DATE 06/28/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024686

APPLICANT SAMMY KEEN PHONE 386.497.2289

ADDRESS 764 SW RIVERSIDE AVENUE FT. WHITE FL 32038

OWNER MIKE HUNTER PHONE 386.288.3613

ADDRESS 724 SW WESTER DRIVE LAKE CITY FL 32024

CONTRACTOR GUY WILLIAMS PHONE 386.365.2289

LOCATION OF PROPERTY 47-S TO WESTER ROAD,TL TO LAST M/H ON RIGHT BEFORE CURVING TO THE R.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 112000.00

HEATED FLOOR AREA 2240.00 TOTAL AREA 4786.00 HEIGHT 22.60 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 7'12 FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 31-4S-17-08920-000 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES 8.50

		CBC050690		
Culvert Permit No.	Culvert Waiver	Contractor's License Number		Applicant/Owner/Contractor
EXISTING	06-0546-N	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: 1 FOOT ABOVE ROAD. 30 DAYS TO REMOVE M/H AFTER CO ISSUANCE.

Check # or Cash 8788

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power ____ date/app. by | Foundation ____ date/app. by | Monolithic ____ date/app. by

Under slab rough-in plumbing ____ date/app. by | Slab ____ date/app. by | Sheathing/Nailing ____ date/app. by

Framing ____ date/app. by | Rough-in plumbing above slab and below wood floor ____ date/app. by

Electrical rough-in ____ date/app. by | Heat & Air Duct ____ date/app. by | Peri. beam (Lintel) ____ date/app. by

Permanent power ____ date/app. by | C.O. Final ____ date/app. by | Culvert ____ date/app. by

M/H tie downs, blocking, electricity and plumbing ____ date/app. by | Pool ____ date/app. by

Reconnection ____ date/app. by | Pump pole ____ date/app. by | Utility Pole ____ date/app. by

M/H Pole ____ date/app. by | Travel Trailer ____ date/app. by | Re-roof ____ date/app. by

BUILDING PERMIT FEE $ 560.00 CERTIFICATION FEE $ 23.93 SURCHARGE FEE $ 23.93

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 682.86

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

06-206

THIS INSTRUMENT WAS PREPARED BY:
FIRST FEDERAL SAVINGS BANK OF FLORIDA
4705 WEST U.S. HIGHWAY 90
P.O. BOX 2029
LAKE CITY, FLORIDA 32056

PERMIT NO._____ TAX FOLIO NO._____

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF Columbia

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: SEE EXHIBIT "A" ATTACHED HERETO AND MADE A PART HEREOF.

2. General description of improvement: **Construction of Dwelling**

3. Owner information:
   a. Name and address: MICHAEL S. HUNTER and BRENDA L. HUNTER
   724 SW Wester Drive, Lake City, FL 32024

   b. Interest in property: **Fee Simple**

   c. Name and address of fee simple title holder (if other than Owner): **NONE**

4. Contractor (name and address): S L K CONSTRUCTION, INC.
   1999 S Marion Ave., Lake City, FL 32025

5. Surety:
   a. Name and address: _____
   _____

   b. Amount of bond: _____

Inst:2006014427 Date:06/14/2006 Time:16:35
DC,P.DeWitt Cason,Columbia County B:1086 P:2266

6. Lender: **FIRST FEDERAL SAVINGS BANK OF FLORIDA**
   4705 WEST U.S. HIGHWAY 90
   P. O. BOX 2029
   LAKE CITY, FLORIDA 32056

7. Persons within the State of Florida designated by Owner upon whom notices or other document may be served as provided by Section 713.13 (1) (a) 7., Florida Statutes: NONE

8. In addition to himself, Owner designates **PAULA HACKER of FIRST FEDERAL SAVINGS BANK OF FLORIDA, 4705 West U.S. Highway 90 / P. O. Box 2029, Lake City, Florida 32056** to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) (b), Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

Borrower Name

Brenda L Hunter
Co-Borrower Name

The foregoing instrument was acknowledged before me this 8th day of June, 20 06, by MICHAEL S. HUNTER & BRENDA L. HUNTER, who is personally known to me or who has produced driver's license for identification.

Notary Public
My Commission Expires:

TERRY MCDAVID
MY COMMISSION # DD 500758
EXPIRES: January 16, 2010
Bonded Thru Notary Public Underwriters

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By Sharon Teagle
Deputy Clerk
Date 06-14-06

CLERK CIRCUIT COURT
COLUMBIA COUNTY, FLORIDA

EXHIBIT "A"

PART OF THE NORTHEAST 1/4 OF THE NORTHWEST 1/4 OF SECTION 31, TOWNSHIP 4 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA; BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS: FOR POINT OF REFERENCE COMMENCE AT THE NORTHWEST CORNER OF SAID NORTHEAST 1/4 OF THE NORTHWEST 1/4; THENCE RUN SOUTH 01°49'19" WEST ALONG THE WEST LINE OF SAID NORTHEAST 1/4 OF THE NORTHWEST 1/4, A DISTANCE OF 131.28 FEET TO A POINT LYING ON THE SOUTH RIGHT-OF-WAY LINE OF WESTER ROAD; THENCE RUN SOUTH 88°56'11" EAST ALONG SAID SOUTH RIGHT-OF-WAY LINE, A DISTANCE OF 717.46 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE SOUTH 88°56'11" EAST ALONG SAID SOUTH RIGHT-OF-WAY LINE, A DISTANCE OF 422.62 FEET; THENCE RUN SOUTH 02°55'26" WEST, A DISTANCE OF 515.63 FEET; THENCE RUN NORTH 88°56'11" WEST, A DISTANCE OF 422.62 FEET; THENCE RUN NORTH 02°55'26" WEST, A DISTANCE OF 515.63 FEET TO THE POINT OF BEGINNING.

Inst:2006014427 Date:06/14/2006 Time:16:35
_____DC,P.DeWitt Cason,Columbia County B:1086 P:2267

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0606-71 Date Received 6/20 By JW Permit # 24686

Application Approved by - Zoning Official BLK Date 27.06.06 Plans Examiner OK JTH Date 6-27-06

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments - MH will be replaced with new UMH. 30 day to remove MH after CO - NOC - Using Existing Well - ISSUED

Applicants Name SLK Construction Inc. Fax: 386 497 2289 Phone 386 365 3646

Address 764 SW Riverside Av. Ft White FL. 32038

Owners Name Mike Hunter Phone 386 288-3613

911 Address 724 SW Wester Dr. Lake City FL. 32024

Contractors Name Guy Williams Phone 386 497 2289

Address 764 SW Riverside Av. Ft White FL 32038

Fee Simple Owner Name & Address Mike Hunter 724 SW Wester Dr. Lake City FL. 32024

Bonding Co. Name & Address none

Architect/Engineer Name & Address C+G Gary Gill PE PO Box 187 Live OAK FL 32064

Mortgage Lenders Name & Address First Federal Lake City FL.

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 31-4S-17-08920-000 HX Estimated Cost of Construction 270,000.

Subdivision Name _____ Lot ____ Block ____ Unit ____ Phase ____

Driving Directions 47 South to Wester Rd. - Last Mobile Home on Right Before Curve to Right.

Type of Construction New Home (Framed) Number of Existing Dwellings on Property 1 MH

Total Acreage 8.5 Lot Size _____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 95' Side 84' Side 40 Rear 61

Total Building Height 22'6" Number of Stories 1 Heated Floor Area 2240 Roof Pitch 7/12

Porch 1970 GARAGE 576 TOTAL 4,786

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

SLK Construction Inc. Sam L Zeigler Sam L Zeigler Guy Williams

Owner Builder or Agent (Including Contractor) Contractor Signature

Contractors License Number CBC 050640

Competency Card Number 102 00000 54

STATE OF FLORIDA
COUNTY OF COLUMBIA

NOTARY PUBLIC STATE OF FLORIDA Susan Nettles Villegas My Commission DD267694 Expires December 15, 2007

NOTARY STAMP/SEAL

Sworn to (or affirmed) and subscribed before me this 20 day of June 2006.

Personally known ✓ or Produced Identification _____

Susan N. Villegas

Notary Signature

Cr. 8788

Columbia County Property Appraiser

DB Last Updated: 5/5/2006

2006 Proposed Values

Parcel: 31-4S-17-08920-000 HX

Tax Record | Property Card | Interactive GIS Map | Print

<< Prev Search Result: 18 of 24 Next >>

Owner & Property Info

Owner's Name	HUNTER MICHAEL STEPHEN
Site Address	WESTER
Mailing Address	724 SW WESTER DRIVE LAKE CITY, FL 32024
Description	NE1/4 OF NW1/4 AS LIES S OF WESTER RD & A 2.30 AC PARCEL BEING APPROX THE N 70.65 FT OF SE1/4 OF NW1/4 AS DESC IN ORB 855-2445. ORB 339-559, 755-916, 796-478, 855-2440, PROBATE 1075-511 THRU 514. EX 28 AC DESC ORB 1077-925

Use Desc. (code)	IMPROVED A (005000)
Neighborhood	31417.00
Tax District	3
UD Codes	MKTA01
Market Area	01
Total Land Area	8.500 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (2)	$14,480.00
Ag Land Value	cnt: (1)	$2,392.00
Building Value	cnt: (1)	$22,709.00
XFOB Value	cnt: (7)	$4,260.00
Total Appraised Value		$43,841.00

Just Value		$101,449.00
Class Value		$43,841.00
Assessed Value		$21,823.00
Exempt Value	(code: HX)	$21,823.00
Total Taxable Value		$0.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vimp	Sale Qual	Sale RCode	Sale Price
NONE						

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	MOBILE HME (000800)	1986	WD or PLY (08)	1664	2024	$22,709.00
	Note: All S.F. calculations are based on exterior building dimensions.					

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0040	BARN,POLE	0	$300.00	1.000	0 x 0 x 0	(.00)
0296	SHED METAL	1994	$360.00	120.000	10 x 12 x 0	AP (40.00)
0297	SHED CONCR	1994	$540.00	120.000	10 x 12 x 0	AP (40.00)
0070	CARPORT UF	1993	$1,080.00	360.000	18 x 20 x 0	(.00)
0296	SHED METAL	1993	$360.00	120.000	10 x 12 x 0	AP (40.00)

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000102	SFR/MH (MKT)	1.000 AC	1.00/1.00/1.00/1.00	$12,480.00	$12,480.00
005400	TIMBER 1 (AG)	7.500 AC	1.00/1.00/1.00/1.00	$319.00	$2,392.00
009910	MKT.VAL.AG (MKT)	7.500 AC	1.00/1.00/1.00/1.00	$0.00	$60,000.00
009945	WELL/SEPT (MKT)	1.000 UT - (.000AC)	1.00/1.00/1.00/1.00	$2,000.00	$2,000.00

Columbia County Property Appraiser

DB Last Updated: 5/5/2006

FROM :

PHONE NO. :

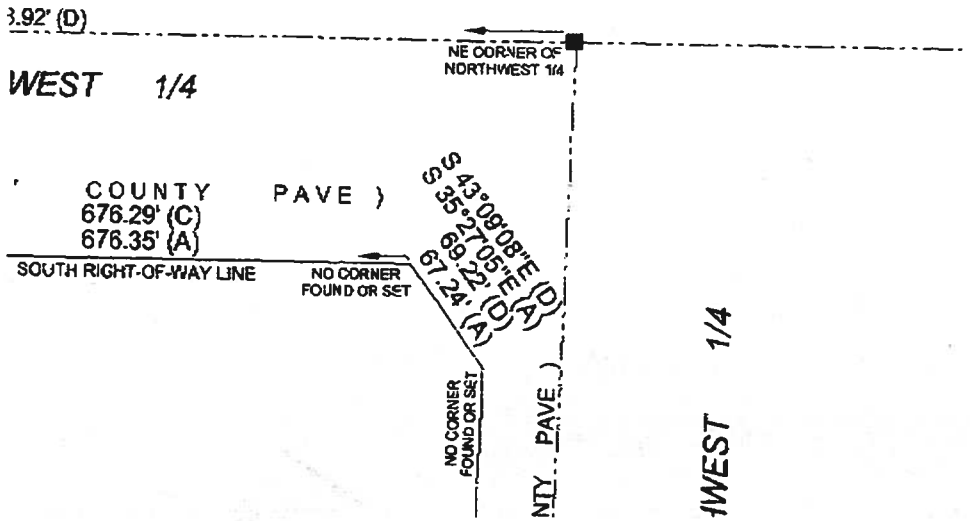
MAY. 23 2006 04:09PM P1

Attn: Mike Hunter

BOUNDARY SURVEY OF

PART OF THE NORTHEAST 1/4 OF THE NORTHWEST 1/4 OF SECTION 31, TOWNSHIP 4 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA; BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS: FOR POINT OF REFERENCE COMMENCE AT THE NORTHWEST CORNER OF SAID NORTHEAST 1/4 OF THE NORTHWEST 1/4; THENCE RUN SOUTH 01°49'19" WEST ALONG THE WEST LINE OF SAID NORTHEAST 1/4 OF THE NORTHWEST 1/4, A DISTANCE OF 131.28 FEET TO A POINT LYING ON THE SOUTH RIGHT-OF-WAY LINE OF WESTER ROAD; THENCE RUN SOUTH 88°56'11" EAST ALONG SAID SOUTH RIGHT-OF-WAY LINE, A DISTANCE OF 717.46 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE SOUTH 88°56'11" EAST ALONG SAID SOUTH RIGHT-OF-WAY LINE, A DISTANCE OF 422.62 FEET; THENCE RUN SOUTH 02°55'26" WEST, A DISTANCE OF 515.63 FEET; THENCE RUN NORTH 88°56'11" WEST, A DISTANCE OF 422.62 FEET; THENCE RUN NORTH 02°55'26" WEST, A DISTANCE OF 515.63 FEET TO THE POINT OF BEGINNING. CONTAINING: 5.00 ACRES MORE OR LESS.

724 S.W. Wester Dr.
Lake City Fl. 32024



LEGEND & NOTES

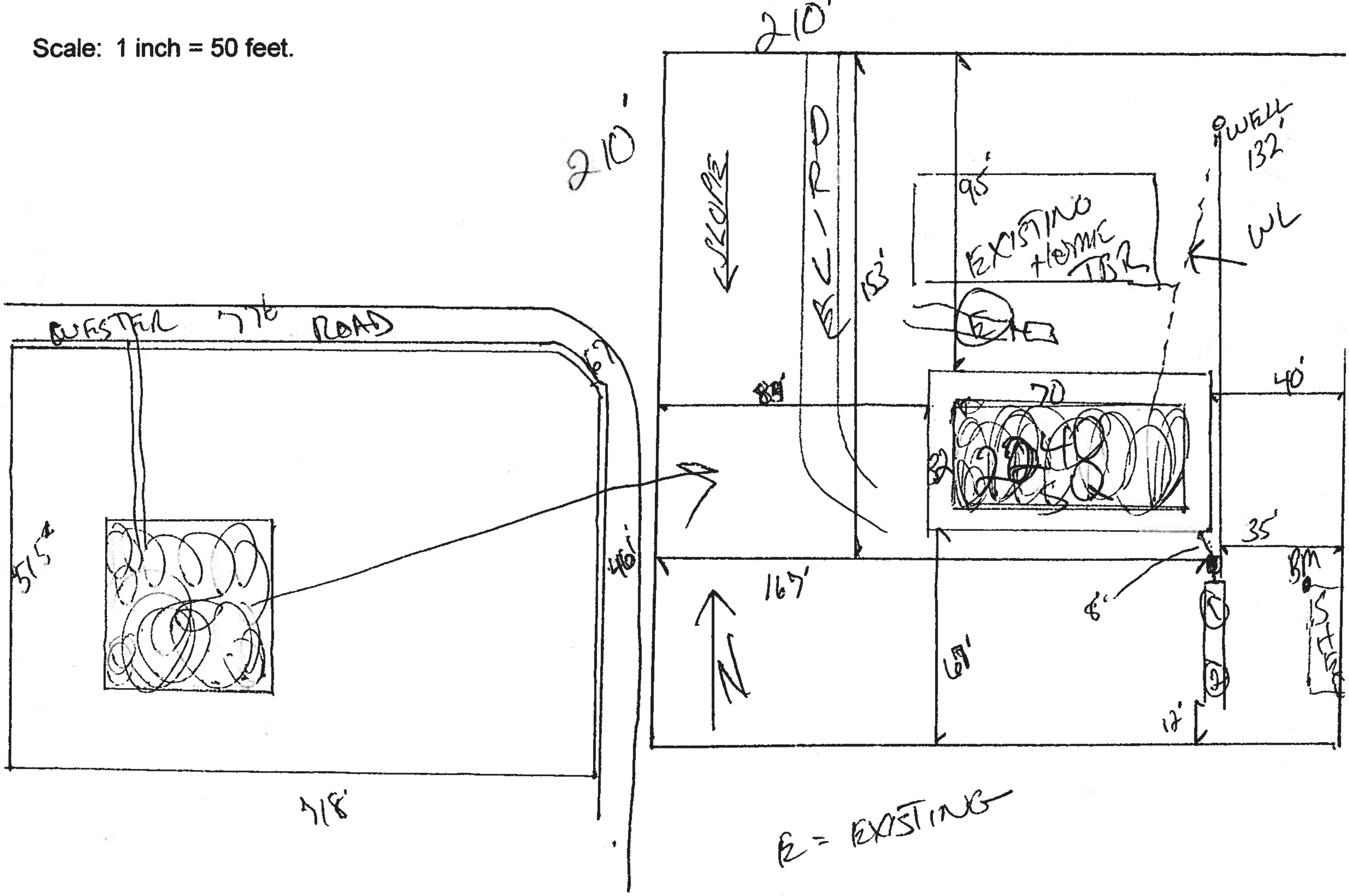
- □ DENOTES 4" X 4" CONCRETE MONUMENT SET, L.B. # 7170
- ■ DENOTES 4" X 4" CONCRETE MONUMENT FOUND, L.B. # 6685
- ○ DENOTES 5/8" REBAR W / CAP SET, L.B. # 7170.
- ● DENOTES IRON PIPE OR REBAR FOUND.
- ⌀ DENOTES POWER POLE
- X----X----X DENOTES EXISTING FENCE.
- E----E----E DENOTES OVERHEAD ELECTRIC.
- 1) FENCE, ROAD AND OVERHEAD ELECTRIC DIMENSIONS MAY NOT BE TO SCALE.
- 2) CLOSURE EXCEEDS 1 : 10,000
- 3) PROPERTY SURVEYED AS PER DESCRIPTION PROVIDED

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION F0R ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-0546N

---------------------------- PART II - SITEPLAN ----------------------------

Scale: 1 inch = 50 feet.



