DATE 06/28	3/2006			ty Buildir]	PERMIT
	C.1.2.C.C.T.T		nit Expires One	Year From the	e Date of PHONE	Issue 386.497.22		000024686
APPLICANT ADDRESS	5AMMY I 764	SW RIVERSIDE A	VENITIE	FT. WHITE	FHONE	380.497.22	FL	32038
OWNER	MIKE HU		VENOE		PHONE	386.288.36		
ADDRESS	724	SW WESTER DRIV	Æ	LAKE CITY			FL	32024
CONTRACTO		WILLIAMS			PHONE	386.365.22	289	
LOCATION O			VESTER ROAD,TI	 L TO LAST M/H ON	N RIGHT I	BEFORE CU	— RVING	
		TO THE I						
TYPE DEVEL	OPMENT	SFD/UTILITY		ESTIMATED COS	ST OF CO	NSTRUCTIO	ON <u>11</u>	2000.00
HEATED FLO	OR AREA	2240.00	TOTAL	AREA 4786.00		HEIGHT	22.60	STORIES 1
FOUNDATIO	N CONC	WAL	LS FRAMED	ROOF PITCH	7'12		FLOOR	CONC
LAND USE &	ZONING	A-3			MAX	HEIGHT	35	
Minimum Set	Back Requir	ments: STREET-	-FRONT 3	0.00	REAR	25.00	SIDE	25.00
NO. EX.D.U.	1	FLOOD ZONE	<u>X</u>	DEVELOPMI	ENT PERM	AIT NO.	****	AN
PARCEL ID	31-4S-17-0	08920-000	SUBDIV	ISION	90.			
LOT	BLOCK	PHASE	UNII		TOTA	L ACRES	8.50	\overline{A}
			CBC050690		XII	In L	5/	- 2
Culvert Permit	No.	Culvert Waiver (Contractor's License	Number	N. M.	Applicant/Ow	ner/Contra	ctor
EXISTING		06-0546-N	BLK			ГН		N
Driveway Conn	ection	Septic Tank Number	LU & 2	Zoning checked by	Ann	roved for Issu	iance	New Resident
		Soptio Tunk I tunioui		Loning checked by	App	10100 101 1550	anico	
COMMENTS:		BOVE ROAD. 30 DA		·	• •			
-		-		·	SUANCE			8788
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COMMENTS:	1 FOOT A	BOVE ROAD. 30 DA	AYS TO REMOVE	·	SSUANCE	Check # or	r Cash	
-	1 FOOT A	BOVE ROAD. 30 DA	AYS TO REMOVE	M/H AFTER CO IS	SSUANCE	Check # or	r Cash	8788 (footer/Slab)
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NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

THIS INSTRUMENT WAS PREPARED BY:
FIRST FEDERAL SAVINGS BANK OF FLORIDA
4705 WEST U.S. HIGHWAY 90
P.O. BOX 2029
LAKE CITY, FLORIDA 32056

PERMIT NO TAX FOLIO NO
NOTICE OF COMMENCEMENT
STATE OF FLORIDA COUNTY OFColumbia
The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.
1. Description of property: SEE EXHIBIT "A" ATTACHED HERETO AND MADE A PART HEREOF.
2. General description of improvement: Construction of Dwelling
3. Owner information: a. Name and address: MICHAEL S. HUNTER and BRENDA L. HUNTER 724 SW Wester Drive, Lake City, FL 32024
 b. Interest in property: <u>Fee Simple</u> c. Name and address of fee simple title holder (if other than Owner): <u>NONE</u>
4. Contractor (name and address): S L K CONSTRUCTION, INC. 1999 S Marion Ave., Lake City, FL 32025
5. Surety: a. Name and address: Inst:2006014427 Date:06/14/2006 Time:16:35 DC,P.DeWitt Cason,Columbia County B:1086 P:2266
 b. Amount of bond:
8. In addition to himself, Owner designates PAULA HACKER of FIRST FEDERAL SAVINGS BANK OF FLORIDA, 4705 West U.S. Highway 90 / P. O. Box 2029, Lake City, Florida 32056 to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) (b), Florida Statutes.
9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified). Borrower Name
Co-Borrower Name
The foregoing instrument was acknowledged before me this 8th day of June 20_06, by MICHAEL S. HUNTER & BRENDA L. HUNTER , who is personally known to me or who has produced driver's license for identification. STATE OF FLOR!DA COUNTY OF GOLUMBIA I HERBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office. P. DeWITT CASON, CLERK OF COURTS By Arabra Septing Clerk Date 06-14-06 Deputy Clerk Date 06-14-06

EXHIBIT "A"

PART OF THE NORTHEAST 1/4 OF THE NORTHWEST 1/4 OF SECTION 31, TOWNSHIP 4 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA; BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS: FOR POINT OF REFERENCE COMMENCE AT THE NORTHWEST CORNER OF SAID NORTHEAST 1/4 OF THE NORTHWEST 1/4; THENCE RUN SOUTH 01°49'19" WEST ALONG THE WEST LINE OF SAID NORTHEAST 1/4 OF THE NORTHWEST 1/4, A DISTANCE OF 131.28 FEET TO A POINT LYING ON THE SOUTH RIGHT-OF-WAY LINE OF WESTER ROAD; THENCE RUN SOUTH 88°56'11" EAST ALONG SAID SOUTH RIGHT-OF-WAY LINE, A DISTANCE OF 717.46 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE SOUTH 88°56'11" EAST ALONG SAID SOUTH RIGHT-OF-WAY LINE, A DISTANCE OF 422.62 FEET; THENCE RUN SOUTH 02°55'26" WEST, A DISTANCE OF 515.63 FEET; THENCE RUN NORTH 02°55'26" WEST, A DISTANCE RUN NORTH 02°55'26" WEST, A DISTANCE OF 515.63 FEET TO THE POINT OF BEGINNING.

Inst:2006014427 Date:06/14/2006 Time:16:35
_____DC,P.DeWitt Cason,Columbia County B:1086 P:2267

For Office Use Only Application # 0606 - 71 Date Received 6/20 By Permit # 24686
Application Approved by - Zoning Official 134 Date 27.06.06 Plans Examiner 0K 71H Date 6-24.06
Flood Zone Development Permit Zoning Land Use Plan Map Category
Comments - MA will be confecto with New UZO. 30 day to remove mit after co
- NOCUSING Eisting WE/1 - 1550-51
Applicants Name 5LK Construction Inc. Fax: 386 497 2289 Applicants Name 5LK Construction Inc. Phone 386 365 3646
Address 764 5W Riverside Av. Ft White FL, 32038
Owners Name <u>Mike Hunter</u> Phone <u>386 288-36/3</u> 911 Address <u>724 SW Wester Dr. Lake City Fl. 32024</u>
1 1/1/1 4/0 2
Address 764 SW Riverside Av. Ft White FL 32038
Fee Simple Owner Name & Address Mike Huster 724 SW Wester Dr. Lake City FL. 32024
Architect/Engineer Name & Address OC+6 Gary Gill PE POBOX 187 Live OAK FL 32064
Mortgage Lenders Name & Address First Federal Lake City FL,
Circle the correct power company - FL Power & Light - Clay Elec Suwannee Valley Elec Progressive Energy
Property ID Number 314517-08920-000 HX Estimated Cost of Construction 270,000,
Subdivision Name Lot Block Unit Phase
Driving Directions 47 South to Wester Dd The Last Mobile Home on
Kight Before Curre to Right.
No H. (E 1)
Type of Construction New Home (Framed) Number of Existing Dwellings on Property / MH
Total Acreage 8.5 Lot Size Do you need a - <u>Culvert Permit</u> or <u>Culvert Waiver</u> or <u>Have an Existing Drive</u>
Actual Distance of Structure from Property Lines - Front 95 Side 40 Rear 61
Total Building Height 22.6" Number of Stories / Heated Floor Area 2240 Roof Pitch 7/12
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or
installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.
OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING
TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.
Letterstanten Fre Son 1900 Sen I Do Vallania
The thing the term of the terms
Susan Nettles Villeges Contractors License Number CRC 050/40
STATE OF FLORIDA COUNTY OF COLUMBIA My Commission DD287694 Competency Card Number /02 0000 574 Expires December 15, 2007 NOTARY STAMP/SEAL
Sworn to (or affirmed) and subscribed before me
this 20 day of June 2006. Swany Wegas
Personally known or Produced Identification Notary Signature

M. 8788_

Page 1 of 2 D_SearchResults

Columbia County Property

Appraiser
DB Last Updated: 5/5/2006

Tax Record **Property Card** Interactive GIS Map Parcel: 31-4S-17-08920-000 HX

Owner & Property Info

Owner's Name	HUNTER MICHAEL STEPHEN
Site Address	WESTER
Mailing Address	724 SW WESTER DRIVE LAKE CITY, FL 32024
Description	NE1/4 OF NW1/4 AS LIES S OF WESTER RD & A 2.30 AC PARCEL BEING APPROX THE N 70.65 FT OF SE1/4 OF NW1/4 AS DESC IN ORB 855-2445. ORB 339-559, 755-916, 796-478, 855-2440, PROBATE 1075-511 THRU 514. EX 28 AC DESC ORB 1077-925

<< Prev Sea	rch Result: 18 of 24 Next >>
Use Desc. (code)	IMPROVED A (005000)
Neighborhood	31417.00
Tax District	3
UD Codes	MKTA01
Market Area	01
Total Land Area	8.500 ACRES

2006 Proposed Values

Print

Property & Assessment Values

Mkt Land Value	cnt: (2)	\$14,480.00
Ag Land Value	cnt: (1)	\$2,392.00
Building Value	cnt: (1)	\$22,709.00
XFOB Value	cnt: (7)	\$4,260.00
Total Appraised Value		\$43,841.00

Just Value		\$101,449.00
Class Value		\$43,841.00
Assessed Value		\$21,823.00
Exempt Value	(code: HX)	\$21,823.00
Total Taxable Value		\$0.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vimp	Sale Qual	Sale RCode	Sale Price		
NONE								

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value	
1	MOBILE HME (000800)	1986	WD or PLY (08)	1664	2024	\$22,709.00	
Note: All S.F. calculations are based on exterior building dimensions.							

Extra Features & Out Buildings

Code	Desc	Year Bit	Value	Units	Dims	Condition (% Good)
0040	BARN,POLE	0	\$300.00	1.000	0 x 0 x 0	(.00)
0296	SHED METAL	1994	\$360.00	120.000	10 x 12 x 0	AP (40.00)
0297	SHED CONCR	1994	\$540.00	120.000	10 x 12 x 0	AP (40.00)
0070	CARPORT UF	1993	\$1,080.00	360.000	18 x 20 x 0	(.00)
0296	SHED METAL	1993	\$360.00	120.000	10 x 12 x 0	AP (40.00)

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000102	SFR/MH (MKT)	1.000 AC	1.00/1.00/1.00/1.00	\$12,480.00	\$12,480.00
005400	TIMBER 1 (AG)	7.500 AC	1.00/1.00/1.00/1.00	\$319.00	\$2,392.00
009910	MKT.VAL.AG (MKT)	7.500 AC	1.00/1.00/1.00/1.00	\$0.00	\$60,000.00
009945	WELL/SEPT (MKT)	1.000 UT - (.000AC)	1.00/1.00/1.00/1.00	\$2,000.00	\$2,000.00

Columbia County Property Appraiser

DB Last Updated: 5/5/2006

Mike +

RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA, BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS. FOR POINT OF REFERENCE COMMENCE AT THE NORTHWEST CORNER OF SAID NORTHEAST 1/4 OF THE NORTHWEST 1/4 OF THE NORTHWEST 1/4 OF THE NORTHWEST 1/4, THENCE RUN SOUTH 01°49'19" WEST ALONG THE WEST LINE OF SAID NORTHEAST 1/4 OF THE NORTHWEST 1/4, A DISTANCE OF 131.28 FEET TO POINT OF BEGINNING: THENCE CONTINUE SOUTH 88°56'11" EAST ALONG SAID SOUTH RIGHT-OF-WAY LINE, A DISTANCE OF 422.62 FEET; THENCE RUN SOUTH 02°55'26" WEST, A DISTANCE 88°56'11" EAST ALONG SAID SOUTH RIGHT-OF-WAY LINE. A DISTANCE OF 717.46 FEET TO THE PART OF THE NORTHEAST 1/4 OF THE NORTHWEST 1/4 OF SECTION 31, TOWNSHIP 4 SOUTH, OF 515.63 FEET; THENCE RUN NORTH 88°56'11" WEST, A DISTANCE OF 422.62 FEET; THENCE A POINT LYING ON THE SOUTH RIGHT-OF-WAY LINE OF WESTER ROAD; THENCE RUN SOUTH RUN NORTH 02"55'26" WEST, A DISTANCE OF 515.63 FEET TO THE POINT OF BEGINNING. CONTAINING: 5.00 ACRES MORE OR LESS. **BOUNDARY SURVEY**

W. Wester Dr.

DENOTES 4" X 4" CONCRETE MONUMENT FOUND, L.B. # 6685. DENOTES 4" X 4" CONCRETE MONUMENT SET, L.8. # 7170 1) FENCE, ROAD AND OVERHEAD ELECTRIC DIMENSIONS MAY NOT BE TO SCALE. DENOTES 5/8" REBAR W / CAP SET, L.B. # 7170. E----E DENOTES OVERHEAD ELECTRIC. **DENOTES IRON PIPE OR REBAR FOUND** X---X---X DENOTES EXISTING FENCE. LEGEND & NOTES 2) CLOSURE EXCEEDS 1:10.000 Ø DENOTES POWER POLE カノレ **INEST** PAVE HO CORNER

AVE

SOUTH RIGHT-OF-WAY LINE

COUNTY 676-29'(C) 676-35'(A)

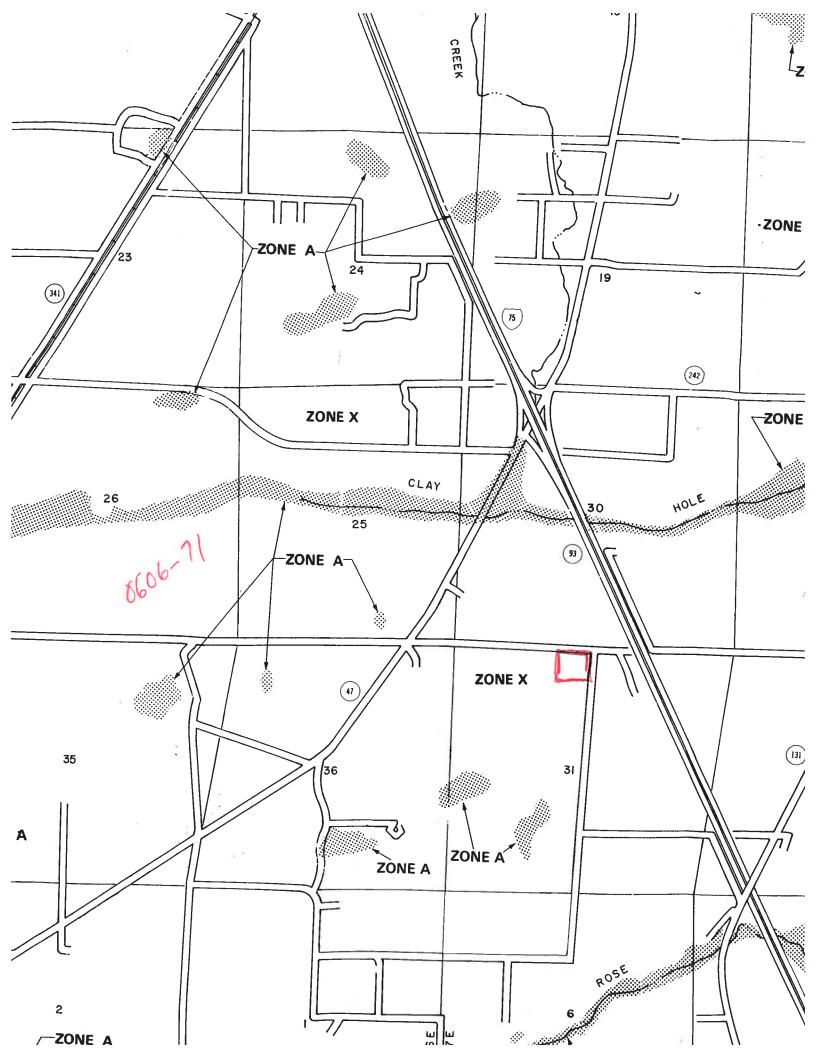
JIN

STATE OF FLORIDA DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

AIT EIGHTION TON ONOTILE	Permit Application	Number 06-0546N
Seale: 1 inch = 50 feet	PART II - SITEPLAN	
Scale: 1 inch = 50 feet.	J. C. J.	13, 19
Notes: 1 of 8.5	ARES	NG-
Site Plan submitted by: Plan Approved 2	Not Approved	MASTER CONTRACTOR Date 6-13-06
By Mn om	(6)0mb?q	County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT



Notice of Treatment 4012129						
Applicator: Florida Pest Control & Chemical Co. (www.flapest.com) Address: 536 58 8 944 08 City Phone (386) 752-1703						
Site Location: Subdivision N/A Lot # Block# Permit # Address 724 Sw wester Rd. Lake Coly						
Product used Active Ingredient % Concentration Premise Imidacloprid 9.1% 05 %						
Termidor Fipronil 0.12%						
Bora-Care Disodium Octaborate Tetrahydrate 23.0%						
Area Treated Square feet Linear feet 36.8 As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior						
If this notice is for the final exterior treatment, initial this line Date Time Print Technician's Name Remarks:						
Applicator - White Permit File Canary Permit Holder - Pink 10/05 ©						

The second section of the second seco							
Notice of Treatment 12129							
Applicator: Florida Pest Control & Chemical Co. (www.flapest.com)							
Address: BA	VAAVE						
City ZF	7Ke City Phone 752-1703						
Site Location: Subdivision							
	Block# Permit # 24686						
Address 724	SW Wester OR						
Product used	Active Ingredient % Concentration						
Premise	Imidacloprid 0.1%						
☐ Termidor	Fipronil 0.12%						
☐ Bora-Care	Disodium Octaborate Tetrahydrate 23.0%						
	Disourant Setabolate Tettanyarate 20.0%						
Type treatment:	☐ Soil ☐ Wood						
Type treatment.	3011 U WOOD						
Area Treated	Square feet Linear feet Gallons Applied						
Duelling Proches	Gorge 4786 368 500						
*							
As per Florida Buil	ding Code 104.2.6 – If soil chemical barrier method for						
termite prevention i	s used, final exterior treatment shall be completed prior						
to final building app	proval.						
If this notice is for t	he final exterior treatment initial this line						
If this notice is for the final exterior treatment, initial this line							
8/28/06	(180) F754 Guerric						
Date	Time Print Technician's Name						
Remarks:							
Applicator - White Permit File - Canary Permit Holder - Pink							
10/05 ©							



OCCUPANC

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

31-48-17-08920-000 Parcel Number

Building permit No. 24686

þ

Waste:

Fire:

Total:

Use Classification SINGLE FAMILY DWELLING

GUY WILLIAMS Permit Holder

MIKE HUNTER Owner of Building Location: 724 SW WESTER DRIVE, LAKE CITY,

Date: 3-13-07

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)



GTC Design Group, LLC P.O. Box 187 Live Oak, FL 32064 (Phone) 386.362.3678 (Fax) 386.362.6133 ggill@gtcdesigngroup.com

June 26, 2006

Joe Haltiwanger Plans Examiner 135 NE Hernando Avenue P. O. Box 1529 Lake City, Florida 32056-1529

SUBJECT: Mike Hunter - Garage Beam

Joe,

The garage header shall be a Truss-Joist Microlam 3 1/2x 11 $\frac{1}{4}$ " (min bending stress 2,600 psi) or equal.

If you have any questions or require additional information, please contact me at your convenience.

Thank you,

Gary Gill, P.E. #51942

Project Manager



From: The Columbia County Building & Zoning Department

Plan Review

135 NE Hernando Av.

P.O. Box 1529

Lake City Florida 32056-1529

Reference to a building permit application Number: 0606–71

Contractor: SLK Construction Owner Mike Hunter 724 SW Wester Dr.

On the date of June 22, 2006 application 0606-71 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0606-71 when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

To help ensure compliance with the Florida Residential Code 2004 the comments below need to be addressed on the plans.

1. Please submit a recorded (with the Columbia County Clerk Office) notice of commencement before any inspections can be preformed by the Columbia County Building Department.

- 2. Sheet S-2.0 of the plans details a typical garage header detail; please have Mr. Gill design the header beam which will span the 18 foot overhead garage door opening.
- 3. On the electrical plan show the location of the electrical panel and include the total amperage rating of the electrical service panel also show the overcurrent protection device which shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

Joe Haltiwanger

Plan Examiner

Columbia County Building Department



GTC Design Group, LLC P.O. Box 187 Live Oak, FL 32064 (Phone) 386 362 3678 (Fax) 386 362 6133 ggill@gtcdesigngroup.com

June 26, 2006

Joe Haltiwanger Plans Examiner 135 NE Hernando Avenue P. O. Box 1529 Lake City, Florida 32056-1529

SUBJECT: Mike Hunter - Garage Beam

Joe,

The garage header sha'l be a Truss-Joist Microlam 3 1/2x 11 1/4" (min bending stress 2,600 psi) or equal.

If you have any questions or require additional information, please contact me at your convenience

Thank you,

Gary Gill, P.E. #51942

Project Manager

6/25/01

Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1SXB487-Z0418171404

Truss Fabricator: Anderson Truss Company

Job Identification: 6-202--SLK Construction Hunter -- , **

Truss Count: 24

Model Code: Florida Building Code 2004
Truss Criteria: ANSI/TPI-2002(STD)/FBC
Engineering Software: Alpine Software, Version 7.24.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-02 -Closed

Notes:

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: BRCLBSUB-A11015EE-GBLLETIN-MAX DEAD LOAD-PIGBACKB-A11030EE-

	#	Ref Description	Drawing#	Date
	1	19268 A	06138095	05/18/06
	2	19269 A2	06138096	05/18/06
	3	19270A3	06138097	05/18/06
	4	19271 A4	06138098	05/18/06
	5	19272 AT	06138099	05/18/06
	6	19273 AV	06138100	05/18/06
	7	19274 AV	06138101	05/18/06
	8	19275 AV - 1	06138102	05/18/06
	9	19276H8A	06138109	05/18/06
	10	19277В	06138086	05/18/06
	11	19278 BGE	06138103	05/18/06
	12	19279 C	06138087	05/18/06
	13	19280EJ8	06138088	05/18/06
	14	19281HJA	06138104	05/18/06
	15	19282 J1A	06138105	05/18/06
	16	19283 J3A	06138089	05/18/06
d	17	19284 J5A	06138090	05/18/06
	18	19285 J7A	06138091	05/18/06
	19	19286M	06138092	05/18/06
	20	19287 M1	06138093	05/18/06
ļ	21	19288MGE	06138106	05/18/06
	22	19289 AP	06138107	05/18/06
	23	19290 DOR	06138094	05/18/06
	24	19291 DORG	06138108	05/18/06

Jul 1-

Seai Date: 05/18/2006

-Truss Design Engineer-Denise Rutledge Florida License Number: 58752 1950 Marley Drive Haines City, FL 33844



- 24' -

-- 16' -

PAGE NO: 1 OF

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Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 PLT TYP. In lieu of structural panels or rigid ceiling use purlins to brace TC $24\,^{\circ}$ OC, BC @ $24\,^{\circ}$ OC. Left end vertical not exposed to wind pressure (6-202 - SLK Construction Hunter Alpine Engineered Products, Inc. ALPINE Wave 1.5X4 **Ⅲ** 4×5/ 33844 n # 567 R-1308 U-180 W-4" **IMPORTANT***QUENTS A GOPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPINE ENGINEERED
PRODUCTS, INC. SMALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE
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DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MIS (MATIONAL DESIGN SPEC, BY AFRA) AND TPI:

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SPACING

24.0"

JREF -

1SXB487 Z04

Left end vertical not exposed to wind pressure

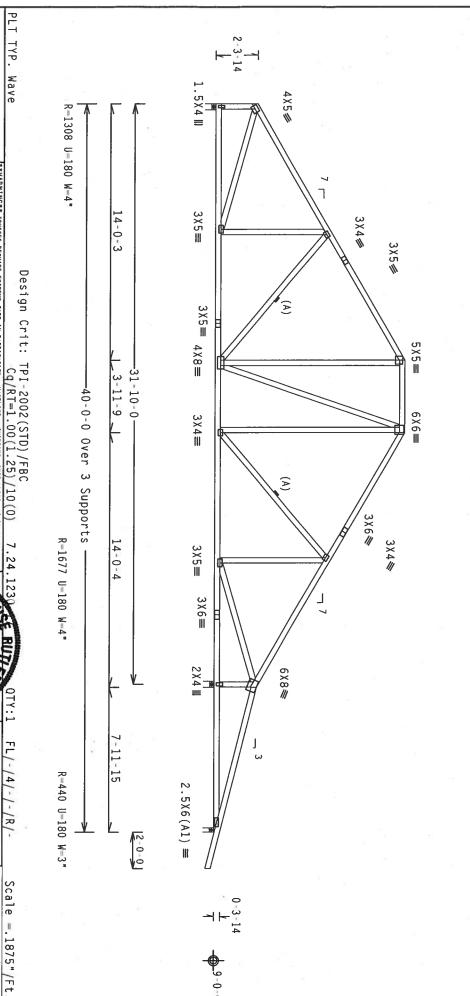
In lieu of structural panels or rigid ceiling use purlins to brace TC $24\mbox{\ensuremath{^{\circ}}}\ 0\mbox{\ensuremath{^{\circ}}}\ 0\mbox{\ensuremath{^{\circ}}}\ 0\mbox{\ensuremath{^{\circ}}}\ 0\mbox{\ensuremath{^{\circ}}}\ 0\mbox{\ensuremath{^{\circ}}}\ 0$

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110 mph wind, 15.00 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) Continuous lateral bracing equally spaced on member

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



IMPORTANTGURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPINE ENGINEERED PRODUCTS, INC., SMALL NOT BE RESONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN CONFORMACE WITH TPI; OR FABRICATION, CHAPLING, SHEPPING, INSTALLING B REACHING TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MDS (MATIONAL DESIGN SPEC, BY ASBA) AND TPI.

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ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF TPI1-2002 SEC.3. A SEAL ON THIS PLATES TO EACH FACE OF TRUSS AND, UP ANY INSPECTION OF PLATES FOLLOWED BY DRAWING INDICATES ACCEPTANCE OF PROF RIGIO CEILING. I, POSITION PER DRAWINGS 160A Z
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Alpine Engineered Products, Inc.
1950 Marley Drive
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(A) Continuous lateral bracing equally spaced on member.

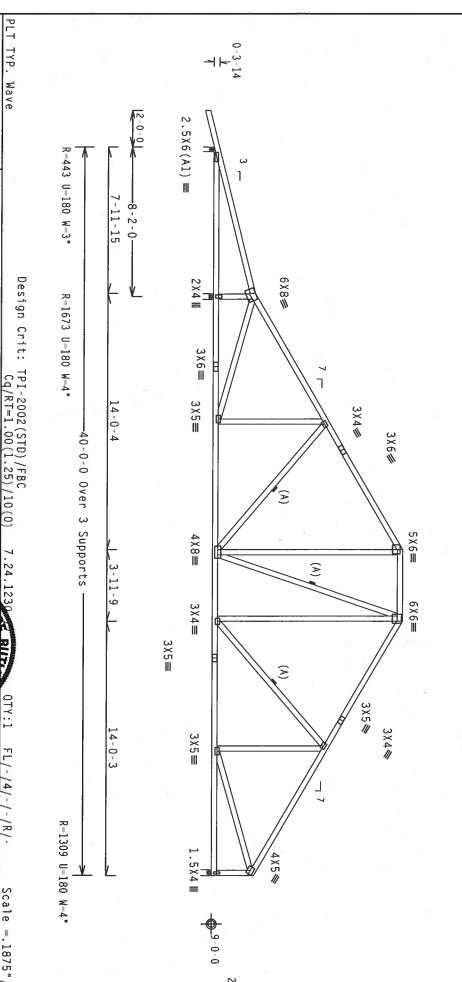
In lieu of structural panels or rigid ceiling use purlins to brace TC 24" 0C, BC @ 24" 0C.

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110 mph wind, 15.00 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



MARNING TRUSSES REGUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. REFER TO BESI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'OMORFICO RR., SUITE 200, MADISON, HI 53719) AND NICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LR. MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

PLT TYP.

Wave

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SMALL NOT BE RESPONSIBLE FOR ANY DEVILATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS. IN CONCORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING BRACKING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC, BY ARBA), AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (N.H/S/N.) ASTM A653 GRADE 40/60 (N. K/H.S), GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISS ANY INSPECTION OF PLATES FOLLONED BY (1) SHALL BE IDRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGI

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Alpine Engineered Products, Inc. 1950 Marley Drive Haines City, FL 33844

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Scale = .1875"/Ft. R487-- 19270

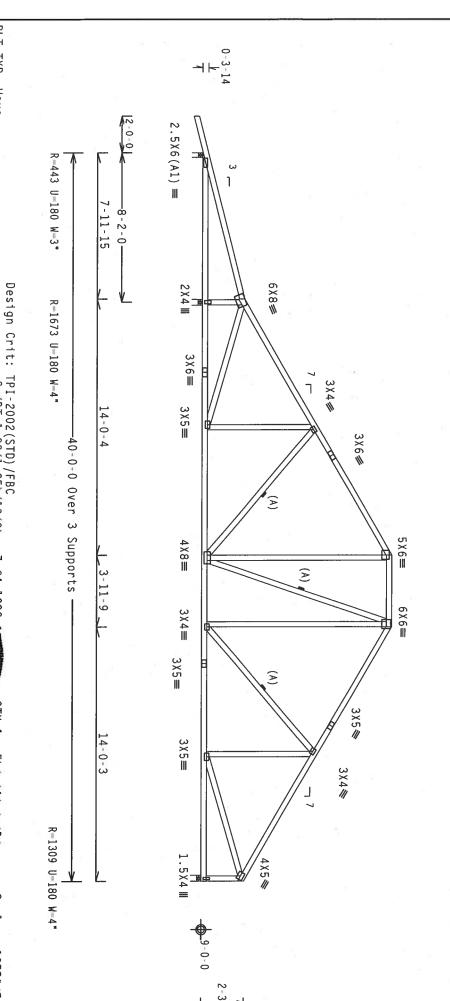
In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ 24" OC. $\widehat{\mathbb{A}}$ Continuous lateral bracing equally spaced on member.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

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Alpine Engineered Products, Inc. 1950 Marley Drive Haines City, FL 33844

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WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION. MANDLING. SHIPPING, INSTALLING AND BRACING. RETER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE. 583 0'OHOFRIO BN. SUITE ZOO, ANDISOM, HI 53719) AND MICA (MODO TRUSS COUNCIL OF AHREIGA, 6300 ENTERPRISE IN, MADISOM, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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Left end vertical not exposed to wind pressure

In lieu of structural panels or rigid ceiling use purlins to brace 24" OC, BC @ 24" OC.

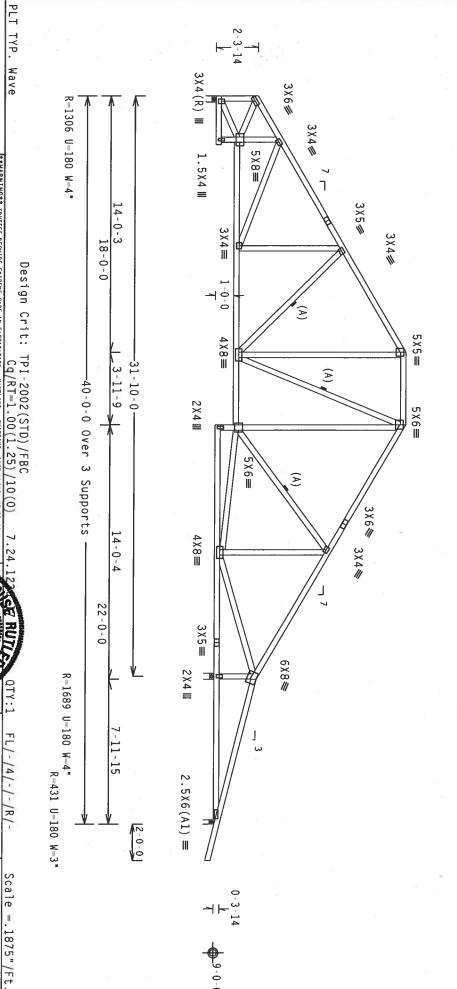
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110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0

(A) Continuous lateral bracing equally spaced on member

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, MANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BOSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583) D'OMORFAIO BR. SUITE 200, ANDISON, MI 53719) AND MICA (MODO TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE IN, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INTOLATED, TOP CHORD SHALL MANE A PROPERLY ATTACHED STRUCTURAL PAMELS AND BOTTOM CHORD SHALL MANE A PROPERLY ATTACHED RIGIO CEILING.

IMPORTANT*DURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

AND PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN.

TRUSS IN COMPRENANCE WITH PIP: OR FARRICATING, HANDLING, SHEPPING, INSTALLING BRACH

DESIGN CONFORMS HITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC, BY ARBYA) AND TPI

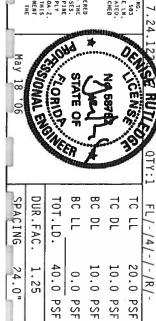
CONNECTOR PLATES ARE MADE OF 20,18/16GA (H.H/S/K) ASTA MASS GRADE 40/60 (H.K/H.S) GALV. S.

