



COS W U.S. HWY LAKE CITY,

BRIAN S. CRAWFORD

ARCHITECTIIRAI DESIGNER: BRIAN CRAWFORD
PHONE: (386) 755-8887

DATE: CHECKED BY:

SHEET NUMBER

**A-2** 

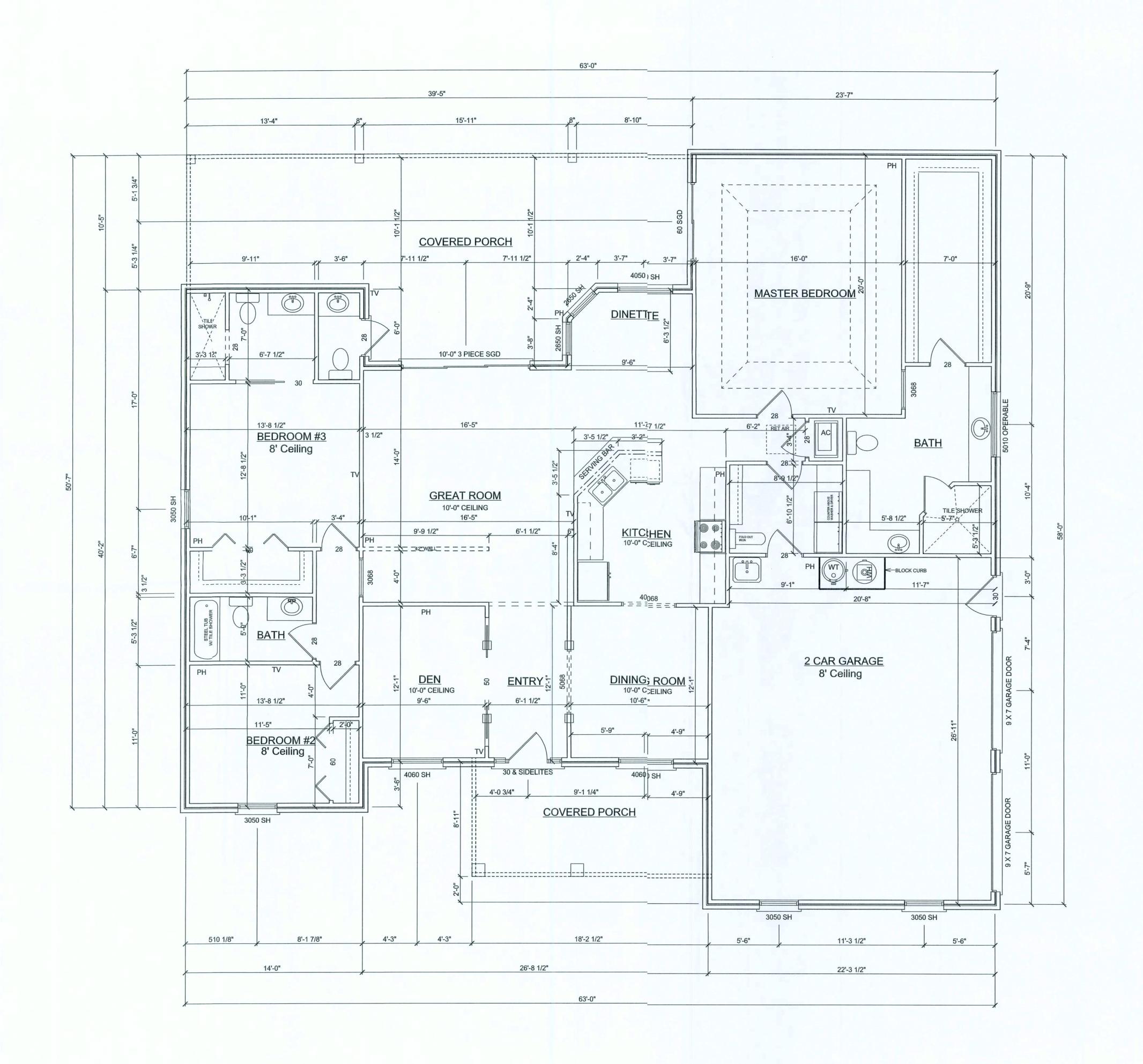
OF 4 SHEETS

AREA SUMMARY

LIVING AREA 2171,4 SF GARAGE 590.1 SF PORCHES 678.6 SF TOTAL 3440.1 SF

MAIN FLOORPLAN

SCALE: 1/4"=1'-0"