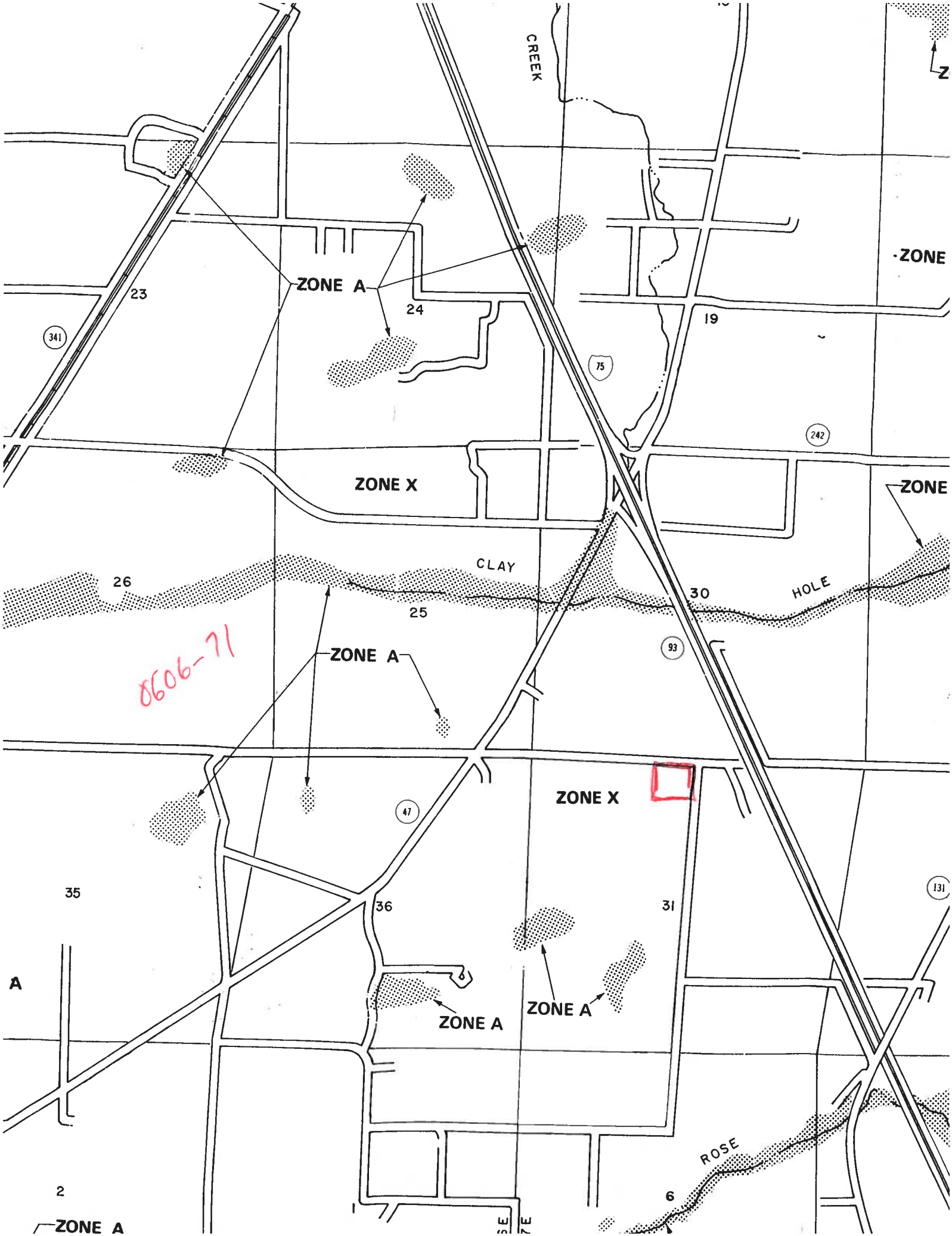
Notes: 1 of 8.5 Acres

Site Plan submitted by: [signature] MASTER CONTRACTOR

Plan Approved ✓ Not Approved_____ Date 6-13-06

By [signature] Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT


CREEK
ZONE A
ZONE X
ZONE
23
24
19
341
75
242
CLAY
26
25
30
HOLE
93
0606-71
47
35
36
31
131
A
ROSE
2
6

24686

Notice of Treatment F012129

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: 5365E BAYA DR.
City Lake City Phone (386)752-1703

Site Location: Subdivision N/A
Lot # — Block# — Permit #
Address 724 SW Wester Rd. Lake City

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	~~0.1%~~ .05 %
☐ Termidor	Fipronil	0.12%
☐ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☐ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Perimeter	—	368	150

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line RDC.

02-13-07 (Date) 10:30 (Time) R D Crawford (Print Technician's Name)

Remarks:

Applicator - White Permit File - Canary Permit Holder - Pink

10/05 ©

Notice of Treatment 12129

Applicator: **Florida Pest Control & Chemical Co. (www.flapest .com)**
Address: BAYA AVE
City LAKE CITY Phone 752-1703

Site Location: Subdivision
Lot # Block# Permit # 24686
Address 724 SW WESTER DR

Product used	Active Ingredient	% Concentration
☑ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☐ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☑ Soil ☐ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling (Porches & Garage)	4786	368	500

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line ________.

8/28/06 — Date
0800 — Time
EZ[illegible] GUINNY — Print Technician's Name

Remarks: ______

Applicator - White **Permit File - Canary** **Permit Holder - Pink**

10/05 ©

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 31-4S-17-08920-000 **Building permit No.** 24686

Use Classification SINGLE FAMILY DWELLING **Fire:** -0-

Permit Holder GUY WILLIAMS **Waste:** -0-

Owner of Building MIKE HUNTER **Total:** -0-

Location: 724 SW WESTER DRIVE, LAKE CITY, FL

Date: 3-13-07

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

GTC DESIGN GROUP

GTC Design Group, LLC
P.O. Box 187
Live Oak, FL 32064
(Phone) 386.362.3678
(Fax) 386.362.6133
ggill@gtcdesigngroup.com

June 26, 2006

Joe Haltiwanger
Plans Examiner
135 NE Hernando Avenue
P. O. Box 1529
Lake City, Florida 32056-1529

SUBJECT: Mike Hunter - Garage Beam

Joe,

The garage header shall be a Truss-Joist Microlam 3 1/2x 11 ¼" (min bending stress 2,600 psi) or equal.

If you have any questions or require additional information, please contact me at your convenience.

Thank you,

Gary Gill, P.E. #51942
Project Manager
6/26/06

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0606-71

Contractor: SLK Construction Owner Mike Hunter 724 SW Wester Dr.

On the date of June 22, 2006 application 0606-71 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0606-71 when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

To help ensure compliance with the Florida Residential Code 2004 the comments below need to be addressed on the plans.

1. Please submit a recorded (with the Columbia County Clerk Office) notice of commencement before any inspections can be preformed by the Columbia County Building Department.

2. Sheet S-2.0 of the plans details a typical garage header detail; please have Mr. Gill design the header beam which will span the 18 foot overhead garage door opening.

3. On the electrical plan show the location of the electrical panel and include the total amperage rating of the electrical service panel also show the overcurrent protection device which shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

Joe Haltiwanger

Plan Examiner
Columbia County Building Department

GTC DESIGN GROUP

GTC Design Group, LLC
P.O Box 187
Live Oak, FL 32064
(Phone) 386 362 3678
(Fax) 386 362 6133
ggill@gtcdesigngroup.com

June 26, 2006

Joe Haltiwanger
Plans Examiner
135 NE Hernando Avenue
P. O. Box 1529
Lake City, Florida 32056-1529

SUBJECT: Mike Hunter - Garage Beam

Joe,

The garage header shall be a Truss-Joist Microlam 3 1/2x 11 ¼" (min bending stress 2,600 psi) or equal.

If you have any questions or require additional information, please contact me at your convenience.

Thank you,

Gary Gill, P.E. #51942
Project Manager

6/26/06

Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
 Document ID:1SXB487-Z0418171404

Truss Fabricator:	Anderson Truss Company
Job Identification:	6-202--SLK Construction Hunter -- , **
Truss Count:	24
Model Code:	Florida Building Code 2004
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Version 7.24.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-02 -Closed

Notes:

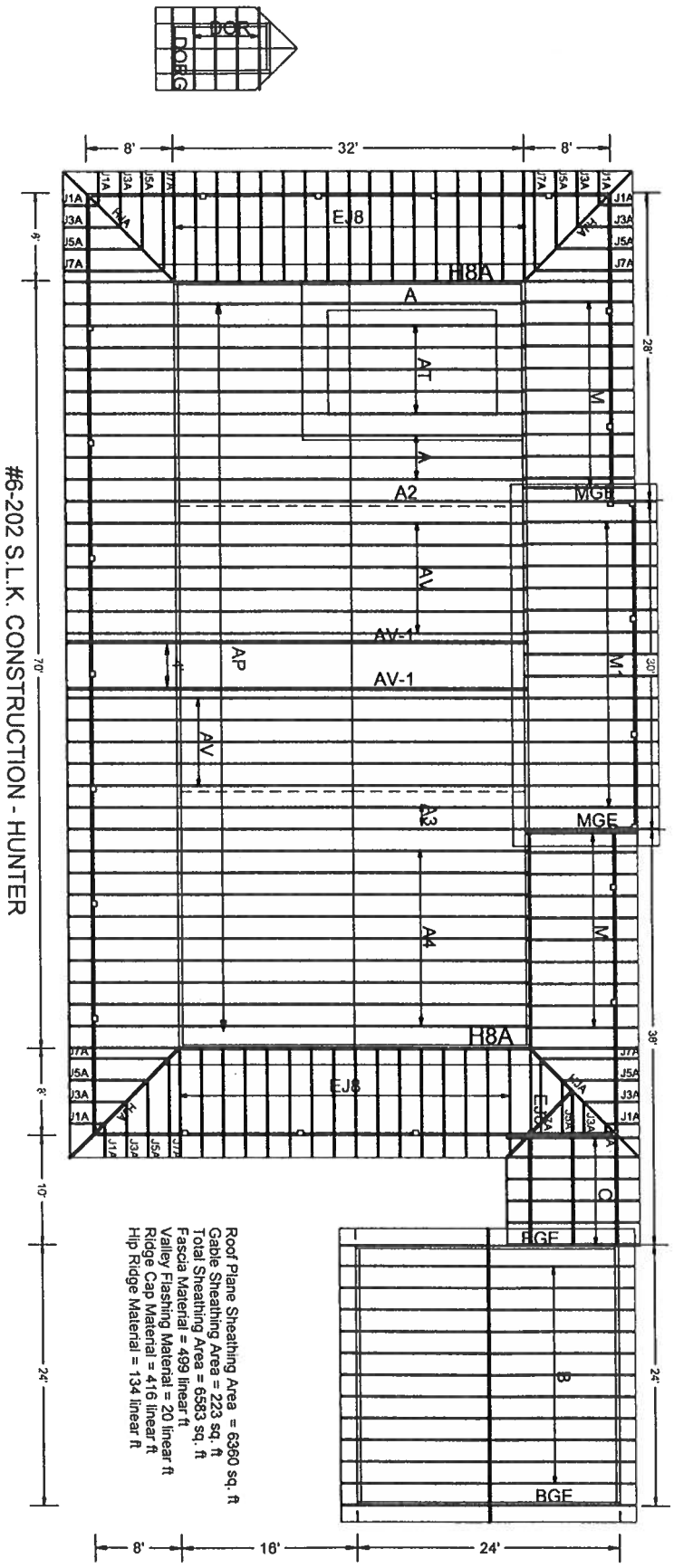
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: BRCLBSUB-A11015EE-GBLLETIN-MAX DEAD LOAD-PIGBACKB-A11030EE-

#	Ref Description	Drawing#	Date
1	19268--A	06138095	05/18/06
2	19269--A2	06138096	05/18/06
3	19270--A3	06138097	05/18/06
4	19271--A4	06138098	05/18/06
5	19272--AT	06138099	05/18/06
6	19273--AV	06138100	05/18/06
7	19274--AV	06138101	05/18/06
8	19275--AV-1	06138102	05/18/06
9	19276--H8A	06138109	05/18/06
10	19277--B	06138086	05/18/06
11	19278--BGE	06138103	05/18/06
12	19279--C	06138087	05/18/06
13	19280--EJ8	06138088	05/18/06
14	19281--HJA	06138104	05/18/06
15	19282--J1A	06138105	05/18/06
16	19283--J3A	06138089	05/18/06
17	19284--J5A	06138090	05/18/06
18	19285--J7A	06138091	05/18/06
19	19286--M	06138092	05/18/06
20	19287--M1	06138093	05/18/06
21	19288--MGE	06138106	05/18/06
22	19289--AP	06138107	05/18/06
23	19290--DOR	06138094	05/18/06
24	19291--DORG	06138108	05/18/06

Seal Date: 05/18/2006

-Truss Design Engineer-
Denise Rutledge
Florida License Number: 58752
1950 Marley Drive
Haines City, FL 33844



#6-202 S.L.K. CONSTRUCTION - HUNTER

Roof Plane Sheathing Area = 6360 sq. ft
Gable Sheathing Area = 223 sq. ft
Total Sheathing Area = 6583 sq. ft
Fascia Material = 499 linear ft
Valley Flashing Material = 20 linear ft
Ridge Cap Material = 416 linear ft
Hip Ridge Material = 134 linear ft

4971-3620
365-3646 ext 11

PAGE NO: 1 OF 1

(6-202--SLK Construction Hunter -- , ** - A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

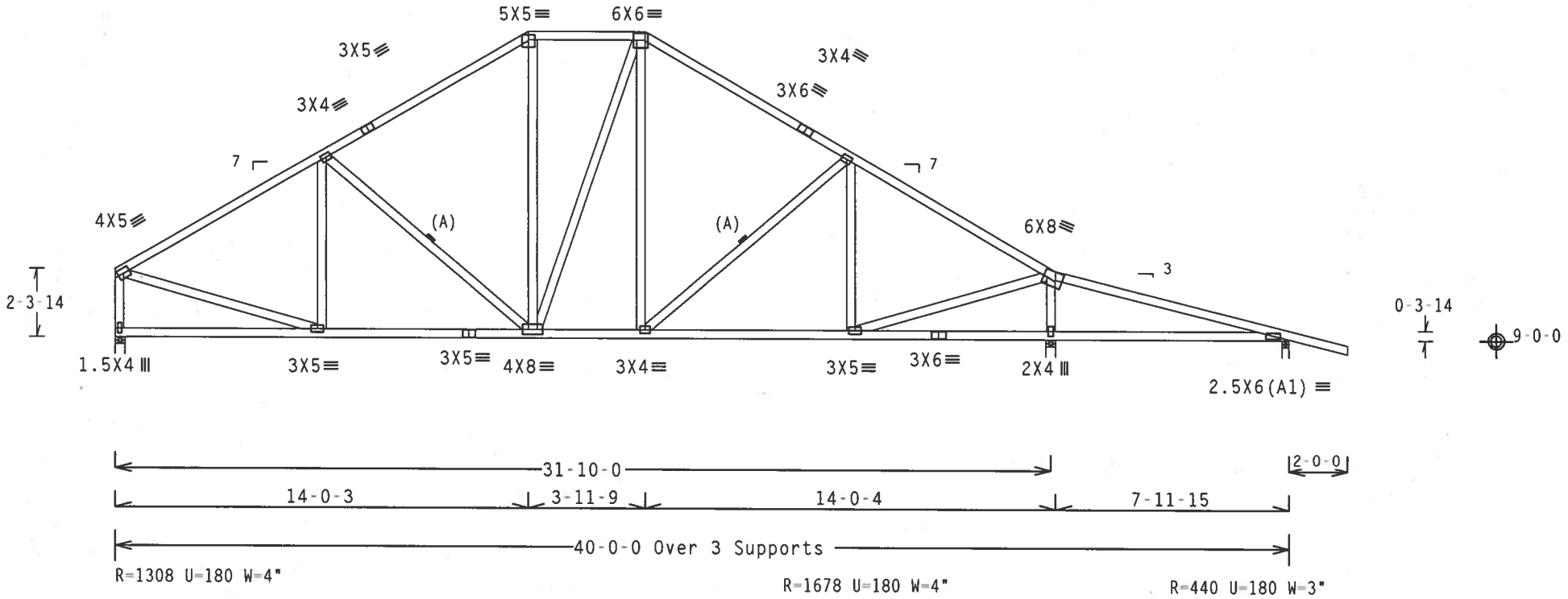
Left end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1230

QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

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DENISE RUTLEDGE LICENSE No. 58758 STATE OF FLORIDA PROFESSIONAL ENGINEER

May 18 06

TC LL	20.0 PSF	REF R487-- 19268
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138095
BC LL	0.0 PSF	HC-ENG JB/ADR
TOT.LD.	40.0 PSF	SEQN- 7816
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

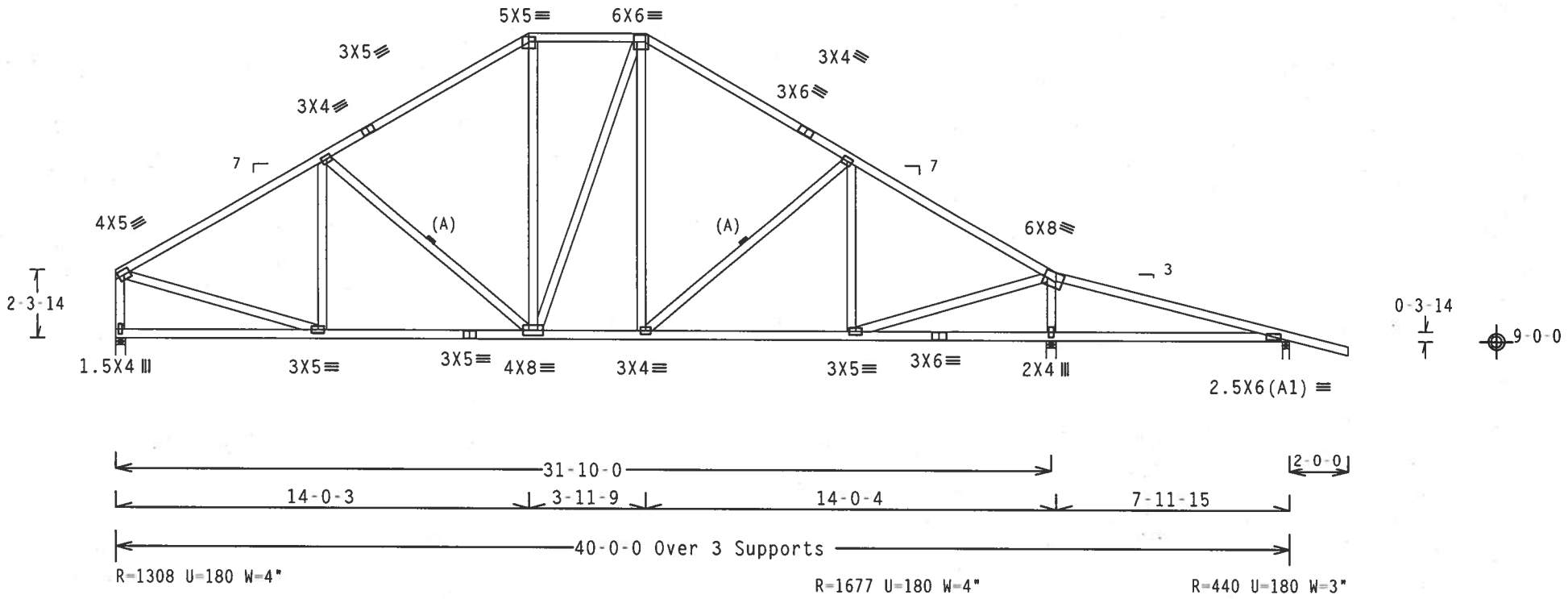
Left end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
Alpine Engineered Products, Inc.
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DENISE RUTLEDGE LICENSE No. 58752 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 18 06

TC LL	20.0 PSF	REF R487-- 19269
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138096
BC LL	0.0 PSF	HC-ENG JB/ADR
TOT.LD.	40.0 PSF	SEQN- 7818
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

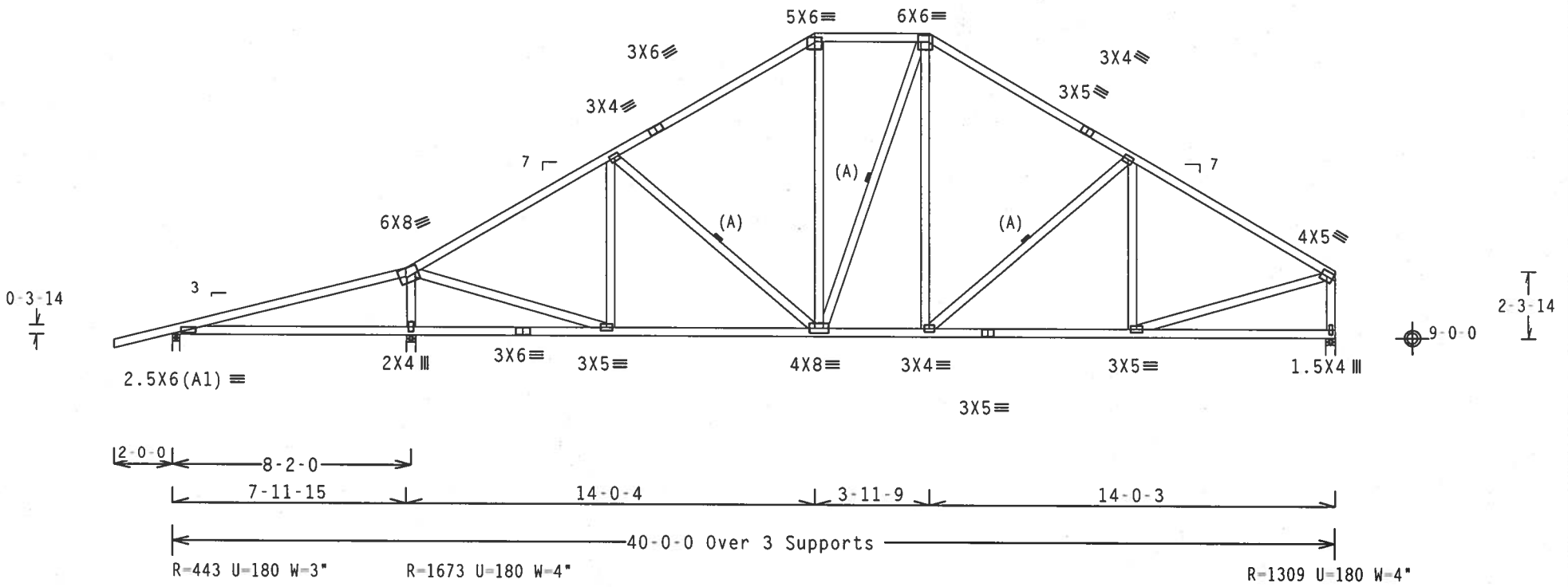
(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1230

