

REVISIONS	

**SOFTPLAN**  
ARCHITECTURAL DESIGN SOFTWARE

**REQUIRED ROOF VENTILATION:**  
AS PER FLORIDA BUILDING CODE 2309.7

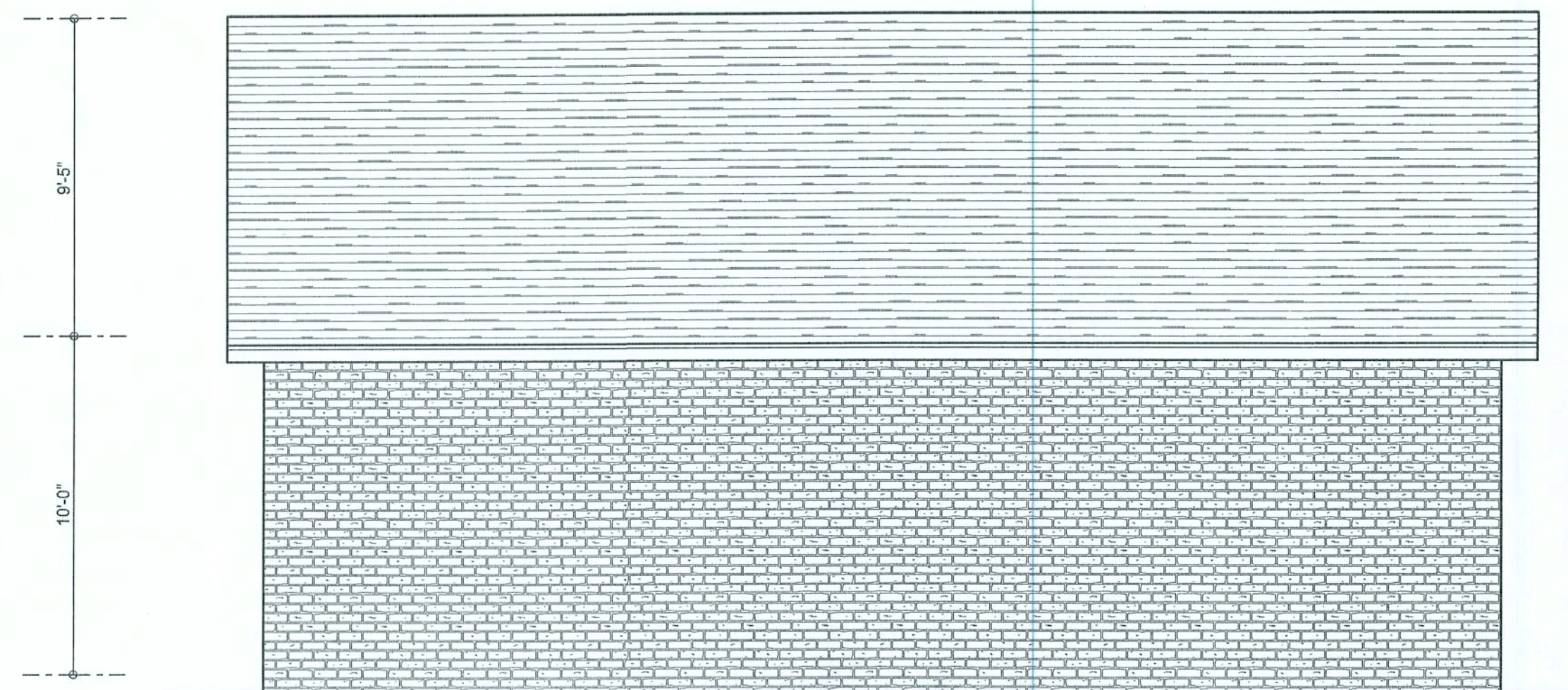
RIDGE VENT  
MIN. 50% TOTAL VENT AREA  
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3' ABOVE EAVE)  
1296 S.F. / 300 x 50% = 3 S.F. RIDGE VENT AREA REQUIRED  
27 FEET OF RIDGE VENT REQUIRED

SOFFIT VENT  
1296 S.F. / 300 x 50% = 3 S.F. SOFFIT VENT AREA REQUIRED  
100 FEET OF SOFFIT VENT REQUIRED

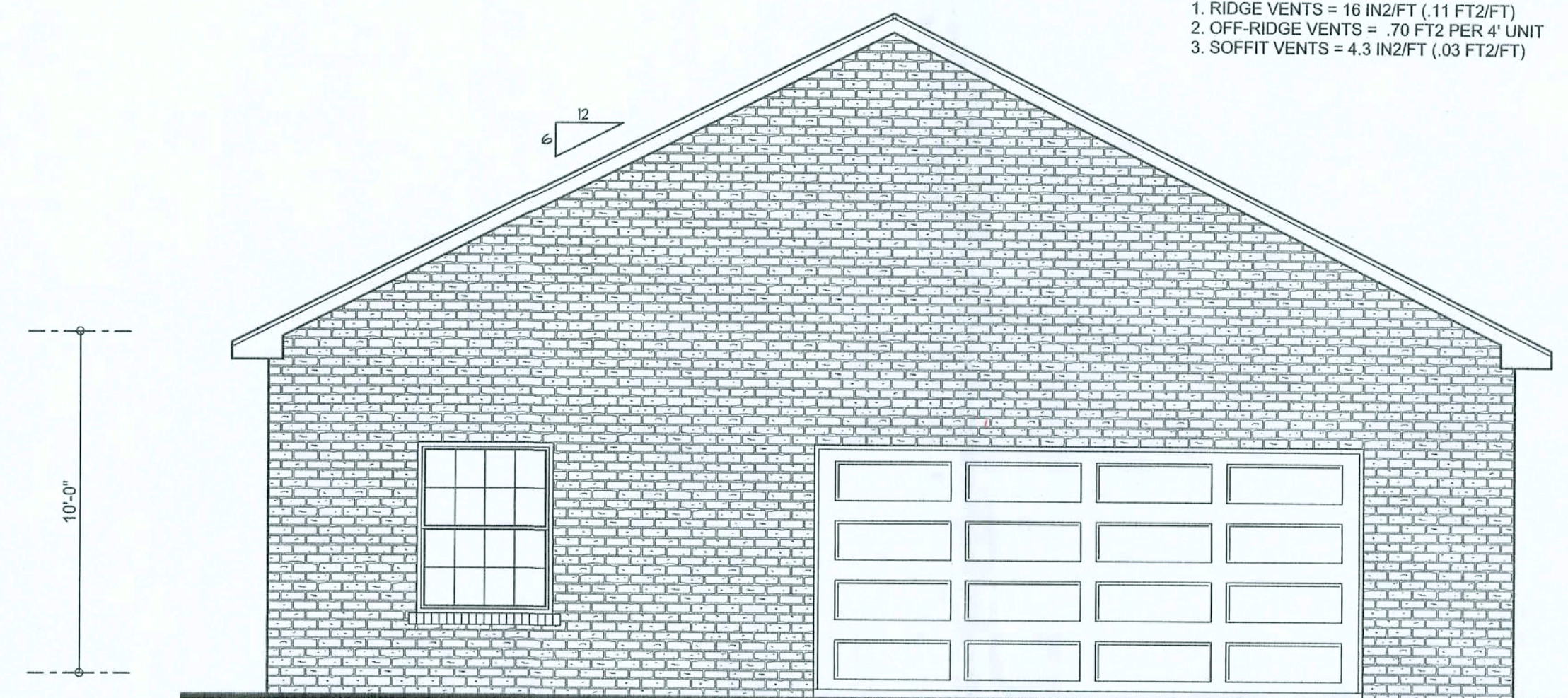
BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

