

MATERIALS

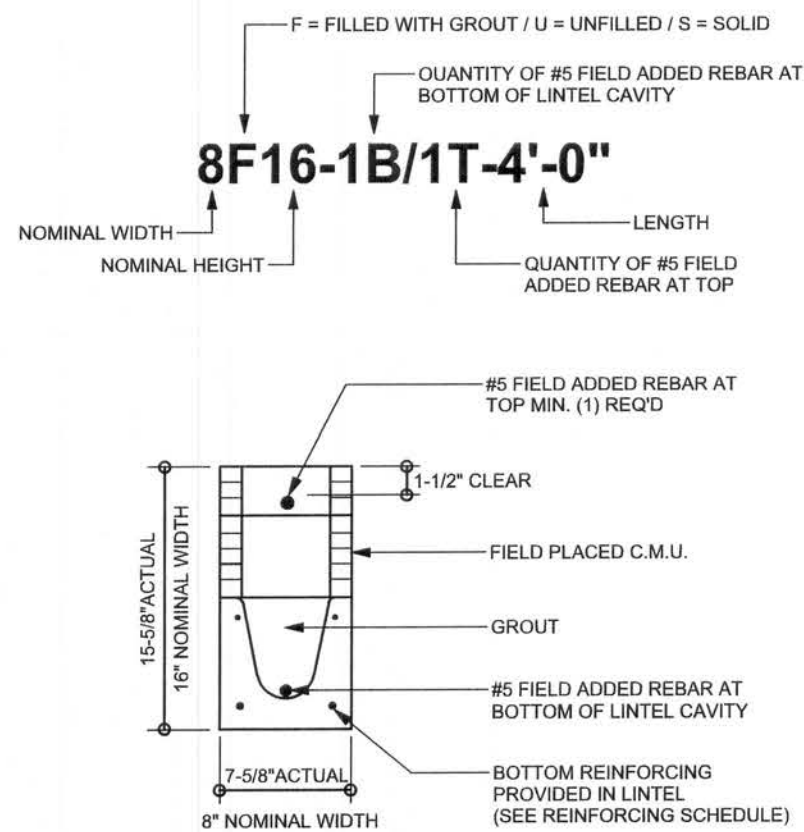
1. Fc 8" precast lintel = 3500 psi
2. Fc prestressed lintel = 6000 psi
3. Grout per ASTM C476 Fc = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump
4. Concrete Masonry Units (CMU) per ASTM C90 w/minimum net area compressive strength = 1900 psi
5. Rebar per ASTM A615 grade 60
6. Prestressing strand per ASTM A416 grade 270 low relaxation
7. Mortar per ASTM C270 type M or S

GENERAL NOTES

1. Provide full mortar bed and head joints.
2. Shore filled lintels as required.
3. Installation of lintel must comply with the architectural and/or structural documents.
4. U-Intels are manufactured with 5 1/2" long notches at the ends to accommodate vertical cell reinforcing and grouting.
5. All lintels meet or exceed L360 deflection, except lintels 17'-4" and longer with a nominal height of 8" meet or exceed U180 deflection.
6. Bottom field added rebar to be located at the bottom of the lintel cavity.
7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage.
8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.
9. Safe load rating based on rational design analysis per ACI 318 and ACI 530
10. Product Approvals: Miami-Dade County, Florida No. 03-0606.05
11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-296 or other approved coating.
12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:

$$\frac{\text{Applied vertical load}}{\text{Safe vertical load}} + \frac{\text{Applied horizontal load}}{\text{Safe horizontal load}} \leq 1.0$$
13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right:

TYPE DESIGNATION



SAFE LOAD TABLE NOTES

1. All values based on minimum 4 inch nominal bearing.
2. N.R. = Not Rated
3. Safe loads are superimposed allowable loads.
4. Safe loads based on grade 40 or grade 60 field rebar.
5. One #7 rebar may be substituted for two #6 rebars in 8" lintels only.
6. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at d-away from face of support.
7. For composite lintel heights not shown, use safe load from next lower height shown.
8. For lintel lengths not shown, use safe load from next longest length shown.
9. All safe loads in units of pounds per linear foot.
10. All safe loads based on simply supported span.
11. The number in the parenthesis indicates the percent reduction for grade 40 field added rebar.
 Example 7'-6" lintel type 8F32-1B safe gravity load = 6472(10.0469)(15)H0.0781; w/ 15% reduction 6472(.85) = 5501 plf

LENGTH	TYPE	S/F LOAD - POUNDS PER LINEAR FOOT									
		8F8-1B	8F12-1B	8F16-1B	8F20-1B	8F24-1B	8F28-1B	8F32-1B	8F36-1B	8F40-1B	8F44-1B
2'-10" (34")	PRECAST	2231	306	4605	6113	7547	8974	10394	11809	13224	14639
3'-6" (42")	PRECAST	2231	306	4605	6113	7547	8974	10394	11809	13224	14639
4'-0" (48")	PRECAST	1966	286	4605	6113	7547	8974	10394	11809	13224	14639
4'-8" (54")	PRECAST	1599	196	2110	2931	3753	4576	5400	6224	7048	7872
5'-4" (64")	PRECAST	1217	136	1438	1999	2560	3121	3682	4243	4804	5365
5'-10" (70")	PRECAST	1062	116	1173	1631	2090	2549	3008	3467	3926	4385
6'-6" (78")	PRECAST	908	122	2177	3480	4801	6122	7443	8764	10085	11406
7'-6" (90")	PRECAST	743	10	1729	2632	3535	4438	5341	6244	7147	8050
9'-4" (112")	PRECAST	554	75	1245	1943	2641	3339	4037	4735	5433	6131
10'-6" (126")	PRECAST	475	64	1052	1533	2013	2493	2973	3453	3933	4413
11'-4" (136")	PRECAST	362	58	945	1386	1826	2266	2706	3146	3586	4026
12'-0" (144")	PRECAST	337	54	873	1254	1684	2114	2544	2974	3404	3834
13'-4" (160")	PRECAST	296	47	755	1075	1428	1838	2238	2638	3038	3438
14'-0" (168")	PRECAST	279	42	706	1002	1326	1697	2127	2537	2947	3357
14'-8" (178")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
15'-4" (184")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
17'-4" (208")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
19'-4" (232")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
21'-4" (256")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
22'-0" (264")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
24'-0" (288")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.

SAFE UPLIFT LOADS FOR 8" PRECAST & PRESTRESSED U-INTELS

LENGTH	TYPE	SAFE UPLIFT LOAD - POUNDS PER LINEAR FOOT									
		8F8-1T	8F12-1T	8F16-1T	8F20-1T	8F24-1T	8F28-1T	8F32-1T	8F36-1T	8F40-1T	8F44-1T
2'-10" (34")	PRECAST	1972	311	4460	5747	7034	8321	9608	10895	12182	13469
3'-6" (42")	PRECAST	1972	311	4460	5747	7034	8321	9608	10895	12182	13469
4'-0" (48")	PRECAST	1599	252	3547	4589	5591	6593	7595	8597	9599	10601
4'-8" (54")	PRECAST	1207	194	2724	3508	4292	5077	5861	6646	7430	8215
5'-4" (64")	PRECAST	1016	165	2290	2949	3607	4265	4924	5582	6241	6899
5'-10" (70")	PRECAST	909	145	2093	2694	3295	3897	4498	5099	5699	6299
6'-6" (78")	PRECAST	835	134	1880	2419	2959	3498	4038	4578	5118	5658
7'-6" (90")	PRECAST	727	116	1634	2102	2571	3039	3508	3977	4446	4915
9'-4" (112")	PRECAST	591	85	1326	1795	2264	2733	3202	3671	4140	4609
10'-6" (126")	PRECAST	530	55	914	1195	1476	1757	2038	2319	2600	2881
11'-4" (136")	PRECAST	474	48	798	1034	1270	1506	1742	1978	2214	2450
12'-0" (144")	PRECAST	470	54	928	1349	1649	1949	2249	2549	2849	3149
13'-4" (160")	PRECAST	418	43	696	945	1194	1443	1692	1941	2190	2439
14'-0" (168")	PRECAST	428	45	770	1142	1444	1746	2048	2350	2652	2954
14'-8" (178")	PRESTRESSED	244	39	655	968	1324	1621	1918	2215	2512	2809
15'-4" (184")	PRESTRESSED	226	30	485	725	965	1205	1445	1685	1925	2165
17'-4" (208")	PRESTRESSED	192	30	500	732	963	1194	1425	1656	1887	2118
19'-4" (232")	PRESTRESSED	162	22	347	511	675	839	1003	1167	1331	1495
21'-4" (256")	PRESTRESSED	142	18	306	432	568	704	840	976	1112	1248
22'-0" (264")	PRESTRESSED	137	18	295	411	547	683	819	955	1091	1227
24'-0" (288")	PRESTRESSED	124	17	267	371	487	603	719	835	951	1067

SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-INTELS

LENGTH	TYPE	SAFE LOAD - POUNDS PER LINEAR FOOT									
		8R8-1B	8R12-1B	8R16-1B	8R20-1B	8R24-1B	8R28-1B	8R32-1B	8R36-1B	8R40-1B	8R44-1B
4'-4" (52")	PRECAST	1635	174	3355	4390	5425	6460	7495	8530	9565	10600
4'-6" (54")	PRECAST	1494	159	3093	4028	4963	5898	6833	7768	8703	9638
5'-8" (68")	PRECAST	866	92	1770	2277	2784	3291	3798	4305	4812	5319
5'-10" (70")	PRECAST	810	85	1653	2160	2667	3174	3681	4188	4695	5202
6'-8" (80")	PRECAST	797	91	1625	2132	2639	3146	3653	4160	4667	5174
7'-6" (90")	PRECAST	669	75	1490	1997	2504	3011	3518	4025	4532	5039
9'-8" (116")	PRECAST	411	46	999	1358	1717	2076	2435	2794	3153	3512

SAFE UPLIFT LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-INTELS

LENGTH	TYPE	SAFE UPLIFT LOAD - POUNDS PER LINEAR FOOT									
		8R8-1T	8R12-1T	8R16-1T	8R20-1T	8R24-1T	8R28-1T	8R32-1T	8R36-1T	8R40-1T	8R44-1T
4'-4" (52")	PRECAST	905	174	2635	3522	4409	5296	6183	7070	7957	8844
4'-6" (54")	PRECAST	867	167	2525	3374	4224	5074	5924	6774	7624	8474
5'-8" (68")	PRECAST	675	130	1960	2618	3277	3935	4594	5253	5912	6571
5'-10" (70")	PRECAST	655	126	1900	2538	3176	3815	4453	5092	5730	6369
6'-8" (80")	PRECAST	570	107	1651	2224	2798	3372	3946	4520	5094	5668
7'-6" (90")	PRECAST	506	97	1462	1952	2442	2932	3422	3912	4402	4892
9'-8" (116")	PRECAST	395	491	931	1211	1491	1771	2051	2331	2611	2891

