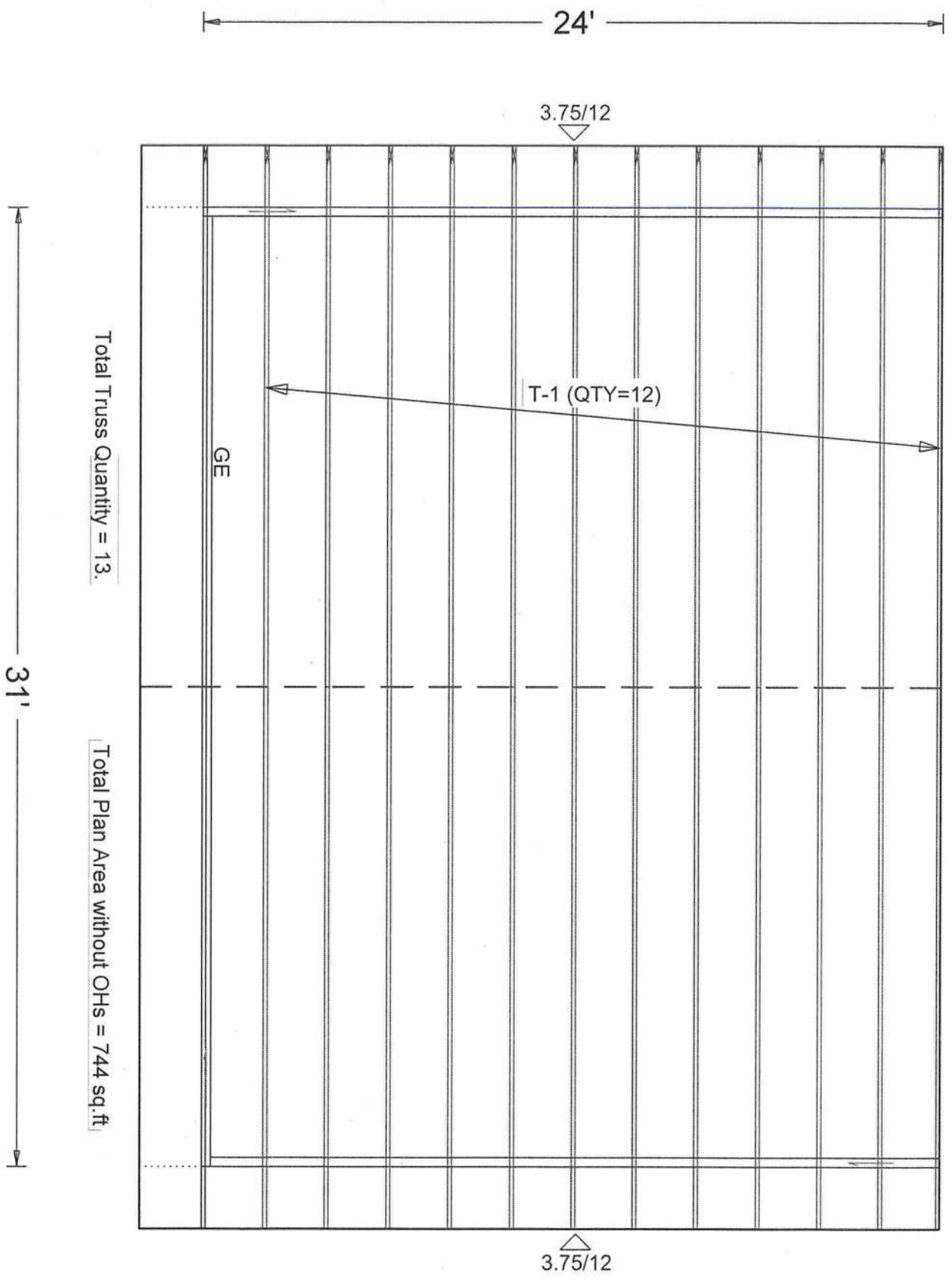




Total Truss Quantity = 13.

Total Plan Area without OHs = 744 sq.ft.

73

Job Name: Laskey Res tree damage
 Customer: Trademark Const Group
 Designer: Curt Burlingame
 PlanName:
 Created : 11-20-2023
 SemRef# : B58167a

JOB NO:
 B58167a

PAGE NO:
 1 OF 1

Alpine, an ITW Company
 155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025
 Phone: (800)755-6001
 www.alpineitw.com



This item has been digitally signed by William H. Krick on the date adjacent to the seal.
 Printed copies of this document are not considered signed and sealed and the signature must
 be verified on any electronic copies.
 COA #0 278
 Florida Certificate of Product Approval #FL 1999
 11/21/2023



Site Information:	
Customer: Seminole Trusses, Inc.	Job Number: B58167a
Job Description: Laskey Res tree damage	
Address: 168 SW Audrey Way, Lake City, FL	
Page 1:	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.01.03A
Wind Standard: ASCE 7-16	JRef #: 1XUY8570001
Building Type: Closed	Design Loading (psf): 37.00
Wind Speed (mph): 130	

This package contains general notes pages, 2 truss drawing(s) and 5 detail(s).