1. RIDGE VENTS = 16 IN<sup>2</sup>/FT (11 FT<sup>2</sup>/FT)
2. OFF-RIDGE VENTS = .70 FT<sup>2</sup> PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN<sup>2</sup>/FT (.03 FT<sup>2</sup>/FT)

Michael Linnane  
758-6946



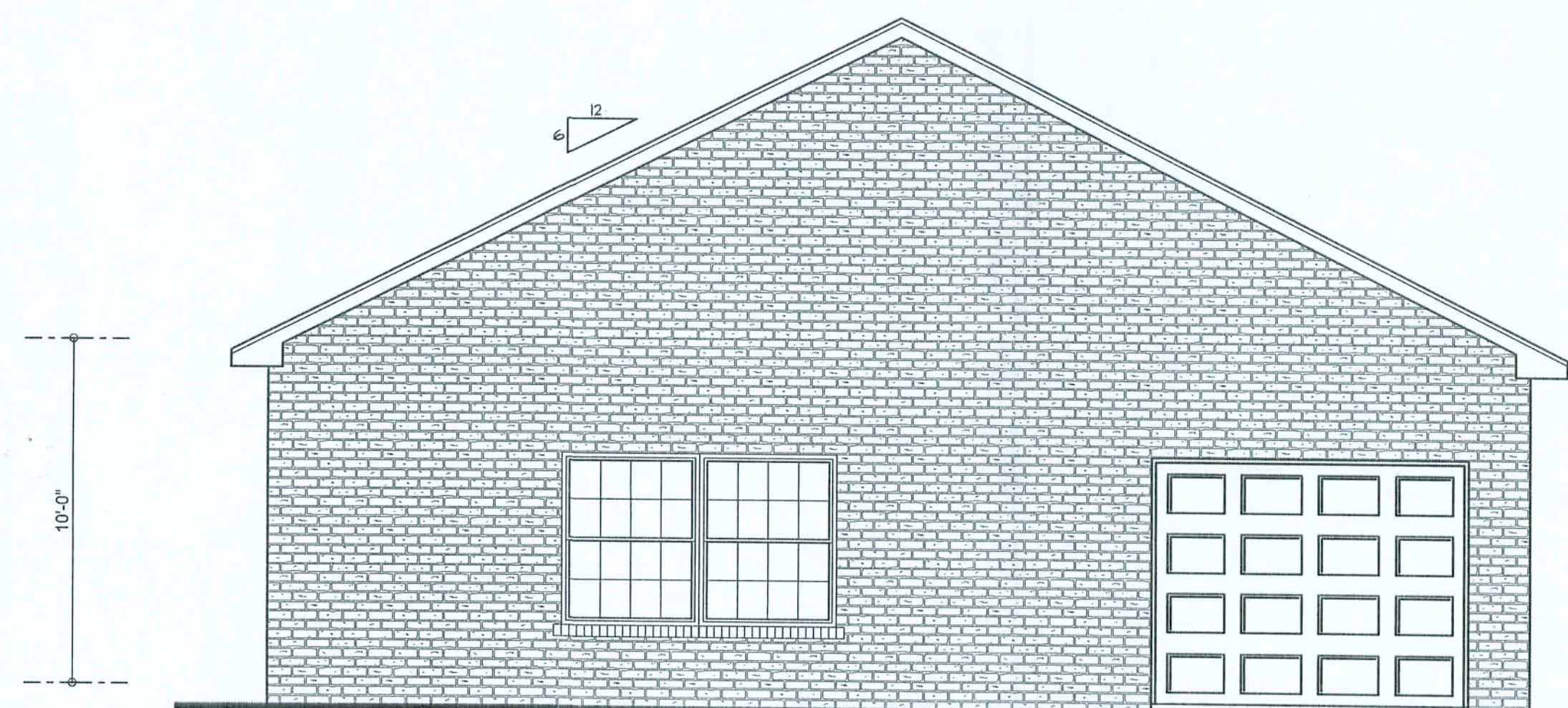
**LEFT ELEVATION**  
SCALE: 1/4" = 1'-0"



**FRONT ELEVATION**  
SCALE: 1/4" = 1'-0"



**RIGHT ELEVATION**  
SCALE: 1/4" = 1'-0"



**REAR ELEVATION**  
SCALE: 1/4" = 1'-0"

WINDLOAD ENGINEER: Mark Disosway,  
P.E. No. 53815, FOR 866, Lake City, FL  
32056, 386-754-5419

**DIMENSIONS:**  
Stated dimensions supersede scaled  
dimensions. Refer all questions to  
Mark Disosway, P.E. for resolution.  
Do not proceed without clarification.

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permission and consent of Mark Disosway.

**CERTIFICATION:** I hereby certify that I have  
examined this plan, and that the applicable  
portions of the plan, relating to wind engineering  
comply with section R301.2.1, Florida building  
code residential 2004, to the best of my  
knowledge.

**LIMITATION:** This design is valid for one  
building, at specified location.

MARK DISOSWAY  
P.E. 53815

Michael Disosway  
07 MAR 06  
SEAL

**JOHNNY JOHNSON**

**MIKE LINNANE  
GARAGE**

**ADDRESS:**  
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Columbia County, Florida

Mark Disosway P.E.  
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Lake City, Florida 32056  
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Fax: (386) 269 - 4871

