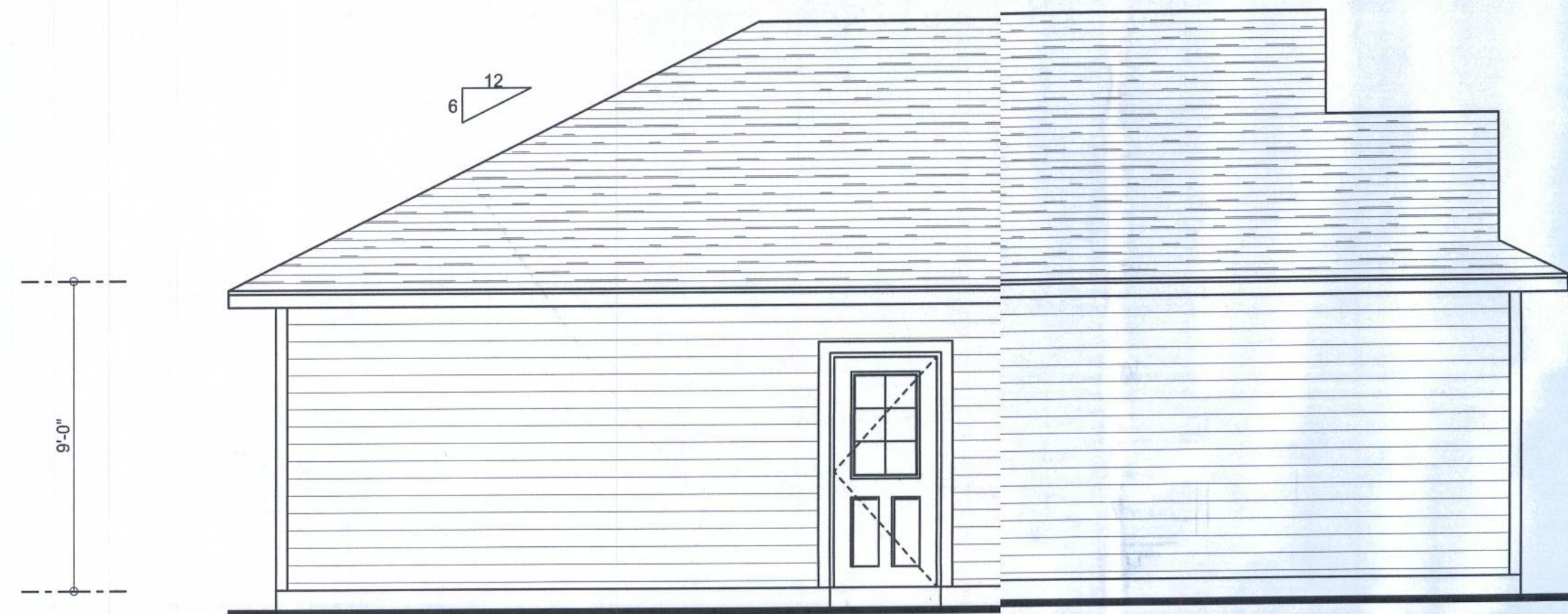
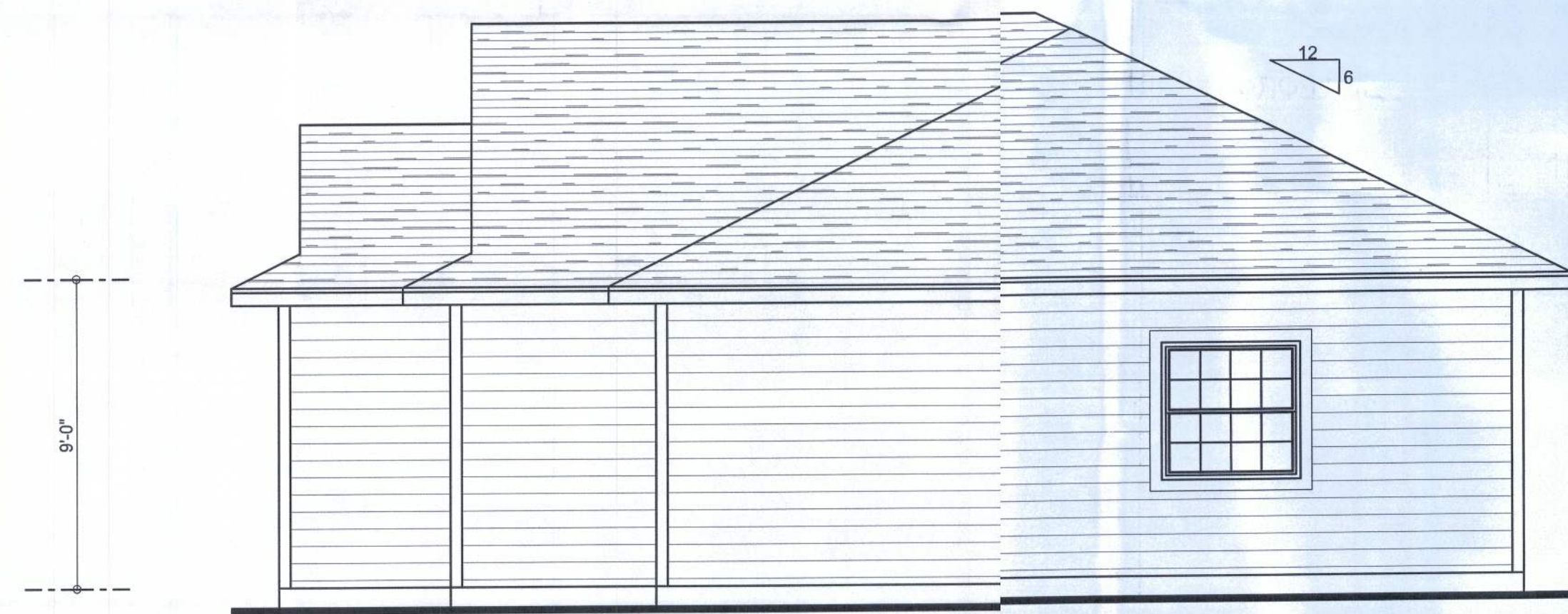




FRONT ELEVATION
SCALE: 1/4" = 1'-0"



LEFT ELEVATION
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"