Item	Drawing Number	Truss
1	324.23.1026.11560	GE
3	A14015ENC160118	
5	PB160160118	
7	REPCHRD1014	

Item	Drawing Number	Truss
2	324.23.1026.17770	T-1
4	GBLLETIN0118	
6	PB180160118	

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
 BCLL = Bottom Chord standard design Live Load in pounds per square foot.
 CL = Certified lumber.
 Des Ld = total of TCCL, TCCL, BCLL and BCDL Design Load in pounds per square foot.
 FRT = Fire Retardant Treated lumber.
 FRT-DB = D-Blaze Fire Retardant Treated lumber.
 FRT-DC = Dricon Fire Retardant Treated lumber.
 FRT-FP = FirePRO Fire Retardant Treated lumber.
 FRT-FL = FlamePRO Fire Retardant Treated lumber.
 FRT-FT = FlameTech Fire Retardant Treated lumber.
 FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
 FRT-PR = ProWood Fire Retardant Treated lumber.
 g = green lumber.
 HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
 HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
 HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
 Ic = Incised lumber.
 FJ = Finger Jointed lumber.
 L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
 L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
 Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
 Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.
 Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.
 Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.
 NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
 PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
 PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds.
 PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
 PP = Panel Point.
 R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
 -R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
 Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
 RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
 Rlw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
 TCCL = Top Chord standard design Dead Load in pounds per square foot.
 TCCL = Top Chord standard design Live Load in pounds per square foot.
 U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
 VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
 VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total Load, including creep adjustment.
 VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
 VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total Load, including creep adjustment.
 W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
 Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catocin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.; 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

*****WARNING*** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**

*****IMPORTANT*** FINISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**

Contractors require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Components Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the details. Unless noted otherwise, unless noted otherwise. Refer to drawings 160A-7 for standard plate positions. Refer to Job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/APA 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/APA 1 Sec.2

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbacancomponents.com; ICC: iccsafe.org; AWC: awc.org

Florida Certificate of Product Approval #FL 1999



Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Apply purins to any chords above or below fillers at 24" OC unless shown otherwise above.

Chord	Spacing (in oc)	Start (ft)	End (ft)
BC	84	0.15	30.85
TC	38	15.50	33.04
TC	38	-2.04	15.50

in lieu of structural panels or rigid ceiling use purlins

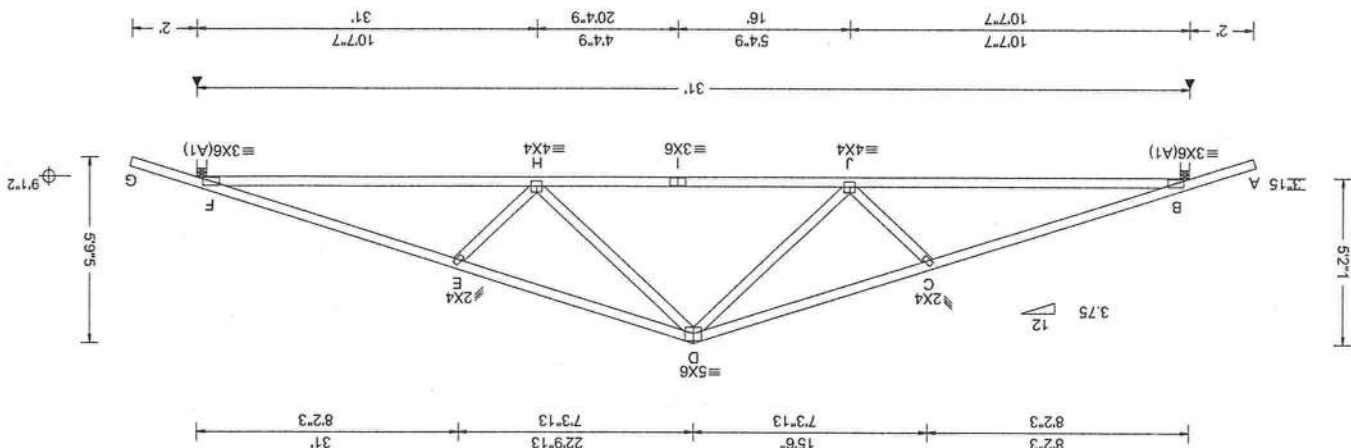
Plates sized for a minimum of 3.50 sq.in./piece.

Plating Notes

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Lumber

Loading Criteria (psf) Wind Criteria TCDL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCCL: 10.00 SoftL: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt NA Mean Height: 15.00 ft TCDL: 0.0 psf BCDL: 0.0 psf C&C Dist: 3.10 ft Loc. from endwall: Any GCpf: 0.18 Wind Duration: 1.60	Wave Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(s)/0(s) Plate Type(s):	VIEW Ver: 21.01.03A.0805.15 Def/CSI Criteria PP Deletion in loc L/del L/# VERT(L): 0.193 H 999 360 VERT(C/L): 0.351 H 999 240 HORZ(L/L): 0.056 F - - HORZ(T/L): 0.102 F - - Creep Factor: 2.0 Max TC CSI: 0.984 Max BC CSI: 0.981 Max Web CSI: 0.291	Non-Gravity Loc R+ /R- /Rh /Rw /U /RL Gravity B 1278 -/- /- /655 /411 /88 B 1278 -/- /- /655 /411 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.6 B Brg Wid = 3.5 Min Req = 1.6 Bearings B & F Fcbrp = 425psf Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens.Comp.