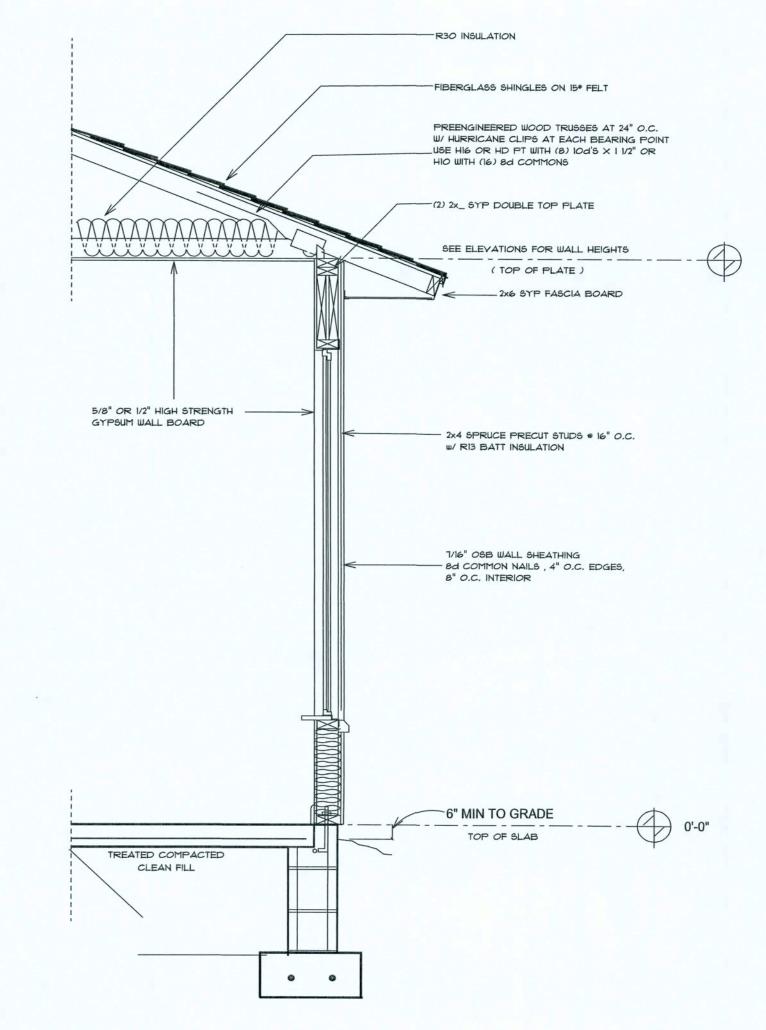


OF 4 SHEETS

AREA SUMMARY

LIVING AREA 2171.4 SF
GARAGE 590.1 SF
PORCHES 678.6 SF
TOTAL 3440.1 SF

ROOF PLAN SCALE: 1/4"=1'-0"



TYPICAL WALL SECTION

 $2 \times 4$  STUD WALL

# **ELECTRICAL PLAN NOTES**

ALL RECEPTICALS IN ALL BEDROOMS SHALL BE AFIC CIRCUITS

WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

INSTALLATION SHALL BE PER NAT'L. ELECTRIC CODE.

ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

OVERCURRENT PROTECTION DEVICE SHALL BE INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECTING MEANS. CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB PANEL SHALL HAVE 4-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR SHALL BE USED AS AN EQUIPMENT GROUND.

TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

ELECTRICAL CONT'R SHALL PREPARE "AS-BUILT" SHOP DWGS INDICATING ALL ELECTRICAL WORK, INCLUDING ANY CHANGES TO THE ELEC. PLAN, ADD'NS TO THE ELEC. PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE W/ ALL CKTS IDENTIFIED W/ CKT Nr., DESCRIPTION & BRKR, SERVICE ENT. & ALL UNDERGROUND WIRE LOCATIONS/ROUTING/DEPTH. RISER DIA. SHALL INCLUDE WIRE SIZES/TYPE & EQUIPMENT TYPE W/ RATINGS & LOADS. CONTRACTOR SHALL PROVIDE 1 COPY OF AS-BUILT DWGS

TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY.

ELECTRICAL	COUNT	SYMBOL
ceiling fan spotlights 2	4	
can light	9	豆
chandelier	2	9 <b>0</b> 9 9 <b>0</b> 9
fluorescent fixture	5	
vanity bar light	2	<u> </u>
wall mount 1	2	Q
electrical panel	1	T1
fan with light	4	<b>(4)</b>
light	10	<b>-</b> \$-
outlet	27	Ф
outlet 220v	2	•
outlet gfi	20	фен
outlet wp gfi	3	∰uip GFI
smoke detector	5	•
switch	32	\$
switch 3 way	11	\$3

# AREA SUMMARY

LIVING AREA	2171,4 SF
GARAGE	590.1 SF
PORCHES	678.6 SF
TOTAL	3440.1 SF

ELECTRICAL PLAN

SCALE: 1/4"=1'-0"

A HOME FOR: LY GRAHAM NEW CUSTOM FRAN AND BILL

CHECKED BY:

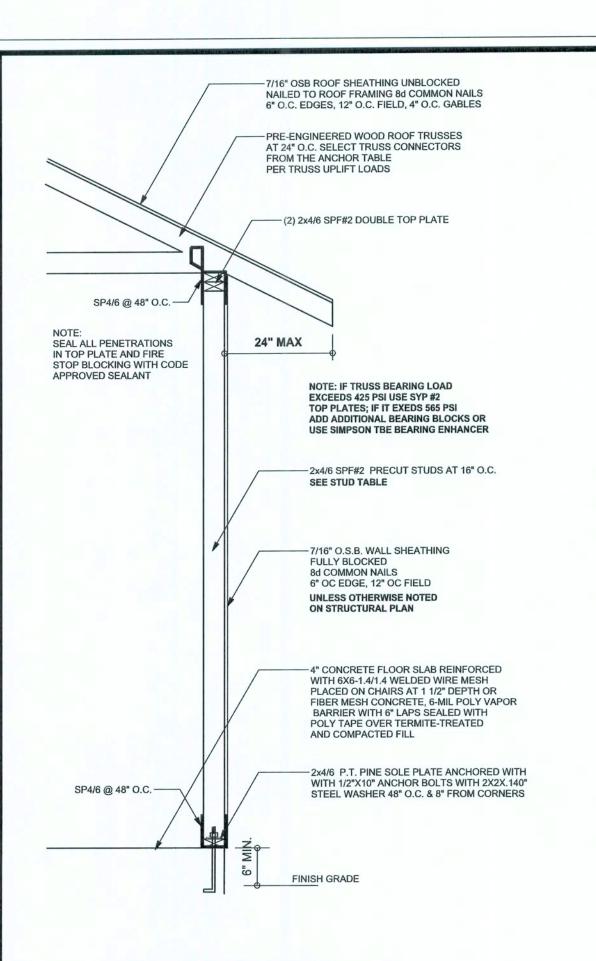
SHEET NUMBER

OF 4 SHEETS

Garage fire separations shall comply with the following: 1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. sleeping purposes shall not be permitted.

