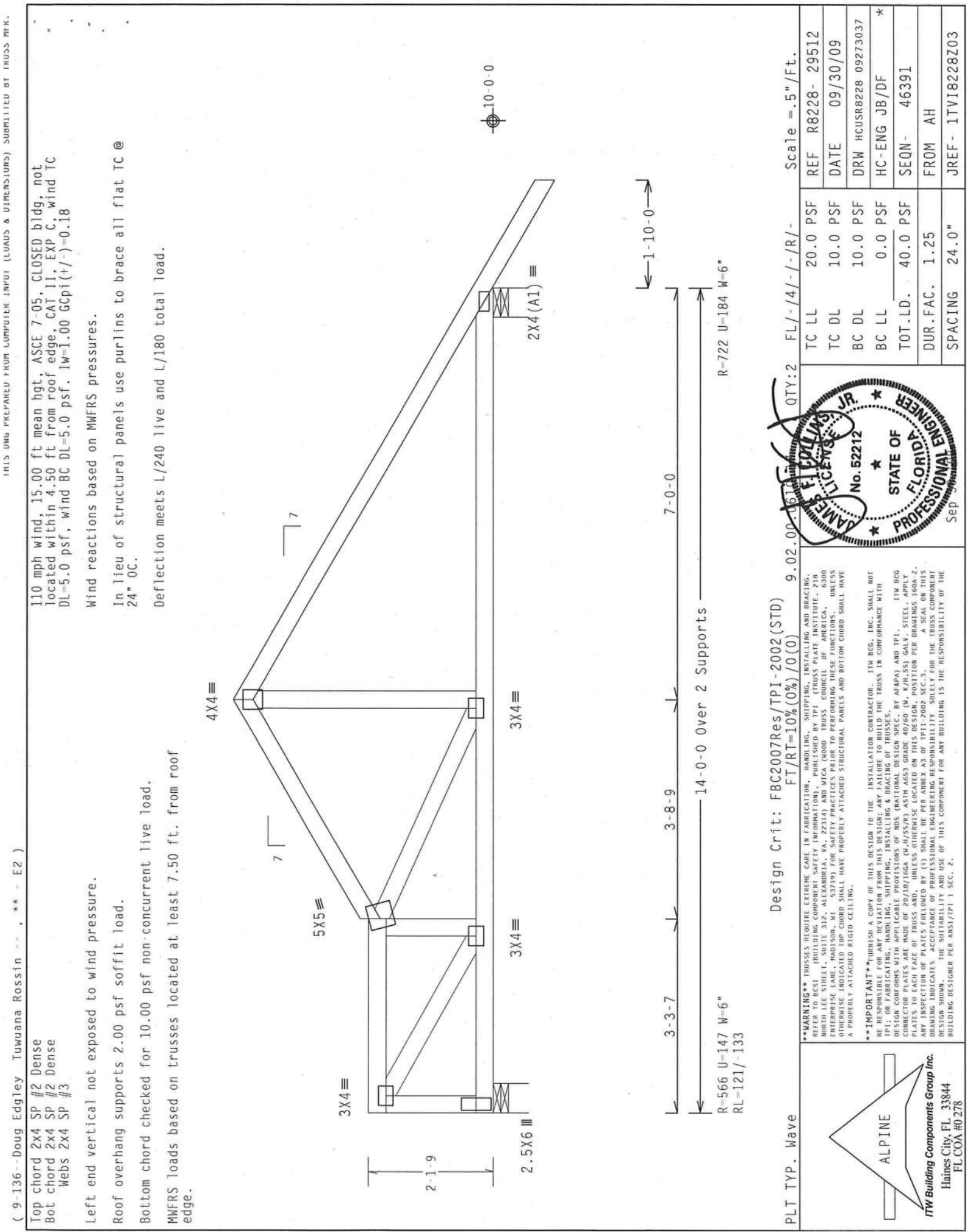
WARNING THUSSE'S REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. ALL WORK MUST BE DONE BY AN EXPERIENCED PROFESSIONAL CONTRACTOR. THE FOLLOWING INFORMATION IS FOR YOUR REFERENCE ONLY.
NORTH LEE STREET, SUITE 302, ALEXANDRIA VA 22314) AND VICA (4800 TRUSS COUNCIL OF AMERICA, 6000 ENTERPRISE LANE, MIDDLEBURY, VT 52719).
THUSSE'S PANELS ARE DESIGNED TO BE USED WITH CHORDS OR OTHERS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

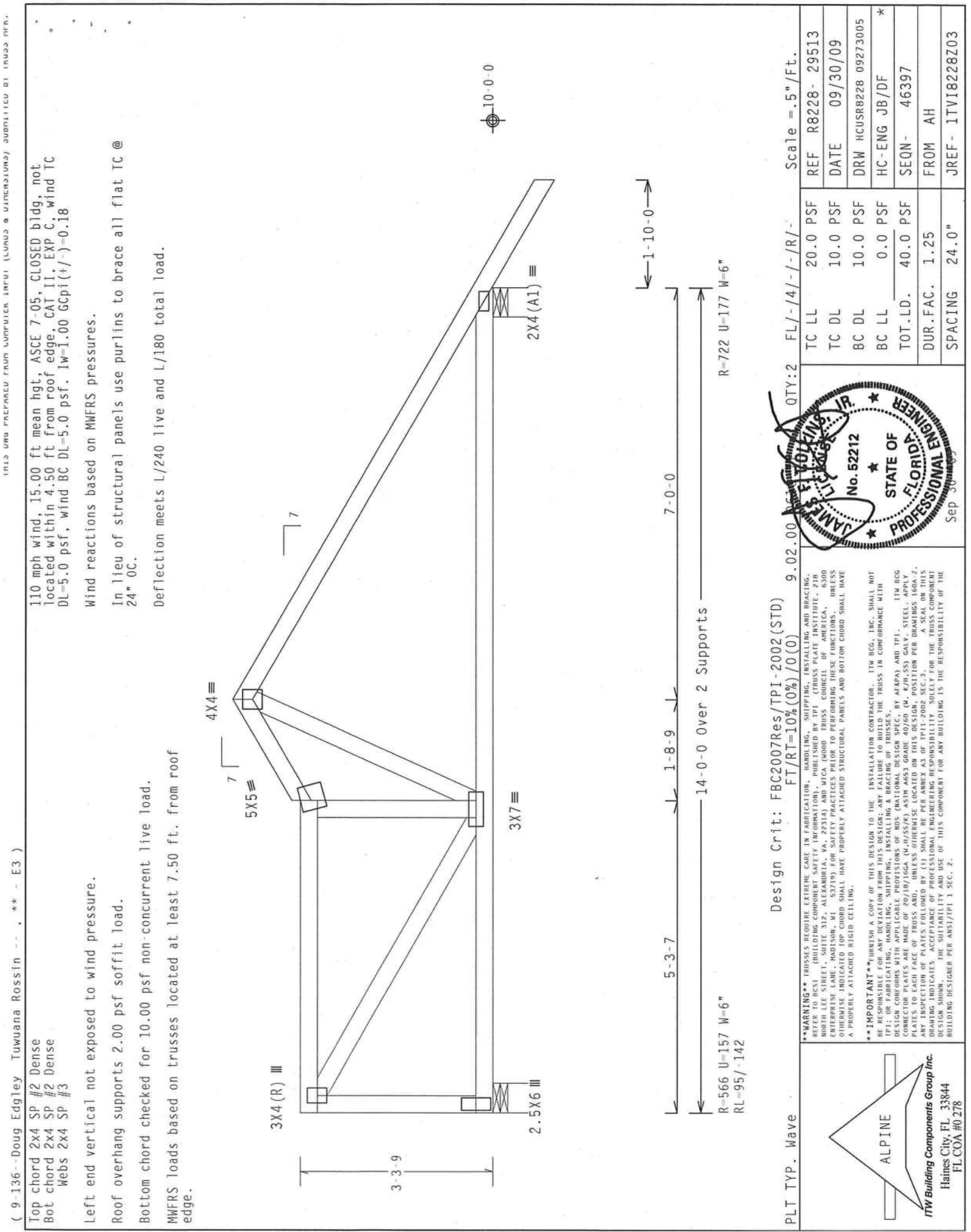
****IMPORTANT****—FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPLIANCE WITH THE SPECIFICATIONS, OR IN FABRICATING, HANDLING, SHIPPING, INSTALLING OR BRACING OF TRUSSES.

DESIGN COMPONDS WITH APPLICABLE PROVISIONS OF NATIONAL DESIGN SPEC., BY AISC) AND TPI-1
CONCRETE PLATES ARE MADE OF 20/817UGA (H/255K) ASTM A653 GRADE 40/60 (N. K/H=55) GALV. STEEL, APPLIED TO EACH FACE OF TRUSS AND OTHERWISE LOCATED IN THIS DESIGN, POSITION PER DRAWINGS 160A-Z.
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AREA AS OF TPI-1-2002 SEC. 3.
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN.
THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.
TPI-1



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278





110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

9-136--Doug Edgley Tuwana Rossin --, ** - E3)

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

Scale = .5" / Ft.

QTY: 2

FL / - / 4 / - / R / -

TC LL 20.0 PSF
TC DL 10.0 PSF
BC DL 10.0 PSF
BC LL 0.0 PSF
TOT.LD. 40.0 PSF
DUR.FAC. 1.25
SPACING 24.0"

REF R8228- 29513
DATE 09/30/09
DRW HCUSR8228 09273005
HC-ENG JB/DF
SEQN- 46397
FROM AH
JREF- 1TVI8228Z03

ALPINE
rtw Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

SEP 30 2009
JAMES F. VOLKING, JR.
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER

1-10-0

7-0-0

14-0-0 Over 2 Supports

R=566 U=157 W=6"
RL=95/-142

R=722 U=177 W=6"

1-10-0

10-0-0

10-0-0

10-0-0

10-0-0

10-0-0

10-0-0

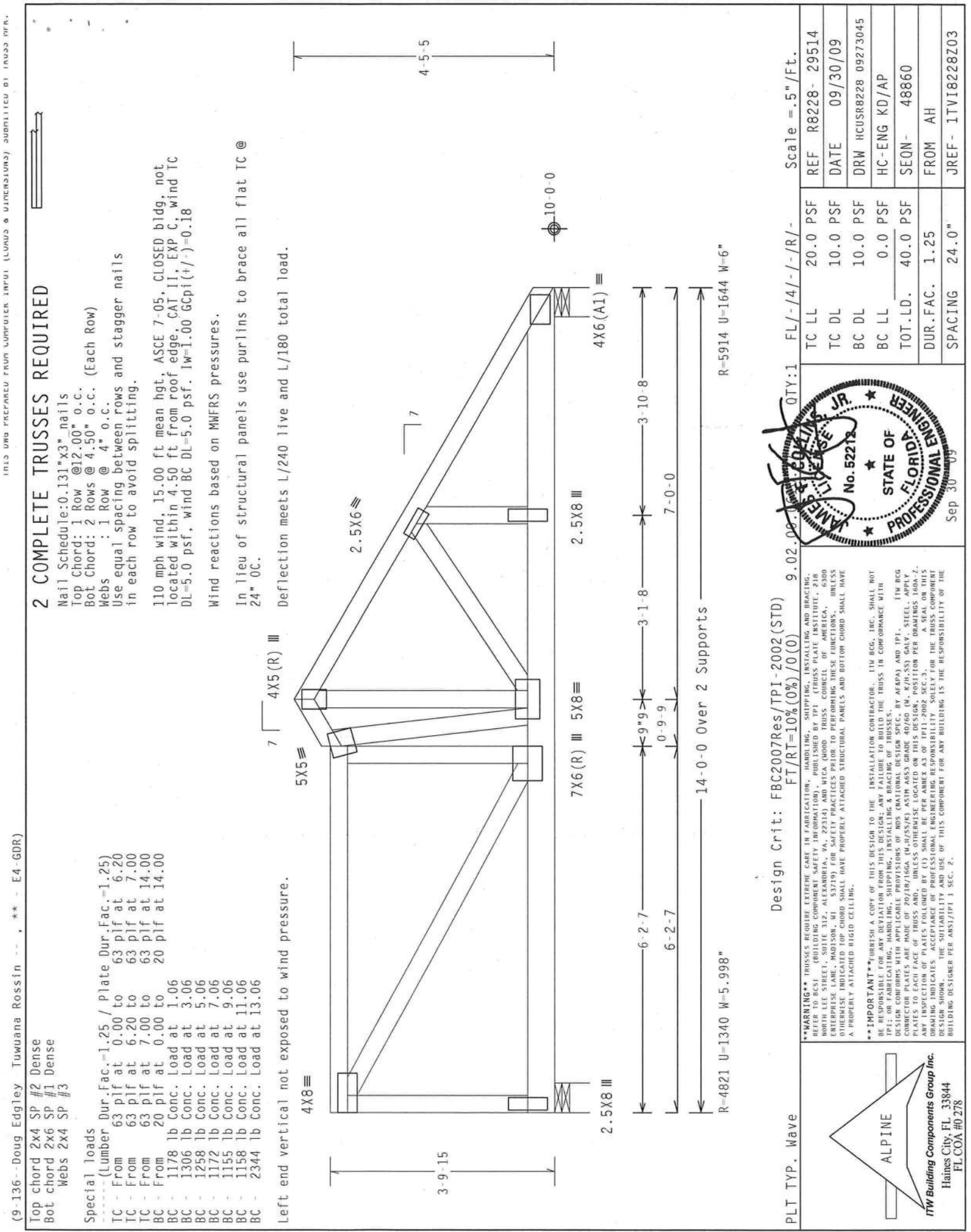
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10-0-0

10-0-0

10-0-0



THIS WAS PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY KNUSS PER.