FROM: CVB SEQN: 19454 COMM Qty: 12 Py: 1 Job Number: B58167a Laskey Res tree damage Truss Label: T-1	Cus: R 857 JRt: 1XUY8570001 T1 DrwNo: 324.23.1026.17770 SSB / WHK 11/20/2023
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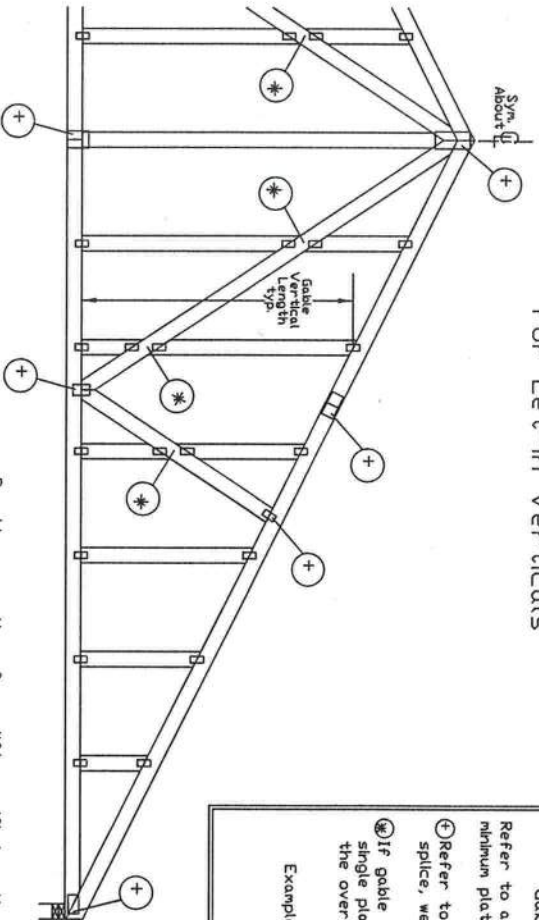
ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Gable Stud Reinforcement Detail

Dr. 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
 Dr. 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
 Dr. 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length														
Gable Vertical Spacing		2x4 Species	Brace	No Braces	(1) 1x4 1' Brace				(1) 2x4 1' Brace				(2) 2x4 1' Brace	
					Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B		
24" o.c.	SPF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"	
		Stud	4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"	
		Standard	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"	
		SP	#1	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	
		#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"		
	DFL	#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	
		Stud	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	
		Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 5"	13' 4"	14' 0"	14' 0"	
		#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	
		SPF	#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	
		Stud	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
16" o.c.	HF	Standard	4' 8"	6' 11"	7' 5"	9' 8"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
		#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"	
		SP	#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	
		#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
		Stud	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
		Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"	
	DFL	#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
		Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
		Standard	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
		SPF	#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	
12" o.c.	SP	#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	
		Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	
		Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
	DFL	#1	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
		Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	

Gable Detail For Let-in Verticals



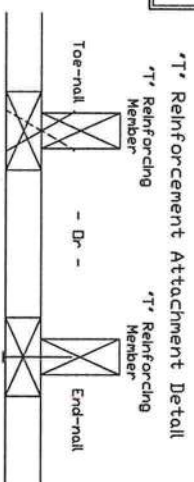
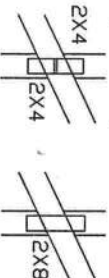
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:

10d Common (0.148" x 3.1" min) Nails at 4' o.c. plus

(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148" x 3.1" min) Toenails at 4' o.c. plus

(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A10015051014, A140015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A20015ENC100118, A14015ENC100118, A16015ENC100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A20015PENC100118, A18030ENC100118, A20030ENC100118, A20030ENC100118, A16030ENC100118, A11515ENC100118, S12015ENC100118, S14015ENC100118, A20030PENC100118, S18015ENC100118, S20015ENC100118, S20015PENC100118, S20015PENC100118, S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118, S18030ENC100118, S20030ENC100118, S20030PENC100118, S20030PENC100118

See appropriate Alpine gable detail for maximum allowable gable vertical length.

To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length based on appropriate Alpine gable detail.

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord. 'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Member Size	'T' Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, K_z = 1.00

Gable Vertical = 24' o.c. SP #3

'T' Reinforcing Member Size = 2x4

'T' Brace Increase (from Above) = 30% = 1.30

(1) 2x4 'L' Brace Length = 8' 7"

Maximum 'T' Reinforced Gable Vertical Length

130 x 8' 7" = 11' 2"

REF LET-IN VERT

DATE 01/02/2018

DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

DUR./FREQ. ANY

1999



AN ITW COMPANY

155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

WARNING: READ AND FOLLOW ALL NOTES ON THIS DRAWING.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Building Component Safety Information, by TPI and SBCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise. Top chord shall have properly attached structural sheathing and built in chord shall have bracing installed per BCSI. Bottom chord shall have bracing installed per BCSI. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any definition, or installation, or bracing of trusses.

This drawing, including the drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sect.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpine.com TPI: www.tpi.org SBCA: www.sbcacomponents.com ICC: www.iccsafe.org

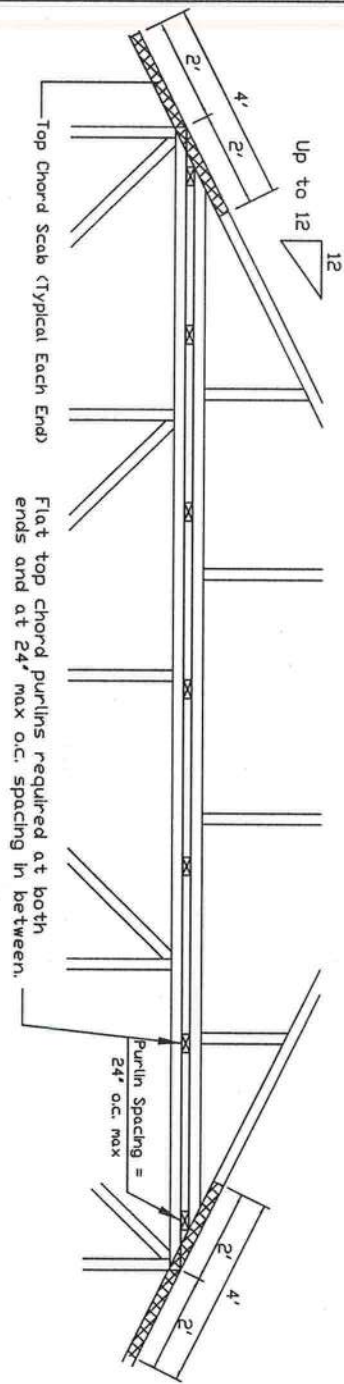
Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