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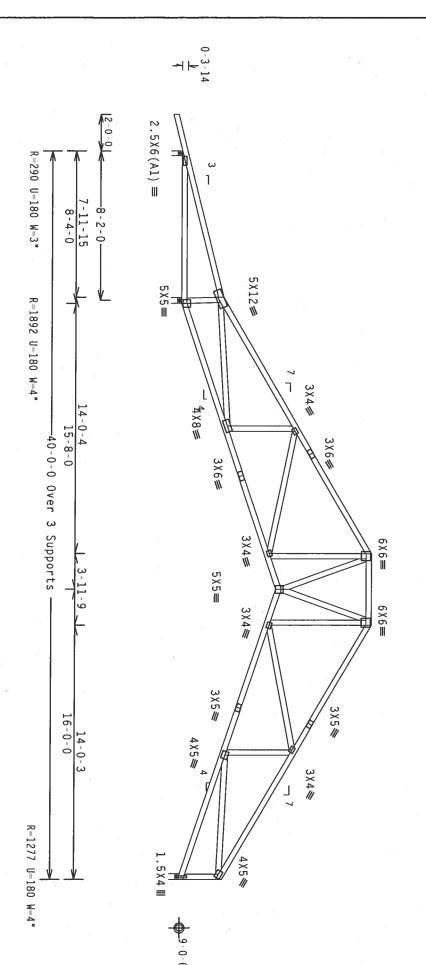
Shim all supports to solid bearing. Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 In lieu of structural panels or rigid ceiling use purlins to brace TC $24\mbox{\ensuremath{^{\circ}}}$ OC, BC @ $24\mbox{\ensuremath{^{\circ}}}$ OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

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Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



RIGID CEILING

PLT TYP.

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Design Crit:

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Alpine Engineered Products, Inc. 1950 Marley Drive Haines City, FL 33844 "Cate of A" n# 567

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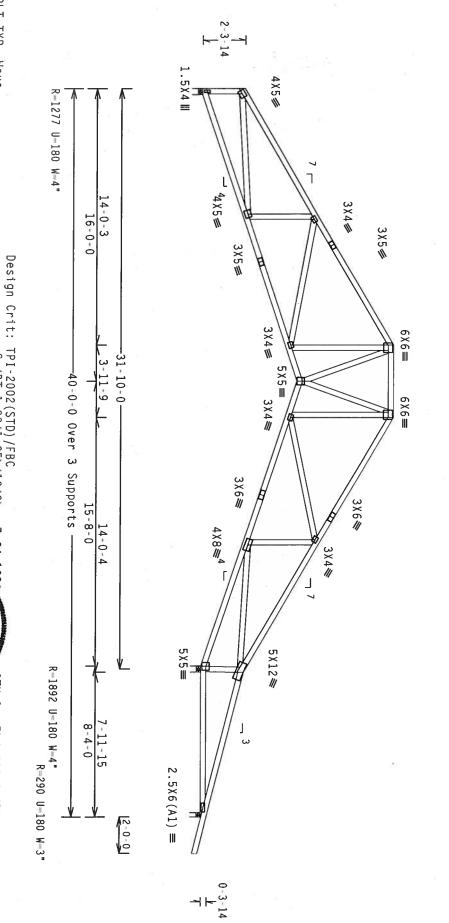
Left end vertical not exposed to wind pressure

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is $1.50\,\mathrm{cm}$

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Shim all supports to solid bearing.



NARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPJ (TRUSS PLATE INSTITUTE: 583 0'-0000 FRIS COUNCIL OF ARREIA, 3000 ENTERPRISE LIN, MADISON, MI \$3219) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE TUNCTIONS. UNLESS OTHERNISE INDICATED, TOP CORDS SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FAILURE TO BUILD THE PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE RUSSES IN CONFORMANCE WITH THE THE FOR FABRICATION, HANDLUNG, SHPPING, INSTALLING & BRACING OF FRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NIDS (MATIONAL DESIGN SPEC, BY ATRAPA) AND TPI. APPLICABLE PROVISIONS OF NIDS (MATIONAL DESIGN SPEC, BY ATRAPA) AND TPI. APPLICABLE OF PROVISIONS OF NIDS SPEC, APPLY PLATES TO EACH FACE OF TRUSS AND, JUNESS OTHERNISE LOCATED ON THIS DESIGN PROSITION PER DRAMINGS 160A. Z. ANY JUSPECTION OF PLATES FOLLOWED BY (1) SHALD BE PER ANNEX SO TIPIT-2002 SEC.). A STAL ON THIS DESIGN SHOWN. THE SUITABLITY AND USE OF THIS CONPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING IS THE RESPONSIBILITY OF THE

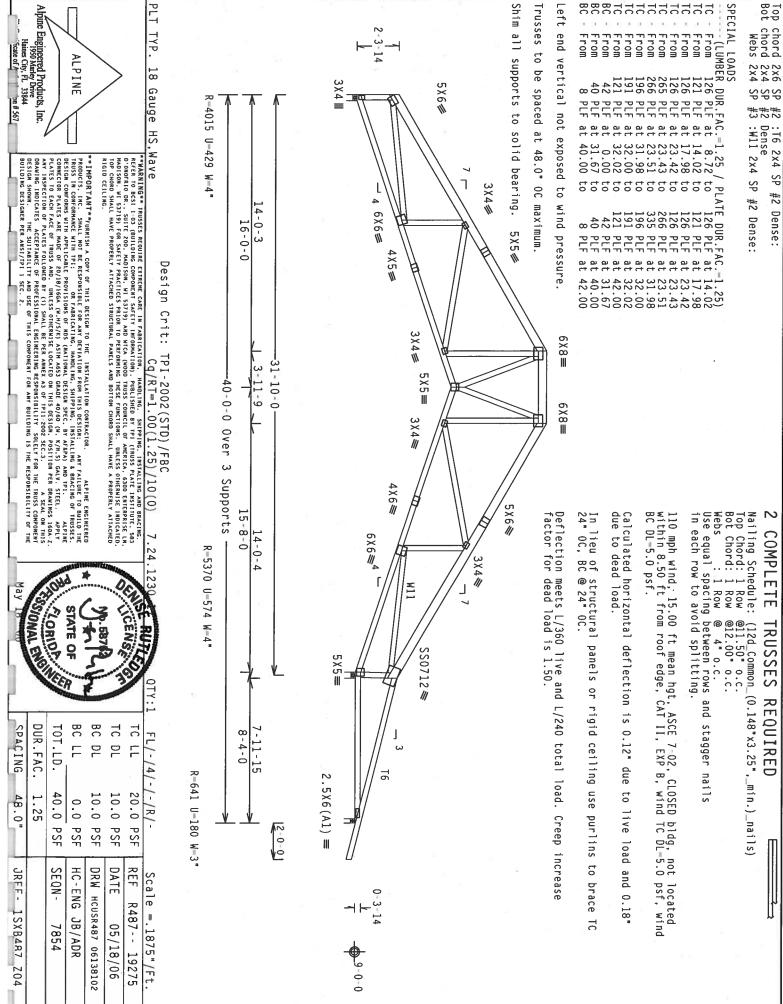
Alpine Engineered Products, Inc. 1950 Marley Drive Haines City, FL 33844

ALPINE

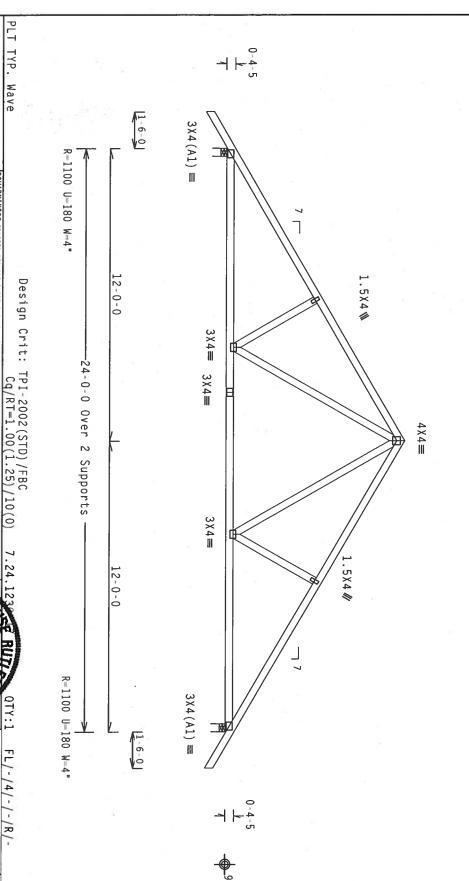
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Scale =.1875"/Ft.



Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 In lieu of structural panels or rigid ceiling use purlins to brace TC @ $24\,\text{m}$ OC, BC @ $24\,\text{m}$ OC. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



HARNING IRUSSES REQUIRE EXTREME CARE IN FARRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. REFER TO BESS 1-0.0 (BULDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPJ (FIRUSS PLAIF INSTITULE; 563) ONORFRIO DR., SHITE 200, HADSSM, HI 53719) AND NITCA (4000 TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE IN, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HESE FINCTIONS. WHEESO THERMISE INDICATED. TO CHING SHALL HAVE PROPERLY ATTACHED REGID CEILING. **IMPORTANT**PUBNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. AND REGIDERAL HAVE A PROPERLY ATTACHED **IMPORTANT**PUBNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. AND REGIDERAL HAVE A PROPERLY ATTACHED **IMPORTANT**PUBNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. AND REGIDERAL HAVE A PROPERLY ATTACHED **IMPORTANT**PUBNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. AND REGIDERAL HAVE A PROPERLY ATTACHED **IMPORTANT**PUBNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. AND REGIDERAL HAVE A PROPERLY ATTACHED **INCOMPLETED TO THE TRUSS AND. UNITESS OF HOST (MATIONAL DESIGN SPEC. BY ASADA) AND TPJ. CONNECTED TO THE STATE OF THE PUBLISH AND USES OF HIS MESS AND. UNITESS OF HIS MESS AND OF THIS DESIGN. POSITION PER DRAWINGS 160A. Z. ANY INSPECTION OF PLATES POLICISED BY (1) SHALL BE PER AND A 400 OF THIS DESIGN SPECE.). A SEAL ON THIS DESIGN SHALL HAVE A DESIGN SECO.). A SEAL ON THIS DESIGN SHALL HAVE A DESIGN SHALL HAVE							
May 18 06	SIONAL ENG	CORNOR	STATE OF	のかって	Mo 5878	CENS	
SPACING 24.0"	DUR.FAC. 1.25	TOT.LD.	BC LL	BC DL	TC DL	TC LL	
24.0"	1.25	TOT.LD. 40.0 PSF SEQN-	0.0 PSF	10.0 PSF	10.0 PSF DATE	20.0 PSF	
JRFF- 1SXB487 Z04		SEQN- 7815	HC-ENG JB/ADR	10.0 PSF DRW HCUSR487 06138086	DATE 05/18/06	20.0 PSF REF R487 19277	

Scale =.25"/Ft.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf.

Dead loads are stated on projected horizontal area basis.

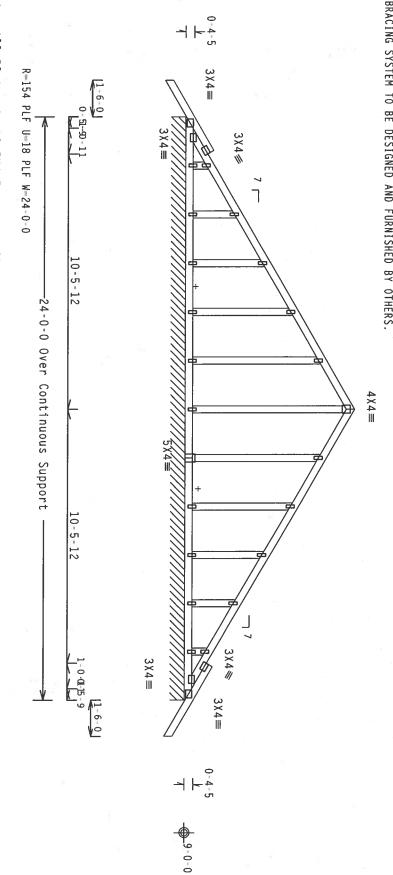
Truss spaced at 24.0" 0C designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of structural panels or rigid ceiling use purlins to brace TC 24" 0C, BC @ 24" 0C.

See DWGS All015EE0405 & GBLLETIN0405 for more requirements.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS.
BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.



Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

HARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPIG, HYSTALING AND BRACING,
REFER TO BCSI 1-03 (BUILDING COMPORENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583)
D'ONORFRIO DR., SUITE 200. ANDISON, HI 53719) AND HICA (MODO TRUSS COUNCIL OF AMERICA, 5000 ENTERPRISE IN.
HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OHERMISE INDICATED.
TOP COMBOD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED
RIGID CEILING.

IMPORTANT*URMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FALLURE TO BUILD THE TRUSS IN CONFERNACE AITH PE:

RUSS IN CONFERNANCE AITH PE:

CONNECTOR PLATES ARE MADE OF 20/18/160A (A.H./5/A), ASTH ASS 3 GANDE 40/56 (W. K/H.S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERNISE LOCATED ON THIS DESIGN. POSITION FER DRAWINGS 160A-2. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERNISE LOCATED ON THIS DESIGN. POSITION FER DRAWINGS 160A-2. AND THIS DESIGN OF PLATES OF PLOUMED BY (1) SHALL BE FOR AMEX AS OF TPI-2002 SEC. 3. A SEAL ON THIS DESIGN SHOWN. THE SUITABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2.

Alpine Engineered Products, Inc. 1950 Marley Drive

ALPINE

Haines City, FL 33844

	12	- 0.18				
SNIJVES	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	ול דר
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
JRFF- 1SXB487 Z04		SEQN- 7844 REV	HC-ENG JB/ADR	DRW HCUSR487 06138103	DATE 05/18/06	REF R487 19278

Scale

=.25"/Ft

THIS UNG TRETAKED FROM COMPUTER INPUT (LUADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Catte of A n # 567 PLT TYP. Provide (2) 16d common nails $(0.162^*x3.5^*)$, toe nailed at Top chord. Provide (2) 16d common nails $(0.162^*x3.5^*)$, toe nailed at Bot chord. In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ 24" OC. (6-202--SLK Construction Hunter ALPINE Wave 0-3-14 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPINE ENGINEERED PRODUCTS, INC. SHALL HOT BE RESPONSIBLE FOR MAY DEVLATION FROM THIS DESIGN: MAY FAILURE TO BUILD THE FUNDS. IN COMPORANCE WITH PIT:

OR FABRICATION, LHADLING, SHIPPING, INSTALLING A BACKLON OF TRUSSES, DESIGN COMPORTS WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC. BY AFAPA) AND TPI.

CONNECTOR PLATES ARE HADE OF 702/18/16GA (M. 14/3/K) ASTM ASS GRADE 40/60 (M. K/H.S) GALY. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND.

FINE STORMACH ACCE OF TRUSS AND. UNICES OTHERS LOCATED ON THIS DESIGN, POSITION BEER ORANIMOS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANKEX AND TETI-2002 SEC.3.

DRAHME INDICATES ACCEPTANCE OF PROFESSIONAL EMIGHERING RESPONSIBILITY OF THE TRUSS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE **MARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BESI 1-03 (BUILDING COMPONENT SAFFIV INFORMATION), PUBLISHED BY IPI (TRUSS PLATE HASTITUTE, 583 D'OMOFRIO DR., SUITE ZOD, MADISON, HI 53718) AND WITCA (MODD TRUSS COUNCIL OF MERICA, 5000 ENTERPRISE LH, MADISON, HI 53718) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CEILING. DESIGN SHOWN. THE S BUILDING DESIGNER PER **-2-0-0-**A SEAL ON THIS SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE RUSS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE RUSS. EJ8) Design Crit: $2X4(A1) \equiv$ R=476 U=180 W=3" MX TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) -8-0-0 Over 3 Supports ര 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is $1.50\,.$ CENS ORIOR HER STATE OF R-94 U-180 R-205 U-180 BC DL TC DL SPACING DUR.FAC. BC LL TC LL TOT.LD. האוו הידט זעו הו לדהטתה מ הזוורעיבותנה! התחוודוובה הו ונחקה ומיני FL/-/4/-/-/R/-40.0 24.0" 1.25 10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF 9-0-0 PSF DATE REF JREF-SEQN-DRW HCUSR487 06138088 HC-ENG Scale =.5"/Ft. R487--1SXB487 Z04 JB/ADR 05/18/06 7829 19280

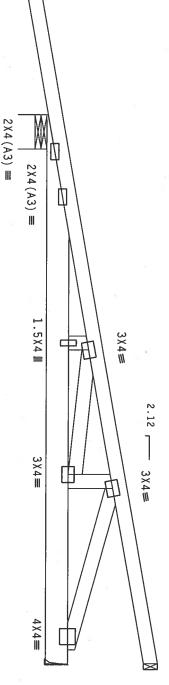
In lieu of structural panels or rigid ceiling use purlins to brace TC $24\mbox{\ensuremath{^{\circ}}}\ 0\mbox{\ensuremath{^{\circ}}}\ 0\mbox{\ensuremath{^{\circ}}}\ BC\ @\ 24\mbox{\ensuremath{^{\circ}}}\ 0\mbox{\ensuremath{^{\circ}}}\ .$

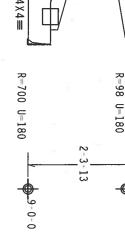
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind psf. 7-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC DL=5.0

Hipjack supports $8\mbox{-}0\mbox{-}0$ setback jacks. Jacks up to 7' have no webs Longer jacks supported to BC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord





R-582 U=180 W-8.485 -11-3-12 Over 3 Supports

Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0)

PLT TYP.