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

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DENISE RUTLEDGE LICENSE NO. 58752 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 18, 06

TC LL	20.0 PSF	REF R487-- 19270
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138097
BC LL	0.0 PSF	HC-ENG JB/ADR
TOT.LD.	40.0 PSF	SEQN- 7826
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

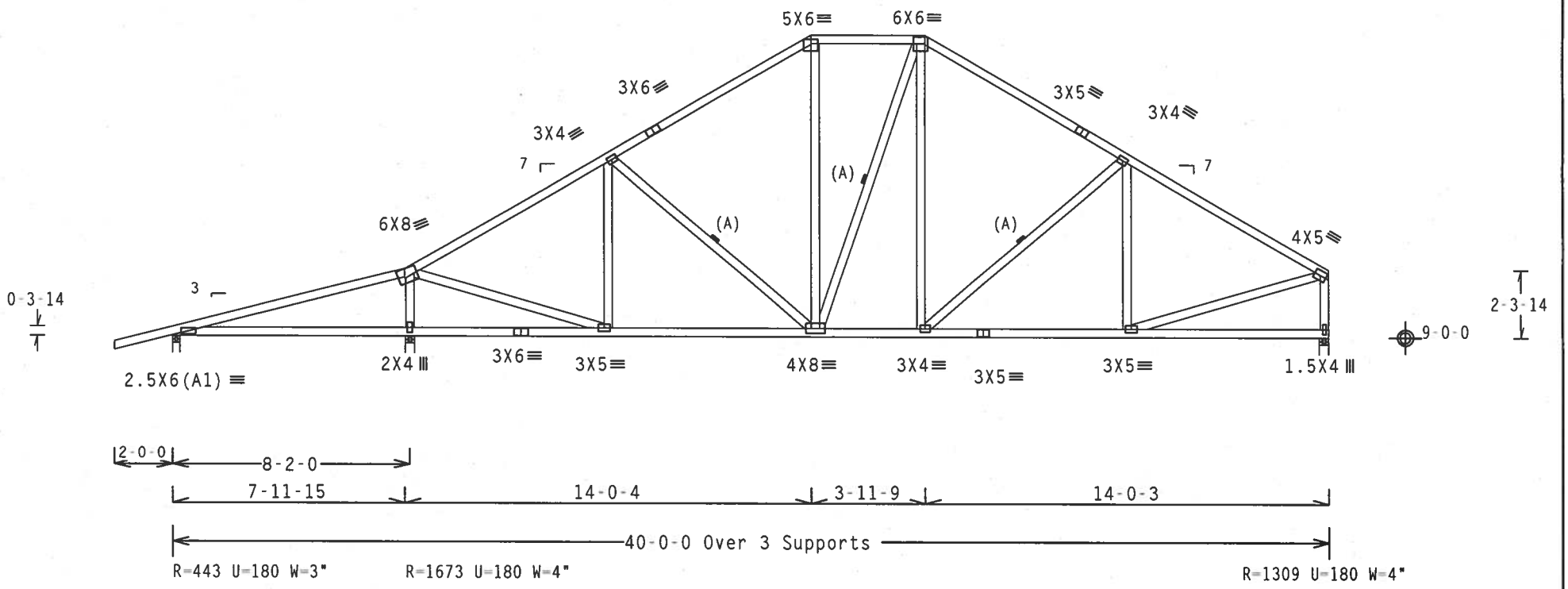
(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1230 1 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

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DENISE RUTLEDGE LICENSE No. 56782 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 18 06

TC LL	20.0 PSF	REF R487-- 19271
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138098
BC LL	0.0 PSF	HC-ENG JB/ADR
TOT.LD.	40.0 PSF	SEQN- 7820
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

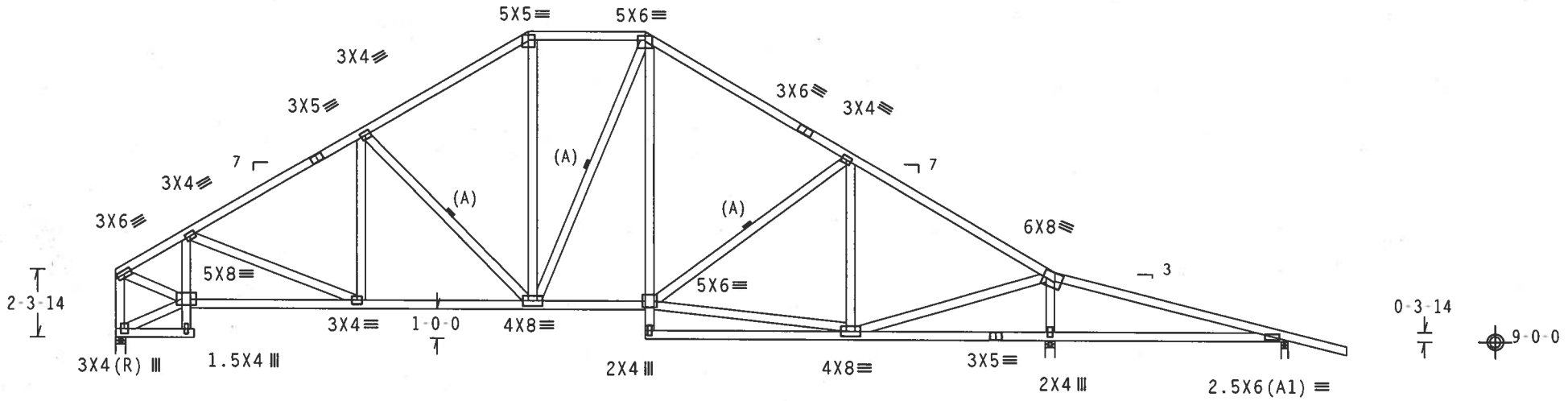
Left end vertical not exposed to wind pressure.

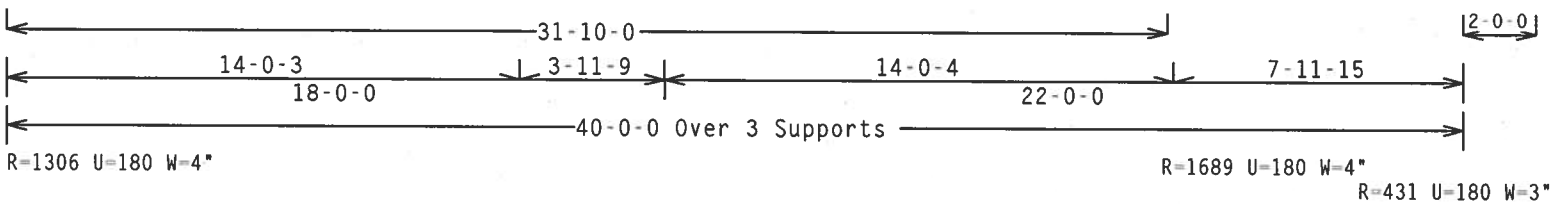
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.122
QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

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FL Certificate of Authorization # 567

DENISE RUTLEDGE LICENSE No 58782 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 18 '06

TC LL	20.0 PSF	REF	R487-- 19272
TC DL	10.0 PSF	DATE	05/18/06
BC DL	10.0 PSF	DRW	HCUSR487 06138099
BC LL	0.0 PSF	HC-ENG	JB/ADR
TOT.LD.	40.0 PSF	SEQN-	7817
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SXB487 Z04

(6-202--SLK Construction Hunter -- , ** - AV)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

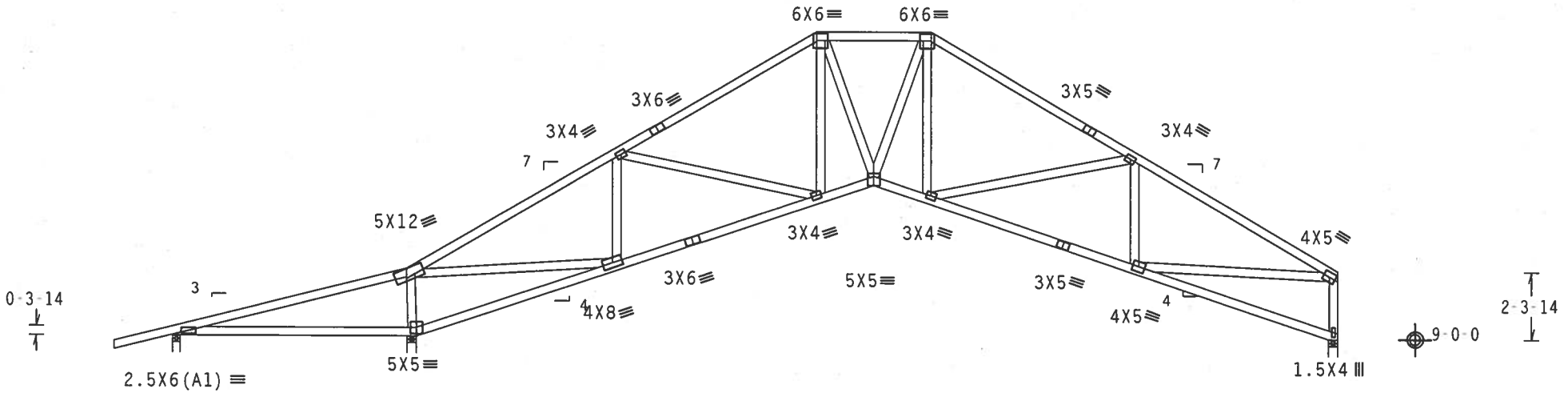
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

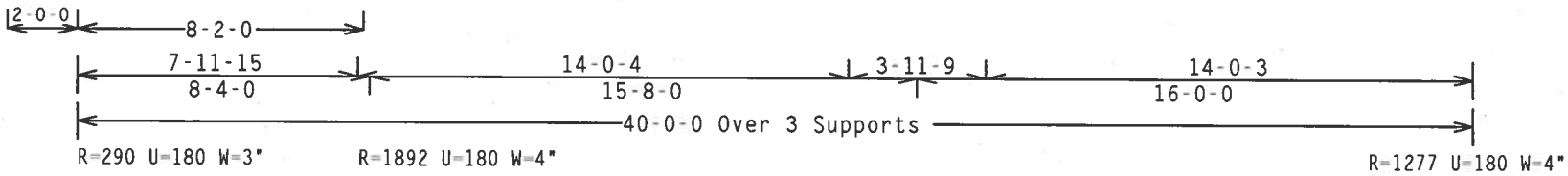
Shim all supports to solid bearing.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

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DENISE RUTLEDGE LICENSE No. 58758 STATE OF FLORIDA PROFESSIONAL ENGINEER

May 18 '06

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TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138100
BC LL	0.0 PSF	HC-ENG JB/ADR
TOT.LD.	40.0 PSF	SEQN- 7825
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

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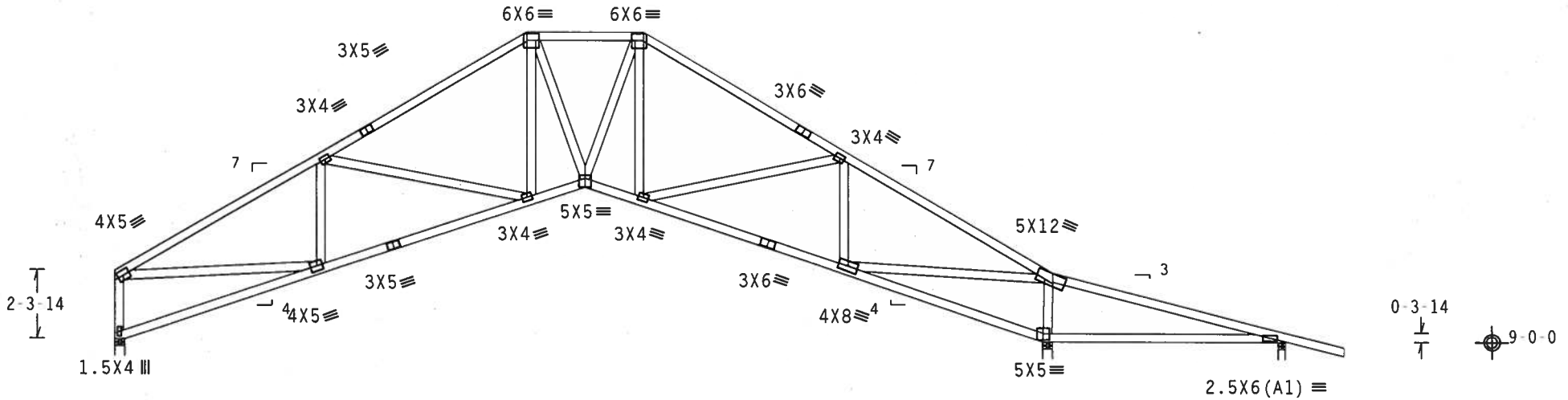
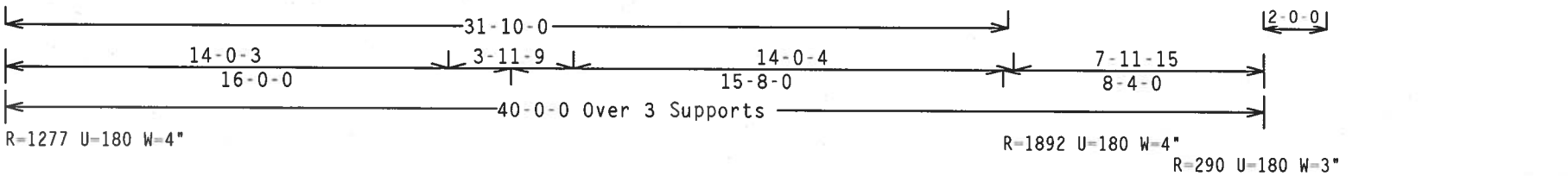
Shim all supports to solid bearing.





Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

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ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

DENISE RUTLEDGE LICENSE No. 58782 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 18 06

TC LL	20.0 PSF	REF R487-- 19274
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138101
BC LL	0.0 PSF	HC-ENG JB/ADR
TOT.LD.	40.0 PSF	SEQN- 7819
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

Top chord 2x6 SP #2 :T6 2x4 SP #2 Dense:
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W11 2x4 SP #2 Dense:

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC	From	126 PLF	at	8.72	to	126 PLF at 14.02
TC	From	121 PLF	at	14.02	to	121 PLF at 17.98
TC	From	126 PLF	at	17.98	to	126 PLF at 23.42
TC	From	126 PLF	at	23.42	to	126 PLF at 23.43
TC	From	265 PLF	at	23.43	to	266 PLF at 23.51
TC	From	266 PLF	at	23.51	to	335 PLF at 31.98
TC	From	196 PLF	at	31.98	to	196 PLF at 32.00
TC	From	191 PLF	at	32.00	to	191 PLF at 32.02
TC	From	121 PLF	at	32.02	to	121 PLF at 42.00
BC	From	42 PLF	at	0.00	to	42 PLF at 31.67
BC	From	40 PLF	at	31.67	to	40 PLF at 40.00
BC	From	8 PLF	at	40.00	to	8 PLF at 42.00

Left end vertical not exposed to wind pressure.

Trusses to be spaced at 48.0" OC maximum.

Shim all supports to solid bearing.

2 COMPLETE TRUSSES REQUIRED

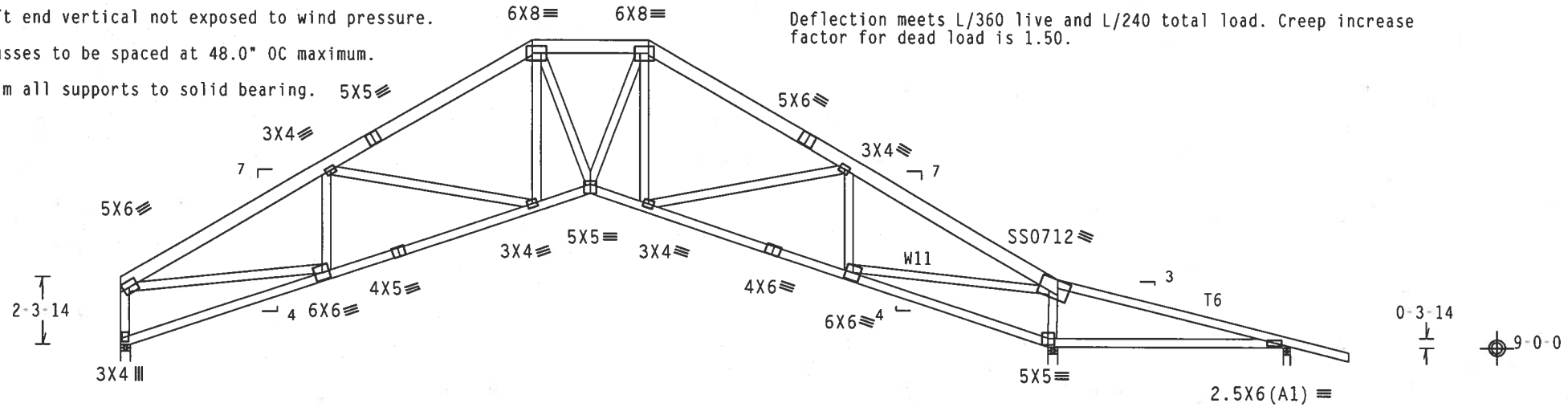
Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @11.50" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

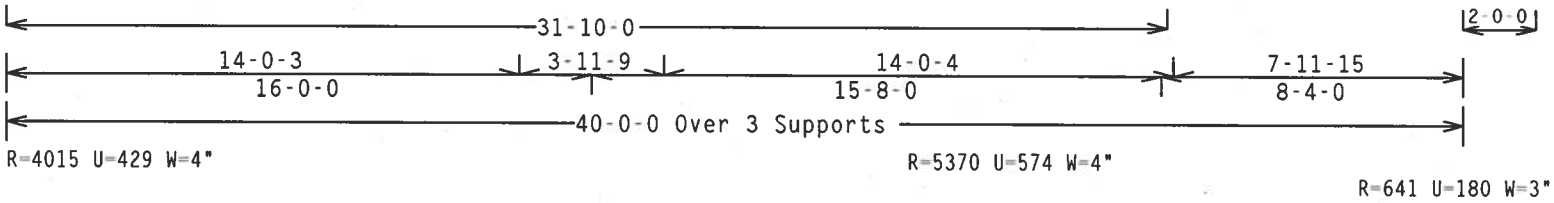
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 8.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.12" due to live load and 0.18" due to dead load.

