DATE 11/07/2005

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000023821

APPLICANT HUGO ESCALANTE PHONE 386-288-8666

ADDRESS PO BOX 280 FORT WHITE FL 32038

OWNER HBM CONSTRUCTION CORPORATION PHONE 386-288-8666

ADDRESS 268 SW PLATEAU GLEN FORT WHITE FL 32038

CONTRACTOR HUGO ESCALANTE PHONE 386-288-8666

LOCATION OF PROPERTY 47 S, R 242, R INTO WISE ESTATES ON GARDNER TERR, L PLATEAU, R ON LOT 36 @ THE END OF SHARD DRIVE TO RIGHT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 96600.00

HEATED FLOOR AREA 1932.00 TOTAL AREA 2640.00 HEIGHT 20.60 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE A DEVELOPMENT PERMIT NO.

PARCEL ID 24-4S-16-03113-166 SUBDIVISION WISE ESTATES

LOT 36 BLOCK C PHASE UNIT TOTAL ACRES .76

000000878		CRC1326967	Hugo Escalante	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
PERMIT	05-1008-N	BK	JH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: NOC ON FILE, MINIMUM FLOOR ELEVATION SET @ 100.5 FT- ELEVATON LETTER NEEDED BEFORE SLAB

Check # or Cash 3348

FOR BUILDING & ZONING DEPARTMENT ONLY

Temporary Power ____ date/app. by

Foundation ____ date/app. by

Monolithic (footer/Slab) ____ date/app. by

Under slab rough-in plumbing ____ date/app. by

Slab ____ date/app. by

Sheathing/Nailing ____ date/app. by

Framing ____ date/app. by

Rough-in plumbing above slab and below wood floor ____ date/app. by

Electrical rough-in ____ date/app. by

Heat & Air Duct ____ date/app. by

Peri. beam (Lintel) ____ date/app. by

Permanent power ____ date/app. by

C.O. Final ____ date/app. by

Culvert ____ date/app. by

M/H tie downs, blocking, electricity and plumbing ____ date/app. by

Pool ____ date/app. by

Reconnection ____ date/app. by

Pump pole ____ date/app. by

Utility Pole ____ date/app. by

M/H Pole ____ date/app. by

Travel Trailer ____ date/app. by

Re-roof ____ date/app. by

BUILDING PERMIT FEE $ 485.00 CERTIFICATION FEE $ 13.20 SURCHARGE FEE $ 13.20

MISC. FEES $ .00 ZONING CERT. FEE $ 50.00 FIRE FEE $ .00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 611.40

INSPECTORS OFFICE L. H. CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Donald F. Lee & Associates, Inc.

Surveyors & Engineers

140 NW Ridgewood Avenue
Lake City, Florida 32055
(386) 755-6166
Fax (386) 755-6167
dfla@suwanneevalley.net

23821

Wednesday, December 28, 2005

TO: EWPL, Inc. – Hugo Escalante

CC: Columbia County Building Department

FROM: Tim Delbene, P.L.S. – Donald F. Lee & Associates, Inc.

RE: Lot 36, Block C, Wise Estates - Elevation check

This letter is to certify that the elevation was measured for the finished floor (at Stemwall) for a house under construction on the above referenced Lot in Wise Estates. The Elevations are as follows:

House Floor: 101.63 - Adjacent grades: 98.6 (lowest) & 99.4 (highest)

The property lies in Flood Zone "A" per Flood Insurance Rate Maps (FIRM). No base flood elevation (BFE) is established for this area. The project Engineer for Wise Estates subdivision, has set the minimum floor elevation for Lot 36, Block C at 100.5 feet (data per record plat).

Timothy A. Delbene, P.L.S.
Florida Cert. No. LS 5594

DATE: 12/28/2005

Donald F. Lee & Associates, Inc.

FEDERAL EMERGENCY MANAGEMENT AGENCY
NATIONAL FLOOD INSURANCE PROGRAM

O.M.B. No. 3067-0077
Expires December 31, 2005

ELEVATION CERTIFICATE

Important: Read the instructions on pages 1 - 7.

SECTION A - PROPERTY OWNER INFORMATION

For Insurance Company Use:

BUILDING OWNER'S NAME
EWPL, Inc. - Hugo Escalante

Policy Number

BUILDING STREET ADDRESS (Including Apt., Unit, Suite, and/or Bldg. No.) OR P.O. ROUTE AND BOX NO.
SW Plateau Glen

Company NAIC Number

CITY	STATE	ZIP CODE
Lake City	FL	32025

PROPERTY DESCRIPTION (Lot and Block Numbers, Tax Parcel Number, Legal Description, etc.)
Lot 36, Block "C" - Wise Estates - Plat Bk 7, Pages 164-167

BUILDING USE (e.g., Residential, Non-residential, Addition, Accessory, etc. Use a Comments area, if necessary.)
Residential

LATITUDE/LONGITUDE (OPTIONAL) (##° - ## - ##.## or ##.#####°) HORIZONTAL DATUM: ☐ NAD 1927 ☐ NAD 1983 SOURCE: ☐ GPS (Type):____ ☐ USGS Quad Map ☐ Other: ____

SECTION B - FLOOD INSURANCE RATE MAP (FIRM) INFORMATION

B1. NFIP COMMUNITY NAME & COMMUNITY NUMBER	B2. COUNTY NAME	B3. STATE
Columbia County, Florida 120070	Columbia	Florida

B4. MAP AND PANEL NUMBER	B5. SUFFIX	B6. FIRM INDEX DATE	B7. FIRM PANEL EFFECTIVE/REVISED DATE	B8. FLOOD ZONE(S)	B9. BASE FLOOD ELEVATION(S) (Zone AO, use depth of flooding)
120070 0175	B	1/6/1988	1/6/1988	X	Sec.D

B10. Indicate the source of the Base Flood Elevation (BFE) data or base flood depth entered in B9.
☐ FIS Profile ☐ FIRM ☐ Community Determined ☒ Other (Describe): No BFE

B11. Indicate the elevation datum used for the BFE in B9: ☐ NGVD 1929 ☐ NAVD 1988 ☒ Other (Describe): No BFE

B12. Is the building located in a Coastal Barrier Resources System (CBRS) area or Otherwise Protected Area (OPA)? ☐ Yes ☒ No Designation Date____

SECTION C - BUILDING ELEVATION INFORMATION (SURVEY REQUIRED)

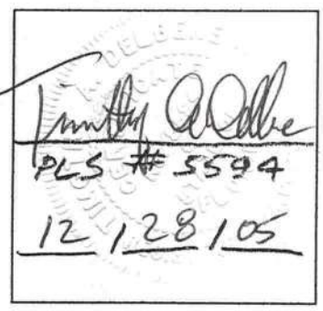
C1. Building elevations are based on: ☐ Construction Drawings* ☒ Building Under Construction* ☐ Finished Construction
*A new Elevation Certificate will be required when construction of the building is complete.

C2. Building Diagram Number 1 (Select the building diagram most similar to the building for which this certificate is being completed - see pages 6 and 7. If no diagram accurately represents the building, provide a sketch or photograph.)

C3. Elevations – Zones A1-A30, AE, AH, A (with BFE), VE, V1-V30, V (with BFE), AR, AR/A, AR/AE, AR/A1-A30, AR/AH, AR/AO
Complete Items C3.-a-i below according to the building diagram specified in Item C2. State the datum used. If the datum is different from the datum used for the BFE in Section B, convert the datum to that used for the BFE. Show field measurements and datum conversion calculation. Use the space provided or the Comments area of Section D or Section G, as appropriate, to document the datum conversion.
Datum NAVD 1988 Conversion/Comments per subdivision design benchmarks
Elevation reference mark used Local Does the elevation reference mark used appear on the FIRM? ☐ Yes ☒ No

- o a) Top of bottom floor (including basement or enclosure) 101. 63 ft.(m)
- o b) Top of next higher floor N/A. __ ft.(m)
- o c) Bottom of lowest horizontal structural member (V zones only) N/A. __ ft.(m)
- o d) Attached garage (top of slab) N/A. _ ft.(m)
- o e) Lowest elevation of machinery and/or equipment servicing the building (Describe in a Comments area) N/A. _ ft.(m)
- o f) Lowest adjacent (finished) grade (LAG) 98. 6 ft.(m)
- o g) Highest adjacent (finished) grade (HAG) 99. 4 ft.(m)
- o h) No. of permanent openings (flood vents) within 1 ft. above adjacent grade N/A
- o i) Total area of all permanent openings (flood vents) in C3.h N/A sq. in. (sq. cm)

License Number, Embossed Seal, Signature, and Date



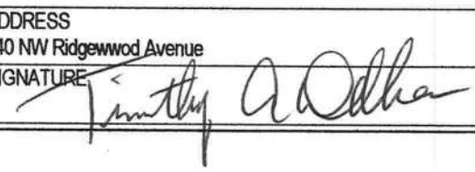
SECTION D - SURVEYOR, ENGINEER, OR ARCHITECT CERTIFICATION

This certification is to be signed and sealed by a land surveyor, engineer, or architect authorized by law to certify elevation information.
I certify that the information in Sections A, B, and C on this certificate represents my best efforts to interpret the data available.
I understand that any false statement may be punishable by fine or imprisonment under 18 U.S. Code, Section 1001.

CERTIFIER'S NAME Timothy A. Delbene, PSM LICENSE NUMBER LS 5594

TITLE Land Surveyor COMPANY NAME Donald F. Lee & Associates, Inc.

ADDRESS	CITY	STATE	ZIP CODE
140 NW Ridgewwod Avenue	Lake City	FL	32055

SIGNATURE  DATE 12/28/2005 TELEPHONE 386-755-6166

IMPORTANT: In these spaces, copy the corresponding information from Section A.			For Insurance Company Use:
BUILDING STREET ADDRESS (Including Apt., Unit, Suite, and/or Bldg. No.) OR P.O. ROUTE AND BOX NO. SW Plateau Glen - Lot 36, Blk C Wise Estates			Policy Number
CITY Lake City	STATE FL	ZIP CODE 32025	Company NAIC Number

SECTION D - SURVEYOR, ENGINEER, OR ARCHITECT CERTIFICATION (CONTINUED)

Copy both sides of this Elevation Certificate for (1) community official, (2) insurance agent/company, and (3) building owner.

COMMENTS

Foundation is under construction. Elevation is on sternwall.

Minimum Floor Elevation is 100.5 - per subdivision engineer and as shown on plat of record.

No Base Flood Elevation (BFE) is established in this area. Lot is in Flood Zone "A"

☐ Check here if attachments

SECTION E - BUILDING ELEVATION INFORMATION (SURVEY NOT REQUIRED) FOR ZONE AO AND ZONE A (WITHOUT BFE)

For Zone AO and Zone A (without BFE), complete Items E1 through E4. If the Elevation Certificate is intended for use as supporting information for a LOMA or LOMR-F, Section C must be completed.

E1. Building Diagram Number _(Select the building diagram most similar to the building for which this certificate is being completed – see pages 6 and 7. If no diagram accurately represents the building, provide a sketch or photograph.)

E2. The top of the bottom floor (including basement or enclosure) of the building is__ ft.(m) __in.(cm) ☐ above or ☐ below (check one) the highest adjacent grade. (Use natural grade, if available).

E3. For Building Diagrams 6-8 with openings (see page 7), the next higher floor or elevated floor (elevation b) of the building is __ ft.(m) __in.(cm) above the highest adjacent grade. Complete items C3.h and C3.i on front of form.

E4. The top of the platform of machinery and/or equipment servicing the building is__ ft.(m) __in.(cm) ☐ above or ☐ below (check one) the highest adjacent grade. (Use natural grade, if available).

E5. For Zone AO only: If no flood depth number is available, is the top of the bottom floor elevated in accordance with the community's floodplain management ordinance?
☐ Yes ☐ No ☐ Unknown. The local official must certify this information in Section G.

SECTION F - PROPERTY OWNER (OR OWNER'S REPRESENTATIVE) CERTIFICATION

The property owner or owner's authorized representative who completes Sections A, B, C (Items C3.h and C3.i only), and E for Zone A (without a FEMA-issued or community-issued BFE) or Zone AO must sign here. *The statements in Sections A, B, C, and E are correct to the best of my knowledge.*

PROPERTY OWNER'S OR OWNER'S AUTHORIZED REPRESENTATIVE'S NAME

ADDRESS | CITY | STATE | ZIP CODE

SIGNATURE | DATE | TELEPHONE

COMMENTS

☐ Check here if attachments

SECTION G - COMMUNITY INFORMATION (OPTIONAL)

The local official who is authorized by law or ordinance to administer the community's floodplain management ordinance can complete Sections A, B, C (or E), and G of this Elevation Certificate. Complete the applicable item(s) and sign below.

G1. ☐ The information in Section C was taken from other documentation that has been signed and embossed by a licensed surveyor, engineer, or architect who is authorized by state or local law to certify elevation information. (Indicate the source and date of the elevation data in the Comments area below.)

G2. ☐ A community official completed Section E for a building located in Zone A (without a FEMA-issued or community-issued BFE) or Zone AO.

G3. ☐ The following information (Items G4-G9) is provided for community floodplain management purposes.

G4. PERMIT NUMBER	G5. DATE PERMIT ISSUED	G6. DATE CERTIFICATE OF COMPLIANCE/OCCUPANCY ISSUED

G7. This permit has been issued for: ☐ New Construction ☐ Substantial Improvement

G8. Elevation of as-built lowest floor (including basement) of the building is: ____.__ft.(m) Datum: ____

G9. BFE or (in Zone AO) depth of flooding at the building site is: ____.__ ft.(m) Datum: ____

LOCAL OFFICIAL'S NAME | TITLE

COMMUNITY NAME | TELEPHONE

SIGNATURE | DATE

COMMENTS

☐ Check here if attachments

Columbia County Building Permit Application

(3348)

Revised 9-23-04

For Office Use Only Application # 0510-91 Date Received 10/31/05 By G Permit # 878/23821

Application Approved by - Zoning Official BLK Date 03.11.05 Plans Examiner OK STH Date 11-4-05

Flood Zone A Development Permit ~~NA~~ Zoning RSF-2 Land Use Plan Map Category RES. LOW DEN.

Comments Elevation ~~Certification~~ ~~Letter~~ 1st Floor Elevation - to be a minimum of 100.5 ft. Letter needed before STAB Ltr needed

Applicants Name Hugo Escalante Phone 386-288-8666

Address P.O. BOX 280 Ford White, FL 32038

Owners Name H&M Construction Corporation Phone 386-288-8666

911 Address 268 S.W. Plateau Glen L.C.

Contractors Name Hugo Escalante Phone 386-288-8666

Address P.O. BOX 280, Ford White, FL 32038

Fee Simple Owner Name & Address None

Bonding Co. Name & Address None

Architect/Engineer Name & Address Daniel Shaheen, Lake City, FL 32038

Mortgage Lenders Name & Address

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 24-4S-16-03113-166 Estimated Cost of Construction $135,000

Subdivision Name WISE Estates Lot 36 Block C Unit ___ Phase ___

Driving Directions 47 South to CR 242, 2 miles to WISE Estates entrance, make Right turn to Gardner Terrace make Left to Plateau Glen turn right lot 36 at end of street at right.

Type of Construction New Single Family Residence Number of Existing Dwellings on Property 0

Total Acreage .76 Acres Lot Size .76 Acres Do you need a - (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 65' Side 50' Side 50' Rear 65'

Total Building Height 20'6" Number of Stories 1 Heated Floor Area 1932 Sq Ft Roof Pitch 6-12'

Porches 227 GARAGE 481 TOTAL 2640

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Hugo Escalante
Owner Builder or Agent (Including Contractor)

05-1008-N

Hugo Escalante
Contractor Signature

Contractors License Number CRC1326967

Competency Card Number

NOTARY STAMP/SEAL

CARRIE L. REVELLE
MY COMMISSION # DD 181697
EXPIRES: February 11, 2007
Bonded Thru Notary Public Underwriters

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 31st day of October 2005.

Personally known X or Produced Identification ___

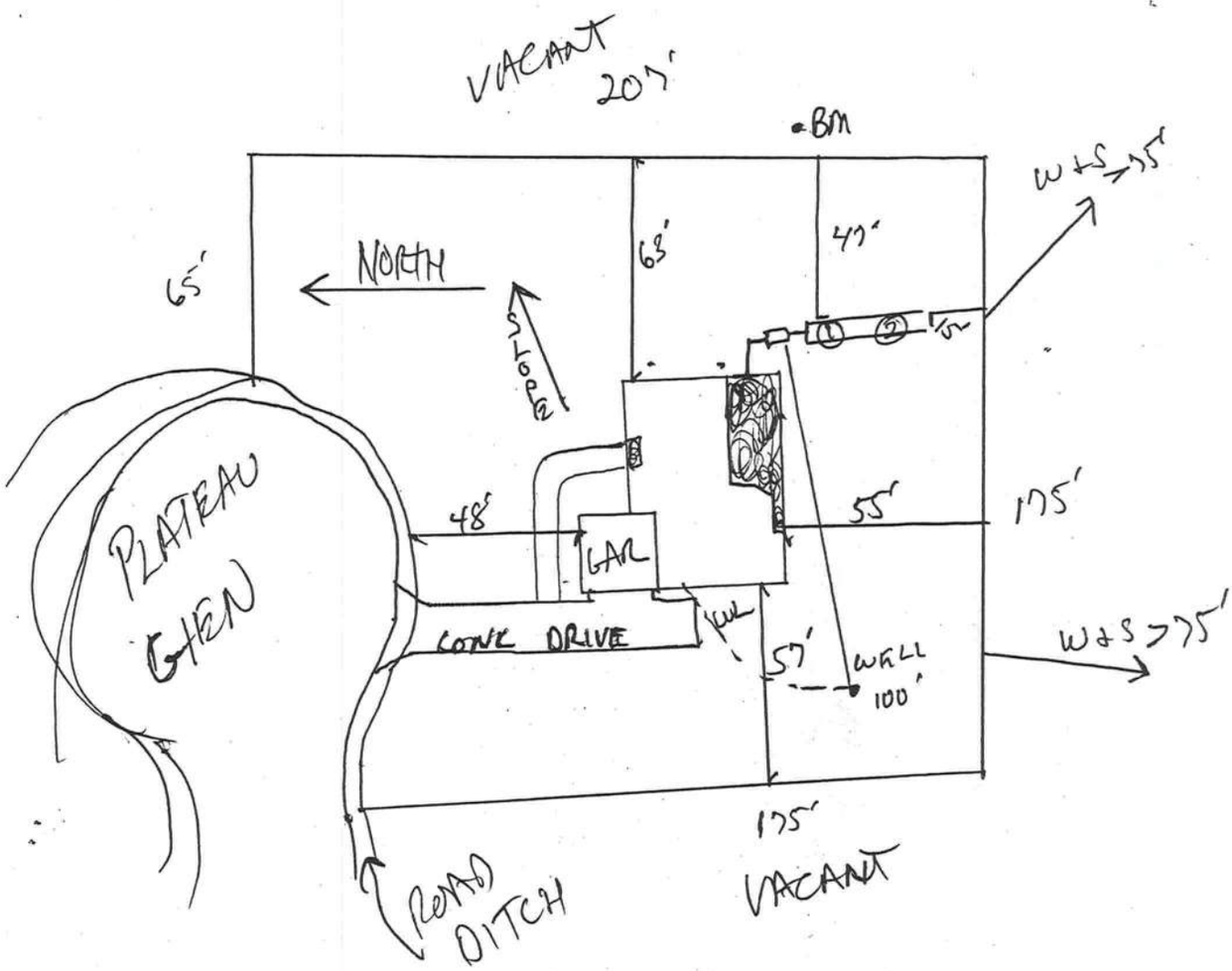
Notary Signature

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 05-1008N

---------- PART II - SITEPLAN ----------

Scale: 1 inch = 50 feet.



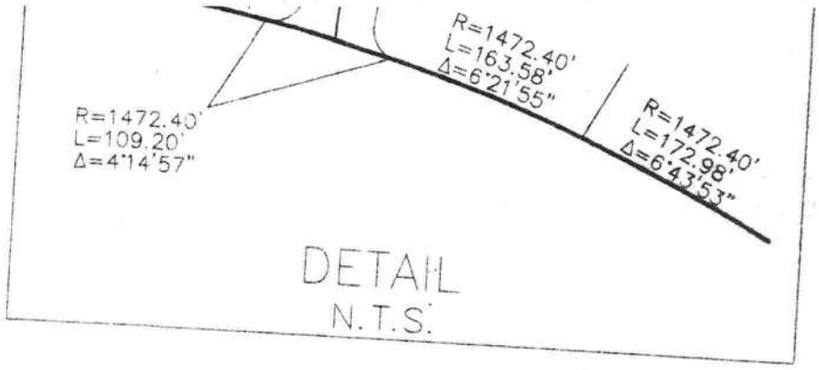
Notes: _____

Site Plan submitted by: Rocky Tyler — MASTER CONTRACTOR

Plan Approved ✓ Not Approved ____ Date 10-3-05

By _____ Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT



DETAIL
N.T.S.

ARK

CE

c. 23
survey

MINIMUM FLOOR ELEVATION

(as determined by Project Engineer)

BLOCK A

1 – 97.5
2 – 99.0
3 – 99.5
4 – 99.5
5 – 98.5
6 – 97.2
7 – 98.0

BLOCK B

1 – 100.0
2 – 99.0
3 – 96.5
4 – 92.5
5 – 92.23
6 – 92.2

BLOCK C

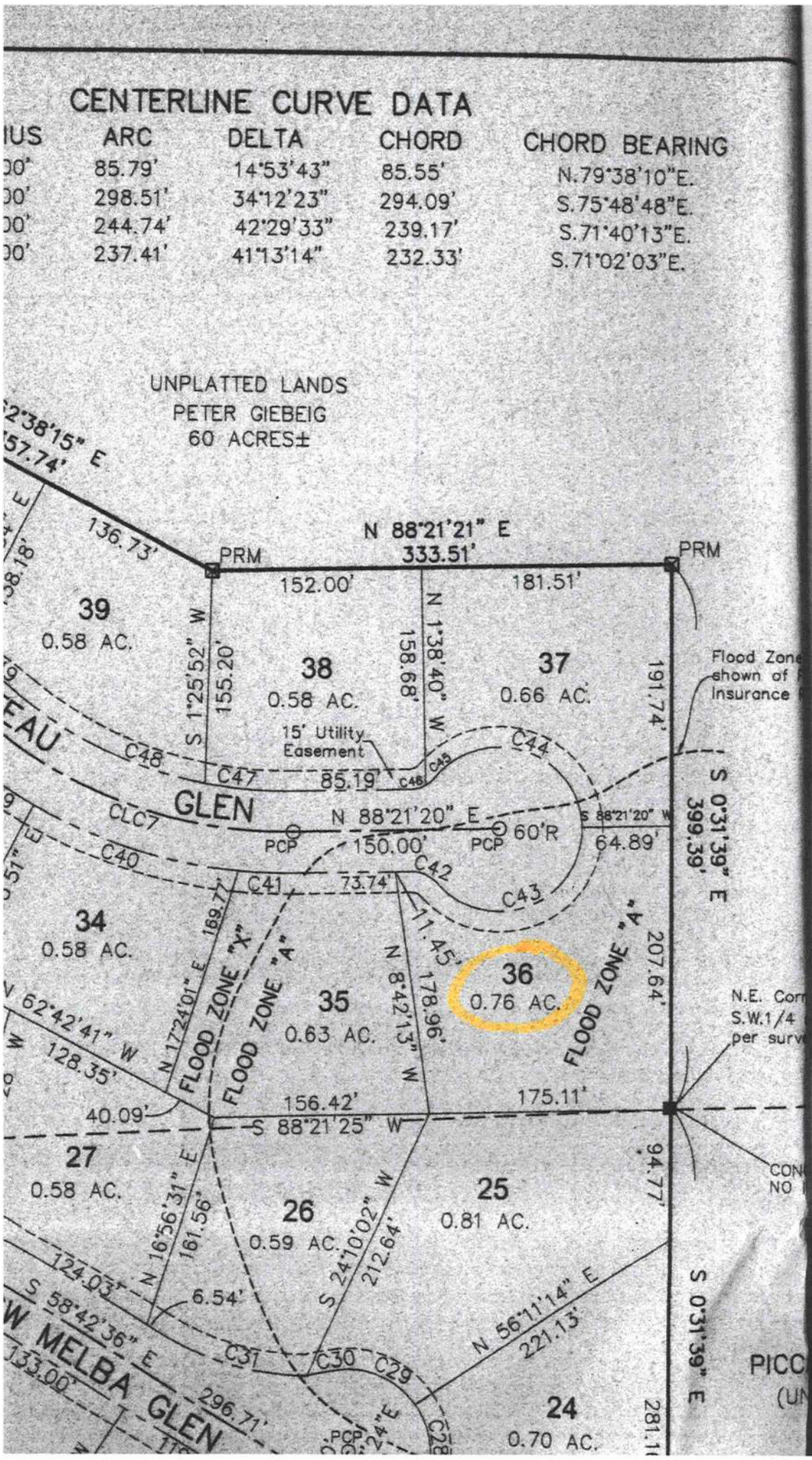
1 – 97.3
2 – 98.5
3 – 99.3
4 – 99.5
5 – 99.0
6 – 97.0
7 – 98.0
8 – 99.0
9 – 96.3
10 – 94.3
11 – 94.3
12 – 95.0
13 – 93.5
14 – 93.0
15 – 93.0
16 – 95.0
17 – 92.5
18 – 93.2
19 – 95.5
20 – 96.5
21 – 96.0
22 – 95.2
23 – 95.2
24 – 97.2
25 – 98.3
26 – 98.3
27 – 97.3
28 – 95.5
29 – 93.0
30 – 92.2
31 – 93.0
32 – 95.0
33 – 96.5
34 – 98.5
35 – 100.5
36 – 100.5
37 – 100.0
38 – 99.5
39 – 95.0
40 – 97.2
41 – 95.0
42 – 93.2

PLAT DATE: 02 / 20 /20 04

Donald F. Lee and Associates, Inc.
SURVEYORS – ENGINEERS
140 Northwest Ridgewood Avenue, Lake City, Florida 32055
Phone: (386) 755-6166 FAX: (386) 755-6167
LB# 7042

CENTERLINE CURVE DATA

IUS	ARC	DELTA	CHORD	CHORD BEARING
00'	85.79'	14°53'43"	85.55'	N.79°38'10"E.
00'	298.51'	34°12'23"	294.09'	S.75°48'48"E.
00'	244.74'	42°29'33"	239.17'	S.71°40'13"E.
00'	237.41'	41°13'14"	232.33'	S.71°02'03"E.



NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 24-4S-16-03113-166

1. **Description of property: (legal description of the property and street address or 911 address)**
   Lot 36 Block C, WISE ESTATES S/D WD 1018-2945
2. **General description of improvement:** NEW SIDGLE FAMILY RESIDENCE
3. **Owner Name & Address** HBM Construction, 10155 Collins Ave Suite 1004, Bal Harbour, FL 33154 **Interest in Property** 100%
4. **Name & Address of Fee Simple Owner (if other than owner):** N/A
5. **Contractor Name** Hugo Escalante **Phone Number** 386-288-8666
   Address P.O. BOX 280, Ft White, FL 32038
6. **Surety Holders Name** NONE
   Address
   Amount of Bond
7. **Lender Name** N/A
   Address

Inst:2005027041 Date:10/31/2005 Time:11:19
DC,P.DeWitt Cason,Columbia County B:1063 P:1324

8. **Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:**
   Name Hugo Escalante **Phone Number** 386-288-8666
   Address P.O. BOX 280 Fort White, FL 32038
9. **In addition to himself/herself the owner designates** Hugo Escalante **of** Fort White **to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) –**
   (a) 7. Phone Number of the designee 386-288-8666
10. **Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified)**

NOTICE AS PER CHAPTER 713, Florida Statutes:
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Signature of Owner

Sworn to (or affirmed) and subscribed before day of 31st October, 2005

NOTARY STAMP/SEAL

CARRIE L. REVELLE
MY COMMISSION # DD 181697
EXPIRES: February 11, 2007
Bonded Thru Notary Public Underwriters

Signature of Notary

Columbia County Property Appraiser

DB Last Updated: 9/16/2005

2005 Proposed Values

Parcel: 24-4S-16-03113-166

Tax Record | Property Card | Interactive GIS Map | Print

Search Result: 1 of 1

Owner & Property Info

Owner's Name	H & M CONSTRUCTION CORPORATION
Site Address	PLATEAU
Mailing Address	10155 COLLINS AVE SUITE 1004 BALL HARBOUR, FL 33154
Brief Legal	LOT 36 BLOCK C WISE ESTATE S/D WD 1018-2245.

Use Desc. (code)	VACANT (000000)
Neighborhood	24416.00
Tax District	2
UD Codes	MKTA06
Market Area	06
Total Land Area	0.760 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	$20,500.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$20,500.00

Just Value	$20,500.00
Class Value	$0.00
Assessed Value	$20,500.00
Exempt Value	$0.00
Total Taxable Value	$20,500.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
6/18/2004	1018/2245	WD	V	Q		$92,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1.000 LT - (.760AC)	1.00/1.00/1.00/1.00	$20,500.00	$20,500.00

Columbia County Property Appraiser — DB Last Updated: 9/16/2005

1 of 1

Disclaimer

LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave
Lake City, FL. 32025
Phone 386-752-6677
Fax 386-752-1477

Lot 36 WISE

Building Permit # _____ Owner's Name Kapkun 2LLC

Well Depth _____ Ft. Casing Depth _____ Ft. Water Level _____ Ft.

Casing Size 4 inch Steel Pump Installation: Deep Well Submersible

Pump Make Red Jacket Pump Model 100F21120G8 HP 1

System Pressure (PSI) _____ On 30 Off 50 Average Pressure ~~XXX~~ 40

Pumping System GPM at average pressure and pumping level 20 (GPM)

Tank Installation: Precharged Bladder Make Challenger Model PC244 Size _____

Tank Draw-down per cycle at system pressure 26.1 gallons

I HEREBY VERTIFY THAT THIS WATER WELL SYSTEM HAS BEEN INSTALLED AS PER THE ABOVE INFORMATION.