2. Ducts in a private garage and ducts penetrating the walls or ceilings separaing the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.

3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.



ONE STORY WALL SECTION

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS** 

(1) 2x4 @ 16" OC TO 11'-9" STUD HEIGHT

(1) 2x6 @ 12" OC | TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B,

EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS

RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B.

LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.

STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

TO 13'-0" STUD HEIGHT

TO 18'-10' STUD HEIGHT

(1) 2x4 @ 12" OC

(1) 2x6 @ 16" OC

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

# SEE STRUCTURAL PLAN STRAP STUDS SP4/6 TOP & OTTOM --2 x 4/6 STUDS AT 16" O.C. OR (2) H2.5A TOP & BOTTO @ 32" O.C. -1/2" X 7" WEDGE ANCHORS AT 48" OC U.N.O. SEE FOUNDATION DETAILS INTERIOR BEARING WALL

TYPICAL GARAGE DOOR HEADER STRAPING DETAIL

(6) .131 x 3 1/4" GUN NAILS -

INTO KING STUD

TOE NAILED THRU HEADER

FOUNDATION SEE

SEE FOOTING DETAILS

-(2) 2X4 SPF #2 TOP PLATE

—(2) 2X12 SYP #2 HEADDER U.N.O

SÉE STRUCTURAL PLLAN

-(2) JACKS STUDS

w/ (2) ROWS 10d @

12" O.C. EACH SIDE

SIMPSCON SP4 @ 48" O.C.

(2) SIMPSON SPH4 w/ (6) - 10d-

(2) SIMPSON LSTA21-

w/ (8) -16d TO HEADER

AND (8) -16d TO STUD PACK

(2) KINGS STUDS -

w/ (2) ROWS 10d @

12" O.C. EACH SIDE

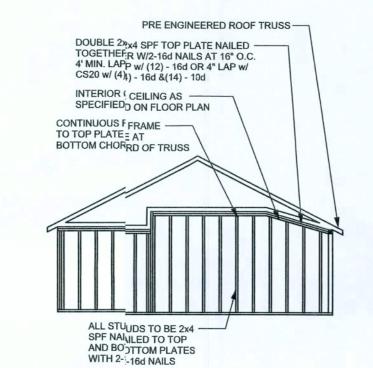
SIMPSON LTTI31-

5/8" x 10" ANCHOR BOLT

w/ (18) - 10d &

### GRADE & SPECIES TABLE

		Fb (psi)	E (10 <sup>6</sup> psi)
2xx8	SYP #2	1200	1.6
2x110	SYP #2	1050	1.6
2x112	SYP #2	975	1.6
GL <sub>LB</sub>	24F-V3 SP	2400	1.8
LS <sub>3L</sub>	TIMBERSTRAND	1700	1.7
LVVL	MICROLAM	1600	1.9
PSSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO **CEILI'NG DIAPHRAGM DETAIL** SCALE: N.TT.S.

> -NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 4" O.C. FOR UPLIFT

# **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'

FIBER CONCRETE SLABS: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**WASHERS:** WASHERS USED WITH 1/2" BOLTS TO BE 2"  $\times$  2"  $\times$  9/64"; WITH 5/8" BOLTS TO BE 3"  $\times$  3"  $\times$  9/64"; WITH 3/4" BOLTS TO BE 3"  $\times$  3"  $\times$  9/64"; WITH 7/8" BOLTS TO BE 3"  $\times$  3"  $\times$  5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

### **BUILDER'S RESPONSIBILITY**

	OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	ONS, FOUNDATION BEARING CAPACITY, GRADE AND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	ID CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 IE STATED WIND VELOCITY AND DESIGN PRESSURES.
	S LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU 'S A CONTINUOUS LOAD PATH CONNECTION, CALL 'ER IMMEDIATELY.
DESIGN, PLACEMENT	UFACTURER'S SEALED ENGINEERING INCLUDES TRUSS ANS, TEMPORARY AND PERMANENT BRACING DETAILS, IECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

# ROOF SYSTEM DESIGN

BEARING LOCATIONS.

(6) .131 x 3 1/4" GUN NAILS

INTO KING STUD

TOE NAILED THRU HEADER

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

**MASONRY NOTES:** 

Grout

CMU standard

Clay brick standard

Movement joints

Reinforcing bars, #3 - #11

Coating for corrosion protection

Coating for corrosion protection

ACI530.1-02 Section

IN WRITING.

1.4A

3.3.E.2

3.3.E.7

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF

ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON

or 304SS

Pipes, conduits, and accessories | Any not shown on the project drawings

splices min 48 bar dia. (30" for #5)

Anchors, sheet metal ties completely

embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS

Joint reinforcement in walls exposed to

Contractor assumes responsibility for type

and location of movement joints if not

require engineering approval.

detailed on project drawings.