Wave

***HARNING** TRUSSES REQUIRE EXTREME CARE IN FARRICATION, HARDLING, SHIPPING, INSTALLING AND BRACING.
***RETER TO BECSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TEY (TRUSS PLATE INSTITUTE, 593)
***D'ONOFRIO OR., SUITE 200, HADISON, HI 53219) AND MTCA (MODD TRUSS COUNCIL OF AMERICA, SDOC ENTERPRISE U.N.
***MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERNISE INDICATED,
***IOPORO SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED
RIGHD CEILING.

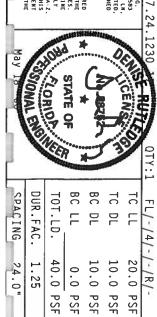
IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FAILURE TO BUILD THE PRODUCTS, IRC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH FIL.

OF ARBEICATHOR, HANDLING, SHIPPING, HAPDLING, SHIPPING, SHIPPIN BUILDING DESIGNER PER DRAWING INDICATES ADE 40/60 (W. K/H.S) GALY. STEEL. APPLY. THIS DEED, POSITION PER DRAMHMS 160A.Z OF TRIT-2002 ESC. 3. A SEAL ON THIS ONSIBILITY SOLELY FOR THE TRUSS COMPONENT ANY BUILDING IS THE RESPONSIBILITY OF THE

Alpine Engineered Products, Inc. 1950 Marley Drive

ALPINE

fcate of / n # 567



PSF

SEQN-

HC-ENG

JB/ADR 7837

DRW HCUSR487 06138104

JREF-

1SXB4R7

Z04

DATE REF

05/18/06

Scale =.5"/Ft.

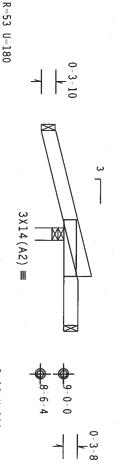
R487-- 19281

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC $24\mbox{\tt "}$ OC, BC @ $24\mbox{\tt "}$ OC. ®

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R-18 U-180

←-4-0-0 Over 3 Supports -> R=152 U=180 W=3"

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT TYP.

Wave

#ARNING FRUSES REQUIRE EXTREME CARE IN FABRICATION, HANDISHE, SHPPING, INSTALLING AND BRACING.

BEFOR TO BCS1 103 (BUILDING COMPOMENT SAFETY INFORMATION), PUBLISHED BY FPI (TRUSS PLATE INSTITUTE, S83
D'ONDFRIO DR., SUITE ZOO, ANDISON, HI 53279) AND NICA (4000 TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE IN, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNIESS OTHERWISE INDICATED.

TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

CENS

BC DL BC LL

10.0 PSF 0.0 PSF PSF

DRW HCUSR487 06138105

HC-ENG SEQN-

JB/ADR

TC DL

DATE REF

05/18/06

TC LL

20.0 PSF 10.0 PSF

FL/-/4/-/-/R/-

Scale =.5"/Ft.

R487-- 19282

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FAILURE TO BUILD THE PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE RUSS IN COMPONEME WITH FPI;

RUSS IN COMPONEME WITH FPI;

OF FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN COMPONEME WITH APPLICABLE PROVISIONS OF HIS GRACING OF TRUSSES, DESIGN COMPONEMS WITH APPLICABLE PROVISIONS OF HIS GRACING OF URLESS OF ADAPT OF THE COMPONEMS OF THE APPLICABLE OF T DESIGN SHOWN. THE SUITABILITY AND BUILDING DESIGNER PER ANSI/TPI 1 SEC.

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Teate of A 'n # 567

dey Drive FL 33844 nn # 567

ALPINE

BILITY SOLELY FOR THE TRUSS COMPONENT BUILDING IS THE RESPONSIBILITY OF THE DUR.FAC. SPACING TOT.LD.

> 1.25 40.0

24.0"

JRFF- 1SXB487

204

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense

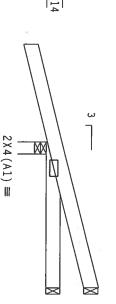
In lieu of structural panels or rigid ceiling use purlins to brace TC $24\mbox{\,^{\circ}}$ OC, BC @ $24\mbox{\,^{\circ}}$ OC.

e

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord. Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

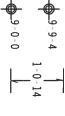
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R-49 U-180

R-16 U-180



-2-0-0-3-0-0 Over 3 Supports =306 U=180 W=3"

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT TYP.

Wave

WARNING TRUSSES REQUIRE EXTRÊME CARE IN FABRICATION, MANDLING. SHIPPING, HEYALLING AND BRACING.
REFER TO BEST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, 583
D'OMOFRIO DR. SUITE ZOO, MADISOM, HI 53719) AND BITCA (MODO TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE HA, MADISOM, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGIO CEILING.

IMPORTANTFORMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FAILURE TO BUILD THE PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN COMPONENCE WITH APPLICABLE PROVISIONS OF ROS (MATIONAL DESIGN SPEC, BY AFAPA) AND TPI.

CONNECTOR PLATES ARE MADE OF ZO/JRJOGAC (M.H.S.Y.) ASTH ASS J GRADE 40/50 (M.K.Y.H.S.) GALV. STEEL. APPLY LATES TO EACH FACE OF TRUSS AND. JUNESS OTHERNISE LOCATEO M.H.S. DESIGN. POSITION PER DRAWINGS 160A.Z.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AIREX AS OF TPIT-2002 SEC.3.

AS SALO MITHS
DRAHIMG INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE RUSS COMPONENT
THE STRUCTURE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE RUSS COMPONENT
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THE STRUCTURE OF THE STRUCTURE OF PROFESSIONAL THIS DESIGN AND THIS DESIGN AS THE STRUCTURE OF THE STRUCTU DESIGN SHOWN. THE SUITABILITY AND I BUILDING DESIGNER PER ANSI/TPI 1 SEC. VY AFRAD, AND TPI.

M. K/H.S) GALV. SIEEL. APPLY

N. POSITION PER DRAMINGS 160A.Z.

ZOZ SEC.J.

SOZ SEC.J.

A SEAL ON THIS

SOZELY FOR THE TRUSS COMPONENT

MG IS THE RESPONSIBILITY OF THE

Alpine Engineered Products, Inc. 1950 Marley Drive Haines City, FL 33844

ALPINE



10.0 PSF 10.0 PSF 20.0 PSF

DRW HCUSR487 06138089

JB/ADR 7823

DATE REF

05/18/06

Scale = .5"/Ft.

R487-- 19283

0.0 PSF PSF

40.0

SEQN-HC-ENG

24.0" 1.25

JRFF-

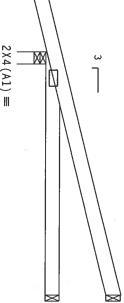
1SXB487 Z04

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense In lieu of structural panels or rigid ceiling use purlins to brace TC $24\,\text{m}$ OC, BC @ $24\,\text{m}$ OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

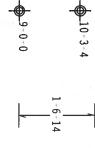
Provide (2) 16d common nails (0.162"x3.5"), toe nailed at Top chord. Provide (2) 16d common nails (0.162"x3.5"), toe nailed at Bot chord **®** Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is $1.50\,.$

ХX



R-51 U-180

R=116 U=180



-2-0-0-R=366 U=180 W=3" -5-0-0 Over 3 Supports

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT TYP.

Wave

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION. MANDLING. SHIPPING. INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING CORPORNENT SAFETY INFORMATION), PUBLISHED BY IPI (TRUSS PLATE INSTITUTE. 883 D'ONDERIO BR. SUITE ZOD. MADISON, HI 53719) AND MICA (MODO TRUSS COUNCIL OF AMERICA, 6300 EMERPRISE IN, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAMELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

***IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FAILURE TO BUILD THE PRODUCTS. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN COMPENSATION FOR THIS PRODUCTS. INC. SHAPLICABLE PROVISIONS OF ROS (MATIONAL DESIGN SPEC, BY AFRA) AND TPI. APPLICABLE PROVISIONS OF ROS (MATIONAL DESIGN SPEC, BY AFRA) AND TPI. APPLICABLE PROVISIONS OF ROS (MATIONAL DESIGN SPEC, BY AFRA) AND TPI. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERNISE LOCATED ON THIS OFSIGN, POSITION PER DRAWHOS 160A-Z. ANY INTES. AND PLATES FOLLOWED BY (1) SHALL BE PER ANKEX A OF TPILSORS SEC. 3. ASSEA. ON INTES. ORACHING INDICATES. ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANKSI/FPI 1 SEC. 3.

Alpine Engineered Products, Inc. 1950 Marley Drive

ALPINE

JEQ r 100 me of Marley Drive as City, FL 33844

May 18 SCIONAL ENGINEE CENSA TATE OF TC -11 FL/-/4/-/-/R/-

Scale =.5"/Ft.

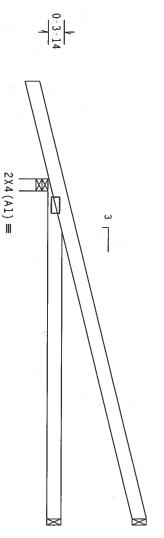
Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC $24\,^{\circ}$ OC, BC @ $24\,^{\circ}$ OC. **@**

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is $1.50\,.$



R=176 U=180

-0-14

R-80 U=180

-2-0-0-R-438 U-180 W-3* -7-0-0 Over 3 Supports

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT TYP.

Wave

WARNING IRUSSES REQUIRE EXTREME CARE IN FABRICATION. MANDLING. SHIPPING. INSTALLING AND BRACING. RECER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'OMOFRIO DN. SUITE 200, MADISOM, HI 53719) AND MICA (MOOD FRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE IN, MADISOM, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. MULESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FALLURE TO BULLO THE PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FALLURE TO BULLO THE RESPONSIBLE FOR THE PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR THE PERIOD FOR THIS DESIGN SHEET. ANY FALLURE TO BULLO THE RESPONSIBLE FOR PLATES IN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC, BY AREA) AND TET. APPLY CONNECTOR PLATES ARE HADE OF 70/18/16GA (M. H/5/K) ASTH AGSS GRADE 40/50 (M. K/H.5) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND MULCSS OTHERSISE (DATED ON THIS DESIGN, POSITION PER DRAWINGS 16GA.2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF TPIT-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL REAL SHEET AND TET. SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING DESIGNER PER ANSI/FPIT 1 SEC. 2.

Alpine Engineered Products, Inc. 1950 Marley Drive Haines City, FL 33844

ALPINE

May SIONAL ENGINEES CENS TATE OF SPACING FL/-/4/-/-/R/-

SPACING	DUR.FAC.	TOT.LD.		BC DL	TC DL	ול דר
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
JREF- 15XB487 Z04		SEQN- 7821	HC-ENG JB/ADR	DRW HCUSR487 06138091	DATE 05/18/06	REF R487 19285

Scale =.5"/Ft

PLT TYP. In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ 24" OC. Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 (6-202--SLK Construction Hunter lpine Engineered Products, Inc. ALPINE City, FL 33844 of / 'm#567 Wave ***IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ANY FAILURE TO BUILD THE PRODUCTS. INC. SMALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE RUSS IN COMPENSANCE WITH PET.

BESIGN COMPENS IN THE APPLICABLE PROVISIONS OF 1005 (MATIONAL DESIGN SPEC, BY AFRA) AND TPT.

BESIGN COMPENS WITH APPLICABLE PROVISIONS OF 1005 (MATIONAL DESIGN SPEC, BY AFRA) AND TPT.

CONNECTOR PLATES ARE MADE OF ZO/189/160A (M-H/S/Y) ASTA MASS GRADE 40/50 (M. K/H-S) GALV. STEEL. APPLY

PLATES TO EACH FACE OF TRUSS AND. DHESS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAMJHOS 150A-Z.

ANY IMPRECIOUS OF PLATES FOLLOWED BY (1) SMALL BE PER AMEX AS OF TPTI-2002 SEC.3.

AS AS ALO. NITHS

BRAHMOR INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT **WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING, REFER TO BCS1 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, 583 D'ONDFRIO BR. SE SUITE ZOO, MADISON, MI 53719) AND MICA (MODO TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LIN, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE A PROPERLY ATTACHED BUILDING DESIGNER PER RIGID CEILING. -2-0-0-3 Design Crit: 2X4(A1) == R-488 U=180 W=3* TPI - 2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) -8-4-0 Over 2 Supports OZ SEC.3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT OF THE 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Right end vertical not exposed to wind pressure. Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. C CENS May SIONAL ENGINEE R-313 U-180 W-4" 1.5X4 III 1.5X4 Ⅲ 网占 BC DL BC LL TC DL SPACING DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/-40.0 20.0 PSF 24.0" 1.25 10.0 PSF 10.0 PSF 0.0 PSF PSF DATE REF JRFF-SEQN-HC-ENG DRW HCUSR487 06138092 Scale =.5"/Ft. R487--1SXB4R7 JB/ADR 05/18/06 7824 19286 204

ים ויימיו הפיוי פורט ליוו פו לרמשמת פ מזורושתזמות / תחמונזיובה מו יוומחת ווו עי

*

MGE)

bldg, not located TC DL=5.0 psf, wind

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Truss spaced at 24.0" OC designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED within 3.56 ft from roof edge, CAT II, EXP B, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

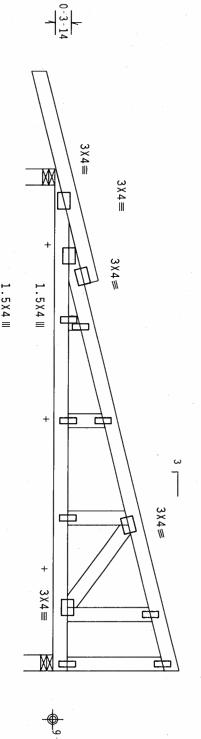
Dead loads are stated on projected horizontal area basis.

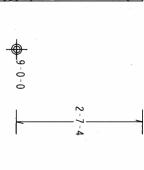
See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

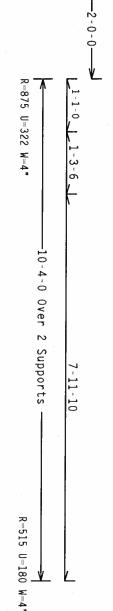
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF ROOF AND CEILING DIABLES OF THE DESIGN OF THE DES

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.







Note: All Plates Are 1.5X4 Except As Shown.

TYP.

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)
7.

RETER TO BCSI 1 03 (BUILDING COMPONENT SERVICE IN FABRICATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE. SEA
D-COMPETO DR., SUITE 200. MADISON, WI 53719) AND BRACING. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE. SEA
HADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED.

TOP CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE A PROPERLY ATTACHED

CENS

IMPORTANTFURMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPHE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FALLURE TO BUILD THE TRUSS IN COMPONANCE HITH PI:

OF SHARD CONTROL OF THE SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION, SHIPPING, INSTALLING BERACING OF RUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SEC. B. METARA) AND IPI.

CONNECTOR PLATES ARE MADE OF 20/18/16/66, (M-M-1/5/1), ASTH MASS GAADE 40/50 (M- 1/M-1), ADVINCE OF PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCALED ON THIS DESIGN POSITION FER DRAWINGS 160A Z.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANKEX AS OF TPIL 2002 SEC.3.

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ANY INSPECTION OF PLATES FOLLOWED BY (2) SHALL BE FER ANKEX AS OF TRIBE

Alpine Engineered Products, Inc. 1950 Marley Drive Haines City, FL 33844

ALPINE

W CONTRACTOR.

ALPINE ENGINEERED

PING, INSTALLING & BRACKING OF TRUSSES.

SPEC, BY AFRA) AND TPI.

ALPINE

40/50 (H. X/H.S) GALV STEEL. APPINE

50/25 (H. X/H.S) GALV STEEL. APPINE

50/25 (GL. X/H.S) GALV STEEL. APPINE

FILITY SOLELY FOR THE RUSS COMPONENT

BULLIDING IS THE RESPONSIBILITY OF THE

May 18 '06

	_		N				:-
SPACING 24.0"	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	TC LL	FL/-/4/-/-/R/-
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF	-/-/R/-
JRFF- 1SXB487 Z04		SEQN- 7849 RE	HC-ENG JB/ADR	DRW HCUSR487 06138106	DATE 05/18/06	REF R487 19288	Scale =.5"/Ft.

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

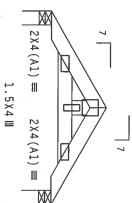
In lieu of structural panels or rigid ceiling use purlins to brace TC $24\mbox{\ensuremath{^{\circ}}}\ 0\text{C},\ \text{BC}\ @\ 24\mbox{\ensuremath{^{\circ}}}\ 0\text{C}.$ @

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 20.08 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0

REFER TO DWG PIGBACKBO405 FOR PIGGYBACK DETAILS.