In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. 18 Gauge HS,Wave Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
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DENISE RUTLEDGE LICENSE No. 58782 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 18, 06

TC LL	20.0	PSF	REF R487-- 19275
TC DL	10.0	PSF	DATE 05/18/06
BC DL	10.0	PSF	DRW HCUSR487 06138102
BC LL	0.0	PSF	HC-ENG JB/ADR
TOT.LD.	40.0	PSF	SEQN- 7854
DUR.FAC.	1.25		
SPACING	48.0"		JREF- 1SXB487 Z04

Top chord 2x4 SP #2 Dense :T2, T5, T7 2x6 SP #2:
Bot chord 2x6 SP #2
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 61 PLF at -2.00 to 61 PLF at 8.57
TC - From 83 PLF at 8.57 to 83 PLF at 20.00
TC - From 85 PLF at 20.00 to 85 PLF at 24.00
TC - From 83 PLF at 24.00 to 83 PLF at 28.00
TC - From 85 PLF at 28.00 to 85 PLF at 30.79
TC - From 83 PLF at 30.79 to 83 PLF at 39.43
TC - From 61 PLF at 39.43 to 61 PLF at 40.00
TC - From 61 PLF at 40.00 to 61 PLF at 50.00
BC - From 4 PLF at -2.00 to 4 PLF at 0.00
BC - From 20 PLF at 0.00 to 20 PLF at 14.00
BC - From 20 PLF at 14.00 to 20 PLF at 48.00
BC - From 4 PLF at 48.00 to 4 PLF at 50.00
TC - 205 LB Conc. Load at 8.06, 10.06, 12.06, 14.06, 16.06 18.06, 20.06, 22.06, 23.94, 25.94, 27.94, 29.94, 31.94, 33.94 35.94, 37.94, 39.94
BC - 794 LB Conc. Load at 8.00, 40.00
BC - 94 LB Conc. Load at 10.06, 12.06, 14.06, 16.06, 18.06 20.06, 22.06, 23.94, 25.94, 27.94, 29.94, 31.94, 33.94, 35.94 37.94

+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS.
BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Truss spaced at 24.0" OC designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

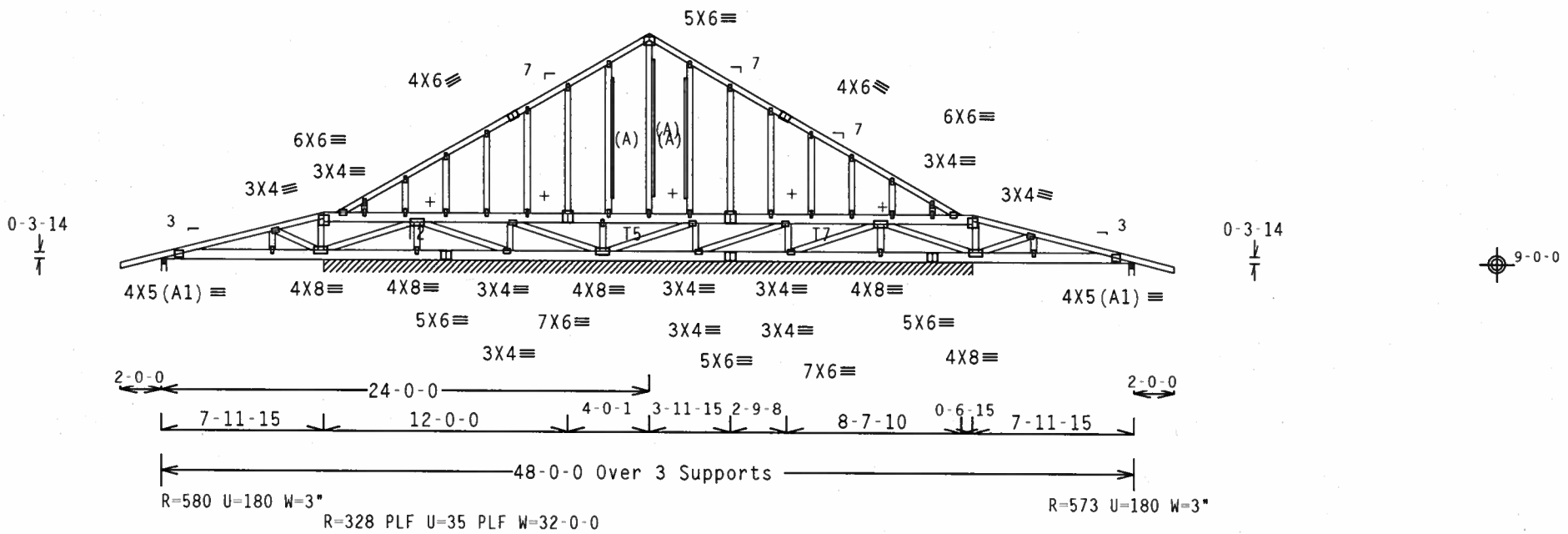
(A) 1x4 SP #3 or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE

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DENISE RUTLEDGE LICENSE No. 58782 STATE OF FLORIDA PROFESSIONAL ENGINEER

May 18, 06

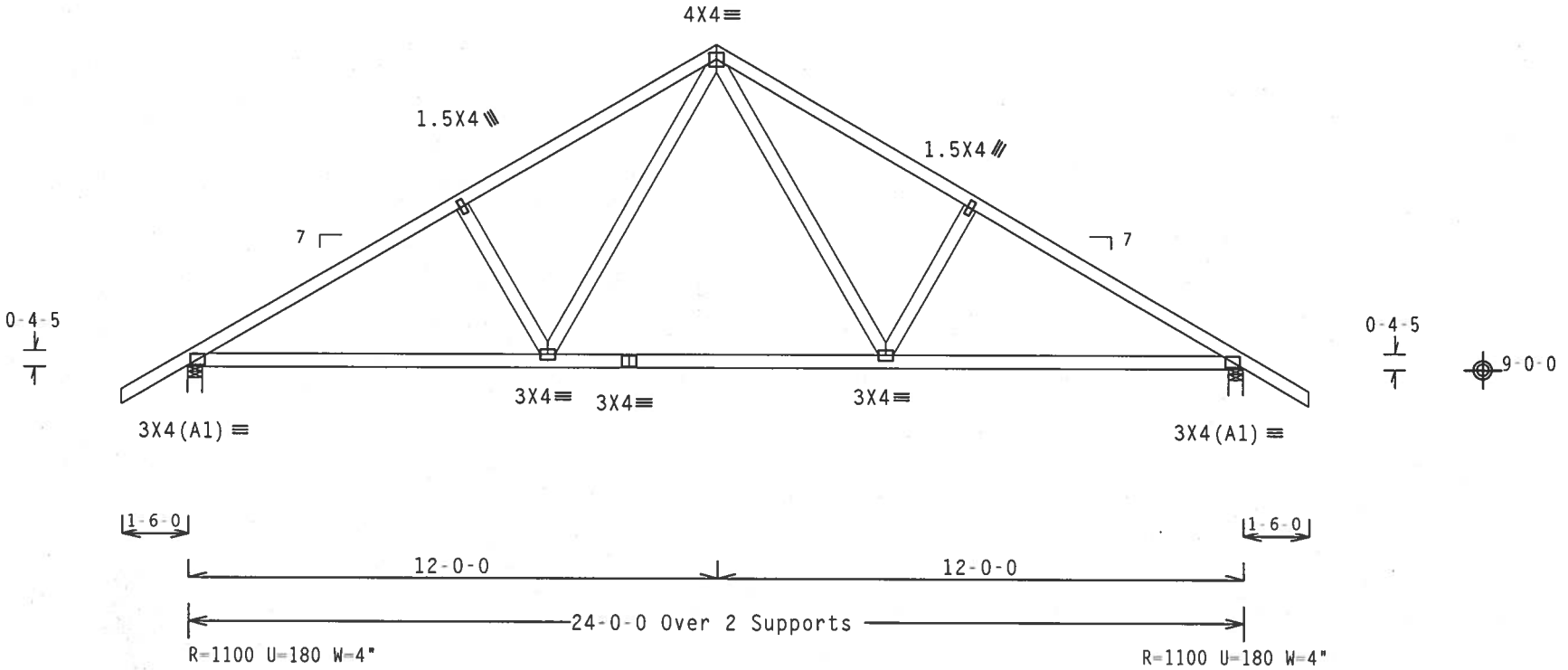
TC LL	20.0 PSF	REF R487-- 19276
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138109
BC LL	0.0 PSF	HC-ENG JB/ADR
TOT.LD.	40.0 PSF	SEQN- 7838
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.123

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/-

Scale =.25"/Ft.

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DENISE RUTLEDGE LICENSE No. 58782 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 18 '06

TC LL	20.0 PSF	REF R487-- 19277
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138086
BC LL	0.0 PSF	HC-ENG JB/ADR *
TOT.LD.	40.0 PSF	SEQN- 7815
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

(6-202--SLK Construction Hunter -- , ** - BGE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	85 PLF	at 12.00	to	85 PLF	at 25.50
BC - From	5 PLF	at -1.50	to	5 PLF	at 0.00
BC - From	20 PLF	at 0.00	to	20 PLF	at 14.00
BC - From	20 PLF	at 14.00	to	20 PLF	at 24.00
BC - From	5 PLF	at 24.00	to	5 PLF	at 25.50

See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

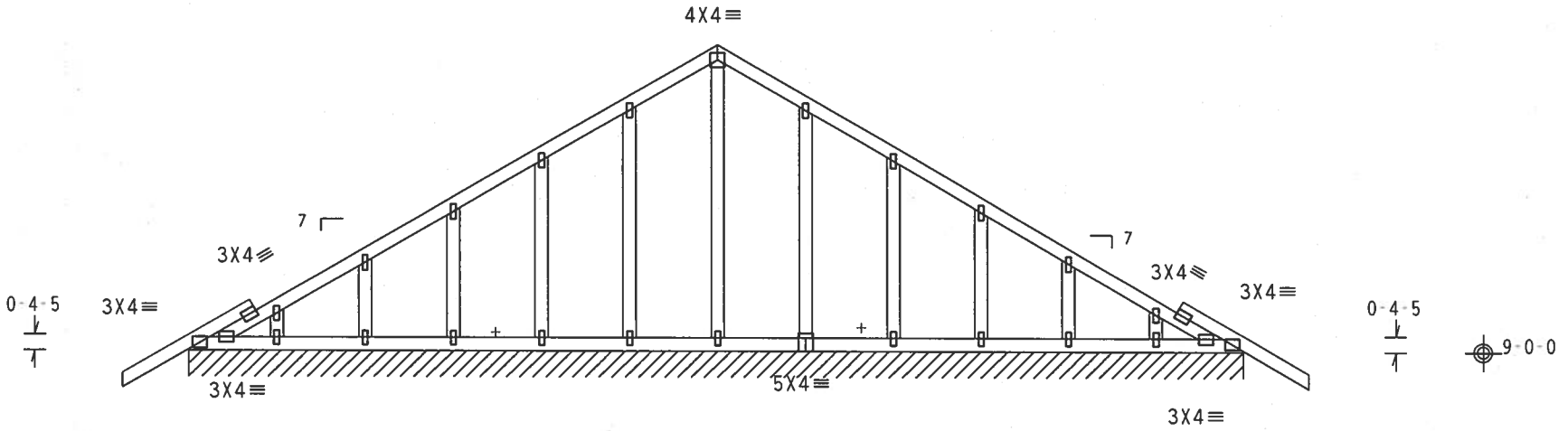
+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS.
BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

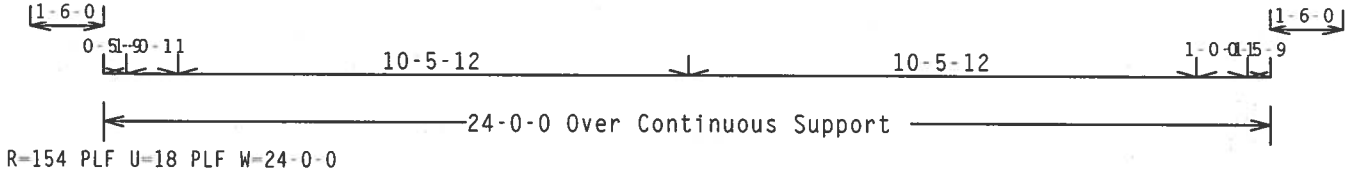
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Dead loads are stated on projected horizontal area basis.

Truss spaced at 24.0" OC designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ 24" OC.





R=154 PLF U=18 PLF W=24-0-0

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.123 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

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DENISE RUTLEDGE LICENSE NO. 58762 STATE OF FLORIDA PROFESSIONAL ENGINEER

May 18 '06

TC LL	20.0 PSF	REF	R487-- 19278
TC DL	10.0 PSF	DATE	05/18/06
BC DL	10.0 PSF	DRW	HCUSR487 06138103
BC LL	0.0 PSF	HC-ENG	JB/ADR
TOT.LD.	40.0 PSF	SEQN-	7844 REV
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SXB487 Z04

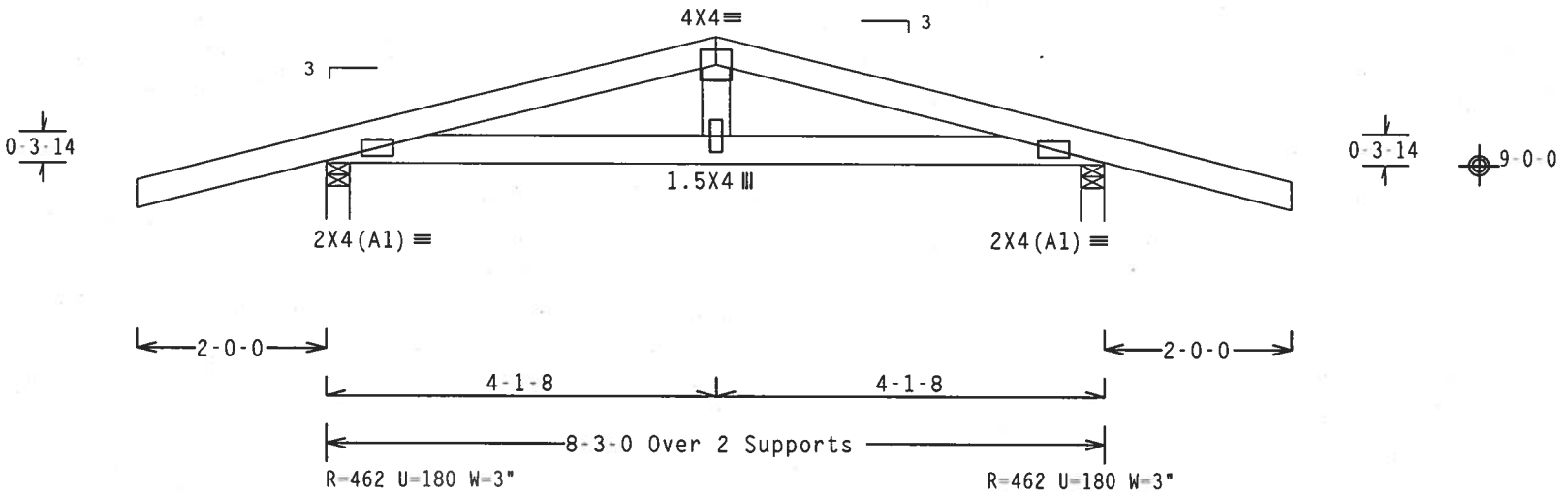
(6-202--SLK Construction Hunter -- , ** - C)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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DENISE RUTLEDGE LICENSE No. 58792 STATE OF FLORIDA PROFESSIONAL ENGINEER

May 18 06

TC LL	20.0 PSF	REF R487-- 19279
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138087
BC LL	0.0 PSF	HC-ENG JB/ADR *
TOT.LD.	40.0 PSF	SEQN- 7830
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

(6-202--SLK Construction Hunter -- , ** - EJ8)

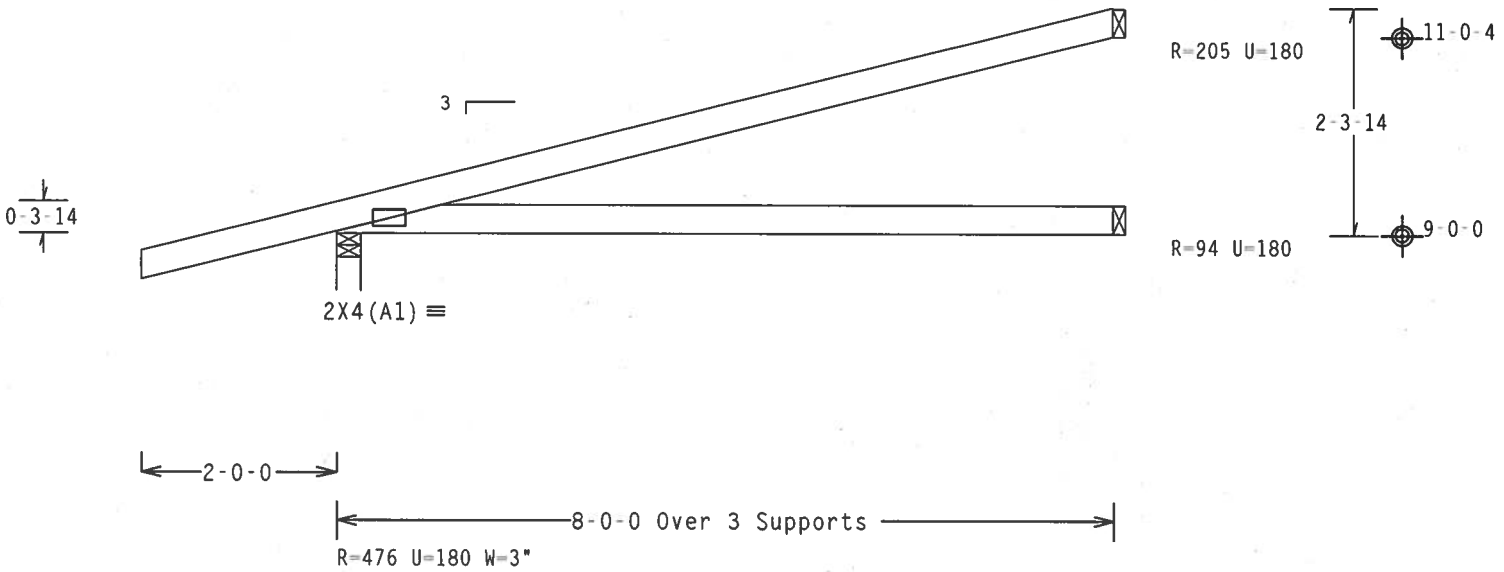
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DENISE RUTLEDGE LICENSE NO. 58752 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 18 06

TC LL	20.0 PSF	REF R487-- 19280
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138088
BC LL	0.0 PSF	HC-ENG JB/ADR *
TOT.LD.	40.0 PSF	SEQN- 7829
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

(6-202--SLK Construction Hunter -- , ** - HJA)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3

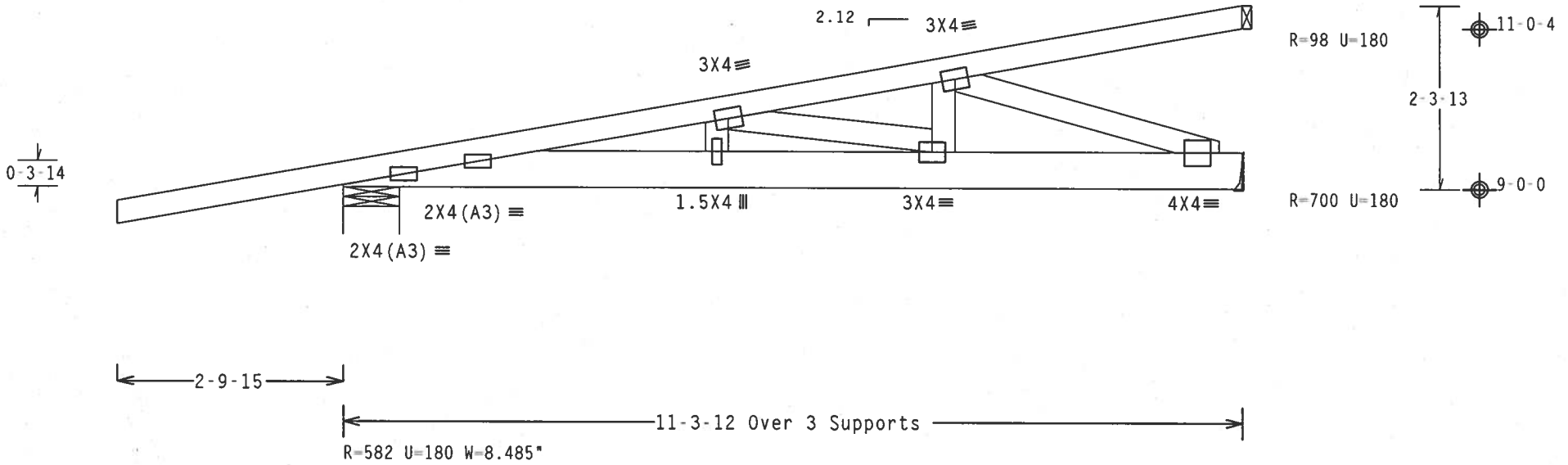
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Hipjack supports 8-0-0 setback jacks. Jacks up to 7' have no webs. Longer jacks supported to BC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1230 1 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

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DENISE RUTLEDGE LICENSE No. 38752 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 18, 06

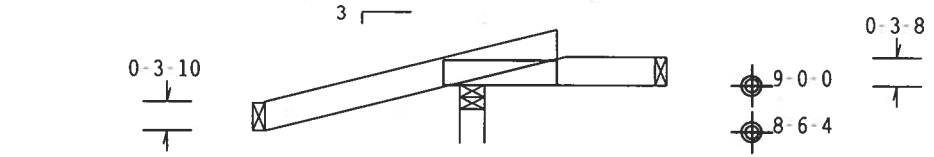
TC LL	20.0 PSF	REF R487-- 19281
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138104
BC LL	0.0 PSF	HC-ENG JB/ADR
TOT.LD.	40.0 PSF	SEQN- 7837
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

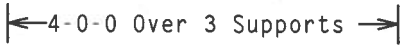
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

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DENISE RUTLEDGE LICENSE No. 58702 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 18 06

TC LL	20.0 PSF	REF R487-- 19282
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138105
BC LL	0.0 PSF	HC-ENG JB/ADR
TOT.LD.	40.0 PSF	SEQN- 7828
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

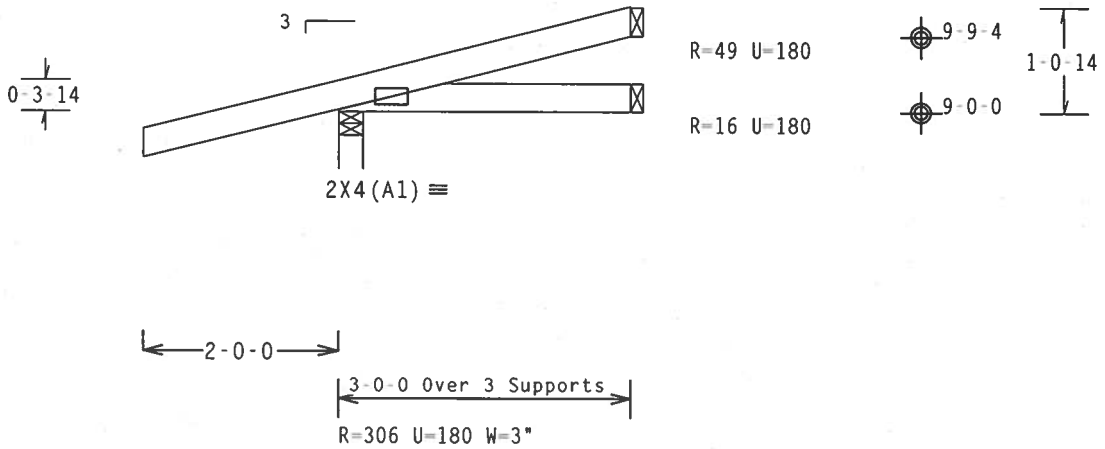
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.123

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/-

Scale =.5"/Ft.