**PRINTED DATE:**  
March 07, 2006

**DRAWN BY:** Ben Sparks  
**STRUCTURAL BY:** Ben Sparks

**FINALS DATE:**  
07/ MAR / 06

**JOB NUMBER:**  
602151

**DRAWING NUMBER**

**A-1**

OF 5 SHEETS



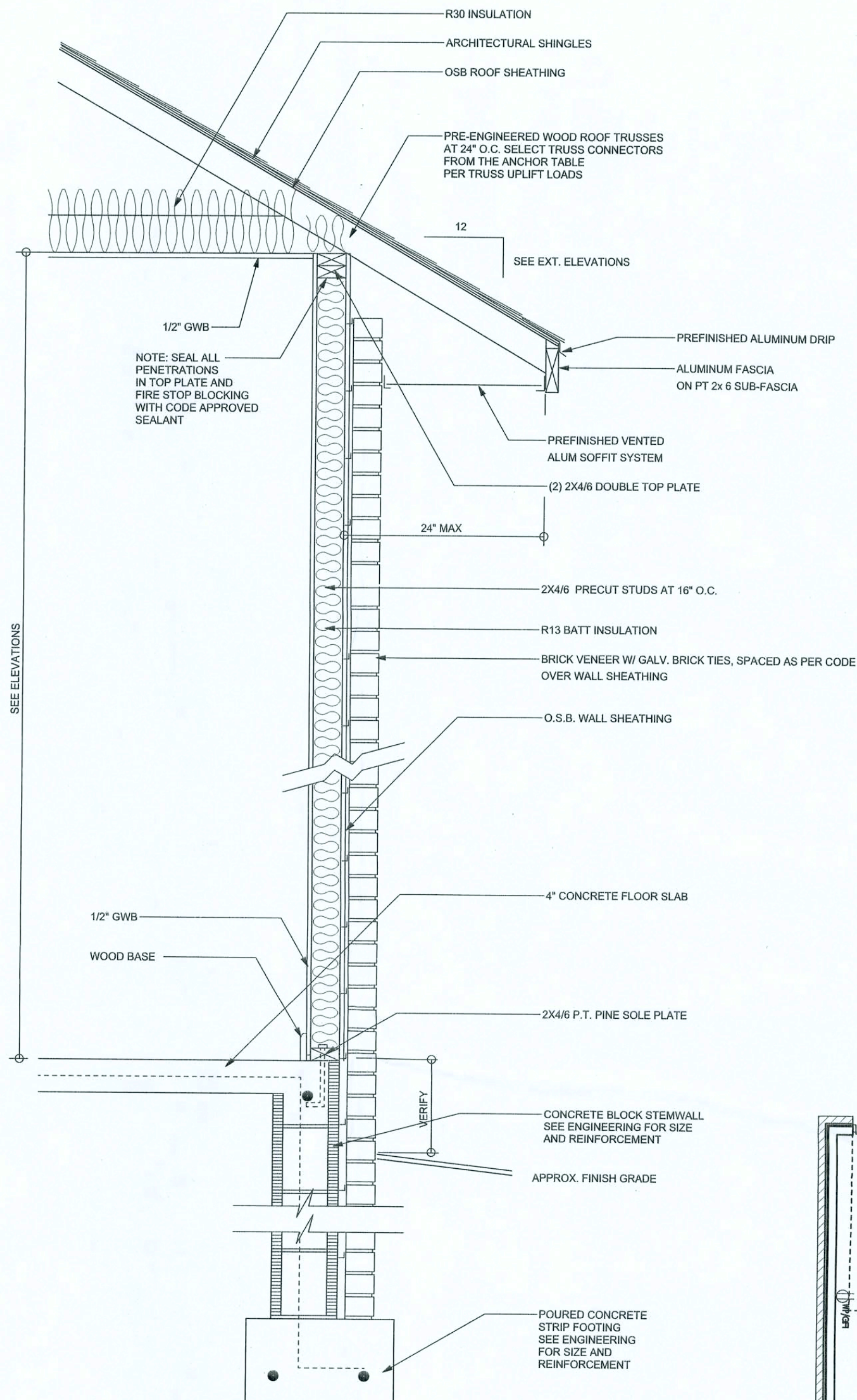
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ELECTRICAL PLAN NOTES

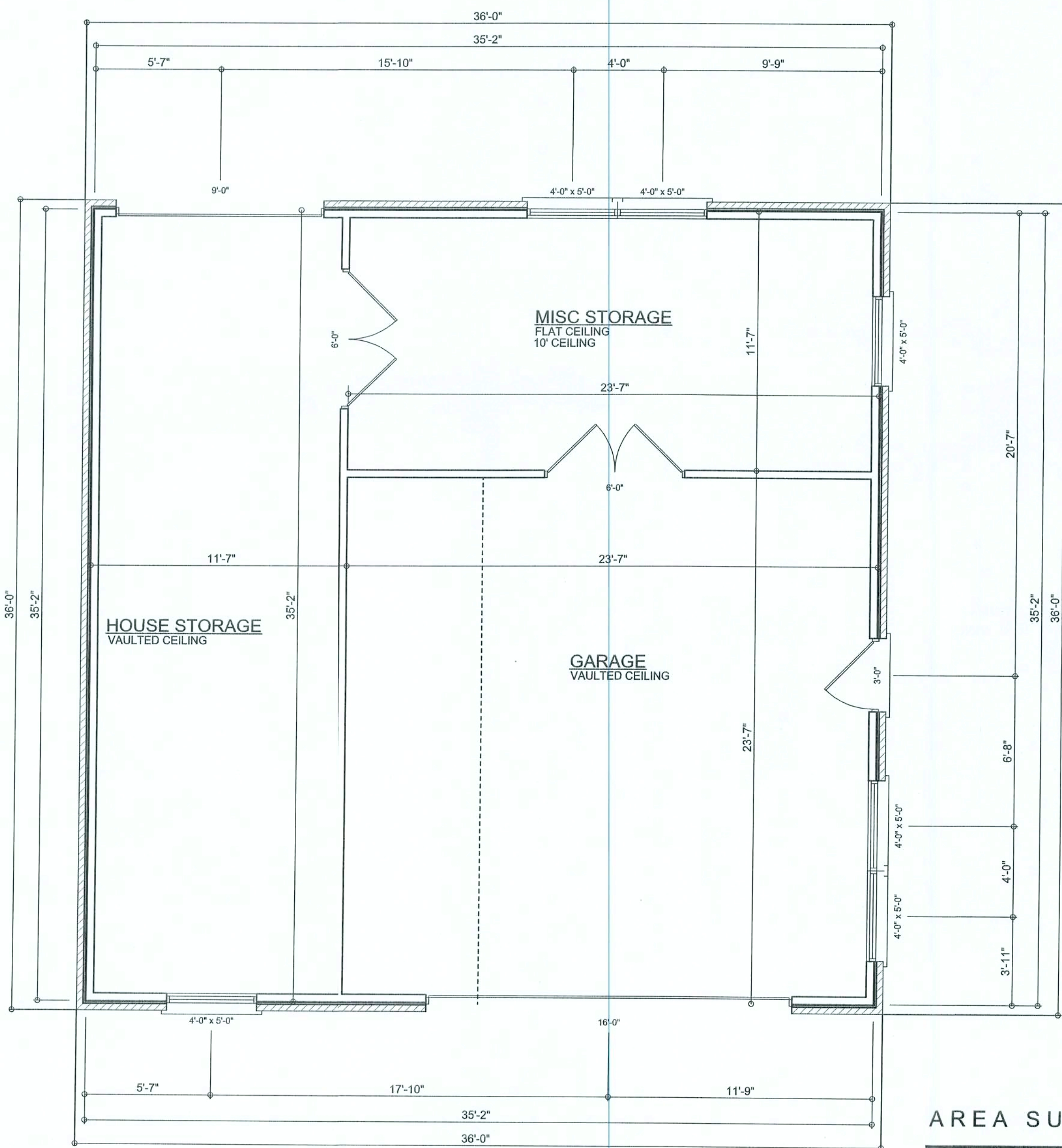
- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE ( UNDERGROUND OR OVERHEAD ) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING
- E -10 SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL.