160 mph Wind, 3000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Dr 140 mph wind, 3000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends. Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

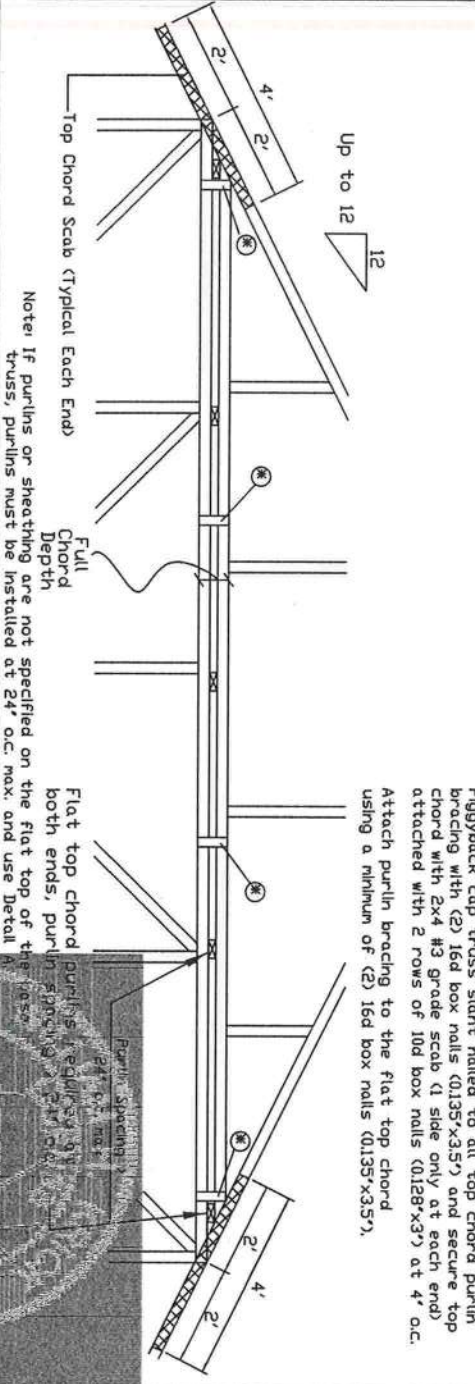


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3x8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate attached to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

*** In addition, provide connection with one of the following methods:**

Trulox:
Use 3x8 Trulox plates for 2x4 chord member, and 3x10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gussset:
8"x8"x7/16" (min) APA rated sheathing gusssets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gussset, (4) in cap bottom chord and (4) in base truss top chord. Gusssets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs:
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

28PB Wave Piggyback Plate:
One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI Building Component Safety Information, by TPI and SBCA for details. Practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Trusses shall be braced in accordance with BCSI sections B3, B7 or B10, as applicable. Apply plates to truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 1604-2 for standard plate positions.

Alpine, a division of ITTV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or covering the truss. A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sect. 2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitv.com TPI: www.tpi.org SBCA: www.sbcacomponents.com ICC: www.iccsafe.org



REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118
SPACING	24"
Product Approval #FL	1999

Piggyback Detail - ASCE 7-16: 180 mph, 30' Mean Hgt, Partially Enclosed, Exp. C, Kzt=1.00

180 mph Wind, 3000 ft Mean Hgt, ASCE 7-16, Part: Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
 Dr 160 mph Wind, 3000 ft Mean Hgt, ASCE 7-16, Part: Enclosed Bldg. located anywhere in roof, Exp D, Wind DL= 5.0 psf (min), Kzt=1.0.

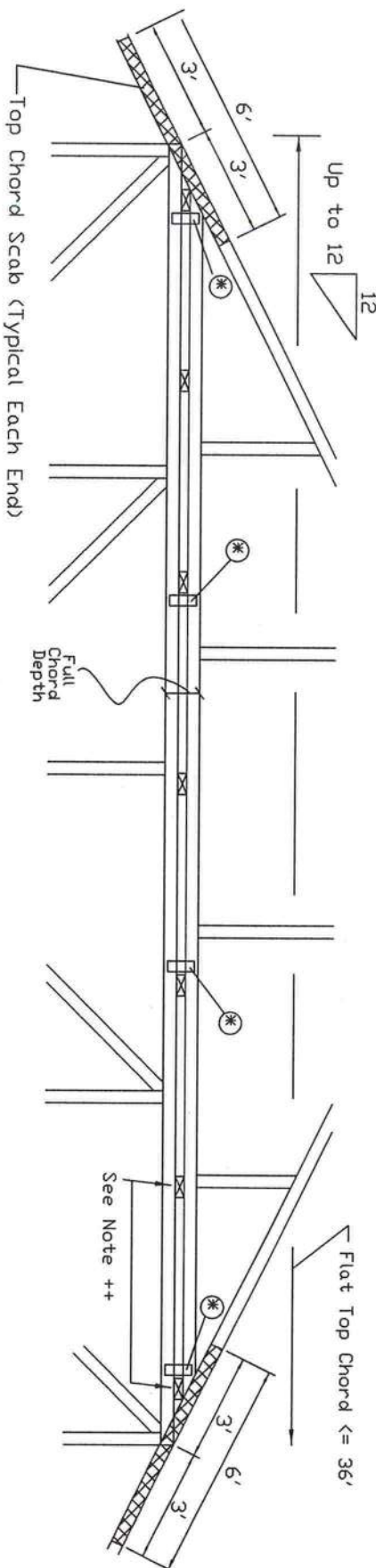
Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

++ Flat top chord purlins required at both ends and at a maximum of 24' intervals unless otherwise noted on base truss design drawing. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



* In addition, provide connection with one of the following methods:

Trulox Use 3x8 Trulox plates for 2x4 chord member, and 3x10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.	28PB Wave Piggyback Plate One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.
APA Rated Gussset 8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (6) 6d common chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.	2x4 Vertical Scabs 2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") perpendicular to bottom chord and (4) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING

Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to the latest edition of BCSI Building Components Safety Information, by TPI and SBCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Trusses shall be properly attached and braced. Locations shown for permanent lateral restraints of trusses shall have been checked and approved by the Engineer of Record. Refer to drawings 160A-2 for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any design, fabrication, installation, or bracing of trusses. The building Engineer of Record shall be responsible for the design, engineering, and bracing of the truss system. The building Engineer of Record shall be responsible for the design, engineering, and bracing of the truss system. The building Engineer of Record shall be responsible for the design, engineering, and bracing of the truss system.