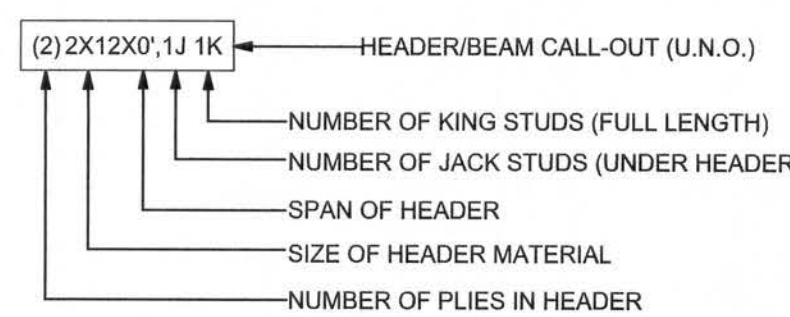
STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS1-B3, BCS1-B1, BCS1-B2, BCS1-B3, BCS1-B1, BCS1-B2, & BCS1-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND



DOOR & WINDOW BUCK ATTACHMENT

- 3/16" TAPCONS @ 16" O.C.
- 2 1/2" MIN. EMB. DISTANCE
- 1 1/4" MIN. EMB. DISTANCE
- 3" MIN. SPACING
- WINDOWS & DOORS UP TO 6'X8'
- WINDOWS & DOORS UP TO 8'X12'
- 3/16" TAPCONS @ 16" O.C.
- 1/4" TAPCONS @ 24" O.C.
- SLIDERS UP TO 8'X20"
- 3/16" TAPCONS @ 12" O.C.
- 1/4" TAPCONS @ 18" O.C.
- GARAGE DOOR UP TO 10'W
- (2) 3/16" TAPCONS & 16" O.C.
- (2) 1/4" TAPCONS & 24" O.C.
- GARAGE DOOR UP TO 18'W
- (2) 3/16" TAPCONS & 8" O.C.
- (2) 1/4" TAPCONS & 12" O.C.

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	15.8'	79.5'
LONGITUDINAL	14.5'	65.0'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, W.B. HOWLAND (JOB #4694)

FEVISIONS

12OCT07

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINNAD ENGINEER: Mark Disosway
P.E. No.3915, P.O.B. 868, Lake City, FL
32056-386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portion of the plan, relating to the engineering work herein, is true and correct to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 3915

SEAL 2007-08-09

Concept Construction
O'North Florida, Inc.

Rice Residence

ADDRESS:
232 NW Key Lime Court
Lake City, Florida

Mark Disosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
October 12, 2007

DRAWN BY:
Dad Disosway

CHECKED BY:

FINLS DATE:
05 Jul / 07

JOB NUMBER:
707032

DRAWING NUMBER
S-3
OF 3 SHEETS



AREA SUMMARY	
LIVING AREA	1961 SF
GARAGE	432 SF
PORCHES	320 SF
TOTAL	2713 SF

ELEVATION PLAN
SCALE: 1/4"=1'-0"

NEW SINGLE FAMILY DWELLING FOR:
THE RICE RESIDENCE
COLUMBIA COUNTY, FLORIDA

CONCEPT CONSTRUCTION
OF NORTH FLORIDA, INC.
2109 W. U.S. HWY 90 SUITE 110-144
LAKE CITY, FL 32255
(386) 185-8881

BRIAN S. CRAWFORD
ARCHITECTURAL DESIGN
DESIGNER: BRIAN CRAWFORD
PHONE: (386) 755-8887

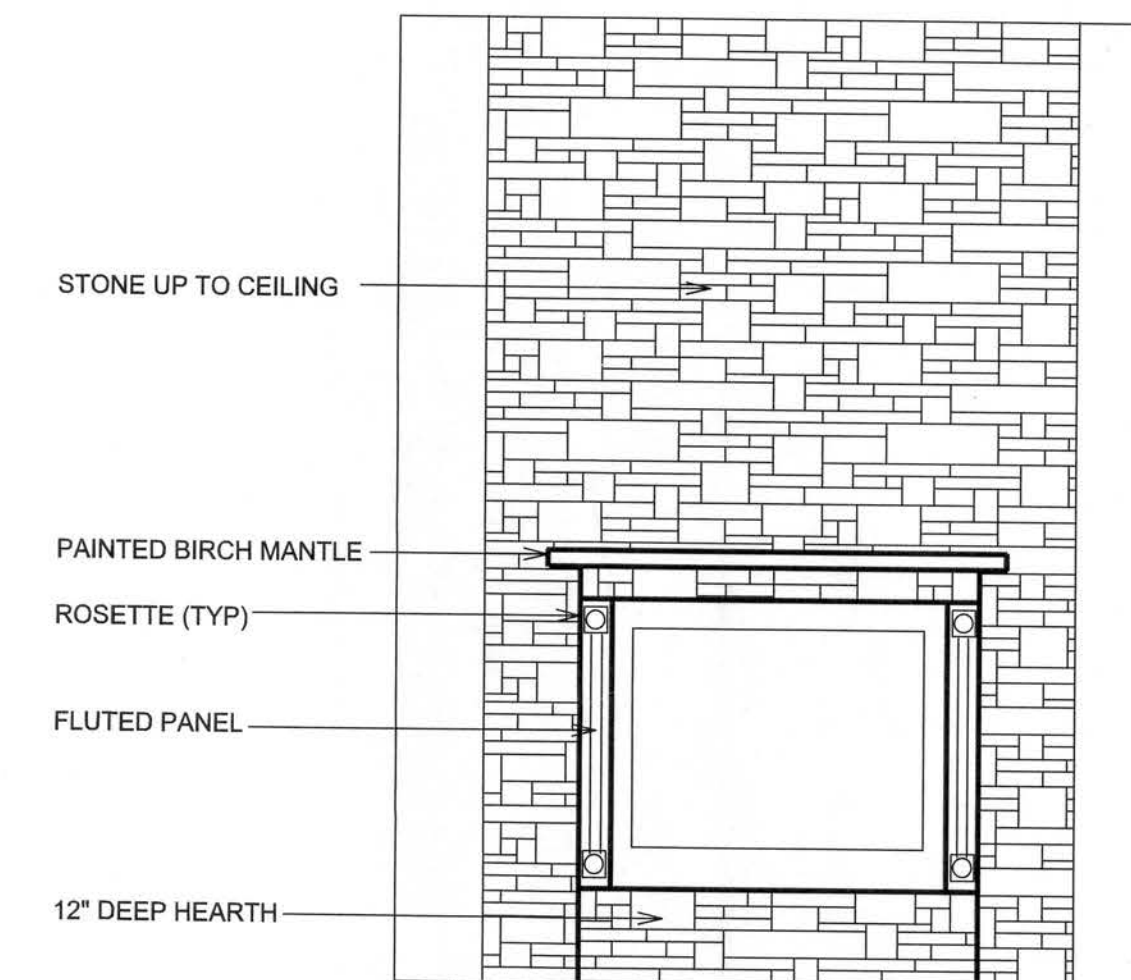
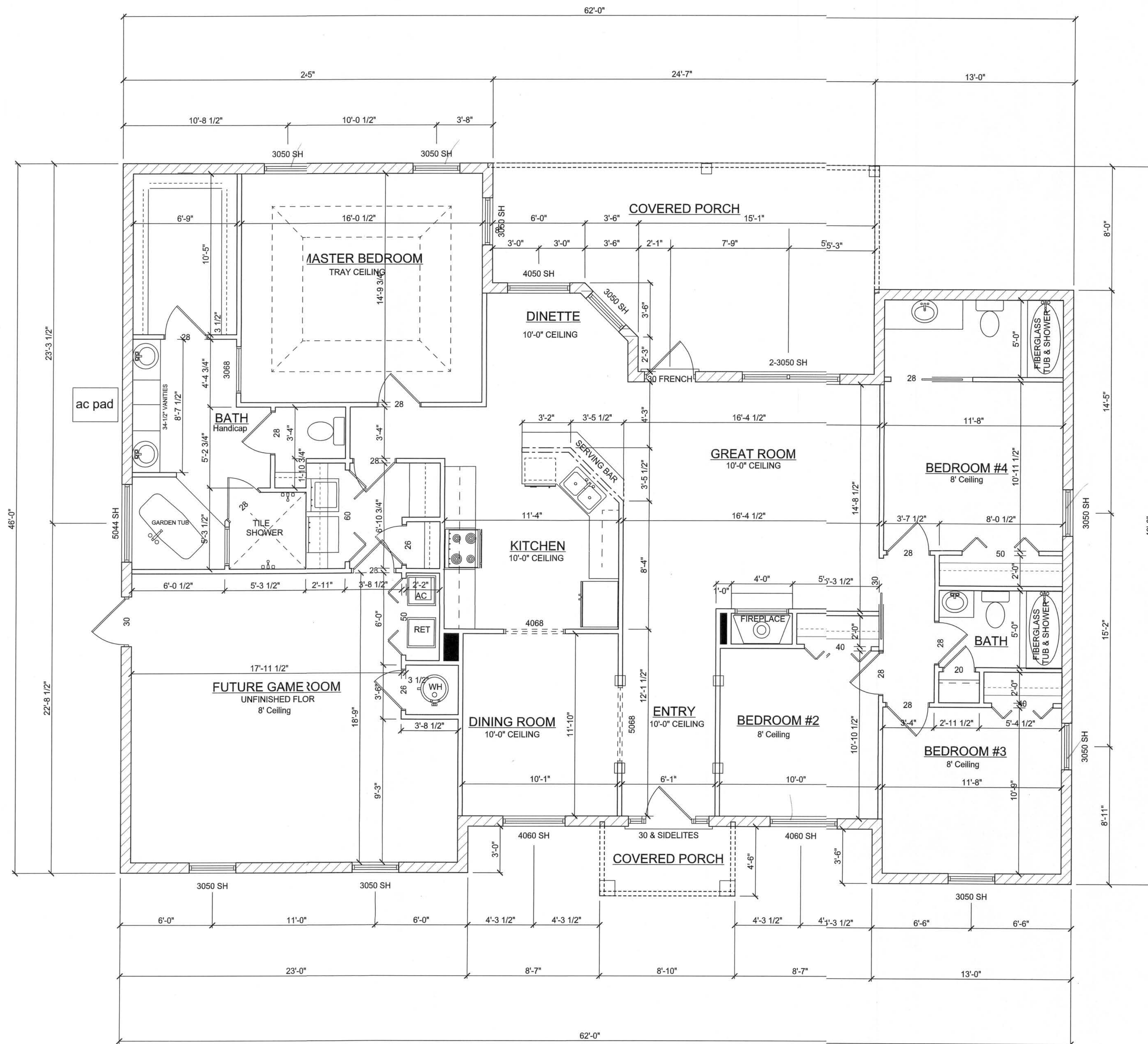
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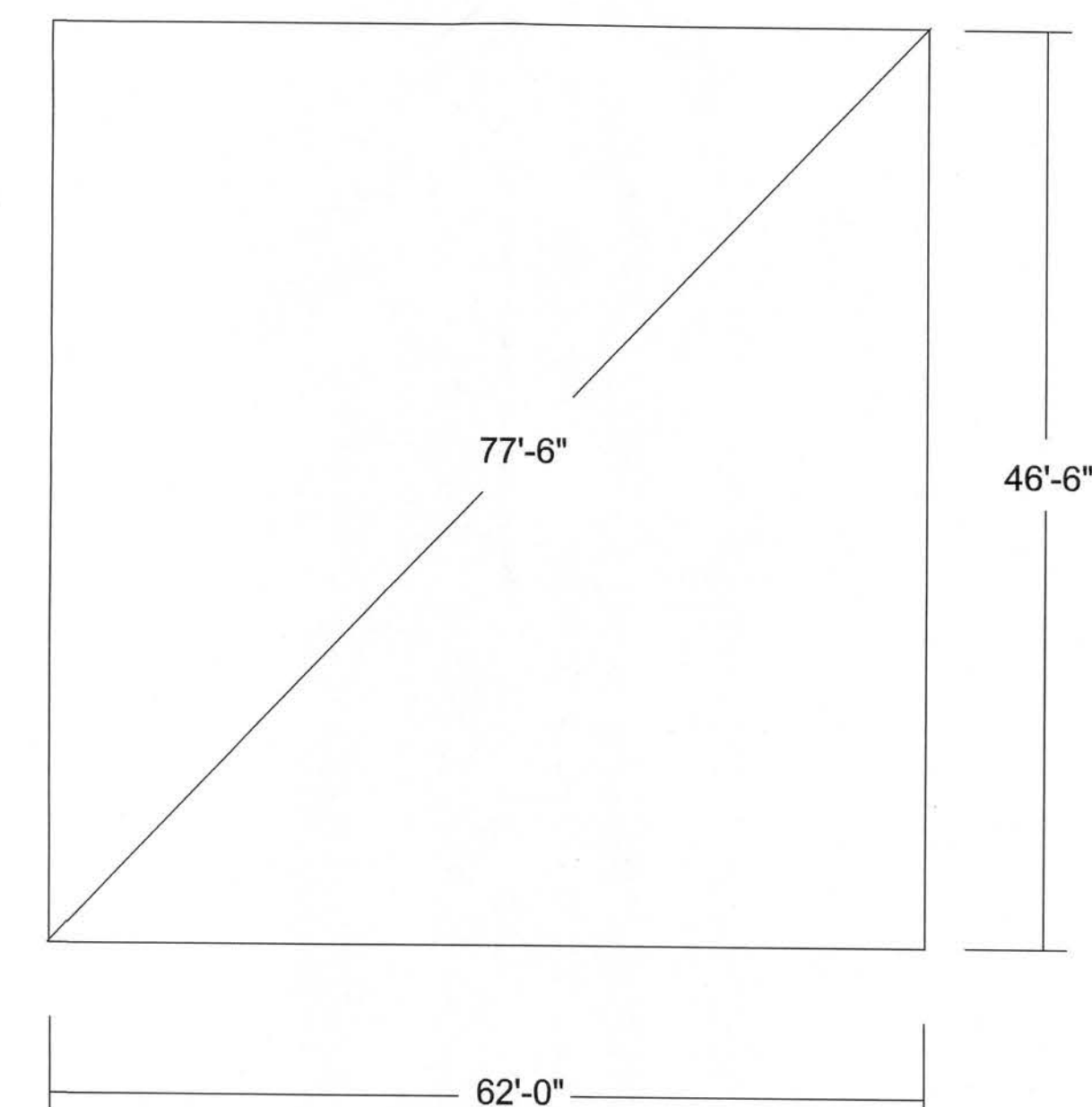
SHEET NUMBER