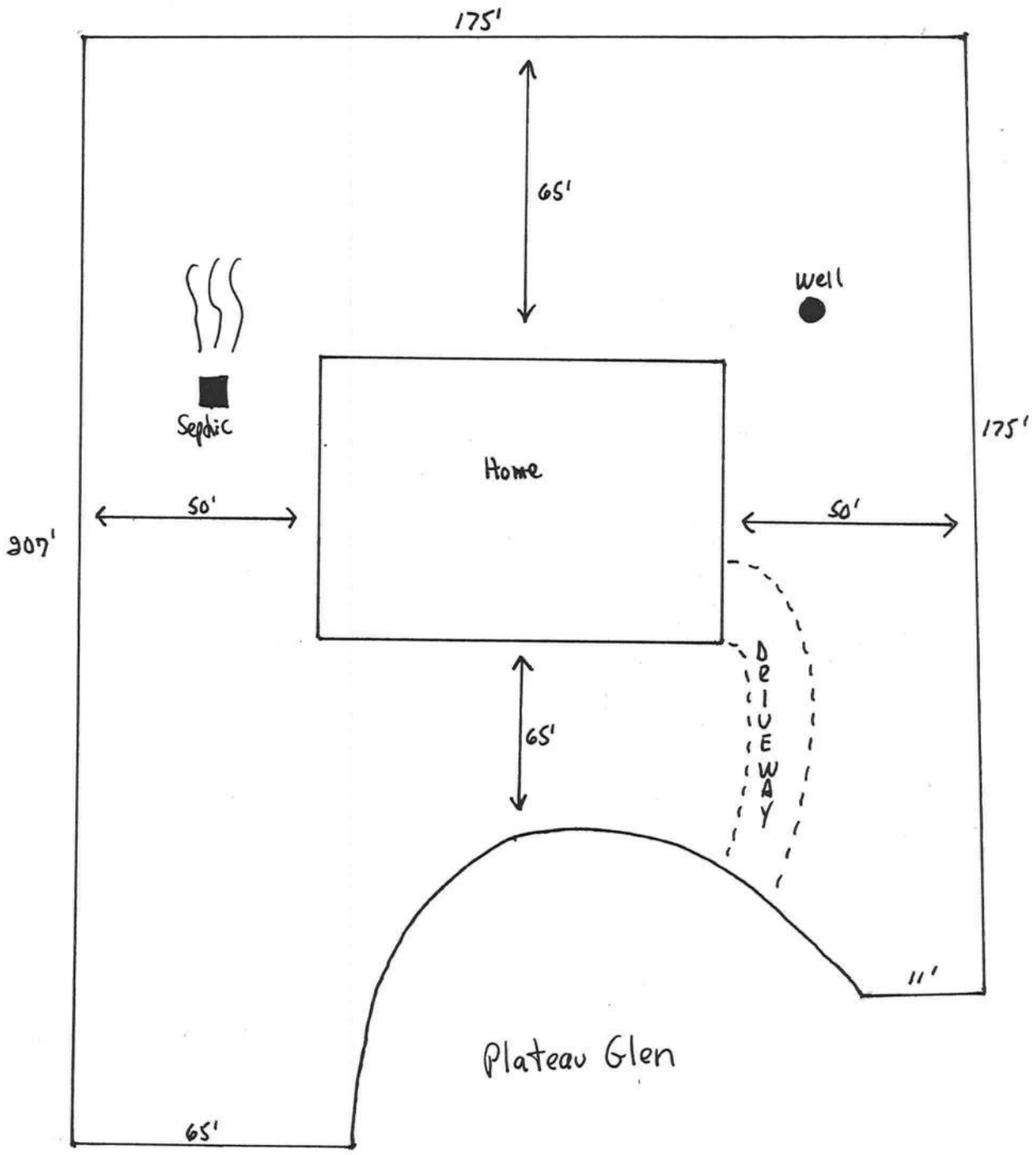
Linda Newcomb
Signature

Linda Newcomb
Print Name

2609
License Number

6-6-05
Date

Wise Estates Lot 36
Parcel # 29-4$-16-03113-166
268 S.W. Plateau Glen



From: The Columbia County Building Department
Plans Review
135 NE Hernando Av.
P. O Box 1529
Lake City Florida, 32056-1529

Reference to: Build permit application Number: 0510-91

Hugo Escalate Owner H & M Construction Corp. Lot 36 Block C of Wise Estates

On the date of November 2, 2005 application 0510-91 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0510-91 when making reference to this application.

1. Application 0510-91 which was filed with the building department on the date of October 31, 2005 will be reviewed under the Florida Building Code 2004. The Wind Load design by Mr. Mark Disosway was design under the Florida Building Code 2001. The wind Load design should reflect the code sections of the Florida Building Code 2004 that relate to wind Load design code requirements.

2. Please show compliance with sections R309 of the FRC-2004 R309.1 Opening protection. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors. R309.1.1 Duct penetration: Ducts in the garage and ducts penetrating the walls or ceilings separating the dwelling from the garage shall be constructed of a

minimum No. 26 gage (0.48 mm) sheet steel or other approved material and shall have no openings into the garage. R309.2 Separation required: The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent.

3. Please show compliance with section R322 of the FRC-2004: All new single-family houses, duplexes, triplexes, condominiums and townhouses shall provide at least one bathroom, located with maximum possible privacy, where bathrooms are provided on habitable grade levels, with a door that has a 29-inch (737 mm) clear opening. However, if only a toilet room is provided at grade level, such toilet rooms shall have a clear opening of not less than 29 inches (737 mm).

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**THE NATHAN 4-BED**	Builder:	**EWPL INC.**
Address:	**Lot: 36, Sub: Wise Estates, Plat:**	Permitting Office:	Columbia County
City, State:	**Lake City, FL 32024-**	Permit Number:	23821
Owner:	**EWPL INC**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	
2. Single family or multi-family	Single family	
3. Number of units, if multi-family	1	
4. Number of Bedrooms	4	
5. Is this a worst case?	No	
6. Conditioned floor area (ft²)	1932 ft²	
7. Glass area & type		
a. Clear - single pane	0.0 ft²	
b. Clear - double pane	339.0 ft²	
c. Tint/other SHGC - single pane	0.0 ft²	
d. Tint/other SHGC - double pane	0.0 ft²	
8. Floor types		
a. Slab-On-Grade Edge Insulation	R=0.0, 204.0(p) ft	
b. N/A		
c. N/A		
9. Wall types		
a. Frame, Wood, Adjacent	R=13.0, 232.0 ft²	
b. Frame, Wood, Exterior	R=13.0, 1660.0 ft²	
c. N/A		
d. N/A		
e. N/A		
10. Ceiling types		
a. Under Attic	R=30.0, 1932.0 ft²	
b. N/A		
c. N/A		
11. Ducts		
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 125.0 ft	
b. N/A		

12. Cooling systems		
a. Central Unit	Cap: 36.0 kBtu/hr	
	SEER: 12.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 36.0 kBtu/hr	
	HSPF: 6.80	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 50.0 gallons	
	EF: 0.88	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.18

Total as-built points: 29571
Total base points: 32701

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 9-20-05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA
IN GOD WE TRUST

BUILDING OFFICIAL: _____

DATE: _____

EnergyGauge® (Version: FLRCPB v3.2)

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 36, Sub: Wise Estates, Plat: , Lake City, FL, 32024-** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 36, Sub: Wise Estates, Plat: , Lake City, FL, 32024-** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
4		2746.00	10984.0	50.0	0.88	4	1.00	2746.00	1.00	10984.0	
				As-Built Total:						**10984.0**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
11044	**10673**	**10984**	**32701**	**8291**	**10296**	**10984**	**29571**

PASS

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 36, Sub: Wise Estates, Plat: , Lake City, FL, 32024-** PERMIT #:

BASE			AS-BUILT						
Winter Base Points:		**17012.0**	**Winter As-Built Points:**						**17666.1**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
17012.0	**0.6274**	**10673.3**	17666.1 **17666.1**	1.000 **1.00**	(1.069 x 1.169 x 0.93) **1.162**	0.501 **0.501**	1.000 **1.000**	10295.8 **10295.8**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 36, Sub: Wise Estates, Plat: , Lake City, FL, 32024-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1932.0	12.74	4430.5	Double, Clear	N	1.5	7.5	84.0	14.30	1.00	1202.9	
				Double, Clear	N	6.0	3.0	12.5	14.30	1.03	183.3	
				Double, Clear	E	1.5	5.5	30.0	9.09	1.04	284.0	
				Double, Clear	S	1.5	7.0	30.0	4.03	1.07	129.9	
				Double, Clear	SW	8.0	7.5	21.0	7.17	1.64	246.6	
				Double, Clear	S	8.0	8.0	70.0	4.03	2.73	770.7	
				Double, Clear	N	1.5	6.0	16.0	14.30	1.00	229.4	
				Double, Clear	W	1.5	7.5	21.0	10.77	1.01	229.1	
				Double, Clear	N	1.5	3.0	12.5	14.30	1.01	180.4	
				Double, Clear	S	1.5	8.0	42.0	4.03	1.04	176.3	
				As-Built Total:				**339.0**			**3632.6**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =	Points		
Adjacent	232.0	3.60	835.2	Frame, Wood, Adjacent			13.0	232.0	3.30	765.6		
Exterior	1660.0	3.70	6142.0	Frame, Wood, Exterior			13.0	1660.0	3.40	5644.0		
Base Total:	**1892.0**		**6977.2**	**As-Built Total:**				**1892.0**		**6409.6**		
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =	Points		
Adjacent	20.0	11.50	230.0	Exterior Wood				20.0	12.30	246.0		
Exterior	60.0	12.30	738.0	Adjacent Wood				20.0	11.50	230.0		
				Exterior Wood				40.0	12.30	492.0		
Base Total:	**80.0**		**968.0**	**As-Built Total:**				**80.0**		**968.0**		
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X WCM =	Points		
Under Attic	1932.0	2.05	3960.6	Under Attic			30.0	1932.0	2.05 X 1.00	3960.6		
Base Total:	**1932.0**		**3960.6**	**As-Built Total:**				**1932.0**		**3960.6**		
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =	Points		
Slab	204.0(p)	8.9	1815.6	Slab-On-Grade Edge Insulation			0.0	204.0(p	18.80	3835.2		
Raised	0.0	0.00	0.0									
Base Total:			**1815.6**	**As-Built Total:**				**204.0**		**3835.2**		
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =	Points		
	1932.0	-0.59	-1139.9					1932.0	-0.59	-1139.9		

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 36, Sub: Wise Estates, Plat: , Lake City, FL, 32024-** PERMIT #:

BASE				AS-BUILT						
Summer Base Points:			**25887.6**	**Summer As-Built Points:**						**25621.7**
Total Summer Points	X System Multiplier	= Cooling Points		Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points	
25887.6	**0.4266**	**11043.6**		25621.7 **25621.7**	1.000 **1.00**	(1.090 x 1.147 x 0.91) **1.138**	0.284 **0.284**	1.000 **1.000**	8290.7 **8290.7**	

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 36, Sub: Wise Estates, Plat: , Lake City, FL, 32024-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1932.0	20.04	6969.1	Double, Clear	N	1.5	7.5	84.0	19.22	0.96	1552.3	
				Double, Clear	N	6.0	3.0	12.5	19.22	0.62	149.7	
				Double, Clear	E	1.5	5.5	30.0	40.22	0.90	1081.5	
				Double, Clear	S	1.5	7.0	30.0	34.50	0.89	925.8	
				Double, Clear	SW	8.0	7.5	21.0	38.46	0.48	385.5	
				Double, Clear	S	8.0	8.0	70.0	34.50	0.52	1257.1	
				Double, Clear	N	1.5	6.0	16.0	19.22	0.94	288.6	
				Double, Clear	W	1.5	7.5	21.0	36.99	0.95	737.2	
				Double, Clear	N	1.5	3.0	12.5	19.22	0.83	199.7	
				Double, Clear	S	1.5	8.0	42.0	34.50	0.92	1337.8	
				As-Built Total:				**339.0**			**7915.2**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	232.0	0.70	162.4	Frame, Wood, Adjacent			13.0	232.0	0.60		139.2	
Exterior	1660.0	1.70	2822.0	Frame, Wood, Exterior			13.0	1660.0	1.50		2490.0	
Base Total:	**1892.0**		**2984.4**	**As-Built Total:**				**1892.0**			**2629.2**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	20.0	2.40	48.0	Exterior Wood				20.0	6.10		122.0	
Exterior	60.0	6.10	366.0	Adjacent Wood				20.0	2.40		48.0	
				Exterior Wood				40.0	6.10		244.0	
Base Total:	**80.0**		**414.0**	**As-Built Total:**				**80.0**			**414.0**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1932.0	1.73	3342.4	Under Attic			30.0	1932.0	1.73 X 1.00		3342.4	
Base Total:	**1932.0**		**3342.4**	**As-Built Total:**				**1932.0**			**3342.4**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	204.0(p)	-37.0	-7548.0	Slab-On-Grade Edge Insulation			0.0	204.0(p	-41.20		-8404.8	
Raised	0.0	0.00	0.0									
Base Total:			**-7548.0**	**As-Built Total:**				**204.0**			**-8404.8**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1932.0	10.21	19725.7					1932.0	10.21		19725.7	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.3

The higher the score, the more efficient the home.

EWPL INC, Lot: 36, Sub: Wise Estates, Plat: , Lake City, FL, 32024-

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family	a.	Central Unit	Cap: 36.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 12.00
4.	Number of Bedrooms	4	b.	N/A	
5.	Is this a worst case?	No			
6.	Conditioned floor area (ft²)	1932 ft²	c.	N/A	
7.	Glass area & type				
a.	Clear - single pane	0.0 ft²	13.	Heating systems	
b.	Clear - double pane	339.0 ft²	a.	Electric Heat Pump	Cap: 36.0 kBtu/hr
c.	Tint/other SHGC - single pane	0.0 ft²			HSPF: 6.80
d.	Tint/other SHGC - double pane	0.0 ft²	b.	N/A	
8.	Floor types				
a.	Slab-On-Grade Edge Insulation	R=0.0, 204.0(p) ft	c.	N/A	
b.	N/A				
c.	N/A		14.	Hot water systems	
9.	Wall types		a.	Electric Resistance	Cap: 50.0 gallons
a.	Frame, Wood, Adjacent	R=13.0, 232.0 ft²			EF: 0.88
b.	Frame, Wood, Exterior	R=13.0, 1660.0 ft²	b.	N/A	
c.	N/A				
d.	N/A		c.	Conservation credits	
e.	N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
a.	Under Attic	R=30.0, 1932.0 ft²	15.	HVAC credits	
b.	N/A			(CF-Ceiling fan, CV-Cross ventilation,	
c.	N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 125.0 ft		RB-Attic radiant barrier,	
b.	N/A			MZ-C-Multizone cooling,	
				MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

GREAT SEAL OF THE STATE OF FLORIDA
IN GOD WE TRUST

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/488-1824.

EnergyGauge® (Version: FLRCPB v3.2)

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000000878

DATE 11/07/2005 PARCEL ID # 24-4S-16-03113-166

APPLICANT HUGO ESCALANTE PHONE 288-8666

ADDRESS PO BOX 280 FORT WHITE FL FL 32038

OWNER HBM CONSTRUCTION COPORATION PHONE 386-288-8666

ADDRESS 268 SW PLATEAU GLEN LAKE CITY FL FL 32025

CONTRACTOR ESCALANTE PHONE 288-8666

LOCATION OF PROPERTY 47 S, L 242 TO WISE ESTATES, R GARDNER TERR, L PLATEAU GLEN,

R TO LOT 36 AT END OF SHARD ROAD

SUBDIVISION/LOT/BLOCK/PHASE/UNIT WISE ESTATES 36 C

SIGNATURE Hugo Escalante

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

Mark Disosway, P.E.
POB 868, Lake City, FL 32056, Ph (386) 754-5419, Fax (386) 269-4871

November 3, 2005

Building Department

Re: Permit 0510-91, Ewpl Inc / Hugo Escalate, Spec House, Lot #36, Wise Estates S/D, Columbia County, Florida

Dear Building Official:

Please accept this letter as addendum to the plans for the above referenced house to change all references to FBC 2001 to FBC 2004.

- The plan was drawn prior to the effective date for FBC 2004, 01 October 2005.
- Since the wind load requirements of FBC 2004 remain basically unchanged from FBC 2001 there are no structural changes required to this plan.

03NOV05

Mark Disosway, PE
Florida Registered Professional Engineer

Cc Ewpl Inc / Hugo Escalate (Builder)

6037 WISE

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☐	☑	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☑	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☑	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
☑	☐	**Elevations including:**
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation
☑	☐	d) Location, size and height above roof of chimneys
☑	☐	e) Location and size of skylights
☑	☐	f) Building height
☑	☐	e) Number of stories

See NOTE 1

Floor Plan including:

a) Rooms labeled and dimensioned
b) Shear walls
c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
f) Must show and identify accessibility requirements (accessible bathroom) See NOTE 3

Foundation Plan including:

a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
b) All posts and/or column footing including size and reinforcing
c) Any special support required by soil analysis such as piling
d) Location of any vertical steel

Roof System:

a) Truss package including:
    1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
    2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
b) Conventional Framing Layout including:
    1. Rafter size, species and spacing
    2. Attachment to wall and uplift
    3. Ridge beam sized and valley framing and support details
    4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

a) Masonry wall
    1. All materials making up wall
    2. Block size and mortar type with size and spacing of reinforcement
    3. Lintel, tie-beam sizes and reinforcement
    4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
    5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
    6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
    7. Fire resistant construction (if required)
    8. Fireproofing requirements
    9. Shoe type of termite treatment (termicide or alternative method)
    10. Slab on grade
        a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
        b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
    11. Indicate where pressure treated wood will be placed
    12. Provide insulation R value for the following:
        a. Attic space
        b. Exterior wall cavity
        c. Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
    a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
b) Floor joist size and spacing
c) Girder size and spacing
d) Attachment of joist to girder
e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
b) Ceiling fans
c) Smoke detectors
d) Service panel and sub-panel size and location(s)
e) Meter location with type of service entrance (overhead or underground)
f) Appliances and HVAC equipment
g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

a) Manual J sizing equipment or equivalent computation
b) Exhaust fans in bathroom

Energy Calculations (dimensions shall match plans)

Gas System Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

***Notice Of Commencement Required Before Any Inspections Will Be Done**

Private Potable Water

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued.
   (386) 758-1058 **(Toilet facilities shall be provided for construction workers)**

4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
   A development permit will also be required. Development permit cost is **$50.00**

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (**$25.00**) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (**$50.00**). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

NOTICE:

ADDRESSES BY APPOINTMENT ONLY!

TO OBTAIN A 9-1-1 ADDRESS THE REQUESTER MUST CONTACT THE COLUMBIA COUNTY 9-1-1 ADDRESSING DEPARTMENT AT (386) 752-8787 FOR AN APPOINTMENT TIME AND DATE:

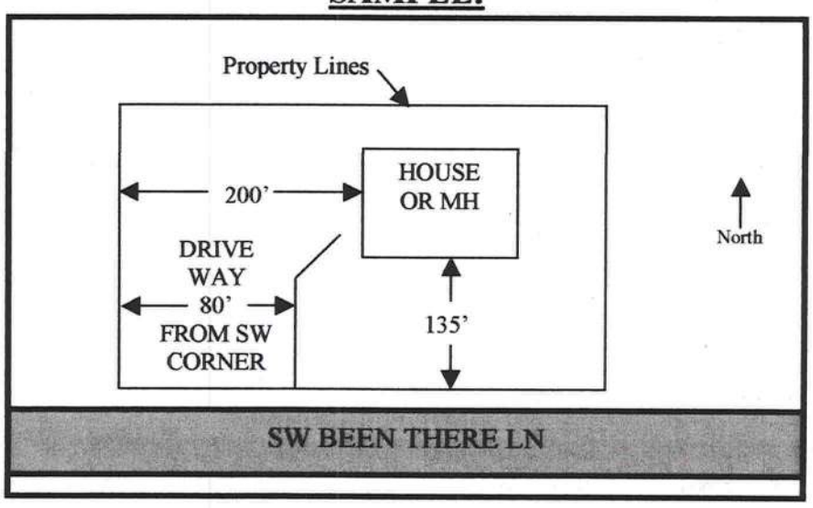
YOU CAN NOT OBTAIN A NEW ADDRESS OVER THE TELEPHONE. MUST MAKE AN APPOINTMENT!

THE ADDRESSING DEPARTMENT IS LOCATED AT 263 NW LAKE CITY AVENUE (OFF OF WEST U.S. HIGHWAY 90 WEST OF INTERSTATE 75 AT THE COLUMBIA COUNTY EMERGENCY OPERATIONS CENTER).

THE REQUESTER WILL NEED THE FOLLOWING:

1. THE PARCEL OR TAX ID NUMBER (SAMPLE: "25-4S-17-12345-123" OR "R12345-123) FOR THE PROPERTY.
2. A PLAT, PLAN, SITE PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
   a. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
   b. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
   c. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



NOTE: 5 TO 7 WORKING DAYS MAY BE REQUIRED IF ADDRESSING DEPARTMENT NEEDS TO CONDUCT AN ON SITE SURVEY.

Project Information for:	L135123		
Builder:	HUGO ESCALANTE	Date:	10/26/2005
Lot:	LOT 36 WISE ESTATES	Start Number:	1049
Subdivision:	N/A		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	40		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	120		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ESCALANTE, HUGO CRC 1326967
Address: P.O. BOX 280
FORT WHITE, FL. 32038 Designer: 28

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1026051049	10/26/2005				
2	CJ3	1026051050	10/26/2005				
3	CJ5	1026051051	10/26/2005				
4	EJ7	1026051052	10/26/2005				
5	EJ7A	1026051053	10/26/2005				
6	EJ7B	1026051054	10/26/2005				
7	EJ7G	1026051055	10/26/2005				
8	EJ7GA	1026051056	10/26/2005				
9	HJ9	1026051057	10/26/2005				
10	T01	1026051058	10/26/2005				
11	T01G	1026051059	10/26/2005				
12	T02	1026051060	10/26/2005				
13	T03	1026051061	10/26/2005				
14	T04	1026051062	10/26/2005				
15	T05	1026051063	10/26/2005				
16	T06	1026051064	10/26/2005				
17	T07	1026051065	10/26/2005				
18	T08	1026051066	10/26/2005				
19	T09	1026051067	10/26/2005				
20	T10	1026051068	10/26/2005				
21	T11	1026051069	10/26/2005				
22	T12	1026051070	10/26/2005				
23	T13	1026051071	10/26/2005				
24	T14	1026051072	10/26/2005				
25	T15	1026051073	10/26/2005				
26	T16	1026051074	10/26/2005				
27	T17	1026051075	10/26/2005				
28	T18	1026051076	10/26/2005				
29	T19	1026051077	10/26/2005				
30	T20	1026051078	10/26/2005				
31	T21	1026051079	10/26/2005				
32	T22	1026051080	10/26/2005				
33	T23	1026051081	10/26/2005				
34	T24	1026051082	10/26/2005				
35	T25	1026051083	10/26/2005				
36	T26	1026051084	10/26/2005				
37	T27	1026051085	10/26/2005				
38	T28	1026051086	10/26/2005				
39	T29	1026051087	10/26/2005				
40	T29G	1026051088	10/26/2005				

OCT 26 2005

02:00:39 PM 10/6/2004

Licensee Details

Licensee Information

Name:	**ESCALANTE, HUGO (Primary Name)**
	EWPL INC (DBA Name)
Main Address:	**P.O. BOX 280** **FORT WHITE, Florida 32038**

License Information

License Type:	**Certified Residential Contractor**
Rank:	**Cert Residental**
License Number:	**CRC1326967**
Status:	**Current, Active**
Licensure Date:	**11/24/2003**
Expires:	**08/31/2006**

Special Qualifications	Effective Date
Qualified Business License Required	11/24/2003

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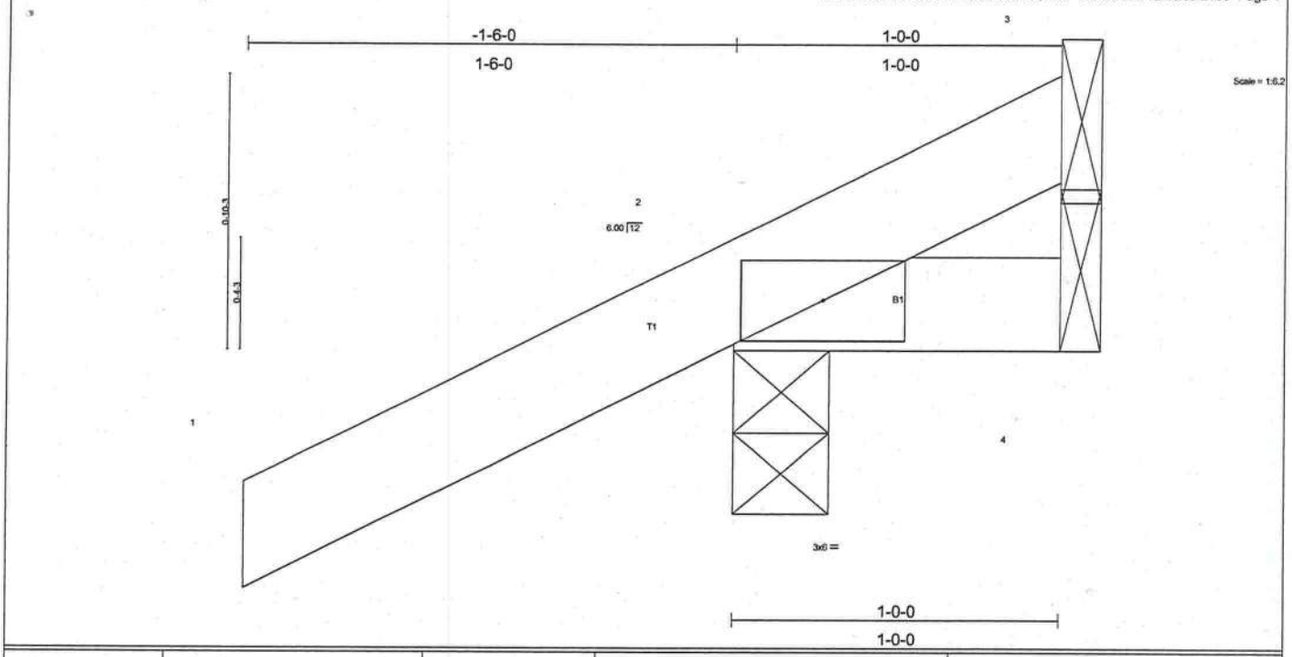
OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg #1026051049

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	CJ1	MONO TRUSS	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:09 2005 Page 1



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TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=189/0-3-8, 4=14/Mechanical, 3=-40/Mechanical
Max Horz 2=84(load case 5)
Max Uplift2=-220(load case 5), 3=-40(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=73(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/41
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 220 lb uplift at joint 2 and 40 lb uplift at joint 3.

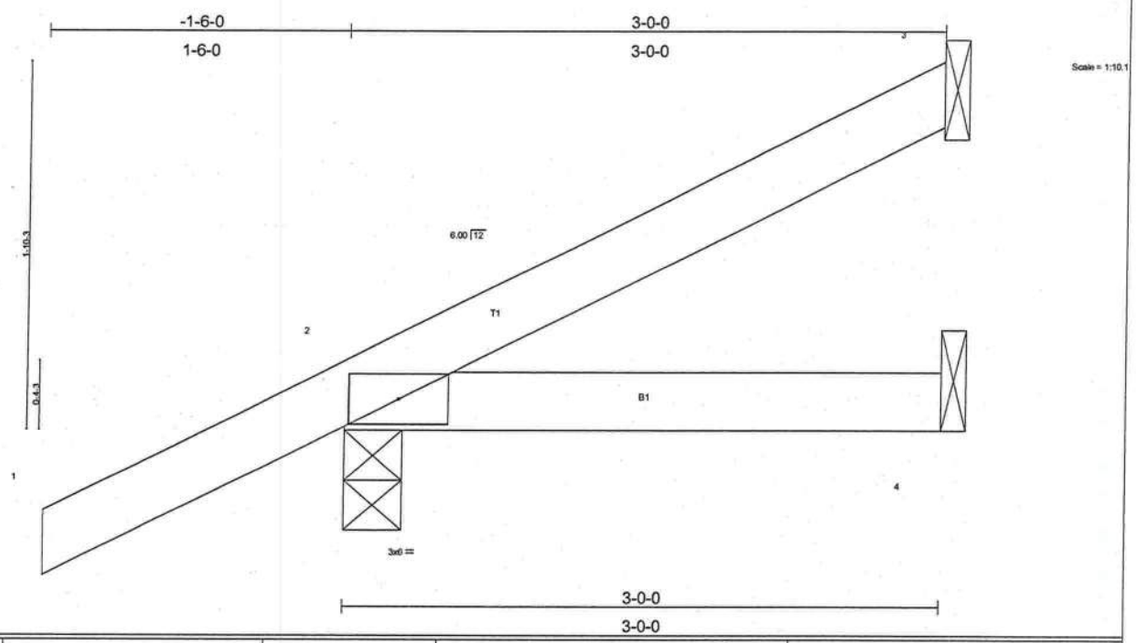
LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051050

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	CJ3	MONO TRUSS	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:10 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical
Max Horz 2=137(load case 5)
Max Uplift3=-47(load case 5), 2=-187(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-52/16
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 3 and 187 lb uplift at joint 2.

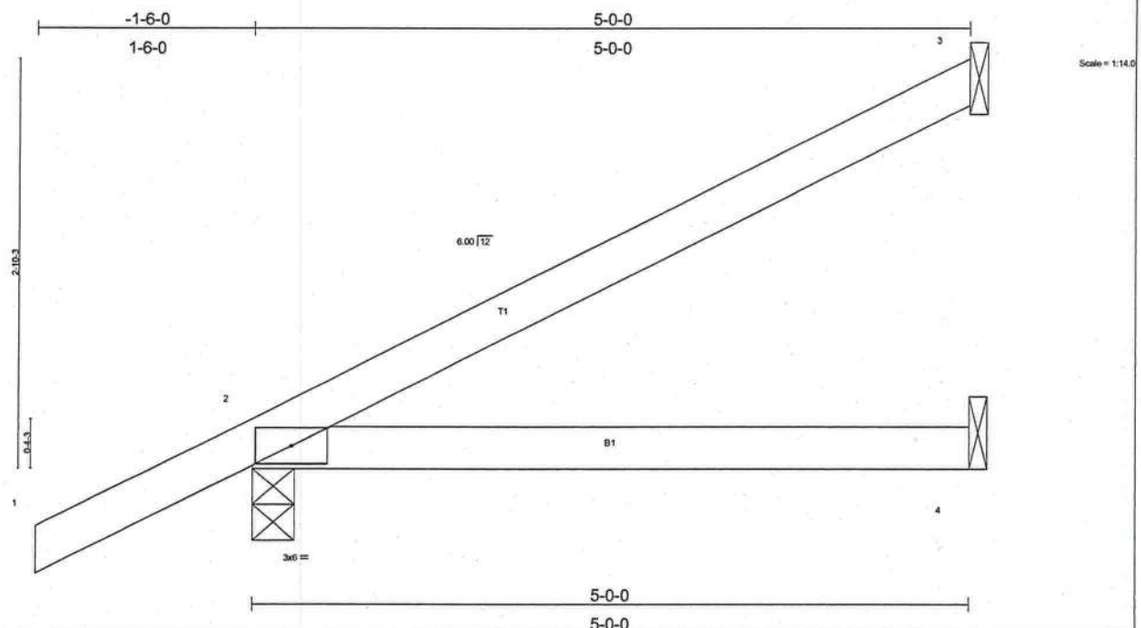
LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051051

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	CJ5	MONO TRUSS	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:10 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=192(load case 5)
Max Uplift3=-124(load case 5), 2=-197(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-112/41
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 3 and 197 lb uplift at joint 2.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051052

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	EJ7	MONO TRUSS	29	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:11 2005 Page 1

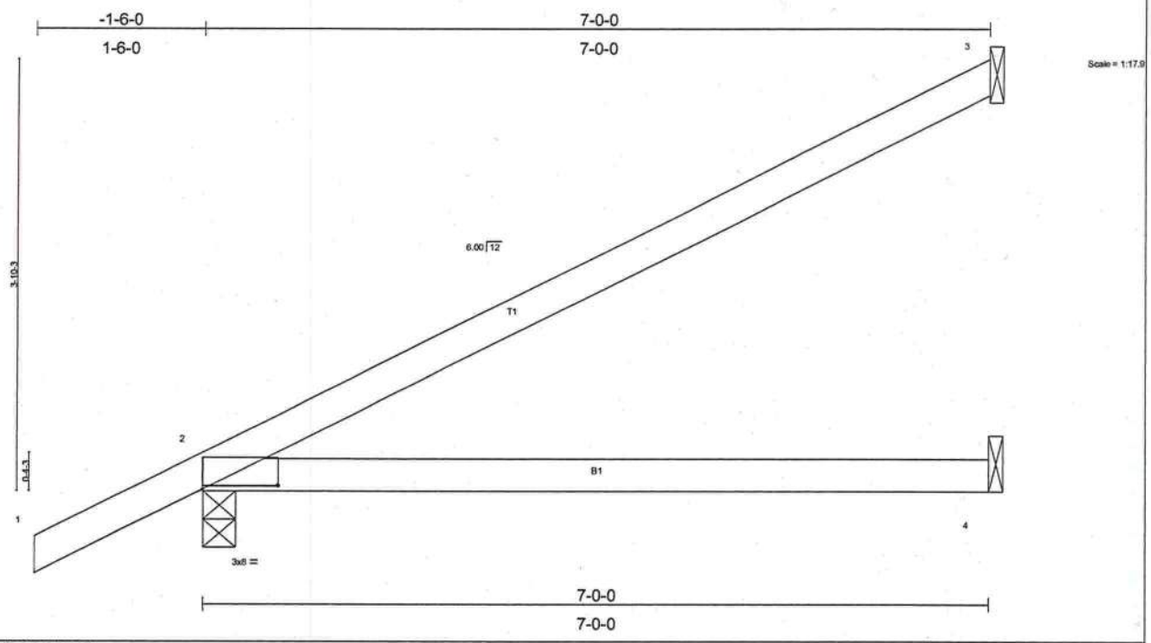


Plate Offsets (X,Y): [2:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.13	2-4	>606	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.22	2-4	>365	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=166/Mechanical, 2=385/0-3-8, 4=108/Mechanical
Max Horz 2=247(load case 5)
Max Uplift3=-170(load case 5), 2=-217(load case 5), 4=-1(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-142/59
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 3, 217 lb uplift at joint 2 and 1 lb uplift at joint 4.