# **DESIGN DATA**

**ANCHOR TABLE** 

< 420

< 455

< 360

< 455

< 415

< 600

< 950

< 745

< 1465

< 1465

< 990

< 760

< 1470

< 1470

< 1000

< 1450

< 2900

< 2050

< 3965

< 10980

< 10530

< 9250

< 435

< 455

< 825

< 825

< 885

< 1240

< 885

< 1240

< 1235

< 1235

< 1030

< 1705

< 1350

< 2310

< 2775

< 4175

< 1400

< 3335

< 2200

< 2300

< 2320

MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP | UPLIFT LBS. SPF |

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

< 245

< 265

< 235

< 320

< 365

< 535

< 820

< 565

< 1050

< 1050

< 850

< 655

< 1265

< 1265

< 860

< 1245

< 2490

< 1785

< 3330

< 9035

< 9250

< 435

< 420

< 825

< 600

< 760

< 1065

< 760

< 1065

< 1165

< 1235

< 1030

< 1705

< 1305

< 2310

< 2570

< 3695

< 1400

< 3335

< 2200

< 2300

< 2320

TRUSS CONNECTOR\*

H5A

H2.5

H2.5A

H14-1

H14-2

H10-1

H10-2

MTS24C

2 - HTS24

LGT2

*IEAVY GIRDER TIEDOWNS* 

HGT-2

HGT-3

HGT-4

STUD STRAP CONNECTOR

SSP DOUBLE TOP PLATE

SSP SINGLE SILL PLATE

DSP DOUBLE TOP PLATE

DSP SINGLE SILL PLATE

SPH4

LSTA18

LSTA21

CS16

STUD ANCHORS\*

LTT19

LTTI31

HD2A

HTT16

PAHD42

HPAHD22

ABU44

ABU66

ABU88

TO PLATES TO RAFTER/TRUSS

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

12-8d, 1 1/2"

12-8d, 1 1/2"

8-8d, 1 1/2"

6-10d

2-10d, 1 1/2"

2-10d, 1 1/2"

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

22 -10d

16 -10d

16 -10d

16 -10d

3-8d

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

13-8d

15-8d

6-10d

10-10d, 1 1/2"

10-10d, 1 1/2"

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

1-10d

6-10d

2 -10d

14-10d

16-10d

18-8d

28-8d

TO STUDS

8-16d

18-10d, 1 1/2'

2-5/8" BOLTS

18 - 16d

16-16d

16-16d

12-16d

12-16d

18 - 16d

8-8d, 1 1/2"

TO STUDS

TO FOUNDATION

12" EMBEDMENT

5/8" THREADED ROD

12" EMBEDMENT

-5/8" THREADED ROD

12" EMBEDMENT

5/8" THREADED ROD

12" EMBEDMENT

TO STUDS

4 -10d

4 -10d

8 -10d

8 -10d

6-10d, 1 1/2"

10-10d, 1 1/2"

10-10d, 1 1/2"

TO FOUNDATION

1/2" AB

1/2" AB

5/8" AB

5/8" AB

1/2" AB

1/2" AB

2-5/8" AB

6-10d, 1 1/2"

WIND LOADS	ER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
MEAN ROOF ON UPPER H	MPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; EIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT LF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% NOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LES:
BUILDING IS	OT IN THE HIGH VELOCITY HURRICANE ZONE

Zone Effective Wind Area (ft2				ea (ft2)
	10		100	
1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 O'hg		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 O'hg		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
	& Wind st Cas 5, 10	е	21.8	-29.1
8x7 Gar	age D	oor	19.5	-22.9
16x7 Ga	rage [	Door	18.5	-21.0
				1

DESIGN	LUADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)	
	30 PSF (SLEEPING ROOMS)	
	30 PSF (ATTICS WITH STORAGE)	
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)	
ROOF	20 PSF (FLAT OR <4:12)	
	16 PSF (4:12 TO <12:12)	
	12 PSE (12:12 AND GREATER)	

REVISIONS

SOFTPLAN

Stated dimensions sujercede scaled dimensions. Refer all juestions to Mark Disosway, P.E. br resolution. Do not proceed without clarification.

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INDLOAD ENGINEIR: Mark Disosway, PE No.53915, POB 868, Lake City, FL

32056, 386-754-5419

DIMENSIONS

CERTIFICATION: I heeby certify that I have examined this plan, and that the applicable portions of the plan, reating to wind engineerin comply with section Ri01.2.1, florida building code residential 2004,to the best of my

LIMITATION: This desgn is valid for one ouilding, at specified lication.

MARK JISOSWAY

Concept Construction Of North Florida, Inc

Fran & Billy Graham Residence

ADDRESS: Lot 9 Civey Court Columbia County, Florida

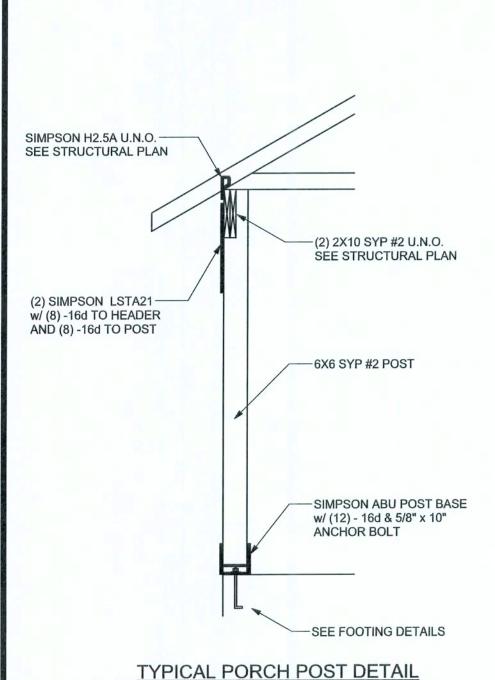
Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (383) 754 - 5419 Fax: (386 269 - 4871

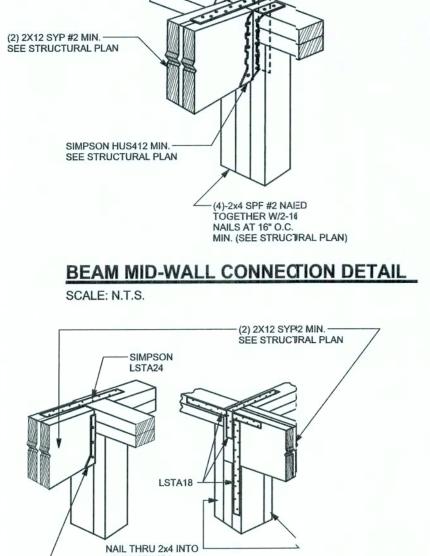
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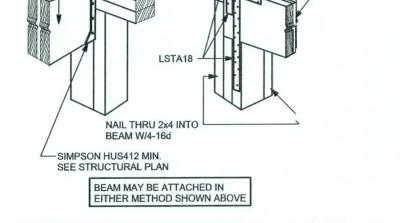
FINALS DATE: 15 / Jun / 06