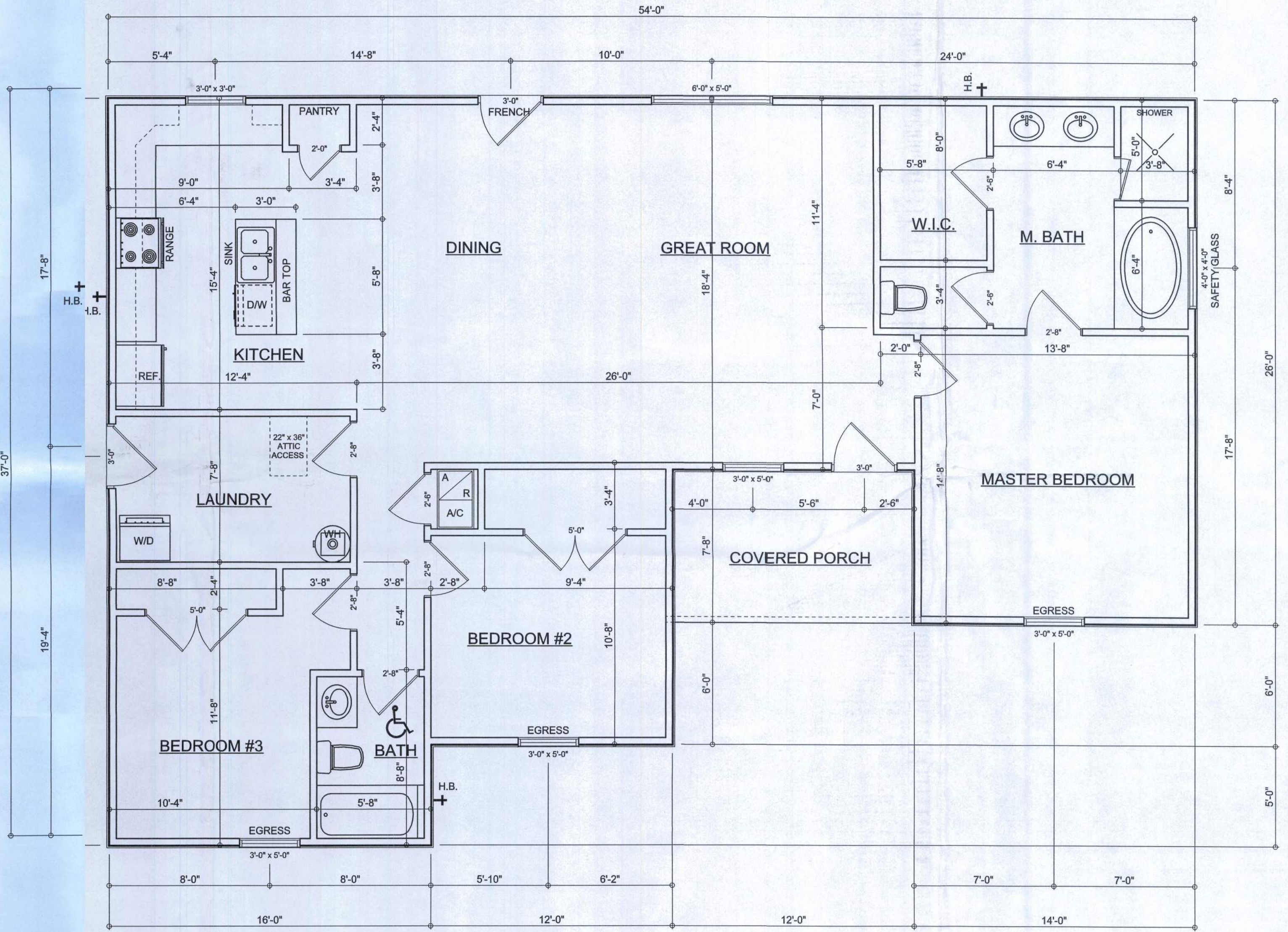
REQUIRED ROOF VENTILATION:
AS PER FLORIDA BUILDING CODE 2309.7

RIDGE VENT
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3' ABOVE EAVE)
1652 S.F. / 300 x 50% = 2.75 S.F. RIDGE VENT AREA REQUIRED
25.00 FEET OF RIDGE VENT REQUIRED

SOFFIT VENT
1652 S.F. / 300 x 50% = 2.75 S.F. SOFFIT VENT AREA REQUIRED
91.66 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

1. RIDGE VENTS = 16 IN2/FT (11 FT2/FT)
2. OFF-RIDGE VENTS = 70 FT2 PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN2/FT (.03 FT2/FT)



FLOOR PLAN

SCALE: 1/4" = 1'-0"
ALL CEILING HEIGHTS TO BE 9'-0" UNLESS NOTED OTHERWISE

AREA SUMMARY

LIVING AREA	1562	S . F .
PORCH AREA	90	S . F .
TOTAL AREA	1652	S . F .

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

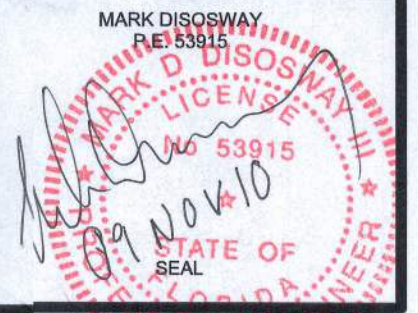
WINLOAD ENGINEER:
Mark Disosway, P.E.
No. 5915, P.O. Box 868, Lake City, FL 32056,
386-54-5419

DIMENSIONS:
Standard dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have executed this plan, and that the applicable portions of the plan, relating to engineering comply with section R302.1, Florida building code residential 2007, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Edgley Construction

Chad & Katie
Cunningham Residence

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Lake City, FL 32085

Mark Disosway P.E.
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Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
November 09, 2010