REFER TO DWG PIGBACKB0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO
BE BRACED @ 24*0.C., UNLESS OTHERWISE SPECIFIED



4 X 4 ≡



1-1-13 | 1-1-13

(♣-3-11-9 Over 2 Support's → R=141 U=180 W=3.5" R=141 U=180 W=3.5"

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT TYP.

Wave

MARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDING, SHIPPING, INSTALLING AND BRACING.
REFER TO BESSEL 1-03 (BUILDING CORPORENT SAFETY INFORMATION), PUBLISHED BY FPI (RUSS PLATE INSTITUTE, 593
D'ONOFRIO DR., SUITE 200, HADISON, NI 53719) AND NICA (MODO) RUSS COUNCIL OF AMERICA, 6300 ENTERPRISE IN,
HADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERMISE INDICATED,
TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED
RIGID CEILING.

PRODUCTS. INC. SHALL MOT BE RESPONSIBLE FOR ANY DEVIATION FRONT HIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH PI;

PRODUCTS. INC. SHALL MOT BE RESPONSIBLE FOR ANY DEVIATION FRONT HIS DESIGN. INSTALLING & BRACHMO OT BUSISES. IN CONFORMANCE WITH IN FI!

DESIGN COMFORMANCE WITH APPLICABLE PROVISIONS OF MOS (MATIONAL DESIGN SPEC, BY ASEA) AND TO!

CONNECTION PLATES ARE MOS FOOLOGIBLE WITH APPLICABLE PROVISIONS OF MOS (MATIONAL DESIGN SPOC, BY ASEA) AND TO!

PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERNISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A-2.

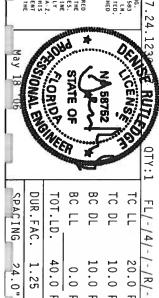
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL HE FER ANNEW AS OF TO!-2002 SEC. 2.

ASEALON MISSES

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITIABLITY MOD USE OF THIS COMPONENT FON ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI I SEC. 2.

Alpine Engineered Products, Inc.

33844 n#567 ALPINE



SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	י בר
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
107 L878XST - 33NP		SEQN- 7842	HC-ENG JB/ADR	DRW HCUSR487 06138107	DATE 05/18/06	REF R487 19289

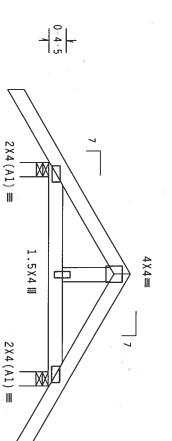
Scale =.5"/Ft

Top chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3

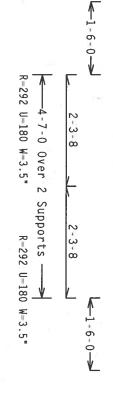
In lieu of structural panels or rigid ceiling use purlins to brace TC $24\mbox{\ensuremath{^{\circ}}}\ 0\text{C},\ BC\ @\ 24\mbox{\ensuremath{^{\circ}}}\ 0\text{C}.$ ര

110 mph wind, 16.91 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.







Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

QTY:1

FL/-/4/-/-/R/-

Scale =.5"/Ft.

PLT

TYP.

Wave

WARNING PRUSSES REQUIRE EXTREME CARE IN FABRICATION. MANDLING. SHIPPING. INSTALLING AND BRACING. REFER TO BEST 10.3 (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPJ (TRUSS PLATE INSTITUTE. 583 0 "ONDERLO DR. SUITE 200. MADISON. MI 53719) AND MICA (MODO TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE IN, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PAMELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

Alpine Engineered Products, Inc. 1950 Marley Drive Haines City, FL 33844

ALPINE

*

May 18 '06	SOUNAL EXT	CORNORING	STATE OF	○ そうてま	No. 58752	CENS, OF
SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	TC LL
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
JRFF- 1SXB487 Z04		SEQN- 7836	HC-ENG JB/ADR	DRW HCUSR487 06138094	DATE 05/18/06	REF R487 19290

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Truss spaced at 24.0" OC designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

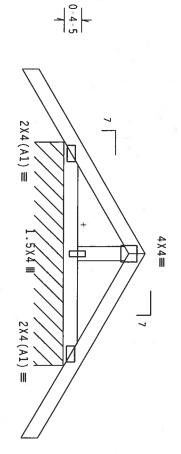
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 16.91 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 3.56 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

See DWGS All030EE0405 & GBLLETIN0405 for more requirements

In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ 24" OC.

MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.





1-6-0→ R-180 PLF U-75 PLF W-4-7-0 ▲ 4-7-0 Over Continuous Support 2-3-8 2-3-8 <u>V</u> **1**-6-0-√

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT TYP.

Wave

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FALLURE TO BUILD THE PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FALLURE TO BUILD THE TRUSS IN CONFORMANCE WITH PD!. OF FABRICATHICH, HANDLUNG, SHPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN COMPORMS WITH APPLICABLE PROVISIONS OF NDS (MATIONAL DESIGN SPEC, BY AFAPA) AND TPI. ALPINE COMMETCIOR PLATES ARE MADE OF 20/18/15GA (M. H/S/K) ASTH AGS3 GAADE 40/50 (M. K/H.S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE (DOATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.Z ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEY A3 OF TPI1-2002 SEC.3. A SEAL ON THIS RIGID CEILING

DRAWING INDICATES POO (M. K/H.5) CALV. STEEL. APPLY
DESIGN. POSITION PER DRAHINGS 160A.Z.
11-2002 SEC.3. A SEAL ON HIS
LITY SOLELY FOR THE TRUSS COMPONENT
UILDING IS THE RESPONSIBILITY OF THE

Jpine Engineered Products, Inc.

ALPINE

Haines City, FL 33844
FI Contificate of Arrhesization # 567

SIONAL EMBRISH TATE OF BC LL BC DL TC DL DUR.FAC. דכ רר SPACING TOT.LD. FL/-/4/-/-/R/-40.0 10.0 PSF 20.0 PSF 24.0" 1.25 10.0 PSF 0.0 PSF PSF

JREF -

1SXB487

Z04

SEQN-

HC-ENG

JB/ADR 7827

DRW HCUSR487 06138108

DATE REF

05/18/06

Scale =.5"/Ft.

R487-- 19291

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

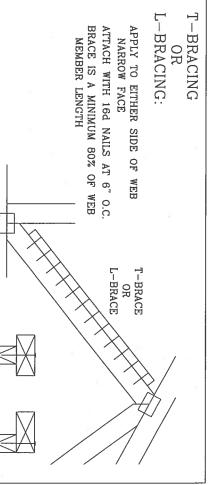
BRACING. THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING

2-2X6(*)	2X6 2X6	2 ROWS	2X8
1-2X6	2X4	1 ROW	2X6
2-2X4(*)	2X6	2 ROWS	2X6
1-2X4	2X4	1 ROW	2X3 OR 2X4
2-2X4	2X6	2 ROWS	2X3 OR 2X4
E BRACING	ALTERNATIVE BRACING T OR L-BRACE SCAB BR	SPECIFIED CLB	WEB MEMBER
SCAB BRACE		BRACING	SIZE

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

* CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

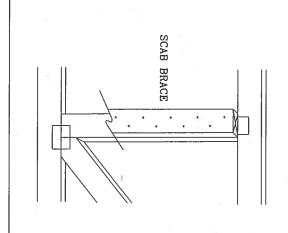


SCAB BRACING:

T-BRACE

L-BRACE

80% OF WEB MEMBER LENGTH NAILS AT 6" O.C. BRACE IS A MINIMUM NO MORE THAN (1) SCAB PER FACE. APPLY SCAB(S) TO WIDE FACE OF WEB. ATTACH WITH 10d OR .128"x3" GUN



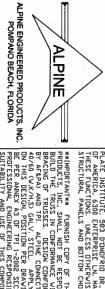
THIS DRAWING REPLACES DRAWING 579,640

PSF REF

CLB SUBST.

H/KAR LBSUB1103 TC LL

FRANTION, PUBLISHED BY TP! (TRRISS COUNCIL) 199 AND WICK -VORDO TRUSS COUNCIL 190 AND WICK -VORDO TRUSS COUNCIL 100 FROM THE PROPERLY ATTACHED ATTACHED RIGID CELLING. CONTRACTOR. ALPINE ENGINEERED CONTRACTOR. ALPINE CONTRA	No. 58752	TC DL PSF DATE 11/26/03 BC DL PSF DRWG BRCLBSUB: BC LL PSF —ENG MLH/KAR TOT. LD. PSF
ING. SHIPPING, INSTALLING &	No.58/52	
IONS OF NDS (NATIONAL DESIGN SPEC, 18/16GA (W.H/S/K) ASTH A653 GRADE	いで計グイー	
OF PLATES FOLLOWED BY (I) SHALL INDICATES ACCEPTANCE OF	OB TOP OF	DIIR FAC
S COMPONENT DESIGN SHOWN. THE HE RESPONSIBILITY OF THE BUILDING	S CONTRACTOR	SPACING.
	NOWAL TO	OF ACTIVG



WIMPORTANT## FURNISH COPY OF THIS DESIGN TO INSTALLATION COPPORTANT## FURNISH COPY OF THIS DESIGN TO INSTALLATION COPPORTATION.

PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION HANDS

BRACING OF TRUSSES. DESIGN CONFORMS WITH FIJE OF ABRICATION, HANDS

BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISES

BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISES

BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE FROWNES

BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE OF 2018

40/60 (VL/K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF THE

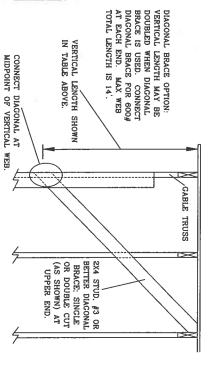
BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAVING

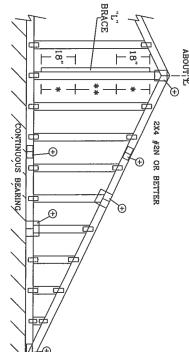
PROFESSIONAL RIGHTERING RESPONSIBILITY SIZELY FOR THE TRUSS

SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE

ASCE 7-02: 110 MPH WIND SPEED, 15 MEAN HEIGHT, ENCLOSED, II 1.00, EXPOSURE α

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		<u> </u>		<u>ر</u> ر)	TIT	I I	בילט			<u> </u>	1	<u>ر</u> ر)	TTT	L T	מלק	Ω]]		<u> </u>	j 	υ. Τ)	TIT	I I	נילו	S D D	SPACING SPECIES GRADE	2X4 GABLE VERTICAL	
	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE	
	4' 11"	5' 0"		5' 3"		4' 9"	4' 9"		4' 11"	4' 5"	4' 6"	4' 6"	4. 9.	4' 10"	1 -	4' 4"	4' 4"	4, 5,	3′ 10″	4' 0"	4' 0"	4, 2,"			3' 9"		3' 10"	BRACES	NO	
	7' 5"	٦	8' 5"	٦	8 5	7' 3"	٦		1 1	G, 5,	7' 6"	7' 7"	7' 8"	7' 8"		7' 4"		7' 8"		6'1"	6' 2"	1 ~	6' 8"	5, 2,	6' 0"	6' 0"	6' 8"	GROUP A	(1) 1X4 "L"	
	7' 5"	8' 7"	8' 5"	9' 1"	9' 1"	7' 3"	8' 5"		8, 8,	6' 5"	7' 6"	7' 7"	8, 3,	8, 3,	1	7' 4"	٠,		۱ -	6' 1"	6' 2"	- ا	7' 2"	5, 5,	6' 0"	6' 0"	6' 10"	GROUP B	" BRACE *	
			10′0″	10' 0"	10' 0"	9' 7"	10' 0"	10' 0"	10' 0"		9' 1"	9' 1"	9' 1"	9' 1"	8' 4"	9' 1"	9' 1"	9' 1"	6' 11"	7' 11"	7' 11"	7' 11"	7' 11"	6' 9"	7' 11"	7' 11"	7' 11"	GROUP A	(1) 2X4 "L"	, s
DI MMAS	· I		10' 6"		10' 9"	1 1	10' 0"	10' 0"	10' 3"	8' 6"				9' 9"		9' 1"	9' 1"	9' 4"	6' 11"	8′0″	"	8' 6"	8' 6"	6, 9,	7' 11"	7' 11"	8' 1"	GROUP B	" BRACE *	
ران بر	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	10' 10"					10' 10"			1 1	"	9' 5"	ي. ه(1	9.	9′ 1″	9 5	٠,	9' 5"	GROUP A	(2) 2X4 "L"	
	-1	- 1	~I	12' 10"	12′10″	11' 11"	11' 11"		12' 3"	11' 1"	11' 4"	11' 4"	11' 8"	11' 8"	10' 10"	10' 10"	10' 10"	11' 1"	9' 4"	9' 11"		10' 2"	10' 2"	- 1	9. 5.	ŀ	9' 8"	GROUP B	BRACE **	
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REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH

SOUTHERN PINE #1 #2	П 6
DOUGLAS FIR-LARCH	GROUP B: HEM-FIR #1 & BTR

DOUGLAS FIR-LARCH
#3
STUD

SOUTHERN PINE #3

STANDARD

STANDARD STUD SPRUCE-PINE-FIR
#1 / #2 STANDARD
#3 STUD

#3

STANDARD GUTS BRACING GROUP SPECIES AND GRADES:

GROUP A:

GABLE TRUSS DETAIL NOTES:

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD). GABLE END SUPPORTS LOAD FROM 4' 0" LIVE LOAD DEFLECTION CRITERIA IS L/240. OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.

* IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

GABLE VERTICAL PLATE SIZES	MEMBER LENGTH.
ATE SIZES	

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB

	·			
	LESS THAN 4' 0"	1X4	0R	1X4 OR 2X3
	GREATER THAN 4' O", BUT		244	
_	LESS THAN 11' 6"		24	
	GREATER THAN 11' 6"	N	2.5X4	
_	- REFER TO COMMON TRUSS DESIGN FOR	DES	SIGN	FOR
	PEAK, SPLICE, AND HEEL PLATES.	PLAT	S	

BRACHOL REFER TO BESS! REQUIRE EXTREME CARE IN FABRICATING, MANDLING, SHEPPING, INSTALLING AND BRACHOS. REFER TO BESS! 1-03 CONLIDING COMPONENT SAFETY INCOMPANION, PUBLISHED BY TPI (TRUSS CALE INSTITUTE, 593 DISMORRO DR., SUITE 200, MADISON, VI. 53719) AND VICTOR COUNCIL RESERVED. AS DO ENTERPRISE LY, MADISON, VI. 53719) FOR SAFETY PRACTICES PRIDR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TIPD CHORD SHALL HAVE A PROPERTY ATTACHED RIGID CEILING. ***********************************	BRACHONG. REFER TO BESSI INCIDENCE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACHONG. REFER TO BESSI INCID COMPONENT SAFTEY HATBOHANDLING, SHIPPING, INSTALLING AND PARTE INSTITUTE. 3803 D'DIMOFRIO DAS, SUITE 200. MADISON, VI. 53779 AND VICA AVOIDD RIVES CHINICE. TO BE SILVER TO BESSI INDICATED. TO PARTICIPE PRODUCTS AND ENTERPRISE IN MISSING VISSATIO, TO PERFORMING TO PERFORMING THESE FOR ANY DEVICATION SHALL HAVE A PROPERTY PRACTICES PRODUCTS. AND BRITCH CHARD SHALL HAVE A PROPERTY PARTICIPED PRODUCTS, INC., SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BRACKING OF THE TRUSS IN COMPONEN VITH APPLICABLE PROPUSIONS OF MISSING AND THE APPINE COMMECTUR PLATES ARE HADE OF PARTISON OF MISSISSE DESIGN COMPONENS WITH APPLICABLE PROPUSIONS OF MISSISSED SHALL HAVE AND MISSISSED AND THE APPINE COMMECTUR PLATES ARE HADE OF PARTISON SHALL HAVE AND THE APPINE DESIGN. AND THE APPINE COMMECTUR PLATES ARE HADE OF PARTISON SHALL HAVE BEEN AND THE APPINE DESIGN SHALL HAV
	No. S.

MAX. SPACING

24.0"

MAX. TOT. LD.