(9-136--Doug Edgley Tuwana Rossin --, ** - E4-GDR)

2 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.131"x3" nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 2 Rows @ 4.50" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 63 plf at 0.00 to 63 plf at 6.20
TC - From 63 plf at 6.20 to 63 plf at 7.00
TC - From 63 plf at 7.00 to 63 plf at 14.00
BC - From 20 plf at 0.00 to 20 plf at 14.00
BC - 1178 lb Conc. Load at 1.06
BC - 1306 lb Conc. Load at 3.06
BC - 1258 lb Conc. Load at 5.06
BC - 1172 lb Conc. Load at 7.06
BC - 1155 lb Conc. Load at 9.06
BC - 1158 lb Conc. Load at 11.06
BC - 2344 lb Conc. Load at 13.06

Left end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load.

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .5" / Ft.

QTY: 1

FL / - / 4 / - / R / -

9.02.00

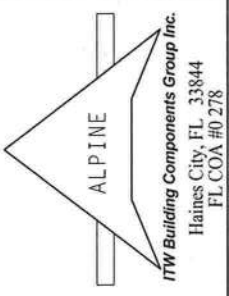
PLT TYP. Wave

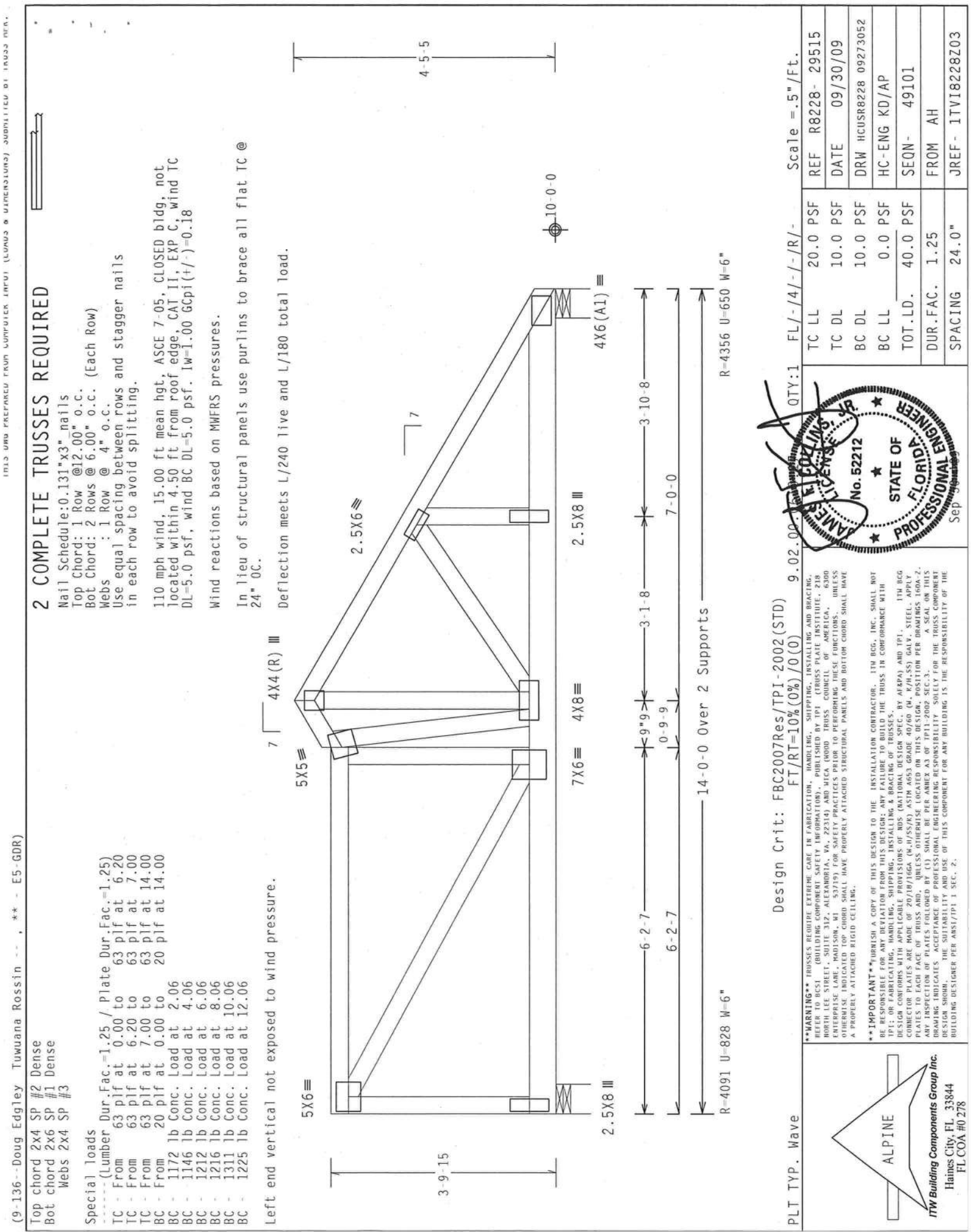
TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD CHORD TRUSS, COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 48224) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2024-T3 ALUMINUM (4090) GRADE 40/60 (4. K/IN.SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED AS OF TPI-2002, SEC.3. A SEAL ON THIS DESIGN SHALL BE REQUIRED FOR THE TRUSS. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.





(9-136--Doug Edgley Tuwuana Rossin -- , ** - E5-GDR)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

Special loads
-----Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
IC - From 63 plf at 0.00 to 63 plf at 6.20
IC - From 63 plf at 6.20 to 7.00
IC - From 63 plf at 7.00 to 63 plf at 14.00
BC - From 20 plf at 0.00 to 20 plf at 14.00
BC - 1172 lb Conc. Load at 2.06
BC - 1146 lb Conc. Load at 4.06
BC - 1212 lb Conc. Load at 6.06
BC - 1216 lb Conc. Load at 8.06
BC - 1311 lb Conc. Load at 10.06
BC - 1225 lb Conc. Load at 12.06

Left end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load.

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Nail Schedule: 0.131"x3" nails

Top Chord: 1 Row @ 12.00" O.C.

Bot Chord: 2 Rows @ 6.00" O.C. (Each Row)

Webs : 1 Row @ 4" O.C.

Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{Cpi}(+/-)=0.18$

Scale = 5" / Ft.

Design Crit: FBC2007Res/TPI-2002(STD)

FT/RT=10%(0%)/0(0)

9.02.00

QTY: 1

FL/- /4/- /- /R/-

TC LL 20.0 PSF

TC DL 10.0 PSF

BC DL 10.0 PSF

BC LL 0.0 PSF

TOT.LD. 40.0 PSF

DUR.FAC. 1.25

SPACING 24.0"

REF R8228- 29515

DATE 09/30/09

DRW HCUSR8228 09273052

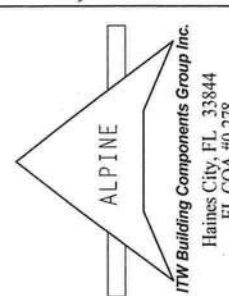
HC-ENG KD/AP

SEQN- 49101

FROM AH

JREF- 1TVI8228Z03

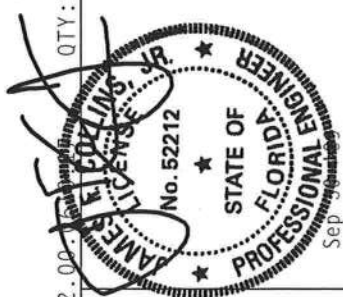
PLT TYP. Wave



WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (CROSS PLATE INSTITUTE, 218 HUNTER LEE STREET, SUITE 100, ALEXANDRIA, VA 22304) AND TPI (TRUSS PANEL INSTITUTE, 1000 W. 10TH STREET, SUITE 100, ALEXANDRIA, VA 22304) FOR ADDITIONAL INFORMATION. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/PS) AND TPI1. 1TH BCG CONNECTOR PLATES ARE MADE OF 2018/16GA (N/A/SS/R) ASTM A653 GRADE 40/60 (H, R/H, SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



3 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.131"x3" nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 2 Rows @ 5.50" o.c. (Each Row)
Webs: 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

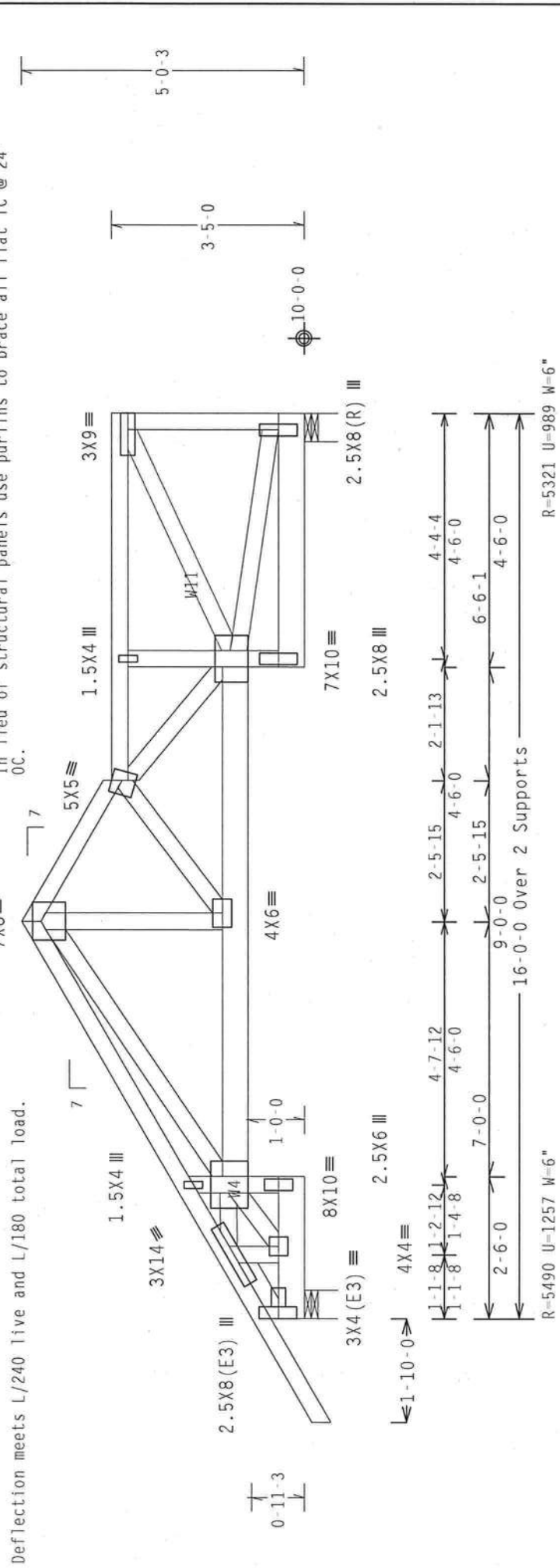
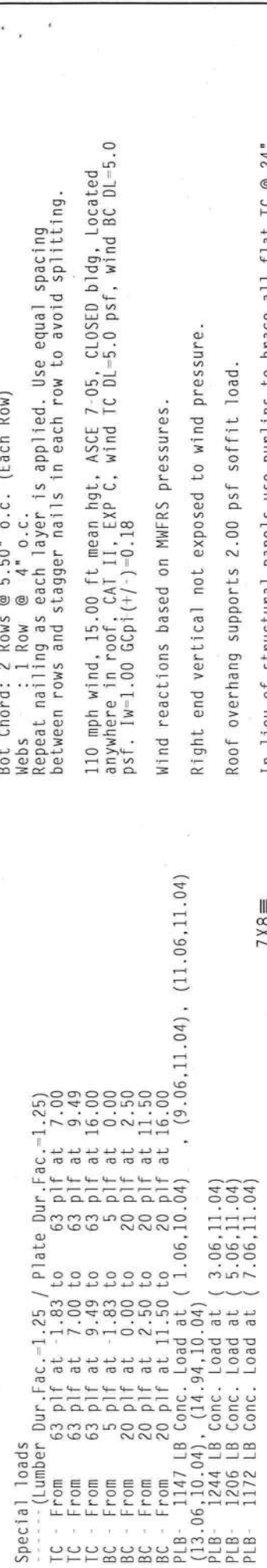
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCpl(+/-)=0.18

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .375"/Ft.