DRAWN BY: David Disosway STRUCTURAL BY: David Disosway

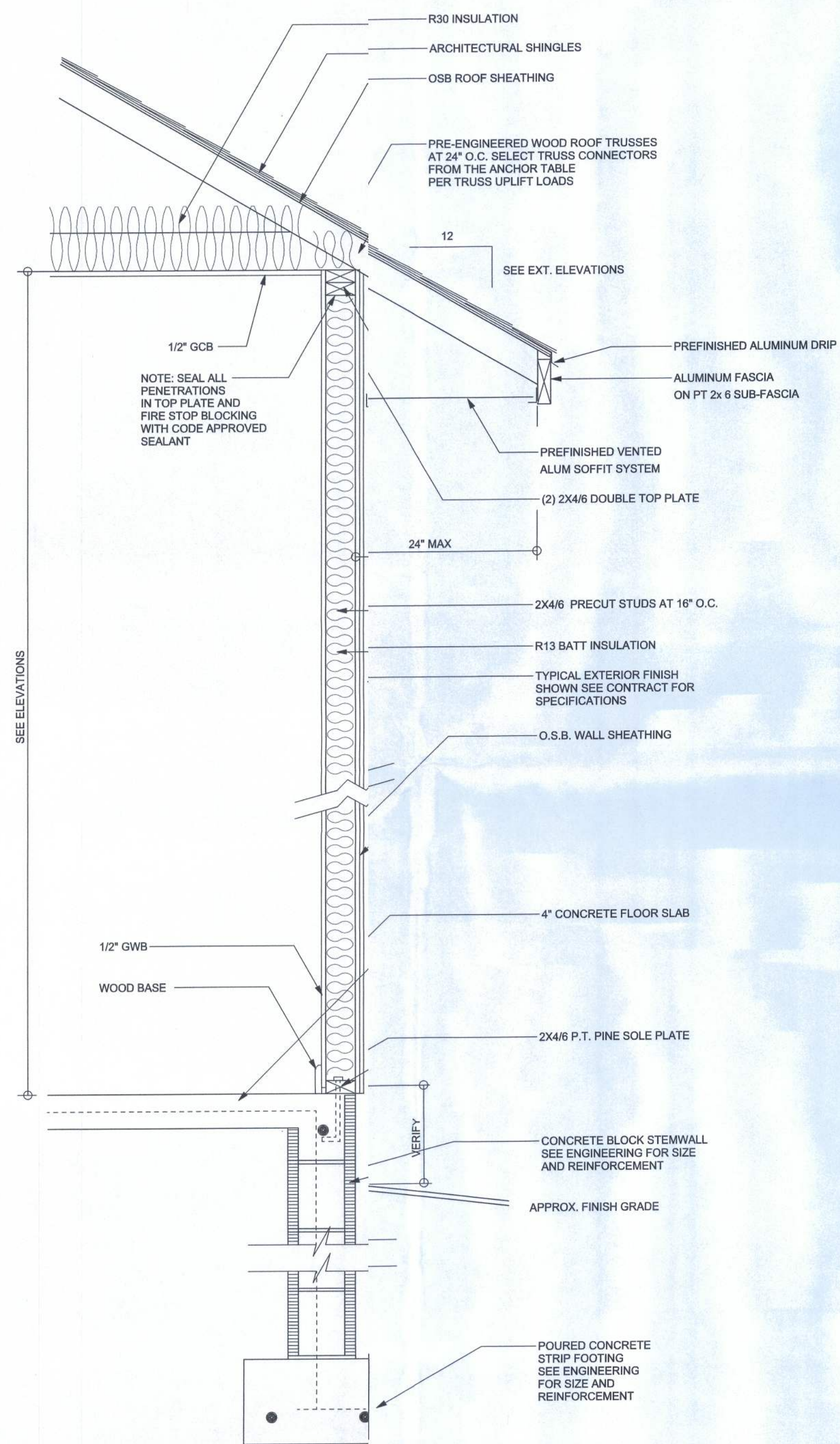
FILES DATE:
9/10/10

JOB NUMBER:
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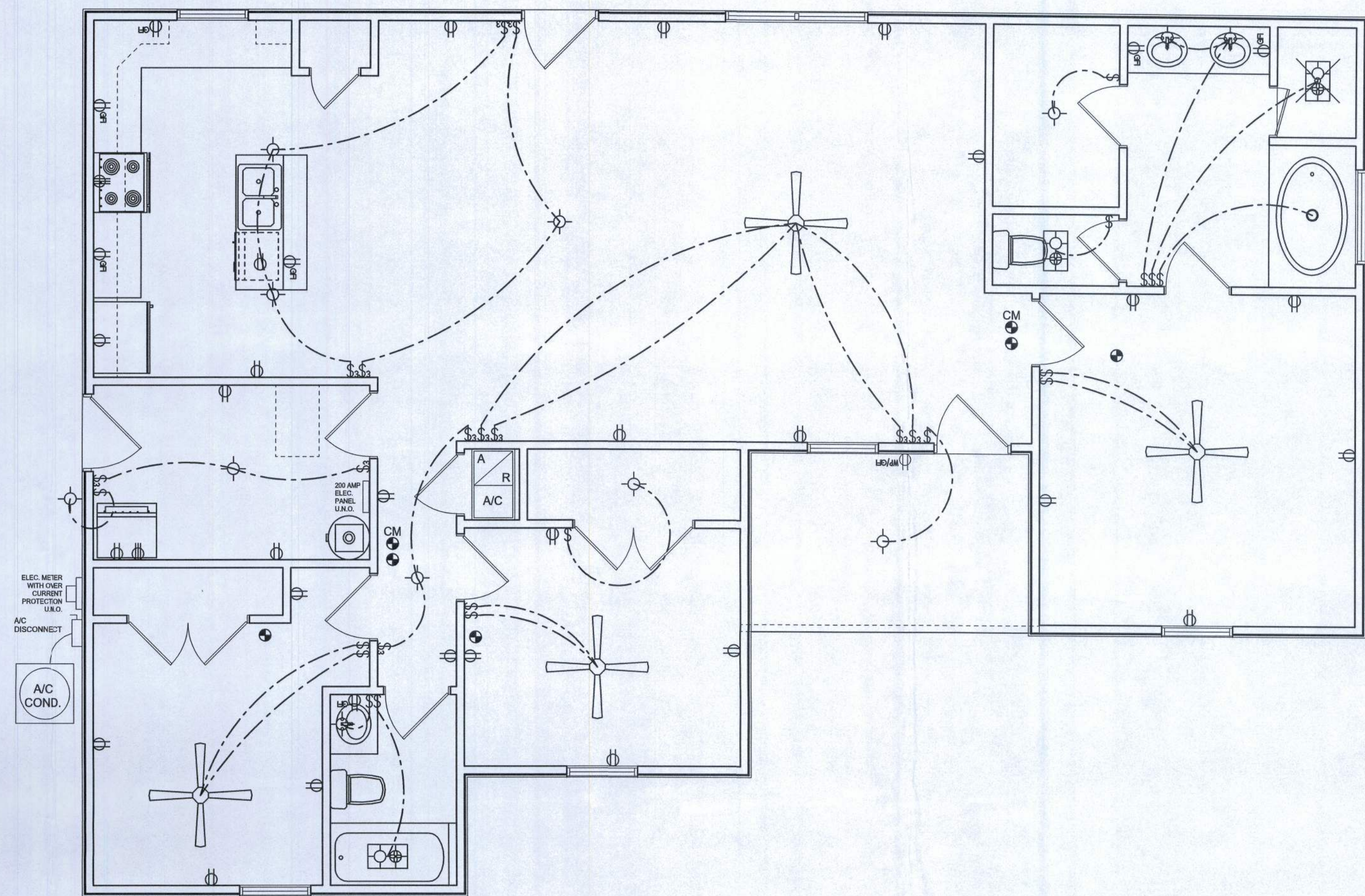
DRAWING NUMBER
1

OF 4 SHEETS





**TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA**
SCALE: 1\"/>



ELECTRICAL PLAN
SCALE: 1/4\"/>

ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPARATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL 120-VOLT, SINGLE-PHASE, 15- AND 20-AMPERE BRANCH CIRCUITS SUPPLYING OUTLETS INSTALLED IN DWELLING UNIT FAMILY ROOMS, DINING ROOMS, LIVING ROOMS, PARLORS, LIBRARIES, DENS, BEDROOMS, SUN ROOMS, RECREATION ROOMS, CLOSETS, HALLWAYS, OR SIMILAR ROOMS OR AREAS SHALL BE PROTECTED BY A LISTED ARC-FAULT CIRCUIT INTERRUPTER, COMBINATION-TYPE INSTALLED TO PROVIDE PROTECTION OF THE BRANCH CIRCUIT.
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL
- E -11 CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHIN 10' OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.
- E -12 ALL OUTLETS LOCATED IN RESIDENTIAL TO BE TAMPER-RESISTANT PER NEC.

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	CARBON MONOXIDE ALARM

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

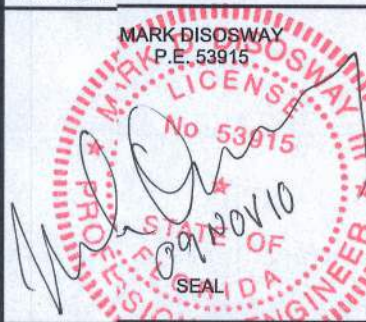
WINDLOADENGINEER:
Mark Disosway, P.E.
No.53915, DB 868, Lake City, FL 32056,
386-754-549

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 207, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



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PRINTED DATE:
November 09, 2010