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	WALL HEATER



TYPICAL DESIGN WALL SECTION  
NON - STRUCTURAL DATA

SCALE: 1" = 1'-0"

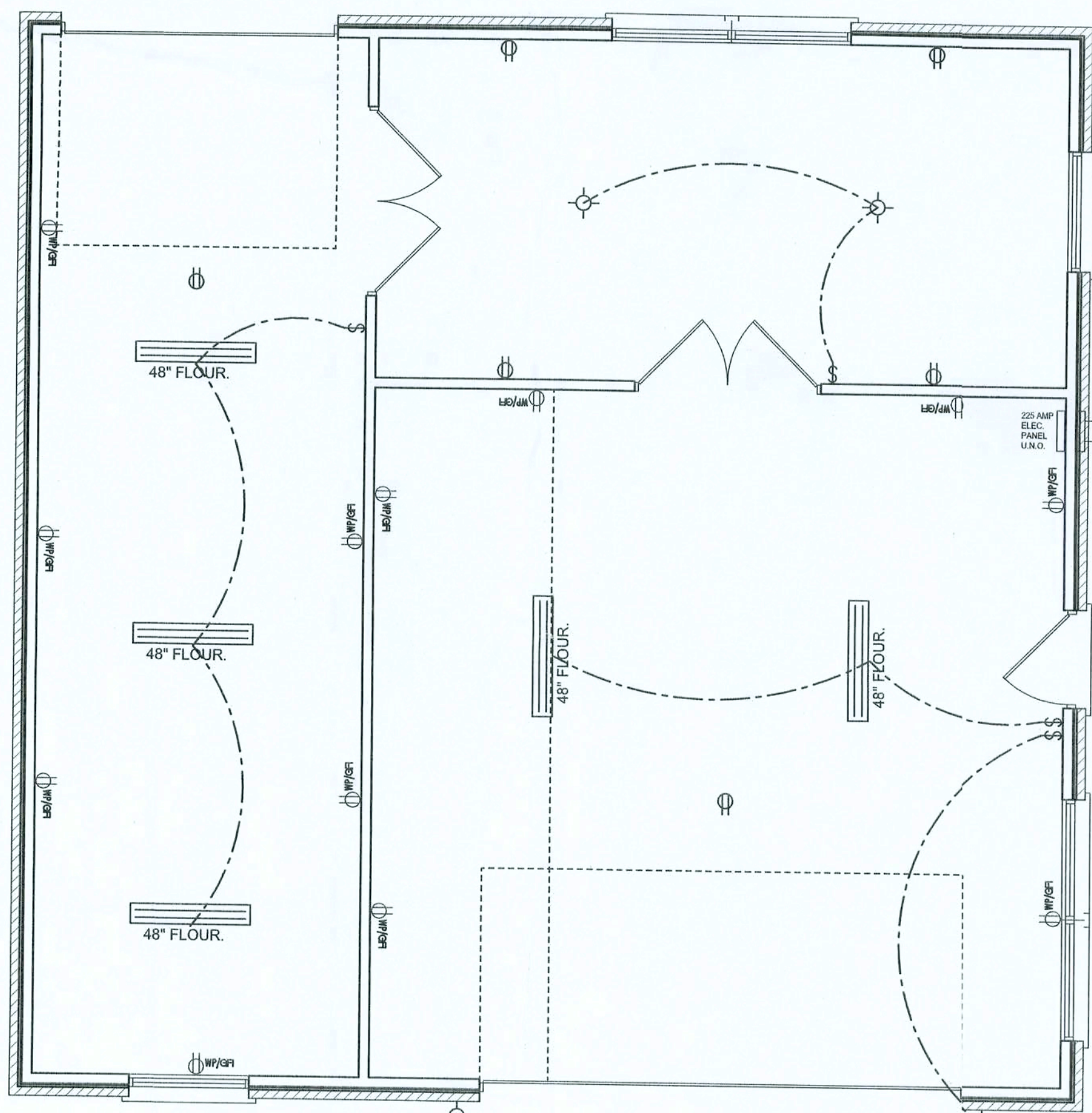


FLOOR PLAN  
SCALE: 1/4" = 1'-0"

VAULT ENTIRE WIDTH  
UNLESS NOTED OTHERWISE

AREA SUMMARY

GARAGE AREA	584	S . F .
STORAGE AREA	712	S . F .
TOTAL AREA	1296	S . F .



ELECTRICAL PLAN  
SCALE: 1/4" = 1'-0"

WINDLOAD ENGINEER: Mark Disosway,  
P.E. No. 53915, P.O.B. 868, Lake City, FL  
33606, 386-754-5419

DIMENSIONS:  
Sited dimensions supercede scaled  
dimensions. Refer all questions to  
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CERTIFICATION: I hereby certify that I have  
examined this plan, and that the applicable  
portions of the plan, relating to wind engineering,  
comply with section R301 2.1, Florida Building  
code residential 2004, to the best of my  
knowledge.

LIMITATION: This design is valid for one  
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MARK DISOSWAY  
P.E. 53915

*Mark Disosway*  
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MIKE LINNANE  
GARAGE

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March 07, 2006

DRAWN BY: Ben Sparks  
STRUCTURAL BY: Ben Sparks

FINALS DATE:  
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DRAWING NUMBER

A-2

OF 5 SHEETS



REVISIONS	

**SOFTPLAN**  
ARCHITECTURAL DESIGN SOFTWARE

## GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBOR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REACTION OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 4x10 EACH END. 2X6 RAFTERS TO BE EACH END.

**SITE PREPARATION:** SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

**FOUNDATION:** CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

**CONCRETE:** MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $F'_c = 3000$  PSI.

**WELDED WIRE REINFORCED SLAB:** 6" x 6" W1.4 x W1.4, FB = 80KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 7'.

**FIBER CONCRETE SLAB:** CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

**CONTROL JOINTS:** WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 308. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

**REBAR:** ASTM A 615, GRADE 60, DEFORMED BARS,  $F_y = 60$  KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

**GLULAM BEAMS:** GLULAM BEAM, GLB, 24F-V3SP,  $F_b = 2.4ksi$ ,  $E = 1800ksi$ . UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

**ROOF SHEATHING:** ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC, UNO.

**STRUCTURAL CONNECTORS:** MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

**ANCHOR BOLTS:** A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE BEAM OR 12" IN GROUTED CMU.

**WASHERS:** WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16", UNO.

**NAILS:** ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

## BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBOR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

## ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBOR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

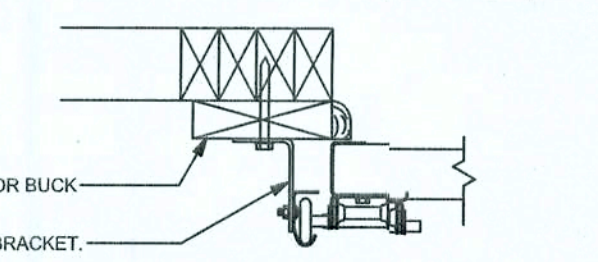
## GRADE & SPECIES TABLE

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

## 2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF 131 x 3 1/4" GN PER TABLE BELOW.

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF 131 x 3 1/4" GN
6' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.