REF	DATE	DRW	HC-ENG	SEQN	FROM	JREF
R8228	09/30/09	HCUSR8228	09273053	49126	1.25	1TVI8228Z03

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

Professional Engineer
No. 52212
STATE OF FLORIDA

Sep 30 '09

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

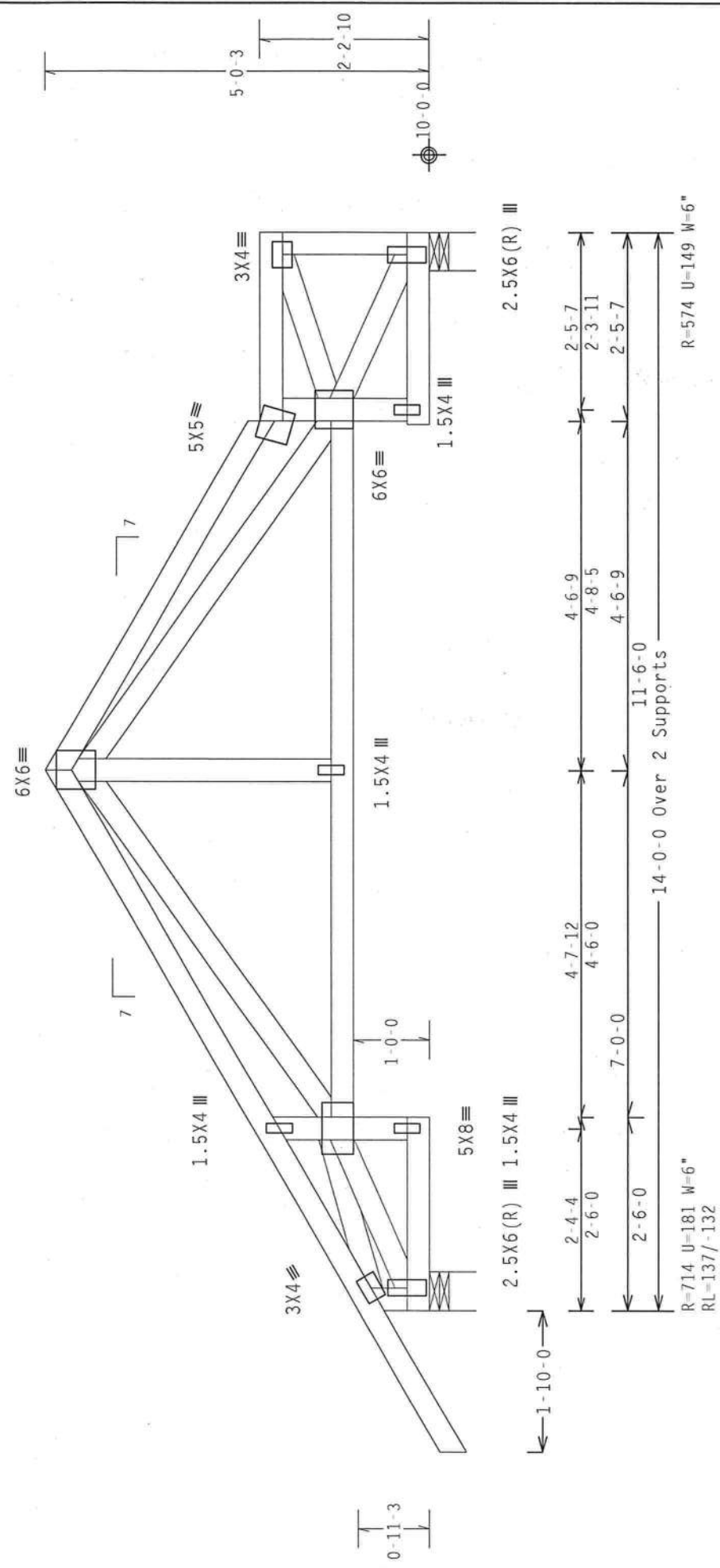
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)-0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

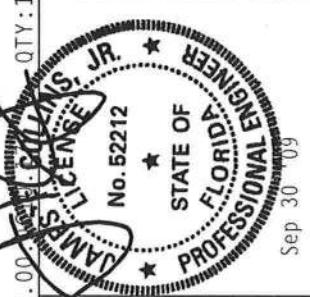
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

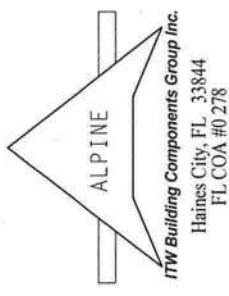
Scale = .5" / Ft.

REF	R8228 - 29519
DATE	09/30/09
DRW	HCUSR8228 09273049
HC-ENG	KD/AP
SEQN	48705
FROM	AH
JREF	1TVI8228Z03



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DESIGN (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 2100 WEST STREET, ALCOHOL, ALA 37008, FOR INSTRUCTIONS ON THE PROPER HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AISC) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/166A (4-10/55/5) ASTM A553 GRADE 40/60 (4, 8/21/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT BUILDING SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



PLT TYP. Wave

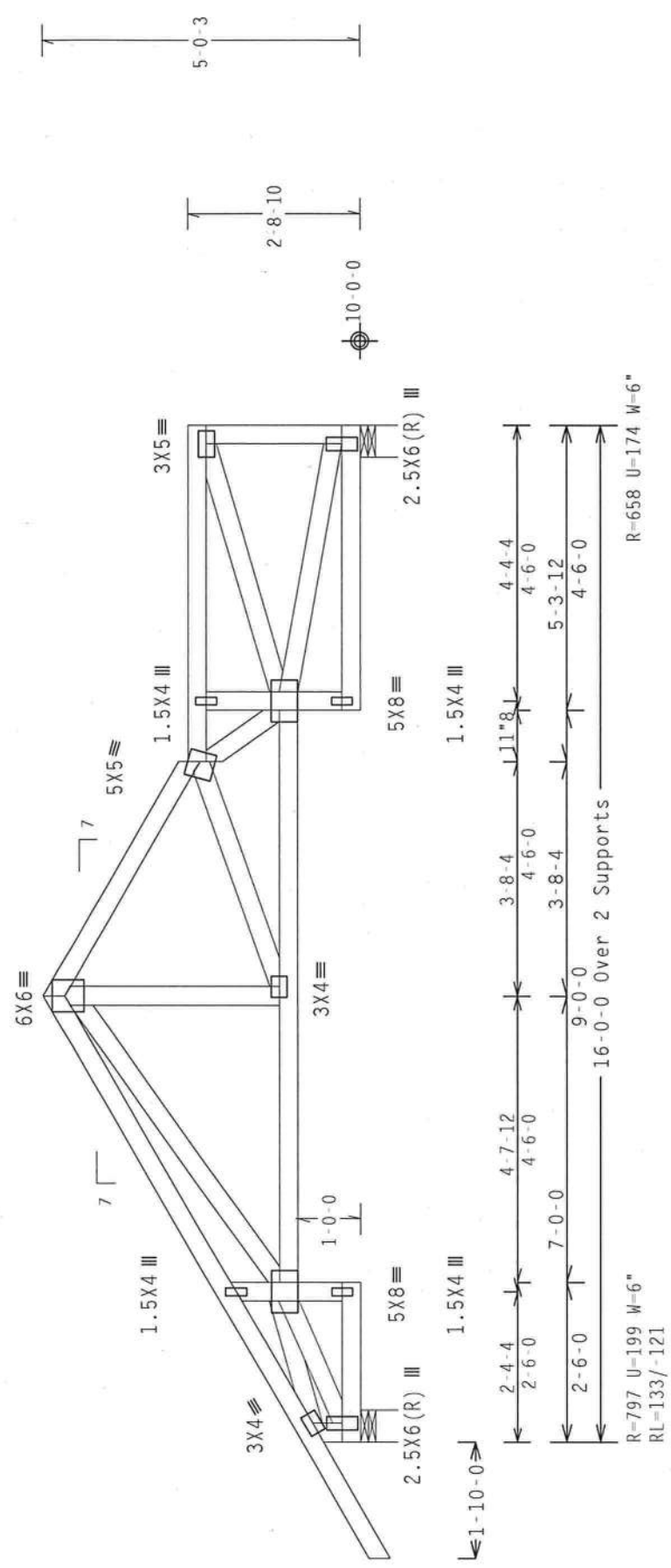
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ Gcpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

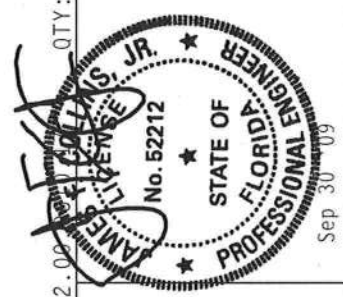
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .375" / Ft.

QTY:1 FL/-/4/-/-/R/-

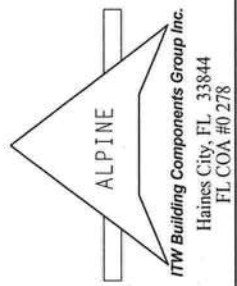
TC LL	20.0	PSF	REF	R8228- 29520
TC DL	10.0	PSF	DATE	09/30/09
BC DL	10.0	PSF	DRW	HCUSR8228 09273051
BC LL	0.0	PSF	HC-ENG	KD/AP *
TOT.LD.	40.0	PSF	SEQN -	48714
DUR.FAC.	1.25		FROM	AH
SPACING	24.0"		JREF -	1TV18228Z03



****WARNING**** TRUSSES ROUTINE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BECSI (BUILDING COMPONENT INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 INTERSTATE LANE, MOULSON, WI 53575) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS A PROPERLY NOTICED WARNING LABEL IS ATTACHED TO EACH TRUSS, ALL TRUSSES SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE OR INJURY TO PERSONS OR PROPERTY THAT RESULTS FROM THE TRUSS IN CONFORMANCE WITH TPI'S SPECIFICATIONS FOR MANUFACTURING, SHIPPING, HANDLING, AND BRACING OF TRUSSES.

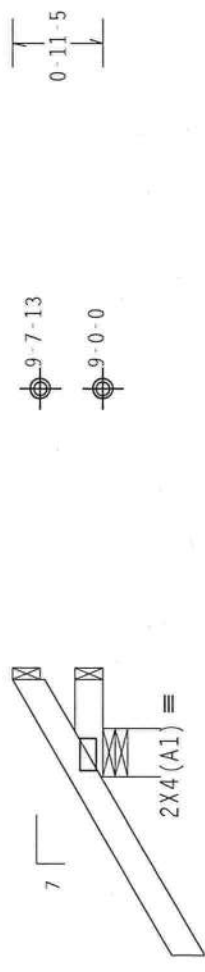
DESIGN CORROSIONS WITH APPLICABLE PROVISIONS OF AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC.) AND AISC DESIGN GUIDE CORRELATION WITH APPLICABLE PROVISIONS OF AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC.) AND AISC DESIGN GUIDE. TPI BEG CORRELATION PLATES ARE MADE OF 2010/1016GA (A/1055/PS) ASTM A653 GRADE 40/60 (90, K/H/55) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AISC A3 OF 1911-2002 SEC.3. A SEAL ON THE DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISI/TPI 1 SEC. 2.



110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART-ENC. bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



R=-.94 RW=63 U=94

R=1 RW=32 U=36



 1-0-0 Over 3 Supports

 R=328 U=129 W=6

 RL=55/-46

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

Scale = .5"/Ft.

TC LL	20.0	PSF	REF	R8228- 29523
TC DL	10.0	PSF	DATE	09/30/09
BC DL	10.0	PSF	DRW	HCUSR8228 09273012
BC LL	0.0	PSF	HC-ENG	JB/DF
TOT.LD.	40.0	PSF	SEQN -	46327
DUR.FAC.	1.25		FROM	AH
SPACING	24.0"		JREF -	1TV18228Z03

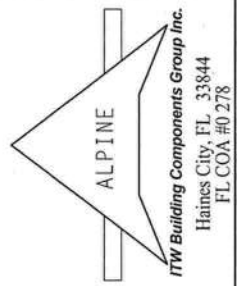


Sep 30 09

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SPECIFICATIONS, PUBLISHED BY JCI CRUISES PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA TRUSS, COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 48271, FOR TRUSS PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY OVIATION OF THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE FOLLOWING FABRICATION, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

1. ALL HIGH CONCENTRATION LOADS SHALL BE DESIGNED TO BE WITHIN THE DESIGN SPEC. 2. ALL TRUSS AND TPI PLATES TO EACH FACE OF TRUSSES AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. 3. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMB AS OF 1/11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES THE ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SAFETY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMB/TPI 1 SEC. 2.



Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense

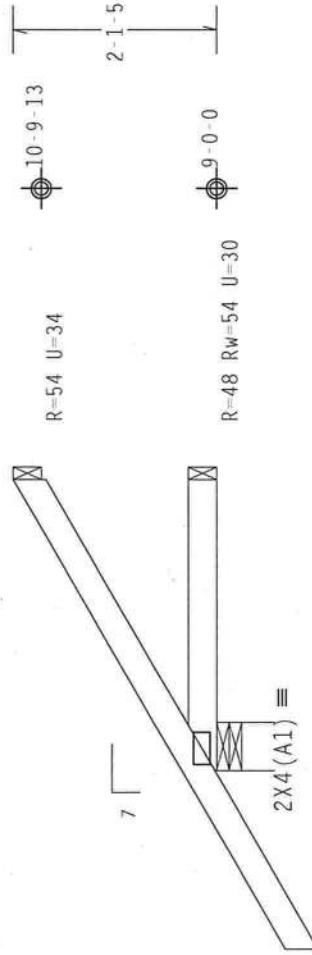
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART_5_ENC. bldg,
Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind
BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

Design Crit: FBC2007Res/TPI - 2002(STD)
 $FT/RT = 10\% (0\%) / 0 (0)$

9.02.00: 6000 16: 20
OTY: 10 FL / - / 4 / - / - / R / -
Scale = 5" / Ft.

PLT TYP. Wave

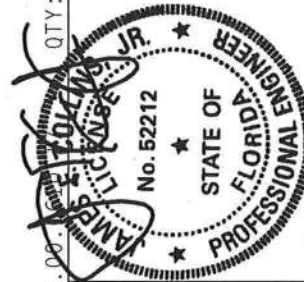
****WARNING**** THUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE SAFETY INFORMATION. PUBLISHED BY THE CROSS PLATE INSTITUTE, 2100 ENTERPRISE LANE, MOULDSBORO, VA 57219. FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, WITHOUT PROPERLY ATTACHED RIGID CELLING, TOP AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CELLING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES.

DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AF&PA) AND TPI. ITW BCG CONNECTION PLATES ARE MADE OF 40D18/16GALV./SS/AISI/A798 STEEL GRADE 50 FEMINUM, PER ASTM SPECIFICATION 307-2. GALVANIZED STEEL GRADE 50 FEMINUM, PER ASTM SPECIFICATION 307-2.

ANY INSPECTION OF PLATES FOLLOWED BY () SHALL BE PER ANNEX 43 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT SHOWN.

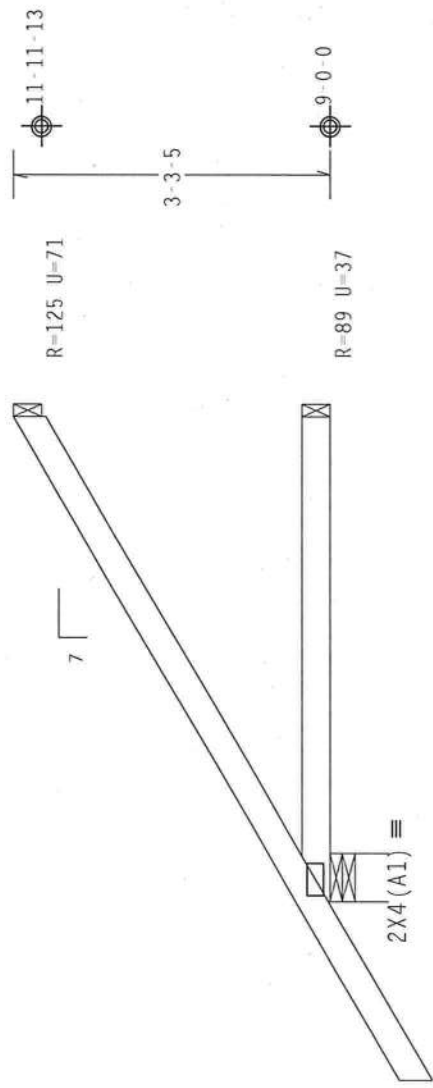
THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Sep 30 09

TC LL	20.0	PSF	REF	R8228-	29524
TC DL	10.0	PSF	DATE	09/30/09	
BC DL	10.0	PSF	DRW	HCUSR8228	09273016
BC LL	0.0	PSF	HC-ENG	JB/DF	*
TOT.LD.	40.0	PSF	SEQN-	46330	
DUR.FAC.	1.25		FROM	AH	
SPACING	24.0"		JREF-	1TVI8228Z03	

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Roof overhang supports 2.00 psf soffit load.
Bottom chord checked for 10.00 psf non-concurrent live load.



$\overleftarrow{\text{1-10-0}}$
 $\overleftarrow{\text{5-0-0}}$ Over 3 Supports

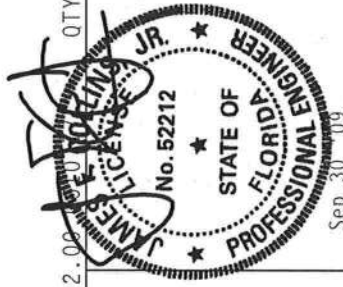
 $R=366$
 $U=114$
 $W=6"$

 $RL=135/-64$

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .5"/Ft.

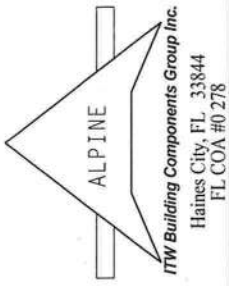
TC LL	20.0 PSF	REF	R8228- 29525
TC DL	10.0 PSF	DATE	09/30/09
BC DL	10.0 PSF	DRW	HCUSR8228 09273025
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN -	46333
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF -	1TVI8228Z03

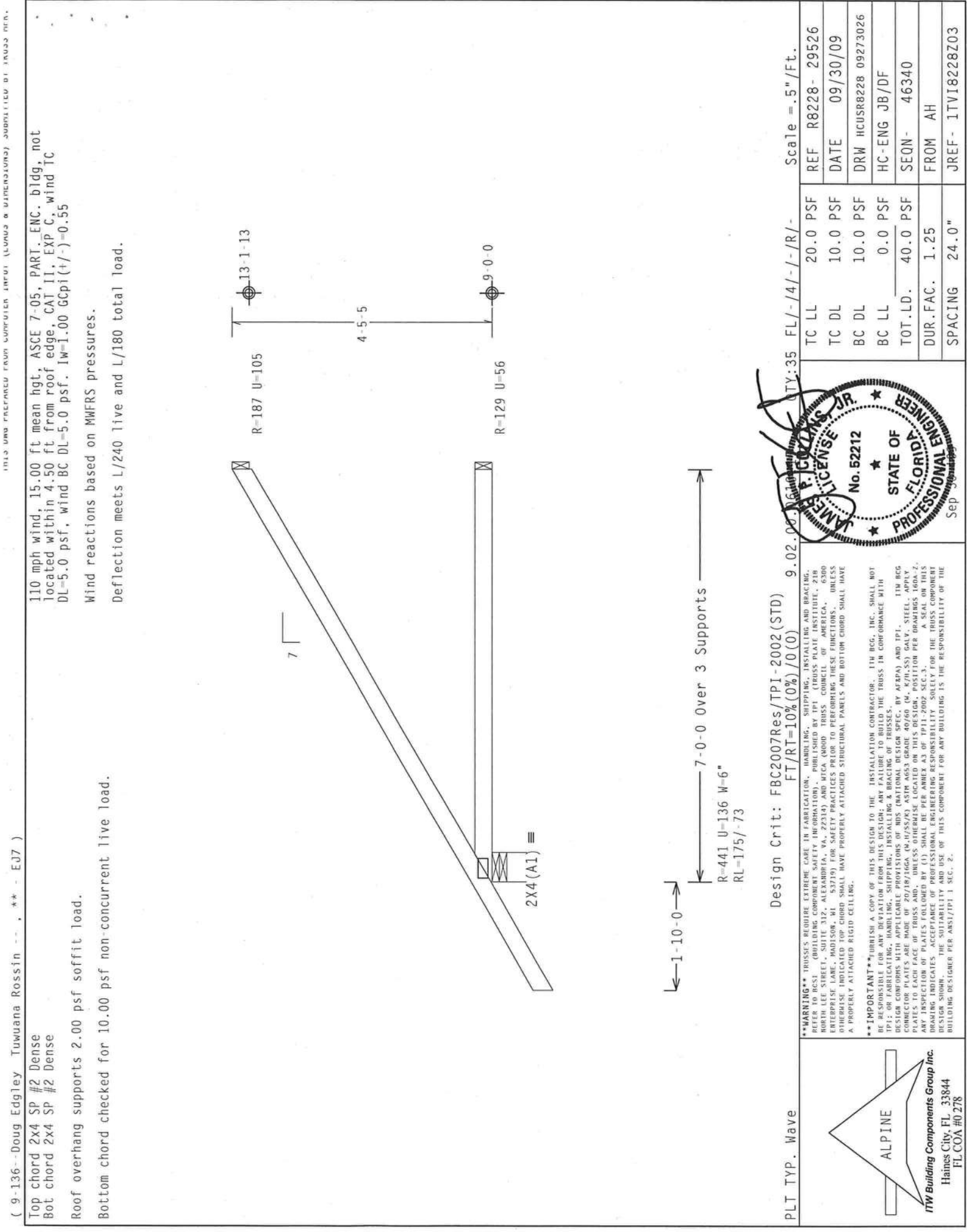


WARNING TRUSSES PRODUCE EXTREME CARE IN FABRICATION, INSTALLATION, SHIPPING, INSTALLING AND BRACING. REFER TO RECSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 219 NORTH LEE STREET, SUITE 312, AUSTIN, TX 78744, AND AISC (WOOD TRUSS COUNCIL OF AMERICA), 6300 UTTERLIFE LANE, HOUSTON, TX 77237, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1; OR FABRICATING, INSTALLING, SHIPPING, INSTALLING OR BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC.), BY AFPA AND TP1. CONNECTOR PLATES ARE MADE OF 2010/11/16GA (14/35/57K) ASTM A653 GRADE 40/60 (W, K/20.55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERX 43 OF TP1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMERX/TP1 1 SEC. 2.





110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind IC DL=5.0 psf, wind BC DL=5.0 psf. IW=1.00 GCPI(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Roof overhang supported 2.00 psf soffit load.
Bottom chord checked for 10.00 psf non-concurrent live load.

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0) 9.02.00.00

PLT TYP. Wave

Scale = .5"/Ft.

REF R8228- 29526
DATE 09/30/09
DRW HCUSR8228 09273026
HC-ENG JB/DF
SEQN- 46340
FROM AH
JREF- 1TVI8228Z03

1TV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AF&PA) AND TPI. CONNECTOR PLATES ARE MADE OF 2018/16GA (40.0/SS/A) ASTM A653 GRADE 40/60 (40.0/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 100A-2. ANY OTHER PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. (AISC) 13TH EDITION, 2005. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

JAMES P. COLLINS JR.
LICENSED PROFESSIONAL ENGINEER
No. 52212
STATE OF FLORIDA
Sep 30, 2009

1TV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AF&PA) AND TPI. CONNECTOR PLATES ARE MADE OF 2018/16GA (40.0/SS/A) ASTM A653 GRADE 40/60 (40.0/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 100A-2. ANY OTHER PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. (AISC) 13TH EDITION, 2005. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense

Roof overhang supports 2.00 psf soffit load.

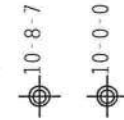
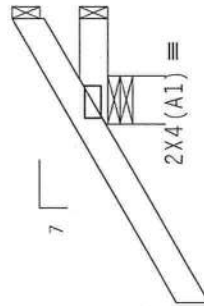
Bottom chord checked for 10.00 psf non-concurrent live load.

MMWFRS loads based on trusses located at least 7.50 ft. from roof edge.

1110 mph wind, 15.00 ft mean ht, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



0-11-15

 $R = -78 \quad R_W = 55 \quad U = 81$

R=3 RW=25 U=28

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

PLT TYP. Wave

Scale = .5"/Ft.

FL/-/4/-/-/R/-/

****WARNING**** THUSSE'S REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. PROPER BOSS POSITIONING IS CRITICAL TO THE STABILITY OF THE NORTH LIFT SYSTEM. SOUTH LIFT SYSTEMS ARE AVAILABLE AT A TOTAL COST OF \$600 PER CHORD. CONTACT US TODAY FOR MORE INFORMATION. CALL 800-975-2211 OR VISIT OUR WEBSITE AT WWW.THUSSE.COM.
THUSSE ENGINEERING LAB., 5527109, MI 552719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

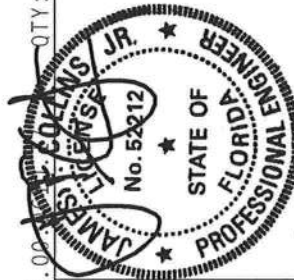
****IMPORTANT****-FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, ITW BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPLIANCE WITH TP-1 OR FABRICATING, WELDING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CHANGES WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., A900), AND TPI (17U BCG TRUSS MANUFACTURING MANUAL, 07/18/2002) SHALL BE MADE BY THE USER. STEEL ELEV. STEEL APPLY PLATES TO FACE OF TRUSS UNLESS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A-2.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AME AS OF TP-1-2002 SEC.3. A SEAL OR THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE INSTALLING DESIGNER PER ANSI/TPI 1 SEC.- 2.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278



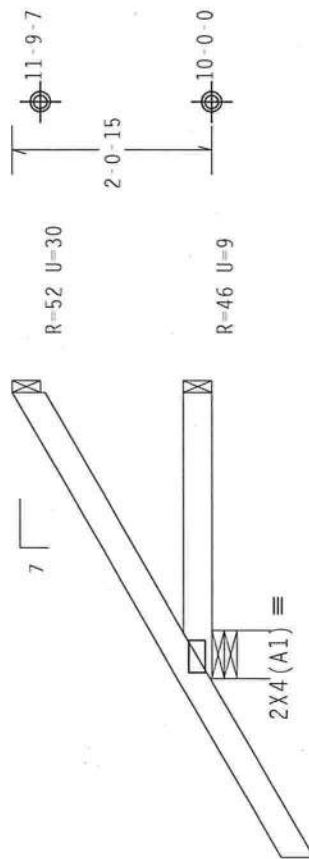
Sep 30 09

TC LL	20.0 PSF	REF R8228-	29527
TC DL	10.0 PSF	DATE	09/30/09
BC DL	10.0 PSF	DRW	HCU8R8228 09273020
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	46348
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TVI8228Z03

110 mph wind, 15.00 ft mean ht, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (w/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

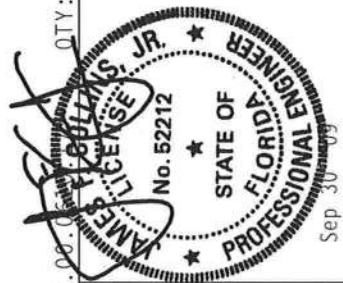


$\leftarrow 1-10-0 \rightarrow$
 $\leftarrow 2-11-6 \text{ Over } 3 \text{ Supports} \rightarrow$
 $R=300 \quad U=73 \quad W=6''$
 $RL=94/-54$

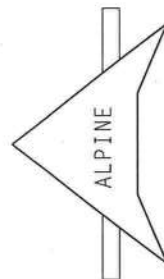
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .5"/Ft.