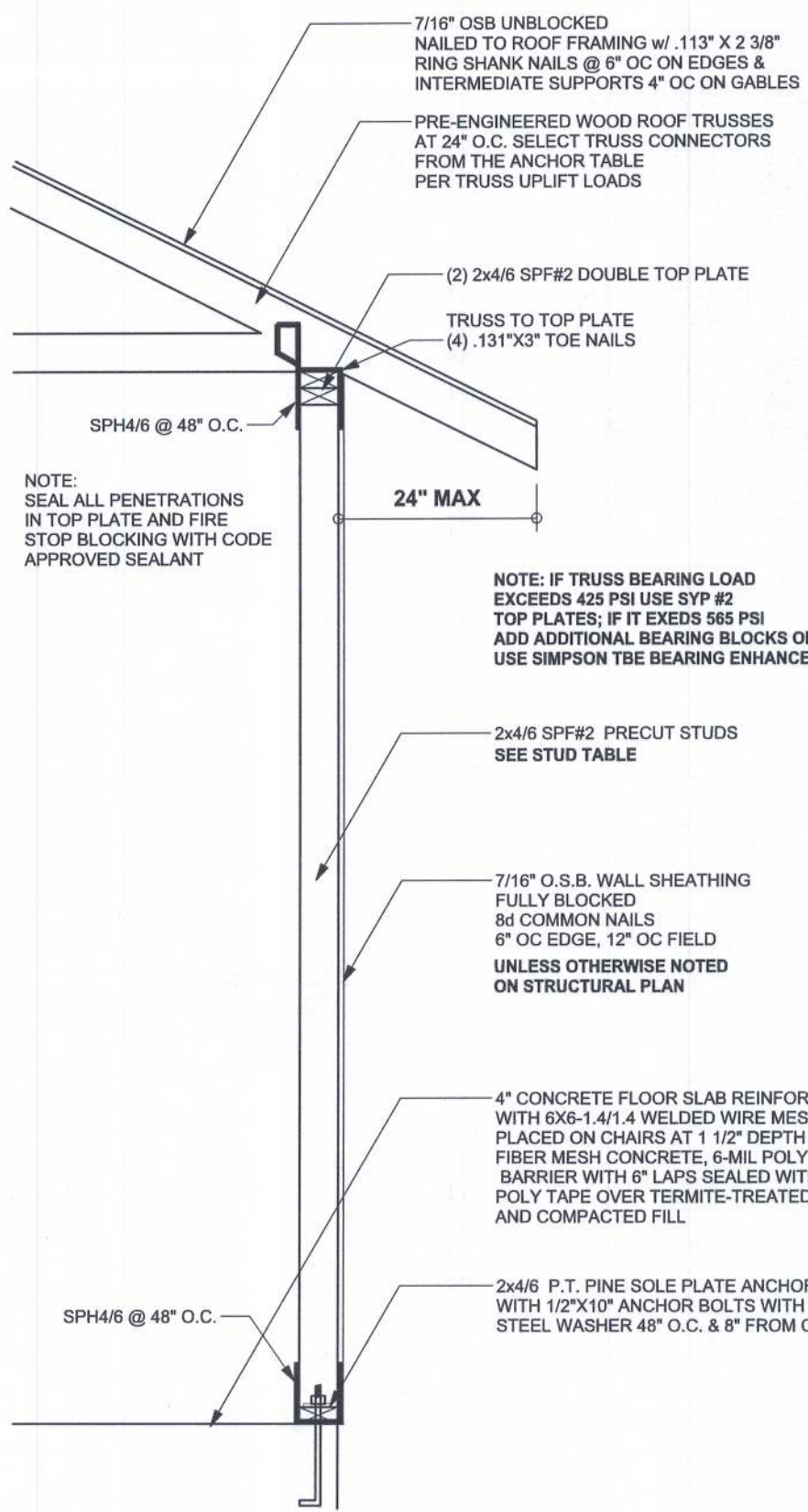
DRAWN BY: David Disosway
STRUCTURAL BY: David Disosway

FINALS DATE:
9 Nov 10

JOB NUMBER:
1010038

DRAWING NUMBER

2
OF 4 SHEETS

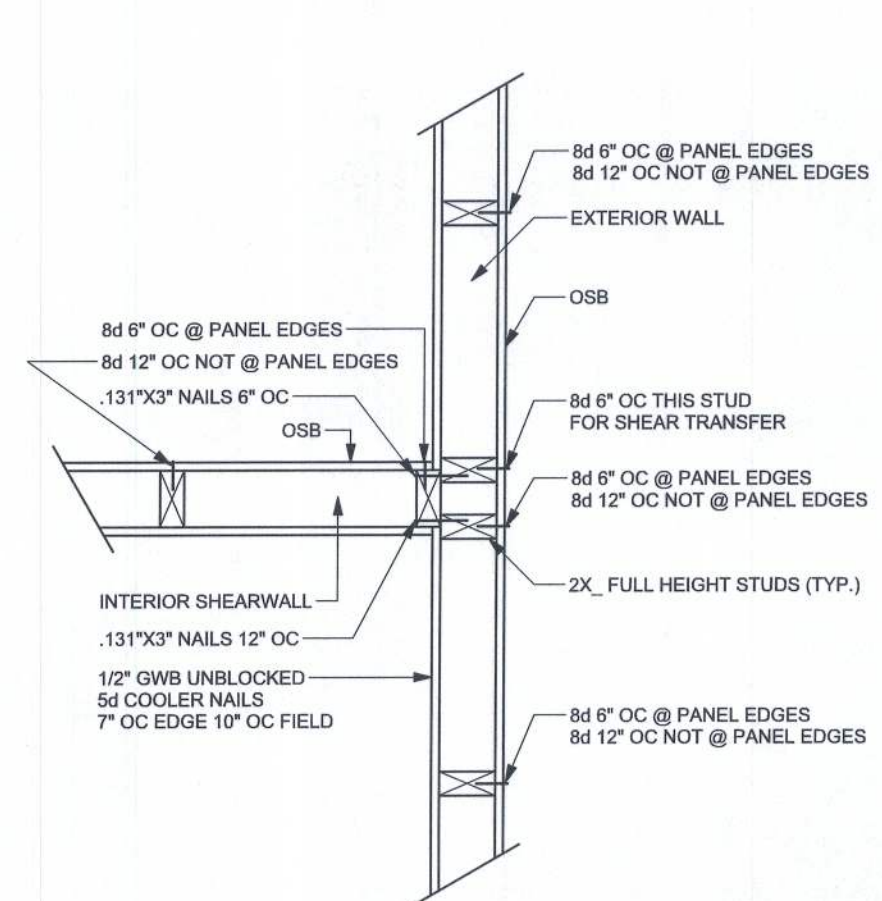


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

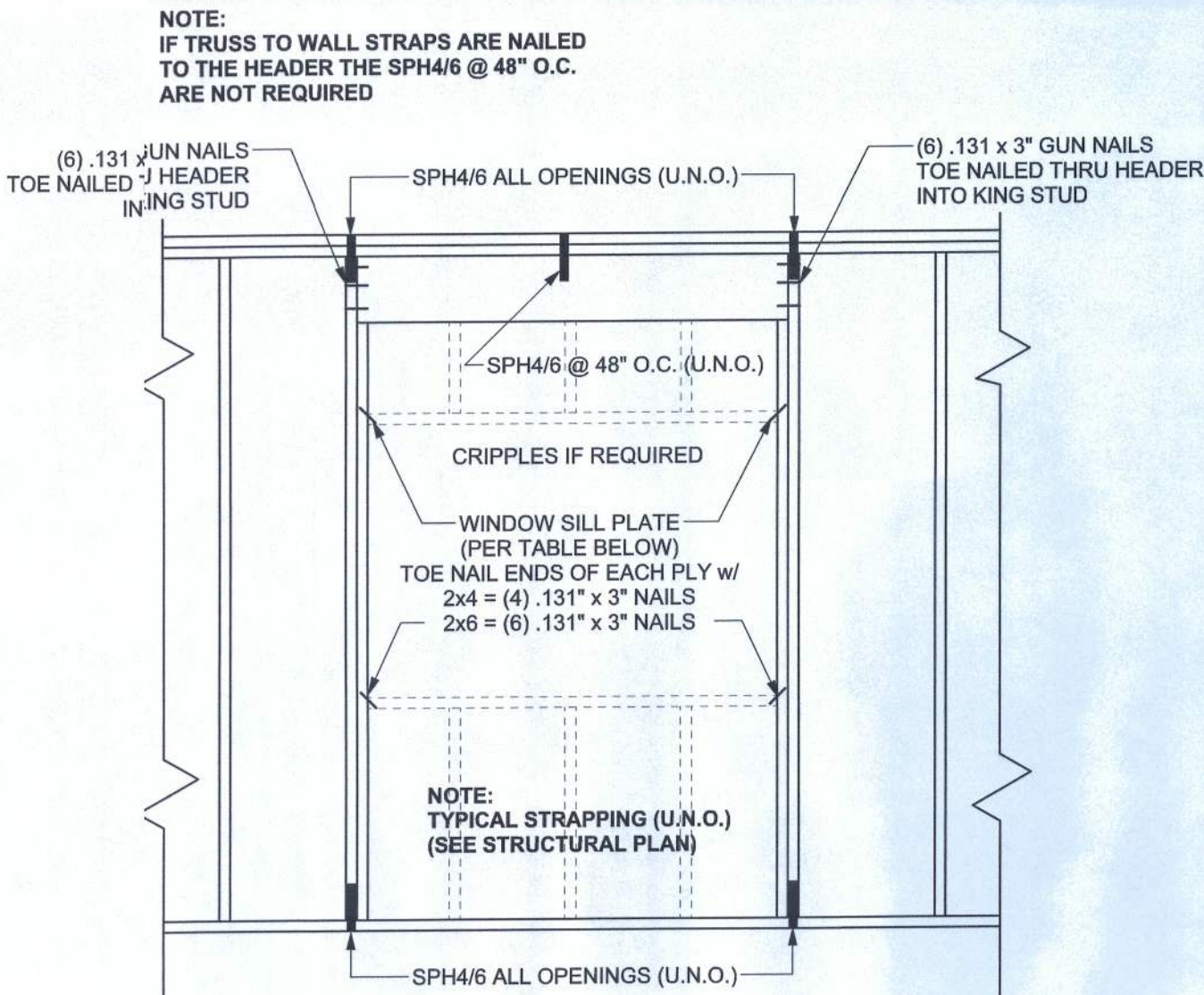
EXTERIOR WALL STUD TABLE
FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-6" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-7" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 18'-7" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE C. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.