155 Hallam Ave
 North Building, 4th Floor
 Glenview, IL 60025



REF PIGGYBACK

DATE 01/02/2018

DRWG PB180160118

COA #0378

11/21/2023

Florida Certified Product Approval #FL-1999

SPACING 24.0'

Cracked or Broken Member Repair Detail

Load Duration = 0%
Member forces may be increased for Duration of Load

This drawing specifies repairs for a truss with broken chord or web member.

This design is valid only for single ply trusses with 2x4 or 2x6 broken members. No more than one break per chord panel and no more than two breaks per truss are allowed. Contact the truss manufacturer for any repairs that do not comply with this detail.

(B) = Damaged area, 12' max length of damaged section
(L) = Minimum nailing distance on each side of damaged area (B)
(S) = Two 2x4 or two 2x6 side members, same size, grade, and species as damaged member. Apply one scab per face. Minimum side member length(s) = (2)(L) + (B)

Scab member length (S) must be within the broken panel.

Nail into 2x4 members using two (2) rows at 4' o.c., rows staggered.

Nail into 2x6 members using three (3) rows at 4' o.c., rows staggered.

Nail using 10d box or gun nails (0.128"x3", min) into each side member.

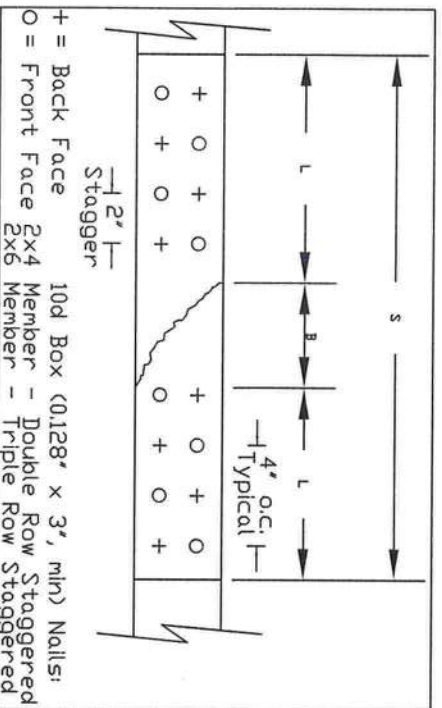
The maximum permitted lumber grade for use with this detail is limited to Visual grade #1 and MSR grade 1650f.

This repair detail may be used for broken connector plate at mid-panel splices.

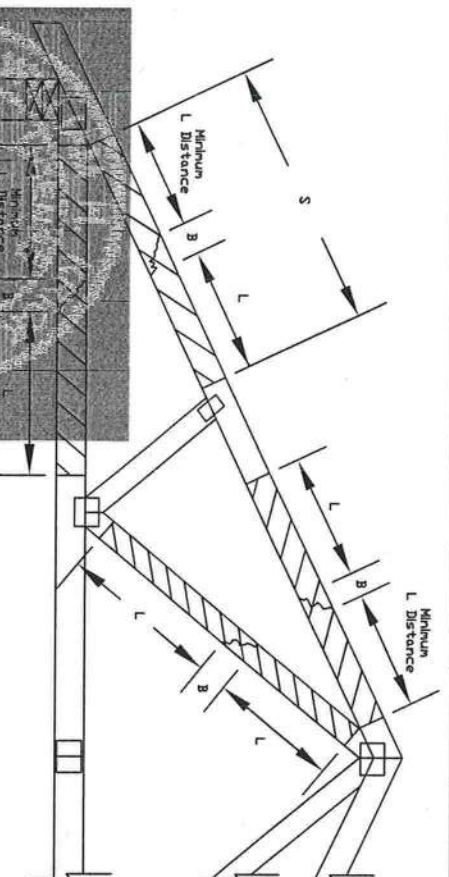
This repair detail may not be used for broken chord or web sections occurring within the connector plate area.

Broken chord may not support any tie-in loads.

Member	Size	L	Maximum Member Axial Force			
			SPF-C	HF	DF-L	SYP
Web Only	2x4	12'	620#	635#	730#	800#
Web Only	2x4	18'	975#	1055#	1295#	1415#
Web or Chord	2x4	24'	975#	1055#	1495#	1745#
Web or Chord	2x6	24'	1465#	1585#	2245#	2620#
Web or Chord	2x4	30'	1910#	1960#	2315#	2555#
Web or Chord	2x6	30'	2230#	2365#	3125#	3575#
Web or Chord	2x4	36'	2470#	2530#	2930#	3210#
Web or Chord	2x6	36'	3535#	3635#	4295#	4745#
Web or Chord	2x4	42'	2975#	3045#	3505#	3835#
Web or Chord	2x6	42'	4395#	4500#	5225#	5725#
Web or Chord	2x4	48'	3460#	3540#	4070#	4445#
Web or Chord	2x6	48'	5165#	5280#	6095#	6660#