A-1

OF 4 SHEETS



FIREPLACE DETAIL



CHECK FOR SQUARE

AREA SUMMARY	
LIVING AREA	1961 SF
GARAGE	432 SF
PORCHES	320 SF
TOTAL	2713 SF

MAIN FLOORPLAN
SCALE: 1/4"=1'-0"

NEW SINGLE FAMILY DWELLING FOR:
THE RICE RESIDENCE
COLUMBIA COUNTY, FLORIDA

CONCEPT CONSTRUCTION
OF NORTH FLORIDA, INC.
2109 W. U.S. HWY 90 SUITE 170-144
LAKE CITY, FL 32055
(386) 755-6661

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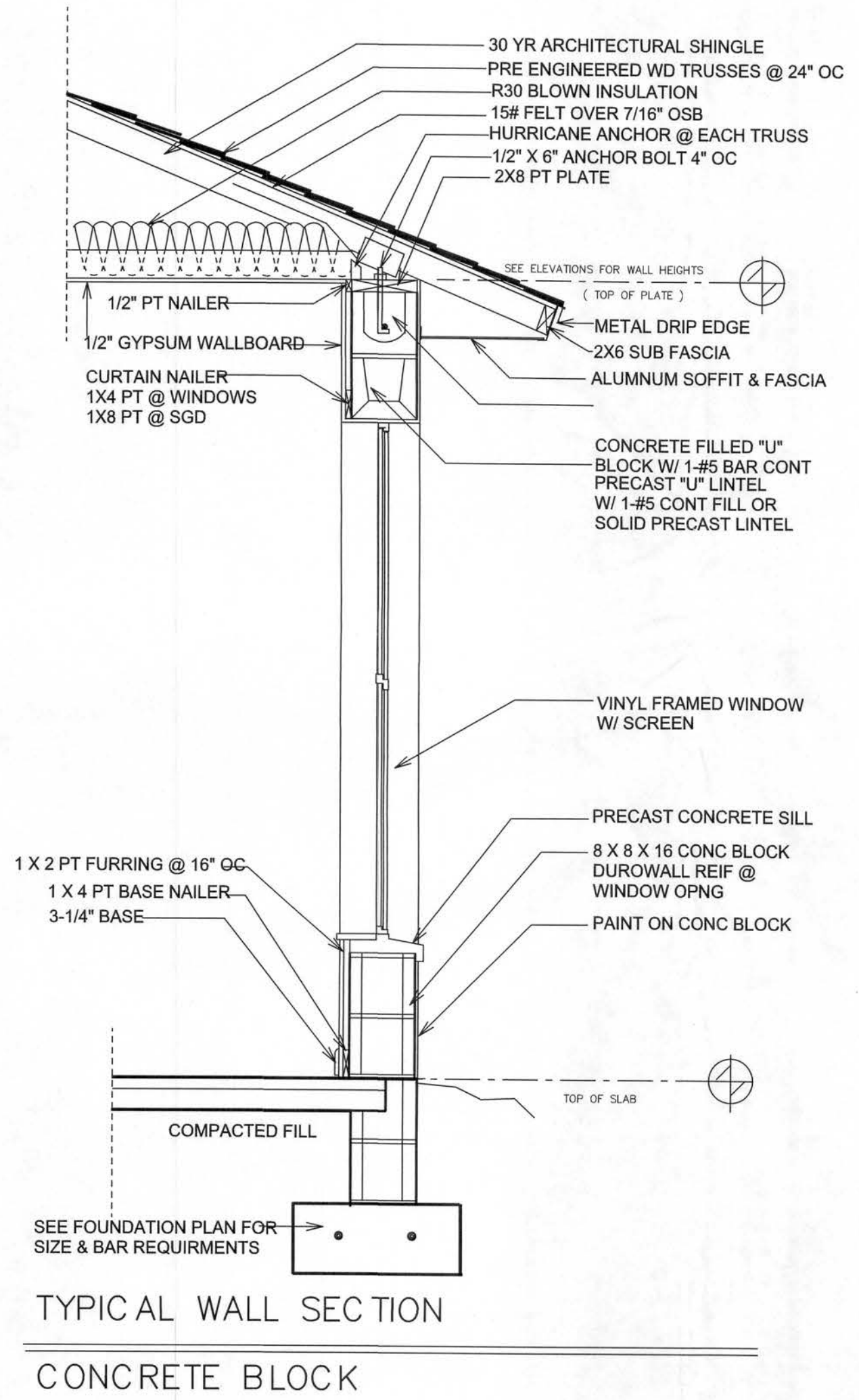
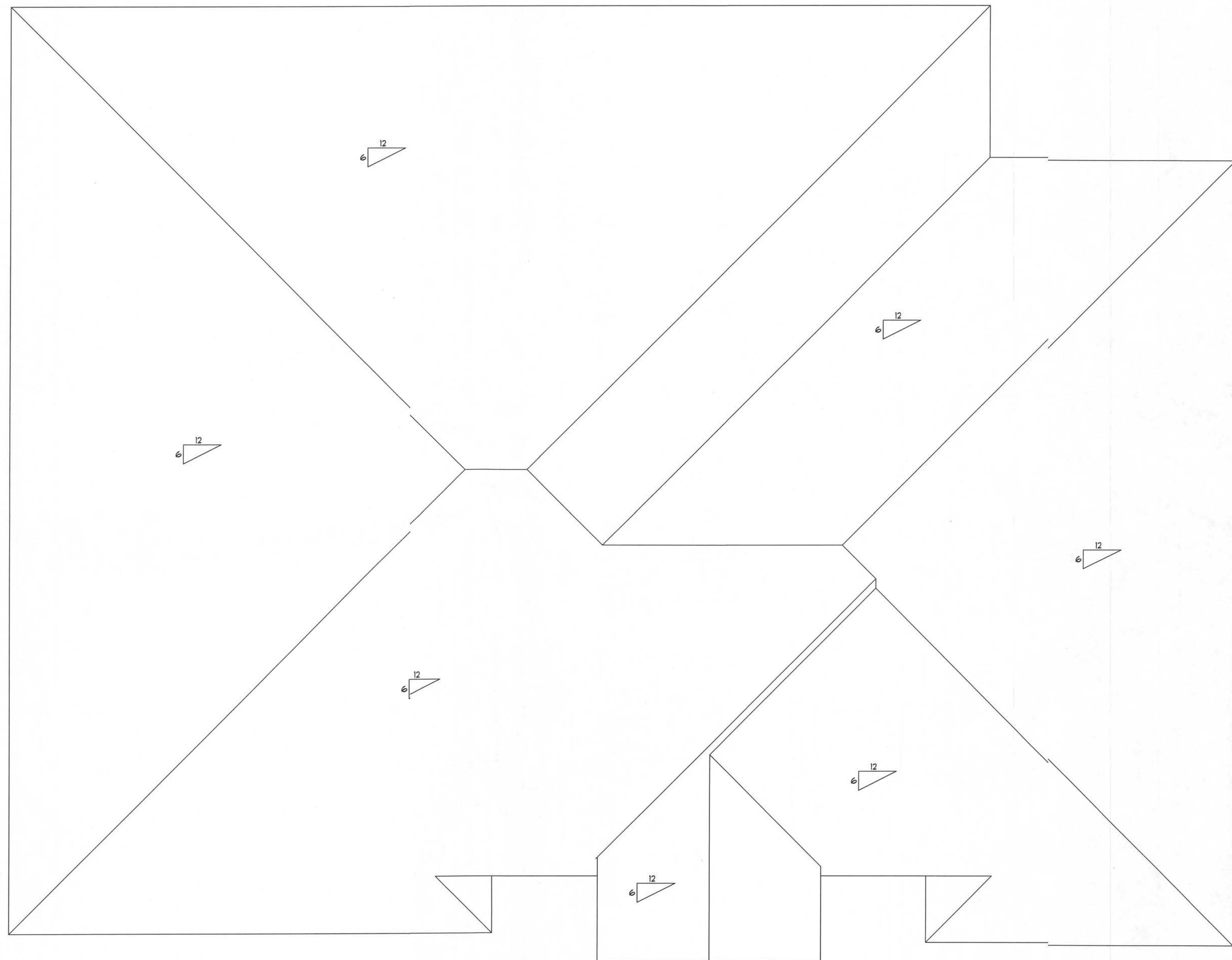
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SHEET NUMBER