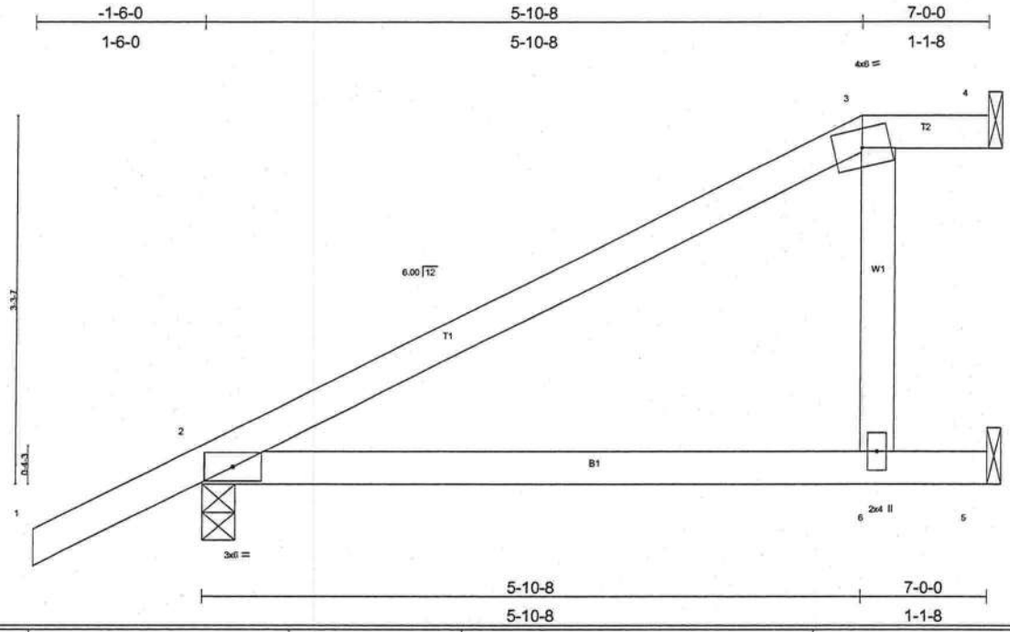
LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051053

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	EJ7A	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:12 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.20	2-6	>404	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.25	2-6	>323	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.06	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=77/Mechanical, 2=385/0-3-8, 5=197/Mechanical
Max Horz 2=219(load case 5)
Max Uplift4=-13(load case 3), 2=-229(load case 5), 5=-160(load case 5)
Max Grav 4=94(load case 10), 2=385(load case 1), 5=197(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-113/42, 3-4=-0/1
BOT CHORD 2-6=-15/5, 5-6=0/0
WEBS 3-6=-112/302

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 4, 229 lb uplift at joint 2 and 160 lb uplift at joint 5.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051054

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	EJ7B	MONO HIP	1	1	Job Reference (optional)

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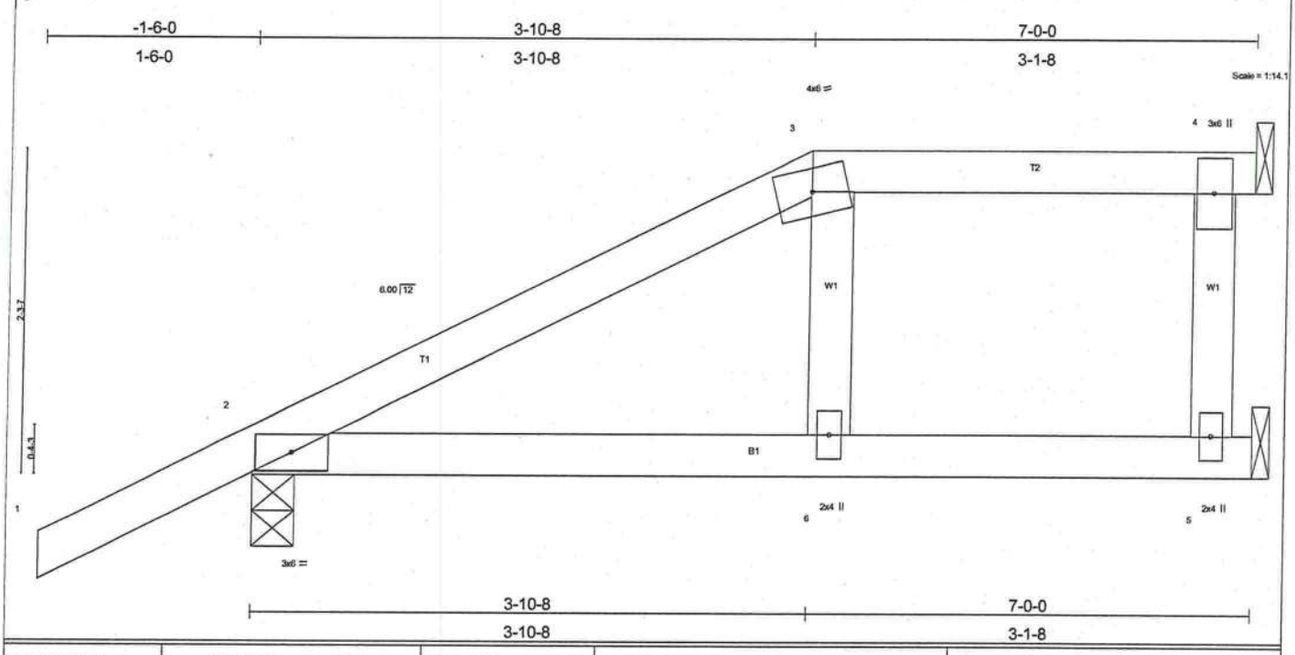

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.19	2-6	>422	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.22	2-6	>355	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.11	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=376/0-3-8, 4=138/Mechanical, 5=127/Mechanical
Max Horz 2=164(load case 5)
Max Uplift2=-236(load case 5), 4=-83(load case 4), 5=-36(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-80/6, 3-4=-0/0
BOT CHORD 2-6=-12/4, 5-6=0/0
WEBS 3-6=-55/167, 4-5=0/0

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 236 lb uplift at joint 2, 83 lb uplift at joint 4 and 36 lb uplift at joint 5.
6) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051055

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	EJ7G	MONO TRUSS	1	1	Job Reference (optional)

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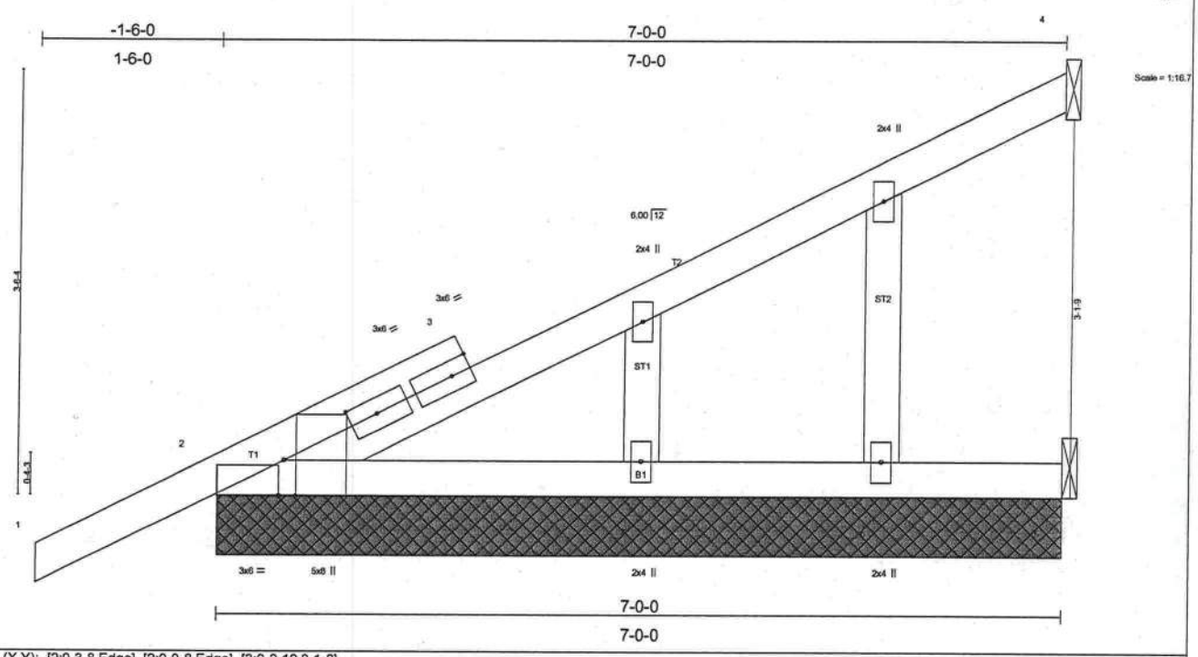

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [3:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.04	2-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	0.03	2-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except*
T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/7-0-0, 4=146/Mechanical, 5=35/Mechanical, 7=243/7-0-0, 6=-27/7-0-0
Max Horz 2=229(load case 5)
Max Uplift2=-175(load case 5), 4=-154(load case 5), 5=-12(load case 5), 7=-116(load case 5), 6=-27(load case 1)
Max Grav 2=266(load case 1), 4=146(load case 1), 5=35(load case 1), 7=243(load case 1), 6=73(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-146/0, 3-4=-112/49
BOT CHORD 2-7=0/0, 6-7=0/0, 5-6=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Gable studs spaced at 2-0-0 oc.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 175 lb uplift at joint 2, 154 lb uplift at joint 4, 12 lb uplift at joint 5, 116 lb uplift at joint 7 and 27 lb uplift at joint 6.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051056

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	EJ7GA	MONO HIP	1	1	Job Reference (optional)

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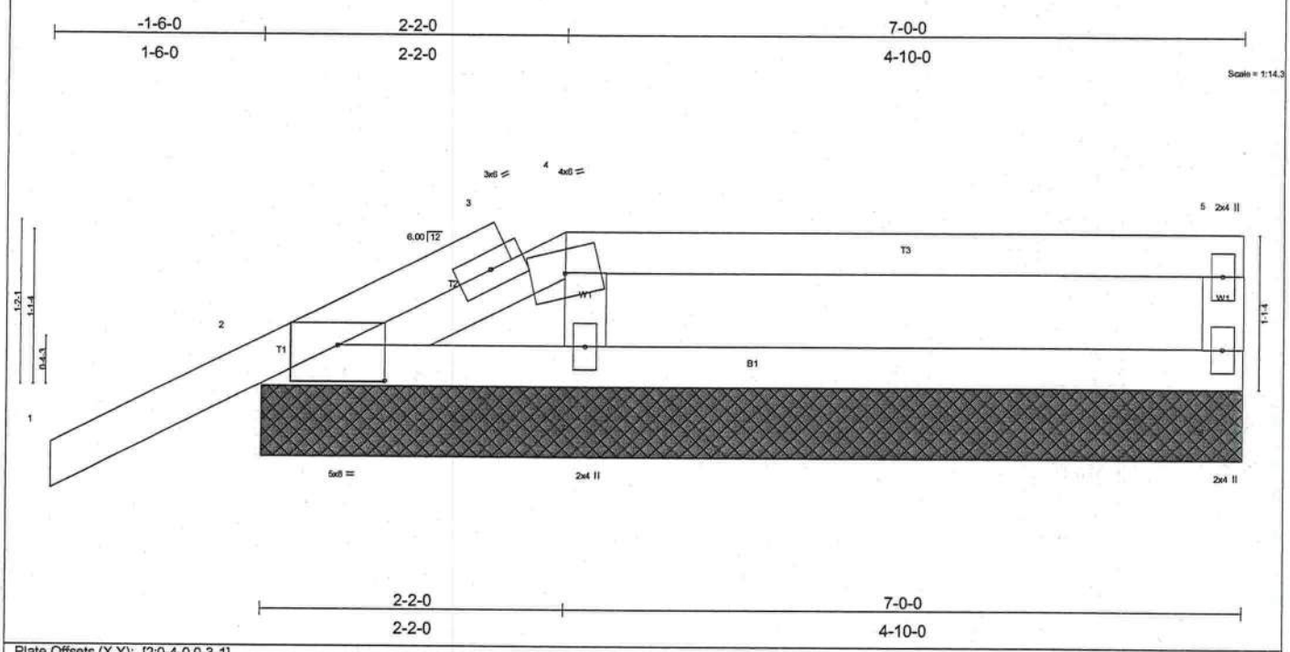

Plate Offsets (X,Y): [2:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.01	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.10	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 27 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=198/7-0-0, 6=204/7-0-0, 7=464/7-0-0
Max Horz 2=99(load case 5)
Max Uplift2=-196(load case 5), 6=-100(load case 3), 7=-206(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8/51, 2-3=-77/28, 3-4=-10/10, 4-5=-0/0
BOT CHORD 2-7=-70/60, 6-7=0/0
WEBS 4-7=-338/393, 5-6=-148/173

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 196 lb uplift at joint 2, 100 lb uplift at joint 6 and 206 lb uplift at joint 7.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-79(F=-25), 4-5=-79(F=-25), 2-6=-30

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051057

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	HJ9	MONO TRUSS	4	1	Job Reference (optional)

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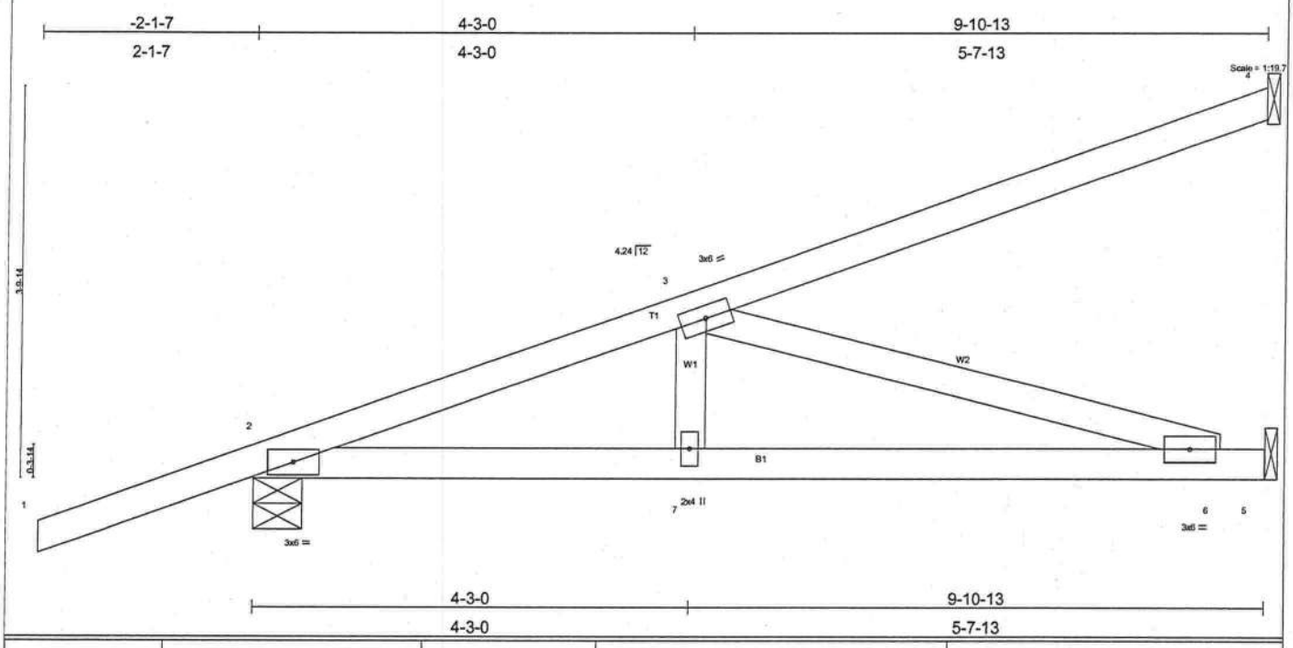

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.18	6-7	>626	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.50	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-6-11 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=486/0-5-11, 5=386/Mechanical
Max Horz 2=303(load case 2)
Max Uplift4=-289(load case 2), 2=-285(load case 2), 5=-107(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-934/246, 3-4=-124/65
BOT CHORD 2-7=-475/869, 6-7=-475/869, 5-6=0/0
WEBS 3-7=0/208, 3-6=-906/495

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 289 lb uplift at joint 4, 285 lb uplift at joint 2 and 107 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051058

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T01	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

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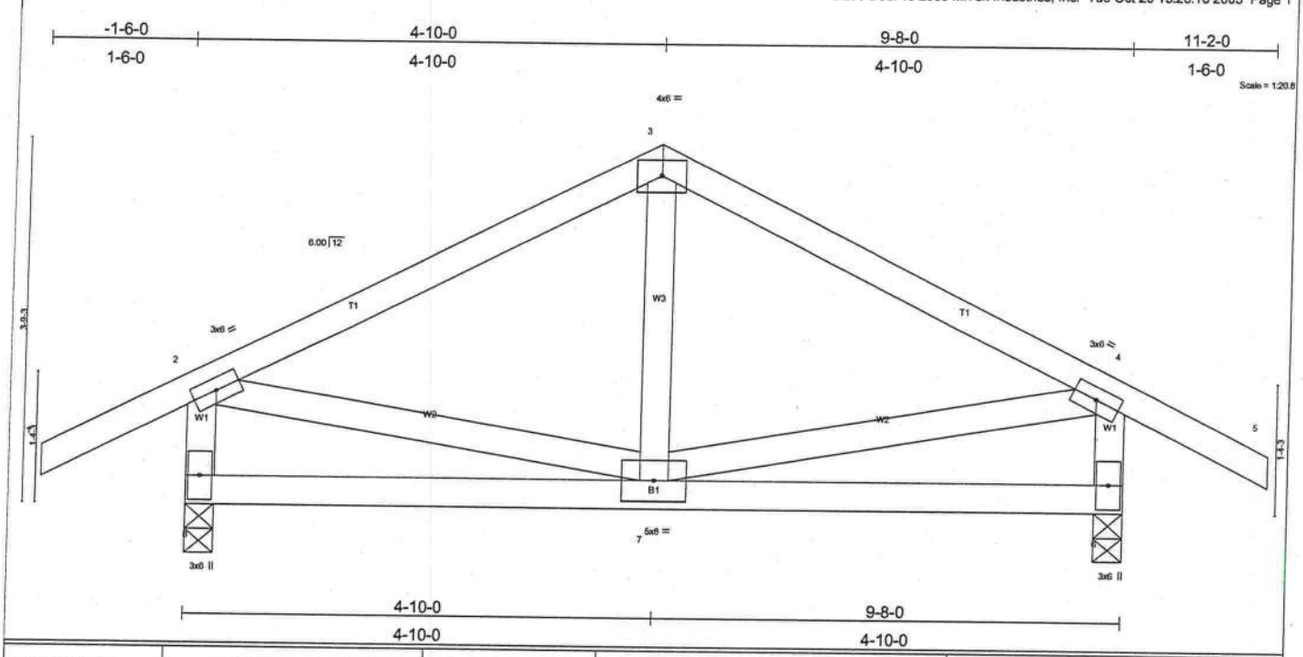

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.03	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	0.03	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 56 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=483/0-3-8, 6=483/0-3-8
Max Horz 8=87(load case 4)
Max Uplift8=-417(load case 5), 6=-417(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-380/578, 3-4=-380/578, 4-5=0/40, 2-8=-413/604, 4-6=-413/604
BOT CHORD 7-8=-134/83, 6-7=-71/83
WEBS 3-7=-223/67, 2-7=-219/222, 4-7=-219/222

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 417 lb uplift at joint 8 and 417 lb uplift at joint 6.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051059

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T01G	COMMON	1	1	Job Reference (optional)

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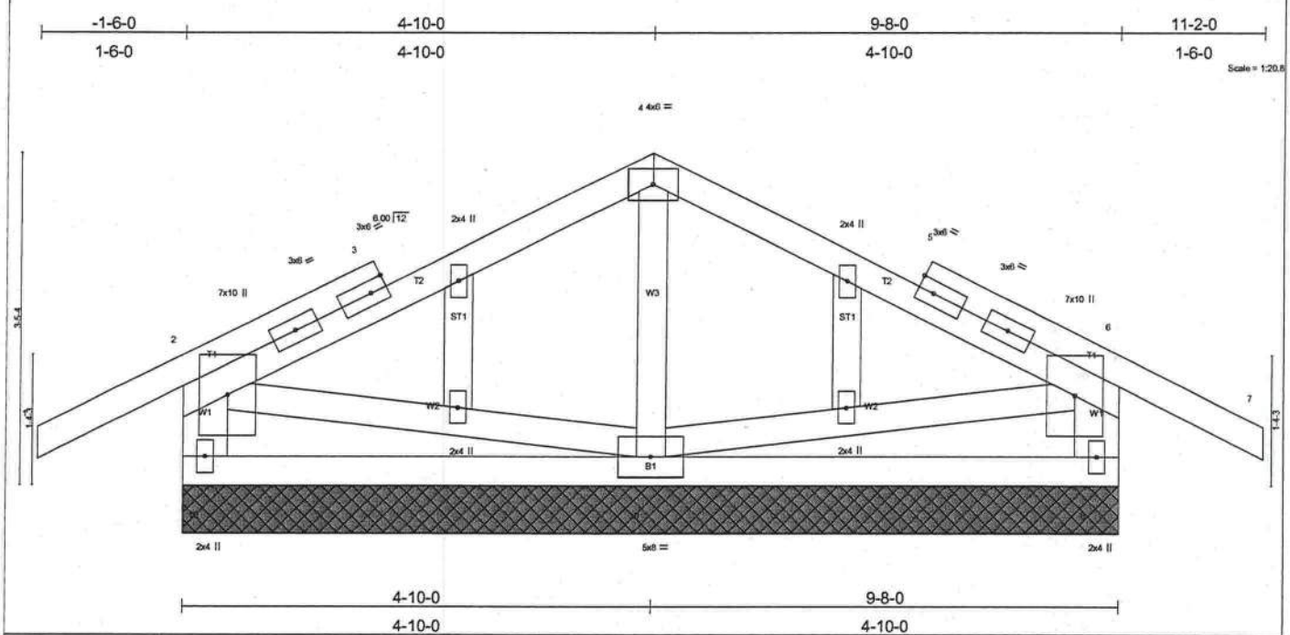

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.01	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.07	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 67 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 6 SYP No.1D, W1 2 X 6 SYP No.1D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 10=401/9-8-0, 8=401/9-8-0, 9=474/9-8-0
Max Horz 10=79(load case 4)
Max Uplift10=-266(load case 5), 8=-269(load case 6), 9=-186(load case 5)
Max Grav 10=405(load case 9), 8=405(load case 10), 9=474(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/51, 2-3=-118/80, 3-4=-41/76, 4-5=-41/76, 5-6=-118/80, 6-7=-6/51, 2-10=-338/386, 6-8=-338/386
BOT CHORD 9-10=-109/151, 8-9=-36/151
WEBS 4-9=-292/263, 2-9=-120/163, 6-9=-120/163

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 10, 269 lb uplift at joint 8 and 186 lb uplift at joint 9.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-79(F=-25), 2-4=-79(F=-25), 4-6=-79(F=-25), 6-7=-79(F=-25), 8-10=-30

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051060

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T02	HIP	1	1	Job Reference (optional)

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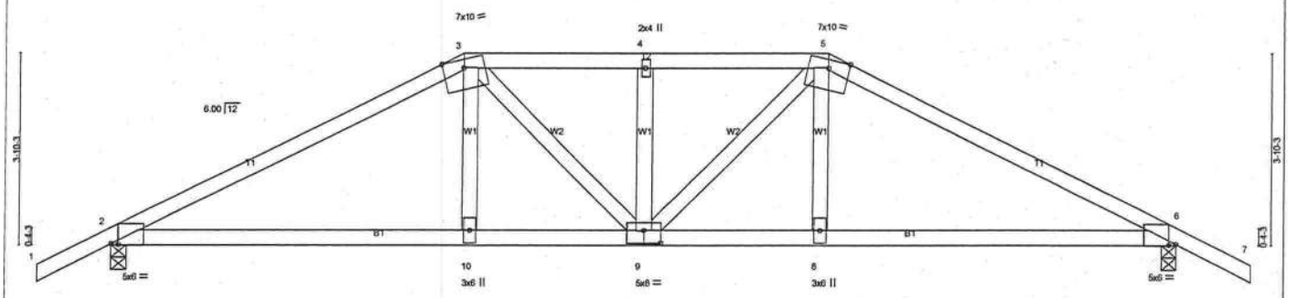
-1-6-0	3-9-4	7-0-0	10-8-0	14-4-0	17-6-12	21-4-0	22-10-0
1-6-0	3-9-4	3-2-12	3-8-0	3-8-0	3-2-12	3-9-4	1-6-0

Scale = 1:40.4



3-9-4	7-0-0	10-8-0	14-4-0	17-6-12	21-4-0
3-9-4	3-2-12	3-8-0	3-8-0	3-2-12	3-9-4

Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-1-11,Edge], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.16	6-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.26	6-8	>955	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 100 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-3-8 oc bracing.

REACTIONS (lb/size) 2=1841/0-3-8, 6=1841/0-3-8
Max Horz 2=-92(load case 5)
Max Uplift2=-978(load case 4), 6=-978(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-3339/1619, 3-4=-3176/1615, 4-5=-3176/1615, 5-6=-3339/1619, 6-7=0/35
BOT CHORD 2-10=-1368/2897, 9-10=-1380/2930, 8-9=-1327/2930, 6-8=-1316/2897
WEBS 3-10=-291/803, 3-9=-315/472, 4-9=-365/397, 5-9=-316/472, 5-8=-291/803

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 978 lb uplift at joint 2 and 978 lb uplift at joint 6.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 321 lb up at 14-4-0, and 539 lb down and 321 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-113(F=-58), 5-7=-54, 2-10=-30, 8-10=-62(F=-33), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-539(F) 8=-539(F)

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051061

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T03	HIP	1	1	Job Reference (optional)

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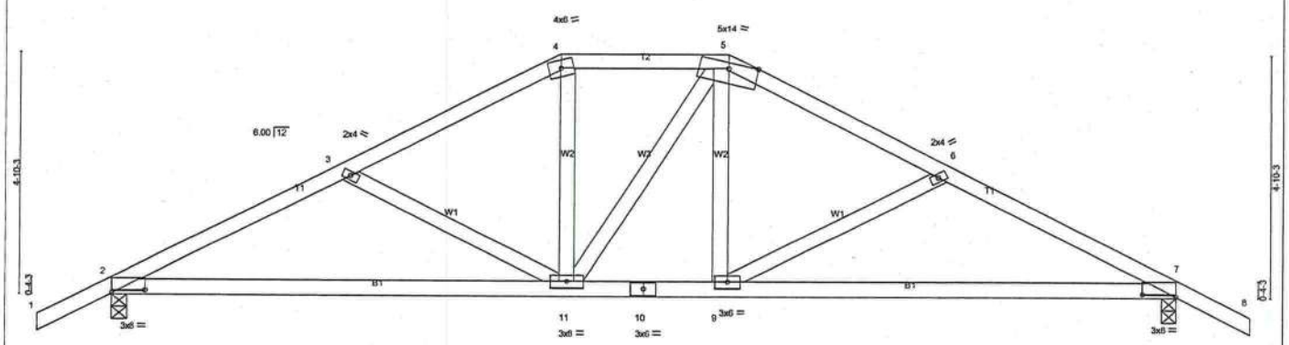
-1-6-0	4-9-4	9-0-0	12-4-0	16-6-12	21-4-0	22-10-0
1-6-0	4-9-4	4-2-12	3-4-0	4-2-12	4-9-4	1-6-0

Scale = 1:40.4



9-0-0	12-4-0	21-4-0
9-0-0	3-4-0	9-0-0

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [7:0-8-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.18	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.30	7-9	>829	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 107 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-7 oc bracing.

REACTIONS (lb/size) 2=973/0-3-8, 7=973/0-3-8
Max Horz 2=-109(load case 6)
Max Uplift2=-475(load case 6), 7=-475(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1468/815, 3-4=-1199/657, 4-5=-1030/648, 5-6=-1198/657, 6-7=-1468/815, 7-8=0/35
BOT CHORD 2-11=-568/1277, 10-11=-304/1028, 9-10=-304/1028, 7-9=-568/1277
WEBS 3-11=-291/300, 4-11=-100/314, 5-11=-102/106, 5-9=-100/315, 6-9=-293/300

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 475 lb uplift at joint 2 and 475 lb uplift at joint 7.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051062

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T04	COMMON	2	1	Job Reference (optional)

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-1-6-0 | 5-7-4 | 10-8-0 | 15-8-12 | 21-4-0 | 22-10-0
1-6-0 | 5-7-4 | 5-0-12 | 5-0-12 | 5-7-4 | 1-6-0
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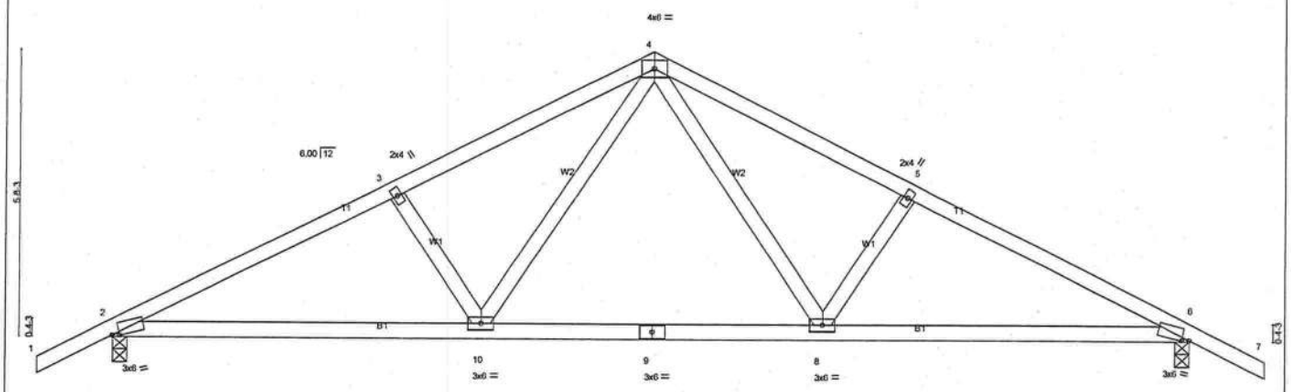

7-3-8 | 14-0-8 | 21-4-0
7-3-8 | 6-9-0 | 7-3-8

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.19	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.31	8-10	>821	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 100 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-10 oc bracing.