TC LL	20.0	PSF	REF	R8228-	29528
TC DL	10.0	PSF	DATE	09/30/09	
BC DL	10.0	PSF	DRW	HCUSR8228	09273024
BC LL	0.0	PSF	HC-ENG	JB/DF	
TOT.LD.	40.0	PSF	SEQN-	46352	
DUR.FAC.	1.25		FROM	AH	
SPACING	24.0"		JREF-	1TV18228Z03	

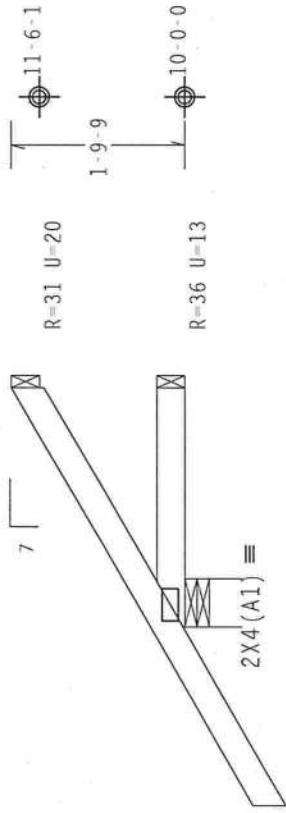
[illegible]

****IMPORTANT****-FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AISC) AND TPI. ITW BEG CONNECTOR PLATES ARE MADE OF 20/18/16GA (9/16X5/8X5/8) ASTM A563 GRADE 40 (60 / 40 / K/H/S) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A-2. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANHX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS IS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ASCE/TPI 1 SEC. 2.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

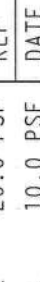
Top chord 2x4 SP #2 Dense	110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located
Bot chord 2x4 SP #2 Dense	anywhere in roof, CAT II, Exp C, wind TC DL=5.0 psf, wind BC
	DL=5.0 psf. Iw=1.00 GCpi(+/-)0.18
Roof overhang supports 2.00 psf soffit load.	Wind reactions based on MWFRS pressures.
Bottom chord checked for 10.00 psf non-concurrent live load.	Deflection meets L/240 live and L/180 total load.



$\overleftarrow{1-10-0}$
 $\overrightarrow{2-5-9}$ Over 3 Supports
 $R=290 \quad U=75 \quad W=6''$
 $RL=85 / .52$

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

~~9.02.00~~ ~~PCH-8000~~ QTY: 8 FL/- /4/- /- /R/-
Scale = 5"/Ft.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BC51 BUILDING COMPONENT SAFETY INFORMATION BY ITW TRUSSES PLATE INSTALLATION, 210 WORTH LEE STREET, SUITE 212, ALEXANDRIA, VA, 22304 AND WCA (WOOD TRUSS) COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, HADDONSBOROUGH, VT 55219 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILINGING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH ITW BCG, INC. FABRICATING, HANDLING, SHIPPING, INSTALLATION AND BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AISC) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/1706GA (WJH/SS/A) ASTM A653 GRADE 40/60 (W, KJH/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABX A3 OF TPI-12002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



REF R8228- 29529

DATE 09/30/09

DRW HCUSR8228 09273018

TC LL 20.0 PSF

TC DL 10.0 PSF

BC DL 10.0 PSF

BC LL 0.0 PSF

TOT.LD. 40.0 PSF

DUR.FAC. 1.25

SPACING 24.0"

HC-ENG JB/DF

SEQN- 46356

FROM AH

JREF- 1TVI8228Z03

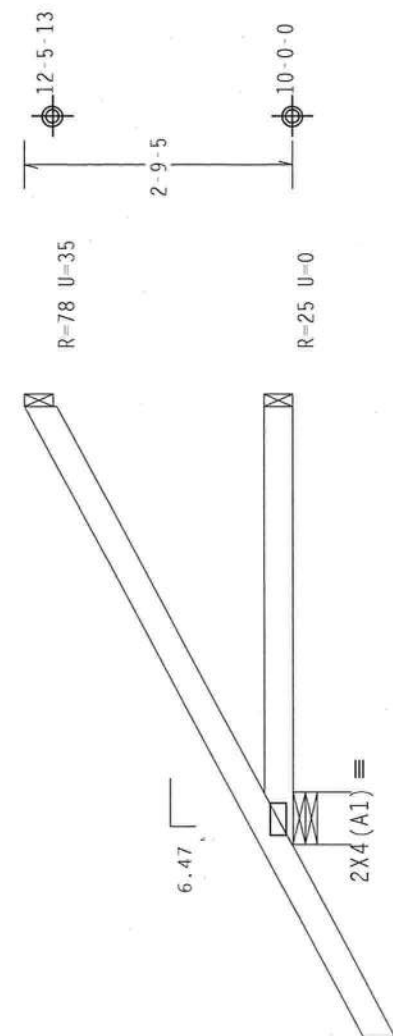
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Hipjack supports 3-2-4 setback jacks with no webs.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.



1-11-13
4-6-2 Over 3 Supports
R=180 U=55 W=6.494"

PLT TYP. Wave

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

9.02.00.16

QTY: 4

Scale = .5"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF R8228- 29530

DATE 09/30/09

DRW HCUSR8228 09273017

HC-ENG JB/DF

SEQN- 46364

FROM AH

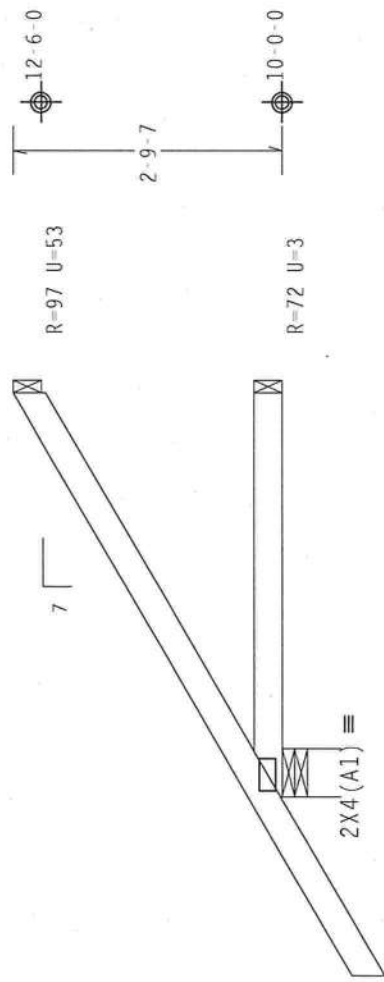
JREF- 1TVI8228Z03

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 HUTHORPE STREET, HAINES CITY, FL 33844) AND TPI (TRUSS PLATE INSTITUTE, 218 HUTHORPE STREET, HAINES CITY, FL 33844) FOR SAFETY INSTRUCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AREA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/16GA (W.H/55/K) ASTM A653 GRADE 40/60 (M. K/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2, ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Roof overhang supports 2.00 psf soffit load.
Bottom chord checked for 10.00 psf non-concurrent live load.
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$
Wind reactions based on MWFRS pressures.
Deflection meets L/240 live and L/180 total load.



1-10-0
4-2-0 Over 3 Supports
R=337 U=72 W=6"
RL=119/-60

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

Scale = .5"/Ft.

QTY: 2

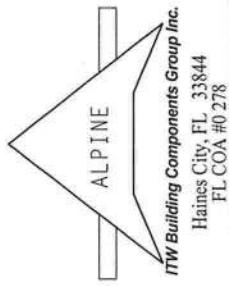
FL/-4/-/-R/-

TC LL	20.0 PSF	REF	R8228 - 29531
TC DL	10.0 PSF	DATE	09/30/09
BC DL	10.0 PSF	DRW	HCUSR8228 09273019
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN	46360
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF	1TVI8228Z03



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND BPCA (WOOD PRODUCT ASSOCIATION, 10000 WOODBRIDGE DRIVE, SUITE 100, FORT WORTH, TX 76116) FOR TRUSS BRACING REQUIREMENTS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/PFA) AND TPI. CONNECTOR PLATES ARE MADE OF 2018/766A (9.4/55/5) ASTM A563 GRADE 40/60 (N, K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

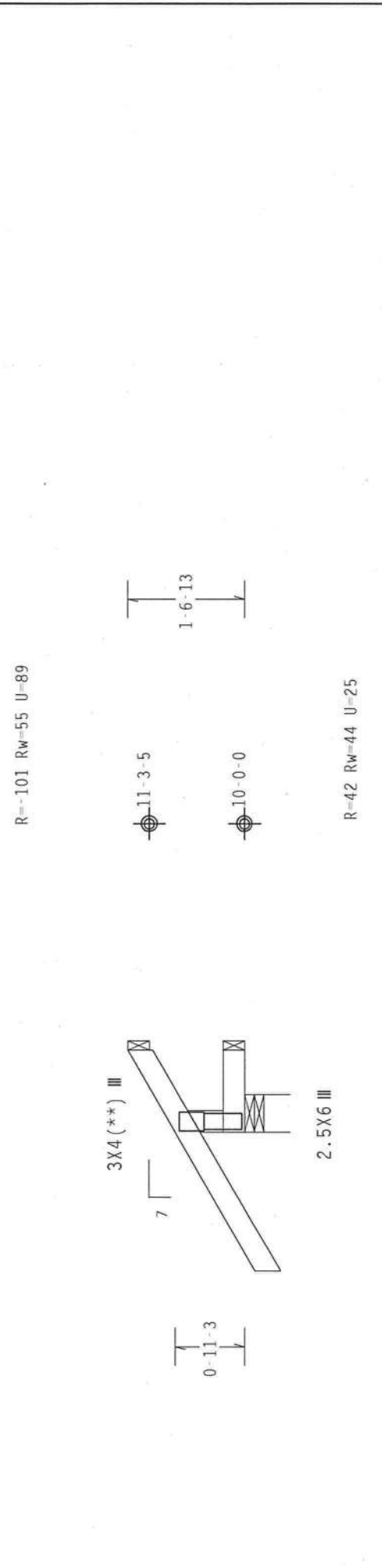
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, Wind BC DL=5.0 psf. $I_w=1.00$ GCpi (+/-)=0.18

Wind reactions based on MWFRS pressures.



$\leftarrow 1-10-0 \rightarrow$
 1-1-1 Over 3 Supports
 R=274 U=74 W=6"
 RL=57 / -46

PLT TYP. Wave
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)
9.02.00
FL/-14/-1-R/
UTY:2
Scale = 5"/Ft.

[illegible]

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

Laterally brace chord above/below filler @ 24" O.C.
(or as designed) including a brace on chord directly
above/below both ends of filler (if no rigid diaphragm
exists at that point)

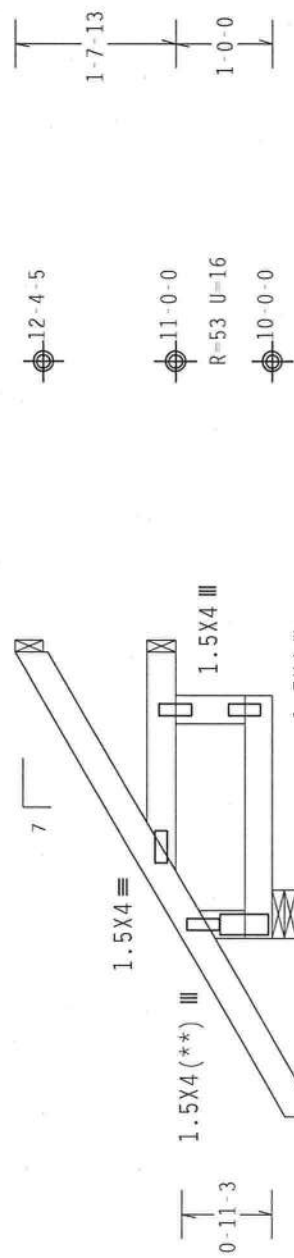
(**) 1 plate(s) require special positioning. Refer to scaled
plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt. ASCE 7-05, CLOSED bldg, Located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC
DL=5.0 psf. IW=1.00 GCpi(+/-)=0.18

Wind reactions based on MMFRS pressures.

Fasten rated sheathing to one face of this frame.

R-58 U=34



←-1-10-0-→

0-8-5 1-9-11 d-5-6
12-11-6 Over 3 Supports

R=285 U=51 W=6"
RL=94/-54

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

QTY:2 FL/-/4/-/-/R/- Scale = .5" /Ft.

TC LL	20.0 PSF	REF	R8228- 29533
TC DL	10.0 PSF	DATE	09/30/09
BC DL	10.0 PSF	DRW	HCUSR8228 09273034
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	46414
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TVI82282703

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. THE TRUSS CONTRACTOR SHALL BE RESPONSIBLE FOR THE TRUSS BEING INSTALLED IN THE CORRECT POSITION AND ORIENTATION. THE TRUSS CONTRACTOR SHALL BE RESPONSIBLE FOR THE TRUSS BEING INSTALLED IN THE CORRECT POSITION AND ORIENTATION. THE TRUSS CONTRACTOR SHALL BE RESPONSIBLE FOR THE TRUSS BEING INSTALLED IN THE CORRECT POSITION AND ORIENTATION.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC, BY AISC) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/1666 (A-70/SS/RT) ASTM A553 GRADE 40/60 (4, 8/11/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGNER. THE LIABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

Sep 30 09

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

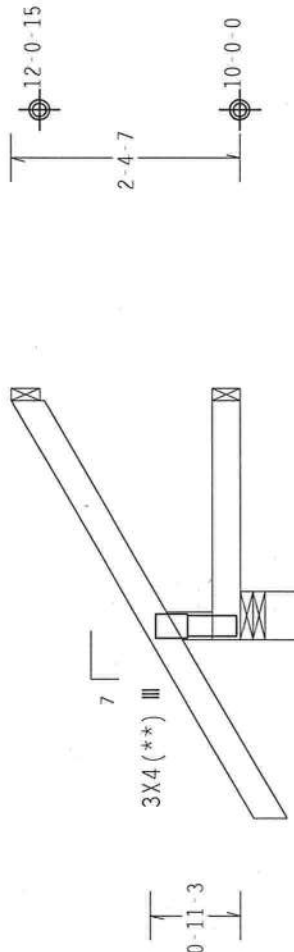
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Roof overhang supports 2.00 psf soffit load.
Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, Exp C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MMFRS pressures.