60 PSF

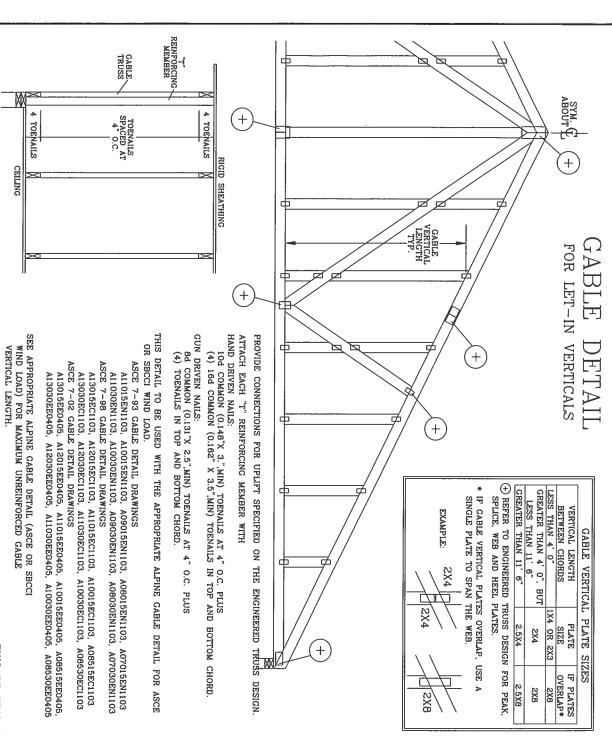
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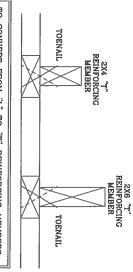
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-ENG

ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

ALPINE





VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE SBCCI WIND LOAD. TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

Ö					_																	_
EXAMPLE:	30 FT	70 MPH	15 FT	70 MPH	30	80 MPH	15 FT	80 MPH	30 FT	90 MPH	15 FT	90 MPH	30	100	15	100	30	110	15	110	DUNA	dNIM
i	7	MPH	FT	MPH	FT	MPH	F	MPH	FT	MPH	FŢ	MPH	FT	MPH	F	MPH	FT	MPH	FT	MPH	MRH	SPEED
	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4		"T" REINF
	10 %	10 %	0%	0 %	20 %	20 %	10 %	10 %	30 %	10 %	20 %	20 %	40 %	10 %	30 %	10 %	50 %	10 %	40 %	10 %	SBCCI	
	30 %	20 %	20 %	20 %	40 %	10 %	30 %	20 %	50 %	10 %	40 %	10 %	40 %	10 %	50 %	10 %	50 %	2 01	50 %	10 %	ASCE	

MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4 ASCE WIND SPEED = 100 MPH

"T" BRACE INCREASE (FROM ABOVE) = 10% = (1) 2X4 "L" BRACE LENGTH = 6' 7"

1.10

MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH 1.10 \times 6' 7" = 7' 3"

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

DATE REF DRWG 04/14/05 DLJ/KAR LET-IN VERT

MAX TOT. DUR. FAC. LD. ANY 60 PSF GBLLETIN0405

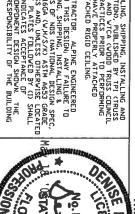
MAX SPACING

24.0

WEMPOREMANTHM FURNISH CODY OF THIS DESIGN TO INSTALLATION FROM THIS DESIGN, AN PINE ENGINEERED PRODUCTS, INC. SHALL AND TER RESENDISIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILLRE TO BUILD THE TRUSS IN COMPORMANCE WITH TPI) OR FARRICATING, HANDLING, SHEPPIN, INSTALLING SPEC, BY AFFENA, AND TPI, ALPINE CONNECTOR PLATES ARE HAD OF 2078/1604 CM/H.S.V. ASTH A653 GRADE 40.60 (V.K.H.S.) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE COGNED ON THIS DESIGN, POSITION PER DRAWFINGS 160A-27. ANY INSPECTION OF PLATES TO LEACH FACE OF TRUSS AND, UNLESS OTHERWISE COGNED ON THIS DESIGN, POSITION PER DRAWFINGS 160A-27. ANY INSPECTION OF PLATES FOLLOWED BY ON SHALL BE PER ANNEX AS OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWFING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELLY FIRM THE TRUSS COMPONENT DESIGN SHOWN. THE WAVARHINGW TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BESI 1—30 BBILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI CYRISS PLATE INSTITUTE, 593 DYNOFRID DR. SUITE 200, MADISON, VI. 537199 AND VICA CYCDID TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, VI. 537199 FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHARD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING. COMPONENT FOR ANY BUILDING IS RESPONSIBILITY SHOWN. THE THE BUILDING E RUTTE CORIOR CENSA

ALPINE ENGINEERED PRODUCTS, INC. POMPANO BEACH, FLORIDA

ALPINE



TOP CHORD CHORD WEBS 2X4 2X4 2X4 ### 0'0'8 9898 BETTER BETTER BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS: 130 MPH WIND, 30' MEAN HGT, ASCE 7-02, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF CLOSED BLDG,

110 MPH WIND, 30' MEAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN WIND TC DL=5 PSF, WIND BC DL=5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-98, BLDG, LOCATED ANYWHERE IN ROOF, CAT II, WIND TC DL=5 PSF CLOSED EXP. C.

(4) 6d BOX (0.099"X 2.", MIN) NAILS.

JOINT

SPANS

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FACE) MAY BE USED IN LIEU OF TRULOX PLATES, ATTACH WITH (8) PER GUSSET. 6d BOX (0.099"X 2.",MIN) NAILS

(4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC

円 Ū a ₩ Þ 4X6 1.5X3 5X4 4X6 2X4 80 OR 3X6 TRULOX AT 4' ROTATED VERTICALLY 2.5X4 1.5X4 5X5 5X6 34 1.5X4 2.5X4 5X5 5X6 38 1.5X4 3X5 5X6 52 00,

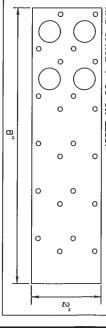
12 12 * A		EITHER PLATE LOCATION IS ACCEPTABLE	FACE (E,*) PLATES MAY S AS LONG AS BOTH FACE A A A A A A A A A A A A A A A A A A
*	B TYP: B THE	OPTIONAL SPLICE D B	FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE A E E E E E E E E E E E E
VC-TYP.		D-SPLICE B	#2 OR BETTER A A E B E B E B E B E B A B E B B B B

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

C	່ດ				
	10' TO 14'	7'9" TO 10'	0' TO 7'9"	WEB LENGTH	
	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135"X 3.5".MIN) NAILS AT 4" OC.	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113"X 2.5",MIN) NAILS AT 4" OC.	NO BRACING	REQUIRED BRACING	WEB BRACING CHART

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FAND SPACE 4' OC OR LESS. TRUSS FACE



SIHT DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

PIGGYBACK

ALPINE ENGINEERED PRODUCTS, INC. POMPANO BEACH, FLORIDA ALPINE

***WAVARNINGSM** TRUSSES REDUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BCS1 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'INDURAID DR., SUITE 200, MADISON, VI. 53719) AND VICA (VODID TRUSS COLUNCIL DE AMERICA, 6300 ENTERPRISE LN, MADISON, VI. 53719) FIRE SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNLESS D'HERVISE NOICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANIELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANIELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED

*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE

WRIPDRIANTAM FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALLE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFIDENANCE WITH 7P1, OR FABRICATING, INAULING, SHIPPING, INSTALLING & BRACKING OF TRUSSES. DESIGN CONFIDENCY WITH APPLICABLE PROVISIONS OF INDS (NATIONAL DESIGN SPEC, BY AFEND AND TP1. ALPINE CONNECTOR PLATES ARE MADE OF 2018/1664 CWH/S/X) ASTH A653 GRADE 40/66 (WK/M-S) GALV. STEEL APPLY PLATES IT EACH FACE OF TRUSS AND, UNLESS OTHERVISE COGNED ON THIS DESIGN, POSITION PER DRAWNING 1604-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF TP1 1-2002 SEC. 3. A SEA LON THIS DRAWNING INDICATES FOLLOWED BY (I) SHALL BROTESSIONAL ENGINEERING GESEONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN THE SUITABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TP1 I SEC. 2.



-480					
47 PSF AT 1.15 DUR. FAC.	1.60 DOR. FAC.	PSF	1.33 DUR. FAC.	PSF	MAX LOADING
		-ENG	DRWG	DATE	REF

DLJ/KAR PIGBACKB0405 04/14/05

24.0"

ASCE 7-02: 110 MPH WIND SPEED, 30, MEAN HEIGHT, ENCLOSED, II 1.00, EXPOSURE \Box

BRACING GROUP SPECIES AND GRADES:

DOUGLAS FIR-LARCH #1 / #2 STANDARD #3 STUD

SOUTHERN PINE

STANDARD

STUD STANDARD

SPRUCE-PINE-FIR

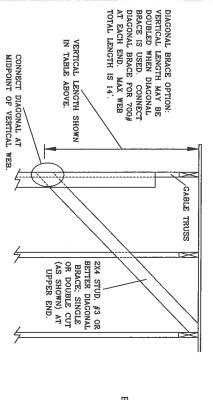
GROUP

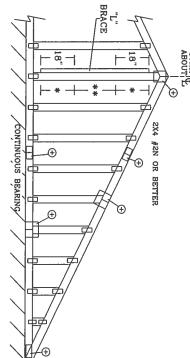
Α

#3 #2

STANDARD

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]	M	A	X	-	(J.E	\]	3.		E		V	E	R	Γ.	Ί	С	A	L		L	Ε	N	IC	ז', ג	ГΗ		
		1	2	,,		Ο	. (Ξ.			1	6	,,		0	. (Э.			2	4	,,		О	. (C	•	SPACING	GABI	
		<u> </u>	j j	<u>ر</u> ال)	TIT	되 디	טרר	U J D			1	<u>ر</u>) j	TIT	二 丁	CLL	Ω J I		L F	j	<u>V.</u>)	TIT	ー 디 丁	ひてっ	טוו	SPECIES	2X4 GABLE VERTICAL	
	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	Γ	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD		#2	#1	STANDARD	STUD	#3	, #1 / #2	GRADE	BRACE	
	4' 7"	4' 9"	4. 9"	4' 11"	5, 1,	4, 6,	4,	1		4.		4. 4.	4. 6.	1 1	Ł	4' 1"	4, 1,"	4. 2.	သ <u>ု</u> 8		3, 9,	3' 11"	4, 0,	3' 7"	1 1	3' 7"	3' 8"	BRACES	NO	
	6' 9"	7' 9"	7' 11"	8,	8,		7' 8"		8, 0,	5' 10"		6' 10"	7' 3"	7' 3"	1	1	6, 8,		4' 9"	5. 6.	5' 7"	6' 4"			5' 5"	5' 5"	6' 4"	GROUP A	(1) 1X4 "L"	
	6′9″	7' 9"	7' 11"	8' 7"	8' 7"	6' 7"	7' 8"	1	8, 2,	1	6′ 9″	6' 10"	- ا	7' 9"	5, 8,		6' 8"	1	4' 9"	5 [']	5' 7"		6' 10"	4' 8"	5' 5"	5, 2,	6' 6"	GROUP B	." BRACE *	
	8' 10"	9. 5.	9' 5"	١ ٠	9, 5,"	8,	9 _.		9. 5.	7' 8"	ľ	8' 7"	8' 7"	8' 7"	1	8' 7"	-	8' 7"	ත _,	7' 3"	7' 4"	7' 6"	7' 6"	6' 1"	7' 1"	7' 2"	7' 6"	GROUP A	(1) 2X4 "L"	
JI MWAS	8' 10"	9' 11"	9' 11"	10' 2"	10' 2"	8' 8"	9' 5"	9' 5"	9' 8"	7' 8"	8' 11"	9' 0"	9' 3"	9' 3"	1 -	8' 7"	8' 7"	8' 10"	6' 3"	7' 3"	7' 4"		8' 1"	6' 1"	7' 1"	7' 2"	7' 8"	GROUP B	" BRACE *	
<u>ال</u>	11' 3"	11' 3"	11' 3"	11' 3"	11' 3"	11' 3"	11' 3"	11' 3"	11′3″	10' 3"	10' 3"	10' 3"		10' 3"		10' 3"	10' 3"	10' 3"	8' 5"	8'11"	8' 11"		8' 11"	8' 3"	8' 11"	8' 11"	8' 11"	GROUP A	(2) 2X4 "L"	
	11' 7"	11' 10"	11' 10"	12' 1"	12' 1"		11′ 3″	11' 3"	11' 7"	10' 4"	10' 9"	10' 9"		11' 0"		10′ 3″		10' 6"	8' 5"	9' 5"	9' 5"	9' 7"	9' 7"	- ا	8' 11"	8' 11"	9, 2,	GROUP B	BRACE **	
	13' 10"	14′0″	14' 0"	14' 0"	14' 0"	13' 6"	14' 0"	14' 0"	14'0"	11'11"	13′ 5″	13' 5"	13′ 5″	13′ 5″	11' 8"	13' 5"	13′ 5″	13' 5"	9′ 9″	11' 4"	11'5"	11' 9"	11' 9"	9' 6"	11' 1"	11' 2"	11' 9"	GROUP A	(1) 2X6 "L"	
									14' 0"			14' 0"	14' 0"	14' 0"	11' 8"	13' 5"	13' 5"	13' 10"	9' 9"		11' 5"				11' 1"		12' 1"	GROUP B	BRACE *	
	- 1	- 1	- 1	14' 0"			- 1		-1			- 1						14' 0"	13′ 3″	- 1	٦,	- 1		12' 11"	14' 0"		14′0″	GROUP A	(2) 2X6 "L"	
								14' 0"				14' 0"					14' 0"	· -	14′0″	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	A GROUP B	BRACE **	





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH

GABLE
TRUSS
DETAIL
NOTES:

SOUTHERN PINE

DOUGLAS FIR-LARCH

HEM-FIR #1 & BTR #1 GROUP

ᄧ

PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD). LIVE LOAD DEFLECTION CRITERIA IS L/240.

GABLE END SUPPORTS LOAD FROM 4' 0"
OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.

* IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB

MEMBER LENGTH.

 PLATES.	PEAK, SPLICE, AND HEEL PLATES.	
DESIGN FOR	+ REFER TO COMMON TRUSS DESIGN FOR	4
2.5X4	GREATER THAN 11' 6"	_
4443	LESS THAN 11' 6"	
344	GREATER THAN 4' O". BUT	
1X4 OR 2X3	LESS THAN 4' 0"	
NO SPLICE	VERTICAL LENGTH	
TE SIZES	GABLE VERTICAL PLATE SIZES	

ALPINE ENGINEERED PRODUCTS, INC. POMPANO BEACH, FLORIDA

ALPINE

A RIVER						REF	ASCE7-02-GAB11030
CENCO						DATE	DATE 04/14/05
0					20200	DRWG	DRWG A11030EE0405
1524623						-ENG	
STATE OF	MAX.	MAX. TOT. LD. 60 PSF	LD.	60	PSF		
OR OF LE							
SIGNAL ENGINE	MAX.	MAX. SPACING 24.0"	ING	24	.0"		

Project Name:

with the Florida Energy Code.

DATE:

OWNER/AGENT:

Address: City, State:

HUNTER RESIDENCE

, FL

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Builder:

Permit Number:

Permitting Office: Counsia

Owner: Climate Zone:	MIKE HUNTER North	÷	Jurisdiction Number: 2	ZIOCU
a. U-factor:	multi-family if multi-family boms see? r area (ft²) area: (Label reqd. by 13-104. Desc uble DEFAULT) 7a. (Dble l t DEFAULT) 7b. dge Insulation	ription Area	12. Cooling systems a. Central Unit b. N/A c. N/A 13. Heating systems a. Electric Heat Pump b. N/A c. N/A 14. Hot water systems a. Electric Resistance b. N/A c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	Cap: 42.0 kBtu/hr SEER: 13.00
Glas	s/Floor Area: 0.08	Total as-built p Total base p	oints: 27745 oints: 32994 PASS	
this calculation are i Code. PREPARED BY DATE:	the plans and specification compliance with the Flance with th	orida Energy	Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908	GREAT CREAT

Florida Statutes.