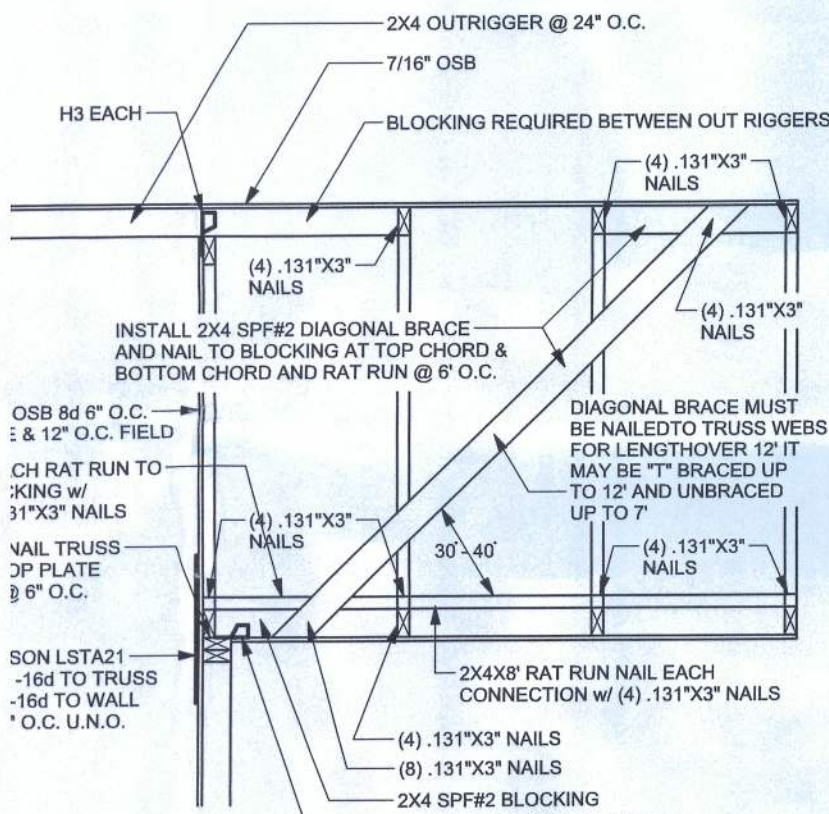
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



SILL PLATE SPANS FOR 10'-0" WALL HEIGHT

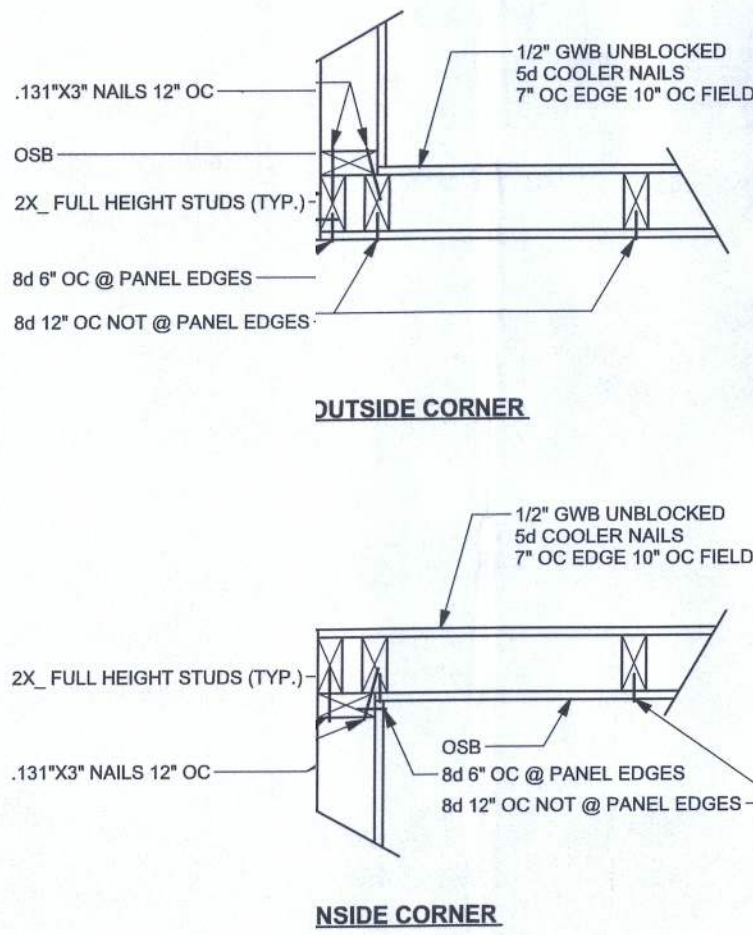
DESIGN WIND SPEED	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	TABLE A-3.20B
90-100 MPH	5'-3"	7'-9"	7'-8"	11'-4"	FOR OTHER WALL HEIGHTS PER SILL SPAN SHALL BE DIVIDED BY 4/5
110-120 MPH	4'-4"	6'-6"	6'-5"	9'-6"	
130 MPH	4'-0"	6'-0"	6'-11"	8'-0"	

TYPICAL HEADER STRAPPING DETAIL
SCALE: 1/2" = 1'-0"