> JOB NUMBER: 606122 DRAWING NUMBER

		MEAN ON U	n Roc Jpper	OF HEI	GHT N OF HI	OT EXC	CEEDING ESCARP	G LEAS	T HORIZO 60FT IN E 50x HEIGI	NTAL D	IMEN OFT II	SION N EXP	OR 60	FT; NO	oʻ %
	E	BUILD	DING	IS NOT	T IN TH	IE HIGH	VELOC	ITY HU	RRICANE	ZONE					_
	E	BUILD	DING	IS NOT	T IN TH	E WIND	D-BORN	E DEBF	RIS REGIO	ON					_
		1.) E	BASIC	WIND	SPEE	D = 11	0 MPH								_
		2.) \	WIND	EXPO	SURE	= B		. 112.							_
		3.) \	WIND	IMPOR	RTANC	E FAC	TOR = 1	1.0							_
		4.) E	BUILD	ING C	ATEGO	DRY = II	1.								_
	5	5.) F	ROOF	ANGL	E = 10	-45 DE	GREES								_
	1	6.)	MEAN	ROOF	FHEIG	HT = <3	30 FT								_
	17	7.) 1	INTER	RNAL P	PRESS	URE CO	DEFFICIE	ENT = N	N/A (ENCL	OSED B	UILD	NG)			-
	8								WIND PR				R301.	2(2))	
										-	F#	41 1AC		(5(0)	
			X	\ A	_					Zone	_	10		ea (ft2)	
LO FOR THIS PROJECT CLASS				$\bigotimes$						1	_	_	18.1	-18.1	
LS FOR THIS PROJECT SHALL PECIFICATION FOR MASONRY		_	4	2	2					2	_	-	18.1	-21.8	
). THE CONTRACTOR AND MASON			1	1		7	7			2 O'hg		-40.6		-40.6	
S, NOTIFY THE ENGINEER OF		1	1)	2	2 2	2 1	7			3	19.9	-25.5	18.1	-21.8	
ND THESE DESIGN DRAWINGS.		9	/	4	7	13	]			3 O'hg		-68.3		-42.4	
BE APPROVED BY THE ENGINEER					Y	1 4				4		-23.6		-20.4	
			×		55					5	21.8	-29.1	18.5	-22.6	
Specific Requirements			73	H	11	4				Doors	& Win	dows	21.8	-29.1	
8" block bearing walls F'm = 1500	psi		/2/	//						100000000000000000000000000000000000000	st Cas	70.70	2.1.0	20.1	
ASTM C 270, Type N, UNO		6	<b>X</b>		1,11					(Zone	5, 10	ft2)			
ASTM C 476, admixtures require a		5	5	1		V32-				8x7 Gar	age D	oor	19.5	-22.9	
ASTM C 90-02, Normal weight, Ho medium surface finish, 8"x8"x16" r		2	X	1		/2/	5			16x7 Ga			18.5	-21.0	
bond and 12"x12" or 16"x16" colu				\	$\sqrt{3}$	4	Z								
block						5	2								
ASTM C 216-02, Grade SW, Type 5.5"x2.75"x11.5"	FBS,				A)	* _									
ASTM 615, Grade 60, Fy = 60 ksi, splices min 48 bar dia. (30" for #5)		DESIG	GN LC	DADS									-		

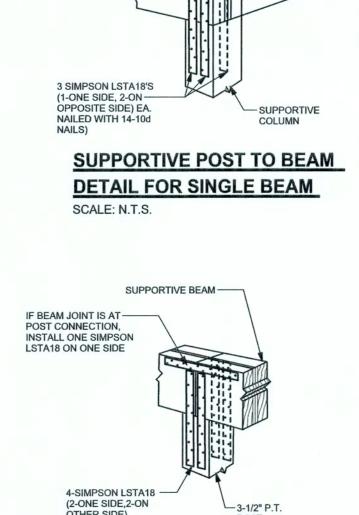






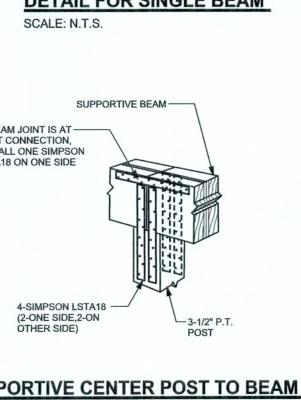
BEAM CORNER CONNECTION. JETAIL

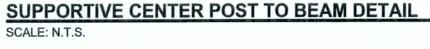
SCALE: N.T.S.