REACTIONS (lb/size) 2=1141/0-3-8, 6=1141/0-3-8
Max Horz 2=-123(load case 6)
Max Uplift2=-568(load case 5), 6=-568(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1888/1014, 3-4=-1732/1007, 4-5=-1732/1007, 5-6=-1888/1014, 6-7=0/35
BOT CHORD 2-10=-732/1619, 9-10=-383/1112, 8-9=-383/1112, 6-8=-732/1619
WEBS 3-10=-250/291, 4-10=-365/734, 4-8=-364/734, 5-8=-250/291

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 568 lb uplift at joint 2 and 568 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051063

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135423	T05	COMMON	4	1	Job Reference (optional)

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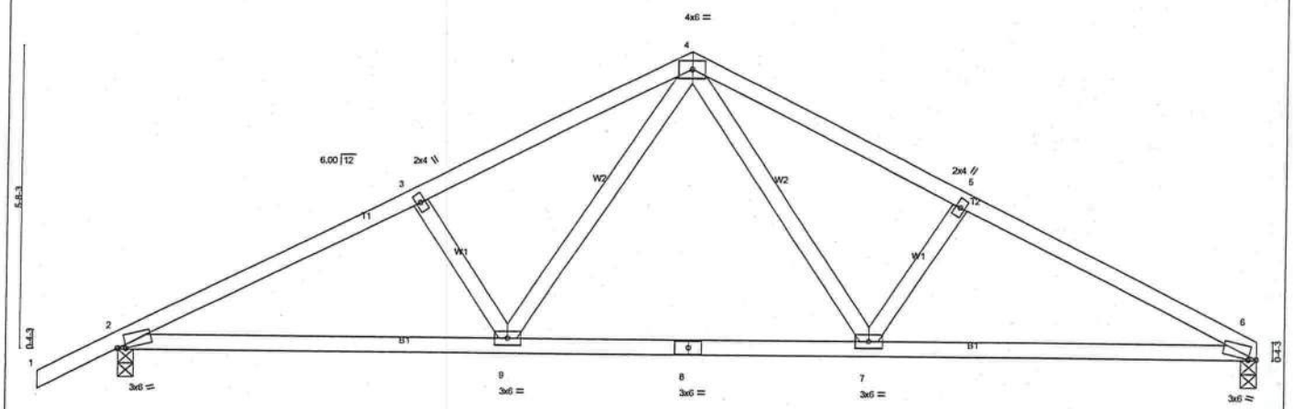
-1-6-0 | 5-7-4 | 10-8-0 | 15-8-12 | 21-4-0

1-6-0 | 5-7-4 | 5-0-12 | 5-0-12 | 5-7-4

Scale = 1:37.7



7-3-8 | 14-0-8 | 21-4-0

7-3-8 | 6-9-0 | 7-3-8

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.19	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.30	7-9	>829	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.29	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 97 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-7-13 oc bracing.

REACTIONS (lb/size) 6=1049/0-3-8, 2=1145/0-3-8
Max Horz 2=144(load case 5)
Max Uplift6=-450(load case 6), 2=-570(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1895/1028, 3-4=-1739/1020, 4-5=-1753/1046, 5-6=-1895/1055
BOT CHORD 2-9=-813/1625, 8-9=-465/1119, 7-8=-465/1119, 6-7=-845/1642
WEBS 3-9=-250/292, 4-9=-361/734, 4-7=-402/753, 5-7=-260/311

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 450 lb uplift at joint 6 and 570 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-9=-30, 7-9=-80(F=-50), 6-7=-30

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051064

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T06	SPECIAL	1	1	Job Reference (optional)

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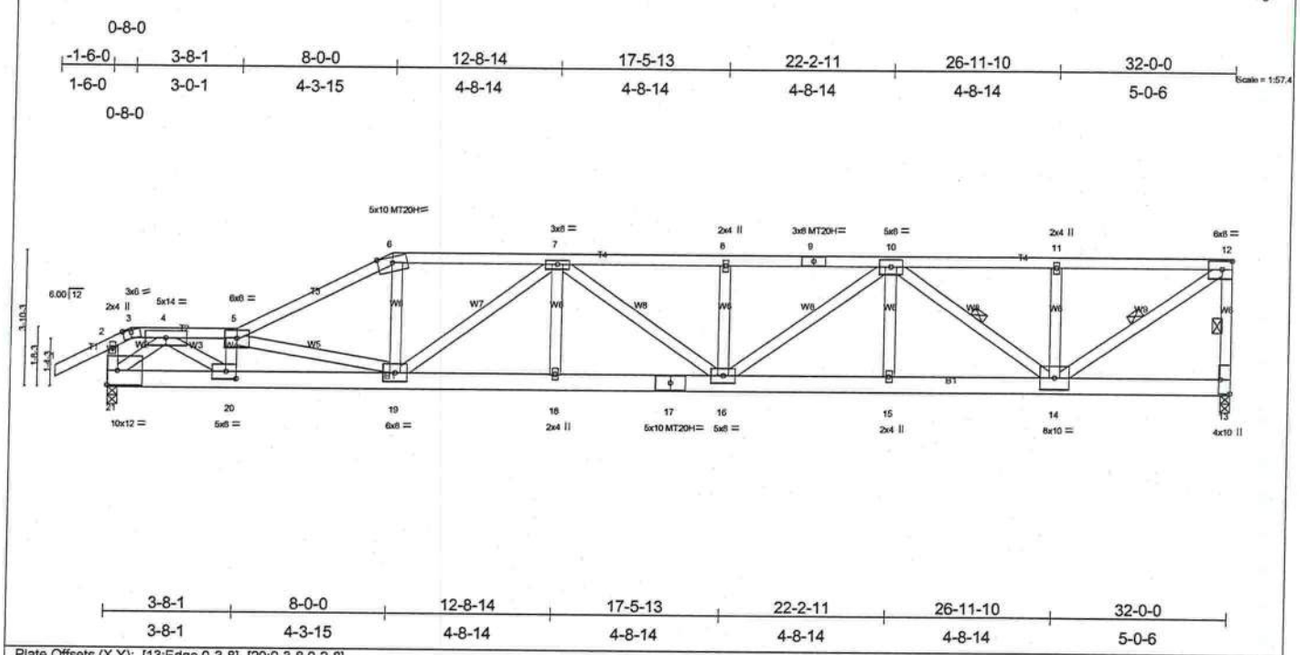


Plate Offsets (X,Y): [13:Edge,0-3-8], [20:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.87	Vert(LL)	0.47	16-18	>814	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.74	16-18	>515	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.84	Horz(TL)	0.14	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 208 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2, W9 2 X 4 SYP No.2, W1 2 X 4 SYP No.1D, W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-4-11 oc bracing.
WEBS 1 Row at midpt 12-13, 10-14, 12-14

REACTIONS (lb/size) 13=2838/0-3-8, 21=2696/0-3-8
Max Horz 21=226(load case 4)
Max Uplift13=-1601(load case 3), 21=-1421(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-258/154, 3-4=-214/142, 4-5=-5976/3114, 5-6=-5829/3177, 6-7=-5292/2935, 7-8=-6576/3693, 8-9=-6576/3693, 9-10=-6576/3693, 10-11=-3449/1939, 11-12=-3449/1939, 12-13=-2651/1587, 2-21=-237/227
BOT CHORD 20-21=-1554/2812, 19-20=-3335/6244, 18-19=-3636/6480, 17-18=-3636/6480, 16-17=-3636/6480, 15-16=-3162/5610, 14-15=-3162/5610, 13-14=-72/100
WEBS 4-20=-1998/3868, 5-20=-2401/1319, 5-19=-1086/592, 6-19=-1072/2149, 7-19=-1489/909, 7-18=0/295, 7-16=-91/119, 8-16=-521/519, 10-16=-659/1198, 10-15=0/335, 10-14=-2678/1516, 11-14=-545/554, 12-14=-2290/4108, 4-21=-3427/1790

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1601 lb uplift at joint 13 and 1421 lb uplift at joint 21.
5) Girder carries hip end with 0-0-0 right side setback, 8-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 616 lb down and 397 lb up at 8-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 3-5=-54, 5-6=-54, 6-12=-113(F=-59), 19-21=-30, 13-19=-62(F=-33)
Concentrated Loads (lb)
Vert: 19=-616(F)

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

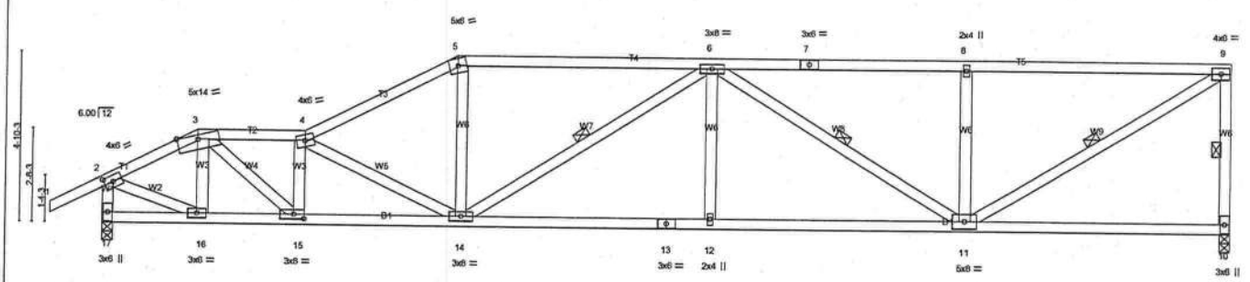
Dwg.#1026051065

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T07	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:21 2005 Page 1

-1-6-0	2-8-0	5-8-1	10-0-0	17-2-13	24-5-11	32-0-0
1-6-0	2-8-0	3-0-1	4-3-15	7-2-13	7-2-13	7-6-5

Scale = 1:57.2



2-8-0	5-8-1	10-0-0	17-2-13	24-5-11	32-0-0
2-8-0	3-0-1	4-3-15	7-2-13	7-2-13	7-6-5

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [15:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.21	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.34	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.08	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 184 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-3-7 oc bracing.
WEBS 1 Row at midpt 9-10, 6-14, 6-11, 9-11

REACTIONS (lb/size) 10=1329/0-3-8, 17=1423/0-3-8
Max Horz 17=283(load case 5)
Max Uplift10=-619(load case 4), 17=-613(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1515/771, 3-4=-2461/1281, 4-5=-2293/1181, 5-6=-2053/1135, 6-7=-1733/901, 7-8=-1733/901, 8-9=-1733/901, 9-10=-1218/698, 2-17=-1373/825
BOT CHORD 16-17=-248/33, 15-16=-829/1292, 14-15=-1445/2516, 13-14=-1257/2373, 12-13=-1257/2373, 11-12=-1257/2373, 10-11=-34/68
WEBS 3-16=-458/255, 3-15=-760/1522, 4-15=-900/520, 4-14=-548/358, 5-14=-214/661, 6-14=-380/269, 6-12=0/217, 6-11=-756/421, 8-11=-406/366, 9-11=-1018/1957, 2-16=-647/1394

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 619 lb uplift at joint 10 and 613 lb uplift at joint 17.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051066

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T08	SPECIAL	1	1	Job Reference (optional)

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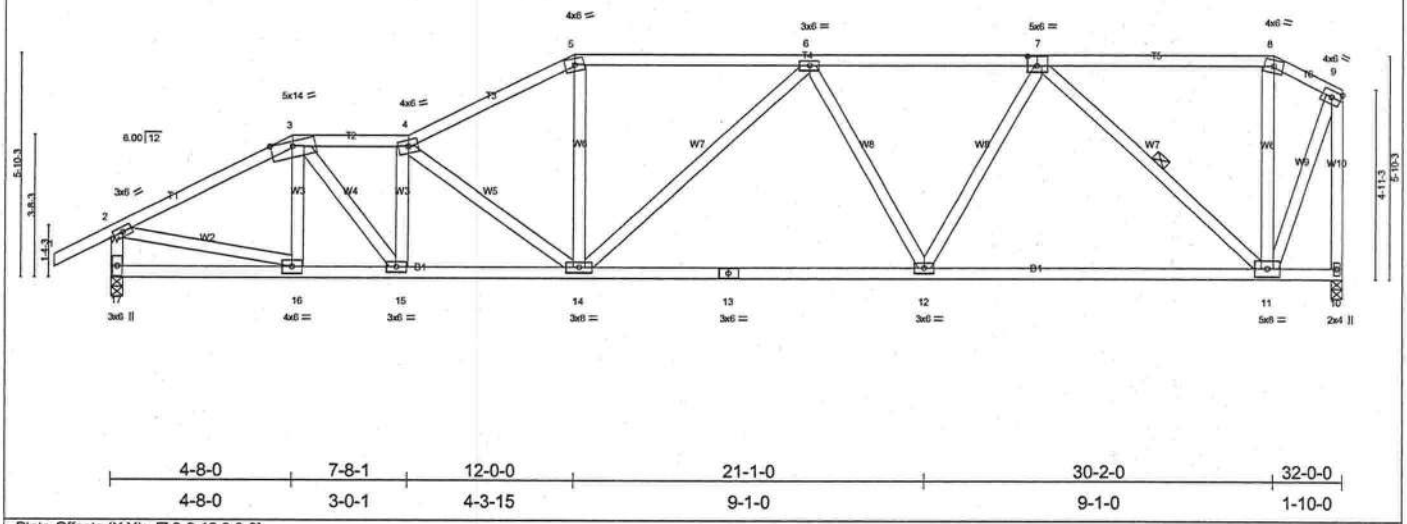
-1-6-0	4-8-0	7-8-1	12-0-0	18-0-11	24-1-5	30-2-0	32-0-0
1-6-0	4-8-0	3-0-1	4-3-15	6-0-11	6-0-11	6-0-11	1-10-0

Scale = 1:57.2



4-8-0	7-8-1	12-0-0	21-1-0	30-2-0	32-0-0
4-8-0	3-0-1	4-3-15	9-1-0	9-1-0	1-10-0

Plate Offsets (X,Y): [7:0-2-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.91	Vert(LL)	-0.21	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.36	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.07	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 202 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-14 oc bracing.
WEBS 1 Row at midpt 7-11

REACTIONS (lb/size) 17=1423/0-3-8, 10=1329/0-3-8
Max Horz 17=305(load case 5)
Max Uplift17=-631(load case 5), 10=-545(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1791/910, 3-4=-2210/1195, 4-5=-2058/1090, 5-6=-1831/1048, 6-7=-1724/926, 7-8=-459/274, 8-9=-500/256, 2-17=-1340/832, 9-10=-1376/671
BOT CHORD 16-17=-316/130, 15-16=-940/1535, 14-15=-1292/2242, 13-14=-1018/1878, 12-13=-1018/1878, 11-12=-760/1416, 10-11=-5/4
WEBS 3-16=-225/177, 3-15=-519/1046, 4-15=-739/414, 4-14=-535/379, 5-14=-206/597, 6-14=-202/170, 6-12=-322/302, 7-12=-237/645, 7-11=-1317/739, 8-11=-56/119, 2-16=-649/1455, 9-11=-604/1254

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 631 lb uplift at joint 17 and 545 lb uplift at joint 10.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051067

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:23 2005 Page 1

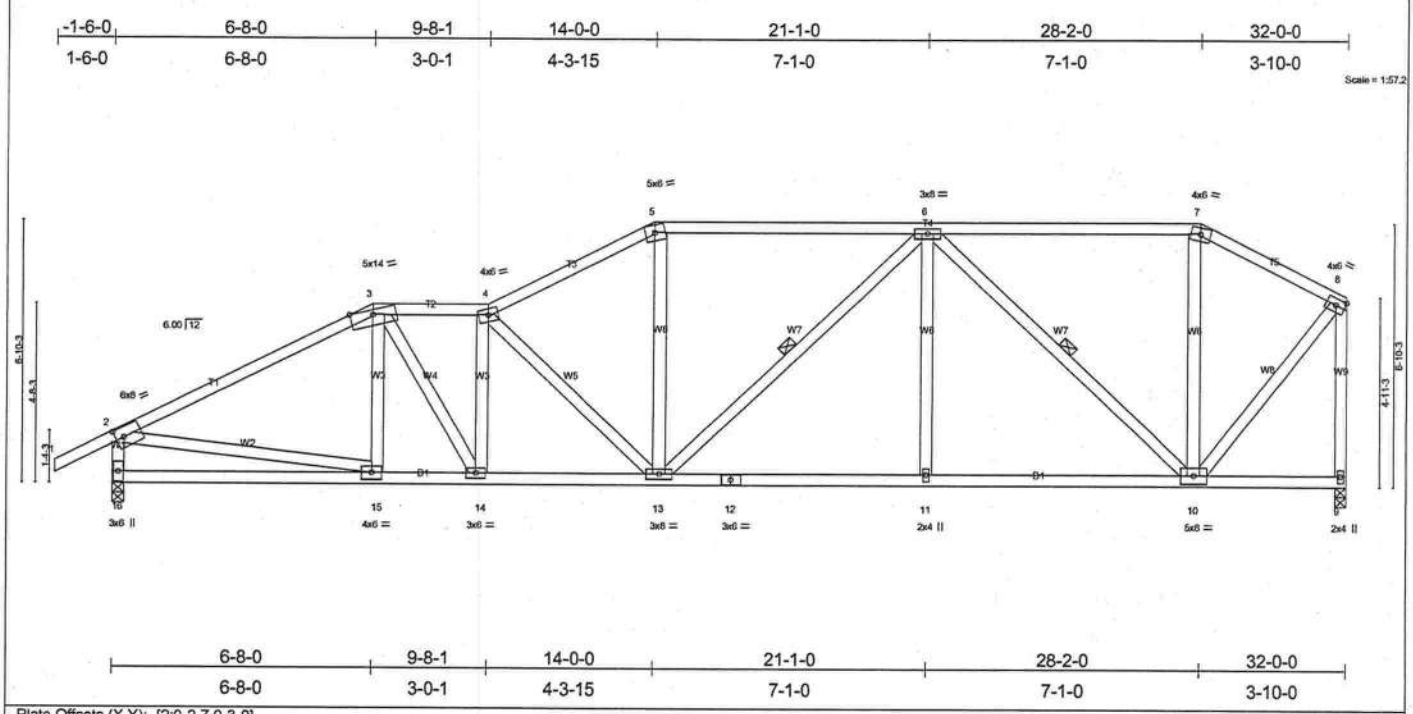


Plate Offsets (X,Y): [2:0-2-7,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.83	Vert(LL)	-0.14	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.22	11-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 210 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-11-4 oc bracing.
WEBS 1 Row at midpt 6-13, 6-10

REACTIONS (lb/size) 16=1423/0-3-8, 9=1329/0-3-8
Max Horz 16=321(load case 5)
Max Uplift16=-648(load case 5), 9=-479(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1887/964, 3-4=-1994/1119, 4-5=-1825/1019, 5-6=-1623/987, 6-7=-719/476, 7-8=-830/462, 2-16=-1310/844, 8-9=-1290/709
BOT CHORD 15-16=-412/271, 14-15=-959/1608, 13-14=-1147/2011, 12-13=-781/1497, 11-12=-781/1497, 10-11=-781/1497, 9-10=-8/7
WEBS 3-15=-73/130, 3-14=-327/706, 4-14=-525/287, 4-13=-559/385, 5-13=-145/463, 6-13=-161/183, 6-11=0/218, 6-10=-1088/575, 7-10=0/93, 2-15=-559/1358, 8-10=-566/1125

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 648 lb uplift at joint 16 and 479 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

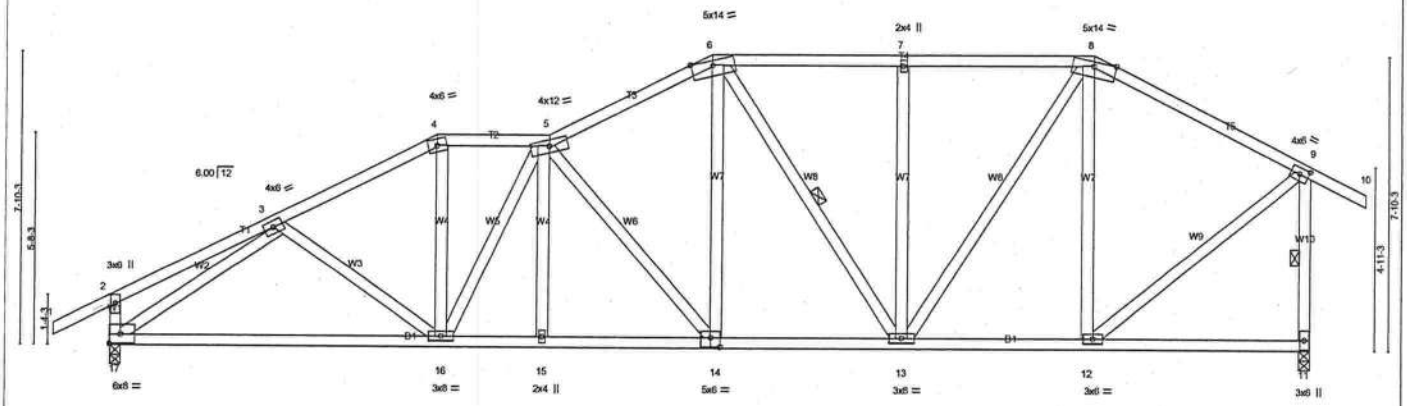
Dwg.#1026051068

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:24 2005 Page 1

-1-6-0	4-5-12	8-8-0	11-8-1	16-0-0	21-1-0	26-2-0	32-0-0	33-6-0
1-6-0	4-5-12	4-2-4	3-0-1	4-3-15	5-1-0	5-1-0	5-10-0	1-6-0

Scale = 1:58.7



8-8-0	11-8-1	16-0-0	21-1-0	26-2-0	32-0-0
8-8-0	3-0-1	4-3-15	5-1-0	5-1-0	5-10-0

Plate Offsets (X,Y): [9:0-2-15,0-2-0], [14:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.13	16-17	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.23	16-17	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.06	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 229 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-1-2 oc bracing.
WEBS 1 Row at midpt 6-13, 9-11

REACTIONS (lb/size) 17=1421/0-3-8, 11=1421/0-3-8
Max Horz 17=297(load case 4)
Max Uplift17=-674(load case 5), 11=-565(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-335/151, 3-4=-1802/1023, 4-5=-1583/979, 5-6=-1611/1004, 6-7=-1264/887, 7-8=-1264/887, 8-9=-1024/684, 9-10=0/40, 2-17=-348/327, 9-11=-1335/865
BOT CHORD 16-17=-715/1467, 15-16=-786/1806, 14-15=-786/1805, 13-14=-558/1415, 12-13=-333/844, 11-12=-46/98
WEBS 3-16=-58/162, 4-16=-262/561, 5-16=-476/290, 5-15=0/46, 5-14=-629/443, 6-14=-312/645, 6-13=-288/181, 7-13=-281/247, 8-13=-376/793, 8-12=-495/280, 3-17=-1538/913, 9-12=-410/1060

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 674 lb uplift at joint 17 and 565 lb uplift at joint 11.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

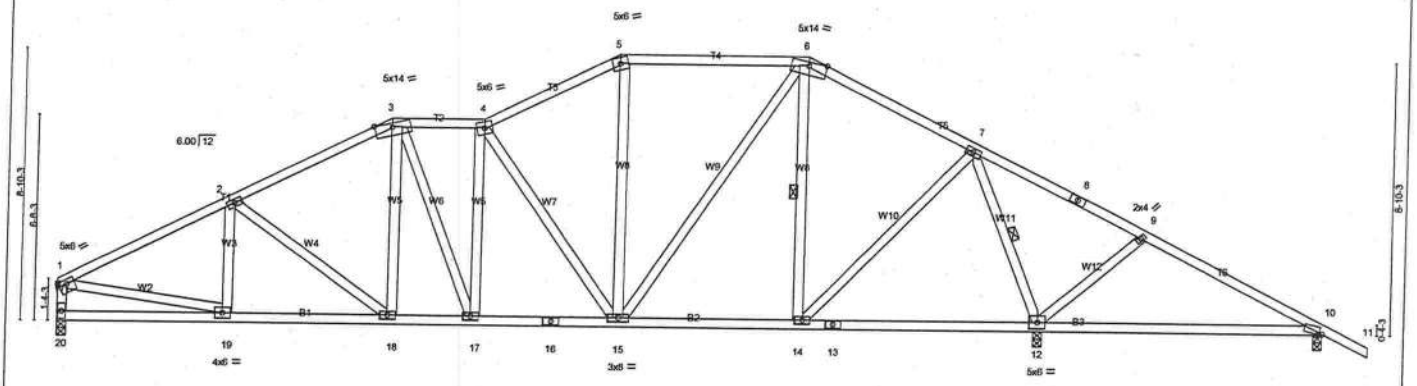
Dwg.#1026051069

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T11	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:25 2005 Page 1

5-5-12	10-8-0	13-8-1	18-0-0	24-2-0	29-7-13	35-1-11	41-2-0	42-8-0
5-5-12	5-2-4	3-0-1	4-3-15	6-2-0	5-5-13	5-5-13	6-0-5	1-6-0

Scale = 1:71.7



5-5-12	10-8-0	13-8-1	18-0-0	24-2-0	31-10-4	41-2-0
5-5-12	5-2-4	3-0-1	4-3-15	6-2-0	7-8-4	9-3-12

Plate Offsets (X,Y): [1:Edge,0-1-12], [10:0-1-5,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	0.41	10-12	>266	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	0.35	10-12	>312	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.04	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 260 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-14, 7-12

REACTIONS (lb/size) 20=1256/0-3-8, 12=2045/0-3-8, 10=221/0-3-8
Max Horz 20=-209(load case 3)
Max Uplift20=-539(load case 5), 12=-913(load case 6), 10=-345(load case 6)
Max Grav 20=1256(load case 1), 12=2045(load case 1), 10=282(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1744/943, 2-3=-1589/943, 3-4=-1461/954, 4-5=-1238/836, 5-6=-1076/816, 6-7=-886/612, 7-8=-158/587, 8-9=-177/465, 9-10=-79/357, 10-11=0/35, 1-20=-1167/676
BOT CHORD 19-20=-195/213, 18-19=-652/1501, 17-18=-494/1368, 16-17=-511/1464, 15-16=-511/1464, 14-15=-114/728, 13-14=0/174, 12-13=0/174, 10-12=-287/160
WEBS 2-19=-118/170, 2-18=-177/204, 3-18=-104/235, 3-17=-111/228, 4-17=-121/128, 4-15=-681/476, 5-15=-97/225, 6-15=-346/648, 6-14=-449/309, 7-14=-301/899, 7-12=-1633/892, 9-12=-351/440, 1-19=-600/1315

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 539 lb uplift at joint 20, 913 lb uplift at joint 12 and 345 lb uplift at joint 10.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

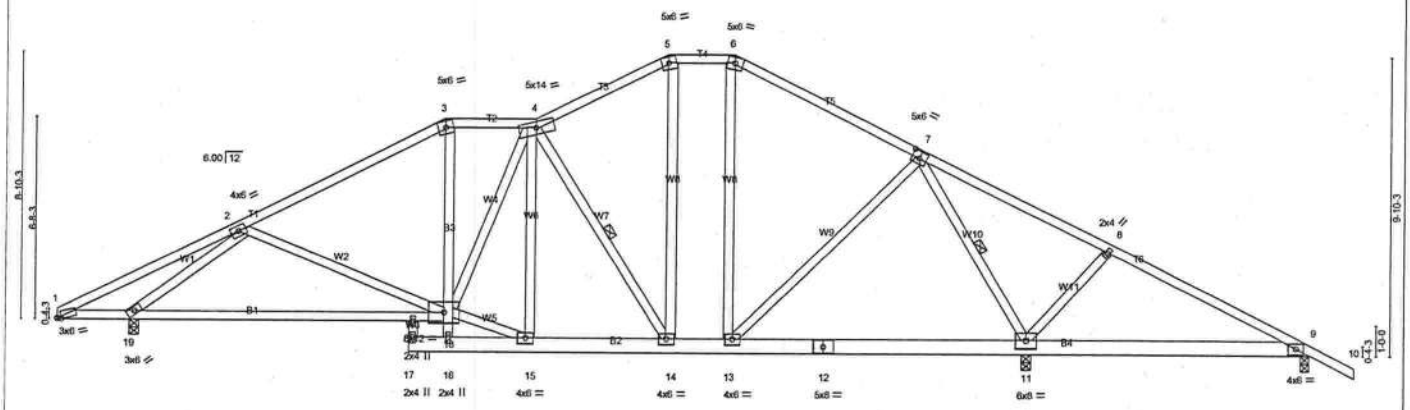
Dwg.#1026051070

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:26 2005 Page 1

6-0-4	11-6-0	12-8-0	15-8-1	20-0-0	22-2-0	28-3-13	34-5-11	41-2-0	42-8-0
6-0-4	5-5-12	1-2-0	3-0-1	4-3-15	2-2-0	6-1-13	6-1-13	6-8-5	1-6-0

Scale = 1:72.4



2-5-12	11-6-0	12-8-0	15-8-1	20-0-0	22-2-0	31-10-4	41-2-0
2-5-12	9-0-4	1-2-0	3-0-1	4-3-15	2-2-0	9-8-4	9-3-12

Plate Offsets (X,Y): [1:0-1-8,0-0-7], [7:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.14	9-11	>768	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	0.12	9-11	>888	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 280 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except* B1 2 X 4 SYP No.2, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-14, 7-11

REACTIONS (lb/size) 11=1806/0-3-8, 9=343/0-3-8, 19=1424/0-3-8
Max Horz 19=-254(load case 6)
Max Uplift11=-872(load case 6), 9=-373(load case 6), 19=-686(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-303/85, 2-3=-1566/836, 3-4=-1310/825, 4-5=-977/724, 5-6=-827/705, 6-7=-998/697, 7-8=-14/329, 8-9=-56/103, 9-10=0/39
BOT CHORD 1-19=-10/341, 18-19=-575/1252, 16-18=0/59, 3-18=-99/383, 16-17=0/0, 15-16=-231/0, 14-15=-380/1204, 13-14=-135/827, 12-13=-69/454, 11-12=-69/454, 9-11=-57/93
WEBS 2-18=-16/179, 15-18=-256/1446, 4-18=-46/282, 4-15=-230/28, 4-14=-758/515, 5-14=-238/318, 6-13=-56/215, 7-13=-92/529, 7-11=-1409/760, 8-11=-335/419, 2-19=-1640/1232

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 872 lb uplift at joint 11, 373 lb uplift at joint 9 and 686 lb uplift at joint 19.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

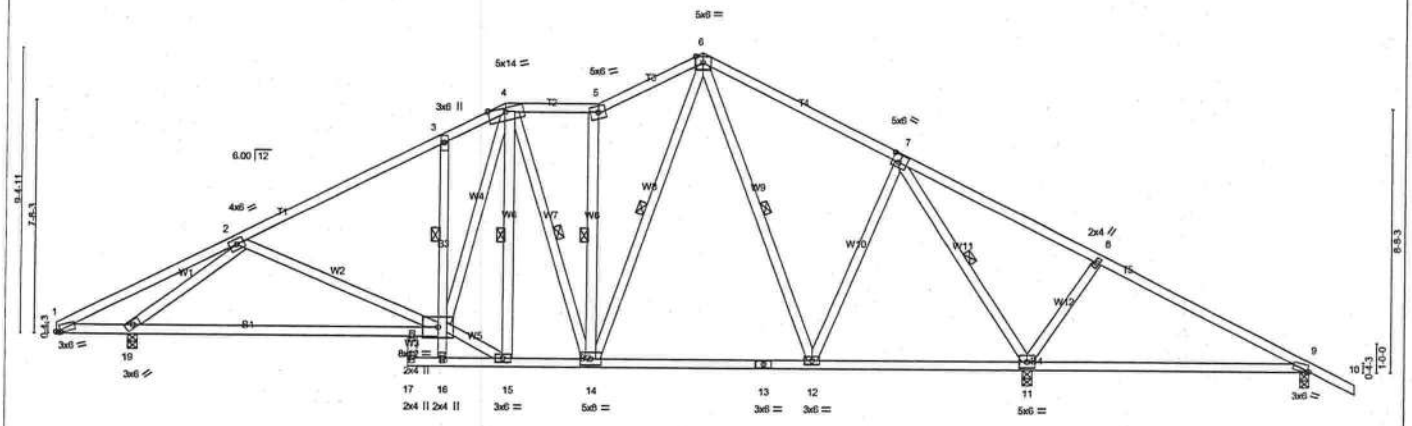
Dwg.#1026051071

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T13	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:27 2005 Page 1