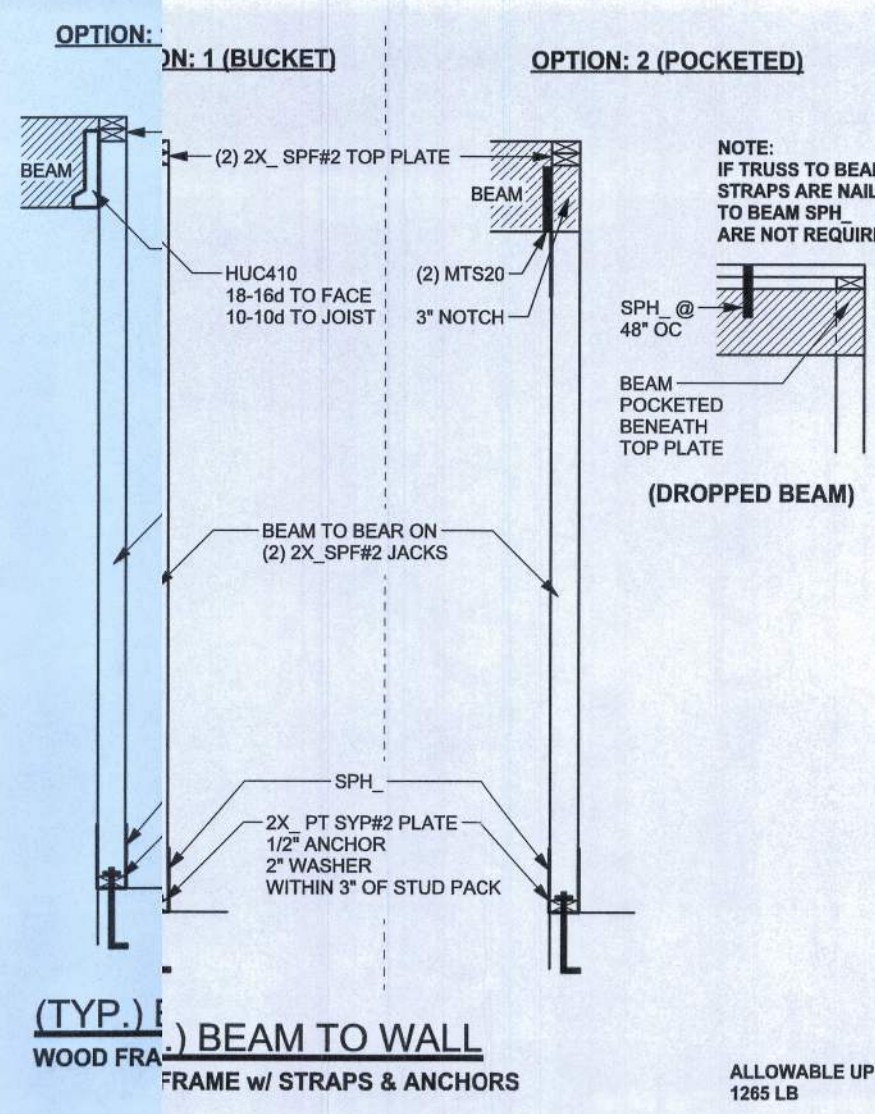


SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.
FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED

(TYP.) GABLE BRACING DETAIL
WOOD FRAME



(TYP.) CORNER FRAMING
WOOD FRAME



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

ALLOWABLE UPLIFT:
1285 LB

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCT 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS RECTIONS ON THE BUILDING STRUCTURE. STRAP 2x6 RAFTERS WITH MIN UPLIFT CONNECTION 419LB EACH END, 2x6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, F_y = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A182, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116, SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT WALL OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI, ALL LAP SPLICES 40" DB (20" FOR #5 BARS); UNO, ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-98, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F_b = 2400, E = 1800ksi, UNO, SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS @ 12" ON PANEL EDGES, 12" ON INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" ON UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT, AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 3" x 3" x 9/64".

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCT 2007 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCT 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCT 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1ASCE 8THMS 802), THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F _m = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 60, F _y = 60 ksi, Lap splices min 48 bar dia, (30" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft ² or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft ² or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 305	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 550	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 890	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LG72	14 -16d	14 -16d	
HEAVY GIRDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MG7		22 -10d	1-58" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-58" THREADED ROD 12" EMBEDMENT
< 10530	< 6035	HGT-3		16 -10d	2-58" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-58" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4 -10d
< 455	< 420	SSP SINGLE BILL PLATE	1-10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8 -10d
< 825	< 600	DSP SINGLE BILL PLATE	2-10d		8 -10d
< 665	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 895	< 760	SP8			6-10d, 1 1/2"
< 1340	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA16	5d-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	16-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*			TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT131	16-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-58" BOLTS		58" AB
< 4175	< 3695	HTT16	18 -16d		58" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 -16d		2-58" AB

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS. MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND >10% SLOPE AND UNOBTSTRUCTED UPLIFT FOR 20d HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = C
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = NA (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)	10	100
1	27.8 -30.5	25.3	-25.3
2	27.8 -35.7	25.3	-30.5
3	27.8 -35.7	25.3	-30.5
4	30.5 -33.0	25.9	-28.5
5	30.5 -40.7	25.9	-31.6
Doors & Windows		30.5	-40.7
Worst Case (Zone 5, 10 ft ²)			
8x7 Garage Door		27.3	-32.0
16x7 Garage Door		25.9	-28.4

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3-12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER:
Mark Disoway, P.E.
P.53915, PCB 688, Lake City, FL 32056,
35-754-5419

(DIMENSIONS:
stated dimensions supercede scaled
dimensions. Refer all questions to
Mark Disoway, P.E. for resolution.
Do not proceed without clarification.

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of Mark Disoway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
provisions of the plan, relating to
wind engineering comply with section
605.2.1, Florida building code
residential 2007,
the best of my knowledge.

NOTATION: This design is valid for one
siding, at specified location.



Edgley Construction

Chad & Katie
Cunningham Residence

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Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
November 09, 2010

DRAWN BY:
David Disoway

STRUCTURAL BY:
David Disoway

FINALS DATE:
3Nov10

JOB NUMBER:
1010038

DRAWING NUMBER

S-1

OF 4 SHEETS

N

ABBREVIATIONS

N - NORTH
S - SOUTH
E - EAST
W - WEST
CONC. - CONCRETE
STY - STORY
I.P. - IRON PIPE
REB. - REBAR
ST. - STREET
AVE. - AVENUE
NO ID. - NO IDENTIFICATION
FD. - FOUND
CM. - CONCRETE MONUMENT
± - MORE OR LESS
ORB - OFFICIAL RECORDS BOOK
PG. - PAGE (S)
(P) - PLAT
(D) - DEED
(C) - CALCULATED
(A) - ACTUAL
(R) - RECORD
D/S - OFFSET
FDOT - FLORIDA DEPARTMENT OF TRANSPORTATION
P.C. - POINT OF CURVATURE
P.T. - POINT OF TANGENCY
P.I. - POINT OF INTERSECTION
P.R.C. - POINT OF REVERSE CURVATURE
P.C.C. - POINT OF COMPOUND CURVATURE
R - RADIUS
RW - RIGHT-OF-WAY
P.C.P. - PERMANENT CONTROL POINT
P.R.M. - PERMANENT REFERENCE MONUMENT
E.P. - EDGE OF PAVE
E.G. - EDGE OF GRADE
C/G - CURB AND GUTTER
ST. MH. - STORM MANHOLE
SS. MH. - SANITARY SEWER MANHOLE
ELEV. - ELEVATION
B.M. - BENCHMARK
C. - CENTERLINE
PB - PLAT BOOK
FM - FIELD MEASURED
UG/E - UNDERGROUND ELECTRIC

PART OF NORTHEAST 1/4 OF SECTION 15, NOT INCLUDED

WEST R/W LINE

(100' RIGHT-OF-WAY) (PAVED)

U.S. HIGHWAY NO. 41

VACANT

PART OF NORTHEAST 1/4 OF SECTION 15, NOT INCLUDED

NORTHWEST 1/4
NORTHEAST 1/4

WEST LINE OF
NORTHEAST 1/4

POINT OF REFERENCE
SW CORNER OF NORTHEAST 1/4

BOUNDARY SURVEY OF

PART OF THE NORTHEAST 1/4 OF SECTION 15, TOWNSHIP 5 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA, BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS: FOR POINT OF REFERENCE COMMENCE AT THE SOUTHWEST CORNER OF SAID NORTHEAST 1/4, THENCE RUN NORTH 0°22'00" EAST, A DISTANCE OF 1449.55 FEET; THENCE RUN NORTH 87°40'25" EAST, A DISTANCE OF 353.08 FEET TO THE POINT OF BEGINNING; THENCE RUN NORTH 4°09'08" WEST, A DISTANCE OF 150.00 FEET; THENCE RUN NORTH 87°40'25" EAST, A DISTANCE OF 340.40 FEET TO THE WEST RIGHT-OF-WAY LINE OF U.S. HIGHWAY NO. 41; THENCE RUN SOUTH 4°08'09" EAST ALONG SAID WEST RIGHT-OF-WAY LINE, A DISTANCE OF 150.00 FEET; THENCE RUN SOUTH 87°40'25" WEST, A DISTANCE OF 340.40 FEET TO THE POINT OF BEGINNING.
CONTAINING: 1.17 ACRES MORE OR LESS.