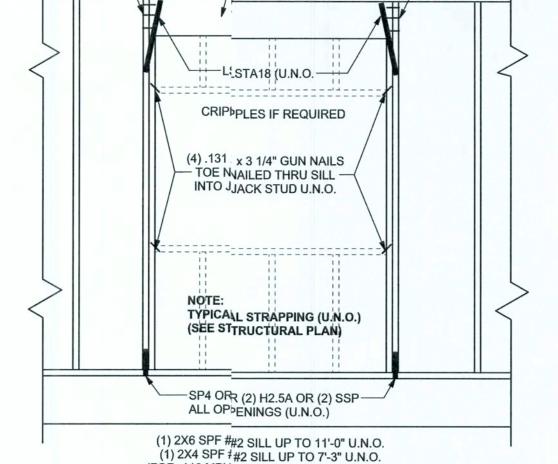


-NON-SUPPORTIVE

2X4 LADDER BEAM







TYPICAL HEADER STRAPING DETAIL

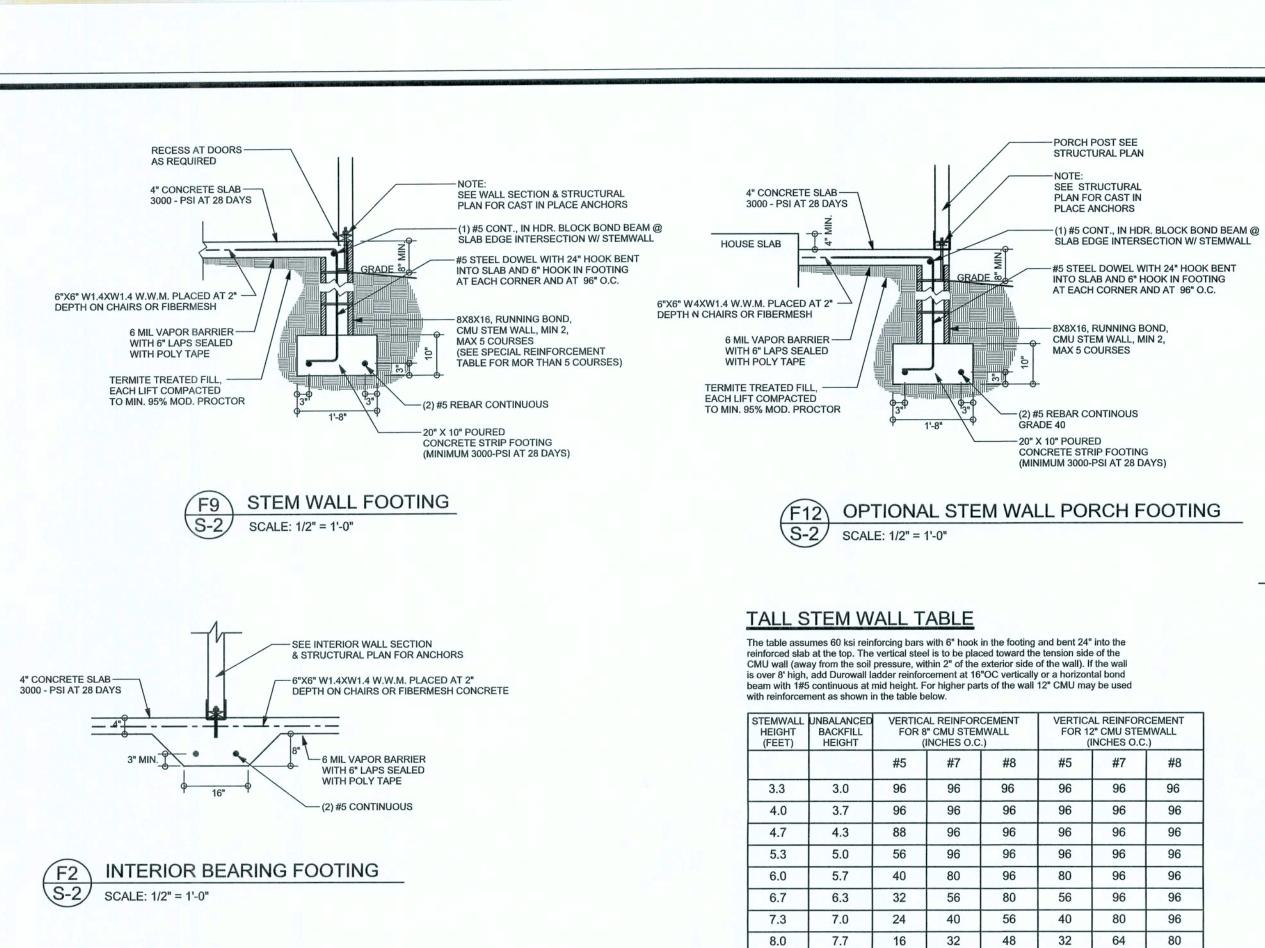
(FOR: 110 MPH-, 10'-0" WALL HIGHT U.N.O.)

moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) SOIL BEARING CAPACITY 1000PSF NOT IN FLOOD ZONE (BUILDER TO VERIFY)

OF 3 SHEETS



8.3

9.0

4" CONCRETE SLAB ---

3000 - PSI AT 28 DAYS

(1) #5 CONTINUOUS -

SCALE: 1/2" = 1'-0"