6-0-4	11-6-0	14-8-0	17-8-1	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
6-0-4	5-5-12	3-2-0	3-0-1	3-4-15	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:72.4



14-8-0						
2-5-12	11-6-0	12-6-0	17-8-1	24-9-2	31-10-4	41-2-0
2-5-12	9-0-4	1-0-0 2-2-0	3-0-1	7-1-2	7-1-2	9-3-12

Plate Offsets (X,Y): [1:0-1-8,0-0-7], [7:0-3-0,0-3-0], [9:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.45	9-11	>243	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	0.38	9-11	>290	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 273 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 3-18
WEBS 1 Row at midpt 4-15, 4-14, 5-14, 6-14, 6-12, 7-11

REACTIONS (lb/size) 11=1892/0-3-8, 9=277/0-3-8, 19=1400/0-3-8
Max Horz 19=-261(load case 6)
Max Uplift11=-918(load case 6), 9=-345(load case 6), 19=-669(load case 5)
Max Grav 11=1892(load case 1), 9=331(load case 10), 19=1400(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-301/91, 2-3=-1523/808, 3-4=-1435/955, 4-5=-994/747, 5-6=-1131/880, 6-7=-798/622, 7-8=-106/477, 8-9=-98/274, 9-10=0/35
BOT CHORD 1-19=-14/339, 18-19=-562/1225, 16-18=0/64, 3-18=-239/341, 16-17=0/0, 15-16=-184/0, 14-15=-267/991, 13-14=-97/737, 12-13=-97/737, 11-12=-5/453, 9-11=-211/208
WEBS 2-18=0/186, 15-18=-167/1241, 4-18=-420/976, 4-15=-472/84, 4-14=-46/71, 5-14=-593/487, 6-14=-511/850, 6-12=-286/169, 7-12=-88/513, 7-11=-1535/789, 8-11=-373/466, 2-19=-1609/1204

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 918 lb uplift at joint 11, 345 lb uplift at joint 9 and 669 lb uplift at joint 19.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

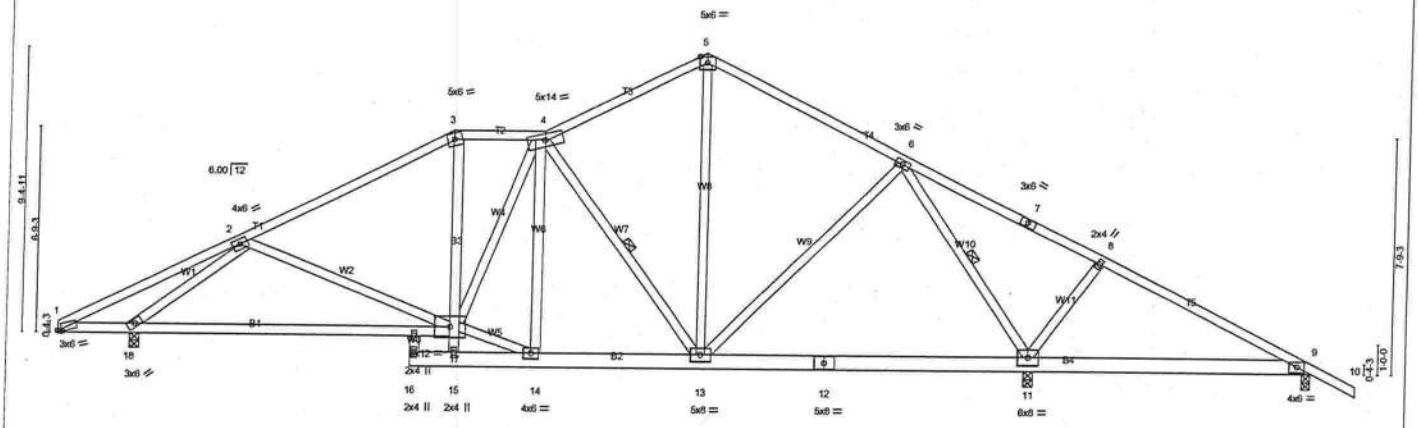
Dwg.#1026051072

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L735123	T14	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:28 2005 Page 1

6-0-4	11-6-0	12-10-0	15-10-1	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
6-0-4	5-5-12	1-4-0	3-0-1	5-2-15	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:72.4



2-5-12	11-6-0	12-10-0	15-10-1	23-10-2	31-10-4	41-2-0
2-5-12	9-0-4	1-4-0	3-0-1	8-0-2	8-0-2	9-3-12

Plate Offsets (X,Y): [1:0-1-8,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.14	9-11	>778	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	0.13	9-11	>877	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 270 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B1 2 X 4 SYP No.2, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-13, 6-11

REACTIONS (lb/size) 11=1903/0-3-8, 9=271/0-3-8, 18=1405/0-3-8
Max Horz 18=-263(load case 6)
Max Uplift11=-914(load case 6), 9=-348(load case 6), 18=-668(load case 5)
Max Grav 11=1903(load case 1), 9=329(load case 10), 18=1405(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-306/78, 2-3=-1527/801, 3-4=-1272/796, 4-5=-916/666, 5-6=-943/661, 6-7=-103/469, 7-8=-116/325, 8-9=-107/287, 9-10=0/39
BOT CHORD 1-18=-3/343, 17-18=-565/1231, 15-17=0/98, 3-17=-71/354, 15-16=0/0, 14-15=-239/0, 13-14=-344/1140, 12-13=-18/441, 11-12=-18/441, 9-11=-233/234
WEBS 2-17=-4/195, 14-17=-205/1367, 4-17=-73/364, 4-14=-360/77, 4-13=-657/484, 5-13=-253/433, 6-13=-105/510, 6-11=-1482/831, 8-11=-345/433, 2-18=-1602/1211

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 914 lb uplift at joint 11, 348 lb uplift at joint 9 and 668 lb uplift at joint 18.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

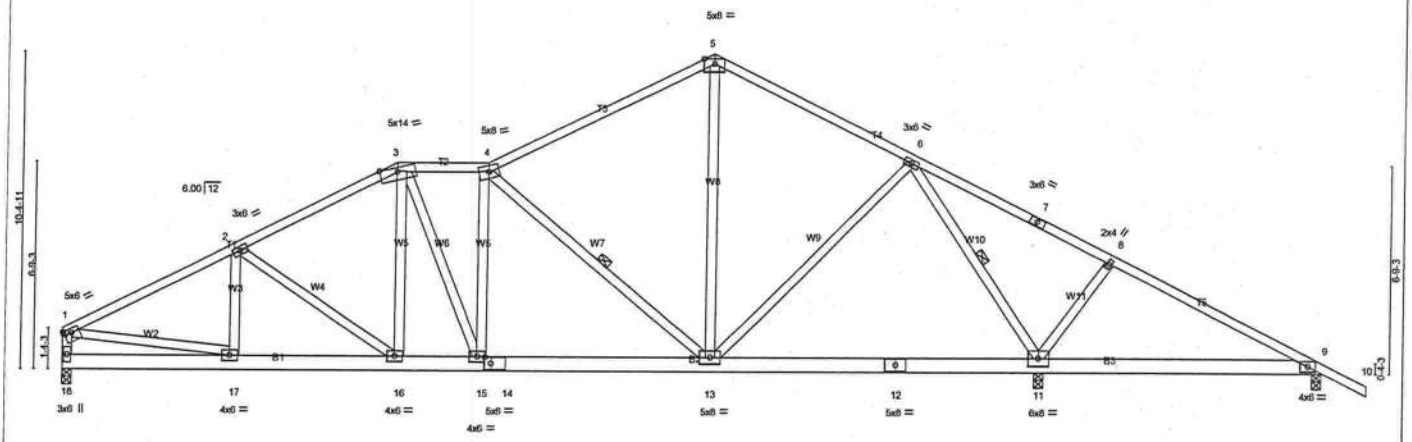
Dwg.#1026051073

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T15	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:28 2005 Page 1

5-6-12	10-10-0	13-10-1	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
5-6-12	5-3-4	3-0-1	7-2-15	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:72.0



5-6-12	10-10-0	13-10-1	21-1-0	31-10-4	41-2-0
5-6-12	5-3-4	3-0-1	7-2-15	10-9-4	9-3-12

Plate Offsets (X,Y): [1:Edge,0-1-12], [14:0-2-7,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.14	9-11	>781	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.12	9-11	>882	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.03	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 276 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-13, 6-11

REACTIONS (lb/size) 18=1264/0-3-8, 11=2012/0-3-8, 9=247/0-3-8
Max Horz 18=-239(load case 6)
Max Uplift18=-549(load case 5), 11=-964(load case 6), 9=-328(load case 6)
Max Grav 18=1264(load case 1), 11=2012(load case 1), 9=315(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1782/963, 2-3=-1613/956, 3-4=-1504/993, 4-5=-1033/702, 5-6=-1024/712, 6-7=-153/524, 7-8=-166/381, 8-9=-140/334, 9-10=0/39, 1-18=-1153/666
BOT CHORD 17-18=-274/293, 16-17=-700/1534, 15-16=-533/1387, 14-15=-580/1507, 13-14=-580/1507, 12-13=-28/460, 11-12=-28/460, 9-11=-275/280
WEBS 2-17=-115/173, 2-16=-207/216, 3-16=-111/258, 3-15=-176/287, 4-15=-141/173, 4-13=-885/648, 5-13=-241/456, 6-13=-164/591, 6-11=-1612/929, 8-11=-349/438, 1-17=-549/1263

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 549 lb uplift at joint 18, 964 lb uplift at joint 11 and 328 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

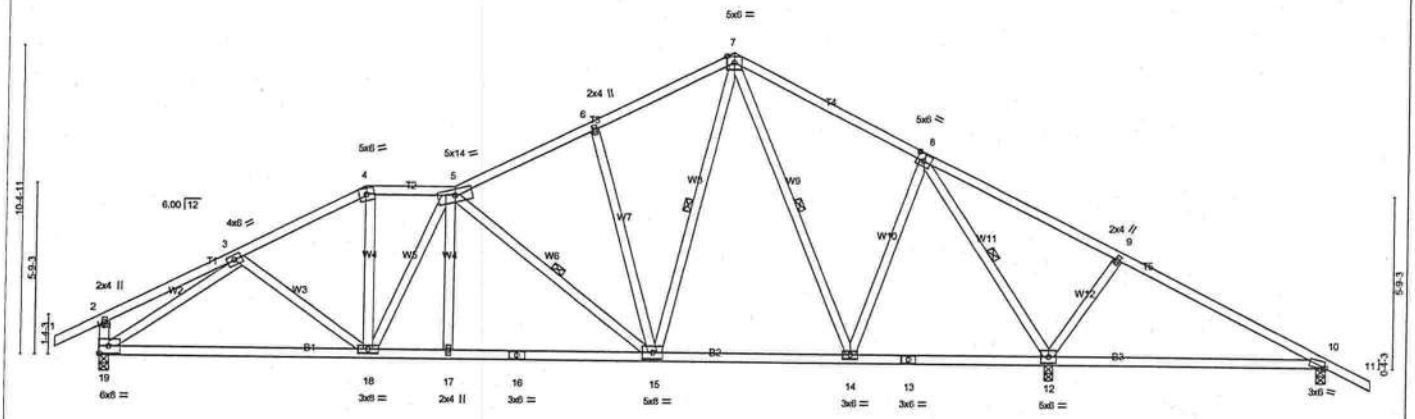
Dwg.#1026051074

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T16	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:29 2005 Page 1

1-6-0	4-6-12	8-10-0	11-10-1	16-5-8	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	4-6-12	4-3-4	3-0-1	4-7-8	4-7-8	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:74.0



8-10-0	11-10-1	18-6-2	25-2-3	31-10-4	41-2-0
8-10-0	3-0-1	6-8-1	6-8-1	6-8-1	9-3-12

Plate Offsets (X,Y): [8:0-3-0,0-3-0], [10:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	0.45	10-12	>246	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	0.37	10-12	>294	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.06	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 258 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-15, 7-15, 7-14, 8-12

REACTIONS (lb/size) 12=2020/0-3-8, 19=1353/0-3-8, 10=239/0-3-8
Max Horz 19=-210(load case 6)
Max Uplift12=-964(load case 6), 19=-661(load case 5), 10=-327(load case 6)
Max Grav 12=2020(load case 1), 19=1353(load case 1), 10=309(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-342/148, 3-4=-1676/956, 4-5=-1466/919, 5-6=-1267/817, 6-7=-1199/887, 7-8=-807/640, 8-9=-155/559, 9-10=-142/356, 10-11=0/35, 2-19=-351/326
BOT CHORD 18-19=-679/1384, 17-18=-659/1659, 16-17=-659/1657, 15-16=-659/1657, 14-15=-135/802, 13-14=-9/458, 12-13=-9/458, 10-12=-275/257
WEBS 3-18=-11/119, 4-18=-240/514, 5-18=-416/241, 5-17=0/103, 5-15=-734/483, 6-15=-247/273, 7-15=-558/946, 7-14=-414/217, 8-14=-133/589, 8-12=-1684/885, 9-12=-374/467, 3-19=-1423/860

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 964 lb uplift at joint 12, 661 lb uplift at joint 19 and 327 lb uplift at joint 10.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg #1026051075

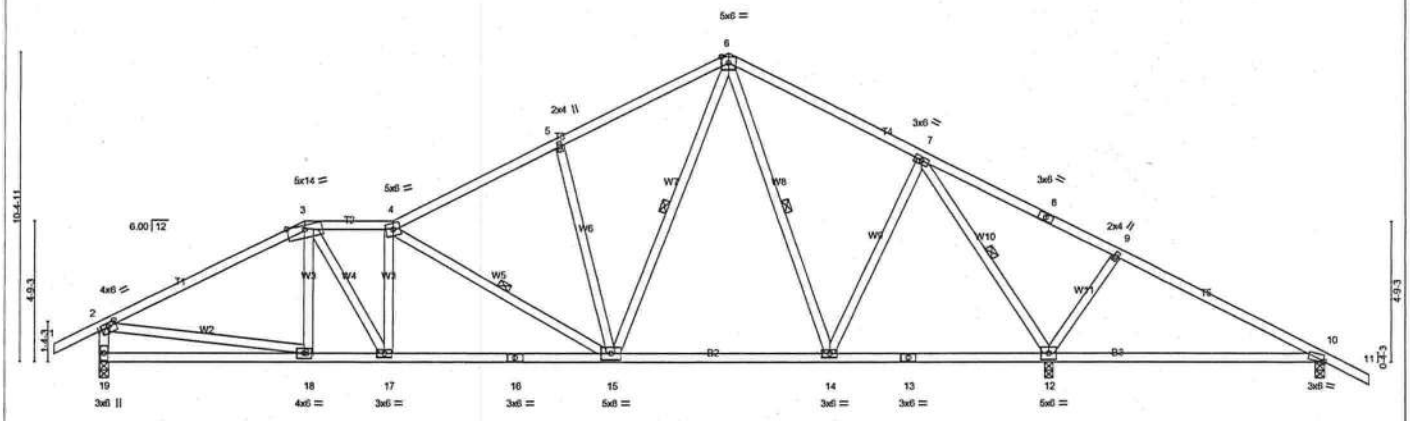
Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T17	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:30 2005 Page 1

1-6-0 | 6-10-0 | 9-10-1 | 15-5-8 | 21-1-0 | 27-7-3 | 34-1-5 | 41-2-0 | 42-8-0

1-6-0 | 6-10-0 | 3-0-1 | 5-7-8 | 5-7-8 | 6-6-3 | 6-6-3 | 7-0-11 | 1-6-0

Scale = 1:74.0



6-10-0 | 9-10-1 | 17-2-2 | 24-6-3 | 31-10-4 | 41-2-0

6-10-0 | 3-0-1 | 7-4-1 | 7-4-1 | 7-4-1 | 9-3-12

Plate Offsets (X,Y): [2:0-3-0,0-1-8], [10:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	0.45	10-12	>242	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	0.38	10-12	>287	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.05	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 248 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-15, 6-15, 6-14, 7-12

REACTIONS (lb/size) 19=1344/0-3-8, 12=2058/0-3-8, 10=209/0-3-8
Max Horz 19=-210(load case 6)
Max Uplift19=-659(load case 5), 12=-974(load case 6), 10=-319(load case 6)
Max Grav 19=1344(load case 1), 12=2058(load case 1), 10=290(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1753/944, 3-4=-1843/1098, 4-5=-1405/866, 5-6=-1334/965, 6-7=-823/642, 7-8=-159/622, 8-9=-181/478, 9-10=-158/419, 10-11=0/35, 2-19=-1227/833
BOT CHORD 18-19=-268/290, 17-18=-654/1486, 16-17=-782/1856, 15-16=-782/1856, 14-15=-133/783, 13-14=-3/421, 12-13=-3/421, 10-12=-328/280
WEBS 3-18=-77/113, 3-17=-324/663, 4-17=-404/308, 4-15=-764/492, 5-15=-307/357, 6-15=-610/992, 6-14=-375/219, 7-14=-144/607, 7-12=-1721/911, 9-12=-376/469, 2-18=-513/1214

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 659 lb uplift at joint 19, 974 lb uplift at joint 12 and 319 lb uplift at joint 10.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

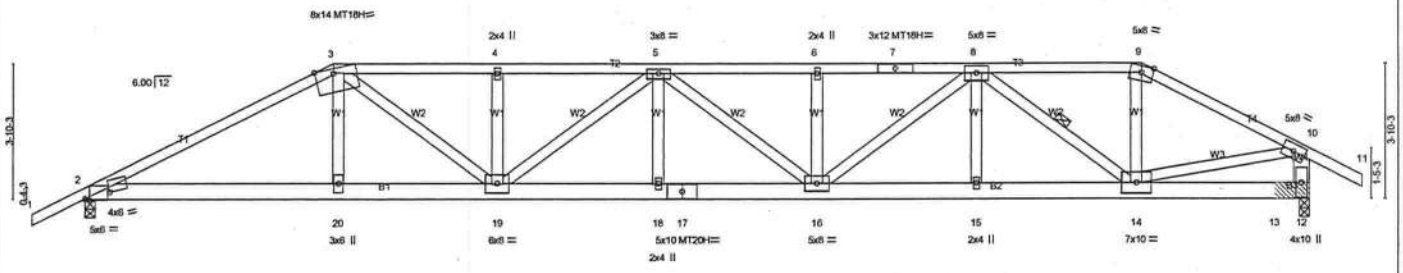
Dwg.#1026051076

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T18	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:31 2005 Page 1

-1-6-0	3-9-4	7-0-0	11-8-4	16-2-12	20-9-4	25-3-12	30-0-0	34-10-0	36-4-0
1-6-0	3-9-4	3-2-12	4-8-4	4-6-8	4-6-8	4-6-8	4-8-4	4-10-0	1-6-0

Scale = 1:62.7



3-9-4	7-0-0	11-8-4	16-2-12	20-9-4	25-3-12	30-0-0	34-10-0
3-9-4	3-2-12	4-8-4	4-6-8	4-6-8	4-6-8	4-8-4	4-10-0

Plate Offsets (X,Y): [2:0-8-10,0-0-7], [2:0-1-10,Edge], [3:0-6-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	0.56	16-18	>736	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.89	16-18	>464	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.74	Horz(TL)	0.17	12	n/a	n/a	MT18H	244/190
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 218 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 6 SYP No.1D, W3 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-9-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-1-3 oc bracing.
WEBS 1 Row at midpt 8-14

REACTIONS (lb/size) 2=2968/0-3-8, 12=3155/0-3-12 (0-3-8 + bearing block)
Max Horz 2=-241(load case 5)
Max Uplift2=-1564(load case 4), 12=-1636(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-5883/3187, 3-4=-6969/3936, 4-5=-6968/3937, 5-6=-7366/4173, 6-7=-7366/4173, 7-8=-7366/4173, 8-9=-3634/2132, 9-10=-4128/2241, 10-11=0/42, 10-12=-2918/1614
BOT CHORD 2-20=-2678/5179, 19-20=-2691/5214, 18-19=-4160/7733, 17-18=-4160/7733, 16-17=-4160/7733, 15-16=-3233/6063, 14-15=-3233/6063, 13-14=-268/477; 12-13=-268/477
WEBS 3-20=-321/838, 3-19=-1356/2280, 4-19=-518/529, 5-19=-991/587, 5-18=0/338, 5-16=-480/258, 6-16=-494/497, 8-16=-909/1655, 8-15=0/337, 8-14=-3088/1740, 9-14=-546/1325, 10-14=-1636/3193

NOTES
1) 2 X 6 SYP No.1D bearing block 12" long at jt. 12 attached to front face with 3 rows of 0.131"x3" Nails spaced 3" o.c. 12 Total fasteners. Bearing is assumed to be SYP.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1564 lb uplift at joint 2 and 1636 lb uplift at joint 12.
7) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 348 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-9=-113(F=-58), 9-10=-112(F=-58), 10-11=-54, 2-20=-30, 12-20=-62(F=-33)
Concentrated Loads (lb)
Vert: 20=-539(F)

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051077

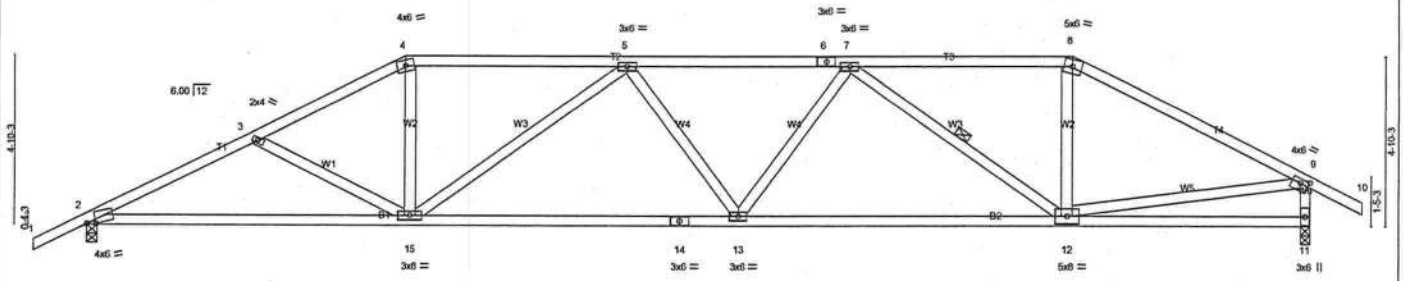
Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T19	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:32 2005 Page 1

-1-6-0 | 4-9-4 | 9-0-0 | 15-4-0 | 21-8-0 | 28-0-0 | 34-10-0 | 36-4-0

1-6-0 | 4-9-4 | 4-2-12 | 6-4-0 | 6-4-0 | 6-4-0 | 6-10-0 | 1-6-0

Scale = 1:62.7



9-0-0 | 18-6-0 | 28-0-0 | 34-10-0

9-0-0 | 9-6-0 | 9-6-0 | 6-10-0

Plate Offsets (X,Y): [2:0-3-0,0-0-11], [9:0-3-0,0-1-8]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.30	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.50	12-13	>836	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.12	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 181 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-9-8 oc bracing.
WEBS 1 Row at midpt 7-12

REACTIONS (lb/size) 2=1540/0-3-8, 11=1540/0-3-8
Max Horz 2=118(load case 5)
Max Uplift2=-650(load case 5), 11=-620(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2681/1409, 3-4=-2457/1276, 8-9=-2081/1066, 9-10=0/40, 9-11=-1444/909, 4-5=-2180/1216, 5-6=-2748/1477, 6-7=-2748/1477, 7-8=-1803/1044
BOT CHORD 2-15=-1100/2343, 14-15=-1194/2715, 13-14=-1194/2715, 12-13=-1127/2582, 11-12=-85/217
WEBS 3-15=-212/258, 4-15=-290/779, 5-15=-749/451, 5-13=0/109, 7-13=-25/315, 7-12=-1035/549, 8-12=-161/598, 9-12=-646/1592

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 650 lb uplift at joint 2 and 620 lb uplift at joint 11.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

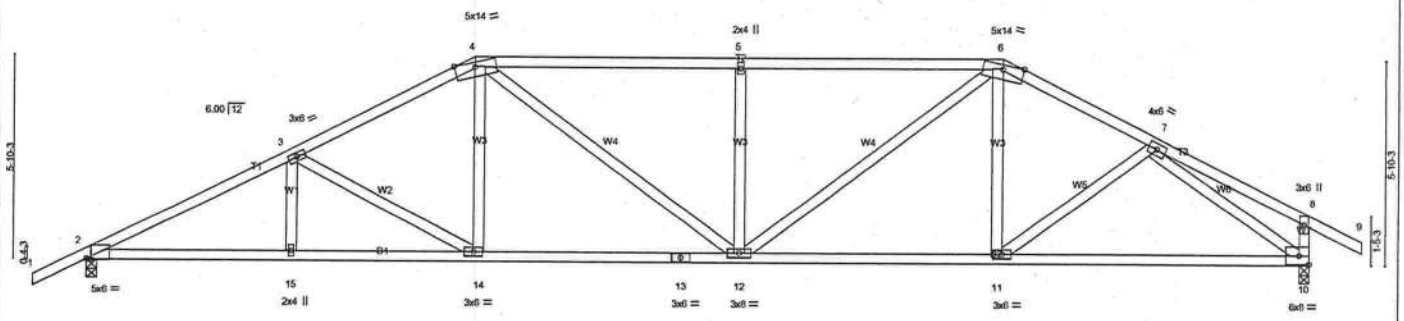
Dwg.#1026051078

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T20	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:33 2005 Page 1

-1-6-0	5-9-4	11-0-0	18-6-0	26-0-0	30-3-4	34-10-0	36-4-0
1-6-0	5-9-4	5-2-12	7-6-0	7-6-0	4-3-4	4-6-12	1-6-0

Scale = 1:62.7



5-9-4	11-0-0	18-6-0	26-0-0	34-10-0
5-9-4	5-2-12	7-6-0	7-6-0	8-10-0

Plate Offsets (X,Y): [2:0-1-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.21	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.35	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.11	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 192 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-1-5 oc bracing.

REACTIONS (lb/size) 2=1540/0-3-8, 10=1540/0-3-8
Max Horz 2=135(load case 5)
Max Uplift2=-672(load case 5), 10=-644(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2733/1390, 3-4=-2270/1241, 4-5=-2303/1341, 5-6=-2303/1341, 6-7=-1994/1106, 7-8=-336/172, 8-9=0/40, 8-10=-358/345
BOT CHORD 2-15=-1073/2365, 14-15=-1073/2365, 13-14=-791/1990, 12-13=-791/1990, 11-12=-662/1754, 10-11=-695/1582
WEBS 3-15=0/165, 3-14=-442/325, 4-14=-119/410, 4-12=-291/513, 5-12=-429/375, 6-12=-377/762, 6-11=-10/135, 7-11=-133/317, 7-10=-1700/954

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 672 lb uplift at joint 2 and 644 lb uplift at joint 10.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

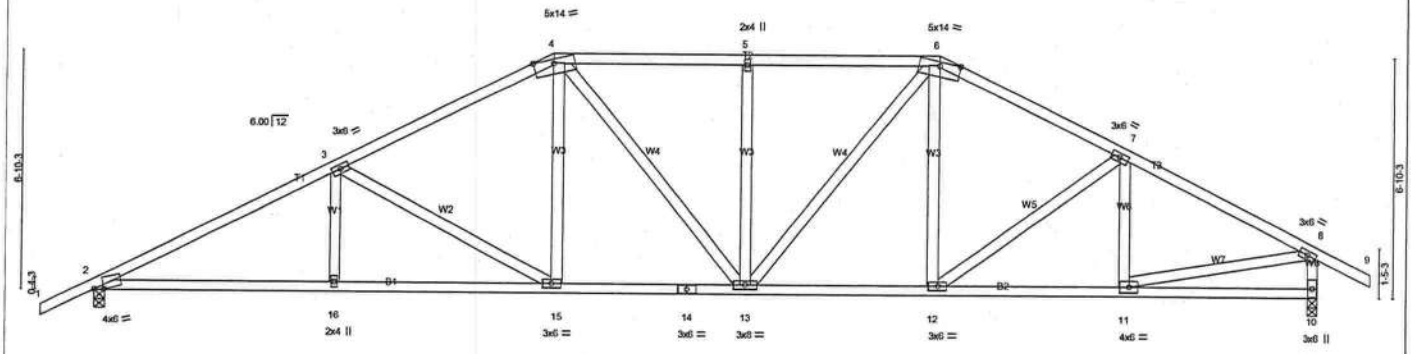
Dwg.#1026051079

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T21	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:34 2005 Page 1

-1-6-0	6-9-4	13-0-0	18-6-0	24-0-0	29-3-4	34-10-0	36-4-0
1-6-0	6-9-4	6-2-12	5-6-0	5-6-0	5-3-4	5-6-12	1-6-0

Scale = 1:62.7



6-9-4	13-0-0	18-6-0	24-0-0	29-3-4	34-10-0
6-9-4	6-2-12	5-6-0	5-6-0	5-3-4	5-6-12

Plate Offsets (X,Y): [2:0-3-4,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.16	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.27	15-16	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.10	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 204 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-8 oc bracing.