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ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

DENISE RUTLEDGE LICENSE No. 58752 STATE OF FLORIDA PROFESSIONAL ENGINEER

May 18 '06

TC LL	20.0 PSF	REF R487-- 19283
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138089
BC LL	0.0 PSF	HC-ENG JB/ADR *
TOT.LD.	40.0 PSF	SEQN- 7823
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

(6-202--SLK Construction Hunter -- , ** - J5A)

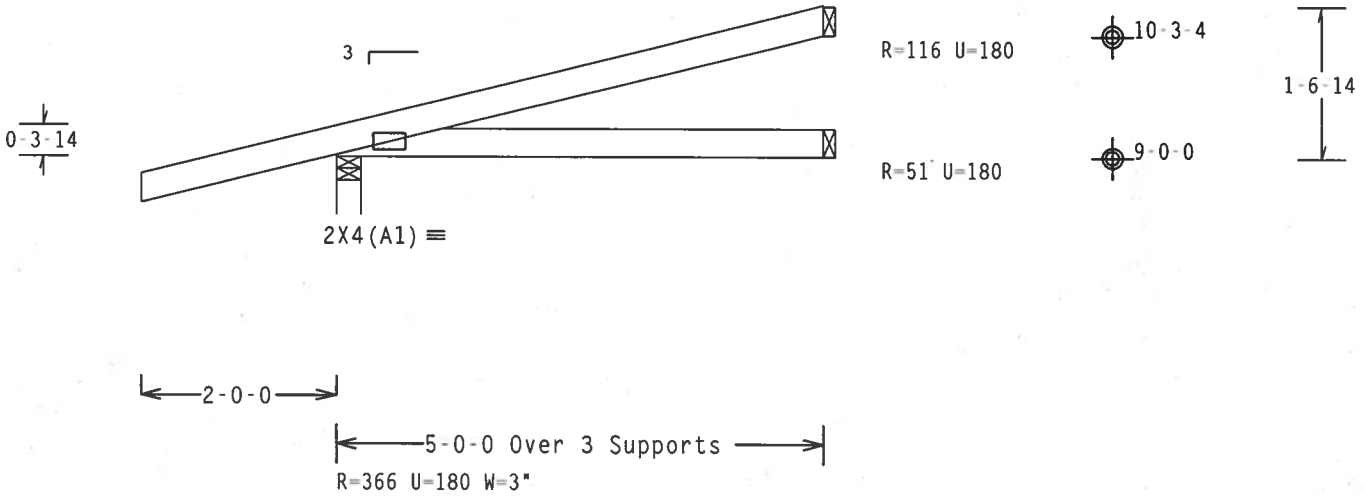
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

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DENISE RUTLEDGE LICENSE No. 58782 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 18 '06

TC LL	20.0 PSF	REF R487-- 19284
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138090
BC LL	0.0 PSF	HC-ENG JB/ADR *
TOT.LD.	40.0 PSF	SEQN- 7822
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

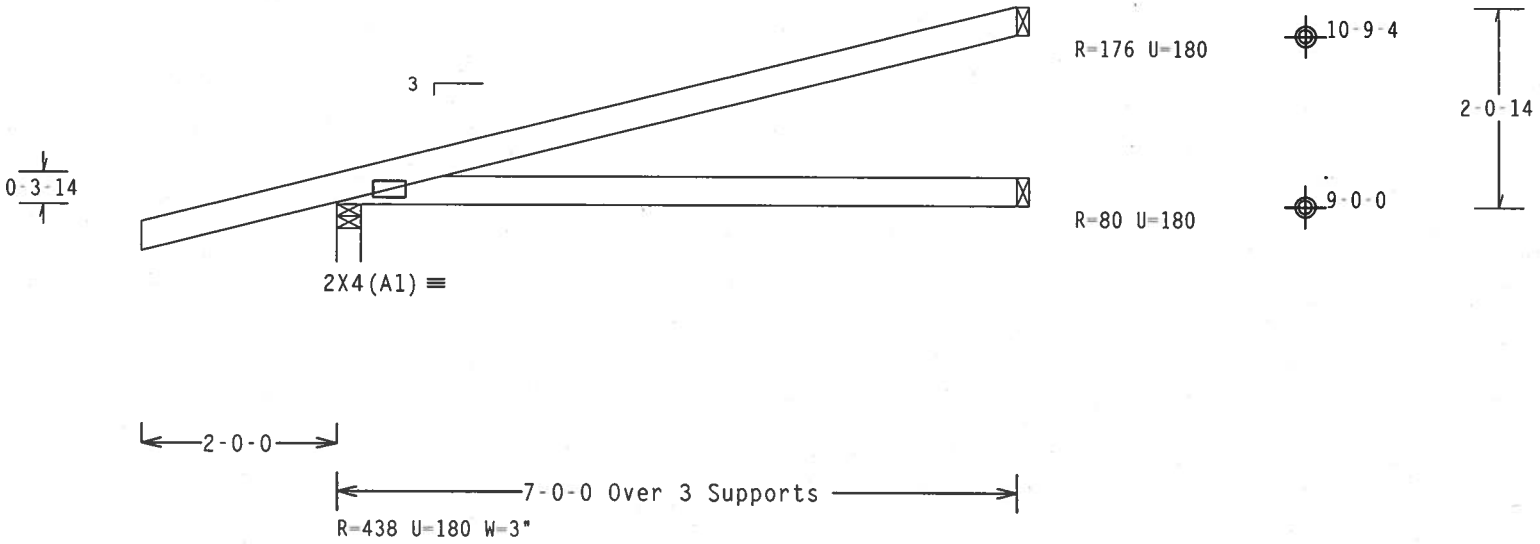
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

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DENISE RUTLEDGE LICENSE No. 58252 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 18 06

TC LL	20.0 PSF	REF R487-- 19285
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138091
BC LL	0.0 PSF	HC-ENG JB/ADR *
TOT.LD.	40.0 PSF	SEQN- 7821
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

(6-202--SLK Construction Hunter -- , ** - M)

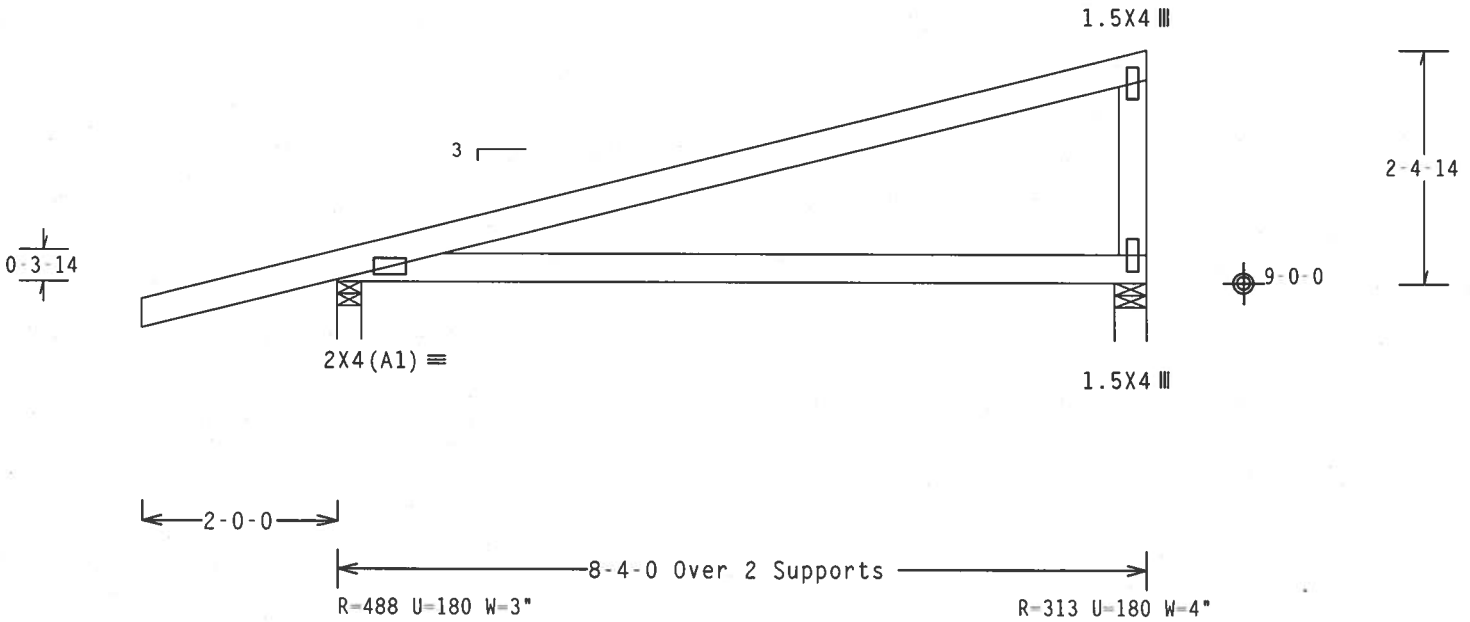
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.123

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

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DENISE RUTLEDGE LICENSE No. 58752 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 18 '06

TC LL	20.0 PSF	REF R487-- 19286
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138092
BC LL	0.0 PSF	HC-ENG JB/ADR *
TOT.LD.	40.0 PSF	SEQN- 7824
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

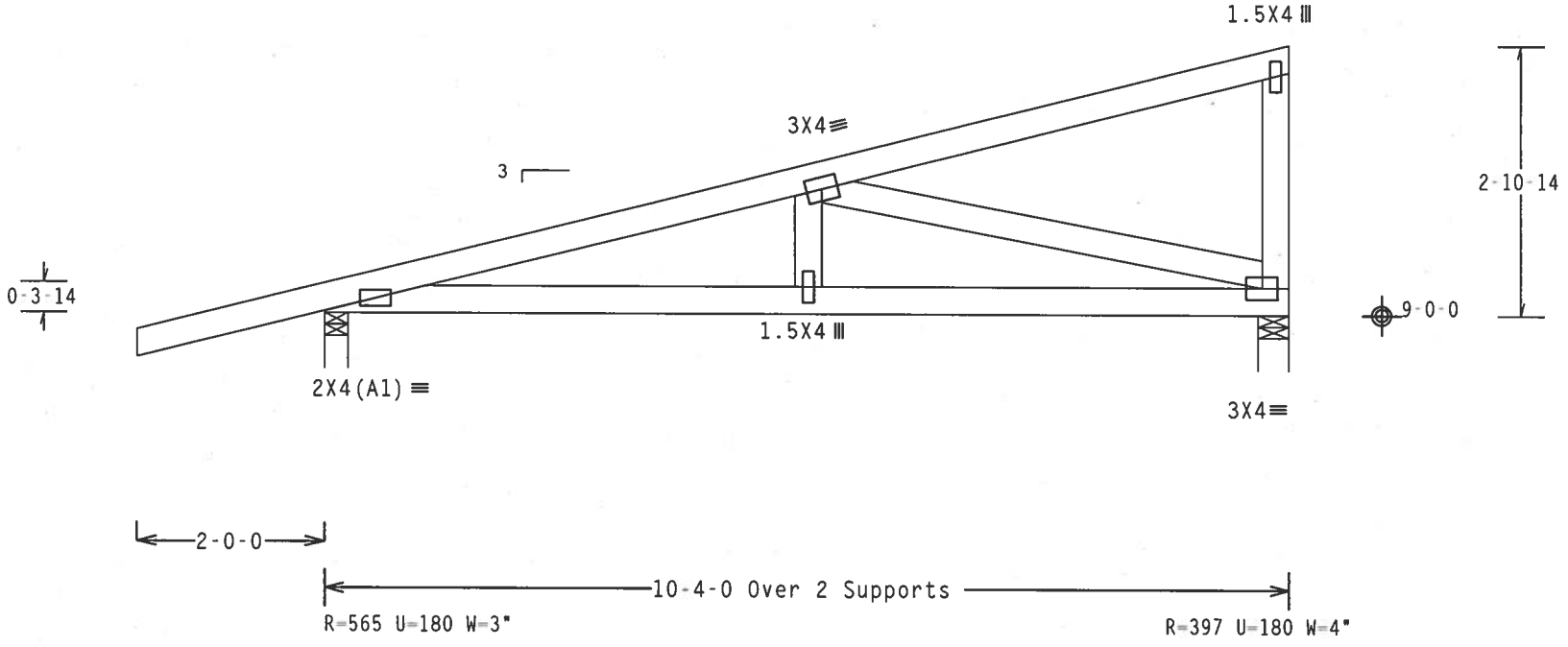
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1230

QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

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ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

DENISE RUTLEDGE LICENSE No. 58752 STATE OF FLORIDA PROFESSIONAL ENGINEER

May 18 '06

TC LL	20.0 PSF	REF R487-- 19287
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138093
BC LL	0.0 PSF	HC-ENG JB/ADR *
TOT.LD.	40.0 PSF	SEQN- 7835
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 0 PLF at 10.33 to 0 PLF at -2.00
BC - From 20 PLF at 0.00 to 20 PLF at 10.33

Truss spaced at 24.0" OC designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS.
BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 3.56 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

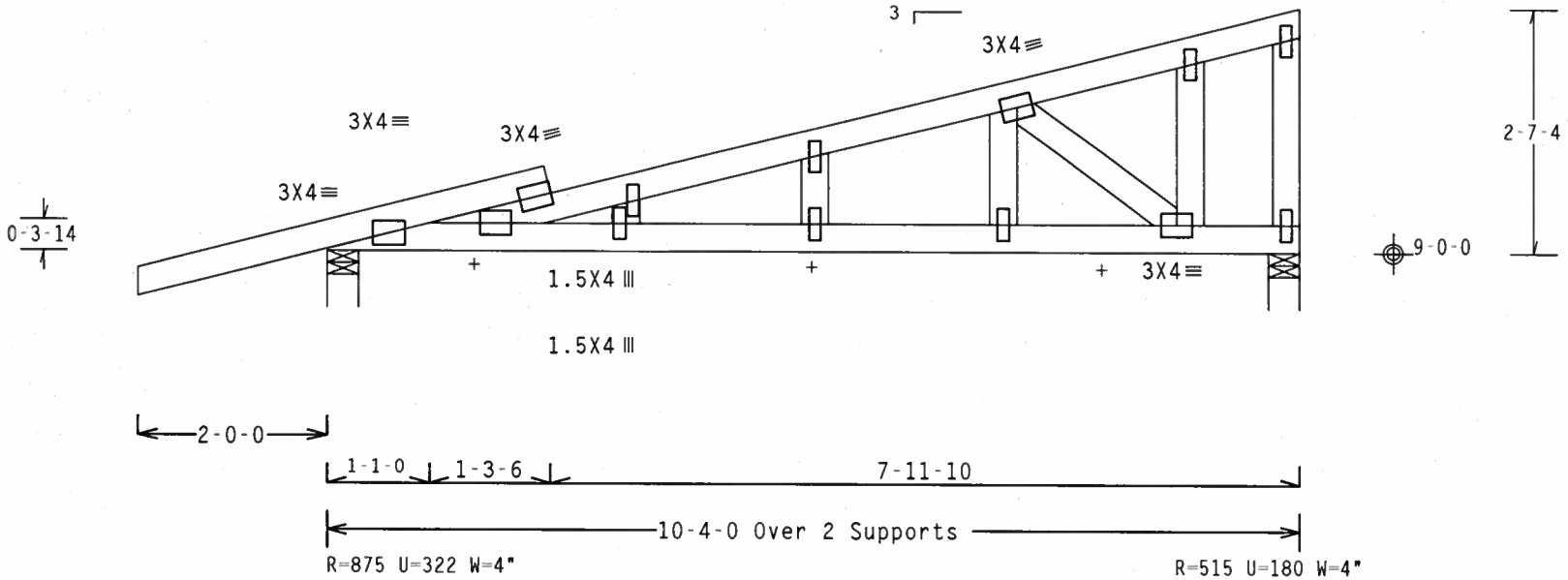
Right end vertical not exposed to wind pressure.

Dead loads are stated on projected horizontal area basis.

See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.123 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

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DENISE RUTLEDGE LICENSE No. 58752 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 18 '06

TC LL	20.0 PSF	REF R487-- 19288
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138106
BC LL	0.0 PSF	HC-ENG JB/ADR
TOT.LD.	40.0 PSF	SEQN- 7849 REV
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

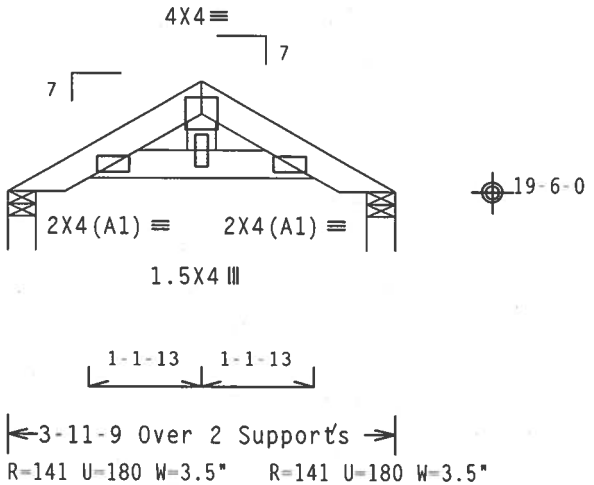
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

REFER TO DWG PIGBACKB0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24"O.C., UNLESS OTHERWISE SPECIFIED

110 mph wind, 20.08 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.123[illegible]

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

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May 18 '06

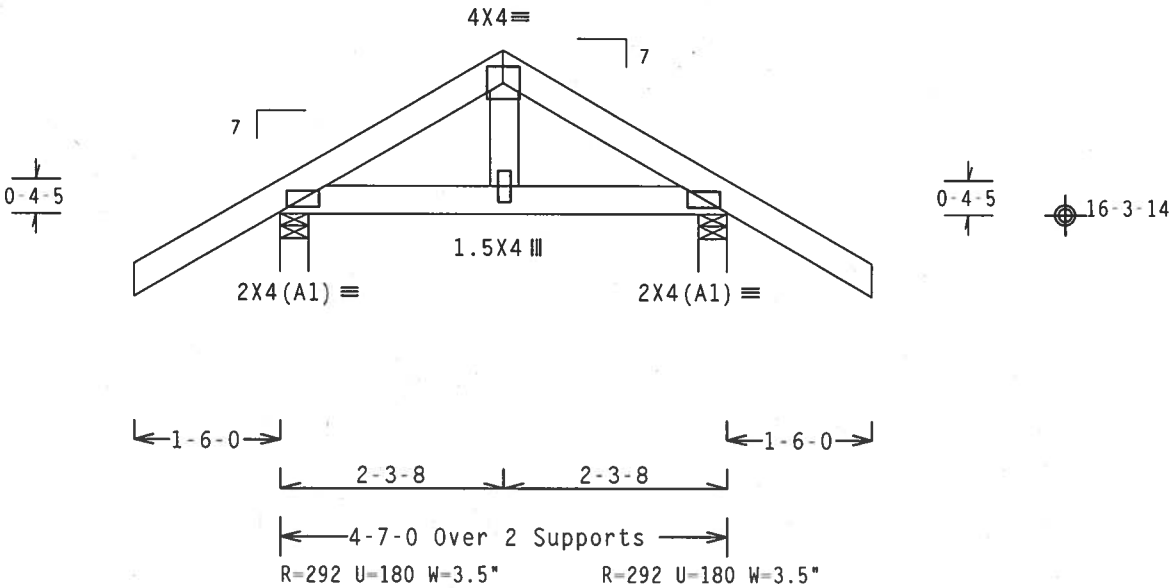
TC LL	20.0 PSF	REF R487-- 19289
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138107
BC LL	0.0 PSF	HC-ENG JB/ADR
TOT.LD.	40.0 PSF	SEQN- 7842
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 16.91 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.123