24 | 32 | 24

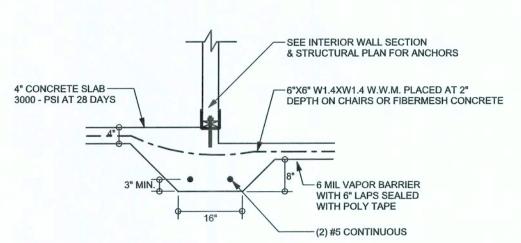
DEPTH ON CHAIRS OR FIBERMESH CONCRETE

TYPICAL NON - BEARING STEP FOOTING

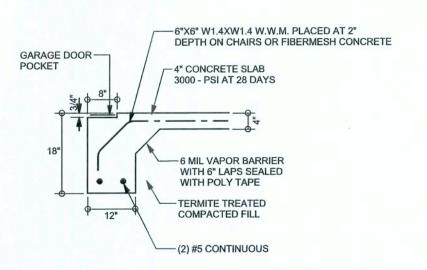
- 6 MIL VAPOR BARRIER

WITH 6" LAPS SEALED WITH POLY TAPE

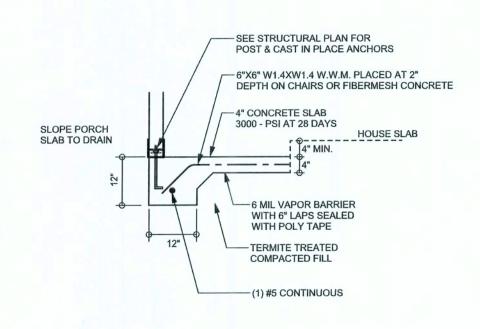
24 16



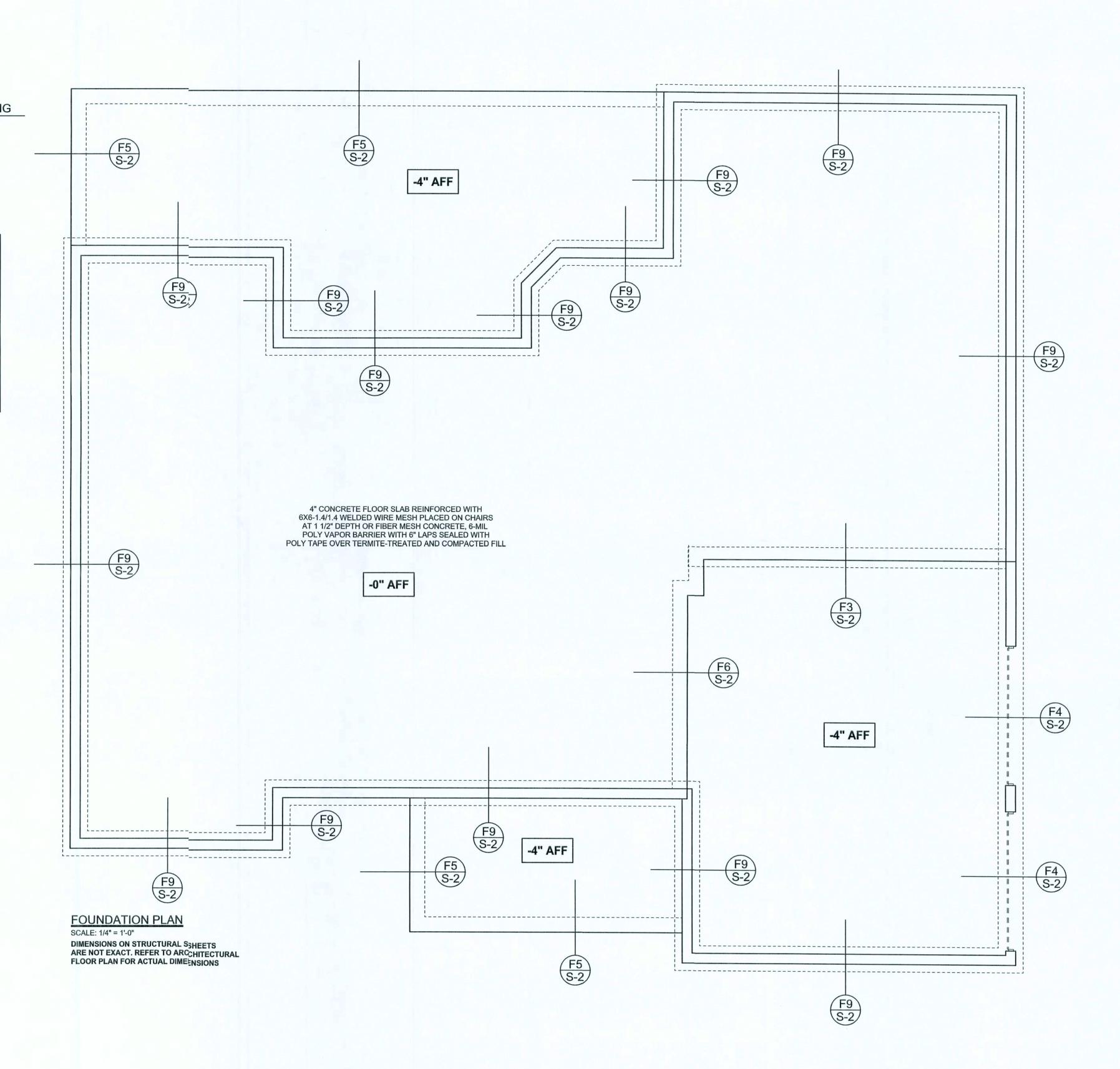
### INTERIOR BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



# F4 GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"



PORCH FOOTING SCALE: 1/2" = 1'-0"



REVISIONS

WINDLOAD ENGINER: Mark Disosway,

PE No.53915, POB 8i8, Lake City, FL 32056, 386-754-5419

Stated dimensions swercede scaled dimensions. Refer all questions to Mark Disosway, P.E. or resolution.

Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable

portions of the plan, rilating to wind engineer comply with section F301.2.1, florida building code residential 2004 to the best of my

LIMITATION: This deign is valid for one building, at specified bcation.

MARK DISOSWAY P.E 53915

:EAL

Concept Construction Of North Florida, Inc.

Fran & Billy **Graham Residence** 

ADDRESS:

Lot 9 Covey Court Columbia County, Florida

Mark Disosway P.E.