LEGEND & NOTES

- DENOTES 4" X 4" CONCRETE MONUMENT SET, L.B. # 7170
- DENOTES 4" X 4" CONCRETE MONUMENT FOUND.
- DENOTES 5/8" REBAR W / CAP SET, L.B. # 7170.
- DENOTES 1/2" REBAR FOUND, R.L.S. # 3180
- ⊙ DENOTES POWER POLE
- X-----X DENOTES EXISTING FENCE.
- E-----E DENOTES OVERHEAD ELECTRIC.
- 1) FENCE, ROAD AND OVERHEAD ELECTRIC DIMENSIONS MAY NOT BE TO SCALE.
- 2) NO RESEARCH DONE ON ADJOINING PROPERTY DEEDS TO DETERMINE DEED OVERLAPS OR BOUNDARY LINE DISPUTES. PROPERTY SURVEYED AS PER DESCRIPTION PROVIDED BY CLIENT, NO ABSTRACT PROVIDED.
- 3) NO UNDERGROUND IMPROVEMENTS, IF ANY, LOCATED BY THIS SURVEY.
- 4) FENCE TIES TAKEN ONLY AT LOCATIONS SHOWN AND DEPICTED HEREON.
- 5) COORDINATES BASED ON UNPUBLISHED DATA (ASSUMED).
- 6) BEARINGS BASED ON WEST RIGHT-OF-WAY LINE OF U.S. HIGHWAY NO. 41, S 4°09'08" E, (ASSUMED).



TIMOTHY B. ALCORN
PROFESSIONAL SURVEYOR AND MAPPER
FLORIDA CERTIFICATE NO. 6332
DATE: NOVEMBER 2, 2010

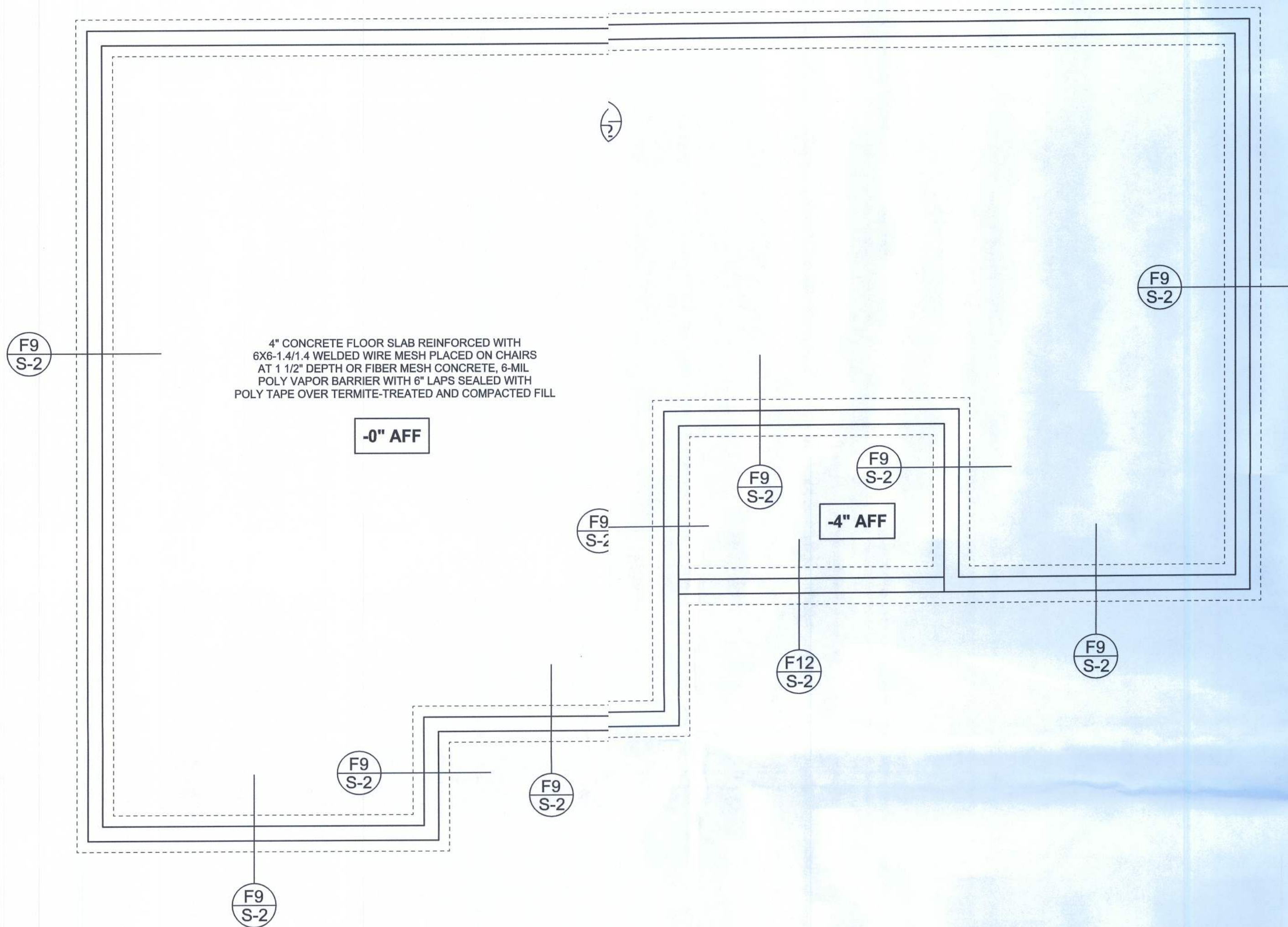
" MAP NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL
RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER "

FOR: CHAD C. & KATHERINE R. CUNNINGHAM

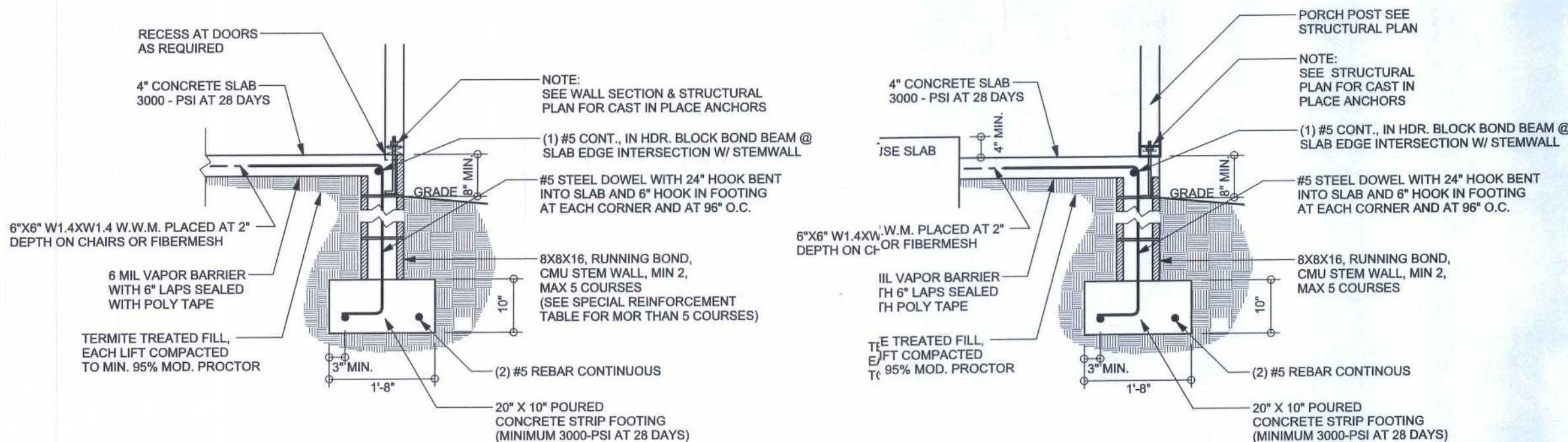
SCALE: 1" = 30'	DATE SURVEYED: 10-26-2010	DATE DRAWN: 11-2-2010
REVISED:	APPROVED BY:	DRAWN BY: KR