A-2

OF 4 SHEETS



TYPICAL WALL SECTION
CONCRETE BLOCK

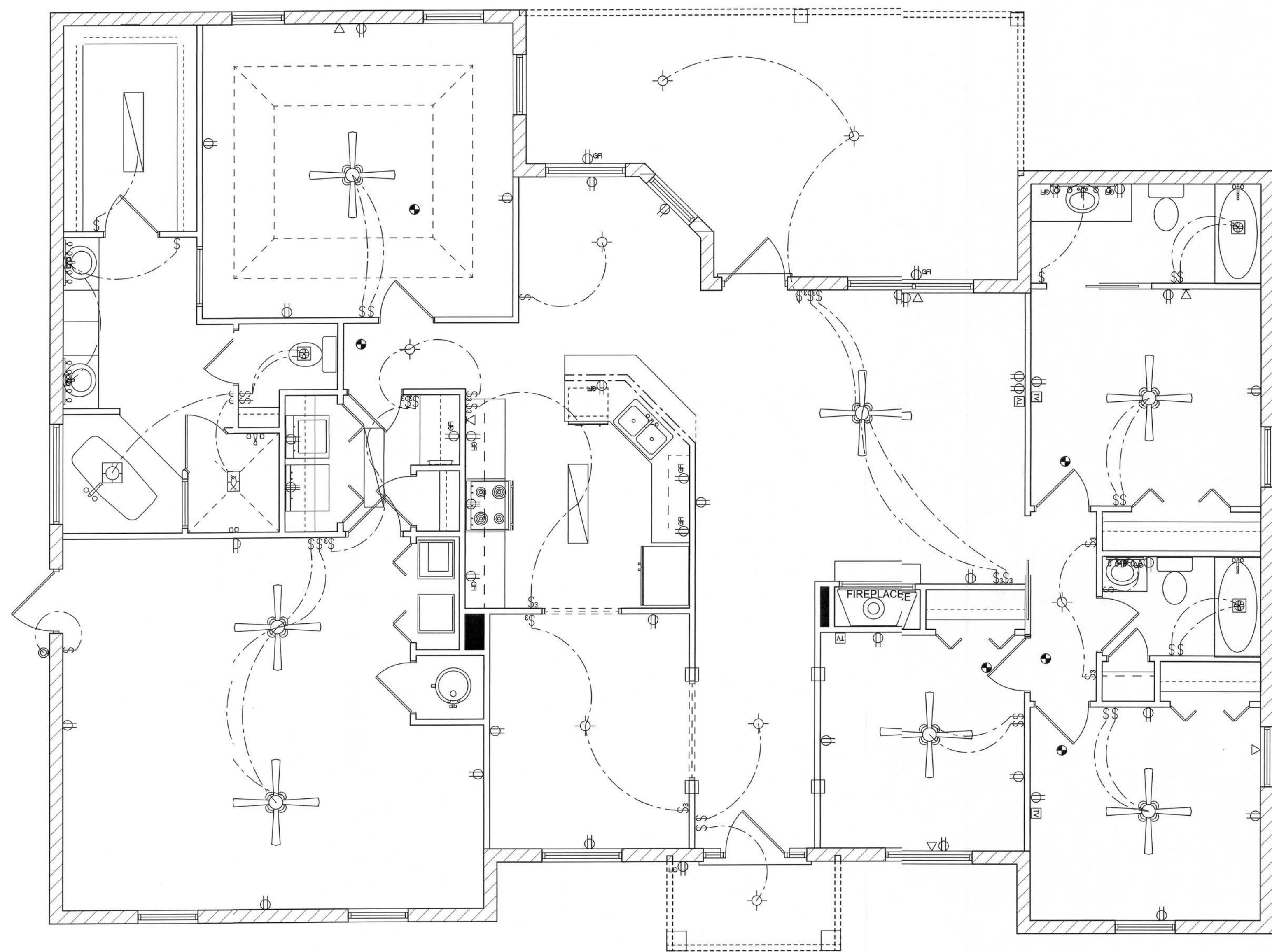
AREA SUMMARY	
LIVING AREA	1961 SF
GARAGE	432 SF
PORCHES	320 SF
TOTAL	2713 SF

MAIN FLOORPLAN
SCALE: 1/4"=1'-0"

NEW SINGLE FAMILY DWELLING FOR:
THE RICE RESIDENCE
COLUMBIA COUNTY, FLORIDA

BRIAN S. CRAWFORD
ARCHITECTURAL DESIGN
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DATE:
CHECKED BY:
SHEET NUMBER
A-3
OF 4 SHEETS



ELECTRICAL	COUNT	SYMBOL
ceiling fan spotlights 2	7	
Can Light	2	
fluorescent fixture	3	
vanity bar light	4	
wall mount 1	1	
electrical panel	1	
Fan - Light Combo	3	
cable tv outlet	4	
light	8	
outlet	30	
outlet 220v	2	
outlet gfi	12	
smoke detector	6	
switch	27	
switch 3 way	14	
telephone	6	

ELECTRICAL PLAN NOTES

ALL RECEPTALS IN ALL BEDROOMS SHALL BE AFCI CIRCUITS
WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

INSTALLATION SHALL BE PER NAT'L. ELECTRIC CODE.

ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

ELECTRICAL CONTR SHALL PREPARE "AS-BUILT" SHOP DWGS INDICATING ALL ELECTRICAL WORK, INCLUDING ANY CHANGES TO THE ELEC. PLAN, ADD'NS TO THE ELEC. PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE W/ ALL CKTS IDENTIFIED W/ CKT N., DESCRIPTION & BRKR. SERVICE ENT. & ALL UNDERGROUND WIRE LOCATIONS/ROUTING/DEPTH. RISER DIA. SHALL INCLUDE WIRE SIZES/TYPE & EQUIPMENT TYPE W/ RATINGS & LOADS. CONTRACTOR SHALL PROVIDE 1 COPY OF AS-BUILT DWGS TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY.

AREA SUMMARY

LIVING AREA	1961 SF
GARAGE	432 SF
PORCHES	320 SF
TOTAL	2713 SF

ELECTRICAL PLAN

SCALE: 1/4"=1'-0"