Nail Spacing Detail



REF MEMBER REPAIR

DATE 10/01/14

DRWG REPCHRD1014



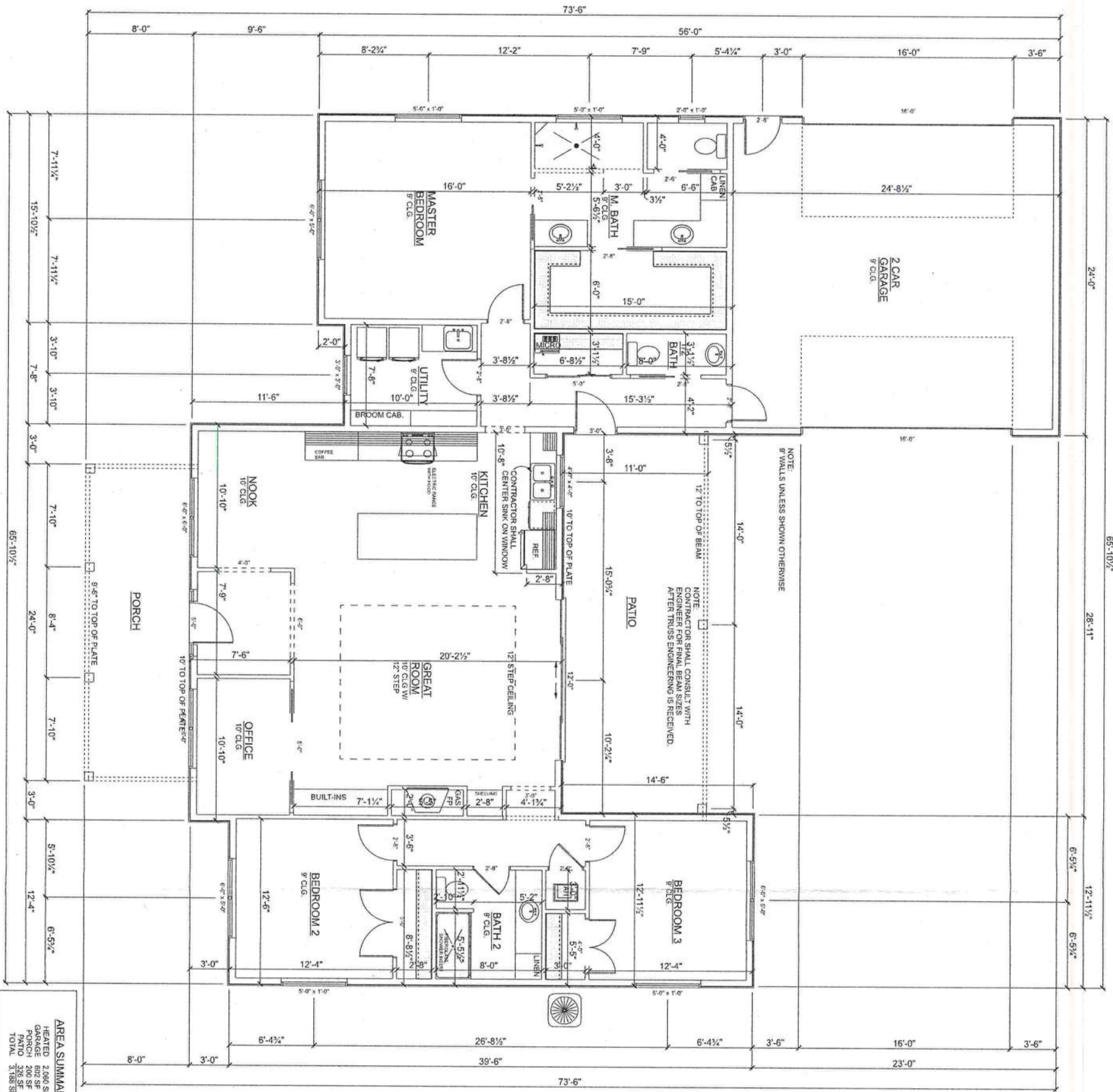
ALLIANCE COMPANY

155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

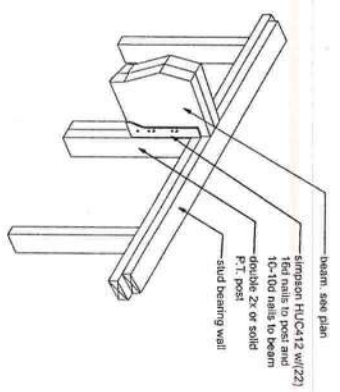
MAINTAINERS READ AND FOLLOW ALL NOTES ON THIS DRAWING. FOLLOW THE LATEST EDITION OF BCSP BUILDING COMPONENT SAFETY INFORMATION, TPI AND SBCA. UNLESS NOTED OTHERWISE, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL SHEETING AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING. LOCATIONS SHOWN FOR PERMANENT LATERAL RESTRAINTS OF TRUSS AND POSITION AS SHOWN ABOVE AND ON THE JOINT DETAILS, UNLESS NOTED OTHERWISE. REFER TO DRAWING 160A-2 FOR STANDARD PLATE POSITIONS.