DATE:

BUILDING OFFICIAL:

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

BASE				AS	-BU	ILT				
GLASS TYPES .18 X Conditioned X BSPM = Floor Area	: Points	Type/SC	O ^r	verhanç t Len	-	Area X	SPM	ΧS	SOF	= Points
.18 2240.0 20.04	8080.1	Double, Clear	NE	0.0	0.0	16.0	29.56	5	1.00	472.9
		Double, Clear	NE	0.0	0.0	12.0	29.56	5	1.00	354.7
		Double, Clear	NE	0.0	0.0	45.0	29.56	;	1.00	1330.0
		Double, Clear	NE	0.0	0.0	15.0	29.56	6	1.00	443.3
		Double, Clear	SE	0.0	0.0	16.0	42.75	j	1.00	684.0
		Double, Clear	SE	0.0	0.0	30.0	42.75	5	1.00	1282.6
		Double, Clear	SW	0.0	0.0	9.0	40.16	;	1.00	361.4
		Double, Clear	SW	0.0	0.0	15.0	40.16	;	1.00	602.4
		Double, Clear	NW	0.0	0.0	30.0	25.97	,	1.00	779.2
		As-Built Total:	8			188.0				6310.5
WALL TYPES Area X BSPI	M = Points	Туре	500	R	-Value	e Area	Х	SPM	=	Points
Adjacent 0.0 0.00 Exterior 1836.0 1.70		Frame, Wood, Exterior			13.0	1836.0		1.50		2754.0
Base Total: 1836.0	3121.2	As-Built Total:				1836.0				2754.0
DOOR TYPES Area X BSP!	VI = Points	Туре			÷	Area	X	SPM	=	Points
Adjacent 0.0 0.00	0.0	Exterior Wood				60.0		6.10		366.0
Exterior 140.0 6.10	854.0	Exterior Wood				80.0	(6.10		488.0
Base Total: 140.0	854.0	As-Built Total:				140.0				854.0
CEILING TYPES Area X BSPI	M = Points	Туре		R-Val	ue ,	Area X S	SPM >	(SCI	M =	Points
Under Attic 2240.0 1.73	3875.2	Under Attic			30.0	2800.0	1.73 X	1.00		4844.0
Base Total: 2240.0	3875.2	As-Built Total:				2800.0				4844.0
FLOOR TYPES Area X BSPN	/I = Points	Туре		R	-Value	e Area	X	SPM	=	Points
Slab 204.0(p) -37.0 Raised 0.0 0.00	-7548.0 0.0	Slab-On-Grade Edge Inst	ulation		0.0	204.0(p	-4	1.20		-8404.8
Base Total:	-7548.0	As-Built Total:				204.0	1			-8404.8
INFILTRATION Area X BSPN	/I = Points					Area	X	SPM	=	Points
2240.0 10.21	22870.4					2240.0) 1	0.21		22870.4

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL,	PERMIT #:	i
	 	_

BASE	AS-BUILT
Summer Base Points: 31252.9	Summer As-Built Points: 29228.1
Total Summer X System = Cooling Points Multiplier Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)
31252.9 0.4266 13332	(sys 1: Central Unit 42000 btuh ,SEER/EFF(13.0) Ducts:Con(S),Con(R),Gar(AH),R6.0(INS) 29228

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

BASE				AS-	BUI	LT	i			
GLASS TYPES		-								
.18 X Conditioned X BWPM = Poi Floor Area	nts	7	Ove rnt	rhang Len	Hgt	Area X	WP	w x	WOF	= Points
.18 2240.0 12.74 51	36.8	Double, Clear	NE	0.0	0.0	16.0	23.5	7	1.00	377.1
		Double, Clear	NE	0.0	0.0	12.0	23.5	7	1.00	282.8
		Double, Clear	NE	0.0	0.0	45.0	23.5	7	1.00	1060.6
		Double, Clear	NE	0.0	0.0	15.0	23.5	7	1.00	353.5
		Double, Clear	SE	0.0	0.0	16.0	14.7	1	1.00	235.3
			SE	0.0	0.0	30.0	14.7	1	1.00	441.2
		1 30 S	w	0.0	0.0	9.0	16.7	4	1.00	150.6
			w	0.0	0.0	15.0	16.7		1.00	251.0
		to Cody	ĮW	0.0	0.0	30.0	24.3		1.00	728.9
		tx vav			55			-		
		As-Built Total:				188.0				3881.0
WALL TYPES Area X BWPM = I	oints	Туре		R-	Value	Area	Х	WPM	=	Points
Adjacent 0.0 0.00 Exterior 1836.0 3.70	0.0 6793.2	Frame, Wood, Exterior			13.0	1836.0		3.40		6242.4
Base Total: 1836.0	6793.2	As-Built Total:				1836.0				6242.4
DOOR TYPES Area X BWPM = F	Points	Туре				Area	X	WPM	=	Points
Adjacent 0.0 0.00	0.0	Exterior Wood				60.0		2.30		738.0
	1722.0	Exterior Wood				80.0		2.30		984.0
·	1722.0	As-Built Total:				140.0				1722.0
Dase rotal.	1722.0	As-Built Potal.				140.0				1722.0
CEILING TYPES Area X BWPM = F	Points	Туре	R	-Value	Ar	ea X W	PM)	(WC	M =	Points
Under Attic 2240.0 2.05	4592.0	Under Attic			30.0	2800.0 2	2.05 X	1.00		5740.0
Base Total: 2240.0	4592.0	As-Built Total:		, '		2800.0				5740.0
FLOOR TYPES Area X BWPM = F	oints	Туре		R-	Value	Area	X	ΝPΜ	=	Points
Slab 204.0(p) 8.9 Raised 0.0 0.00	1815.6 0.0	Slab-On-Grade Edge Insulation			0.0	204.0(p	1	8.80		3835.2
Base Total:	1815.6	As-Built Total:				204.0				3835.2
INFILTRATION Area X BWPM = F	Points					Area	ΧV	NPM	=	Points
2240.0 -0.59 -	1321.6					2240.0) -	-0.59		-1321.6

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

BASE			AS-BUILT				
Winter Base	Points:	18738.0	Winter As-Built Points:	20099.0			
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)	= Heating Points			
18738.0	0.6274	11756.2	(sys 1: Electric Heat Pump 42000 btuh ,EFF(7.0) Ducts:Con(S),Con(R),G 20099.0 1.000 (1.000 x 1.169 x 1.00) 0.487 1.000 20099.0 1.00 1.169 0.487 1.000	ar(AH),R6.0 11445.8 11445.8			

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

	BASE	AS-BUILT								
WATER HEATIN Number of X Bedrooms		= Total	Tank Volume	EF	Number of Bedrooms	Х	Tank X Ratio	Multiplier	X Credit Multiplie	
3	2635.00	7905.0	40.0	0.97	3		1.00	2499.18	1.00	7497.5
- W _ A - A - L - L			As-Built To	otal:						7497.5

	CODE COMPLIANCE STATUS												
BASE			AS-BUILT										
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
13332		11756		7905		32994	8802		11446		7498		27745

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall;	
		foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility	
		penetrations; between wall panels & top/bottom plates; between walls and floor.	
27		EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends	
= .		from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members.	
		EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed	-
		to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases,	i
Ų.		soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate;	
		attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is	
		installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a	=
		sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from	-
		conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA,	
		have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir	
		breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools	
		must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically	
		attached, sealed, insulated, and installed in accordance with the criteria of Section 610.	
		Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides.	
		Common ceiling & floors R-11.	



PROJECT NAME: HUNTER PROJECT NUMBER: PF06-078

WIND LOAD AND STRUCTURAL CALCULATIONS FOR

MIKE HUNTER New Residence

GARY GILL, PE GTC DESIGN GROUP, LLC P.O. BOX 187 LIVE OAK, FL 32064 386-362-3678 386-362-6133 (FAX) AUTH. # 9461

Project name: MIKE HUNTER (HOUSE)

Project:

PF06-078 MIKE HUNT

Client Calculations:

Gary Gill, PE

Date:

4/25/2006

Design Basis

Design Loads

Wind Load 110

Floor Live Load

30 psf Sleep Areas = All Others = 40 psf

10 psf Floor Dead Load 10 psf Wall Dead Load Roof Live Load 20 psf 10 psf Roof Dead Load

Load Combinations

DL + LL(floor) + LL (roof)

DL + LL(floor) +WL

DL + WL Wind load

Exposure B

25		0		ii :
1.55	490	 		

Building Information

Shape

Rectangle

Length

86 ft

Width

48 ft

Type

1 storey sog

References

2004 Florida Building Code

ASCE 7-98 Minimum Design Loads for Buildings and Other Structures

AITC Timber Construction Manual

Wind Load Design per ASCE 7-98

Description: MIKE HUNTER (HOUSE)

Analysis by: Gary Gill

User Input	User Input Data						
Structure Type	Building						
Basic Wind Speed (V)	110	mph					
Structural Category	11						
Exposure	В	- 37 4					
Struc Nat Frequency (n1)	.: 1 ···	Hz					
Slope of Roof (Theta)	30.256	Deg					
Type of Roof	Gabled						
Kd (Directonality Factor)	0.85						
Eave Height (Eht)	9.00	ft					
Ridge Height (RHt)	21.00	ft					
Mean Roof Height (Ht)	15.00	ft					
Width Perp. To Wind Dir (B)	86.00	ft					
Width Paral. To Wind Dir (L)	48.00	ft -					
Damping Ratio (beta)	0.02	=					

Red values should be changed only through "Main Menu"

Calculated Parameter	S
Type of Structure	
Height/Least Horizontal Dim	0.31
Flexible Structure	No

Calculated Parameters							
Importance Factor	1						
Hurricane Prone	Region (V>100	mph)					
Table C	6-4 Values						
Alpha = 7.000							
zg =	1200.000						
At =	0.143						
D.							
Bt =	0.840						
Am =	0.840						
Am =	0.250						
Am = Bm =	0.250 0.450	ft					
Am = Bm = Cc =	0.250 0.450 0.300	ft					

Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85
	Gust Factor Category II: Rigid Structures - Complete Analysis	S TO A STATE OF
Zm	Zmin	30.00 f
lzm	Cc * (33/z)^0.167	0.3048
Lzm	I*(zm/33)^Epsilon	309.99 f
Q	(1/(1+0.63*((Min(B,L)+Ht)/Lzm)^0.63))^0.5	0.9013
Gust2	0.925*((1+1.7*lzm*3.4*Q)/(1+1.7*3.4*lzm))	0.8668
	Gust Factor Summary	
G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85

Wind Load Design per ASCE 7-98

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev	Kz	Kzt	qz	Pressure (lb/ft^2)		
				Windward Wall*		
ft			lb/ft^2	+GCpi	-GCpi	
21	0.63	1.00	16.66	8.60	14.05	
20	0.62	1.00	16.43	8.45	13.90	
15	0.57	1.00	15.13	7.57	13.01	

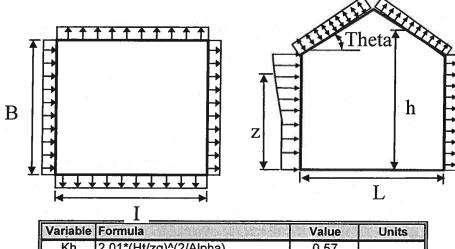
Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

Condition	Gcpi			
	Max +	Max -		
Open Buildings	0.00	0.00		
Partially Enclosed Buildings	0.55	-0.55		
Enclosed Buildings	0.18	-0.18		
Enclosed Buildings	0.18	-0.18		

Wind Load Design per ASCE 7-98

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(Ht/zg)^(2/Alpha)	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	11
Qh	.00256*(V)^2*I*Kh*Kht*Kd	15.13	psf
Khcc	Comp & Clad: Table 6-5 Case 2	0.70	
Qhcc	.00256*V^2*I*Khcc*Kht*Kd	18.45	psf

Wall Pressure Coefficients, Cp	建 建二二二
Surface	Ср
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.8

	Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)		-
Reduction Factor		1.00

Calculations for Wind Normal to 86 ft Face	Ср	Pressur	e (psf)
Additional Runs may be req'd for other wind directions		+GCpi	-GCpi
Leeward Walls (Wind Dir Normal to 86 ft wall)	-0.50	-9.15	-3.71
Side Walls	-0.70	-11.73	-6.28
Roof - Wind Normal to Ridge (Theta>=10) - for	r Wind Norr	nal to 86 ft fa	ce
Windward - Max Negative	-0.19	-5.20	0.25
Windward - Max Positive	0.28	0.88	6.33
Leeward Normal to Ridge	-0.60	-10.44	-4.99
Overhang Top (Windward)	-0.19	-2.47	-2.47
Overhang Top (Leeward)	-0.60	-7.72	-7.72
Overhang Bottom (Applicable on Windward only)	0.80	10.29	10.29
Roof - Wind Parallel to Ridge (All Theta) - for	Wind Norm	al to 86 ft fac	e
Dist from Windward Edge: 0 ft to 7.5 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 7.5 ft to 15 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 15 ft to 30 ft	-0.50	-9.15	-3.71
Dist from Windward Edge: > 30 ft	-0.30	-6.58	-1.13

^{*} Horizontal distance from windward edge

Wind Load Design per ASCE 7-98

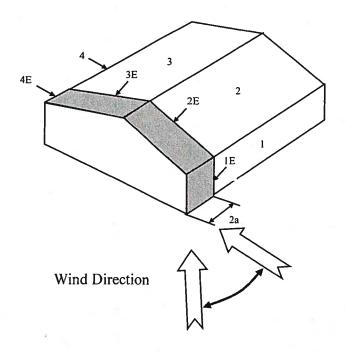
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh=	2.01*(Ht/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	15.13

	Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)	
1	0.56	0.18	-0.18	15.13	5.75	11.20	
2	0.21	0.18	-0.18	15.13	0.45	5.90	
3	-0.43	0.18	-0.18	15.13	-9.23	-3.78	
4	-0.37	0.18	-0.18	15.13	-8.32	-2.88	
5	0.00	0.18	-0.18	15.13	-2.72	2.72	
6	0.00	0.18	-0.18	15.13	-2.72	2.72	
1E	0.69	0.18	-0.18	15.13	7.72	13.16	
2E	0.27	0.18	-0.18	15.13	1.36	6.81	
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30	
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54	
5E	0.00	0.18	-0.18	15.13	-2.72	2.72	
6E	0.00	0.18	-0.18	15.13	-2.72	2.72	

^{*} p = qh * (GCpf - GCpi)



Wind Load Design per ASCE 7-98

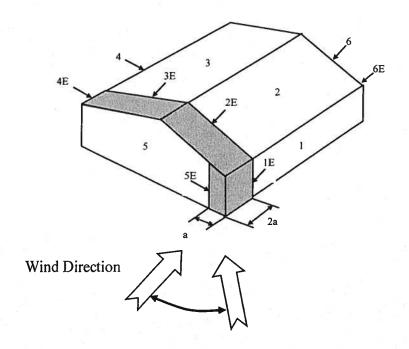
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	= =	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*impFac*Kh*Kht*Kd	=	15.13

	Case B						
Surface	GCpf	+GCpi	-GCpi	qh	Min P	Max P	
			-1	(psf)	(psf)	(psf)	
1	-0.45	0.18	-0.18	15.13	-9.53	-4.09	
2	-0.69	0.18	-0.18	15.13	-13.16	-7.72	
3	-0.37	0.18	-0.18	15.13	-8.32	-2.88	
4	-0.45	0.18	-0.18	15.13	-9.53	-4.09	
5	0.40	0.18	-0.18	15.13	3.33	8.78	
6	-0.29	0.18	-0.18	15.13	-7.11	-1.66	
1E	-0.48	0.18	-0.18	15.13	-9.99	-4.54	
2E	-1.07	0.18	-0.18	15.13	-18.92	-13.47	
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30	
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54	
5E	0.61	0.18	-0.18	15.13	6.51	11.95	
6E	-0.43	0.18	-0.18	15.13	-9.23	-3.78	

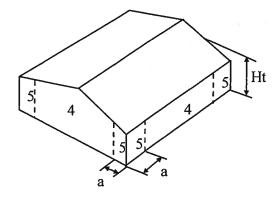
^{*} p = qh * (GCpf - GCpi)

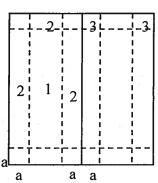


Wind Load Design per ASCE 7-98

Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft





Gabled Roof 10 < Theta <= 45

a = 4.8 ==> 4.80 ft

Component	Width	Span	Area	Zone	GCp Wind Press (II		ss (lb/ft^2)	
	(ft)	(ft)	(ft^2)		Max	Min	Max	Min
ROOF	10	1	10.00	1	0.90	-1.00	19.92	-21.77
Walls	10	1	10.00	4	1.00	-1.10	21.77	-23.61
roof edge	10	1	10.00	2	0.90	-1.20	19.92	-25.46
Wall edge	10	1	10.00	5	1.00	-1.40	21.77	-29.15
Roof overhang	10	1	10.00	2H	0.90	-2.00	16.60	-36.89

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Shearwall Design - N/S Direction

Rigid Diaphragm Analysis

Wind load acting on building

Gen	era	I D	ata

Ochoral Data			
Roof Pitch (x:12)		7 Roof Dia	13.89
N III		Length of	
Vertical Roof height		12.00 Building	86
		Width of	
2nd Floor height	0	Building	48
1st Floor height	9		

Wind Pressure per ASCE 7- Normal to surface Case A

Windward Roof - Surface 2	5.90	psf	Wall -	11.2 psf
			Leeward	
			Wall -	
Leeward Roof - Surface 3	-3.78	psf	Surface 4	-2.88 psf
			Total Wall	14.08 psf

Horizontal loads from wind perpendicular to ridge (N / S)

Roof Pressure (interior)

Windward Roof Horz.(psf)	2.97
Leeward Roof Horz.(psf)	-1.90
Total	4.88
Tributary area (roof)	516.00
Roof shear values	2516.78

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	
Tributary area to each Shearwall	
(sf)	0.00
Wall shear values to each	
shearwall	0.00

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	14.08
Tributary area to each Shearwall	
(sf)	315.00
Wall shear values to each	
shearwall	4435.20

Total shear to top of 2nd floor (lb)	
per wall (actual)	0.00
Total shear to top of 1st floor (lb)	
per wall (actual)	6951.98

e <u> </u>	Shearwall column #	
2nd Floor shearwalls		
Number of shearwall segments in		
each column		
Shearwall #1 length		
Shearwall #2 length		
Shearwall #3 length		
Lateral load on shear wall column		
(lbs)	*	
Percent Full-Height Sheathing		
Shear capacity adjustment	=	
Shearwall rating (plf) w/ 1.4		
Design Shear Capacity		
Stress Ratio		
uplift at shear ends		
shear and uplift between holddown,		
v and u		