NEW SINGLE FAMILY DWELLING FOR:
THE RICE RESIDENCE
COLUMBIA COUNTY, FLORIDA

CONCEPT CONSTRUCTION
OF NORTH FLORIDA, INC.
2105 W U.S. HWY 90 SUITE 170-144
LAKE CITY, FL 32055
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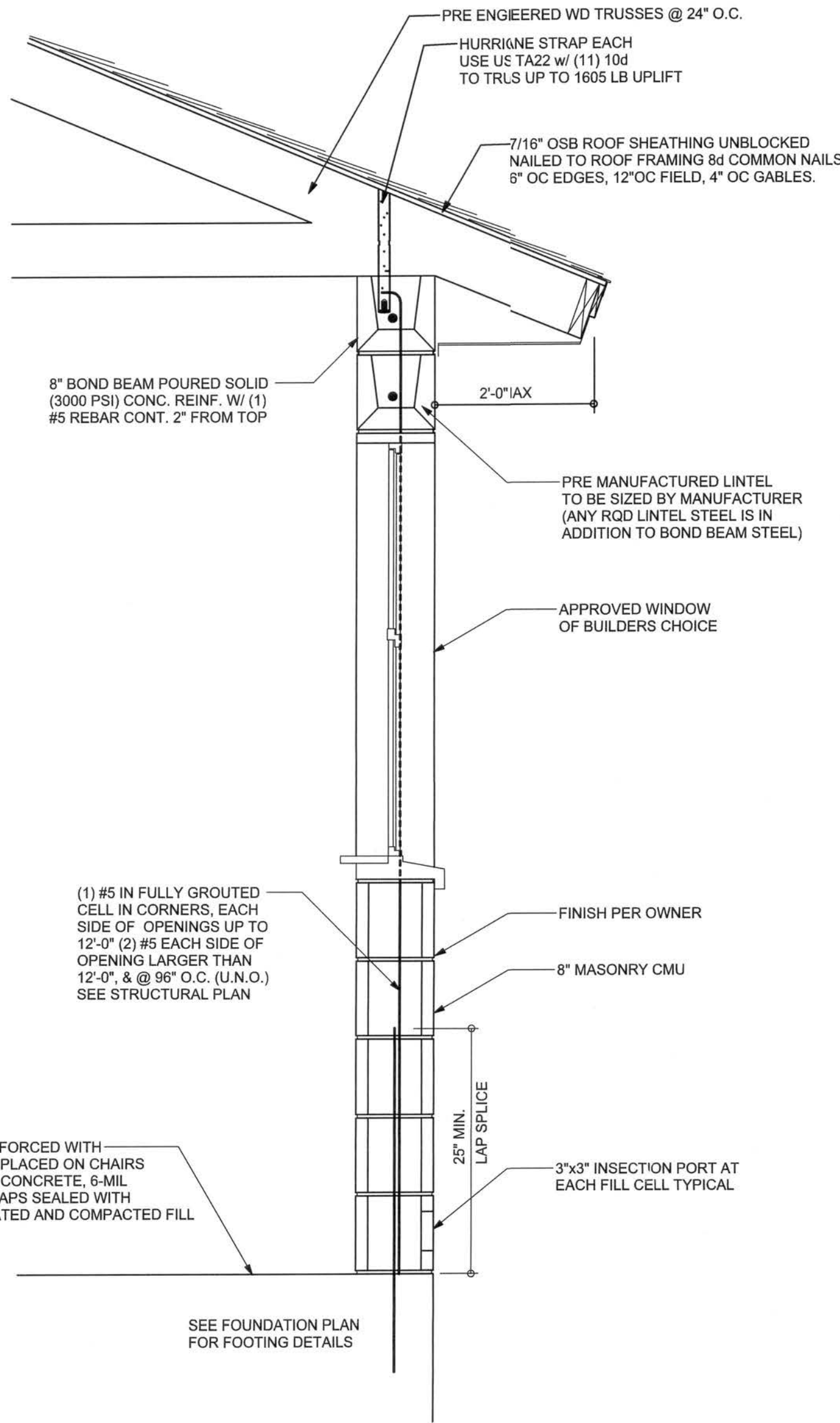
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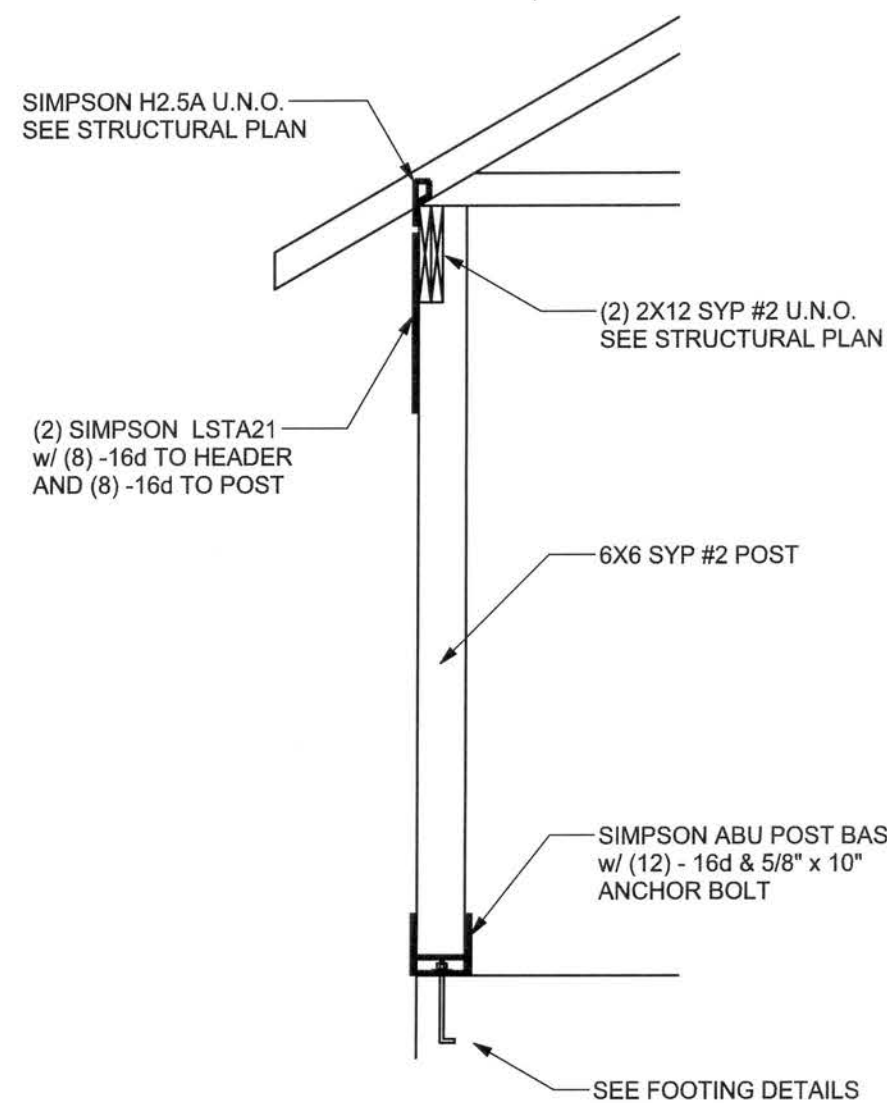
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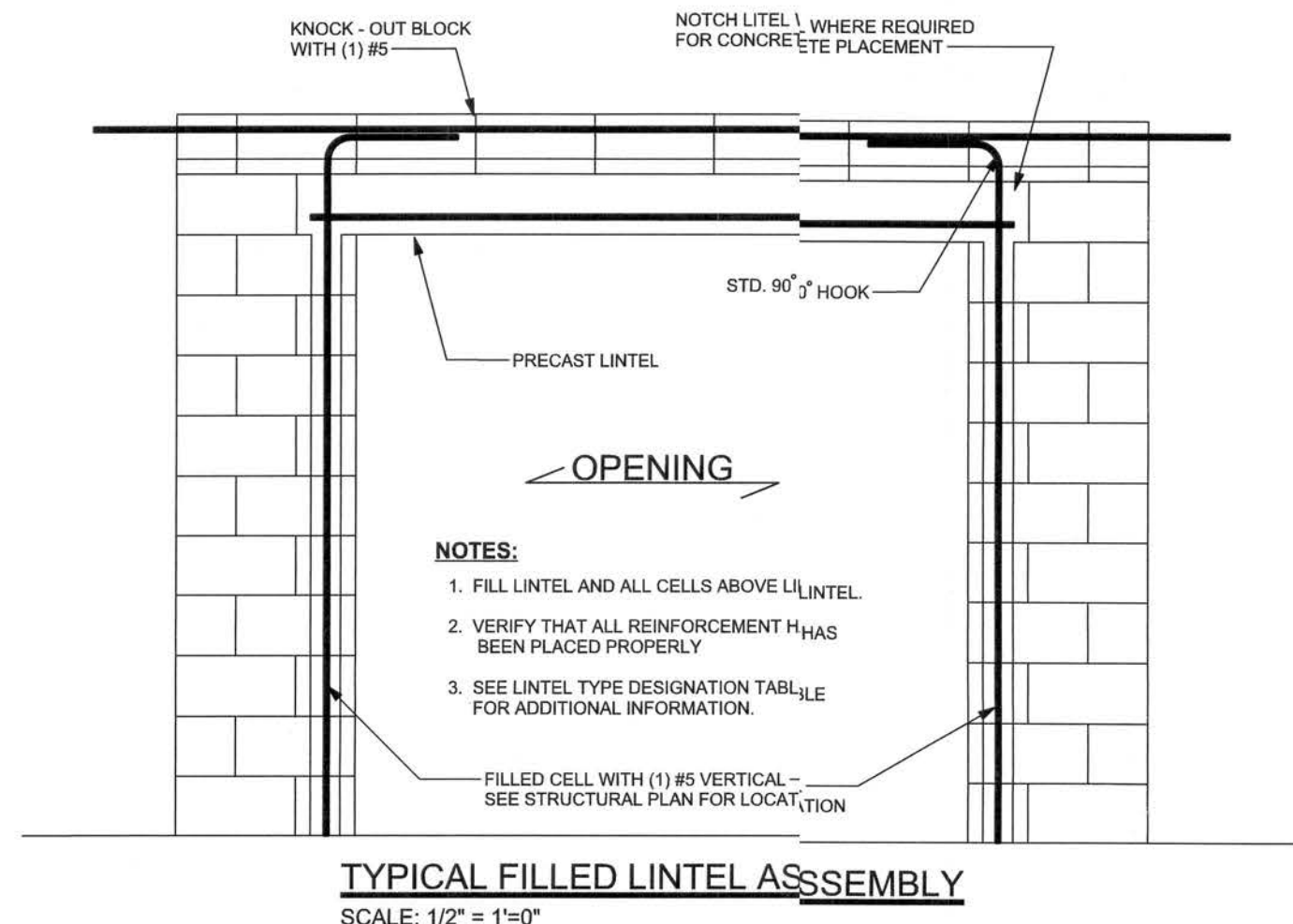
OF 4 SHEETS



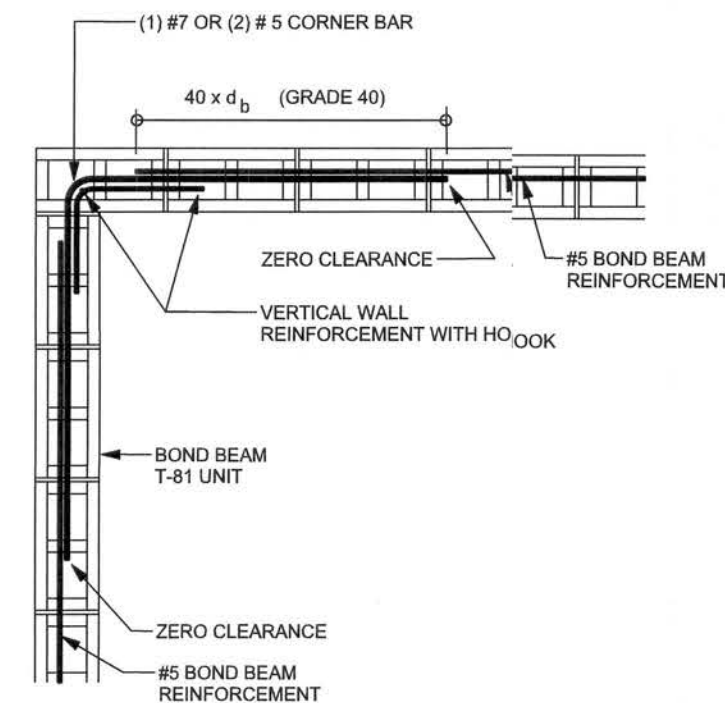
TYPICAL 1 STORY BLOCK WALL SECTION
SCALE: 1" = 1'-0"