ALPINE, a division of ITW Building Components Group Inc. shall not be responsible for any design, installation, or bracing of trusses. This drawing, including this drawing, indicates acceptance of professional engineering responsibility for the design of the building designer per ANSI/TPI 1 Sec 2. For more information see this job's general notes page and these web sites: ALPINE: www.alpine.com TPI: www.tpi.org SBCA: www.sbcacomponents.com ICC: www.iccsafe.org

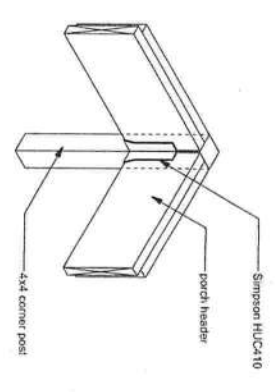
COA #0278 11/21/2023
Florida Certified Architect AP#24987 FAX



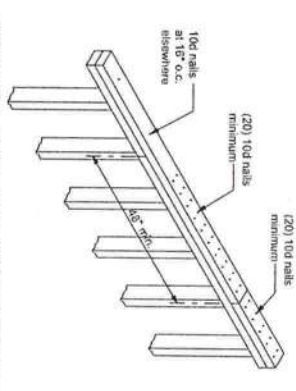
AREA SUMMARY	
HEATED	2,080 SF
GARAGE	602 SF
POUCH	200 SF
SCREENED	200 SF
TOTAL	3,082 SF



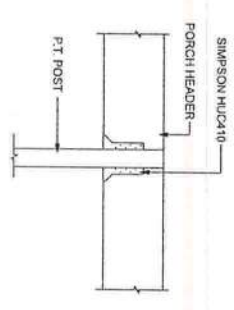
BEAM/WALL CONNECTION
TAXX CAPACITY - SCREW DOWN 18" OR DEEPER - NOT TO SCALE



CORNER POST
NTS



TOP PLATE SPLICE DETAILS
SCALE: 1/2" = 1'-0"

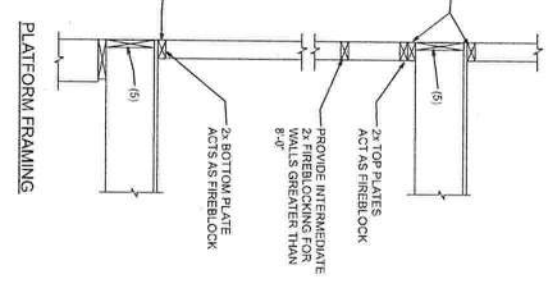
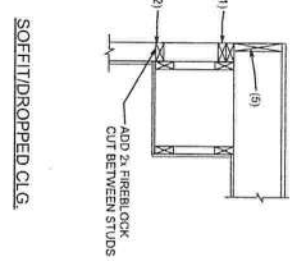
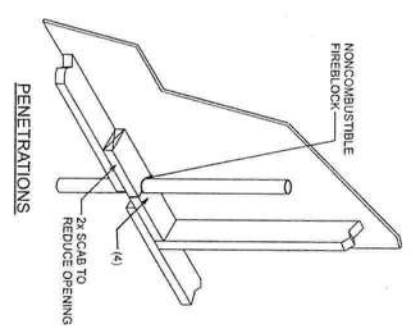
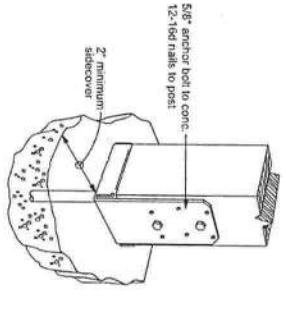


INTERMEDIATE POST
NTS

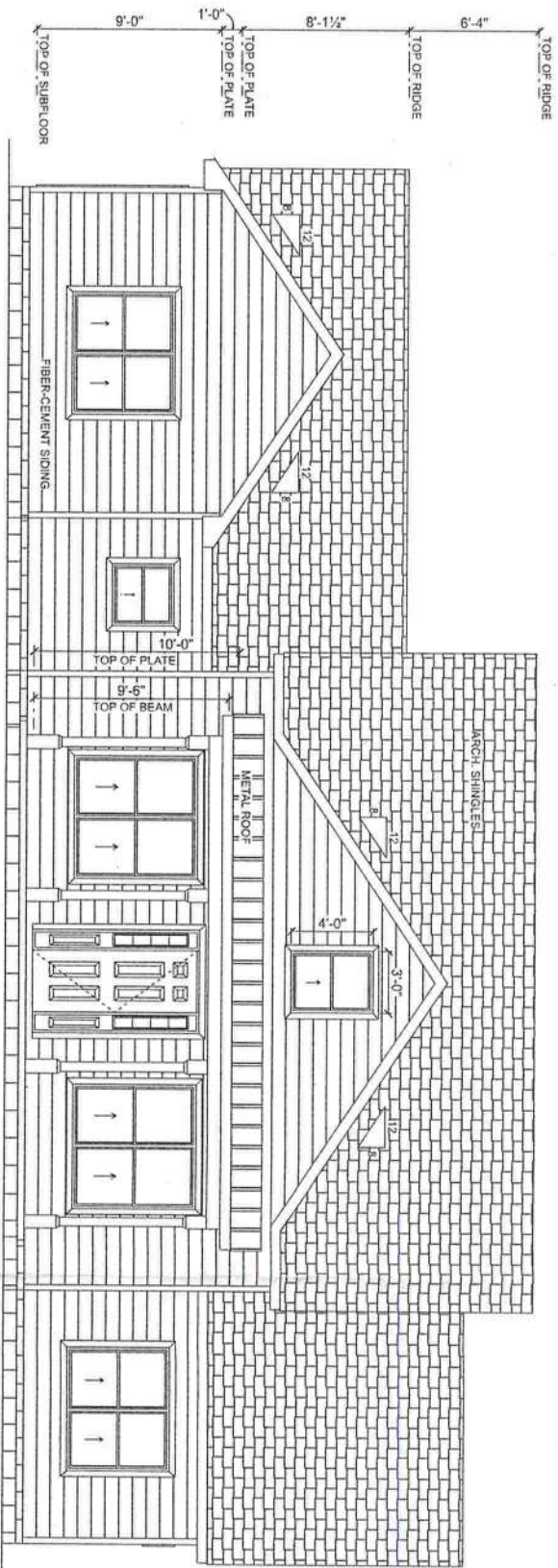
FIREBLOCKING NOTES:

- FIREBLOCKING SHALL BE INSTALLED IN WOOD FRAME CONSTRUCTION IN THE FOLLOWING LOCATIONS:
- IN CONCEALED SPACES OF STUD WALLS AND PARTITIONS INCLUDING FURRED SPACES AT CEILING AND FLOOR LEVELS.
- AT ALL INTERCONNECTIONS BETWEEN CONCEALED VERTICAL AND HORIZONTAL SPACES SUCH AS OCCUR AT SOFFITS, DROP CEILING, COVE CEILING, ETC.
- IN CONCEALED SPACES BETWEEN STAIR STRINGERS AT THE TOP AND BOTTOM OF THE RUN.
- AT OPENINGS AROUND VENTS, PIPES, DUCTS, CHIMNEYS AND FIREPLACES AT CEILING AND FLOOR LEVELS WITH PYROPLANET MULTIFLEX SEALANT
- AT ALL INTERCONNECTIONS BETWEEN CONCEALED VERTICAL STUD WALL OR CEILING AND FLOOR LEVELS CREATED BY AN ASSEMBLY OF FLOOR JOISTS. FIREBLOCKING SHALL BE PROVIDED FOR THE FULL DEPTH OF THE JOIST AT THE ENDS AND OVER THE SUPPORTS.

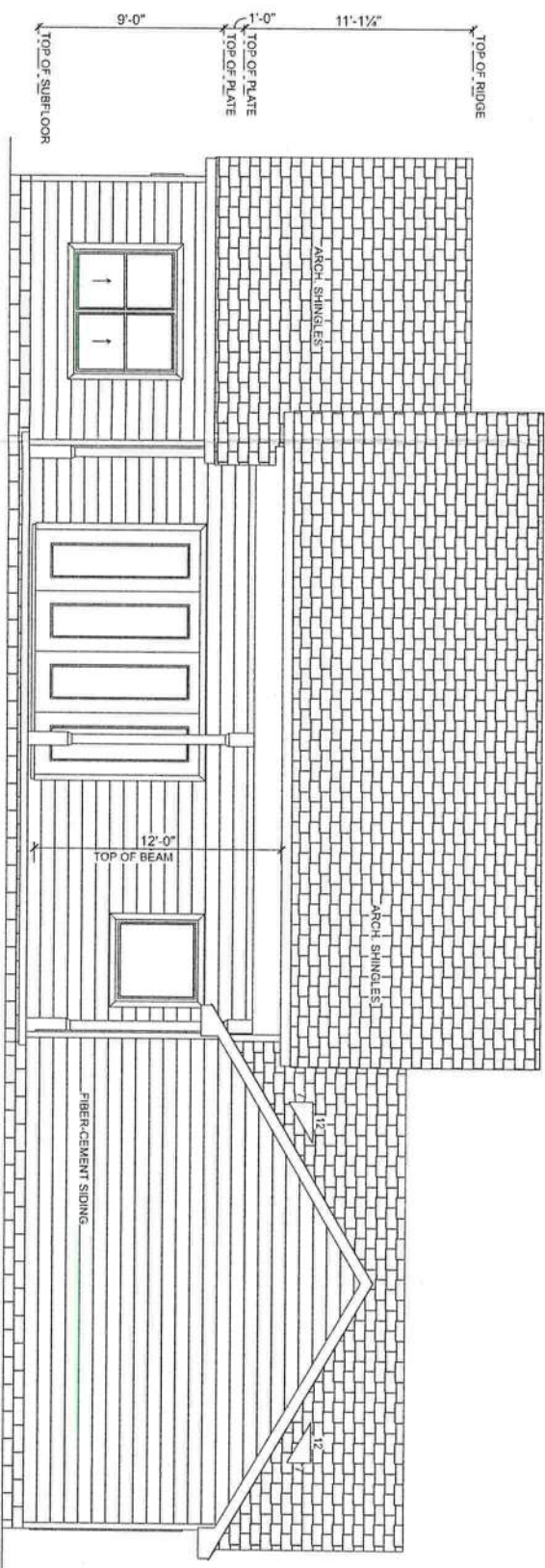
Simpson ABU66



REVISIONS		DATE	BY	DESCRIPTION
DESIGN BY: TRADEMARK Construction Group, Inc.				CERTIFIED GENERAL CONTRACTOR CC03191789 1613 SW MIDTOWN PL. STE 101 LAKE CITY, FL 32025 (386) 755-5254
CES CREWS Engineering Services, LLC				CERTIFICATE OF AUTHORIZATION NO. 28022 349 SW CREWS FARM TERRACE STE 101 LAKE CITY, FL 32025 PHONE: 386.623.4303
Brett A. Crews, P.E. 65592				DRAWN BY: TM APPROVED BY: BC
FLOOR PLAN				PROJECT NO. R22.014
SHEET: A-2				



FRONT ELEVATION
SCALE: 1/4" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"

REVISIONS		DESIGN BY:		CERTIFICATE OF AUTHORIZATION		DRAWN BY:		PROJECT NO.:	
DATE	BY	DESCRIPTION		NO. 28022		TM		R22.014	
				349 SW CREWS FARM TERRACE LAKE CITY, FL 32025 PHONE: 386.623.4303		BC		ELEVATIONS FRONT AND REAR	SHEET: A-3

TRADEMARK
Construction Group, Inc.

CERTIFIED GENERAL CONTRACTOR
CSC 0519189
163 SW MIDTOWN PL.
LAKE CITY, FL 32025
(386)755-5254

CES
Crews Engineering Services, LLC

Brett A. Crews, P.E. 65592