J. SHERMAN FRIER & ASSOCIATES, INC.
LAND SURVEYORS CERTIFICATE OF AUTHORIZATION - LB# 7170

130 W. HOWARD ST. / P.O. BOX 580 LIVE OAK, FL 32064 PHONE: 386-362-4629 FAX: 386-362-5270	DRAWING NUMBER: 182-2010
135 NE BLOXHAM STREET / P.O. BOX 226 MAYO, FL 32066 PHONE: 386-294-1223 FAX: 386-294-1363	



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS



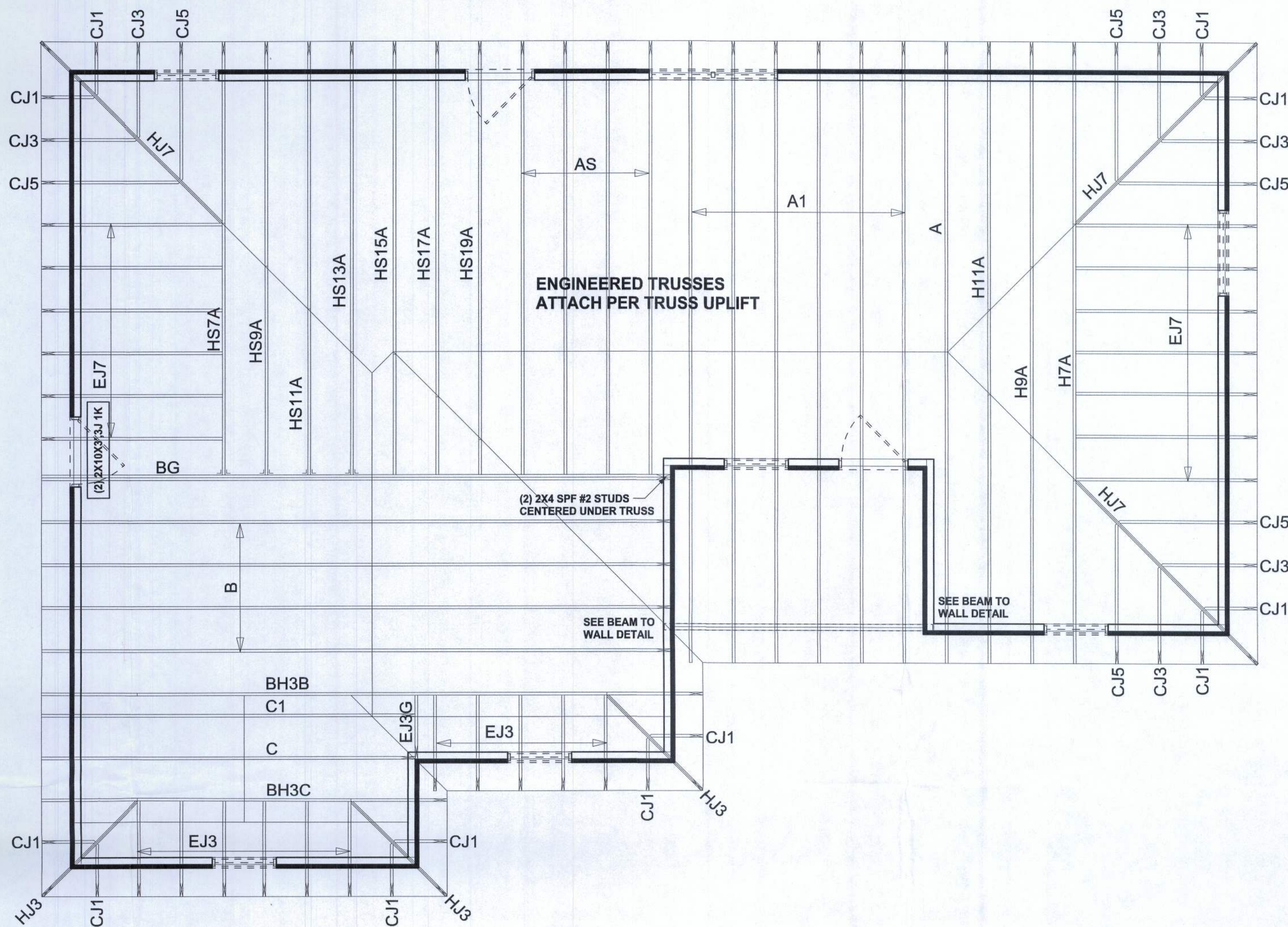
STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in reinforced slab at the top. The vertical steel is to be placed in the footing and bent 24" into the CMU wall (away from the soil pressure, within 2" of the exposed toward the tension side of the wall). If the wall is over 8' high, add Diagonal ladder reinforcement at 16" O.C. exterior side of the wall. If the wall beam with 185 continuous at mid height. For higher parts 16" O.C. vertically or a horizontal bond with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)		VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)	
		#5	#7	#8	#5
3.3	3.0	96	96	96	96
4.0	3.7	96	96	96	96
4.7	4.3	88	96	96	96
5.3	5.0	56	96	96	96
6.0	5.7	40	80	96	80
6.7	6.3	32	56	80	56
7.3	7.0	24	40	56	40
8.0	7.7	16	32	48	32
8.7	8.3	8	24	32	24
9.3	9.0	8	16	24	16



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

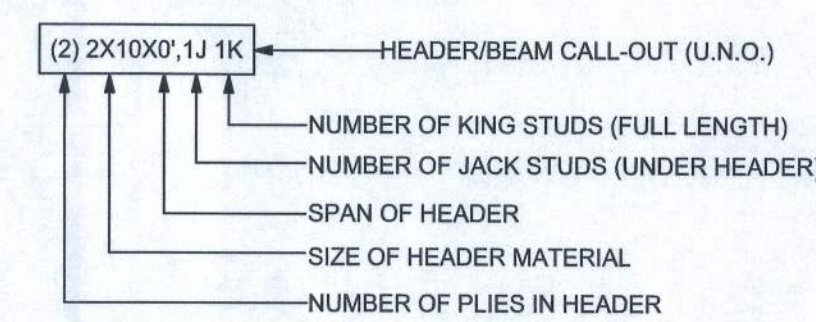
STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS11-03, BCS11-01, BCS11-02, & BCS11-03. BCS11-01, BCS11-02, & BCS11-03 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	38.5'	68.5'
LONGITUDINAL	35.5'	67.3'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS JOB #10-217

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

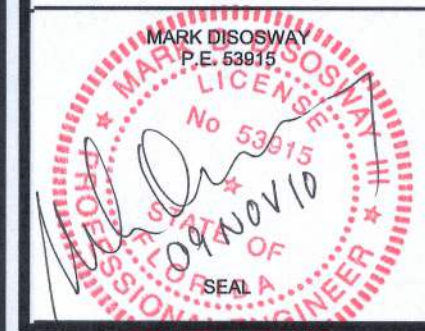
WINDLOAD ENGINEER:
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No. 53915, POB 868, Lake City, FL 32066,
386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section RS01.2.1, Florida building code residential 2007, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Edgley Construction

Chad & Katie
Cunningham Residence

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PRINTED DATE:
November 09, 2010

DRAWN BY: David Disoway
STRUCTURAL BY: David Disoway

FINALS DATE:
9Nov10

JOB NUMBER:
1010038

DRAWING NUMBER
S-2
OF 4 SHEETS