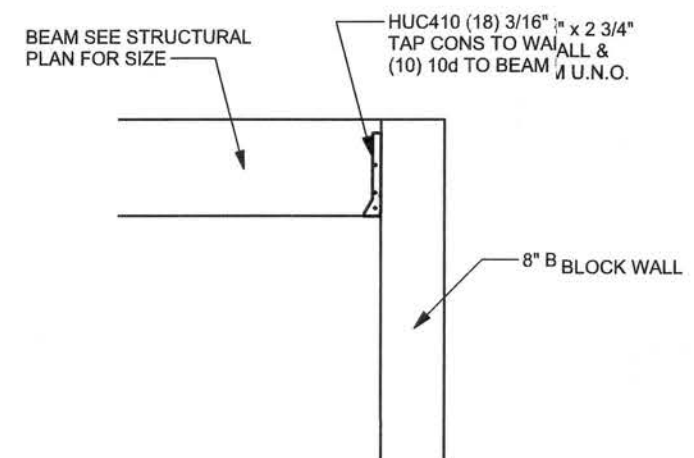
TYPICAL PORCH POST DETAIL
SCALE: 1/2" = 1'-0"



TYPICAL FILLED LINTEL ASSEMBLY
SCALE: 1/2" = 1'-0"



TYPICAL BOND BEAM CORNER DETAIL
SCALE: 1/2" = 1'-0"



BEAM TO BLOCK CONNECTION DETAIL
SCALE: 1/2" = 1'-0"

MASONRY TRUSS ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS.	TRUSS CONNECTOR MASONRY	
< 1205	TA22	10-10d x 1 1/2"
< 1605	TA22	11-10d
< 860	MTSM20	4 - 1/4" x 2 1/4" TITEN IN BLOCK 7 - 10d IN TRUSS
< 1175	HTSM20	4 - 1/4" x 2 1/4" TITEN IN BLOCK 10 - 10d IN TRUSS
< 1040	META20	7-10d, 1 1/2"
< 1490	META20	10-10d, 1 1/2"
< 1780	HETA20	7-16d
< 1780	LGT2	4 - 1/4" x 2 1/4" TITEN IN BLOCK 14 - 16d SINKER IN GIRDER
< 2130	HHETA20	10-10d, 1 1/2"
< 2310	HHETA24	21-10d, 1 1/2"
< 3965	MG1	22-10d TO TRUSS 5/8 AB TO WALL 15" EMBEDMENT
< 10980	HGT-2	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT
< 10530	HGT-3	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	HA	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 380	< 235	HA	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H8	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 650	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS34C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 - 16d	14 - 16d	
HEAVY GIRDER TIEDOWNS					
< 3965	< 3330	MG1		22 - 10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					
< 435	< 435	SSP DOUBLE TOP PLATE	3 - 10d		TO STUDS
< 455	< 420	SSP SINGLE SILL PLATE	1 - 10d		4 - 10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 - 10d		8 - 10d
< 825	< 600	DSP SINGLE SILL PLATE	2 - 10d		8 - 10d
< 885	< 780	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 780	SP8			6-10d, 1 1/2"
< 1240	< 1065	SPH8			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14 - 10d		
< 1235	< 1235	LSTA21	16 - 10d		
< 1030	< 1030	CS20	18 - 8d		
< 1705	< 1705	CS16	28 - 8d		
STUD ANCHORS*					
< 1350	< 1305	LTT19	8 - 16d		1/2" AB
< 2310	< 2310	LTT31	18 - 10d, 1 1/2"		1/2" AB
< 2775	< 2570	H2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	HPAHD2	16 - 16d		
< 3335	< 3335	HPAHD22	16 - 16d		
< 2200	< 2200	ABU44	12 - 16d		1/2" AB
< 2300	< 2300	ABU66	12 - 16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

GRADE & SPECIES TABLE

		F _b (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-9" WALL HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" WALL HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" WALL HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" WALL HEIGHT

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 4x16d EACH END, 2X6 RAFTERS TO 16d EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSF.

WELDED WIRE REINFORCED SLAB: 6" x 8" W1.4 x W1.4, F_y = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT 3" SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD. PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116, SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1/2 HOURS OF SLAB PLACEMENT, THE LENGTH / CUT W/M OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F_b = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT, AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE TRUSS DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.) BASIC WIND SPEED = 110 MPH
2.) WIND EXPOSURE = B
3.) WIND IMPORTANCE FACTOR = 1.0
4.) BUILDING CATEGORY = II
5.) ROOF ANGLE = 10-45 DEGREES
6.) MEAN ROOF HEIGHT = <30 FT
7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)	
1	19.9 - 21.8	18.1 - 18.1
2	19.9 - 25.5	18.1 - 21.8
2 Oth	40.6	40.6
3	19.9 - 25.5	18.1 - 21.8
3 Oth	68.3	42.4
4	21.8 - 23.6	18.5 - 20.4
5	21.8 - 29.1	18.5 - 22.6
Doors & Windows (Zone 5, 10 ft ²)	21.8	29.1
Worst Case		
8x7 Garage Door	19.5	22.9
16x7 Garage Door	18.5	21.0

DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
	STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)
	SOIL BEARING CAPACITY 1000PSF
	NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway,
P.E. No. 53915, POB 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915

SEAL

Concept Construction
Of North Florida, Inc.

Rice Residence

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PRINTED DATE:
July 05, 2007

DRAWN BY: David Disoway

CHECKED BY:

FINALS DATE:
05 / Jul / 07

JOB NUMBER:
707032

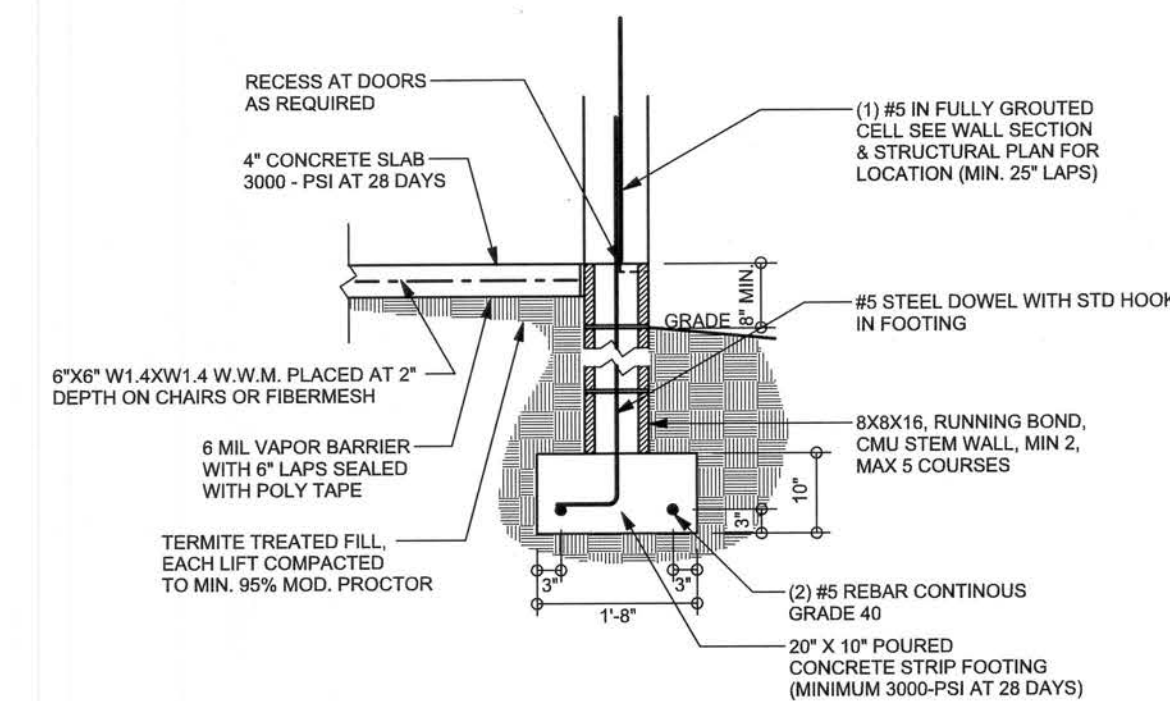
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S-1