REACTIONS (lb/size) 2=1540/0-3-8, 10=1540/0-3-8
Max Horz 2=151(load case 5)
Max Uplift2=-691(load case 5), 10=-665(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2705/1394, 3-4=-2097/1185, 4-5=-1920/1187, 5-6=-1920/1187, 6-7=-1911/1107, 7-8=-1990/1068, 8-9=0/40, 8-10=-1448/924
BOT CHORD 2-16=-1066/2337, 15-16=-1066/2337, 14-15=-687/1814, 13-14=-687/1814, 12-13=-606/1659, 11-12=-730/1717, 10-11=-34/162
WEBS 3-16=0/220, 3-15=-608/435, 4-15=-172/462, 4-13=-189/307, 5-13=-303/269, 6-13=-244/505, 6-12=-78/233, 7-12=-109/183, 7-11=-196/203, 8-11=-742/1592

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 691 lb uplift at joint 2 and 665 lb uplift at joint 10.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051080

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T22	HIP	1	1	Job Reference (optional)

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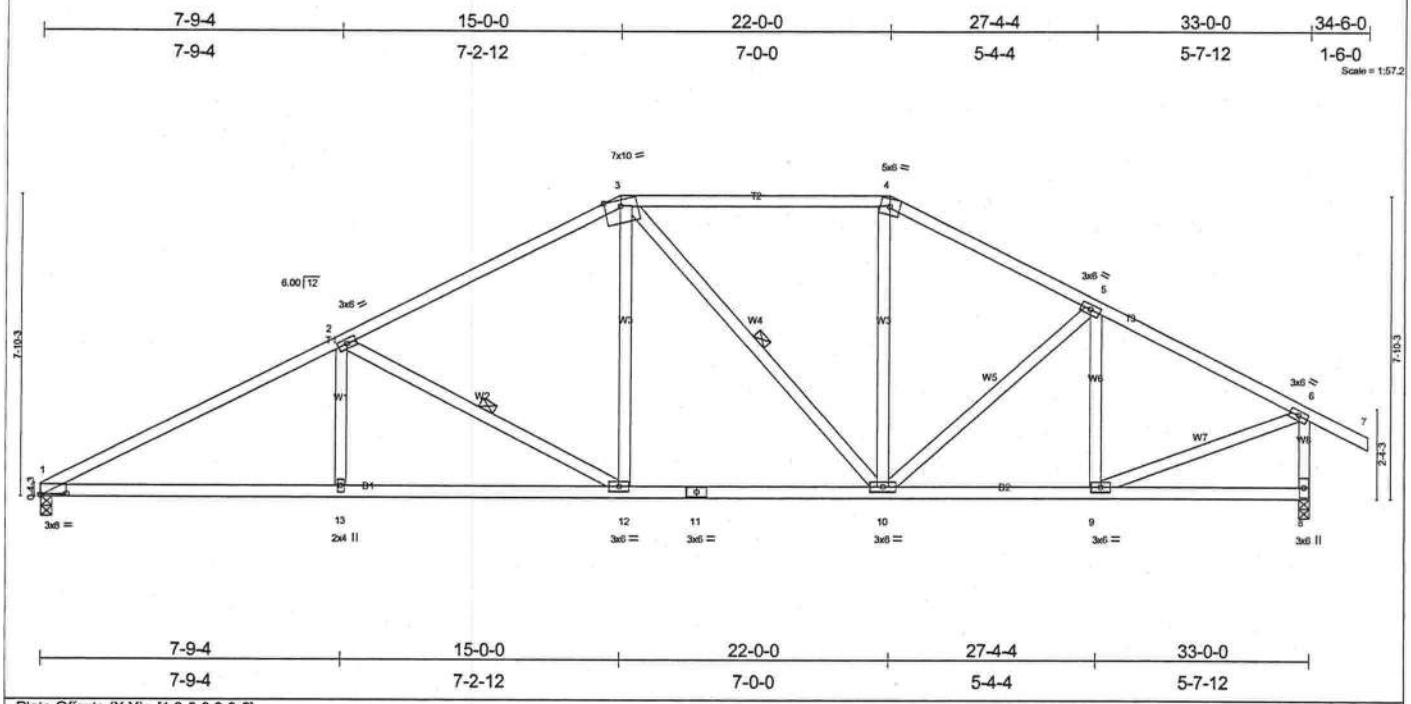


Plate Offsets (X,Y): [1:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.20	1-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.32	1-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 187 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-9-5 oc bracing.
WEBS 1 Row at midpt 2-12, 3-10

REACTIONS (lb/size) 1=1372/0-3-8, 8=1465/0-3-8
Max Horz 1=181(load case 4)
Max Uplift1=-566(load case 5), 8=-640(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2511/1342, 2-3=-1777/1055, 3-4=-1356/950, 4-5=-1568/977, 5-6=-1539/883, 6-7=0/40, 6-8=-1378/897
BOT CHORD 1-13=-1025/2160, 12-13=-1025/2160, 11-12=-537/1518, 10-11=-537/1518, 9-10=-523/1314, 8-9=-18/77
WEBS 2-13=0/267, 2-12=-740/558, 3-12=-211/552, 3-10=-346/175, 4-10=-135/335, 5-10=-98/168, 5-9=-322/252, 6-9=-597/1325

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 566 lb uplift at joint 1 and 640 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

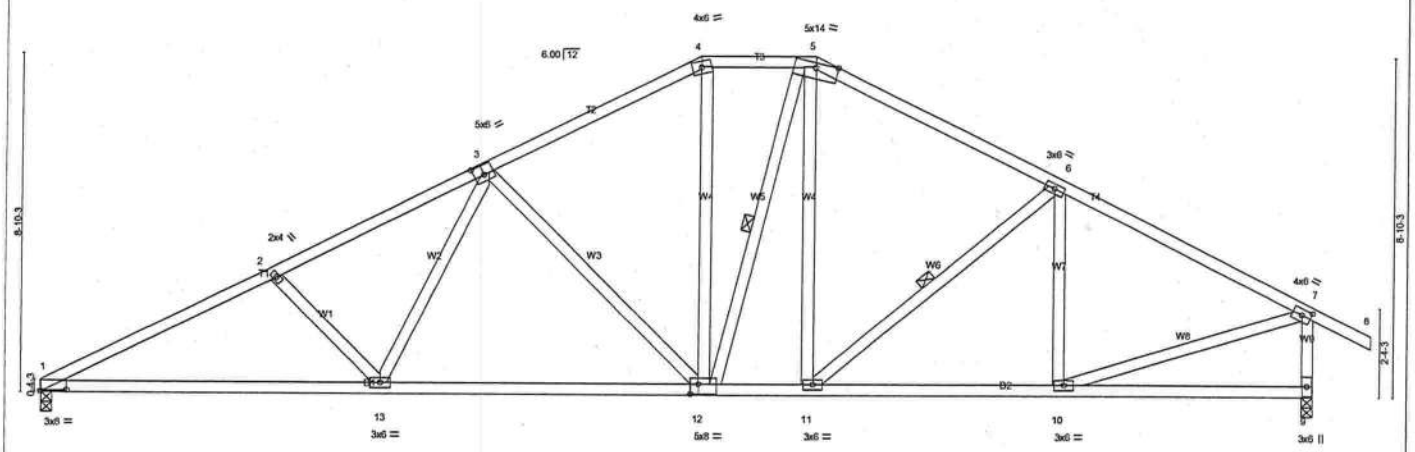
Dwg.#1026051081

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T23	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:35 2005 Page 1

6-0-5	11-6-3	17-0-0	20-0-0	26-4-4	33-0-0	34-6-0
6-0-5	5-5-13	5-5-13	3-0-0	6-4-4	6-7-12	1-6-0

Scale = 1:57.2



8-9-4	17-0-0	20-0-0	26-4-4	33-0-0
8-9-4	8-2-12	3-0-0	6-4-4	6-7-12

Plate Offsets (X,Y): [1:0-8-0,0-0-6], [3:0-3-0,0-3-0], [7:0-2-15,0-2-0], [12:0-2-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.20	1-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.34	1-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.08	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 202 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-9-10 oc bracing.
WEBS 1 Row at midpt 5-12, 6-11

REACTIONS (lb/size) 1=1372/0-3-8, 9=1465/0-3-8
Max Horz 1=198(load case 4)
Max Uplift1=-601(load case 5), 9=-657(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2520/1409, 2-3=-2309/1330, 3-4=-1560/1016, 4-5=-1332/978, 5-6=-1499/974, 6-7=-1620/934, 7-8=0/40, 7-9=-1368/908
BOT CHORD 1-13=-1111/2200, 12-13=-768/1768, 11-12=-389/1270, 10-11=-555/1379, 9-10=-13/102
WEBS 2-13=-291/352, 3-13=-236/551, 3-12=-634/513, 4-12=-259/452, 5-12=-151/328, 5-11=-130/267, 6-11=-241/236, 6-10=-213/233, 7-10=-598/1343

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 601 lb uplift at joint 1 and 657 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051082

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T24	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:36 2005 Page 1

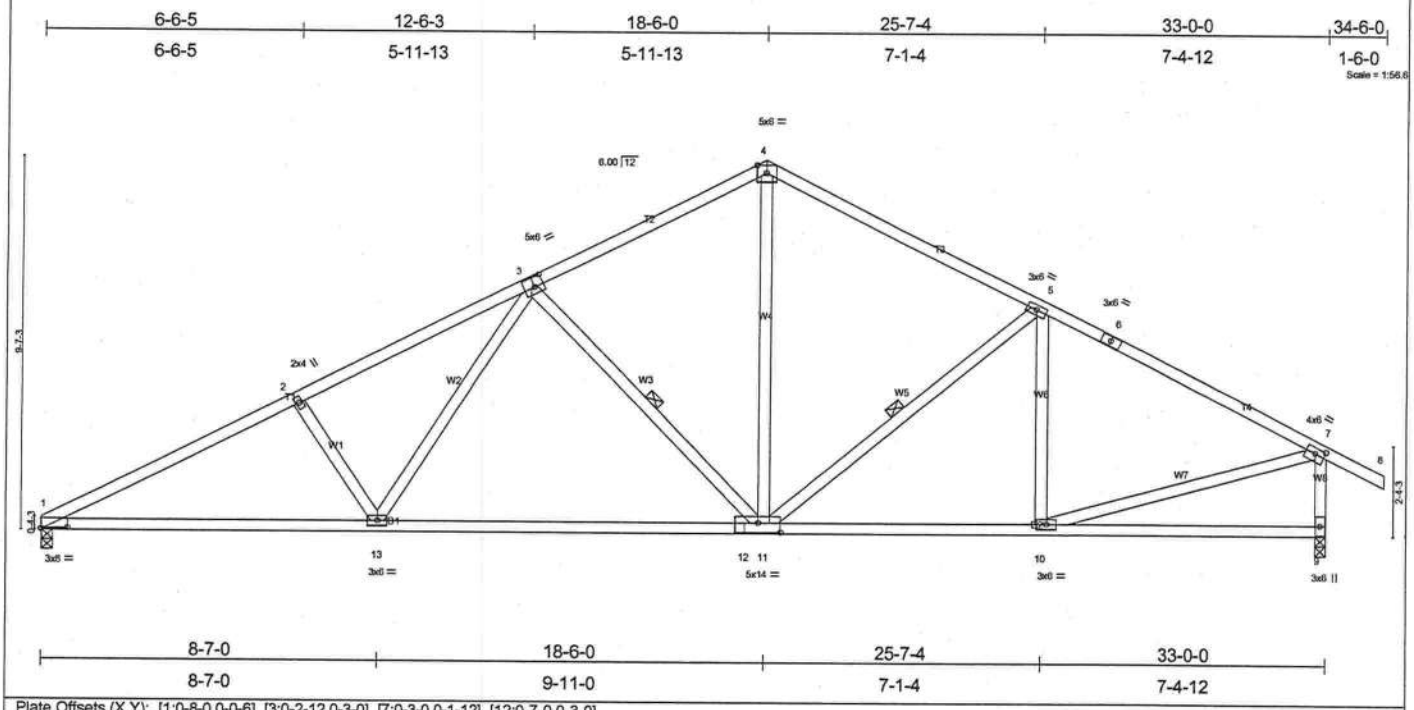


Plate Offsets (X,Y): [1:0-8-0,0-0-6], [3:0-2-12,0-3-0], [7:0-3-0,0-1-12], [12:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.26	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.44	11-13	>886	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(TL)	0.08	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 183 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-13 oc bracing.
WEBS 1 Row at midpt 3-11, 5-11

REACTIONS (lb/size) 1=1372/0-3-8, 9=1465/0-3-8
Max Horz 1=211(load case 5)
Max Uplift1=-585(load case 5), 9=-670(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2520/1398, 2-3=-2346/1379, 3-4=-1442/968, 4-5=-1463/960, 5-6=-1494/947, 6-7=-1651/933, 7-8=0/40, 7-9=-1350/921
BOT CHORD 1-13=-1188/2195, 12-13=-807/1676, 11-12=-807/1676, 10-11=-648/1399, 9-10=-55/141
WEBS 2-13=-304/381, 3-13=-324/666, 3-11=-669/537, 4-11=-506/868, 5-11=-303/287, 5-10=-171/219, 7-10=-617/1310

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 585 lb uplift at joint 1 and 670 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

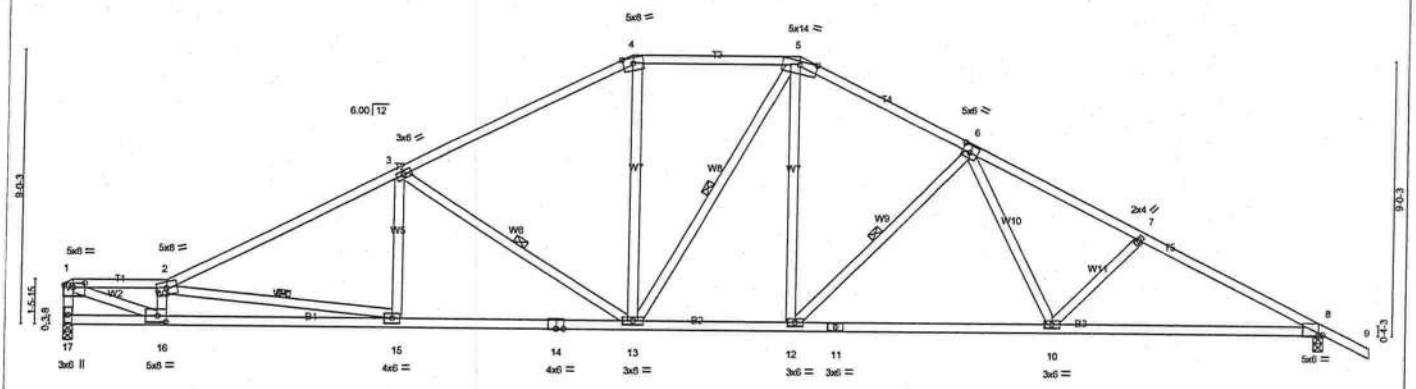
Dwg.#1026051083

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T25	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:37 2005 Page 1

0-3-8	3-3-9	10-9-12	18-4-0	23-10-0	29-5-3	35-0-5	41-2-0	42-8-0
0-3-8	3-0-1	7-6-4	7-6-4	5-6-0	5-7-3	5-7-3	6-1-11	1-6-0

Scale = 1:72.0



3-3-9	10-9-12	18-4-0	23-10-0	32-2-12	41-2-0
3-3-9	7-6-4	7-6-4	5-6-0	8-4-12	8-11-4

Plate Offsets (X,Y): [1:0-4-8,0-1-12], [6:0-3-0,0-3-0], [8:0-1-11,Edge], [16:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.35	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.57	15-16	>866	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.16	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 236 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D *Except*
B2 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-5-11 oc bracing.
WEBS 1 Row at midpt 2-15, 3-13, 5-13, 6-12

REACTIONS (lb/size) 17=1715/0-3-8, 8=1807/0-3-8
Max Horz 17=-246(load case 6)
Max Uplift17=-560(load case 6), 8=-812(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4004/2182, 2-3=-3243/1755, 3-4=-2343/1393, 4-5=-2018/1343, 5-6=-2278/1388, 6-7=-3030/1682, 7-8=-3239/1756, 8-9=0/35, 1-17=-1616/867
BOT CHORD 16-17=-109/191, 15-16=-2155/4182, 14-15=-1314/2833, 13-14=-1314/2833, 12-13=-720/1986, 11-12=-1068/2417, 10-11=-1068/2417, 8-10=-1386/2833
WEBS 1-16=-2263/4124, 2-16=-1468/950, 2-15=-1366/851, 3-15=-99/488, 3-13=-983/682, 4-13=-288/656, 5-13=-156/242, 5-12=-325/654, 6-12=-633/506, 6-10=-206/544, 7-10=-278/334

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 560 lb uplift at joint 17 and 812 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051084

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T26	SPECIAL	1	1	

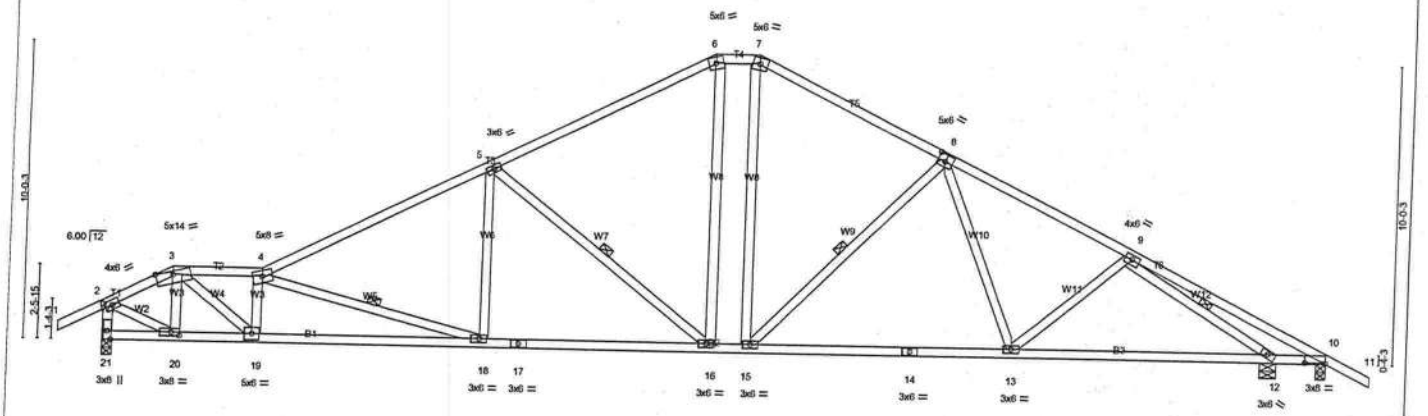
Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:38 2005 Page 1

1-6-0 2-3-8 5-3-9 12-9-12 20-4-0 21-10-0 28-1-3 34-4-5 41-2-0 42-8-0

1-6-0 2-3-8 3-0-1 7-6-4 7-6-4 1-6-0 6-3-3 6-3-3 6-9-11 1-6-0

Scale = 1:74.0



2-3-8 5-3-9 12-9-12 20-4-0 21-10-0 30-6-6 39-2-12 41-2-0

2-3-8 3-0-1 7-6-4 7-6-4 1-6-0 8-8-6 8-8-6 1-11-4

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [8:0-3-0,0-3-0], [10:0-8-0,0-0-6], [20:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.31	16-18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.49	16-18	>959	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.74	Horz(TL)	0.14	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 251 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-9-3 oc bracing.
WEBS 1 Row at midpt 4-18, 5-16, 8-15, 9-12

REACTIONS (lb/size) 21=1739/0-3-8, 12=1760/0-6-7, 10=113/0-3-8
Max Horz 21=-204(load case 6)
Max Uplift21=-814(load case 5), 12=-627(load case 6), 10=-269(load case 6)
Max Grav 21=1739(load case 1), 12=1760(load case 1), 10=118(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1763/976, 3-4=-3327/1878, 4-5=-2818/1590, 5-6=-1970/1251, 6-7=-1682/1217, 7-8=-1948/1262, 8-9=-2450/1437, 9-10=-242/163, 10-11=0/35, 2-21=-1682/1052
BOT CHORD 20-21=-139/209, 19-20=-688/1488, 18-19=-1733/3410, 17-18=-1103/2449, 16-17=-1103/2449, 15-16=-560/1682, 14-15=-877/2032, 13-14=-877/2032, 12-13=-1076/2109, 10-12=-1/138
WEBS 3-20=-687/349, 3-19=-1294/2318, 4-19=-1256/851, 4-18=-1001/656, 5-18=-149/547, 5-16=-1000/694, 6-16=-328/609, 7-15=-376/635, 8-15=-574/463, 8-13=-96/315, 9-13=-27/230, 9-12=-2382/1347, 2-20=-829/1643

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 814 lb uplift at joint 21, 627 lb uplift at joint 12 and 269 lb uplift at joint 10.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051085

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T27	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:39 2005 Page 1

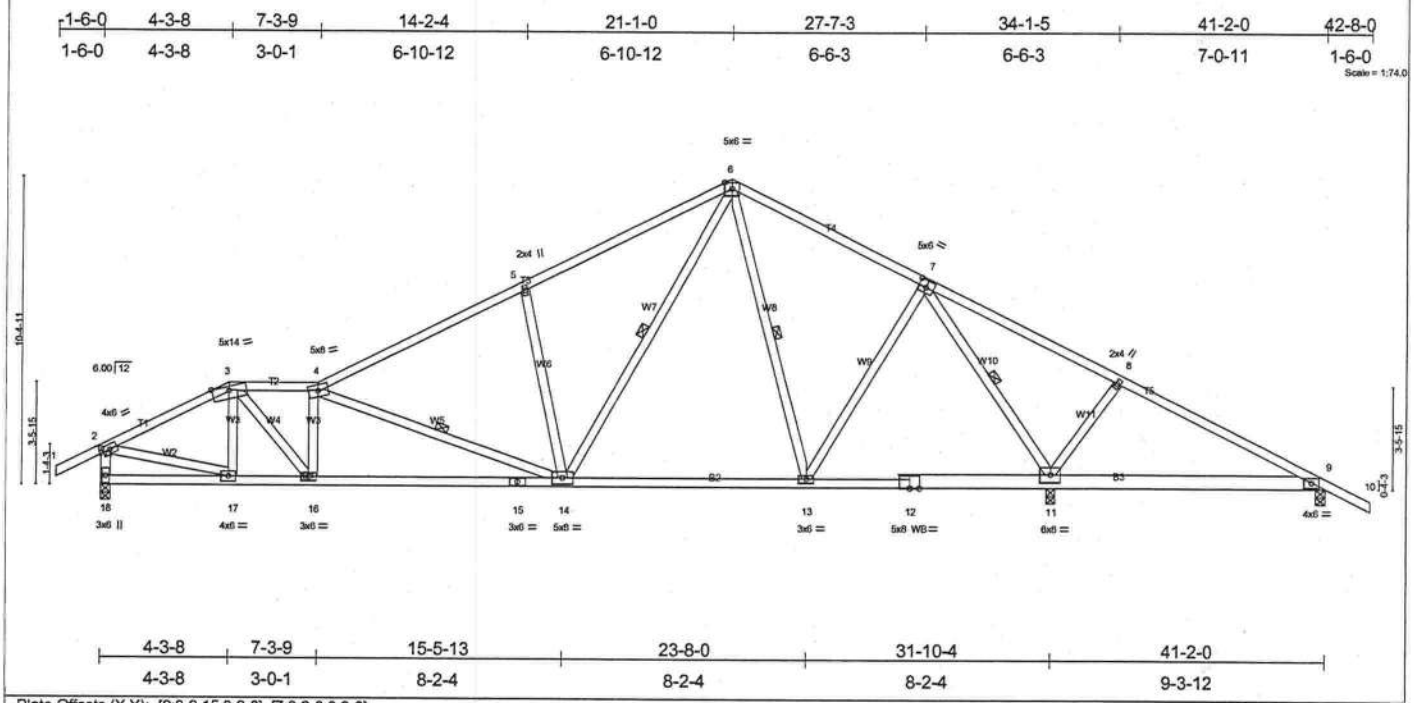


Plate Offsets (X,Y): [2:0-2-15,0-2-0], [7:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.14	9-11	>789	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	0.12	9-11	>918	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 251 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* B3 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-14, 6-14, 6-13, 7-11

REACTIONS (lb/size) 18=1332/0-3-8, 11=2113/0-3-8, 9=166/0-3-8
Max Horz 18=-212(load case 6)
Max Uplift18=-651(load case 5), 11=-995(load case 6), 9=-303(load case 6)
Max Grav 18=1332(load case 1), 11=2113(load case 1), 9=268(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1610/869, 3-4=-2127/1211, 4-5=-1585/920, 5-6=-1513/1062, 6-7=-836/636, 7-8=-266/715, 8-9=-234/510, 9-10=0/39, 2-18=-1247/817
BOT CHORD 17-18=-166/215, 16-17=-637/1371, 15-16=-968/2157, 14-15=-968/2157, 13-14=-121/761, 12-13=0/374, 11-12=0/376, 9-11=-426/370
WEBS 3-17=-283/164, 3-16=-596/1133, 4-16=-648/493, 4-14=-865/557, 5-14=-377/449, 6-14=-684/1077, 6-13=-342/233, 7-13=-174/633, 7-11=-1771/987, 8-11=-349/436, 2-17=-613/1305

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 651 lb uplift at joint 18, 995 lb uplift at joint 11 and 303 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051086

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T28	SPECIAL	1	1	

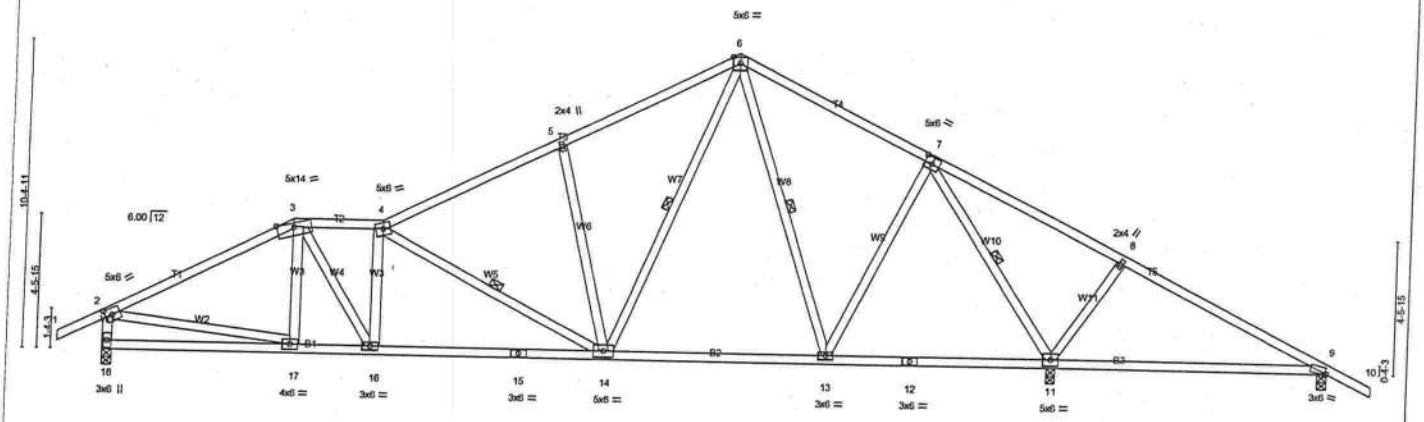
Builders FirstSource, Lake City, Fl 32055

Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:39 2005 Page 1

1-6-0	6-3-8	9-3-9	15-2-4	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	6-3-8	3-0-1	5-10-12	5-10-12	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:74.0



6-3-8	9-3-9	16-9-13	24-4-0	31-10-4	41-2-0
6-3-8	3-0-1	7-6-4	7-6-4	7-6-4	9-3-12

Plate Offsets (X,Y): [2:0-2-11,0-2-8], [7:0-3-0,0-3-0], [9:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.46	9-11	>241	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	0.39	9-11	>286	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 246 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-14, 6-14, 6-13, 7-11

REACTIONS (lb/size) 18=1342/0-3-8, 11=2068/0-3-8, 9=202/0-3-8
Max Horz 18=-210(load case 6)
Max Uplift18=-658(load case 5), 11=-976(load case 6), 9=-318(load case 6)
Max Grav 18=1342(load case 1), 11=2068(load case 1), 9=286(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1737/937, 3-4=-1900/1121, 4-5=-1444/880, 5-6=-1372/987, 6-7=-827/642, 7-8=-189/637, 8-9=-165/435, 9-10=0/35, 2-18=-1232/830
BOT CHORD 17-18=-242/248, 16-17=-657/1475, 15-16=-818/1916, 14-15=-818/1916, 13-14=-131/779, 12-13=0/411, 11-12=0/411, 9-11=-342/287
WEBS 3-17=-117/121, 3-16=-378/754, 4-16=-458/351, 4-14=-779/502, 5-14=-322/376, 6-14=-625/1007, 6-13=-365/221, 7-13=-149/613, 7-11=-1730/919, 8-11=-376/470, 2-17=-546/1251

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 658 lb uplift at joint 18, 976 lb uplift at joint 11 and 318 lb uplift at joint 9.

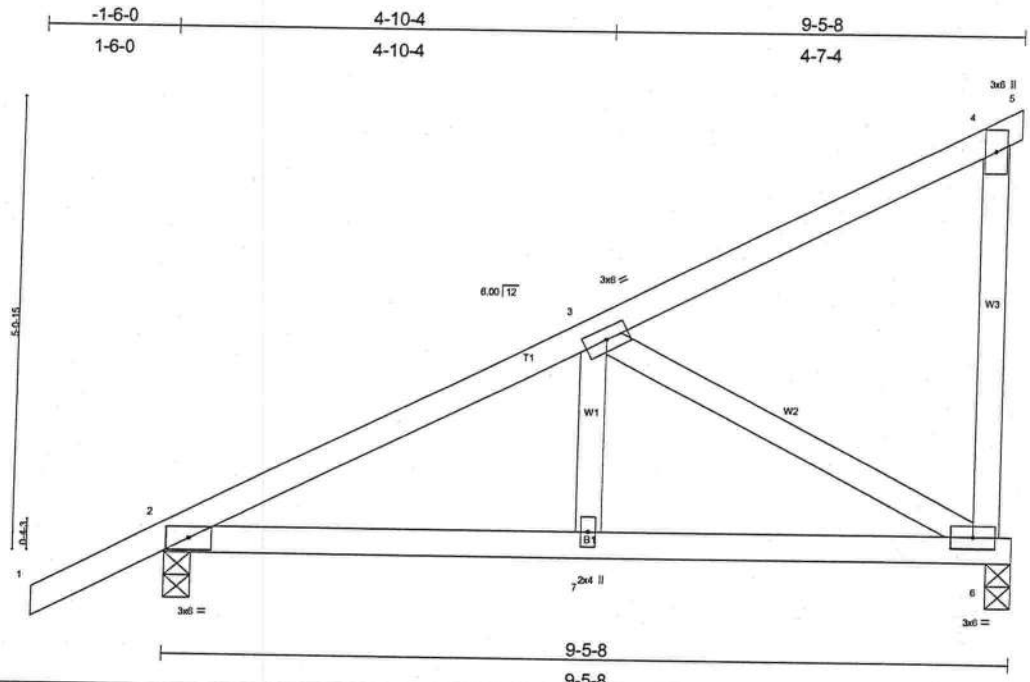
LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051087

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T29	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:40 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	0.04	2-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	0.04	2-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.20	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-3-6 oc bracing.

REACTIONS (lb/size) 6=375/0-3-8, 2=476/0-3-8
Max Horz 2=315(load case 5)
Max Uplift6=-385(load case 5), 2=-374(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-512/540, 3-4=-81/27, 4-5=-2/0, 4-6=-104/147
BOT CHORD 2-7=-723/407, 6-7=-723/407
WEBS 3-6=-439/781, 3-7=-324/147

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 385 lb uplift at joint 6 and 374 lb uplift at joint 2.

LOAD CASE(S) Standard

OCTOBER 26,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1026051088

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T29G	MONO TRUSS	1	1	

Builders FirstSource, Lake City, Fl 32055

Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 15:26:41 2005 Page 1

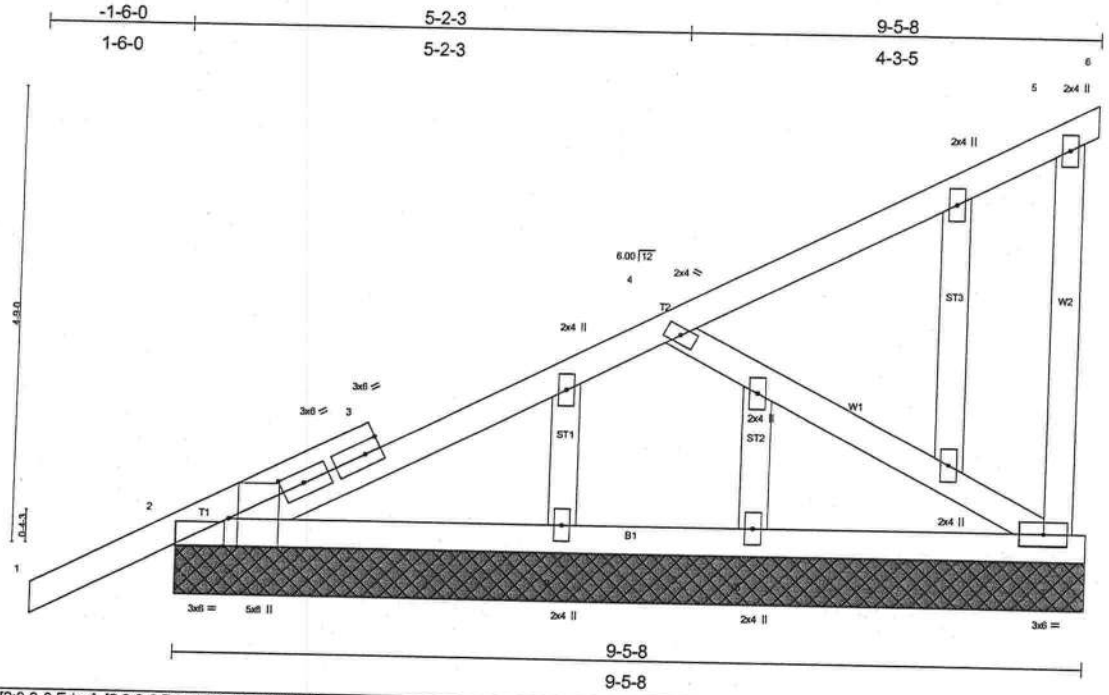


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [3:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.02	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	0.03	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.18	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 56 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 9-0-2 oc bracing.