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

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DENISE RUTLEDGE LICENSE No. 58752 STATE OF FLORIDA PROFESSIONAL ENGINEER

May 18 '06

TC LL	20.0 PSF	REF R487-- 19290
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138094
BC LL	0.0 PSF	HC-ENG JB/ADR *
TOT.LD.	40.0 PSF	SEQN- 7836
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Truss spaced at 24.0" OC designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

110 mph wind, 16.91 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 3.56 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

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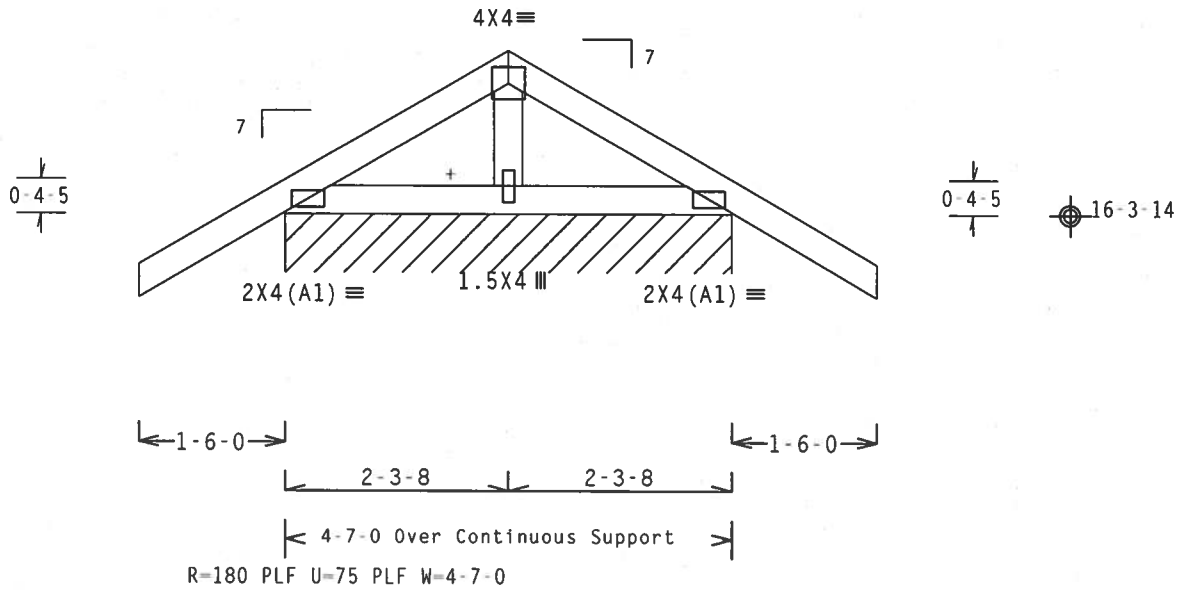
In lieu of structural panels or rigid ceiling use purlins to brace TC 24" OC, BC @ 24" OC.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

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ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

DENISE RUTLEDGE LICENSE No. 58756 STATE OF FLORIDA PROFESSIONAL ENGINEER
May 18 06

TC LL	20.0 PSF	REF R487-- 19291
TC DL	10.0 PSF	DATE 05/18/06
BC DL	10.0 PSF	DRW HCUSR487 06138108
BC LL	0.0 PSF	HC-ENG JB/ADR
TOT.LD.	40.0 PSF	SEQN- 7827
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SXB487 Z04

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

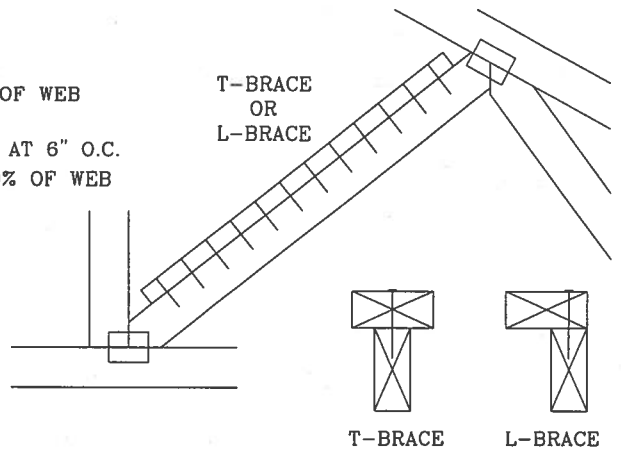
(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE

ATTACH WITH 16d NAILS AT 6" O.C.

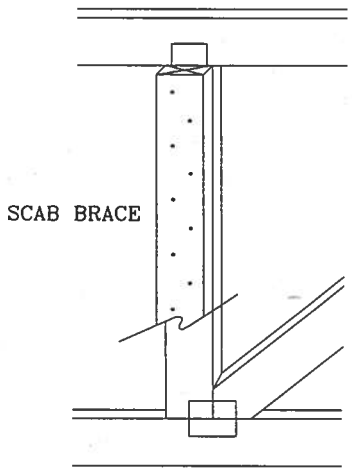
BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE.

ATTACH WITH 10d OR .128"x3" GUN NAILS AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640

ALPINE

ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

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TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	BRCLBSUB1103
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

ASCE 7-02: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

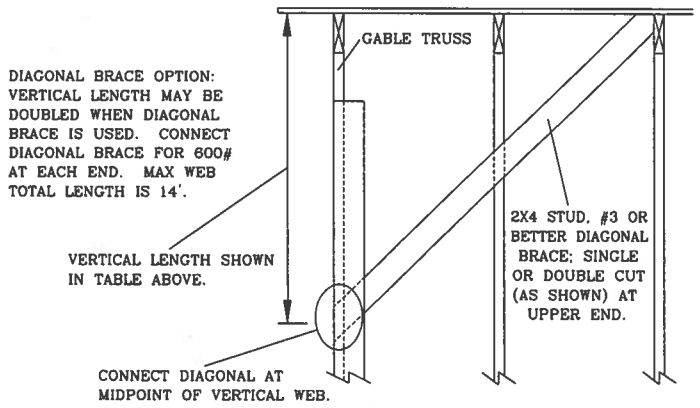
SOUTHERN PINE
#3
STUD
STANDARD

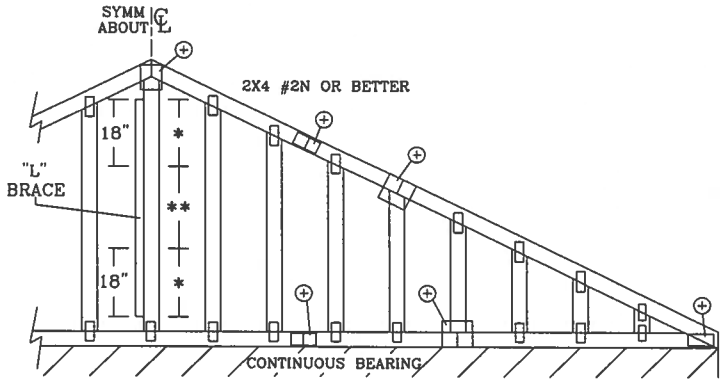
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

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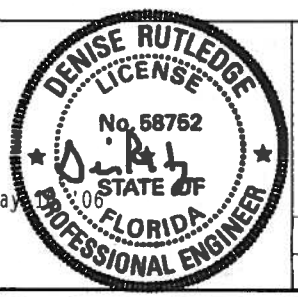


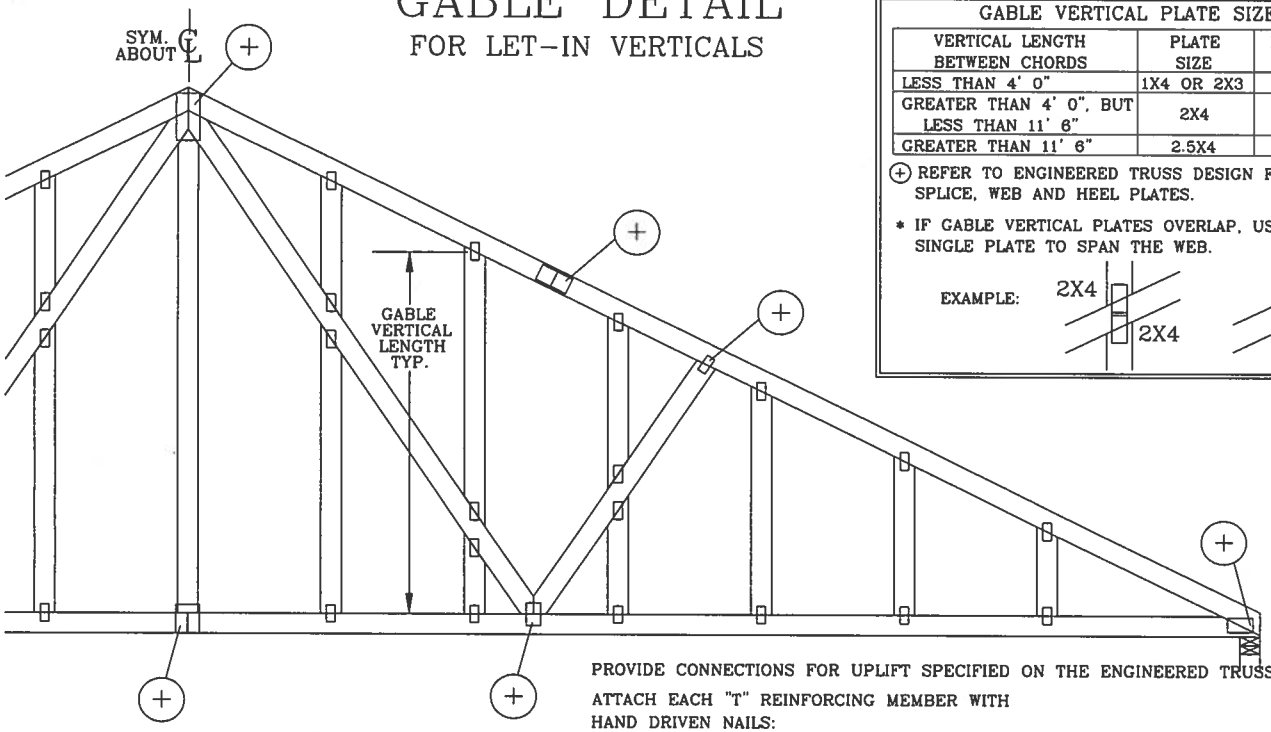
May 06

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB11015
DATE	04/15/05
DRWG	A11015EE0405
-ENG	

GABLE DETAIL
FOR LET-IN VERTICALS

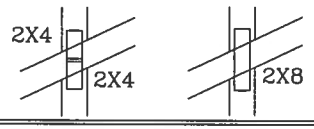


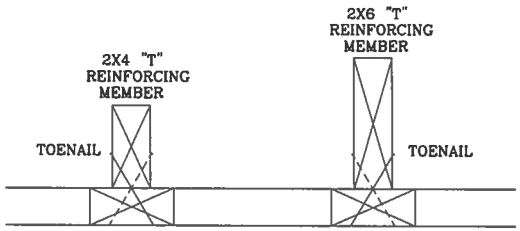
GABLE VERTICAL PLATE SIZES		
VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE:





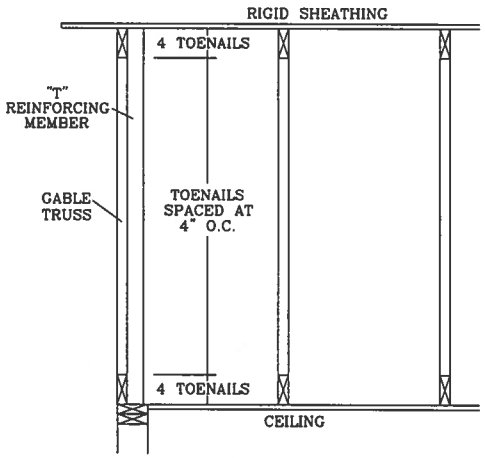
TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH 15 FT	2x4	10 %	10 %
	2x6	40 %	50 %
110 MPH 30 FT	2x4	10 %	10 %
	2x6	50 %	50 %
100 MPH 15 FT	2x4	10 %	10 %
	2x6	30 %	50 %
100 MPH 30 FT	2x4	10 %	10 %
	2x6	40 %	40 %
90 MPH 15 FT	2x4	20 %	10 %
	2x6	20 %	40 %
90 MPH 30 FT	2x4	10 %	10 %
	2x6	30 %	50 %
80 MPH 15 FT	2x4	10 %	20 %
	2x6	10 %	30 %
80 MPH 30 FT	2x4	20 %	10 %
	2x6	20 %	40 %
70 MPH 15 FT	2x4	0 %	20 %
	2x6	0 %	20 %
70 MPH 30 FT	2x4	10 %	20 %
	2x6	10 %	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH
HAND DRIVEN NAILS:
10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS
(4) 16d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AND BOTTOM CHORD.

GUN DRIVEN NAILS:
8d COMMON (0.131"X 2.5",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.
ASCE 7-93 GABLE DETAIL DRAWINGS
A11015EN1103, A10015EN1103, A09015EN1103, A08015EN1103, A07015EN1103
A11030EN1103, A10030EN1103, A09030EN1103, A08030EN1103, A07030EN1103
ASCE 7-98 GABLE DETAIL DRAWINGS
A13015EC1103, A12015EC1103, A11015EC1103, A10015EC1103, A08515EC1103
A13030EC1103, A12030EC1103, A11030EC1103, A10030EC1103, A08530EC1103
ASCE 7-02 GABLE DETAIL DRAWINGS
A13015EE0405, A12015EE0405, A11015EE0405, A10015EE0405, A08515EE0405,
A13030EE0405, A12030EE0405, A11030EE0405, A10030EE0405, A08530EE0405

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

ALPINE
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POMPANO BEACH, FLORIDA

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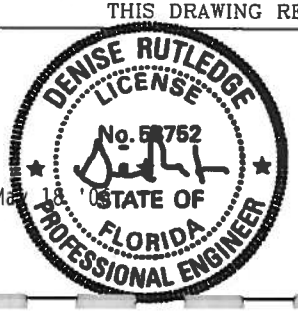


MAX TOT. LD. 60 PSF
DUR. FAC. ANY
MAX SPACING 24.0"

REF	LET-IN VERT
DATE	04/14/05
DRWG	GBLLETIN0405
-ENG	DLJ/KAR

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

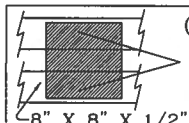
REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF



(4) 6d BOX (0.099"X 2.",MIN) NAILS.

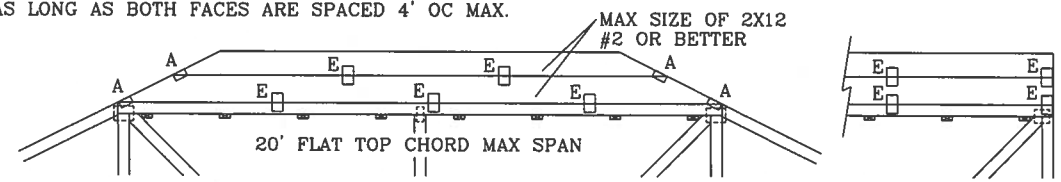
8" X 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES, ATTACH WITH (8) 6d BOX (0.099"X 2.",MIN) NAILS PER GUSSET.

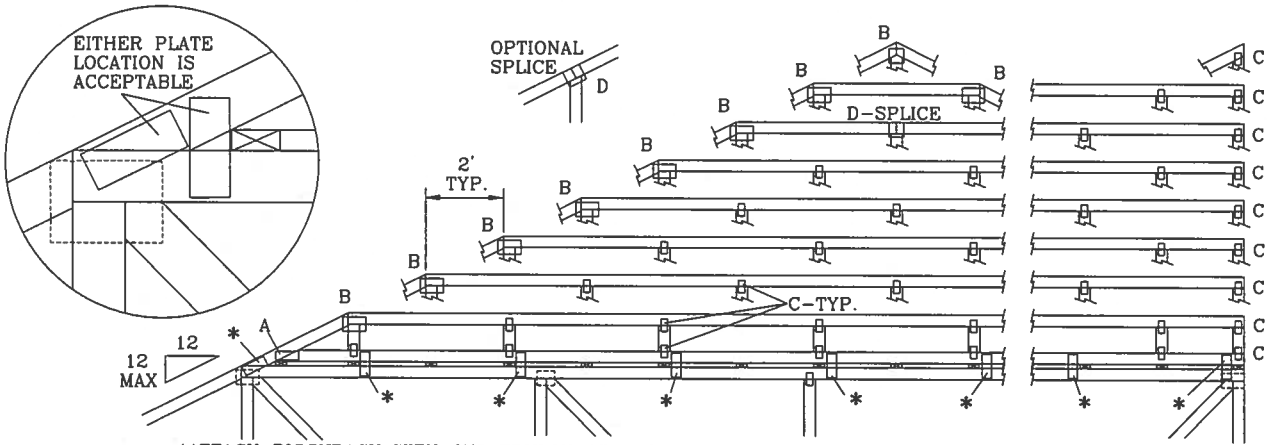
(4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.



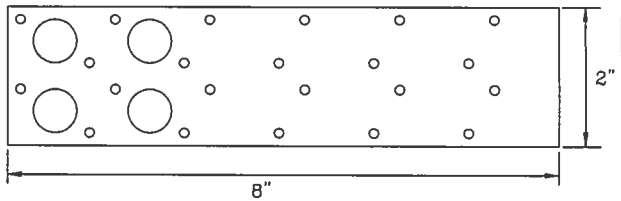


*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113"X 2.5",MIN) NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135"X 3.5",MIN) NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

ALPINE

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POMPANO BEACH, FLORIDA

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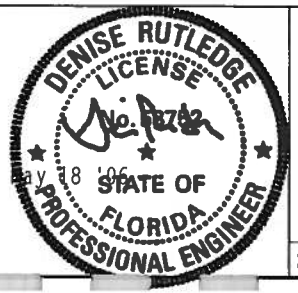


MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF	PIGGYBACK
DATE	04/14/05
DRWG	PIGBACKB0405
-ENG	DLJ/KAR

ASCE 7-02: 110 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	7' 1"	7' 1"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 1"	6' 1"	8' 3"	8' 3"	9' 6"	9' 6"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 5"	11' 5"	11' 5"	14' 0"	14' 0"
		STUD	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 2"	7' 3"	7' 5"	8' 7"	8' 10"	10' 3"	10' 6"	13' 5"	13' 10"	14' 0"	14' 0"
		#3	4' 1"	6' 8"	6' 8"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
		STUD	4' 1"	8' 0"	8' 0"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
		STANDARD	4' 1"	5' 8"	5' 8"	7' 6"	7' 6"	10' 1"	10' 1"	11' 8"	11' 8"	14' 0"	14' 0"
	SP DFL	#1	4' 7"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
		#2	4' 6"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	6' 10"	6' 10"	8' 7"	9' 0"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	6' 9"	6' 9"	8' 7"	8' 11"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 10"	5' 10"	7' 8"	7' 8"	10' 3"	10' 4"	11' 11"	11' 11"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 7"	8' 0"	8' 2"	9' 5"	9' 8"	11' 3"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 6"	6' 7"	6' 7"	8' 8"	8' 8"	11' 3"	11' 3"	13' 6"	13' 6"	14' 0"	14' 0"
	SP DFL	#1	5' 1"	8' 0"	8' 7"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 11"	8' 0"	8' 7"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	7' 11"	7' 11"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	7' 9"	7' 9"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 7"	6' 9"	6' 9"	8' 10"	8' 10"	11' 3"	11' 7"	13' 10"	13' 10"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