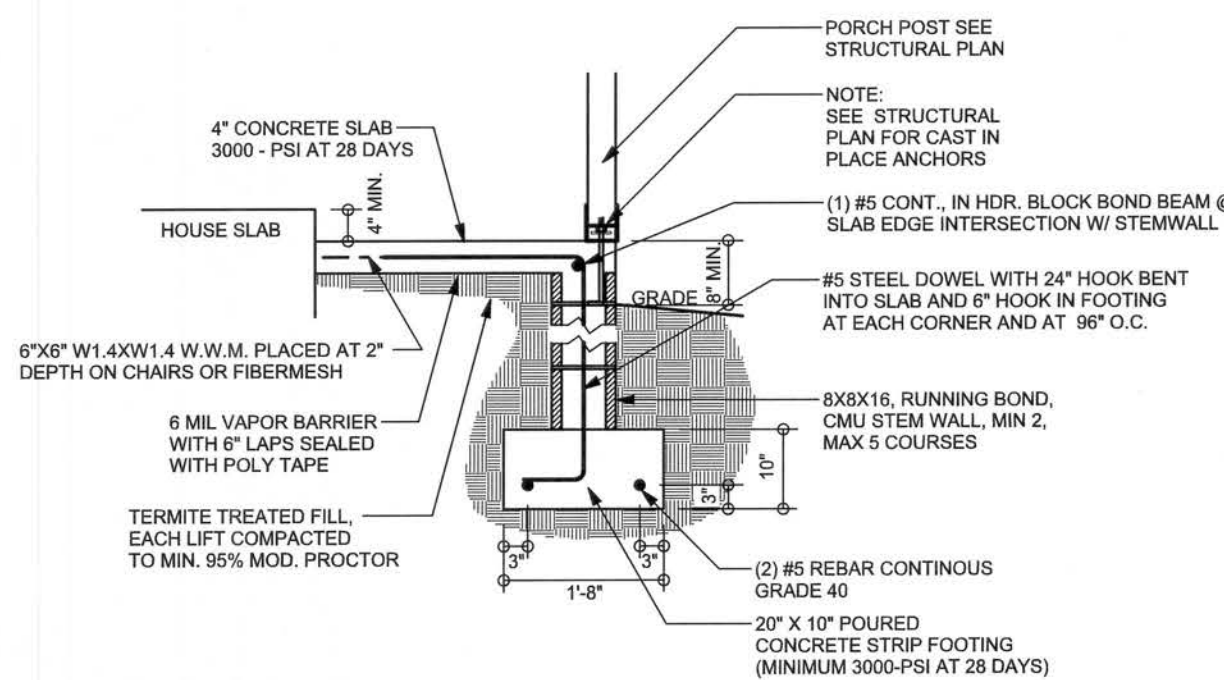
OF 3 SHEETS

REVISIONS

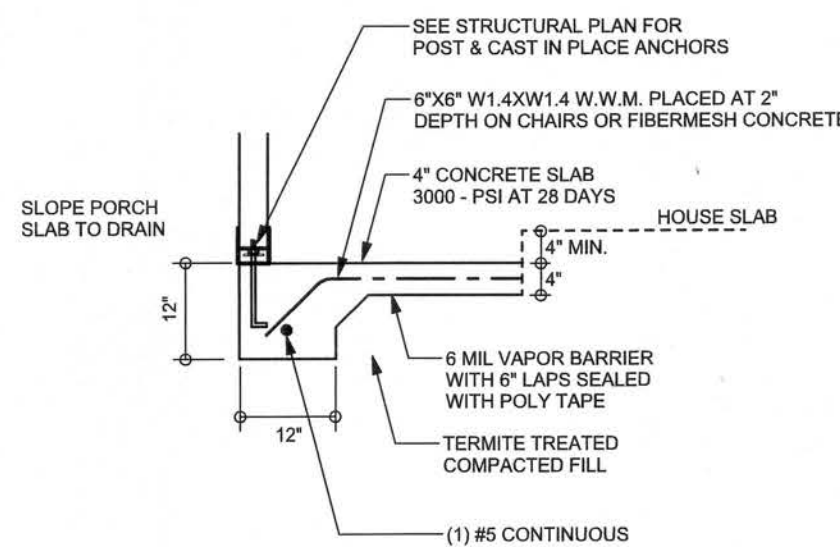
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



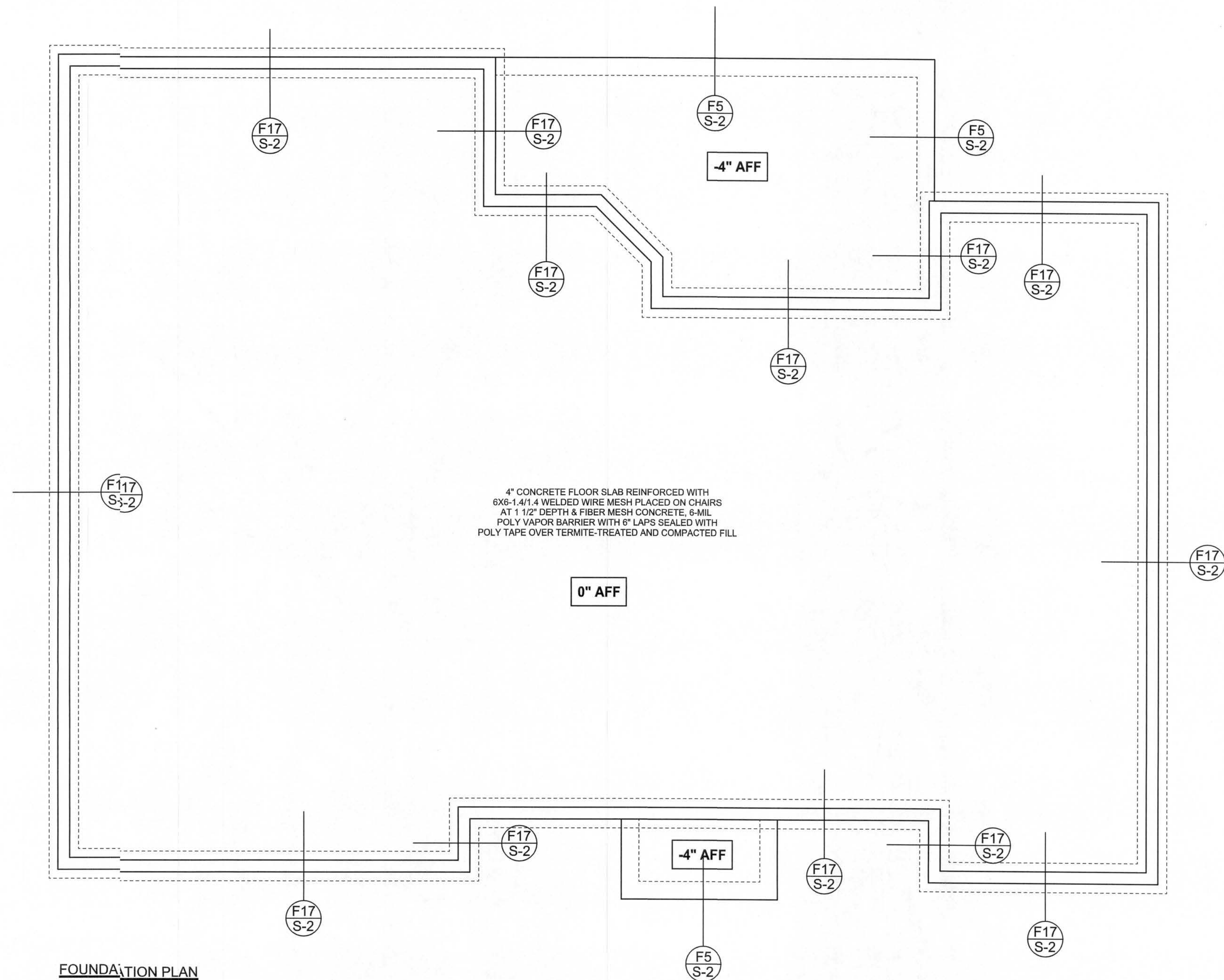
F17 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



F12 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS CON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN 1 FOR ACTUAL DIMENSIONS

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI/530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F'm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/#2 or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/#2 or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

WINDLOAD ENGINEER: Mark Disosway,
FE No. 53915, POB 868, Lake City, FL
33506, 386-754-5419

NOTES:
Sized dimensions supersede scaled
dimensions. Refer all questions to
Mark Disosway, P.E. for resolution.
Do not proceed without clarification.

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form or manner without first the express written
permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
provisions of the plan, relating to wind engineering
comply with section R301.2.1, Florida building
code residential 2004, to the best of my
knowledge.

LIMITATION: This design is valid for one
building, at specified location.

MARK DISOSWAY
P.E. 53915

05 JUL 07
SEAL

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PRINTED DATE:
July 05, 2007

DRAWN BY:
David Disosway

CHECKED BY:

FINALS DATE:
05 / Jul / 07

JOB NUMBER:
707032

DRAWING NUMBER

S-2

OF 3 SHEETS

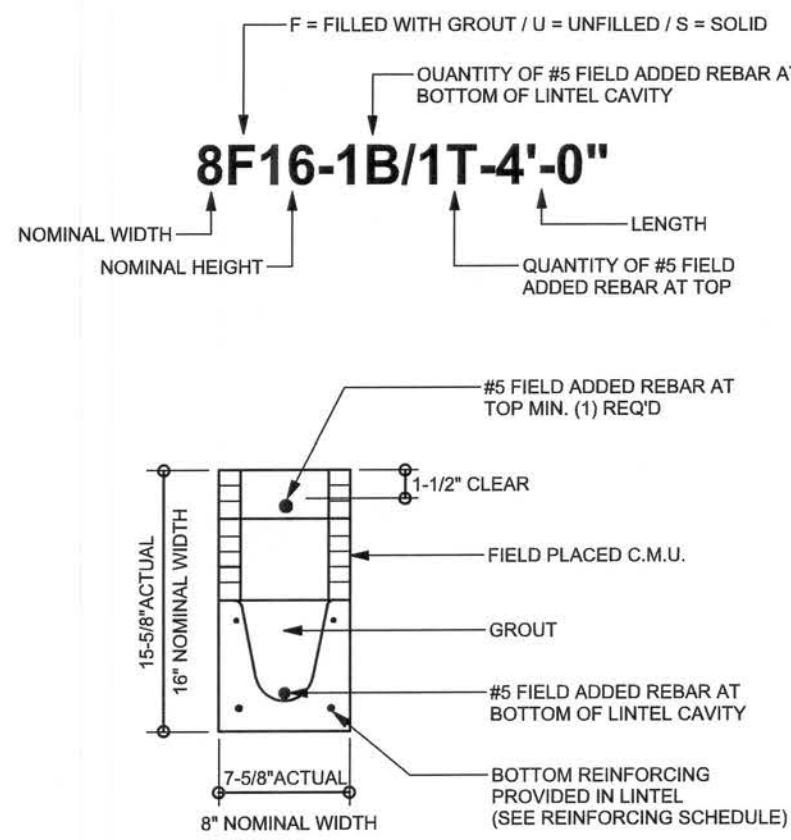
MATERIALS

1. F_c 8" precast lintel = 3500 psi
2. F_c prestressed lintel = 6000 psi
3. Grout per ASTM C478 F_c = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump
4. Concrete Masonry Units (CMU) per ASTM C90 minimum net area compressive strength = 1800 psi
5. Rebar per ASTM A615 grade 60
6. Prestressing strand per ASTM A416 grade 270 low relaxation
7. Mortar per ASTM C270 type M or S

GENERAL NOTES

1. Provide full mortar bed and head joints.
 2. Shore filled lintels as required.
 3. Installation of lintel must comply with the architectural and/or structural documents.
 4. U-Intels are manufactured with 5 1/2" long notches at the ends to accommodate vertical cell reinforcing and grouting.
 5. All lintels meet or exceed L360 deflection, except lintels 17'-4" and longer with a nominal height of 8' meet or exceed L180 deflection.
 6. Bottom field added rebar to be located at the bottom of the lintel cavity.
 7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage.
 8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.
- Applied vertical load + Applied horizontal load
Safe vertical load + Safe horizontal load ≤ 1.0
9. Safe load rating based on rational design analysis per ACI 318 and ACI 530
 10. Product Approvals: Miami-Dade County, Florida No. 03-0606.05
 11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-296 or other approved coating.
 12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:
 13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right.