REACTIONS (lb/size) 2=522/9-5-8, 6=-161/9-5-8, 7=582/9-5-8, 9=147/9-5-8, 8=52/9-5-8
Max Horz 2=298(load case 5)
Max Uplift2=-313(load case 5), 6=-161(load case 1), 7=-486(load case 5), 9=-12(load case 5)
Max Grav 2=522(load case 1), 6=161(load case 5), 7=582(load case 1), 9=147(load case 1), 8=52(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/51, 2-3=-506/240, 3-4=-450/238, 4-5=-92/54, 5-6=-82/100, 5-7=-323/433
BOT CHORD 2-9=-462/402, 8-9=-462/402, 7-8=-462/402
WEBS 4-7=-447/515

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Gable requires continuous bottom chord bearing.
4) Gable studs spaced at 2-0-0 oc.
5) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 2, 161 lb uplift at joint 6, 486 lb uplift at joint 7 and 12 lb uplift at joint 9.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-79(F=-25), 5-6=-79(F=-25), 2-7=-30

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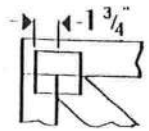
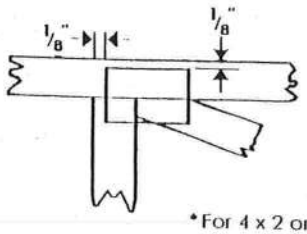
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

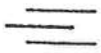
PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

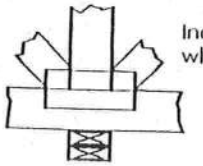
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

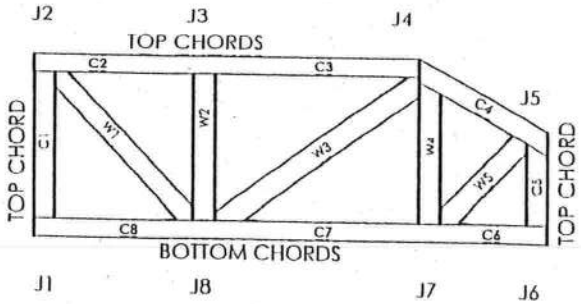
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

Residential System Sizing Calculation

Summary

EWPL INC
209 SW Paisley Ct.
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

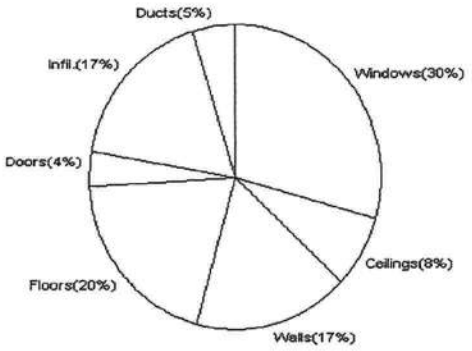
9/20/2005

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**32409**	**Btuh**	**Total cooling load calculation**	**31653**	**Btuh**
Submitted heating capacity	36000	Btuh	Submitted cooling capacity	36000	Btuh
Submitted as % of calculated	111.1	%	Submitted as % of calculated	113.7	%

WINTER CALCULATIONS

Winter Heating Load (for 1932 sqft)

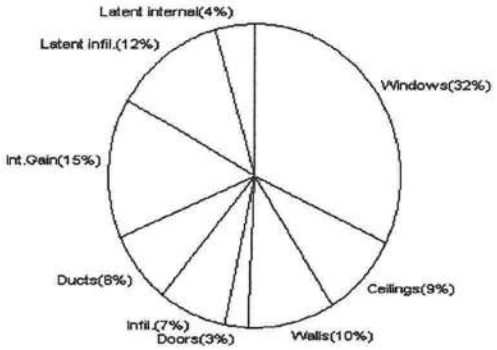
Load component			Load	
Window total	339	sqft	9594	Btuh
Wall total	1892	sqft	5517	Btuh
Door total	80	sqft	1260	Btuh
Ceiling total	1932	sqft	2512	Btuh
Floor total	204	ft	6446	Btuh
Infiltration	129	cfm	5537	Btuh
Subtotal			**30866**	**Btuh**
Duct loss			1543	Btuh
TOTAL HEAT LOSS			**32409**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1932 sqft)

Load component			Load	
Window total	339	sqft	10254	Btuh
Wall total	1892	sqft	3130	Btuh
Door total	80	sqft	798	Btuh
Ceiling total	1932	sqft	2743	Btuh
Floor total			0	Btuh
Infiltration	113	cfm	2236	Btuh
Internal gain			4800	Btuh
Subtotal(sensible)			**23961**	**Btuh**
Duct gain			2396	Btuh
Total sensible gain			**26357**	**Btuh**
Latent gain(infiltration)			3916	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**5296**	**Btuh**
TOTAL HEAT GAIN			**31653**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY:
DATE: 9-20-05

Manual J Winter Calculations

Residential Load - Component Details (continued)

EWPL INC
209 SW Paisley Ct.
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

9/20/2005

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Winter

Residential Load - Component Details

EWPL INC
209 SW Paisley Ct.
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

9/20/2005

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	84.0	28.3	2377 Btuh
2	2, Clear, Metal, DEF	N	12.5	28.3	354 Btuh
3	2, Clear, Metal, DEF	E	30.0	28.3	849 Btuh
4	2, Clear, Metal, DEF	S	30.0	28.3	849 Btuh
5	2, Clear, Metal, DEF	SW	21.0	28.3	594 Btuh
6	2, Clear, Metal, DEF	S	70.0	28.3	1981 Btuh
7	2, Clear, Metal, DEF	N	16.0	28.3	453 Btuh
8	2, Clear, Metal, DEF	W	21.0	28.3	594 Btuh
9	2, Clear, Metal, DEF	N	12.5	28.3	354 Btuh
10	2, Clear, Metal, DEF	S	42.0	28.3	1189 Btuh
	Window Total		339		9594 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Adjacent	13.0	232	1.6	371 Btuh
2	Frame - Exterior	13.0	1660	3.1	5146 Btuh
	Wall Total		1892		5517 Btuh
Doors	Type		Area X	HTM=	Load
1	Wood - Exter		20	17.9	359 Btuh
2	Wood - Adjac		20	9.2	184 Btuh
3	Wood - Exter		40	17.9	718 Btuh
	Door Total		80		1260Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1932	1.3	2512 Btuh
	Ceiling Total		1932		2512Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	204.0 ft(p)	31.6	6446 Btuh
	Floor Total		204		6446 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	19320(sqft)	129	5537 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			129	5537 Btuh

Totals for Heating	**Subtotal**	**30866 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1543 Btuh**
	Total Btuh Loss	**32409 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

EWPL INC
209 SW Paisley Ct.
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

9/20/2005

Totals for Cooling	**Subtotal**	**23961 Btuh**
	Duct gain(using duct multiplier of 0.10)	**2396 Btuh**
	Total sensible gain	**26357 Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**3916 Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380 Btuh**
	Latent other gain	**0 Btuh**
	TOTAL GAIN	**31653 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

System Sizing Calculations - Summer

Residential Load - Component Details

EWPL INC
209 SW Paisley Ct.
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F 9/20/2005

Window	Type Panes/SHGC/U/InSh/ExSh	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, DEF, N, N	N	1.5	7.5	84.0	0.0	84.0	22	22	1848 Btuh
2	2, Clear, DEF, N, N	N	6	3	12.5	0.0	12.5	22	22	275 Btuh
3	2, Clear, DEF, N, N	E	1.5	5.5	30.0	4.5	25.5	22	72	1936 Btuh
4	2, Clear, DEF, N, N	S	1.5	7	30.0	30.0	0.0	22	37	660 Btuh
5	2, Clear, DEF, N, N	SW	8	7.5	21.0	21.0	0.0	22	62	462 Btuh
6	2, Clear, DEF, N, N	S	8	8	70.0	35.0	35.0	22	37	2065 Btuh
7	2, Clear, DEF, N, N	N	1.5	6	16.0	0.0	16.0	22	22	352 Btuh
8	2, Clear, DEF, N, N	W	1.5	7.5	21.0	1.1	19.9	22	72	1456 Btuh
9	2, Clear, DEF, N, N	N	1.5	3	12.5	0.0	12.5	22	22	275 Btuh
10	2, Clear, DEF, N, N	S	1.5	8	42.0	42.0	0.0	22	37	924 Btuh
	Window Total				339					10254 Btuh

Walls	Type	R-Value	Area	HTM	Load
1	Frame - Adjacent	13.0	232.0	1.0	241 Btuh
2	Frame - Exterior	13.0	1660.0	1.7	2888 Btuh
	Wall Total		1892.0		3130 Btuh

Doors	Type	Area	HTM	Load
1	Wood - Exter	20.0	10.0	200 Btuh
2	Wood - Adjac	20.0	10.0	200 Btuh
3	Wood - Exter	40.0	10.0	399 Btuh
	Door Total	80.0		798 Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load
1	Under Attic/Dark	30.0	1932.0	1.4	2743 Btuh
	Ceiling Total		1932.0		2743 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab-On-Grade Edge Insulation	0.0	204.0 ft(p)	0.0	0 Btuh
	Floor Total		204.0		0 Btuh

Infiltration	Type	ACH	Volume	CFM=	Load
	Natural	0.35	19320	112.9	2236 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			113	2236 Btuh

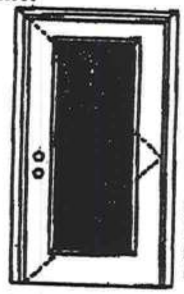
Internal gain	Occupants	Btuh/occupant		Appliance	Load
	6	X 300	+	3000	4800 Btuh

X
Glazed Inswing Unit

COP-WL-EN4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Windowsill Hurricane

Test Data Review Certificate #30264470 and COP/Test Report Validation Matrix #30264470-001 provides additional information - available from the ITS/WH website (www.etlwh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+50.5/-50.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

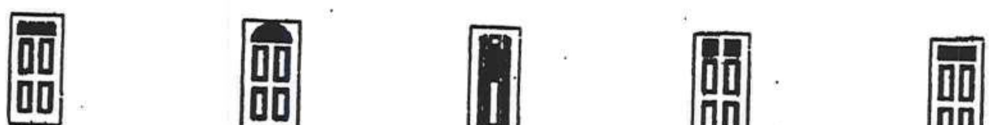
Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:



106 Series — 133, 136 Series — 126 Series — 680 Series — 822 Series

1/2 GLASS:

105 Series* — 108, 160 Series* — 120 Series* — 200 Series* — 12 R/L, 23 R/L, 24 R/L Series* — 167 Series* — 108 Series — 204 Series

*This glass kit may also be used in the following door styles: 6-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 6-panel with scroll.

Entergy
Entry Systems
June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors
Exclusively from Masonite
Masonite International Corporation

X
Glazed Inswing Unit

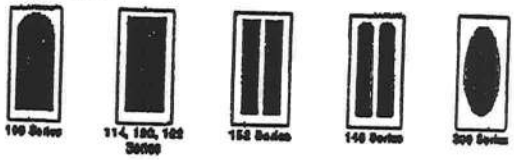
COP WI FN4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:

FULL GLASS:



CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #0029447C and COP/Test Report Validation Matrix #0029447C-001 provides additional information - available from the ITS/WH website (www.retemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

7

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Entry Systems

June 17, 2002
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PREMDOR Limited Premium Quality Doors

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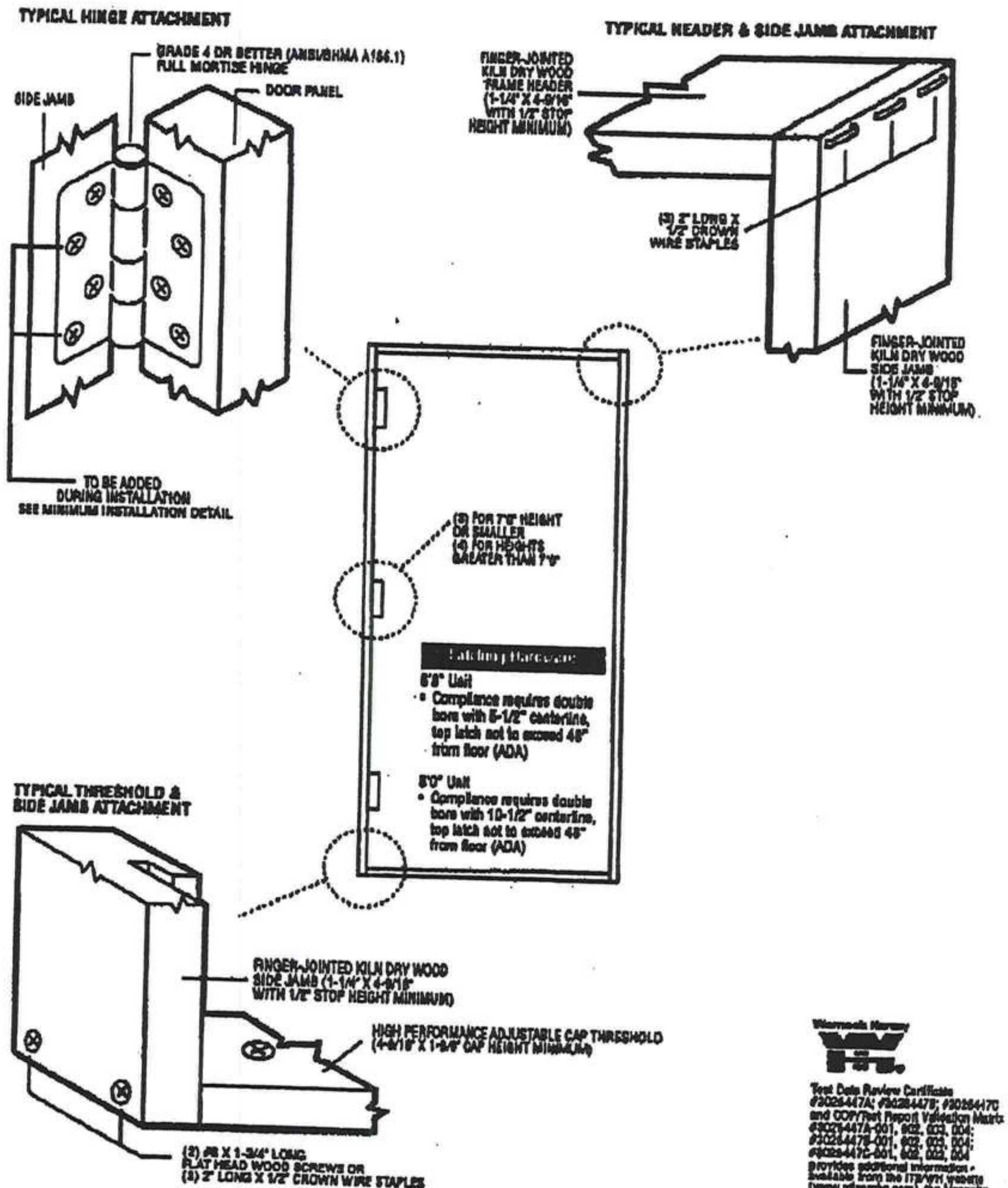
X
Unit

MAD-WL-MA0001-02

INSWING UNIT WITH SINGLE DOOR

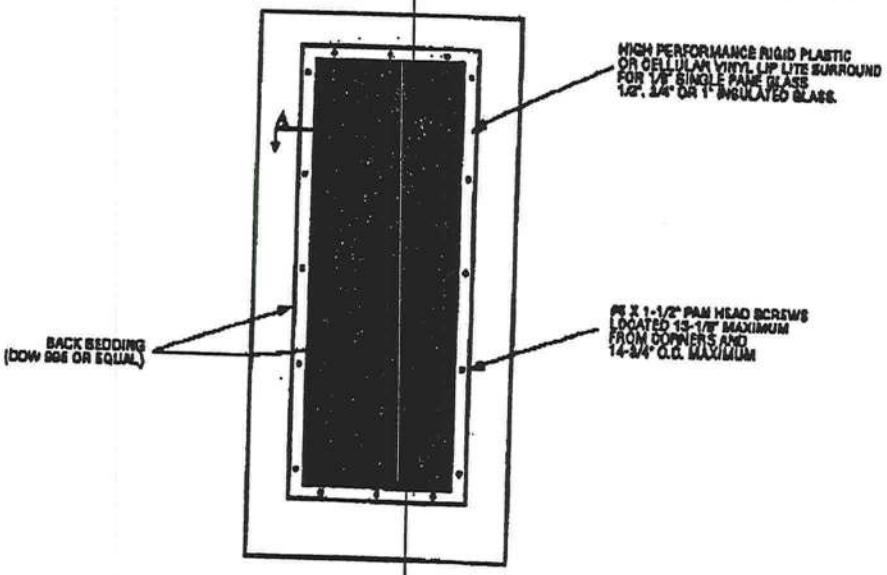


Warnock Hersey

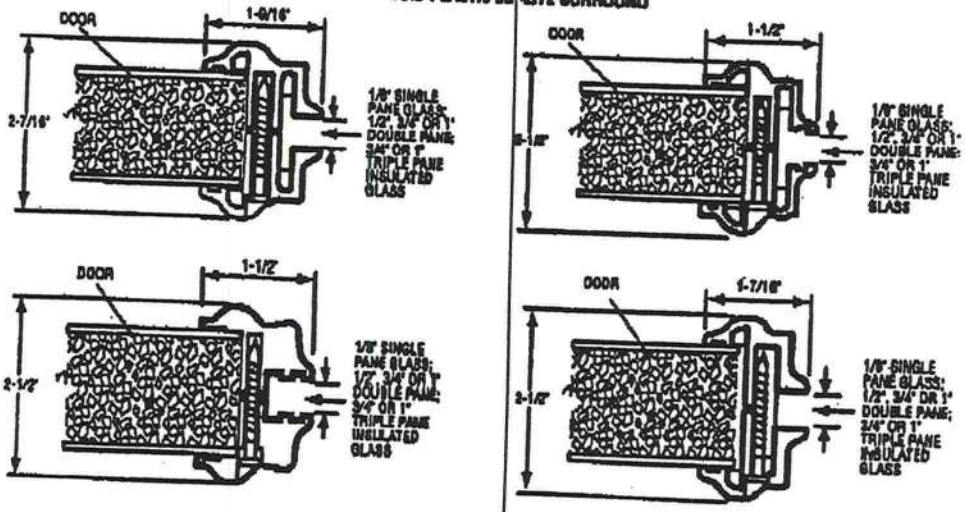
Test Data Review Certificate #3028447A; #3028447B; #3028447C and COP/Test Report Validation Matrix #3028447A-001, 002, 003, 004; #3028447B-001, 002, 003, 004; #3028447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

MAD-WL-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

Warnock Hersey

Test Data Review Certificate #30264478; #30264479; #30264470 and CDF/Test Report Validation Matrix #30264447A-001, 002, 003; #30264478-001, 002, 003; #30264470-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

PREMDOR Collection Premium Quality Doors

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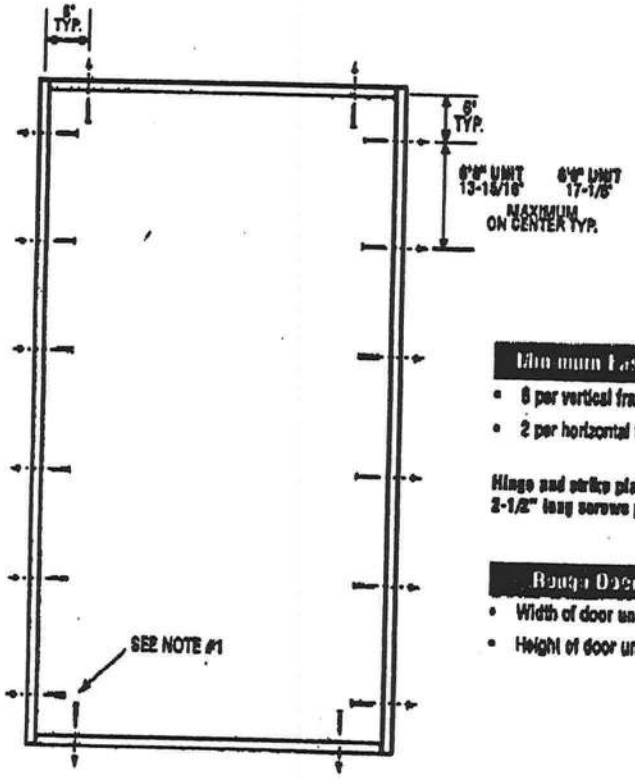
X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count:

- 8 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO):

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate #30264471; #30264478; #30264470 and COR/Test Report Validation Matrix #30264471-001, 002, 003, 004; #30264478-001, 002, 003, 004; #30264470-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etsamia.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0248*, 0256*, 3241*, 3248, 3251* or 3256**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

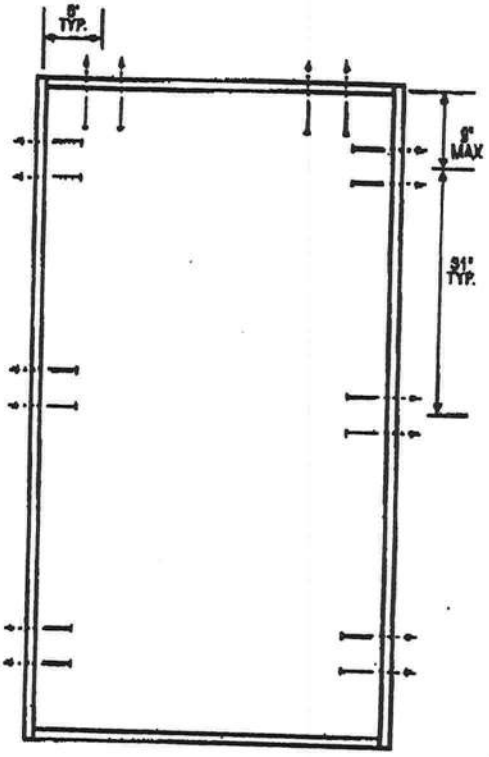
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 8 per vertical framing member for 7'0" height and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 4 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey — Test Data Review Certificate #30284478; #30284478; #30284479 and COP/Test Report Validation Matrix #30284478-001, 002, 003, 004; #30284478-001, 002, 003, 004; #30284479-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etsqwh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0248*, 3288*, 3241*, 3248, 3281* or 3288**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

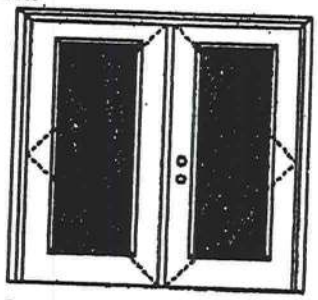
Masonite

XX
Glazed Outswing Unit

COP-WL-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Double Door
Maximum unit size – 6'0" x 6'8"

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Test Data Review Certificate #3025447C and COP/Test Report Validation Matrix #3025447C-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Design Pressure
+50.5/-50.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

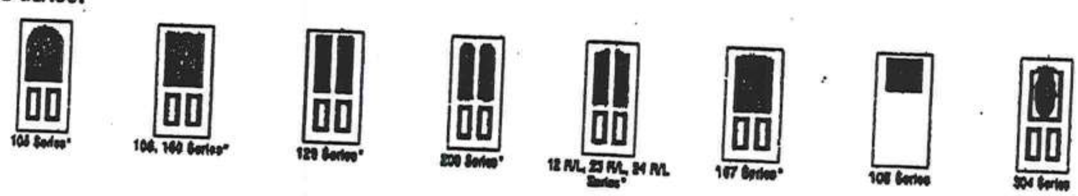
Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:
1/4 GLASS:

1/2 GLASS:



*This glass kit may also be used in the following door styles: 6-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 6-panel with scroll.

Entergy
Entry Systems
June 17, 2002
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PREMDOOR
Exclusively from Masonite
Masonite International Corporation

XX
Glazed Outswing Unit

COP-WI-FN4162-02

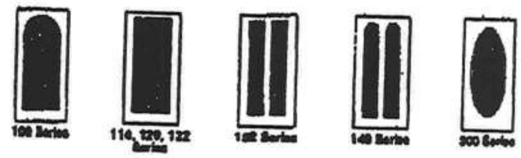
WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:

404 Series | 418 Series | 430 Series

FULL GLASS:



100 Series | 114, 120, 122 Series | 132 Series | 140 Series | 300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested In Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447C and COP/Test Report Validation Matrix #3026447C-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Entergy
Entry Systems

June 17, 2002
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PREMDOR Collection Premium Quality Doors

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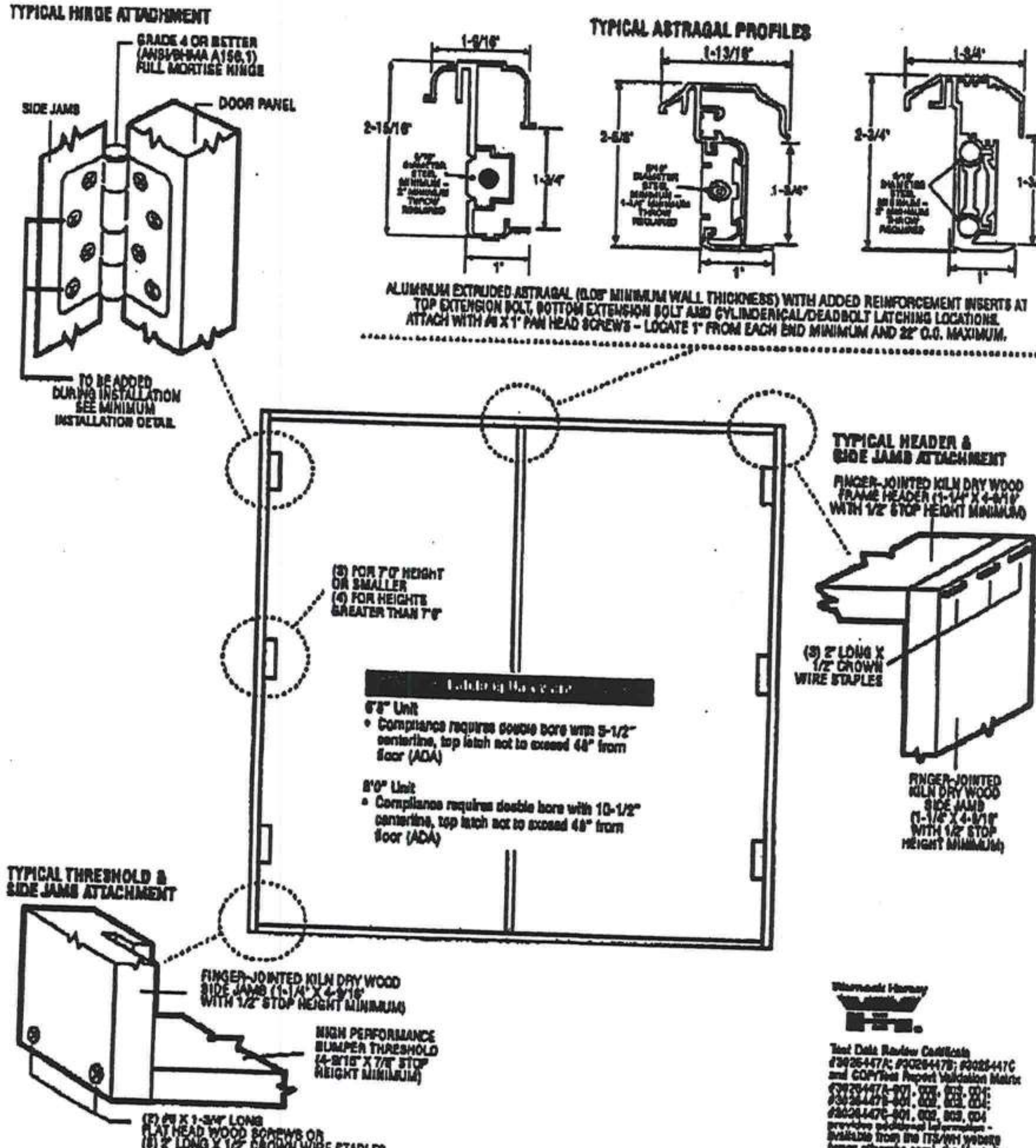
XX
Unit

MAD-WL-MA0012-02

OUTSWING UNITS WITH DOUBLE DOOR

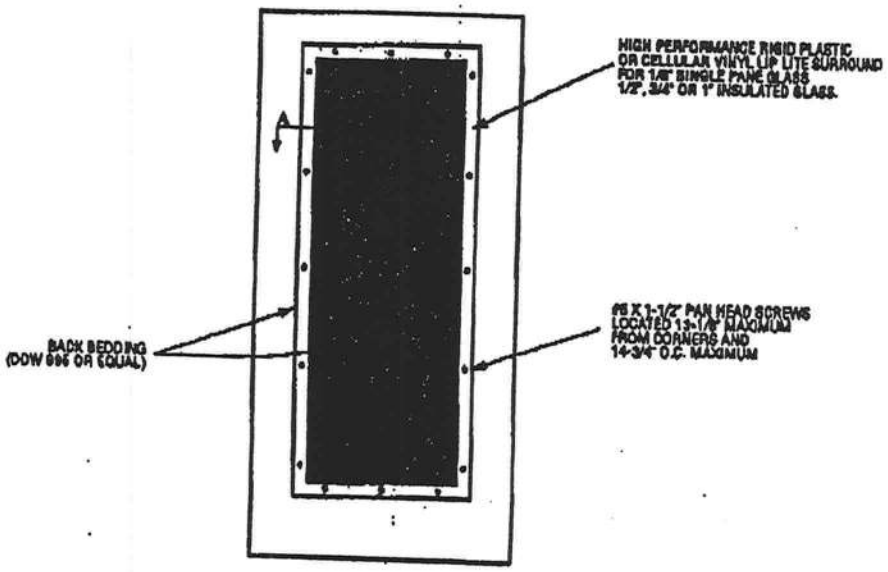


Test Data Review Certificate #3026447A; #3026447B; #3026447C and COPYTest Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

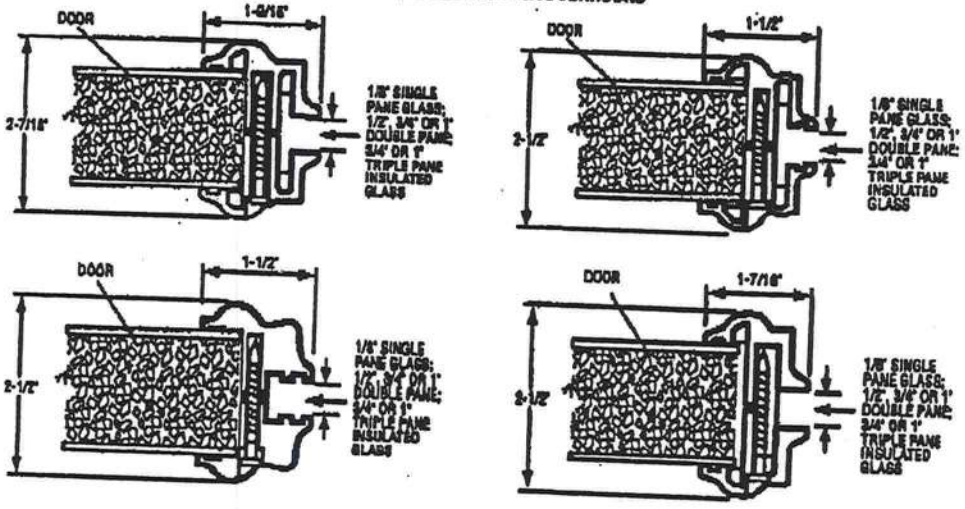
Masonite

MAD-WI-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

Test Data Review Certificate #36284478; #36284478; #36284478 and CCP/Test Report Validation Sheets #36284478-001, 002, 003; #36284478-007, 002, 003; #36284478-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

FREMDORA Premium Quality Doors

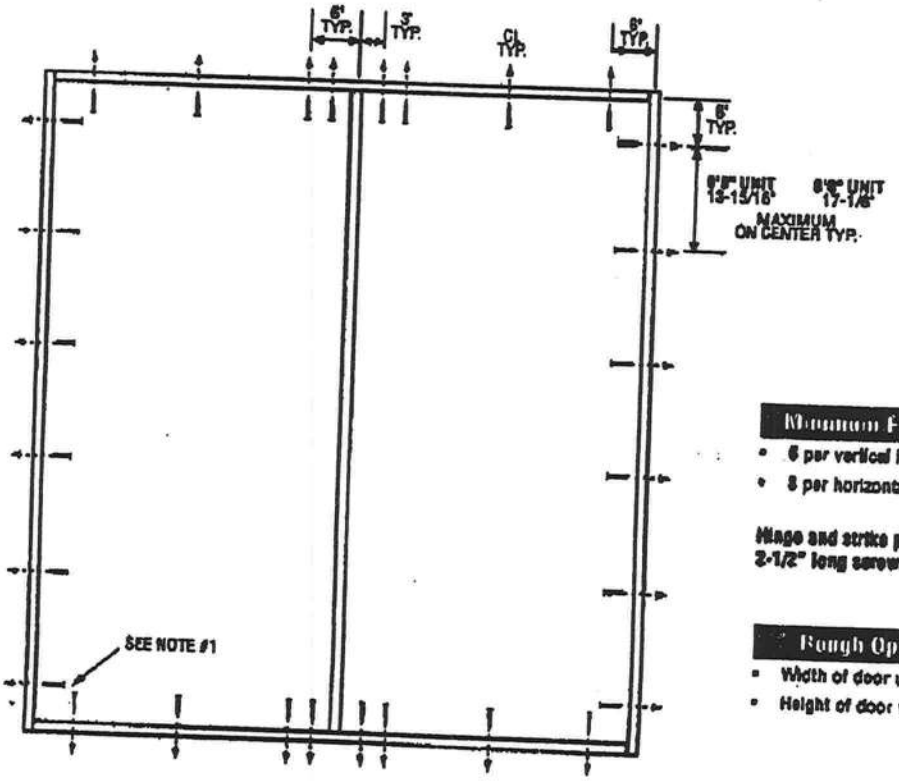
Exclusively from Masonite

Masonite International Corporation

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4" .