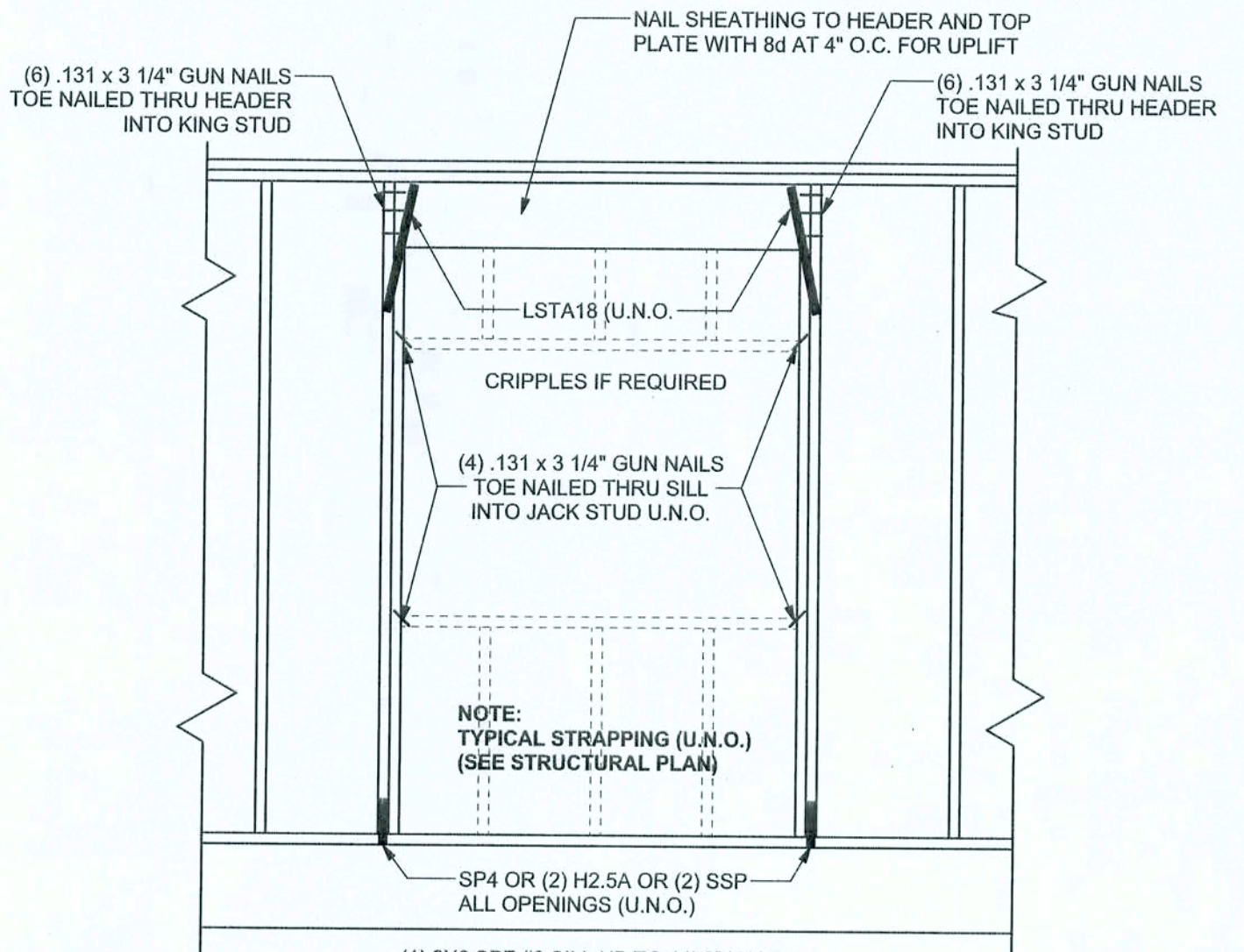


## GARAGE DOOR BUCK INSTALLATION DETAIL

SCALE: N.T.S.

## CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

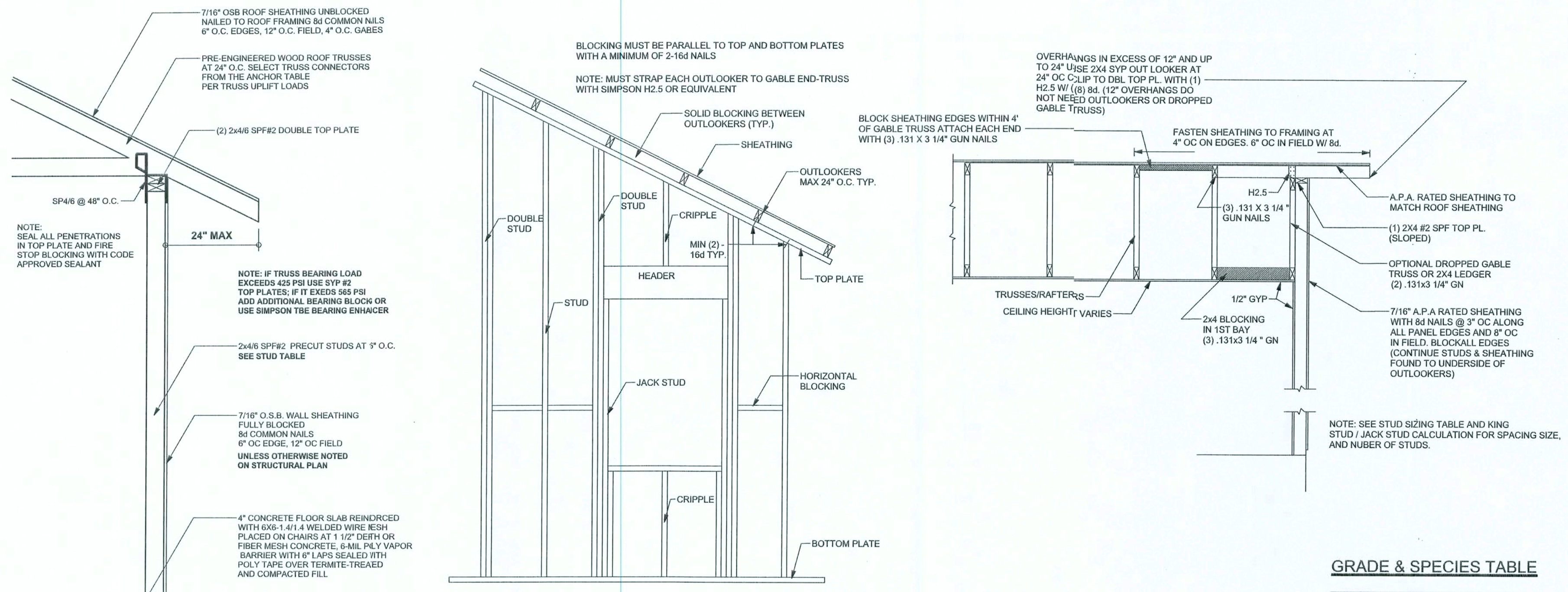
SCALE: N.T.S.



(1) 2X6 SPF #2 SILL UP TO 11'-0" U.N.O.  
(1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O.  
(FOR: 110 MPH, 10'-0" WALL HEIGHT U.N.O.)

## TYPICAL HEADER STRAPING DETAIL

SCALE: 1/2" = 1'-0"



## GABLE END WALL BALLOON FRAMING DETAIL

SCALE: 1/2" = 1'-0"

## ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

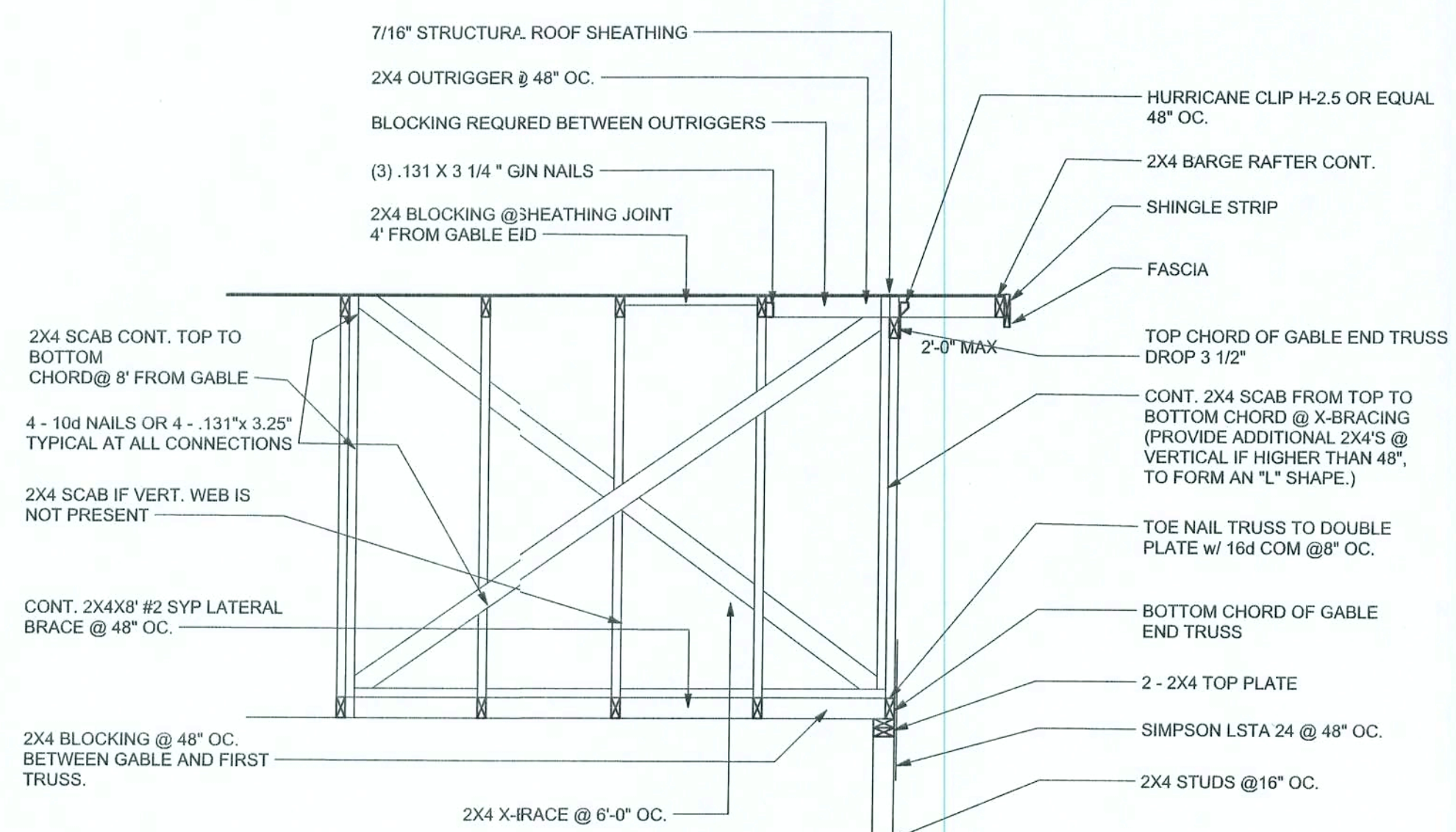
## EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-0" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE: 16" O.C. x 0.85 = 13.6" O.C.

## TYPICAL GARAGE DOOR HEADER STRAPING DETAIL

SCALE: 1/2" = 1'-0"



## TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP

2X4 X-RACE @ 6'-0" OC.

WINDLOAD ENGINEER: Mark Discoway, P.E. No. 33915, P.O. Box 868, Lake City, FL 32056, 386-754-5419

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISCOWAY  
P.E. 33915

*Mark Discoway*  
8/7/2006  
SEAL

## JONNY JOHNSON

## MIKE LINNANE GARAGE

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## ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTERS/TRUSSES	TO STUDS
< 420	< 245	H5A	3-8d	3-4d	
< 455	< 265	H5	4-8d	4-4d	
< 360	< 235	H4	4-8d	4-4d	
< 455	< 320	H3	4-8d	4-4d	
< 415	< 365	H2.5	5-8d	5-4d	
< 600	< 535	H2.5A	5-8d	5-4d	
< 950	< 820	H6	8-8d	8-4d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 780	< 655	H10-2	6-10d	6-1d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LG12	14 - 16d	14 - 5d	
<b>HEAVY GIRDER TIEDOWNS*</b>					<b>TO FOUNDATION</b>
< 3065	< 3330	MG1		22 - 0d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 - 0d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 - 0d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 - 0d	2-5/8" THREADED ROD 12" EMBEDMENT
<b>STUD STRAP CONNECTOR*</b>					<b>TO STUDS</b>
< 435	< 435	SSP DOUBLE TOP PLATE	3 - 10d		4 - 10d
< 455	< 420	SSP SINGLE SILL PLATE	1 - 10d		4 - 10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 - 10d		8 - 10d
< 825	< 600	DSP SINGLE SILL PLATE	2 - 10d		8 - 10d
< 885	< 780	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 780	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
<b>STUD ANCHORS*</b>					<b>TO FOUNDATION</b>
< 1350	< 1305	LTT19	8-16d		1 1/2" AB
< 2310	< 2310	LTT31	18-10d, 1 1/2"		1 1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3665	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1 1/2" AB
< 2300	< 2300	ABU66	12-16d		1 1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

## DESIGN DATA

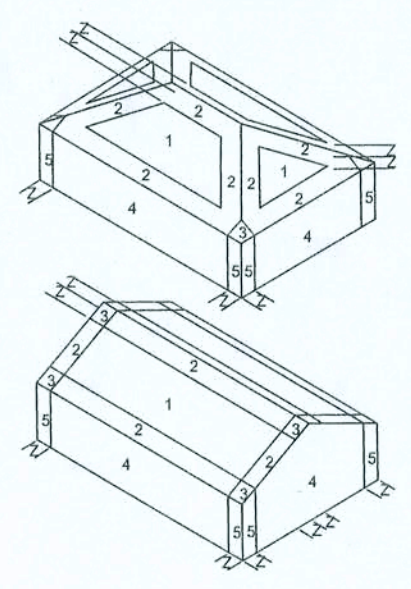
### WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT. NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT. IN EXP. B, 30 FT. IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