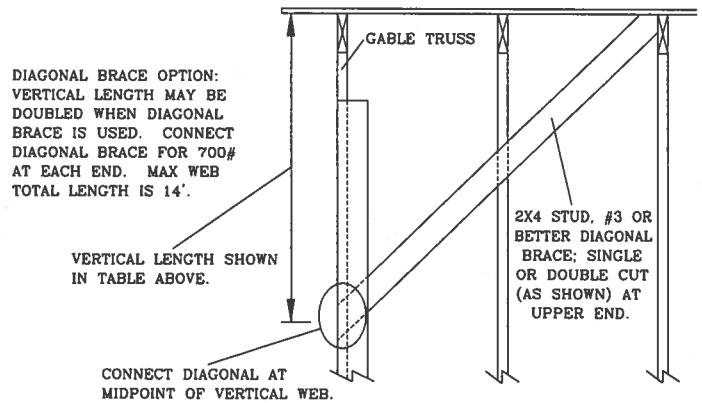
SOUTHERN PINE
#3
STUD
STANDARD

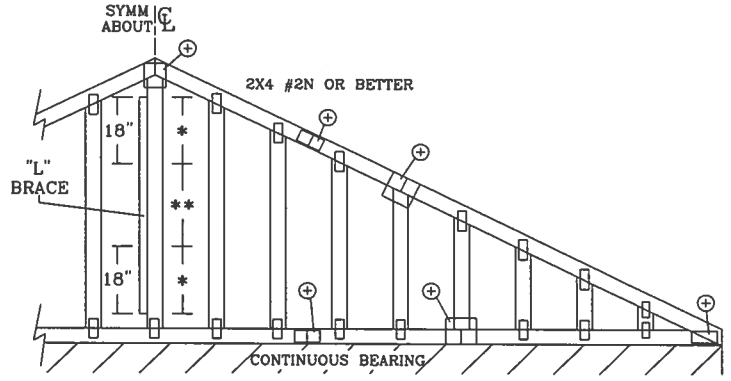
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W,K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB11030
DATE	04/14/05
DRWG	A11030EE0405
-ENG	

FORM 600A-2004 EnergyGauge® 4.0

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**HUNTER RESIDENCE**	Builder:	
Address:		Permitting Office:	Columbia
City, State:	**, FL**	Permit Number:	24686
Owner:	**MIKE HUNTER**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	3	
5.	Is this a worst case?	Yes	
6.	Conditioned floor area (ft²)	2240 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
a.	U-factor: (or Single or Double DEFAULT)	7a. Description (Dble Default) Area 158.0 ft²	
b.	SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 158.0 ft²	
8.	Floor types		
a.	Slab-On-Grade Edge Insulation	R=0.0, 204.0(p) ft	
b.	N/A		
c.	N/A		
9.	Wall types		
a.	Frame, Wood, Exterior	R=13.0, 1836.0 ft²	
b.	N/A		
c.	N/A		
d.	N/A		
e.	N/A		
10.	Ceiling types		
a.	Under Attic	R=30.0, 2800.0 ft²	
b.	N/A		
c.	N/A		
11.	Ducts		
a.	Sup: Con. Ret: Con. AH: Garage	Sup. R=6.0, 150.0 ft	
b.	N/A		

12.	Cooling systems		
a.	Central Unit	Cap: 42.0 kBtu/hr SEER: 13.00	
b.	N/A		
c.	N/A		
13.	Heating systems		
a.	Electric Heat Pump	Cap: 42.0 kBtu/hr HSPF: 7.00	
b.	N/A		
c.	N/A		
14.	Hot water systems		
a.	Electric Resistance	Cap: 40.0 gallons EF: 0.97	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.08

Total as-built points: 27745
Total base points: 32994

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Gary Gall

DATE: 4/25/06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	2240.0	20.04	8080.1	Double, Clear	NE	0.0	0.0	16.0	29.56	1.00	472.9
				Double, Clear	NE	0.0	0.0	12.0	29.56	1.00	354.7
				Double, Clear	NE	0.0	0.0	45.0	29.56	1.00	1330.0
				Double, Clear	NE	0.0	0.0	15.0	29.56	1.00	443.3
				Double, Clear	SE	0.0	0.0	16.0	42.75	1.00	684.0
				Double, Clear	SE	0.0	0.0	30.0	42.75	1.00	1282.6
				Double, Clear	SW	0.0	0.0	9.0	40.16	1.00	361.4
				Double, Clear	SW	0.0	0.0	15.0	40.16	1.00	602.4
				Double, Clear	NW	0.0	0.0	30.0	25.97	1.00	779.2
				As-Built Total:				**188.0**			**6310.5**
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1836.0	1.50		2754.0
Exterior	1836.0	1.70	3121.2								
Base Total:	**1836.0**		**3121.2**	**As-Built Total:**				**1836.0**			**2754.0**
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points
Adjacent	0.0	0.00	0.0	Exterior Wood				60.0	6.10		366.0
Exterior	140.0	6.10	854.0	Exterior Wood				80.0	6.10		488.0
Base Total:	**140.0**		**854.0**	**As-Built Total:**				**140.0**			**854.0**
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points
Under Attic	2240.0	1.73	3875.2	Under Attic			30.0	2800.0	1.73 X 1.00		4844.0
Base Total:	**2240.0**		**3875.2**	**As-Built Total:**				**2800.0**			**4844.0**
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Slab	204.0(p)	-37.0	-7548.0	Slab-On-Grade Edge Insulation			0.0	204.0(p	-41.20		-8404.8
Raised	0.0	0.00	0.0								
Base Total:			**-7548.0**	**As-Built Total:**				**204.0**			**-8404.8**
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points
	2240.0	10.21	22870.4					2240.0	10.21		22870.4

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL,	PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 31252.9			**Summer As-Built Points:**					**29228.1**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 42000 btuh ,SEER/EFF(13.0) Ducts:Con(S),Con(R),Gar(AH),R6.0(INS)					
			29228	1.00	(1.00 x 1.147 x 1.00)	0.263	1.000	8801.5
31252.9	**0.4266**	**13332.5**	**29228.1**	**1.00**	**1.147**	**0.263**	**1.000**	**8801.5**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	2240.0	12.74	5136.8	Double, Clear	NE	0.0	0.0	16.0	23.57	1.00	377.1	
				Double, Clear	NE	0.0	0.0	12.0	23.57	1.00	282.8	
				Double, Clear	NE	0.0	0.0	45.0	23.57	1.00	1060.6	
				Double, Clear	NE	0.0	0.0	15.0	23.57	1.00	353.5	
				Double, Clear	SE	0.0	0.0	16.0	14.71	1.00	235.3	
				Double, Clear	SE	0.0	0.0	30.0	14.71	1.00	441.2	
				Double, Clear	SW	0.0	0.0	9.0	16.74	1.00	150.6	
				Double, Clear	SW	0.0	0.0	15.0	16.74	1.00	251.0	
				Double, Clear	NW	0.0	0.0	30.0	24.30	1.00	728.9	
				As-Built Total:				**188.0**			**3881.0**	

WALL TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	13.0	1836.0	3.40	6242.4
Exterior	1836.0	3.70	6793.2					
Base Total:	**1836.0**		**6793.2**	**As-Built Total:**		**1836.0**		**6242.4**

DOOR TYPES	Area X	BWPM	= Points	Type	Area X	WPM	= Points
Adjacent	0.0	0.00	0.0	Exterior Wood	60.0	12.30	738.0
Exterior	140.0	12.30	1722.0	Exterior Wood	80.0	12.30	984.0
Base Total:	**140.0**		**1722.0**	**As-Built Total:**	**140.0**		**1722.0**

CEILING TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM X WCM	= Points
Under Attic	2240.0	2.05	4592.0	Under Attic	30.0	2800.0	2.05 X 1.00	5740.0
Base Total:	**2240.0**		**4592.0**	**As-Built Total:**		**2800.0**		**5740.0**

FLOOR TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Slab	204.0(p)	8.9	1815.6	Slab-On-Grade Edge Insulation	0.0	204.0(p	18.80	3835.2
Raised	0.0	0.00	0.0					
Base Total:			**1815.6**	**As-Built Total:**		**204.0**		**3835.2**

INFILTRATION	Area X	BWPM	= Points	Area X	WPM	= Points
	2240.0	-0.59	-1321.6	2240.0	-0.59	-1321.6

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL,	PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**18738.0**	**Winter As-Built Points:**					**20099.0**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 42000 btuh ,EFF(7.0) Ducts:Con(S),Con(R),Gar(AH),R6.0					
			20099.0	1.000	(1.000 x 1.169 x 1.00)	0.487	1.000	11445.8
18738.0	**0.6274**	**11756.2**	**20099.0**	**1.00**	**1.169**	**0.487**	**1.000**	**11445.8**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL, PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2635.00	7905.0	40.0	0.97	3	1.00	2499.18	1.00	7497.5	
				As-Built Total:						**7497.5**	

CODE COMPLIANCE STATUS

BASE					AS-BUILT				
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points		Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	
13332	**11756**	**7905**	**32994**		**8802**	**11446**	**7498**	**27745**	

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , FL,	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

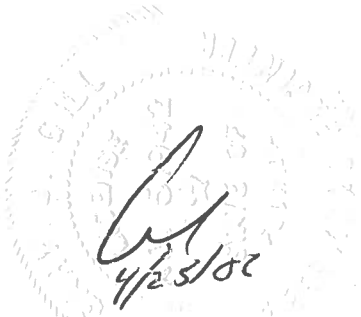
COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

GTC DESIGN GROUP

PROJECT NAME: HUNTER
PROJECT NUMBER: PF06-078

WIND LOAD AND STRUCTURAL CALCULATIONS
FOR

MIKE HUNTER
New Residence



GARY GILL, PE
GTC DESIGN GROUP, LLC
P.O. BOX 187
LIVE OAK, FL 32064
386-362-3678
386-362-6133 (FAX)
AUTH. # 9461

Project name: MIKE HUNTER (HOUSE)
Project: PF06-078
Client MIKE HUNT
Calculations: Gary Gill, PE
Date: 4/25/2006

Design Basis

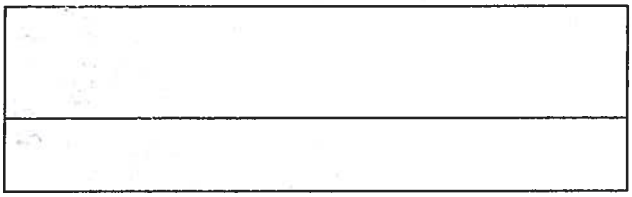
Design Loads

Wind Load	110	
Floor Live Load		
	Sleep Areas =	30 psf
	All Others =	40 psf
Floor Dead Load		10 psf
Wall Dead Load		10 psf
Roof Live Load		20 psf
Roof Dead Load		10 psf

Load Combinations

DL + LL(floor) + LL (roof)
DL + LL(floor) +WL
DL + WL
Wind load

Exposure B



Building Information

Shape	Rectangle
Length	86 ft
Width	48 ft
Type	1 storey sog

References

2004 Florida Building Code
ASCE 7-98 Minimum Design Loads for Buildings and Other Structures
AITC Timber Construction Manual

WIND98 v3-02

Wind Load Design per ASCE 7-98

Description: MIKE HUNTER (HOUSE)
Analysis by: Gary Gill

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	30.256	Deg
Type of Roof	Gabled	
Kd (Directonality Factor)	0.85	
Eave Height (Eht)	9.00	ft
Ridge Height (RHt)	21.00	ft
Mean Roof Height (Ht)	15.00	ft
Width Perp. To Wind Dir (B)	86.00	ft
Width Paral. To Wind Dir (L)	48.00	ft
Damping Ratio (beta)	0.02	

Red values should be changed only through "Main Menu"

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.31
Flexible Structure	No

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((Min(B,L)+Ht)/Lzm)^0.63))^0.5	0.9013	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8668	
Gust Factor Summary			
G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85	

WIND98 v3-02

Wind Load Design per ASCE 7-98

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev	Kz	Kzt	qz	Pressure (lb/ft^2)	
				Windward Wall*	
ft			lb/ft^2	+GCpi	-GCpi
21	0.63	1.00	16.66	8.60	14.05
20	0.62	1.00	16.43	8.45	13.90
15	0.57	1.00	15.13	7.57	13.01

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

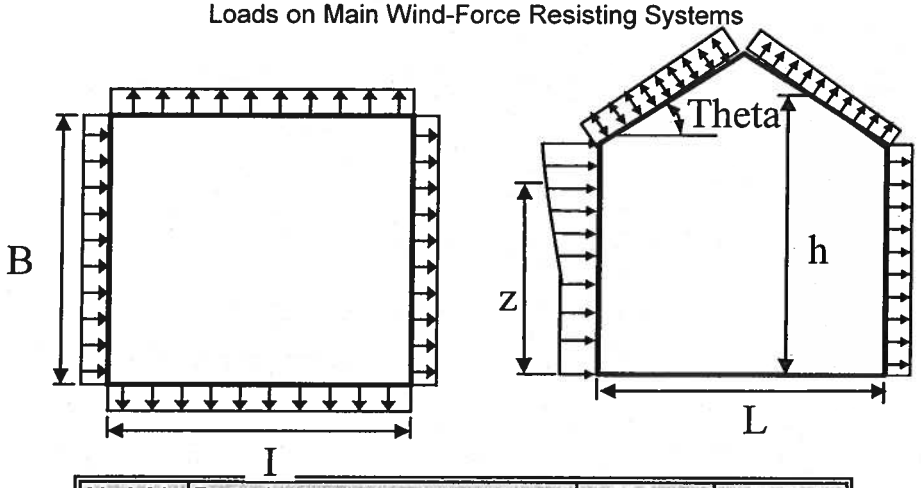
Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

WIND98 v3-02

Wind Load Design per ASCE 7-98

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(Ht/zg)^(2/Alpha)	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*I*Kh*Kht*Kd	15.13	psf
Khcc	Comp & Clad: Table 6-5 Case 2	0.70	
Qhcc	.00256*V^2*I*Khcc*Kht*Kd	18.45	psf

Wall Pressure Coefficients, Cp	
Surface	**Cp**
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.8

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Calculations for Wind Normal to 86 ft Face *Additional Runs may be req'd for other wind directions*	**Cp**	**Pressure (psf)** **+GCpi**	**-GCpi**
Leeward Walls (Wind Dir Normal to 86 ft wall)	-0.50	-9.15	-3.71
Side Walls	-0.70	-11.73	-6.28
Roof - Wind Normal to Ridge (Theta>=10) - for Wind Normal to 86 ft face			
Windward - Max Negative	-0.19	-5.20	0.25
Windward - Max Positive	0.28	0.88	6.33
Leeward Normal to Ridge	-0.60	-10.44	-4.99
Overhang Top (Windward)	-0.19	-2.47	-2.47
Overhang Top (Leeward)	-0.60	-7.72	-7.72
Overhang Bottom (Applicable on Windward only)	0.80	10.29	10.29
Roof - Wind Parallel to Ridge (All Theta) - for Wind Normal to 86 ft face			
Dist from Windward Edge: 0 ft to 7.5 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 7.5 ft to 15 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 15 ft to 30 ft	-0.50	-9.15	-3.71
Dist from Windward Edge: > 30 ft	-0.30	-6.58	-1.13

* Horizontal distance from windward edge

WIND98 v3-02

Wind Load Design per ASCE 7-98

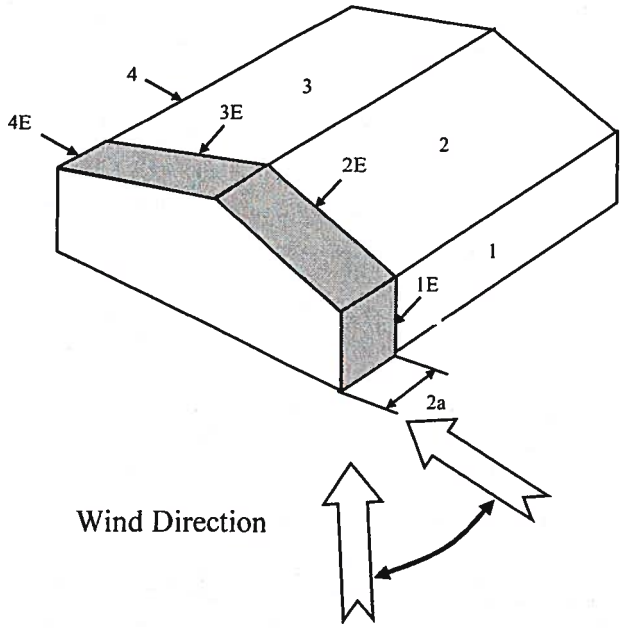
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	15.13

Case A						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	0.56	0.18	-0.18	15.13	5.75	11.20
2	0.21	0.18	-0.18	15.13	0.45	5.90
3	-0.43	0.18	-0.18	15.13	-9.23	-3.78
4	-0.37	0.18	-0.18	15.13	-8.32	-2.88
5	0.00	0.18	-0.18	15.13	-2.72	2.72
6	0.00	0.18	-0.18	15.13	-2.72	2.72
1E	0.69	0.18	-0.18	15.13	7.72	13.16
2E	0.27	0.18	-0.18	15.13	1.36	6.81
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
5E	0.00	0.18	-0.18	15.13	-2.72	2.72
6E	0.00	0.18	-0.18	15.13	-2.72	2.72

* p = qh * (GCpf - GCpi)



WIND98 v3-02

Wind Load Design per ASCE 7-98

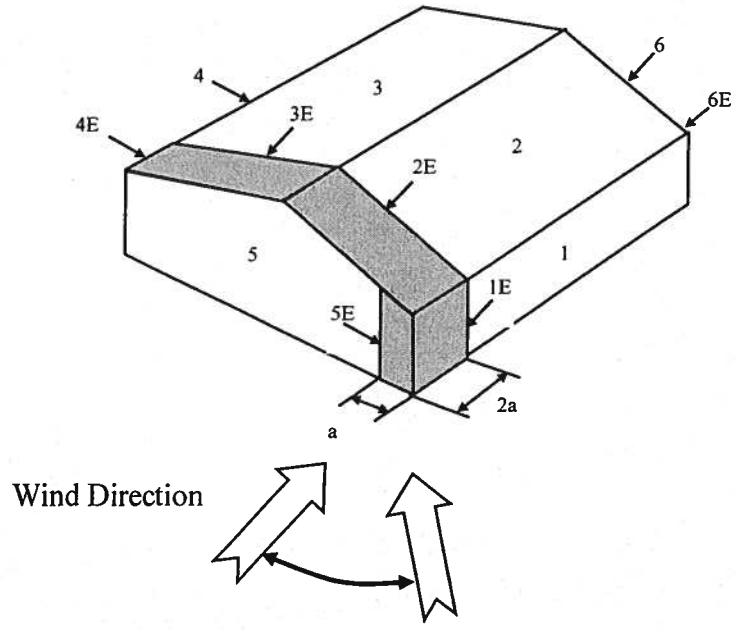
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	15.13

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	15.13	-9.53	-4.09
2	-0.69	0.18	-0.18	15.13	-13.16	-7.72
3	-0.37	0.18	-0.18	15.13	-8.32	-2.88
4	-0.45	0.18	-0.18	15.13	-9.53	-4.09
5	0.40	0.18	-0.18	15.13	3.33	8.78
6	-0.29	0.18	-0.18	15.13	-7.11	-1.66
1E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
2E	-1.07	0.18	-0.18	15.13	-18.92	-13.47
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
5E	0.61	0.18	-0.18	15.13	6.51	11.95
6E	-0.43	0.18	-0.18	15.13	-9.23	-3.78

* p = qh * (GCpf - GCpi)

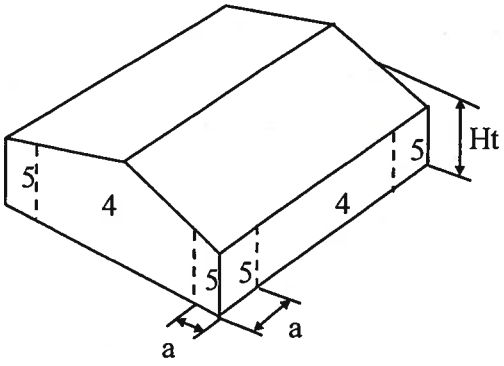


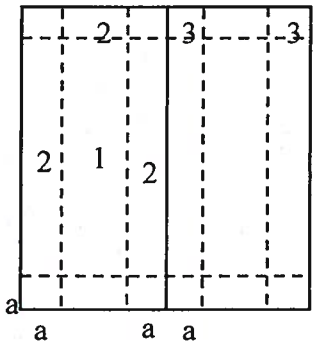
WIND98 v3-02

Wind Load Design per ASCE 7-98

Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft





Gabled Roof

10 < Theta <= 45

a = 4.8 ==> 4.80 ft

Component	Width (ft)	Span (ft)	Area (ft^2)	Zone	GCp		Wind Press (lb/ft^2)	
					Max	Min	Max	Min
ROOF	10	1	10.00	1	0.90	-1.00	19.92	-21.77
Walls	10	1	10.00	4	1.00	-1.10	21.77	-23.61
roof edge	10	1	10.00	2	0.90	-1.20	19.92	-25.46
Wall edge	10	1	10.00	5	1.00	-1.40	21.77	-29.15
Roof overhang	10	1	10.00	2H	0.90	-2.00	16.60	-36.89