R-19 Rw=24 U=26



R-54 U=13

1-10-0
2-5-9 Over 3 Supports
R=273 U=51 W=6"
RL=85/-52

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

Scale = .5"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

QTY: 4

9.02.00

PLT TYP. Wave

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 2108 ROBERT STREET, SUITE 312, ALPHARETTA, GA 30004-1000. TRUSSES ARE TO BE CONSIDERED PRE-ENGINEERED UNLESS OTHERWISE STATED. ADDITIONAL PRECAUTIONS PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. CONNECTOR PLATES ARE MADE OF 2018/1666A (H/H/SS/RS) ASTM A653 GRADE 40/60 (H, K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

James P. Bullins, Jr.
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Sep 30 '09

REF	R8228- 29534
DATE	09/30/09
DRW	HCUSR8228 09273028
HC-ENG	JB/DF
SEQN	46421
FROM	AH
JREF	ITV18228Z03

(9-136--Doug Edgley Tuwuana Rossin -- , ** - HJ4B)

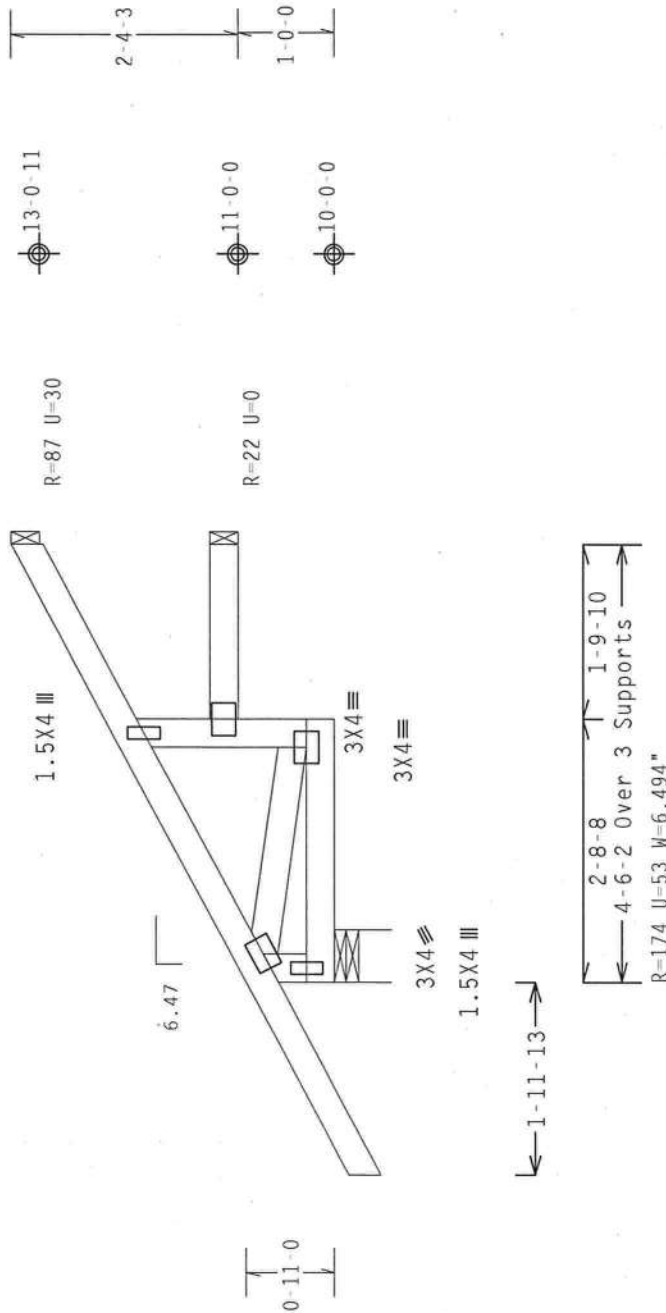
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Hipjack supports 3-2-4 setback jacks with no webs.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.



Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

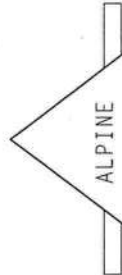
Scale = .5"/Ft.

PLT TYP. Wave

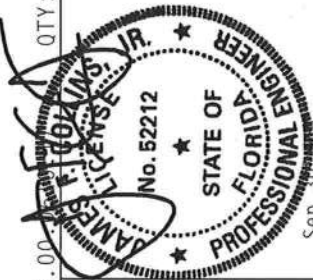
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TC DL		10.0 PSF	DRW HCUSR8228 09273027
BC DL		10.0 PSF	HC-ENG JB/DF
BC LL		0.0 PSF	SEQN- 46438
TOT.LD.		40.0 PSF	FROM AH
DUR.FAC.		1.25	JREF- 1TV18228Z03
SPACING		24.0"	

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/SS/RS) ASTM A653 GRADE 40/60 (W. K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. AN INTERRUPTION OF PLATE STANCHIONS (1) SHALL BE FOR AMERICAN STANDARD TPI-2002 (C-3). (2) SHALL BE FOR THE DESIGNER'S RESPONSIBILITY. THE DESIGNER SHALL BE RESPONSIBLE FOR THE TRUSS CONFORMS TO THE DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278



(9-136--Doug Edgley Tuwuana Rossin -- ** - EJ4B)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MMFRS loads based on trusses located at least 7.50 ft. from roof edge.

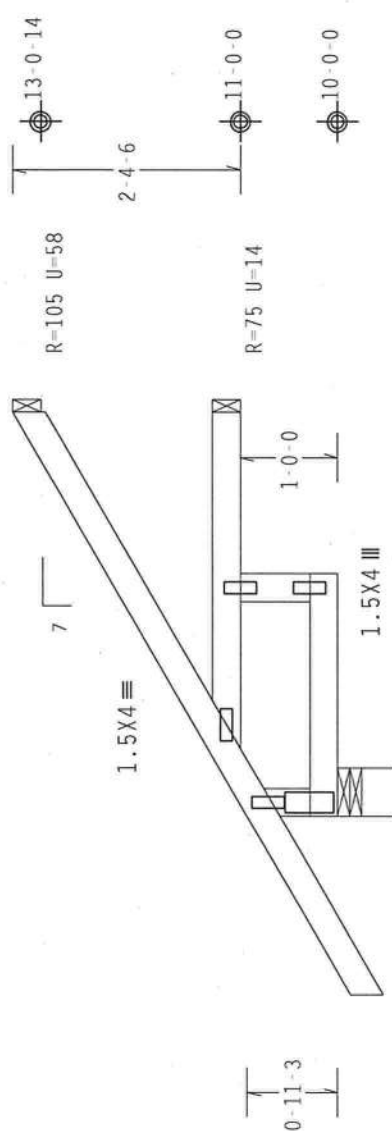
Laterally brace chord above/below filler @ 24" O.C.
(or as designed) including a brace on chord directly above/below both ends of filler (if no rigid diaphragm exists at that point)

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi(+/-)=0.18

Wind reactions based on MMFRS pressures.

Fasten rated sheathing to one face of this frame.



1.5X4 (**) III
2.5X6 III

1-10-0

0-8-5 1-9-11 1-8-0

4-2-0 Over 3 Supports

R=325 U=54 W=6"
RL=119/60

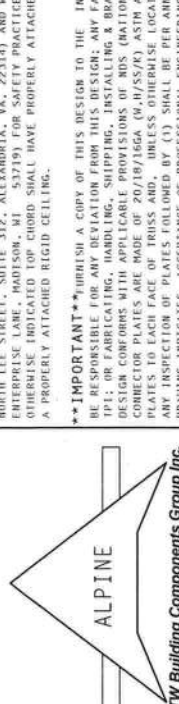
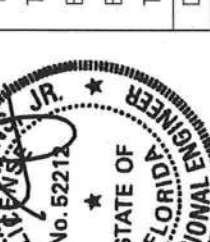
Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

9.02.00

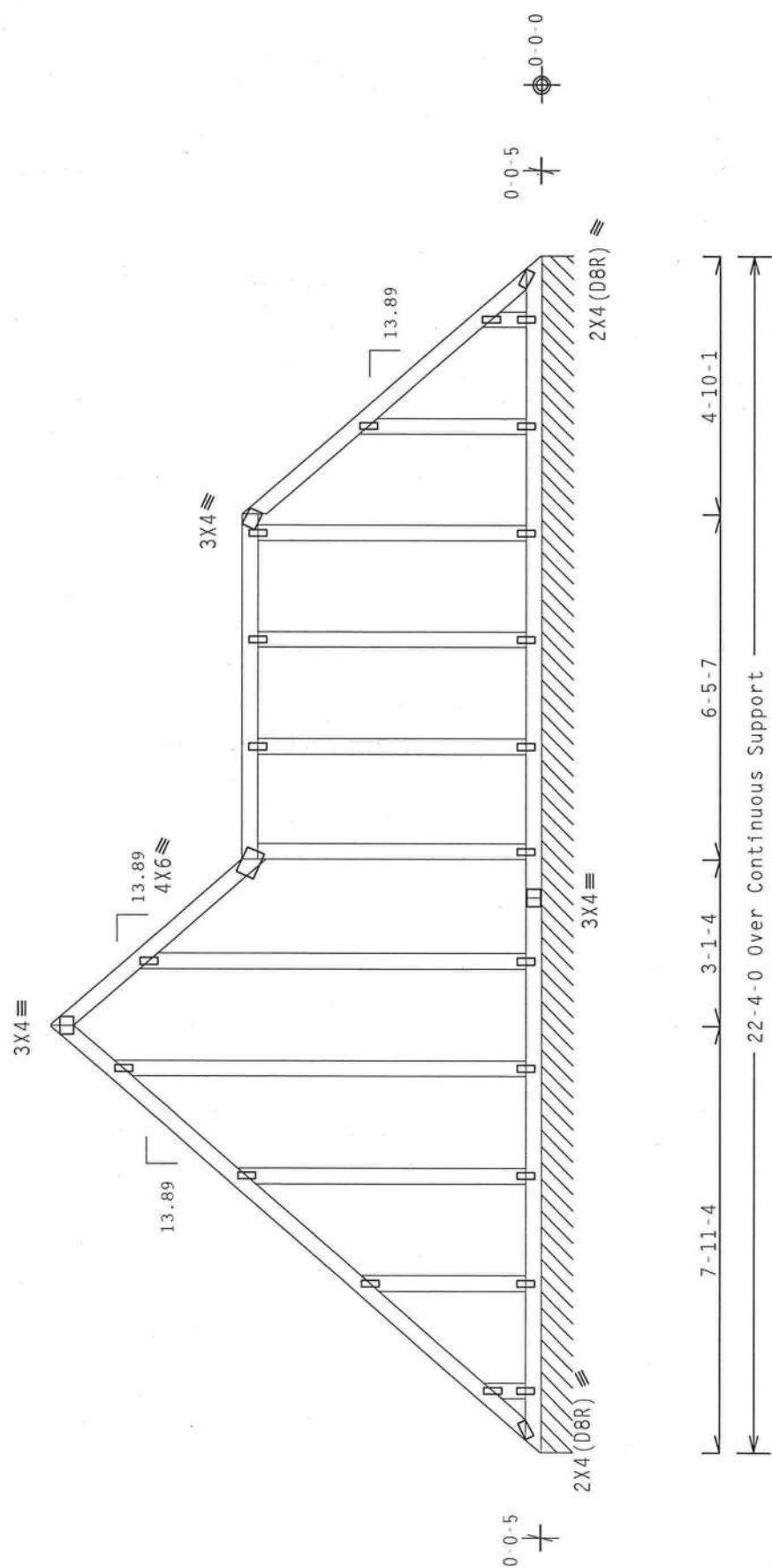
FL/-4/-/-R/-

Scale = .5"/Ft.

 ALPINE ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0278	**WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WPCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 48070) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.		 Sep 30 2009		TC LL	20.0 PSF	REF	R8228- 29536
					TC DL	10.0 PSF	DATE	09/30/09
					BC DL	10.0 PSF	DRW	HCUSR8228 09273029
					BC LL	0.0 PSF	HC-ENG	JB/DF
					TOT.LD.	40.0 PSF	SEQN-	46431
					DUR.FAC.	1.25	FROM	AH
				SPACING	24.0"	JREF-	1TVI8228Z03	

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

NOTE: THIS DESIGN IS NOT TO BE USED AS A ROOF TRUSS. IT IS ONLY TO BE USED AS A ROOF HIP FRAME.
SEE DETAIL HIPFRAME0109 FOR MORE INFORMATION.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

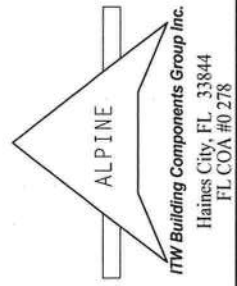
Scale = .3125"/Ft.