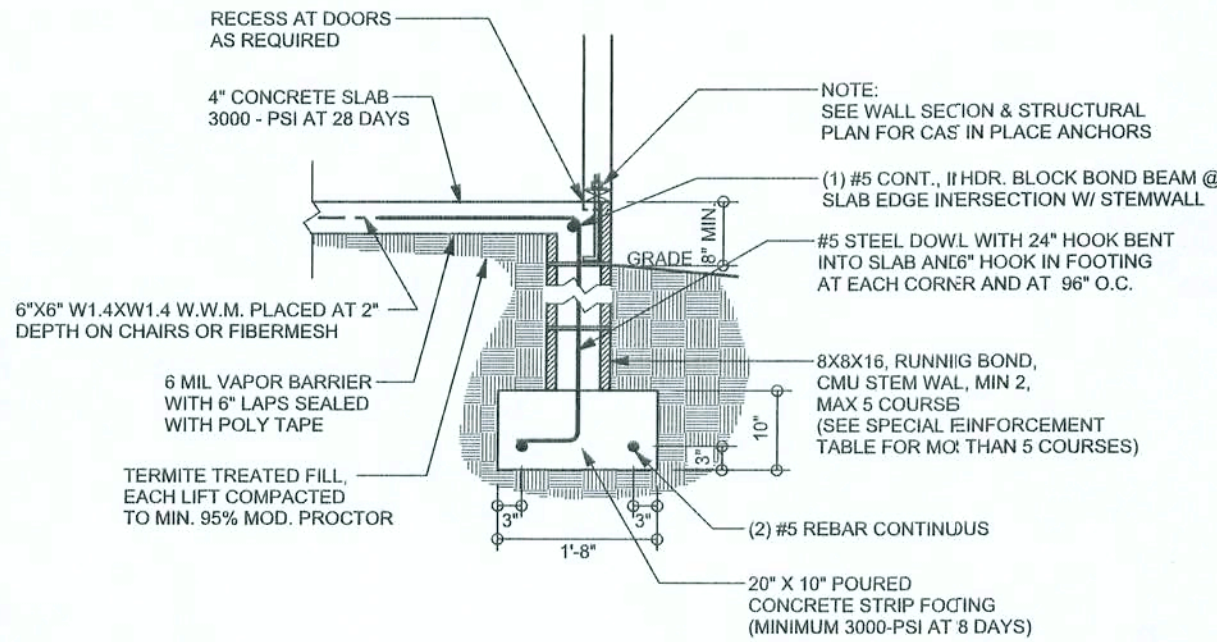
Zone	Effective Wind Area (ft <sup>2</sup> )	10	100
1	119.9 - 21.8	18.1	-18.1
2	119.9 - 25.5	18.1	-21.8
2 Othg	-40.6	-40.6	
3	119.9 - 25.5	18.1	-21.8
3 Othg	-68.3	-42.4	
4	2.8 - 23.6	18.5	-20.4
5	2.8 - 29.1	18.5	-22.6

### DESIGN LOADS

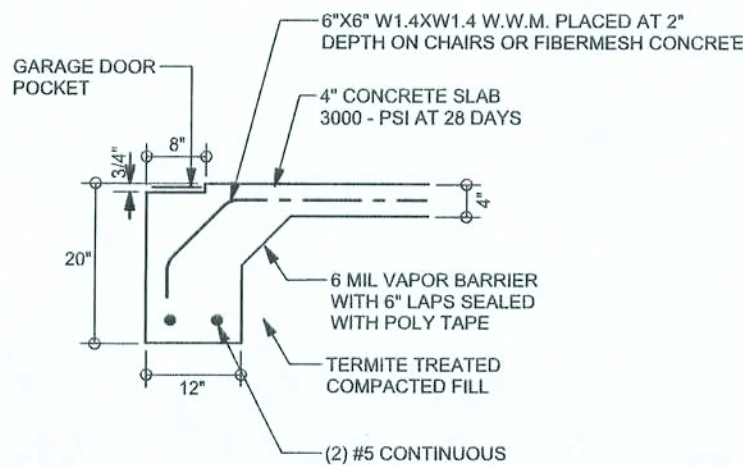
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3.12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
	SOIL BEARING CAPACITY 1000PSF
	NOT IN FLOOD ZONE (BUILDER TO VERIFY)



REVISIONS	



**F9**  
**S-2**    **STEM WALL FOOTING**  
SCALE: 1/2" = 1'-0"

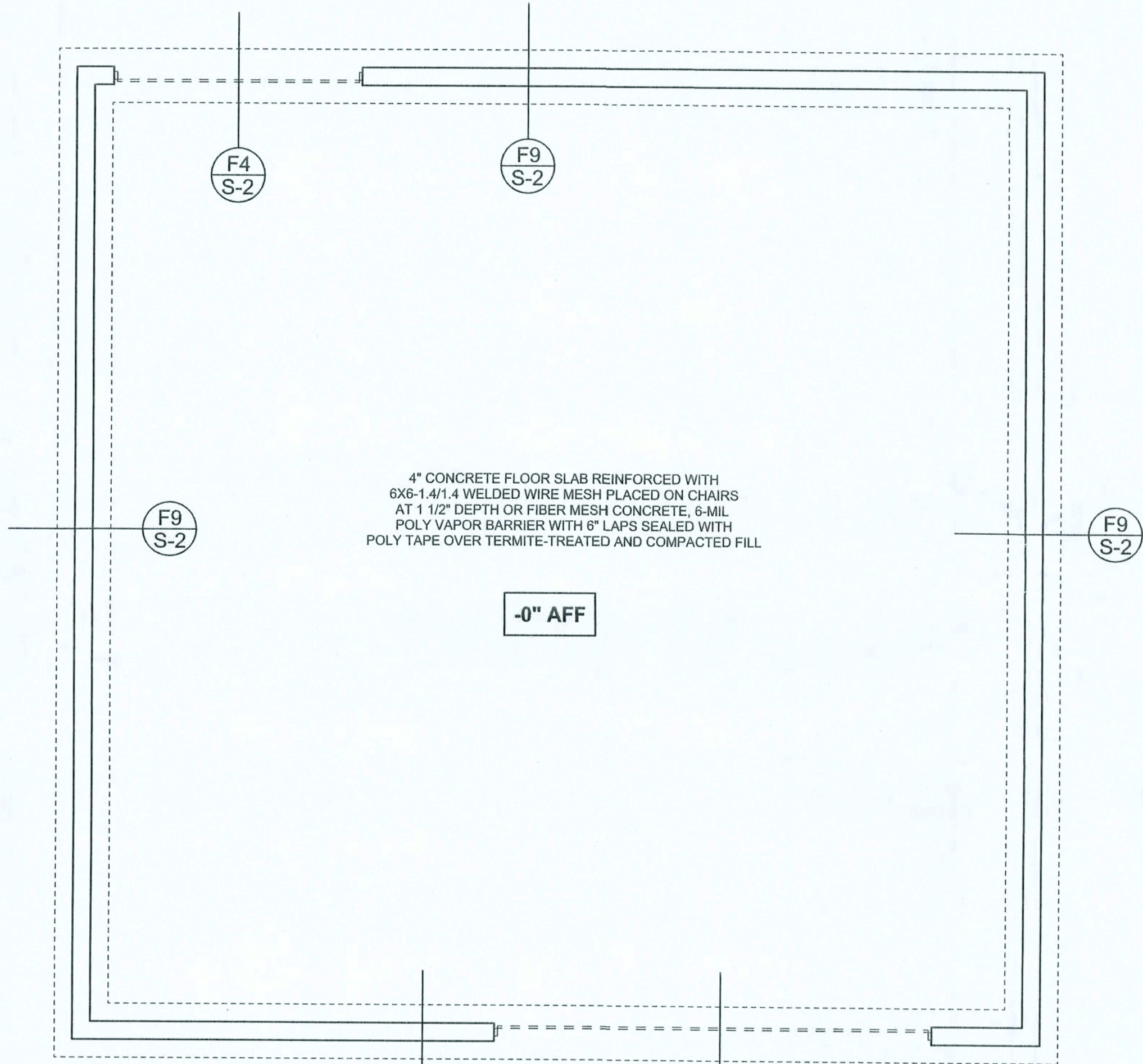


**F4**  
**S-2**    **GARAGE DOOR FOOTING**  
SCALE: 1/2" = 1'-0"

**TALL STEM WALL TABLE**

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Diaphragm ladder reinforcement at 16"OC vertically or a horizontal bent beam with 16" continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#	#8
3.3	3.0	96	96	96	96	9i	96
4.0	3.7	96	96	96	96	9i	96
4.7	4.3	88	96	96	96	9i	96
5.3	5.0	56	96	96	96	9i	96
6.0	5.7	40	80	96	80	9i	96
6.7	6.3	32	56	80	56	9i	96
7.3	7.0	24	40	56	40	8i	96
8.0	7.7	16	32	48	32	6i	80
8.7	8.3	8	24	32	24	4i	64
9.3	9.0	8	16	24	16	4i	48