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Shearwall Design - N/S Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)	7	Roof Dia	13.89
Vertical Roof height	12.00	Length of Building	86
2nd Floor height	0	Width of Building	48
1st Floor height	9		

Wind Pressure per ASCE 7- Normal to surface Case A

Windward Roof - Surface 2	5.90	psf	Wall - Leeward	11.2 psf
Leeward Roof - Surface 3	-3.78	psf	Wall - Surface 4	-2.88 psf
			Total Wall	14.08 psf

Horizontal loads from wind perpendicular to ridge (N / S)

Roof Pressure (interior)

Windward Roof Horz.(psf)	2.97
Leeward Roof Horz.(psf)	-1.90
Total	4.88
Tributary area (roof)	516.00
Roof shear values	**2516.78**

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	14.08
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	**0.00**

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	14.08
Tributary area to each Shearwall (sf)	315.00
Wall shear values to each shearwall	**4435.20**

Total shear to top of 2nd floor (lb) per wall (actual)	**0.00**
Total shear to top of 1st floor (lb) per wall (actual)	**6951.98**

2nd Floor shearwalls	Shearwall column #		
Number of shearwall segments in each column			
Shearwall #1 length			
Shearwall #2 length			
Shearwall #3 length			
Lateral load on shear wall column (lbs)			
Percent Full-Height Sheathing			
Shear capacity adjustment			
Shearwall rating (plf) w/ 1.4			
Design Shear Capacity			
Stress Ratio			
uplift at shear ends			
shear and uplift between holddown, v and u			

1st Floor shearwall (ft)

Number of shearwall segments in each column	1	1	
Full wall length	32	32	
Shearwall #1 length	22	23	
Shearwall #2 length	0	0	
Wall height ratio (h/b)	0.41	0.39	
Rigidities of shearwalls	7.72	8.10	
Lateral load on shearwall column (lbs) based on rigidity	**3390.92**	**3561.06**	
Percent Full-Height Sheathing Shearwall #1	68.75%	71.88%	
Shear capacity adjustment	0.77	0.69	
Shearwall rating (plf) w/ 1.4 increase for wind	483	483	
Design Shear Capacity	**8182.02**	**7665.21**	
Stress Ratio	0.41	0.46	
uplift at shear ends	**1801.55**	**2019.50**	
shear and uplift between holddown, v and u	**200.17**	**224.39**	

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

Shearwall Design - E/W Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)	7	Roof Dia	13.89
Vertical Roof height	12.00	Length of Building	86
2nd Floor height	0	Width of Building	48
1st Floor height	9		

Wind Pressure per ASCE 7- Normal to surface Case B

Windward Wall - Surface 5	8.78	psf
Leeward Wall - Surface 6	-1.66	psf
Total Wall	10.44	

Horizontal loads from parallel to ridge (N/S)

Roof Pressure (interior)	
Windward Roof Horz.(psf)	8.78
Leeward Roof Horz.(psf)	-1.66
Total	10.44
Tributary area (roof) to each shearwall (sf)	144.00
Roof shear values to each shearwall	**1503.36**

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	0
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	**0.00**

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	10.44
Tributary area to each Shearwall (sf)	144.00
Wall shear values to each shearwall	**1503.36**

Total shear to top of 2nd floor (lb) per wall (actual)	**0.00**
Total shear to top of 1st floor (lb) per wall (actual)	**3006.72**

2nd Floor shearwalls	Shearwall column #		
Number of shearwall segments in each column			
Full wall length			
Shearwall #1 length			
Shearwall #2 length			
Shearwall #3 length			
Lateral load on shear wall column (lbs)			
Percent Full-Height Sheathing			
Shear capacity adjustment			
Shearwall rating (plf) w/ 1.4			
Design Shear Capacity			
Stress Ratio			
uplift at shear ends			
shear and uplift between holddown, v and u			

1st Floor shearwall (ft)

Number of shearwall segments in each column	A	B	
Full wall length	70	70	
Shearwall #1 length	48	49	
Shearwall #2 length	0	0	
Wall height ratio (h/b)	0.19	0.18	
Rigidities of shearwalls	17.57	17.95	
Lateral load on shearwall column (lbs) based on rigidity	**1487.51**	**1519.21**	
Percent Full-Height Sheathing Shearwall #1	68.57%	70.00%	
Shear capacity adjustment	0.69	0.69	
Shearwall rating (plf) w/ 1.4 increase for wind	483	483	
Design Shear Capacity	**15996.96**	**16330.23**	
Stress Ratio	0.09	0.09	
uplift at shear ends	**404.21**	**404.40**	
shear and uplift between holddown, v and u	**44.91**	**44.93**	

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

Project name: MIKE HUNTER (GARAGE)
Project: PF06-078
Client MIKE HUNT
Calculations: Gary Gill, PE
Date: 4/25/2006

Design Basis

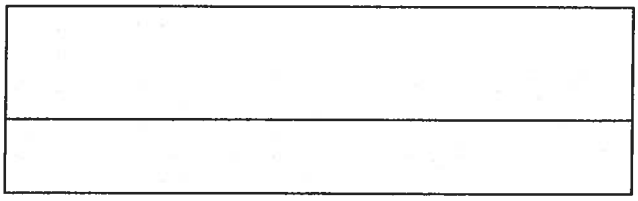
Design Loads

Wind Load	110	
Floor Live Load		
	Sleep Areas =	30 psf
	All Others =	40 psf
Floor Dead Load		10 psf
Wall Dead Load		10 psf
Roof Live Load		20 psf
Roof Dead Load		10 psf

Load Combinations

DL + LL(floor) + LL (roof)
DL + LL(floor) +WL
DL + WL
Wind load

Exposure B



Building Information

Shape	Rectangle
Length	24 ft
Width	24 ft
Type	1 storey sog

References

2004 Florida Building Code
ASCE 7-98 Minimum Design Loads for Buildings and Other Structures
AITC Timber Construction Manual

WIND98 v3-02

Wind Load Design per ASCE 7-98

Description: MIKE HUNTER (GARAGE)
Analysis by: Gary Gill

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	30.256	Deg
Type of Roof	Gabled	
Kd (Directonality Factor)	0.85	
Eave Height (Eht)	9.00	ft
Ridge Height (RHt)	17.17	ft
Mean Roof Height (Ht)	13.08	ft
Width Perp. To Wind Dir (B)	24.00	ft
Width Paral. To Wind Dir (L)	24.00	ft
Damping Ratio (beta)	0.02	

Red values should be changed only through "Main Menu"

Calculated Parameters		
Type of Structure		
Height/Least Horizontal Dim	0.55	
Flexible Structure	No	

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((Min(B,L)+Ht)/Lzm)^0.63))^0.5	0.9264	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8815	
Gust Factor Summary			
G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85	

WIND98 v3-02

Wind Load Design per ASCE 7-98

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev	Kz	Kzt	qz	Pressure (lb/ft^2)	
				Windward Wall*	
ft			lb/ft^2	+GCpi	-GCpi
17.167	0.60	1.00	15.73	7.97	13.42
15	0.57	1.00	15.13	7.57	13.01

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

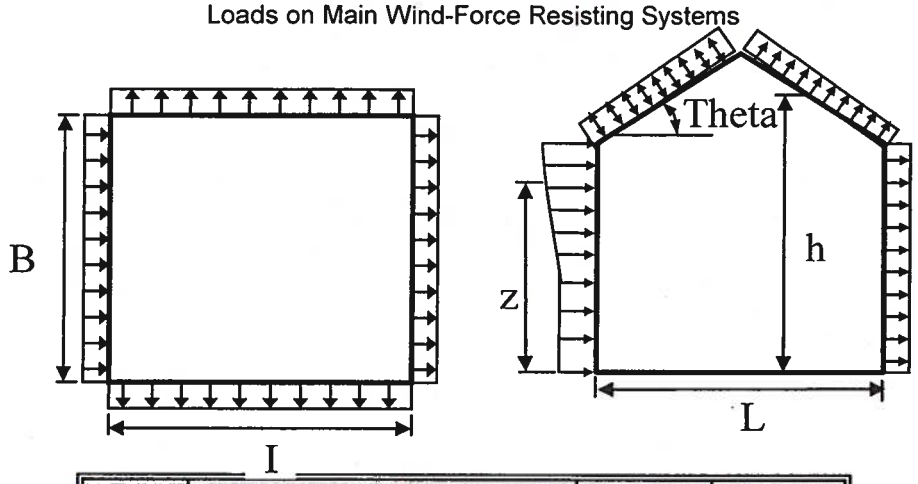
Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

WIND98 v3-02

Wind Load Design per ASCE 7-98

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(15/zg)^(2/Alpha)	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*I*Kh*Kht*Kd	15.13	psf
Khcc	Comp & Clad: Table 6-5 Case 2	0.70	
Qhcc	.00256*V^2*I*Khcc*Kht*Kd	18.45	psf

Wall Pressure Coefficients, Cp	
Surface	**Cp**
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.8

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Calculations for Wind Normal to 24 ft Face *Additional Runs may be req'd for other wind directions*	**Cp**	**Pressure (psf)** **+GCpi**	**-GCpi**
Leeward Walls (Wind Dir Normal to 24 ft wall)	-0.50	-9.15	-3.71
Side Walls	-0.70	-11.73	-6.28
Roof - Wind Normal to Ridge (Theta>=10) - for Wind Normal to 24 ft face			
Windward - Max Negative	-0.21	-5.41	0.04
Windward - Max Positive	0.20	-0.09	5.36
Leeward Normal to Ridge	-0.60	-10.44	-4.99
Overhang Top (Windward)	-0.21	-2.68	-2.68
Overhang Top (Leeward)	-0.60	-7.72	-7.72
Overhang Bottom (Applicable on Windward only)	0.80	10.29	10.29
Roof - Wind Parallel to Ridge (All Theta) - for Wind Normal to 24 ft face			
Dist from Windward Edge: 0 ft to 6.54 ft	-0.94	-14.76	-9.32
Dist from Windward Edge: 6.54 ft to 13.08 ft	-0.88	-14.07	-8.62

* Horizontal distance from windward edge

WIND98 v3-02

Wind Load Design per ASCE 7-98

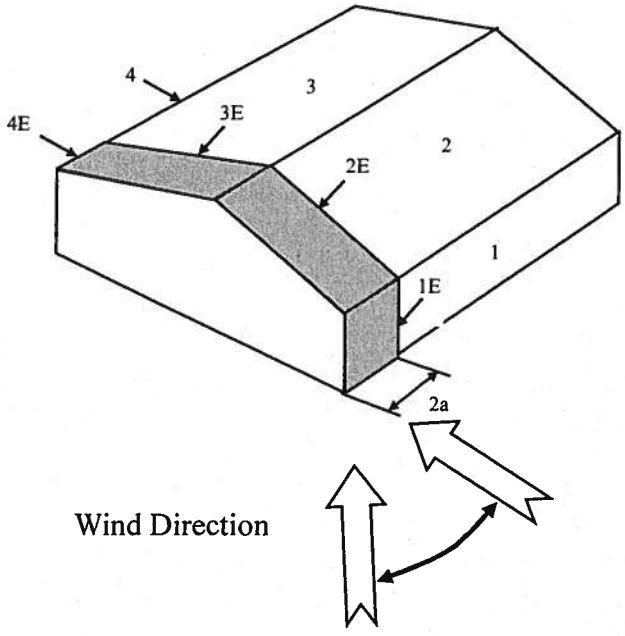
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	15.13

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.56	0.18	-0.18	15.13	5.75	11.20
2	0.21	0.18	-0.18	15.13	0.45	5.90
3	-0.43	0.18	-0.18	15.13	-9.23	-3.78
4	-0.37	0.18	-0.18	15.13	-8.32	-2.88
5	0.00	0.18	-0.18	15.13	-2.72	2.72
6	0.00	0.18	-0.18	15.13	-2.72	2.72
1E	0.69	0.18	-0.18	15.13	7.72	13.16
2E	0.27	0.18	-0.18	15.13	1.36	6.81
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
5E	0.00	0.18	-0.18	15.13	-2.72	2.72
6E	0.00	0.18	-0.18	15.13	-2.72	2.72

* p = qh * (GCpf - GCpi)



WIND98 v3-02

Wind Load Design per ASCE 7-98

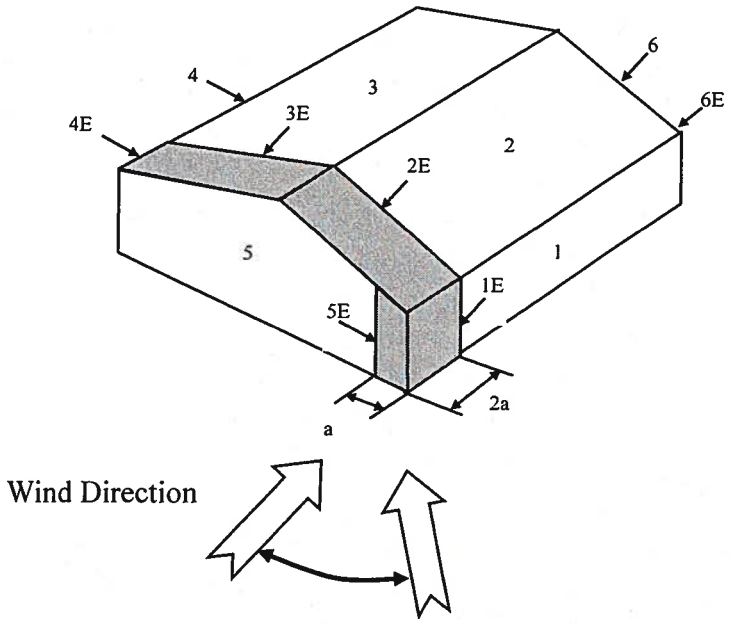
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	15.13

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	15.13	-9.53	-4.09
2	-0.69	0.18	-0.18	15.13	-13.16	-7.72
3	-0.37	0.18	-0.18	15.13	-8.32	-2.88
4	-0.45	0.18	-0.18	15.13	-9.53	-4.09
5	0.40	0.18	-0.18	15.13	3.33	8.78
6	-0.29	0.18	-0.18	15.13	-7.11	-1.66
1E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
2E	-1.07	0.18	-0.18	15.13	-18.92	-13.47
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
5E	0.61	0.18	-0.18	15.13	6.51	11.95
6E	-0.43	0.18	-0.18	15.13	-9.23	-3.78

* p = qh * (GCpf - GCpi)

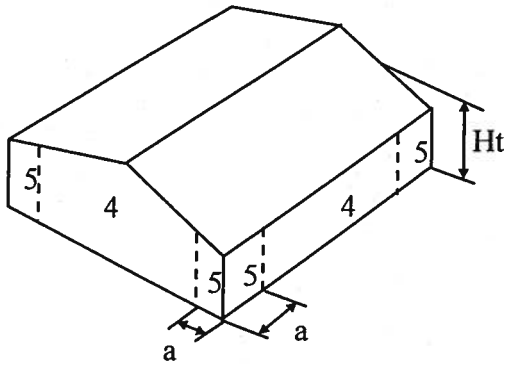


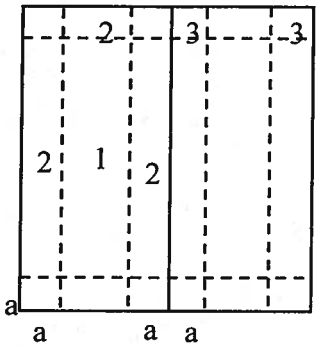
WIND98 v3-02

Wind Load Design per ASCE 7-98

Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft





Gabled Roof

10 < Theta <= 45

a = 2.4 ==> 3.00 ft

Component	Width (ft)	Span (ft)	Area (ft^2)	Zone	GCp		Wind Press (lb/ft^2)	
					Max	Min	Max	Min
ROOF	10	1	10.00	1	0.90	-1.00	19.92	-21.77
Walls	10	1	10.00	4	1.00	-1.10	21.77	-23.61
roof edge	10	1	10.00	2	0.90	-1.20	19.92	-25.46
Wall edge	10	1	10.00	5	1.00	-1.40	21.77	-29.15
Roof overhang	10	1	10.00	2H	0.90	-2.00	16.60	-36.89

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Shearwall Design - N/S Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)		7	Roof Dia	13.89
Vertical Roof height		8.17	Length of Building	24
2nd Floor height	0		Width of Building	24
1st Floor height	9			

Wind Pressure per ASCE 7- Normal to surface Case A

Windward Roof - Surface 2	5.90	psf	Wall - Leeward	11.2 psf
Leeward Roof - Surface 3	-3.78	psf	Wall - Surface 4	-2.88 psf
			Total Wall	14.08 psf

Horizontal loads from wind perpendicular to ridge (N / S)

Roof Pressure (interior)

Windward Roof Horz.(psf)	2.97
Leeward Roof Horz.(psf)	-1.90
Total	4.88
Tributary area (roof)	98.00
Roof shear values	**477.99**

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	14.08
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	**0.00**

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	14.08
Tributary area to each Shearwall (sf)	108.00
Wall shear values to each shearwall	**1520.64**

Total shear to top of 2nd floor (lb) per wall (actual)	**0.00**
Total shear to top of 1st floor (lb) per wall (actual)	**1998.63**

2nd Floor shearwalls	Shearwall column #		
Number of shearwall segments in each column			
Shearwall #1 length			
Shearwall #2 length			
Shearwall #3 length			
Lateral load on shear wall column (lbs)			
Percent Full-Height Sheathing			
Shear capacity adjustment			
Shearwall rating (plf) w/ 1.4			
Design Shear Capacity			
Stress Ratio			
uplift at shear ends			
shear and uplift between holddown, v and u			

1st Floor shearwall (ft)

Number of shearwall segments in each column	1	1	
Full wall length	24	24	
Shearwall #1 length	21	6	
Shearwall #2 length	0	0	
Wall height ratio (h/b)	0.43	1.50	
Rigidities of shearwalls	7.33	1.27	
Lateral load on shearwall column (lbs) based on rigidity	**1703.48**	**295.15**	
Percent Full-Height Sheathing Shearwall #1	87.50%	25.00%	
Shear capacity adjustment	0.87	0.42	
Shearwall rating (plf) w/ 1.4 increase for wind	483	483	
Design Shear Capacity	**8824.41**	**1217.16**	
Stress Ratio	0.19	0.24	
uplift at shear ends	**839.15**	**1054.10**	
shear and uplift between holddown, v and u	**93.24**	**117.12**	

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

Shearwall Design - E/W Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)		7	Roof Dia	13.89
Vertical Roof height		8.17	Length of Building	24
2nd Floor height	0		Width of Building	24
1st Floor height	9			

Wind Pressure per ASCE 7- Normal to surface Case B

Windward Wall - Surface 5	8.78	psf
Leeward Wall - Surface 6	-1.66	psf
Total Wall	10.44	

Horizontal loads from parallel to ridge (N/S)

Roof Pressure (interior)	
Windward Roof Horz.(psf)	8.78
Leeward Roof Horz.(psf)	-1.66
Total	10.44
Tributary area (roof) to each shearwall (sf)	49.00
Roof shear values to each shearwall	**511.56**

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	0
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	**0.00**

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	10.44
Tributary area to each Shearwall (sf)	108.00
Wall shear values to each shearwall	**1127.52**

Total shear to top of 2nd floor (lb) per wall (actual)	**0.00**
Total shear to top of 1st floor (lb) per wall (actual)	**1639.08**

2nd Floor shearwalls	Shearwall column #		
Number of shearwall segments in each column			
Full wall length			
Shearwall #1 length			
Shearwall #2 length			
Shearwall #3 length			
Lateral load on shear wall column (lbs)			
Percent Full-Height Sheathing			
Shear capacity adjustment			
Shearwall rating (plf) w/ 1.4			
Design Shear Capacity			
Stress Ratio			
uplift at shear ends			
shear and uplift between holddown, v and u			

1st Floor shearwall (ft)

Number of shearwall segments in each column	A	B	
Full wall length	24	24	
Shearwall #1 length	18	24	
Shearwall #2 length	0	0	
Wall height ratio (h/b)	0.50	0.38	
Rigidities of shearwalls	6.15	8.49	
Lateral load on shearwall column (lbs) based on rigidity	**688.76**	**950.32**	
Percent Full-Height Sheathing Shearwall #1	75.00%	100.00%	
Shear capacity adjustment	0.83	1	
Shearwall rating (plf) w/ 1.4 increase for wind	483	483	
Design Shear Capacity	**7216.02**	**11592.00**	
Stress Ratio	0.10	0.08	
uplift at shear ends	**414.91**	**356.37**	
shear and uplift between holddown, v and u	**46.10**	**39.60**	

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465