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TC DL	10.0 PSF	DATE	09/30/09
BC DL	10.0 PSF	DRW	HCUSR8228 09273032
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	47407
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	ITV18228Z03



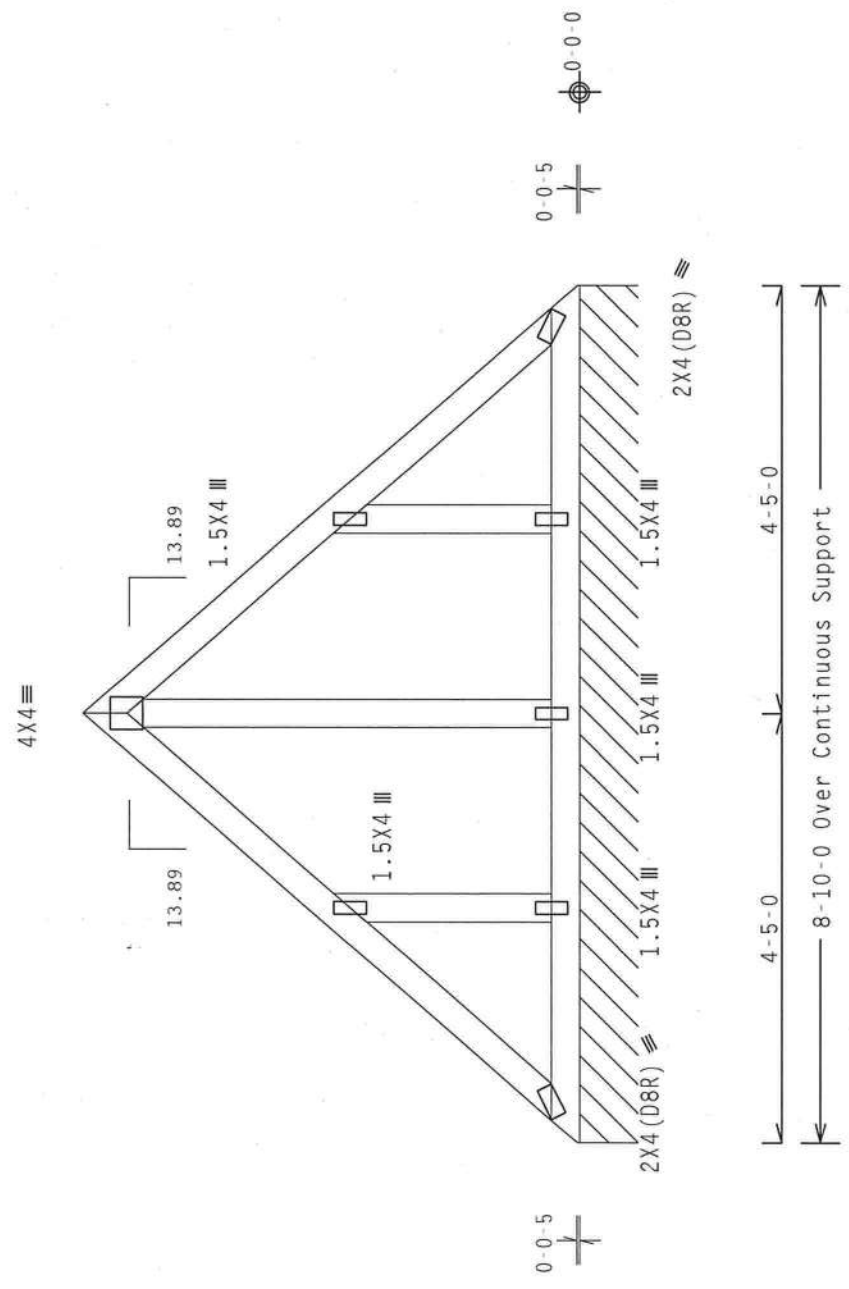
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. THE TRUSS DESIGNER SHALL BE RESPONSIBLE FOR THE TRUSS IN CONFORMANCE WITH THE FOLLOWING: NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314; AND VICA (WOOD TRUSS COUNCIL OF AMERICA), ENTERPRISE LANE, MADISON, WI 53719. FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 2019/1666 (W/55/8) ASTM A653 GRADE 40/60 (W/55/8) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DIMENSIONS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANSI A3 OF TPI-2002 SEC.3. A SEAL ON THIS DESIGN SIGNIFIES THE DESIGNER'S RESPONSIBILITY FOR THE TRUSS CONFORMS TO THE DESIGN AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

NOTE: THIS DESIGN IS NOT TO BE USED AS A ROOF TRUSS. IT IS ONLY TO BE USED AS A ROOF HIP FRAME.
SEE DETAIL HIPFRAME0109 FOR MORE INFORMATION.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

QTY: 1 FL/-/4/-/-/R/- Scale = .5"/Ft.

 ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0 278	TC LL 20.0 PSF		REF R8228- 29538
	TC DL 10.0 PSF		DATE 09/30/09
	BC DL 10.0 PSF		DRW HCUSR8228 09273002
	BC LL 0.0 PSF		HC-ENG JB/DF
	TOT.LD. 40.0 PSF		SEQN- 47410
	DUR.FAC. 1.25		FROM AH
SPACING 24.0"		JREF- 1TV18228Z03	

JAMES E. COLLINS JR.
LICENSED PROFESSIONAL ENGINEER
STATE OF FLORIDA
No. 52212

Sep 30 09

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY ITPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA), ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

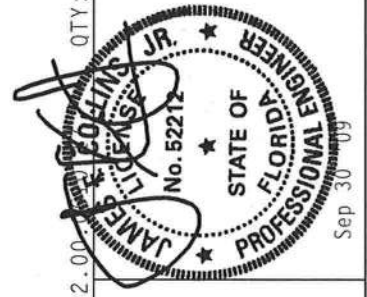
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH ITPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/P&A) AND ITPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/V/SS/K) ASTM A653 GRADE 40/60 (M, K/H-55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMBEX A3 OF ITPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMBEX/ITPI 3 SEC. 2.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

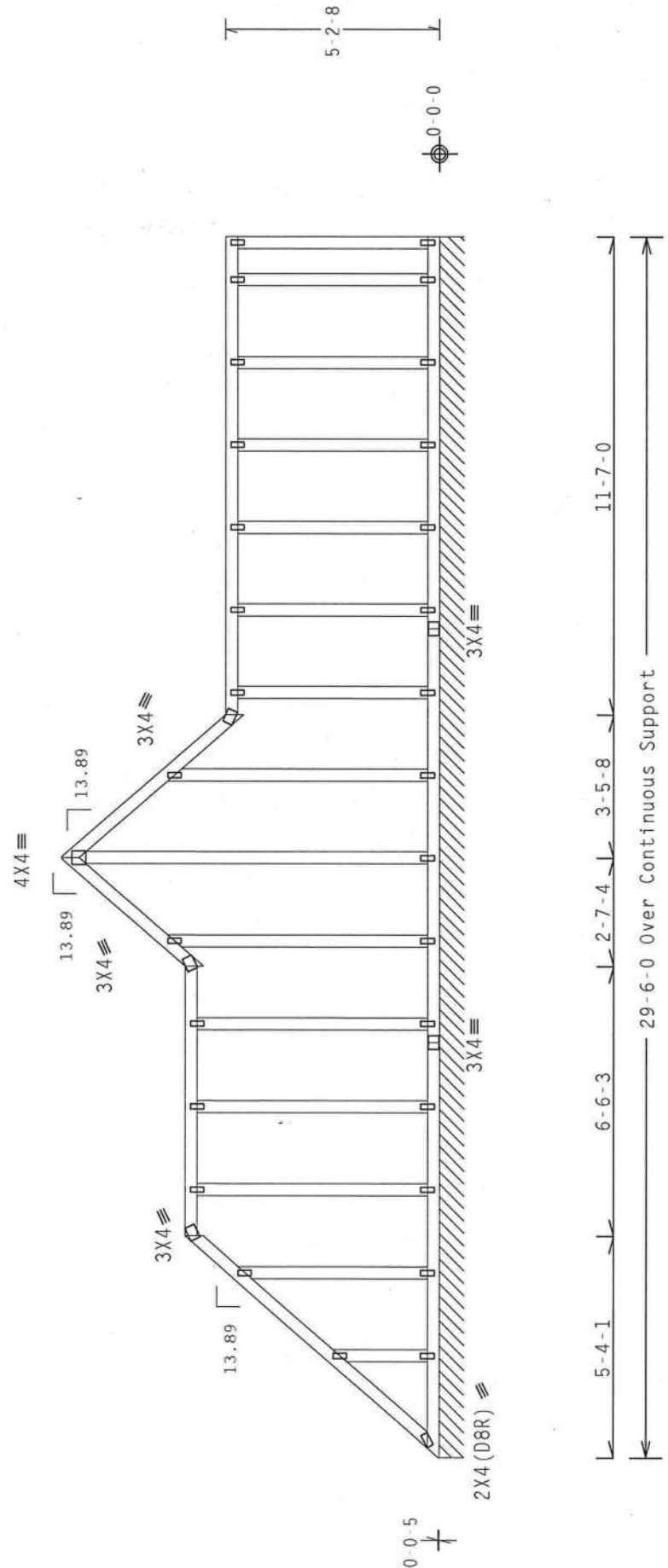
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/SS/RS) ASTM A653 GRADE 40/60 (W/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. PROVISIONS OF PLATES FOLLOWED (UNLESS OTHERWISE NOTED) SHALL BE THE RESPONSIBILITY OF THE TRUSS COMPANY. DRAWING INDICATES THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.


Sep 30 2009

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

NOTE: THIS DESIGN IS NOT TO BE USED AS A ROOF TRUSS. IT IS ONLY TO BE USED AS A ROOF HIP FRAME.
SEE DETAIL HIPFRAME0109 FOR MORE INFORMATION.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

9.02.00

QTY:1

FL/-/4/-/R/-

Scale = .25" / Ft.

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TC DL	10.0 PSF	DATE	09/30/09
BC DL	10.0 PSF	DRW	HCUSR8228 09273021
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN -	47388
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF -	1TVI8228Z03

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 ROTHREE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 WOODHURST DRIVE, SUITE 100, FORT WORTH, TEXAS 76116) FOR TRUSS BRACING REQUIREMENTS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AF&PA) AND TPI. 1TH BCG CONNECTOR PLATES ARE MADE OF 2018/16GA (44/SS/RS) ASTM A653 GRADE 40/60 (H, K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANNI/TPI 1 SEC. 2.

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

STATE OF FLORIDA
PROFESSIONAL ENGINEER
No. 52212
Sep 2009

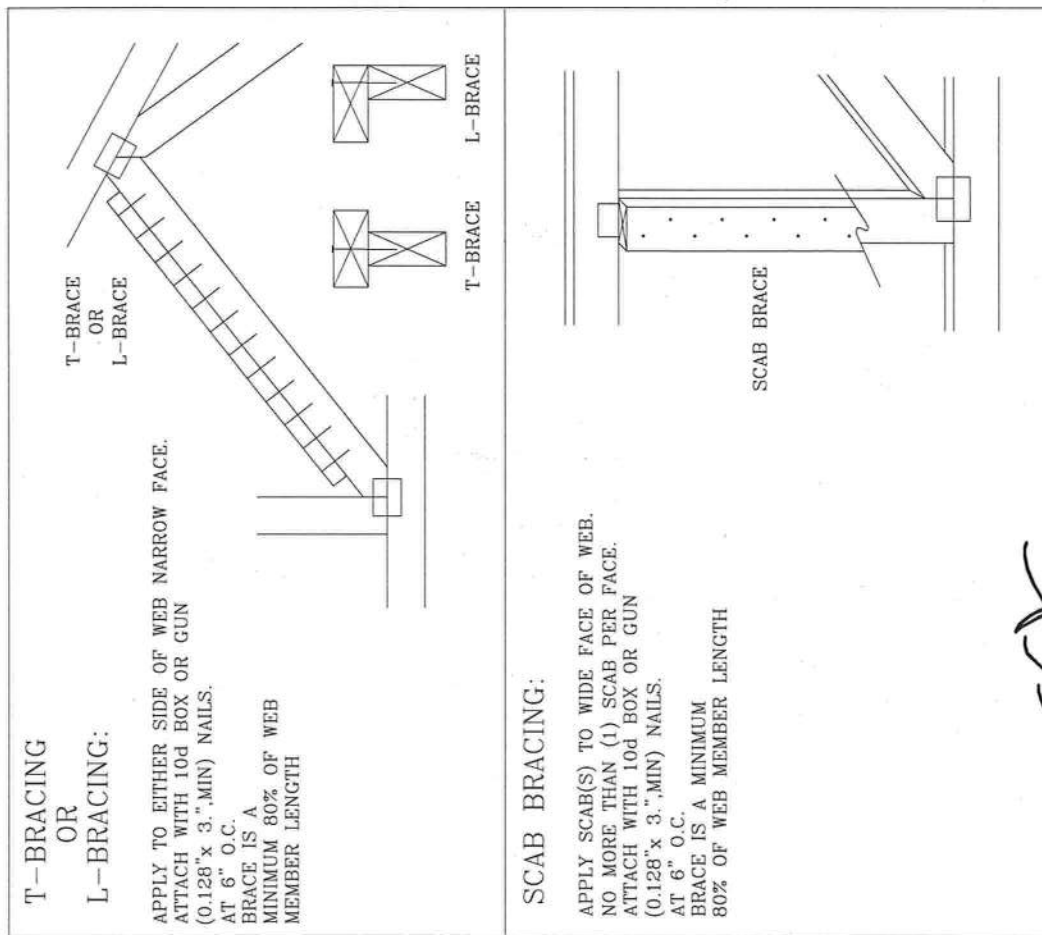
THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED
C1CB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB
BRACING.

WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

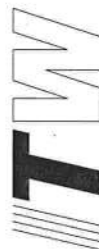


*****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the BCS1 Building Component Safety Information, by TPI and WTCA for safety practices prior to performing any work. Trusses shall have property attached otherwise, top chord shall have property attached structural panels and bottom chord shall have a property attached rigid bracing. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCS1 sections B3 & B7. See this job's general notes page for more information.

****IMPORTANT**** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

The Building Components Group Inc. (BTWCG) shall not be responsible for any deviation from this design, or any failure to build the truss in conformance with TPI, or for overloading, handling, shipping, installing, storing, or otherwise misusing BTWCG components. The purchase of additional BTWCG products per drawings shall require approval by BTWCG. Details A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANS/TPI 1 Sec. 2.



Building Components Group Inc.

Earth City, MO 63045

TO TC BC BC TC BC SE

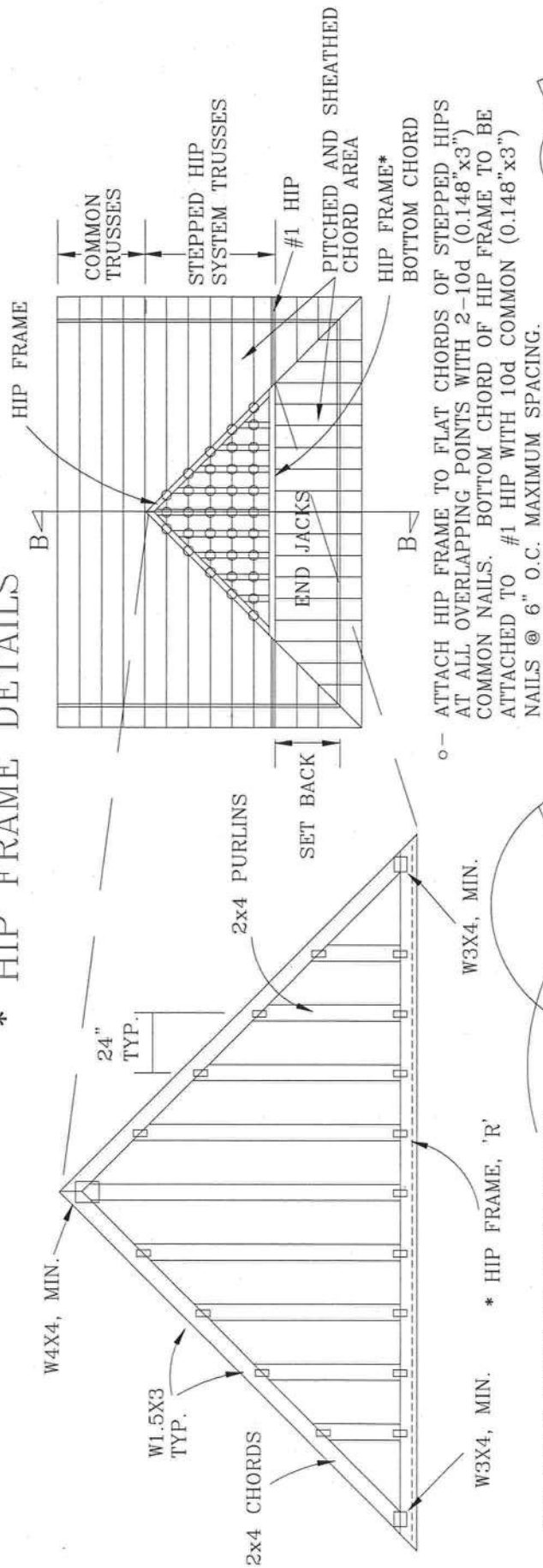
Handwritten signature: *[Signature]*

Seal: JAMES F. COLLINS, JR. ★
LICENSE No. 52212 ★
STATE OF FLORIDA ★
PROFESSIONAL ENGINEER

Stamp: SEP 30 '09

TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	1/1/09
BC DL	PSF	DRWG	BRCLBSUB0109
BC LL	PSF		
TOT. LD.	PSF		
EUR. FAC.			
SPACING			

* HIP FRAME DETAILS



HIP FRAME STOPS AT PLUMB CUT OF JACKS TO MAINTAIN PITCH CONTINUITY.

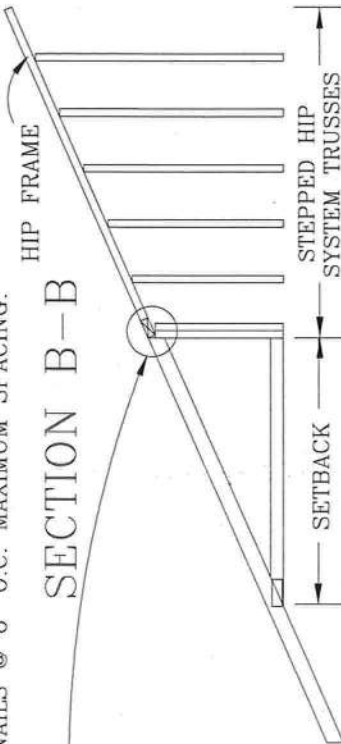
USE THIS DETAIL FOR ASCE 7-02 OR ASCE 7-05 WIND, 110 MPH, 30' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=4.2 PSF, Kzt=1.00

* HIP FRAME LUMBER IS SPF, SO. PINE, HF, OR DFL STANDARD, STUD GRADE, OR BETTER.

SEE ENGINEER'S SEALED DESIGN FOR SETBACK, LUMBER, PLATING, LOADING AND DURATION FACTOR REQUIRED.

'R' HIP FRAME CHORDS MAY BE TRIMMED UP TO 2" TO FIT.
PURLINS MUST BE INTACT AND PROPERLY ATTACHED.

HIP FRAME - PROVIDED BY TRUSS MANUFACTURER.
HIP FRAME IS DESIGNED TO PROVIDE BRACING FOR FLAT
TOP CHORDS OF HIP FRAME SYSTEM WHERE INDICATED.
STRUCTURAL PANELS MUST BE PROPERLY ATTACHED DIRECTLY
TO HIP FRAME PURLINS.



*****WARNING*** READ AND FOLLOW ALL NOTES ON THIS SHEET**

*****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET**

*****IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.**

THE BUILDING COMPONENTS GROUP, INC. (BTBCG) shall not be responsible for any deviation from this design, specification or drawing. THE BUILDING COMPONENTS GROUP, INC. (BTBCG) shall not be responsible for any failure to build the truss in conformance with TPI, or fabricating, installing, and erecting of trusses. BTBCG connector plates are made of 2018/T8GA (WH/S/K) ASTM A653 grade 37/40/60 galv. steel. Apply plates at each face of truss, positioned as shown above and on Joint Details. The drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

REF HIP FRAME

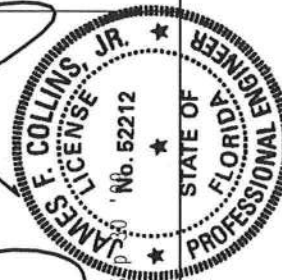
DATE 1/1/09

DRWG HIPFRAME0109

W
T

Building Components Group Inc.

Earth City, MO 63045





Permit # 28035
GTC Design Group, LLC
176 NW Lake Jeffery Road
Lake City, FL 32643
(Phone) 386.719.9985
(Fax) 386.719.8828
cwilliams@gtcdesigngroup.com

Finish Floor Elevation Letter

Contractor: Edgley Construction Co.

Owner: Tuwuana Rossin

Parcel Number: 18-5s-16-03644-107

Parcel Description: Lot 7, White Oak Plantation

For protection against water damage, the minimum finish floor elevation of the proposed structure shall be 8 inches above the existing ground at any point along the perimeter of the proposed structure. In no case shall the finish floor elevation be greater than 34" below the centerline elevation of the adjacent roadway.

The ground around the proposed structure shall be graded such as to convey all stormwater runoff away from the proposed structure.

The above elevations are based on the structure's proposed location, south of SW CR 240.



8-28-09

Chad Williams
P.E. License Number: 63144
August 28, 2009

finished floor
cont. elev. per
ads @ sub.
9-1-09

SUBCONTRACTOR VERIFICATION FORM

Permit # 28035 APPLICATION NUMBER 28035 CONTRACTOR Edgley Const Co. PHONE 386-752-0580
 THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is **REQUIRED** that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL	Print Name _____ License #: _____	Signature _____ Phone #: _____
MECHANICAL/ A/C _____	Print Name _____ License #: _____	Signature _____ Phone #: _____
PLUMBING/ GAS	Print Name _____ License #: _____	Signature _____ Phone #: _____
ROOFING	Print Name _____ License #: _____	Signature _____ Phone #: _____
SHEET METAL	Print Name _____ License #: _____	Signature _____ Phone #: _____
FIRE SYSTEM/ SPRINKLER	Print Name _____ License #: _____	Signature _____ Phone #: _____
SOLAR	Print Name _____ License #: _____	Signature _____ Phone #: _____

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON	000097	Kenneth Loudon	Kenneth Loudon
CONCRETE FINISHER			
FRAMING			
INSULATION			
STUCCO			
DRYWALL			
PLASTER			
CABINET INSTALLER			
PAINTING			
ACOUSTICAL CEILING			
GLASS			
CERAMIC TILE			
FLOOR COVERING			
ALUM/VINYL SIDING			
GARAGE DOOR			
METAL BLDG ERECTOR			

F. S. 440.103 Building permits; identification of minimum premium policy.—Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each

SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER #28035 CONTRACTOR Edgley Const Co. PHONE 752-0580

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is **REQUIRED** that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL	Print Name _____ License #: _____	Signature _____ Phone #: _____
MECHANICAL/ A/C	Print Name _____ License #: _____	Signature _____ Phone #: _____
PLUMBING/ GAS	Print Name _____ License #: _____	Signature _____ Phone #: _____
ROOFING	Print Name _____ License #: _____	Signature _____ Phone #: _____
SHEET METAL	Print Name _____ License #: _____	Signature _____ Phone #: _____
FIRE SYSTEM/ SPRINKLER	Print Name _____ License #: _____	Signature _____ Phone #: _____
SOLAR	Print Name _____ License #: _____	Signature _____ Phone #: _____

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON			
CONCRETE FINISHER			
FRAMING			
INSULATION			
STUCCO			
DRYWALL			
PLASTER	000256	REN DAVID PLASTERING LLC	Ron David
CABINET INSTALLER			
PAINTING			
ACOUSTICAL CEILING			
GLASS			
CERAMIC TILE			
FLOOR COVERING			
ALUM/VINYL SIDING			
GARAGE DOOR			
METAL BLDG ERECTOR			

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each

J. Kossin

SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER

28035

CONTRACTOR

DOUG EDGLEY

PHONE

386.7520580

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is **REQUIRED** that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL	Print Name _____ License #: _____	Signature _____ Phone #: _____
MECHANICAL/ A/C _____	Print Name _____ License #: _____	Signature _____ Phone #: _____
PLUMBING/ GAS	Print Name _____ License #: _____	Signature _____ Phone #: _____
ROOFING	Print Name _____ License #: _____	Signature _____ Phone #: _____
SHEET METAL	Print Name _____ License #: _____	Signature _____ Phone #: _____
FIRE SYSTEM/ SPRINKLER	Print Name _____ License #: _____	Signature _____ Phone #: _____
SOLAR	Print Name _____ License #: _____	Signature _____ Phone #: _____

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON			
CONCRETE FINISHER			
FRAMING			
INSULATION			
STUCCO			
DRYWALL			
✓ PLASTER JW	000256	BEN DAVID PLASTERING LLC	Ryan David
✓ CABINET INSTALLER JW	000762	Bordeaux Cabinets Inc.	Stew Bordeaux
PAINTING			
ACOUSTICAL CEILING			
GLASS			
CERAMIC TILE			
FLOOR COVERING			
ALUM/VINYL SIDING			
GARAGE DOOR			
METAL BLDG ERECTOR			

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

COLUMBIA COUNTY OFFICE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 18-5S-16-03644-107

Building permit No. 000028035

Use Classification SFD/UTILITY

Fire: 25.68

Permit Holder DOUG EDGLEY

Waste: 67.00

Owner of Building TUWUANA O. ROSSIN

Total: 92.68

Location: 1159 SW ICHETUCKNEE AVE., LAKE CITY, FL



Date: 06/10/2010

Fanny Dickie

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



GTC Design Group, LLC
176 NW Lake Jeffery Road
Lake City, FL 32643
(Phone) 386.719.9985
(Fax) 386.719.8828
cwilliams@gtcdesigngroup.com

Finish Floor Elevation Certification

Contractor: Edgley Construction Co.

Owner: Tuwuana Rossin

Parcel Number: 18-5s-16-03644-107

Parcel Description: Lot 7, White Oak Plantation

On September 18, 2009, GTC Design Group LLC verified the heated and cooled Finish Floor Elevation with a Nikon, Automatic Level, Model AP-8. The following are actual elevation shots.

Finish Floor Elevation (FFE)	4.26'
Centerline SW Ichetucknee Avenue	5.26'
Centerline SW CR 240	2.04'

The above numbers translate to:

1. The FFE is 12" higher than centerline SW Ichetucknee Ave
2. The FFE is 27" lower than the centerline of SW CR 240.

The above elevations are based on the relationship between the centerline elevations of the denoted county roads at a location perpendicular to the structures proposed front door location.

Chad Williams
P.E. License Number: 63144
September 21, 2009

Permit # 28035
OK CSB
9-22-09