P.O. 3ox 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

> PRINTED DATE: June 15 2006

JOB NUMBER:

OF 3 SHEETS

603122 DRAWING NUMBER

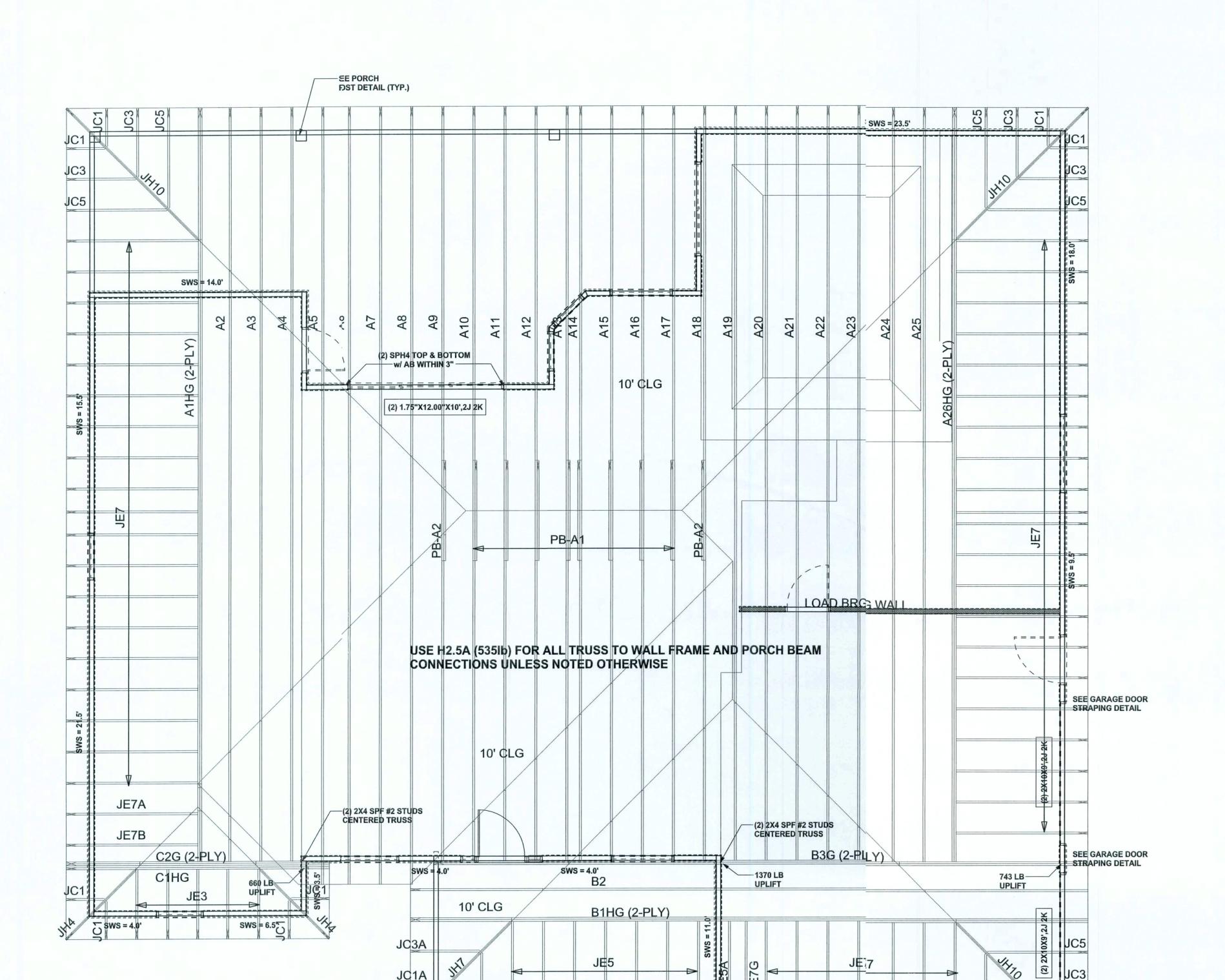
CHECKED BY:

DRAWN BY:

**David Disosway** 

FINALS DATE: 15 / Jun / 06

5UW 06



-SEE PORCH

POST DETAIL (TYP.)

STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

REVISIONS

SOFTPIAN

### STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

DIMENSIONS ON STRUCTURAL SHEETS SN-3 ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

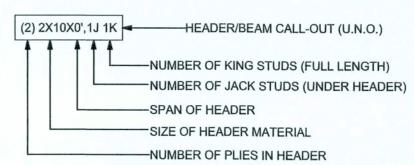
PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED

### WALL LEGEND

TRUSS PACKAGE

sws = 0.0'	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
SWS = 0.0'	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

### **HEADER LEGEND**



### TOTAL SHEAR WALL SEGMENTS

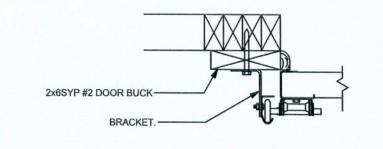
WS	= 0.0' INDICATES	S SHEAR WA	LL SEGMEN
		REQUIRED	ACTUAL
	TRANSVERSE	34.5'	79.0'
	LONGITUDINAL	31.5'	72.0'

SEE GARAGE DOOR

STRAPING DETAIL

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK, HORIZONTAL JAMBS DO NOT TRANSFER LOAD, CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4" GN PER TABLE BELOW:

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" G		
8' - 10'	24" O.C.	5" O.C.	5" O.C.		
11' - 15'	18" O.C.	4" O.C.	4" O.C.		
16' - 18'	16" O.C.	3" O.C.	3" O.C.		



### **GARAGE DOOR BUCK INSTALLATION DETAIL**

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. W.B. HOWLAND JOB #3536

WINDLOAD ENGILEER: Mark Disosway, PE No.53915, POI 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS:

dimensions. Referall questions to Mark Disosway, PE. for resolution. Do not proceed winout clarification. COPYRIGHTS AND PROPERTY RIGHTS:
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permission and coisent of Mark Disosway. CERTIFICATION: hereby certify that I have examined this plan and that the applicable portions of the plar, relating to wind engineerin comply with section R301.2.1, florida building code residential 204, to the best of my

LIMITATION: This lesign is valid for one building, at specified location. MAFK DISOSWAY .E. 53915

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Mark Cisosway P.E. P.C. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRNTED DATE: June15, 2006 DRAWN BY: CHECKED BY: David Disosway

FINALS DATE 15 / Jun / 06

JOB NUMBER: 606122 DRAWNG NUMBER

**S-3** OF3 SHEETS