1st Floor shearwall (ft)

15th 100h Shearwall (1t)			
Number of shearwall segments in			
each column	1	1	
Full wall length	32	32	
Shearwall #1 length	22	23	
Shearwall #2 length	0	0	
Wall height ratio (h/b)	0.41	0.39	
Rigidities of shearwalls	7.72	8.10	
Lateral load on shearwall column			
(lbs) based on rigidity	3390.92	3561.06	<u>.</u>
Percent Full-Height Sheathing			
Shearwall #1	68.75%	71.88%	
Shear capacity adjustment	0.77	0.69	
Shearwall rating (plf) w/ 1.4			
increase for wind	483	483	
Design Shear Capacity	8182.02	7665.21	
Stress Ratio	0.41	0.46	
uplift at shear ends	1801.55	2019.50	
shear and uplift between holddown,			
v and u	200.17	224.39	

Anchor Bolt Shear Capacity	/ plf	•	
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

Shearwall Design - E/W Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)		7 Roof Dia Length of	13.89
Vertical Roof height		12.00 Building Width of	86
2nd Floor height	0	Building	48
1st Floor height	9	_	

Wind Pressure per ASCE 7- Normal to surface Case B

Windward Wall - Surface 5	8.78	psf
Leeward Wall - Surface 6	-1.66	psf
Total Wali	10	0.44

Horizontal loads from parallel to ridge (N/S)

Roof Pressure (interior)	
Windward Roof Horz.(psf)	8.78
Leeward Roof Horz.(psf)	-1.66
Total	10.44
Tributary area (roof) to each	
shearwall (sf)	144.00
Roof shear values to each	
shearwall	1503.36

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	0
Tributary area to each Shearwall	
(sf)	0.00
Wall shear values to each	
shearwall	0.00

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	10.44
Tributary area to each Shearwall	
(sf)	144.00
Wall shear values to each	
shearwall	1503.36

Total shear to top of 2nd floor (lb)	
per wall (actual)	0.00
Total shear to top of 1st floor (lb)	
per wall (actual)	3006.72

1	Channall and
	Shearwall column #
2nd Floor shearwalls	
Number of shearwall segments in	
each column	
Full wall length	
Shearwall #1 length	
Shearwall #2 length	
Shearwall #3 length	
Lateral load on shear wall column	
(lbs)	
Percent Full-Height Sheathing	
Shear capacity adjustment	
Shearwall rating (plf) w/ 1.4	
Design Shear Capacity	
Stress Ratio	
uplift at shear ends	
shear and uplift between holddown,	
v and u	

1st Floor shearwall (ft)

Number of shearwall segments in			
each column		В	
Full wall length	70	70	
Shearwall #1 length		49	50
Shearwall #2 length	0	0	
Wall height ratio (h/b)		0.18	
Rigidities of shearwalls		17.95	
Lateral load on shearwall column			
(lbs) based on rigidity	1487.51	1519.21	
Percent Full-Height Sheathing			
Shearwall #1	68.57%	70.00%	
Shear capacity adjustment	0.69	0.69	
Shearwall rating (plf) w/ 1.4			
increase for wind	483	483	
Design Shear Capacity	15996.96	16330.23	
Stress Ratio	0.09	0.09	
uplift at shear ends	404.21	404.40	
shear and uplift between holddown,			
v and u	44.91	44.93	

Anchor Bolt Shear Capacity	plf		
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

Project name: MIKE HUNTER (GARAGE)

Project:

PF06-078

Client Calculations:

MIKE HUNT Gary Gill, PE

Date:

4/25/2006

Design Basis

Design Loads

Wind Load 110

Floor Live Load

Sleep Areas = 30 psf
All Others = 40 psf
Floor Dead Load 10 psf
Wall Dead Load 10 psf

Roof Live Load 20 psf Roof Dead Load 10 psf

Load Combinations

DL + LL(floor) + LL (roof)

DL + LL(floor) +WL

DL + WL Wind load

Exposure B

11	KT \$1		····

Building Information

Shape

Rectangle

Length

24 ft

Width

24 ft

Type

1 storey sog

References

2004 Florida Building Code

ASCE 7-98 Minimum Design Loads for Buildings and Other Structures

AITC Timber Construction Manual

Wind Load Design per ASCE 7-98

Description: MIKE HUNTER (GARAGE)

Analysis by: Gary Gill

User Input Data					
Structure Type	Building				
Basic Wind Speed (V)	110	mph			
Structural Category	- 11				
Exposure	В				
Struc Nat Frequency (n1)	1	Hz			
Slope of Roof (Theta)	30.256	Deg			
Type of Roof	Gabled				
Kd (Directonality Factor)	0.85				
Eave Height (Eht)	9.00	ft			
Ridge Height (RHt)	17.17	ft			
Mean Roof Height (Ht)	13.08	ft			
Width Perp. To Wind Dir (B)	24.00	ft			
Width Paral. To Wind Dir (L)	24.00	ft			
Damping Ratio (beta)	0.02				

Red values	should be	changed	only th	rouah	"Main	Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.55
Flexible Structure	No

Calculated Parameters			
Importance Factor	1	A-1-1-10-10-10-10-10-10-10-10-10-10-10-10	
Hurricane Prone Re	gion (V>100 i	nph)	
Table C6-	4 Values		
Alpha =	7.000		
zg =	1200.000		
At =	0.143		
Bt =	0.840		
Am =	0.250		
Bm =	0.450		
Cc =	0.300		
<u> </u>	320.00	ft	
Epsilon =	0.333		
Zmin =	30.00	ft	

	Gust Factor Category I: Rigid Structures - Simplified Method	美国新疆市
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85
	Gust Factor Category II: Rigid Structures - Complete Analysis	S
Zm	Zmin	30.00 ft
lzm	Cc * (33/z)^0.167	0.3048
Lzm	I*(zm/33)^Epsilon	309.99 ft
Q	(1/(1+0.63*((Min(B,L)+Ht)/Lzm)^0.63))^0.5	0.9264
Gust2	0.925*((1+1.7*lzm*3.4*Q)/(1+1.7*3.4*lzm))	0.8815
	Gust Factor Summary	
G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85

Wind Load Design per ASCE 7-98

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev	Kz	Kzt	qz	Pressure	(lb/ft^2)
		1912	Windward W	rd Wall*	
ft				+GCpi	-GCpi
17.167	0.60	1.00	15.73	7.97	13.42
15	0.57	1.00	15.13	7.57	13.01

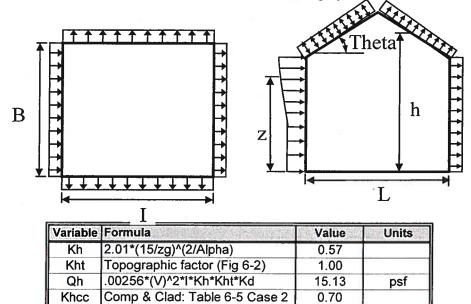
Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

Condition	Gcpi		
	Max +	Max -	
Open Buildings	0.00	0.00	
Partially Enclosed Buildings	0.55	-0.55	
Enclosed Buildings	0.18	-0.18	
Enclosed Buildings	0.18	-0.18	

Wind Load Design per ASCE 7-98

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Wall Pressure Coefficients, Cp	
Surface	Ср
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.8

18.45

psf

.00256*V^2*I*Khcc*Kht*Kd

Roof Pressure Coefficients, Cp	Salat da
Roof Area (sq. ft.)	71-
Reduction Factor	1.00

Calculations for Wind Normal to 24 ft Face	Ср	Pressure	e (psf)
Additional Runs may be req'd for other wind directions		+GCpi	-GCpi
Leeward Walls (Wind Dir Normal to 24 ft wall)	-0.50	-9.15	-3.71
Side Walls	-0.70	-11.73	-6.28
Roof - Wind Normal to Ridge (Theta>=10) - for	or Wind Norr	nal to 24 ft fa	ce
Windward - Max Negative	-0.21	-5.41	0.04
Windward - Max Positive	0.20	-0.09	5.36
Leeward Normal to Ridge	-0.60	-10.44	-4.99
Overhang Top (Windward)	-0.21	-2.68	-2.68
Overhang Top (Leeward)	-0.60	-7.72	-7.72
Overhang Bottom (Applicable on Windward only)	0.80	10.29	10.29
Roof - Wind Parallel to Ridge (All Theta) - fo	r Wind Norm	al to 24 ft fac	е
Dist from Windward Edge: 0 ft to 6.54 ft	-0.94	-14.76	-9.32
Dist from Windward Edge: 6.54 ft to 13.08 ft	-0.88	-14.07	-8.62

^{*} Horizontal distance from windward edge

Qhcc

Wind Load Design per ASCE 7-98

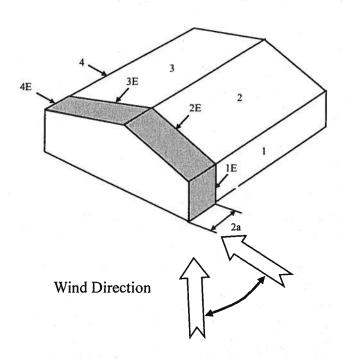
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	= 1	15.13

	Case A							
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)		
1	0.56	0.18	-0.18	15.13	5.75	11.20		
2	0.21	0.18	-0.18	15.13	0.45	5.90		
3	-0.43	0.18	-0.18	15.13	-9.23	-3.78		
4	-0.37	0.18	-0.18	15.13	-8.32	-2.88		
5	0.00	0.18	-0.18	15.13	-2.72	2.72		
6	0.00	0.18	-0.18	15.13	-2.72	2.72		
1E	0.69	0.18	-0.18	15.13	7.72	13.16		
2E	0.27	0.18	-0.18	15.13	1.36	6.81		
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30		
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54		
5E	0.00	0.18	-0.18	15.13	-2.72	2.72		
6E	0.00	0.18	-0.18	15.13	-2.72	2.72		

^{*} p = qh * (GCpf - GCpi)



Wind Load Design per ASCE 7-98

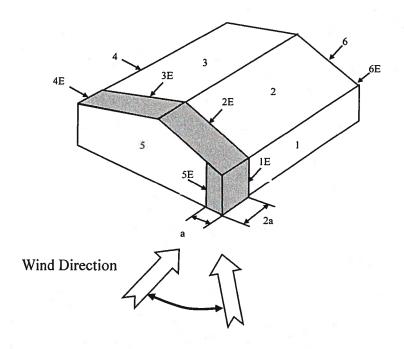
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	:: =	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	15.13

	Case B							
Surface	GCpf	+GCpi	-GCpi	qh	Min P	Max P		
				(psf)	(psf)	(psf)		
11	-0.45	0.18	-0.18	15.13	-9.53	-4.09		
2	-0.69	0.18	-0.18	15.13	-13.16	-7.72		
3	-0.37	0.18	-0.18	15.13	-8.32	-2.88		
4	-0.45	0.18	-0.18	15.13	-9.53	-4.09		
5	0.40	0.18	-0.18	15.13	3.33	8.78		
6	-0.29	0.18	-0.18	15.13	-7.11	-1.66		
1E	-0.48	0.18	-0.18	15.13	-9.99	-4.54		
2E	-1.07	0.18	-0.18	15.13	-18.92	-13.47		
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30		
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54		
5E	0.61	0.18	-0.18	15.13	6.51	11.95		
6E	-0.43	0.18	-0.18	15.13	-9.23	-3.78		

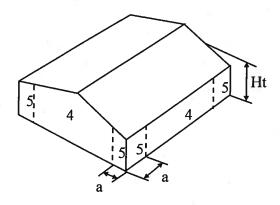
^{*} p = qh * (GCpf - GCpi)

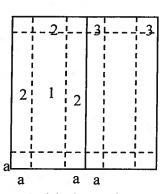


Wind Load Design per ASCE 7-98

Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft





Gabled Roof 10 < Theta <= 45

a = 2.4 ==> 3.00 ft

Component	Width	Span	Area	Area Zone GCp	GCp		ss (lb/ft^2)	
	(ft)	(ft)	(ft^2)	• .	Max	Min	Max	Min
ROOF	10	1	10.00	1	0.90	-1.00	19.92	-21.77
Walls	10	.1	10.00	4	1.00	-1.10	21.77	-23.61
roof edge	10	1 1	10.00	2	0.90	-1.20	19.92	-25.46
Wall edge	10	1	10.00	5	1.00	-1.40	21.77	-29.15
Roof overhang	10	1	10.00	2H	0.90	-2.00	16.60	-36.89

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Shearwall Design - N/S Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)		7 Roof Dia	13.89
		Length of	
Vertical Roof height		8.17 Building	24
		Width of	
2nd Floor height	0	Building	24
1st Floor height	9		

Wind Pressure per ASCE 7- Normal to surface Case A

Windward Roof - Surface 2	5.90	psf	Wall -	11.2 psf
			Leeward	
			Wall -	
Leeward Roof - Surface 3	-3.78	psf	Surface 4	-2.88 psf
		-	Total Wall	14.08 psf

Horizontal loads from wind perpendicular to ridge (N / S)

Roof Pressure (interior)

Windward Roof Horz.(psf)	2.97
Leeward Roof Horz.(psf)	-1.90
Total	4.88
Tributary area (roof)	98.00
Roof shear values	477.99

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	14.08
Tributary area to each Shearwall	
(sf)	0.00
Wall shear values to each	
shearwall	0.00

Wall Pressure - 1st Floor

14.08	Sum. of wind. & lee. (psf)
	Tributary area to each Shearwall
108.00	(sf)
	Wall shear values to each
1520.64	shearwall

Total shear to top of 2nd floor (lb)	
per wall (actual)	0.00
Total shear to top of 1st floor (lb)	10
per wall (actual)	1998.63

**	Shearwall column #
2nd Floor shearwalls	12
Number of shearwall segments in	51
each column	× = × =
Shearwall #1 length	
Shearwall #2 length	n n n
Shearwall #3 length	
Lateral load on shear wall column	
(lbs)	1
Percent Full-Height Sheathing	
Shear capacity adjustment	
Shearwall rating (plf) w/ 1.4	
Design Shear Capacity	
Stress Ratio	
uplift at shear ends	F (1
shear and uplift between holddown,	
v and u	

1st Floor shearwall (ft)

1	1	
24	24	
21	6	
0	0	
0.43	1.50	
7.33	1.27	
	=	
1703.48	295.15	
87.50%	25.00%	
0.87	0.42	
1 0		
483	483	
8824.41	1217.16	
0.19	0.24	
839.15	1054.10	
		_=
93.24	117.12	F
	1 24 21 0 0.43 7.33 1703.48 87.50% 0.87 483 8824.41 0.19 839.15	1 1 24 24 21 6 0 0 0 0.43 1.50 7.33 1.27 1703.48 295.15 87.50% 25.00% 0.87 0.42 483 483 8824.41 1217.16 0.19 0.24 839.15 1054.10

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

Shearwall Design - E/W Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)		7 Roof Dia	13.89
		Length of	
Vertical Roof height		8.17 Building	24
		Width of	
2nd Floor height	0	Building	24
1st Floor height	9	1/	

Wind Pressure per ASCE 7- Normal to surface Case B

Windward Wall - Surface 5 Leeward Wall - Surface 6	8.78	psf
Total Wall	<u>-1.66</u>	psf 0.44

Horizontal loads from parallel to ridge (N/S)

Roof Pressure (interior)	
Windward Roof Horz.(psf)	8.78
Leeward Roof Horz.(psf)	-1.66
Total	10.44
Tributary area (roof) to each	
shearwall (sf)	49.00
Roof shear values to each	
shearwall	511.56

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	0
Tributary area to each Shearwall	
(sf)	0.00
Wall shear values to each	
shearwall	0.00

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	10.44
Tributary area to each Shearwall	
(sf)	108.00
Wall shear values to each	
shearwall	1127.52

Total shear to top of 2nd floor (lb) per wall (actual)	
A STATE OF THE PARTY OF THE PAR	0.00
Total shear to top of 1st floor (lb)	
per wall (actual)	1639.08

	Shearwall column #
2nd Floor shearwalls	
Number of shearwall segments in	
each column	
Full wall length	
Shearwall #1 length	
Shearwall #2 length	
Shearwall #3 length	
Lateral load on shear wall column	
(lbs)	
Percent Full-Height Sheathing	
Shear capacity adjustment Shearwall rating (plf) w/ 1.4	
Design Shear Capacity	4
Stress Ratio	
uplift at shear ends	
shear and uplift between holddown,	
v and u	

1st Floor shearwall (ft)

		ĺ
Α	В	
24	24	
18	24	
0	0	
0.50	0.38	
6.15	8.49	
		_
688.76	950.32	
75.00%	100.00%	
0.83	1	
483	483	
7216.02	11592.00	
0.10	0.08	
414.91	356.37	
46.10	39.60	
	A 24 18 0 0.50 6.15 688.76 75.00% 0.83 7216.02 0.10 414.91	A B 24 24 18 24 0 0 0 0.50 0.38 6.15 8.49 688.76 950.32 75.00% 100.00% 0.83 1 483 483 7216.02 11592.00 0.10 0.08 414.91 356.37

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465