**FOUNDATION PLAN**  
SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway, P.E. No. 53915, P.O. Box 868, Lake City, FL 32056, 386-754-5419

**DIMENSIONS:**  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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Mark Disosway, P.E. hereby expressly reserves his common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with section RS01.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOSWAY  
P.E. 53915

*Mark Disosway*  
SEAL

**IONNY JOHNSON**

**MIKE LINNANE  
GARAGE**

**ADDRESS:**  
153 SW Bethany Place  
Phase #3 Lot #9 Ruswood Estates,  
Columbia County, Florida

**Mark Disosway P.E.**  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871

**PRINTED DATE:**  
March 07, 2006

**DRAWN BY:** Ben Sparks    **STRUCTURAL BY:** Ben Sparks

**FINALS DATE:**  
07/ MAR / 06

**JOB NUMBER:**  
602151

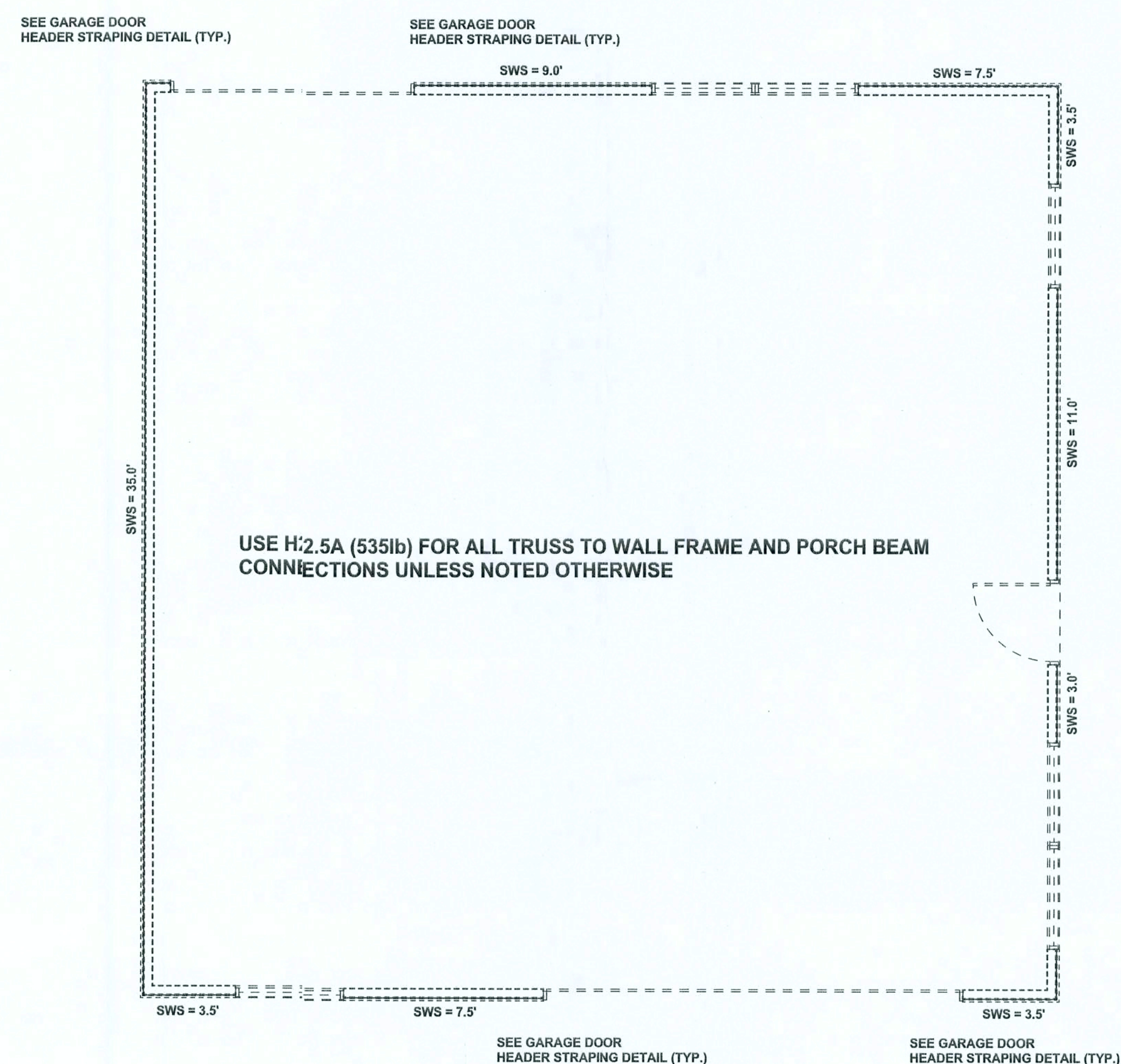
**DRAWING NUMBER**

**S-2**

OF 5 SHEETS



REVISIONS	







STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

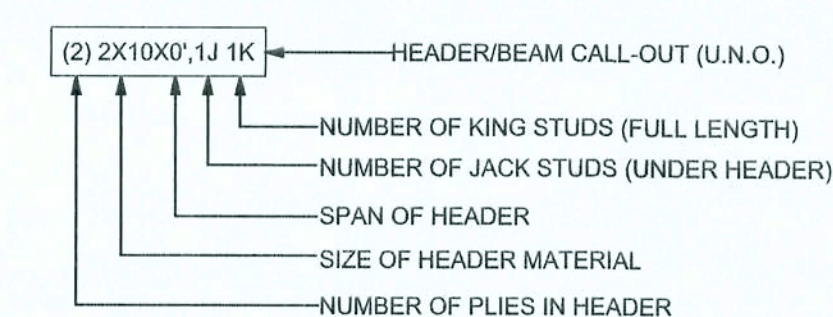
### STRUCTURAL PLAN NOTES

- |      |   |
|------|---|
| SN-1 | ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)  |
| SN-2 | ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)   |
| SN-3 | DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS  |
| SN-4 | PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BC51-13, BC51-81, BC51-82, & BC51-83. BC51-81, BC51-82, & BC51-83 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE |

WALL LEGEND

 <p>SWS = 0.0'</p>	<p>1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)</p>
 <p>SWS = 0.0'</p>	<p>2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)</p>
 <p>IBW</p>	<p>1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1</p>
 <p>IBW</p>	<p>2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1</p>

### HEADER LEGEND



## TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	13.0'	31.0'
LONGITUDINAL	10.6'	52.5'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED  
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING  
FURNISHED BY BUILDER. MAYO TRUSS CO.  
JOB #

JONNY JOHNSON

MIKE LINNANE  
GARAGE

ADDRESS:  
153 SW Bethany Place  
Phase #3 Lot #9 Russwood Estates,  
Columbia County, Florida

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Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871

PRINTED DATE

PRINTED DATE  
March 07, 2006

DRAWN BY:	STRU
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Ben Sparks	Ben Sp
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[illegible]

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FINALS DATE:	
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07/ MAR / 06

JOB NUMBER

602151

DRAWING NUMBER

53

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