TYPE DESIGNATION



SAFE LOAD TABLE NOTES

1. All values based on minimum 4 inch nominal bearing.
- Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 6 1/2 inches.
2. N.R. = Not Rated
3. Safe loads are superimposed allowable loads.
4. Safe loads based on grade 40 or grade 60 field rebar.
5. One #7 rebar may be substituted for two #5 rebars in 8" lintels only
6. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at d-away from face of support.

7. For composite lintel heights not shown, use safe load from next lower height shown.
 8. For lintels lengths not shown, use safe load from next longest length shown.
 9. All safe loads in units of pounds per linear foot.
 10. All safe loads based on simply supported span.
 11. The number in the the parenthesis indicates the percent reduction for grade 40 field added rebar.
- Example 7'-5" lintel type 8F32-1B safe gravity load = 6472(0.0469)(15)H0.0781 w/ 15% reduction 6472 $\Rightarrow$ (85) = 5501 plf

SAFE GRAVITY LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

LENGTH	TYPE	#5	SAFE LOAD - POUNDS PER LINEAR FOOT							
			8F8-0B	8F12-0B	8F16-0B	8F20-0B	8F24-0B	8F28-0B	8F32-0B	8F32-1B
2'-10" (34")	PRECAST	21	3089	4605	6113	7547	8974	10394	11809	
3'-6" (42")	PRECAST	21	3089	4605	6113	7547	8974	10394	11809	
4'-0" (48")	PRECAST	16	2561	2751	3820	4890	5961	7034	8107	
4'-6" (54")	PRECAST	19	2693	4605	6113	7547	8974	10394	11809	
5'-4" (64")	PRECAST	17	1989	2110	2921	3753	4576	5400	6224	
5'-10" (70")	PRECAST	12	1399	1438	1999	2590	3123	3686	4248	
6'-6" (78")	PRECAST	9	1105	1173	1631	2080	2549	3009	3470	
7'-6" (90")	PRECAST	3	1451	2622	4360	7169 (45)	6036 (19)	7181 (19)	8328 (20)	
9'-4" (112")	PRECAST	8	1238	2177	3480	5051	5707	6383	7061	
10'-6" (126")	PRECAST	6	1011	1729	2632	3205	3698	4191	4685	
11'-4" (136")	PRECAST	2	689	1160	1625	2564	3486	4408	5330	
12'-0" (144")	PRECAST	7	752	1245	1843	2564	3486	4408	5330	
13'-4" (160")	PRECAST	3	535	966	1247	2080	2777	3486	4191	
14'-0" (168")	PRECAST	3	442	706	1002	1326	1697	2127	2630	
14'-8" (176")	PRESTRESSED	R	NR	NR	NR	NR	NR	NR	NR	
15'-4" (184")	PRESTRESSED	R	NR	NR	NR	NR	NR	NR	NR	
17'-4" (208")	PRESTRESSED	R	NR	NR	NR	NR	NR	NR	NR	
19'-4" (232")	PRESTRESSED	R	NR	NR	NR	NR	NR	NR	NR	
21'-4" (256")	PRESTRESSED	R	NR	NR	NR	NR	NR	NR	NR	
22'-0" (264")	PRESTRESSED	R	NR	NR	NR	NR	NR	NR	NR	
24'-0" (288")	PRESTRESSED	R	NR	NR	NR	NR	NR	NR	NR	

SAFE UPLIFT LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

LENGTH	TYPE	#5	SAFE UPLIFT LOADS - POUNDS PER LINEAR FOOT							
			8F8-1T	8F12-1T	8F16-1T	8F20-1T	8F24-1T	8F28-1T	8F32-1T	8F32-2T
2'-10" (34")	PRECAST	72	3173	4460	5747	7034	8321	9608		
3'-6" (42")	PRECAST	39	2524	3547	4569	5591	6613	7636		
4'-0" (48")	PRECAST	33	2192	3079	3966	4853	5740	6627		
4'-6" (54")	PRECAST	33	2192	3079	3966	4853	5740	6627		
5'-4" (64")	PRECAST	17	1940	2724	3508	4292	5077	5861		
5'-10" (70")	PRECAST	16	1832	2290	2949	3607	4265	4924		
6'-6" (78")	PRECAST	9	1492	2083	2694	3295	3897	4498		
7'-6" (90")	PRECAST	5	1340	1880	2415	2959	3498	4038		
9'-4" (112")	PRECAST	7	1021	1634 (12)	2102 (11)	2571 (10)	3039 (10)	3508 (10)		
10'-6" (126")	PRECAST	0	542	914	1185 (14)	1458 (13)	1732 (12)	2007 (11)		
11'-4" (136")	PRECAST	0	686	1183	1526	1865	2204	2544		
12'-0" (144")	PRECAST	4	485	798 (11)	1034 (10)	1272 (9)	1510 (8)	1749 (7)		
13'-4" (160")	PRECAST	0	599	1028	1422	1738	2053	2369		
14'-0" (168")	PRECAST	0	543	908	1244	1569	1894	2219		
14'-8" (176")	PRESTRESSED	8	373	606 (10)	783 (9)	962 (8)	1141 (7)	1321 (6)		
15'-4" (184")	PRESTRESSED	0	455	770	1145	1444	1718	1993		
17'-4" (208")	PRESTRESSED	0	420	709	1050	1434	1804	2174		
19'-4" (232")	PRESTRESSED	2	303	500	732	993	1288	1583		
21'-4" (256")	PRESTRESSED	2	222	347 (10)	446 (9)	546 (8)	646 (7)	746 (6)		
22'-0" (264")	PRESTRESSED	2	198	306 (11)	393 (10)	480 (9)	567 (8)	654 (7)		
24'-0" (288")	PRESTRESSED	4	200	316	450	600 (9)	756 (8)	912 (7)		

SAFE GRAVITY LOADS OR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

LENGTH	TYPE	#5	SAFE LOAD - POUNDS PER LINEAR FOOT							
			8F8-0B	8F12-0B	8F16-0B	8F20-0B	8F24-0B	8F28-0B	8F32-0B	8F32-1B
4'-4" (52")	PRECAST	115	1749	3355	5280	7209	9138	11067	13000	
4'-6" (54")	PRECAST	144	1596	3083	4605	6113	7621	9129	10637	
5'-8" (68")	PRECAST	85	1167	2481	3753	5025	6297	7569	8841	
5'-10" (70")	PRECAST	63	859	1653	2442	3231	4020	4809	5598	
6'-8" (80")	PRECAST	77	1113	2342	3531	4720	5909	7098	8287	
7'-6" (90")	PRECAST	69	901	1825	2714	3603	4492	5381	6270	
9'-8" (116")	PRECAST	41	466	999	1568	2137	2706	3275	3844	

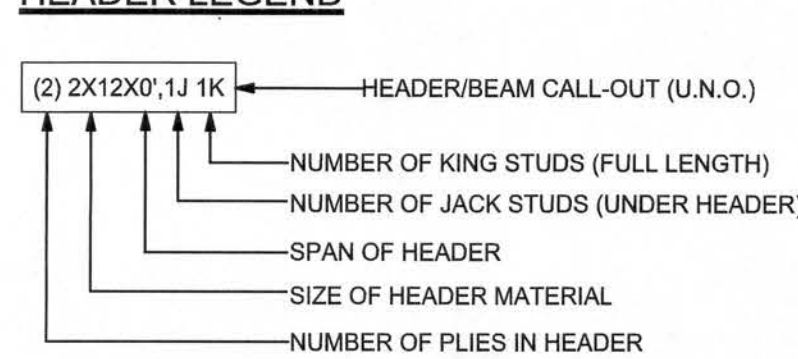
SAFE UPLIFT LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

LENGTH	TYPE	#5	SAFE UPLIFT LOADS - POUNDS PER LINEAR FOOT							
			8F8-1T	8F12-1T	8F16-1T	8F20-1T	8F24-1T	8F28-1T	8F32-1T	8F32-2T
4'-4" (52")	PRECAST	5	1748	2635	3522	4409	5296	6183		
4'-6" (54")	PRECAST	7	1675	2525	3374	4224	5074	5924		
5'-8" (68")	PRECAST	6	1301	1960	2618	3277	3935	4594		
5'-10" (70")	PRECAST	6	1262	1900	2538	3176	3815	4453		
6'-8" (80")	PRECAST	5	1012	1651	2204	2758	3312	3865		
7'-6" (90")	PRECAST	5	797	1462 (11)	1952 (10)	2442 (9)	2931 (8)	3421 (7)		
9'-8" (116")	PRECAST	3	491	931 (12)	1301 (11)	1640 (10)	1980 (9)	2322 (8)		

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND



DOOR & WINDOW BUCK ATTACHMENT

1. TAPCONS IN FACE OF CMU 2 1/2" MIN. EDGE DISTANCE 1 1/4" MIN. EMBEDMENT 3" MIN. SPACING
2. WINDOWS & DOORS UP TO 8'X8'
3. 3/16" TAPCONS @ 2' O.C. 1/4" TAPCONS @ 3' O.C.
4. WINDOWS & DOORS UP TO 8'X12'
5. 3/16" TAPCONS @ 16" O.C. 1/4" TAPCONS @ 24" O.C.
6. SLIDERS UP TO 8'X20'W
7. 3/16" TAPCONS @ 12" O.C. 1/4" TAPCONS @ 18" O.C.
8. GARAGE DOOR UP TO 10'W
9. (2) 3/16" TAPCONS & 16" O.C. (2) 1/4" TAPCONS & 24" O.C.
10. GARAGE DOOR UP TO 18'W
11. (2) 3/16" TAPCONS & 8" O.C. (2) 1/4" TAPCONS & 12" O.C.

TOTAL SHEAR WALL SEGMENTS

TRANSVERSE	REQUIRED ACTUAL	
	15.8'	79.5'
LONGITUDINAL	REQUIRED ACTUAL	
	14.5'	65.0'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, W.B. HOWLAND (JOB #4694)

REVISIONS

NO.	DESCRIPTION	DATE

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway
PE No.53915, POB 868, Lake City, FL 32066, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E., 53915
Seal

Concept Construction
Of North Florida, Inc.

Rice Residence

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Fax: (386) 269 - 4871

PRINTED DATE:
July 05, 2007

DRAWN BY:
David Disoway

CHECKED BY:

FINALS DATE:
05 / Jul / 07

JOB NUMBER:
707032

DRAWING NUMBER
S-3
OF 3 SHEETS