Test Data Review Certificate #30254478; #30254478; #30254470 and CCRTest Report Validation Matrix #3025447A-001, 002, 003, 004; #30254478-001, 002, 003, 004; #30254470-001, 002, 003, 004 provides additional information - available from the ITW-WH website (www.etsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 8247*, 8287*, 3242*, 3247, 3282* or 3267**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

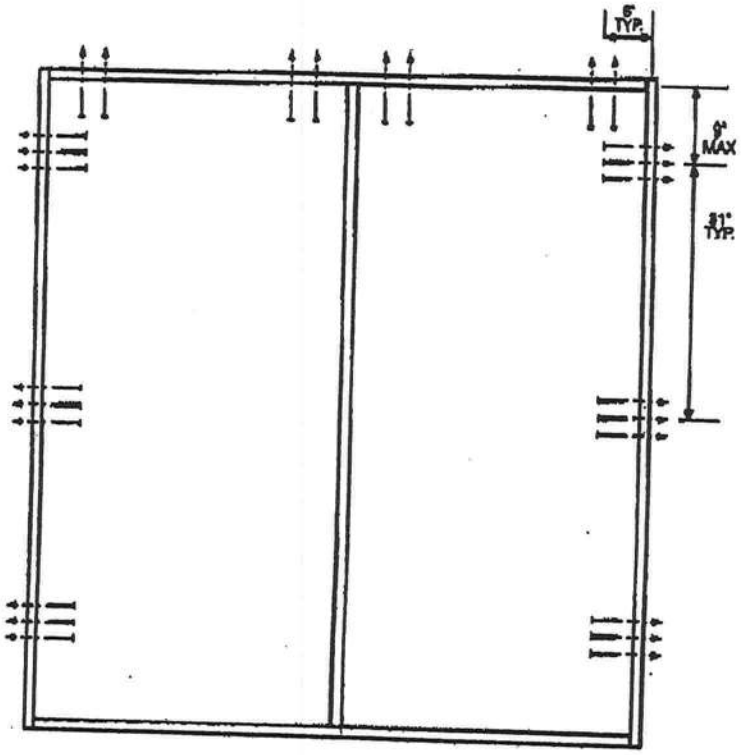
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #6 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite

XX
Unit

MID-WL-MA0002 02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0" heights and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey — Test Data Review Certificate #30225447A; #30225447B; #30225447C and COP/Test Report Validation Matrix #30225447A-001, 002, 003, 004; #30225447B-001, 002, 003, 004; #30225447C-001, 002, 003, 004 provides additional information - available from the ITS/Intertek website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0247*, 0257*, 3242*, 3247, 3252* or 3257**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 wood screws and 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw and common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and schlavement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite

MI HOME PRODUCTS

MI Home Products, Inc.
650 West Market St.
P.O. Box 370
Gratz, PA 17030-0370

(717) 365-3300
(717) 362-7025 Fax

740/744 Single Hung (Fin & Flange)
165 Single Hung (Fin & Flange)
BB165/740/744 Fixed (Fin & Flange)

- Test Reports
  - 165 Single Hung
    - #CTLA-787W (Fin)
    - #CTLA-787W-1 (Flange)
  - 740/744 Single Hung
    - #01-40351.03 (Fin)
    - #01-40351.04 (Flange)
  - 165/740/744 Fixed
    - #NCTL-310-0005-2.1 (Fin)
    - # NCTL-310-0005-5.1 (Flange)
    - #01-40486.03 (2-Panel Fixed)
- Installation Instructions
- Sample 110/120/140 MPH Labels

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 740/744
TYPE: Aluminum Single Hung Window with Nail Fin

Title of Test	Results
Rating	H R45 52 x 72
Overall Design Pressure	45 psf
Operating Force	24 lb max.
Air Infiltration	0.10 cfm/ft^2
Water Resistance	6.75 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-40351.03 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess

Mark A. Hess, Technician

MAH:baw

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
NO. 193
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

THIS FENESTRATION PRODUCT COMPLIES* WITH THE

NEW FLORIDA BUILDING CODE

FOR RESIDENTIAL BUILDINGS WITH A MEAN ROOF HEIGHT OF 30 FT. OR LESS, *EXPOSURE "B"* (WHICH IS INLAND OF A LINE THAT IS 1500 FT. FROM THE COAST), AND *WALL ZONE "5"* (INSTALLED NEAR THE CORNER OF THE BUILDING).

PER *ASTM E1300*, THE CORRECT GLASS THICKNESS, BASED ON THE *NEGATIVE* DESIGN PRESSURE (DP) LISTED BELOW, HAS BEEN INSTALLED IN THIS UNIT. THE GLASS THICKNESS IS BASED ON ITS' WIDTH, HEIGHT, AND ASPECT RATIO.

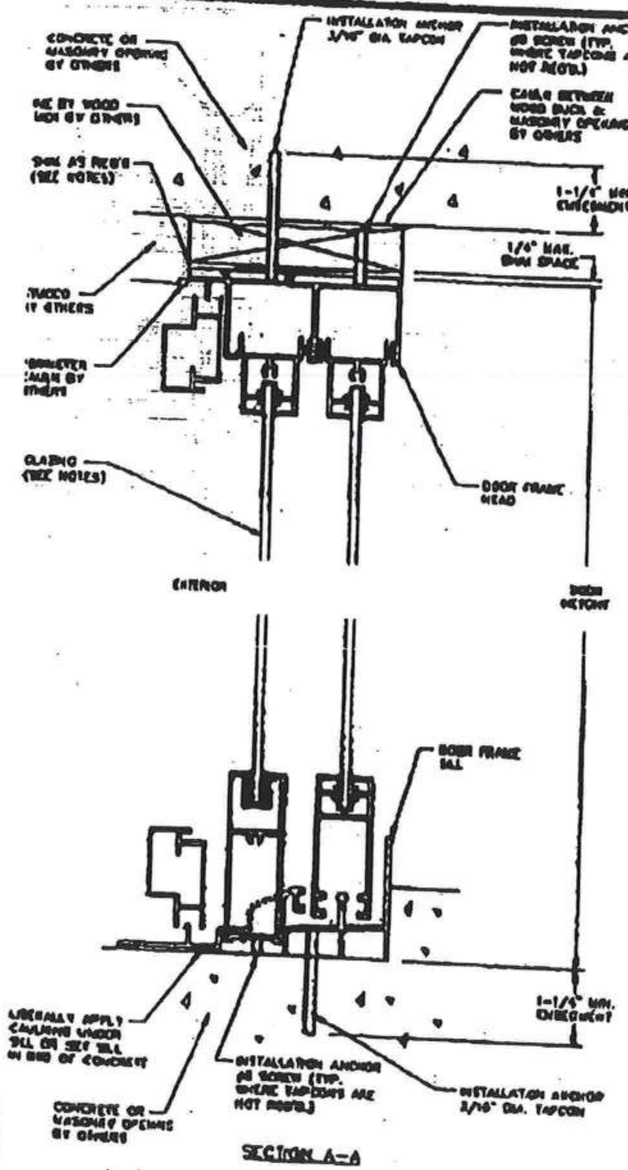
Series 470HP SLIDING GLASS DOOR – all 6'- 8" High Panels

- 2'- 6" WIDE DP + 40.0 / - 55.4
- 3'- 0" WIDE DP + 40.0 / - 48.5
- 4'- 0" WIDE DP + 40.0 / - 40.3

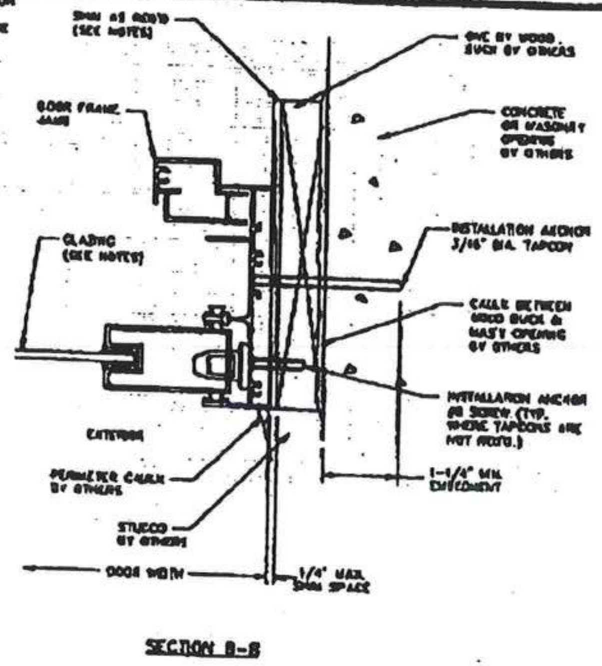
THIS PRODUCT MEETS THE REQUIREMENTS FOR STRUCTURAL LOADS, WATER AND AIR INFILTRATION PER ATTACHED *AAMA* PERFORMANCE LABEL. BE ADVISED THAT IF LOADS ARE PLACED UP TO OR EXCEEDING THE TESTED LEVELS, THIS PRODUCT MAY BE ALTERED IN SUCH A WAY THAT FUTURE PERFORMANCE WILL BE REDUCED.

* COMPLIANCE MUST INCLUDE INSTALLATION ACCORDING TO MANUFACTURER'S INSTRUCTIONS AND FLORIDA CODE REQUIREMENTS.

MIP-686



SECTION A-A

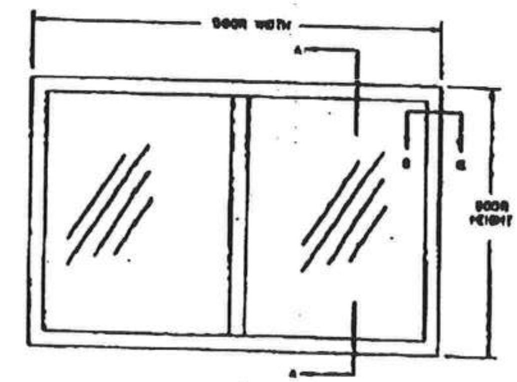


SECTION B-B

NOTES:

1) DOOR MATERIAL: ALUMINUM ALLOY [illegible]
2) [illegible] 1-1/4" [illegible]
3) ALTERNATE TAPCON INSTALLATION ANCHORS FRONT TO BACK IN ALL FRAME MEMBERS. [illegible]
4) [illegible]
5) [illegible]
6) [illegible]
7) [illegible]
8) [illegible]
9) [illegible]
10) [illegible]
11) [illegible]
12) [illegible]
13) [illegible]
14) SERIES 470 [illegible] 470 & 440 [illegible]
15) [illegible] 1-1/4" [illegible]

TAPCON INSTALLATION CHART					
CALL SIZE	DOOR SIZE	LOCATION IN HEAD & SILL			EA. JAMB
		UP TO 45 PSF DP	46 PSF TO 60 PSF DP	61 PSF TO 70 PSF DP	UP TO 70 PSF DP
[illegible]	60 x 80	3	3	3	4
[illegible]	72 x 80	4	4	4	4
[illegible]	96 x 80	5	5	5	4
[illegible]	120 x 80	6	6	7	4
[illegible]	60 x 96	3	3	3	6
[illegible]	72 x 96	3	3	3	6
[illegible]	96 x 96	4	4	5	5
[illegible]	120 x 96	6	6	7	5
[illegible]	[illegible] x 80	5	5	5	4
[illegible]	[illegible] x 80	7	5	6	4
[illegible]	[illegible] x 80	7	7	8	4
[illegible]	[illegible] x 80	8	10	10	4
[illegible]	[illegible] x 96	5	5	5	5
[illegible]	[illegible] x 96	5	5	6	5
[illegible]	[illegible] x 96	7	8	9	5
[illegible]	[illegible] x 96	8	11	13	5
[illegible]	[illegible] x 80	5	5	5	4
[illegible]	[illegible] x 80	5	5	6	4
[illegible]	[illegible] x 80	7	7	8	4
[illegible]	[illegible] x 80	8	10	11	4
[illegible]	[illegible] x 96	5	5	5	5
[illegible]	[illegible] x 96	5	5	6	5
[illegible]	[illegible] x 96	7	8	9	5
[illegible]	[illegible] x 96	8	11	13	5
[illegible]	[illegible] x 80	6	6	7	6
[illegible]	[illegible] x 80	7	7	8	6
[illegible]	[illegible] x 80	9	10	13	6
[illegible]	[illegible] x 80	8	8	7	5
[illegible]	[illegible] x 96	7	8	9	5
[illegible]	[illegible] x 96	9	12	14	5



EXTERIOR ELEVATION

NO. 47182
STATE OF FLORIDA

MI HOME PRODUCTS GRATZ, PA.		
TITLE: SERIES 470 SLIDING GLASS DOOR INSTALLATION WITH TAPCONS		
MANUAL MARTINEZ	BB	1/29/02
CIVIL	SCALE N.T.S.	MHF0018
FL. REG. NO. 47182		SHEET: 1 OF 1

TOTAL P.04

DOCUMENT CONTROL ADDENDUM #01-40351.00

Current Issue Date: 02/15/02

Report No.: 01-40351.01

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 744 aluminum single hung window with flange.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Report No.: 01-40351.02

Requested by: William Emley, MI Home Products, Inc.
Purpose: Change of glass type.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories.

Report No.: 01-40351.03

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 740/744 aluminum single hung window with nail fin.
Issued Date: 02/15/02
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Allen N. Reeves
15 FEBRUARY 2002

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meting rail) (Loads were held for 52 seconds)		
	@ 45.0 psf (positive)	0.91"*	0.29" max.
	@ 45.0 psf (negative)	0.97"*	0.29" max.
4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads held for 10 seconds)		
	@ 67.5 psf (positive)	0.14"	0.20" max.
	@ 67.5 psf (negative)	0.19"	0.20" max.
4.4.2	@ 70.8 psf (negative)	0.20"	0.20" max.

** Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:baw
01-40351.03

Allen N. Reeves, P.E.
Director - Engineering Services
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
NO. 19354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into the #2 2 x 8 Spruce-Pine-Fir wood buck with 1" galvanized roofing nails through the nail fin every 8" on center. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	24 lbs	30 lbs max.
2.1.2	Air Infiltration (ASTM E 283) @ 1.57 psf (25 mph)	0.10 cfm/ft^2	0.30 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-96) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.86"*	0.29" max.
	@ 15.0 psf (negative)	0.81"*	0.29" max.

*Note: * Exceeds L/175 for deflection, but meets all other test requirements.*

2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.20" max.
	@ 22.5 psf (negative)	<0.01"	0.20" max.
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction at 70 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.03"/6%	0.50"/100%
	Right stile	0.03"/6%	0.50"/100%

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
No. 19354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.330" high by 0.187" backed polypile with center fin	1 Row	Fixed meeting rail interlock
0.170" high by 0.187" backed polypile with center fin	1 Row	Fixed lite, stiles and top rail
3/8" diameter hollow bulb gasket	1 Row	Bottom rail
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash stiles
0.150" high by 0.187" wide polypile	1 Row	Active sash stiles

Frame Construction: All frame members were constructed of extruded aluminum with coped, butted and sealed corners fastened with two screws each. Fixed meeting rail was secured utilizing one screw in each end directly through exterior face into jamb. Silicone was utilized around exterior meeting rail/jamb joinery.

Sash Construction: All sash members were constructed of extruded aluminum with coped and butted corners fastened with one screw each.

Screen Construction: The screen frame was constructed from roll-formed aluminum members with plastic keyed corners. The screening consisted of a fiberglass mesh and was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Plastic tilt latch	2	One each end of the interior Meeting rail
Metal sweep lock	2	13" from meeting rail ends
Balance assembly	2	One per jamb
Screen tension spring	2	One per end of screen stile
Tilt pin	2	One each end of bottom rail

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
NO. 19354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-40351.03
Test Dates: 10/22/01
And: 10/23/01
Report Date: 02/15/02
Expiration Date: 10/23/05

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness performance testing on a Series/Model 740/744, aluminum single hung window at MI Home Products, Inc.'s test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R45 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 740/744

Type: Aluminum Single Hung Window With Nail Fin

Overall Size: 4' 4-1/8" wide by 5' 11-5/8" high

Active Sash Size: 4' 2-3/4" wide by 2' 11-5/8" high

Fixed Daylight Opening Size: 4' 1-1/8" wide by 2' 9" high

Screen Size: 4' 1-7/8" wide by 2' 11-5/16" high

Finish: All aluminum was polished.

Glazing Details: The active sash and fixed lite were glazed with one sheet of 1/8" thick clear tempered glass. Each sash was channel glazed using a flexible vinyl gasket.

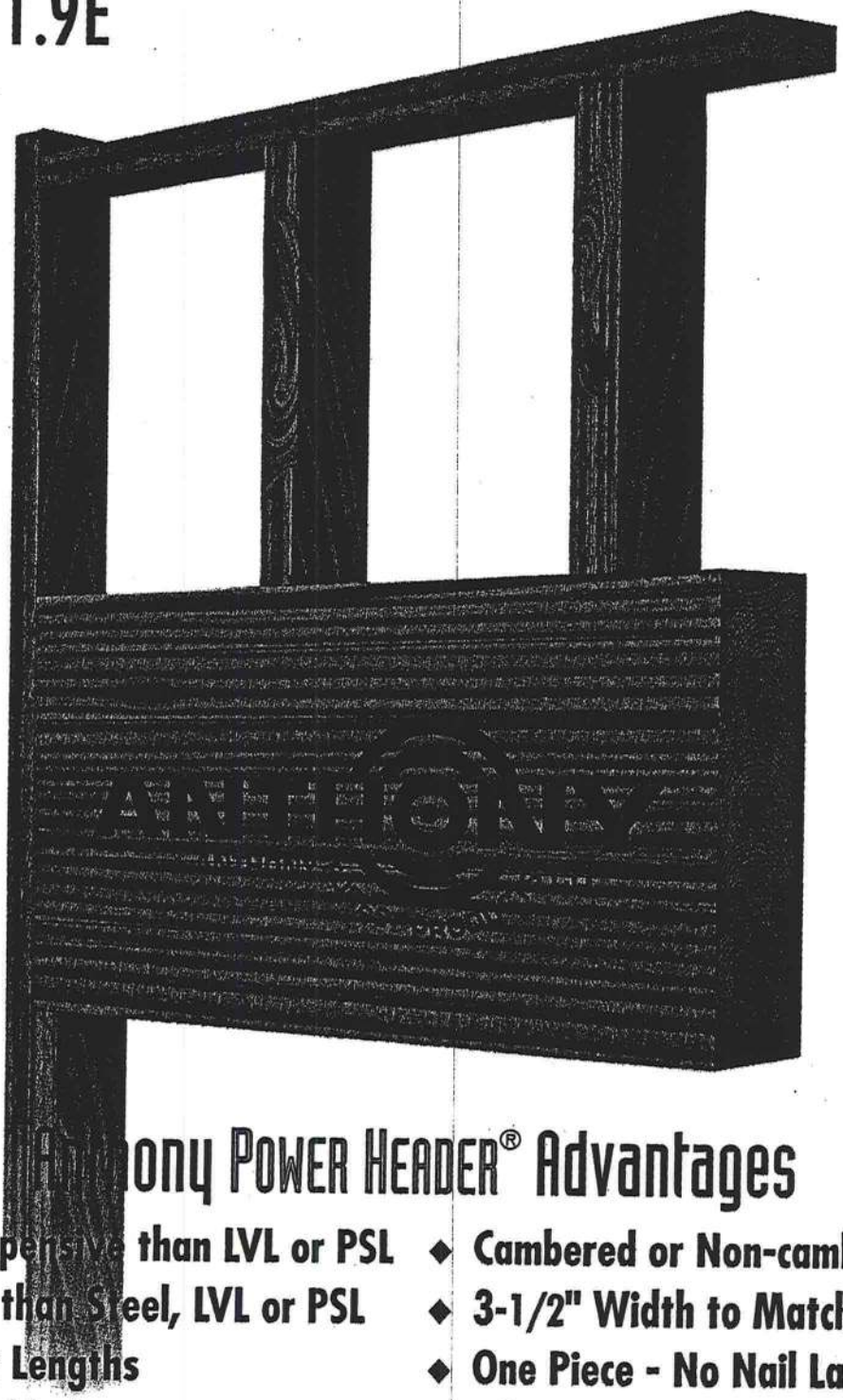
ALLEN M. REEVES
CERTIFICATE
NO. 1935
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen M. Reeves

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.testati.com

Anthony POWER HEADER®

$2600F_b$ - 1.9E



Anthony POWER HEADER® Advantages

- Less Expensive than LVL or PSL
- Lighter than Steel, LVL or PSL
- Pre-Cut Lengths
- Renewable Resource
- Cambered or Non-cambered
- 3-1/2" Width to Match Framing
- One Piece - No Nail Laminating
- Lifetime Warranty

Garage Header Sizing Tables

ANTHONY®
ANTHONY FOREST PRODUCTS CO.

Anthony Power Header®

$26F_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER APPLICATION - SINGLE STORY

HEADER SUPPORTING: 1/2 ROOF SPAN

ROOF LOAD (PSF) / ROOF TRUSS SPACING (IN.) / SOFFIT ASSUMED	SNOW LOAD AREAS USING LOAD DURATION FACTOR = 1.15																	
	20									30								
	12			16			24			12			16			24		
	0			2			4			0			2			4		
ROUGH OPENING	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	16-3/4
	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	
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	8-3/8	14	15-3/8	8-3/8	15-3/8		8-3/8	15-3/8		9-3/4			9-3/4			11-1/4		
	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4			11-1/4		

ROOF LOAD (PSF) / ROOF TRUSS SPACING (IN.) / SOFFIT ASSUMED	NON-SNOW LOAD AREAS USING LOAD DURATION FACTOR = 1.25														
	20								30						
	12			16			24		12			16			24
	0			2			4		0			2			4
ROUGH OPENING	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14
	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14
	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14
	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14
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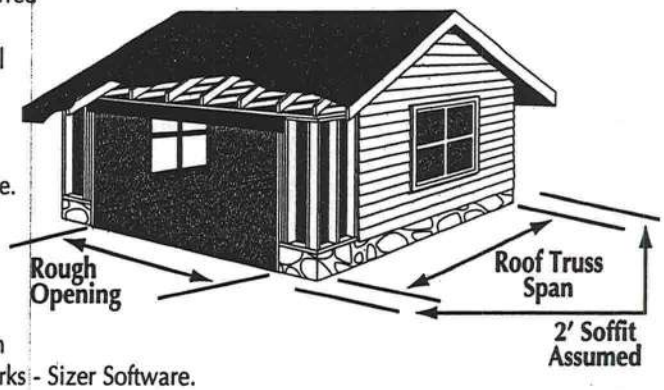
	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	

NOTES:

1. Table assumes a simple span header supporting a uniform load transferred from 1/2 the roof span plus a 2' soffit.
2. Roof live and dead loads shown are applied vertically to the horizontal projection. No reductions in roof live loads or snow loads were considered. The header weight is accounted for in the table.
3. Deflection is limited to L/240 for live load and L/180 for total load.
4. Headers are assumed to have continuous lateral support along top edge.
5. Bearing length based on full width bearing is indicated as follows:
   Non-shaded sizes require two trimmers (3" bearing).
   Shaded sizes require three trimmers (4.5" bearing).
   Shaded & outlined sizes require four trimmers (6" bearing).
6. ** Applications where load carrying capacity of 16-3/4" depth has been exceeded. See AFP $30F_b$ POWER BEAM® literature or AFP's WoodWorks - Sizer Software.



Anthony Power Header®

$26F_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

GARAGE HEADER SUPPORTING ROOF LOADS ONLY - 115% SNOW LOAD AREA

844	896		1216		1573						
161	207	254	330	390	510	552	669	752	824		
114	145	180	231	277	359	391	510	534	653	707	789

GARAGE HEADER SUPPORTING ROOF LOADS ONLY - 125% NON-SNOW LOAD AREA

844	975		1322								
161	207	254	330	390	510	552	724	752	897		
114	145	180	231	277	359	391	510	534	699	693	

GARAGE HEADER SUPPORTING ROOF, WALL AND FLOOR LOADS - 100% LOAD DURATION

562	778	888	1056	1363	1367		1582						
107	153	169	245	260	380	368	540	501	715	664	864	840	
76	107	120	171	185	267	261	380	356	521	471	684	609	813

NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

Anthony Power Header®

$26F_b$ - 1.9E

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS (F_b) =	2600
COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) =	740
HORIZONTAL SHEAR (F_v) =	225
MODULUS OF ELASTICITY (MOE) =	1.9×10^6

Standard Beam Width (in)							
Standard Beam Depth (in)	7	9	10	11	12	14	16
WEIGHT (lbs/lin.ft.)	7.7	9.0	10.4	11.7	12.9	14.2	15.5
I (in.^{4})	326	514	789	1115	1521	2014	2604
MOMENT CAPACITY (ft-lbs)	8865	12015	15996	20145	24772	29877	35460
SHEAR CAPACITY (lbs)	3908	4550	5250	5892	6533	7175	7817

NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b, shall be modified by the Volume Factor, C_v, as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1997.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for Power Header®.

GARAGE HEADER COMPARISONS

Roof Load (psf) / Ground Snow Load (psf)	GARAGE HEADER REQUIRED				
	Span 16 ft	Span 18 ft	Span 20 ft	Span 22 ft	Span 24 ft
810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-5/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
640 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-3/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"*
765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"*
750 / 480	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"*
900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	-----

For more information on Power Header®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

Power Header® is a trademark of

Anthony Forest Products Company

Post Office Box 1877 • El Dorado, Arkansas 71731

Internet address: http:// www.anthonyforest.com

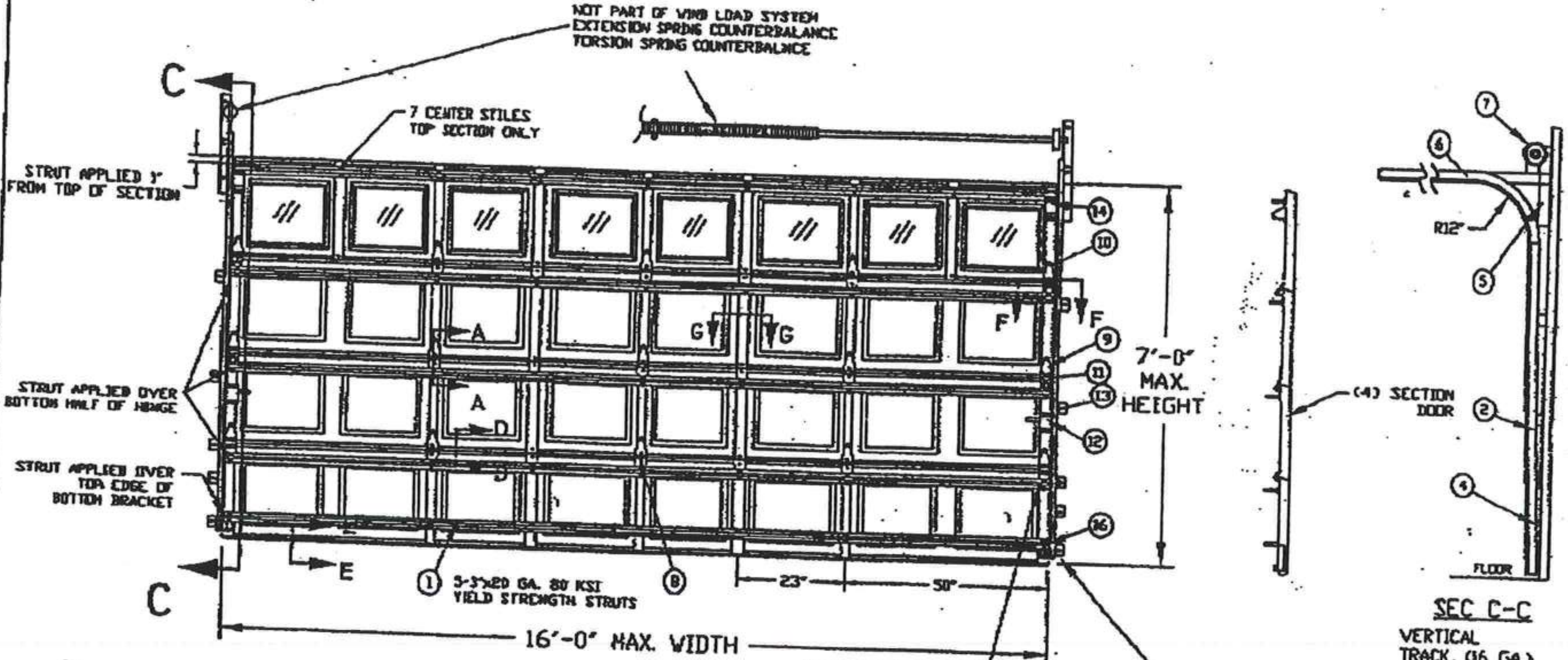
e-mail: info@anthonyforest.com

Distributed by:

REV.	DATE	BY	DESCRIPTION
A–A	11-10-00	DW	SEE E.C.N. 831

NOTES

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT= 21"
3. SECTION HEIGHTS OF 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION. (AS TESTED WITH 1/8" DSB GLASS OR EQUIVALENT) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MINIMUM LENGTH OF ROLLER STEM IS 5" (7" AS TESTED)
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOWN.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE 0,1, OR 2 AS TESTED.
9. (△) DROP IN TYPE OF INSULATION IS OPTIONAL.



INSIDE ELEVATION

ALL ROLLER CARRIERS AND HINGES ARE 14 GA.

12 GA. JAMB BRACKETS, MAXIMUM SPACING = 19-1/2" WITH LOWEST BRACKET APPROX. 3" FROM FLOOR, 2ND BRACKET NEAR THE HORIZONTAL ℄ OF THE BOTTOM SECTION, AND 3RD BRACKET NEAR THE TOP OF THE BOTTOM SECTION

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.

NORTH CAROLINA PROFESSIONAL SEAL PE No. 024280 ENGINEER NASER R. REYVAN

LISTED SBCCI PST & ESI REPORT No. 2202

TEST REPORTS ON FILE VIDEO 10/19/00 (00293)

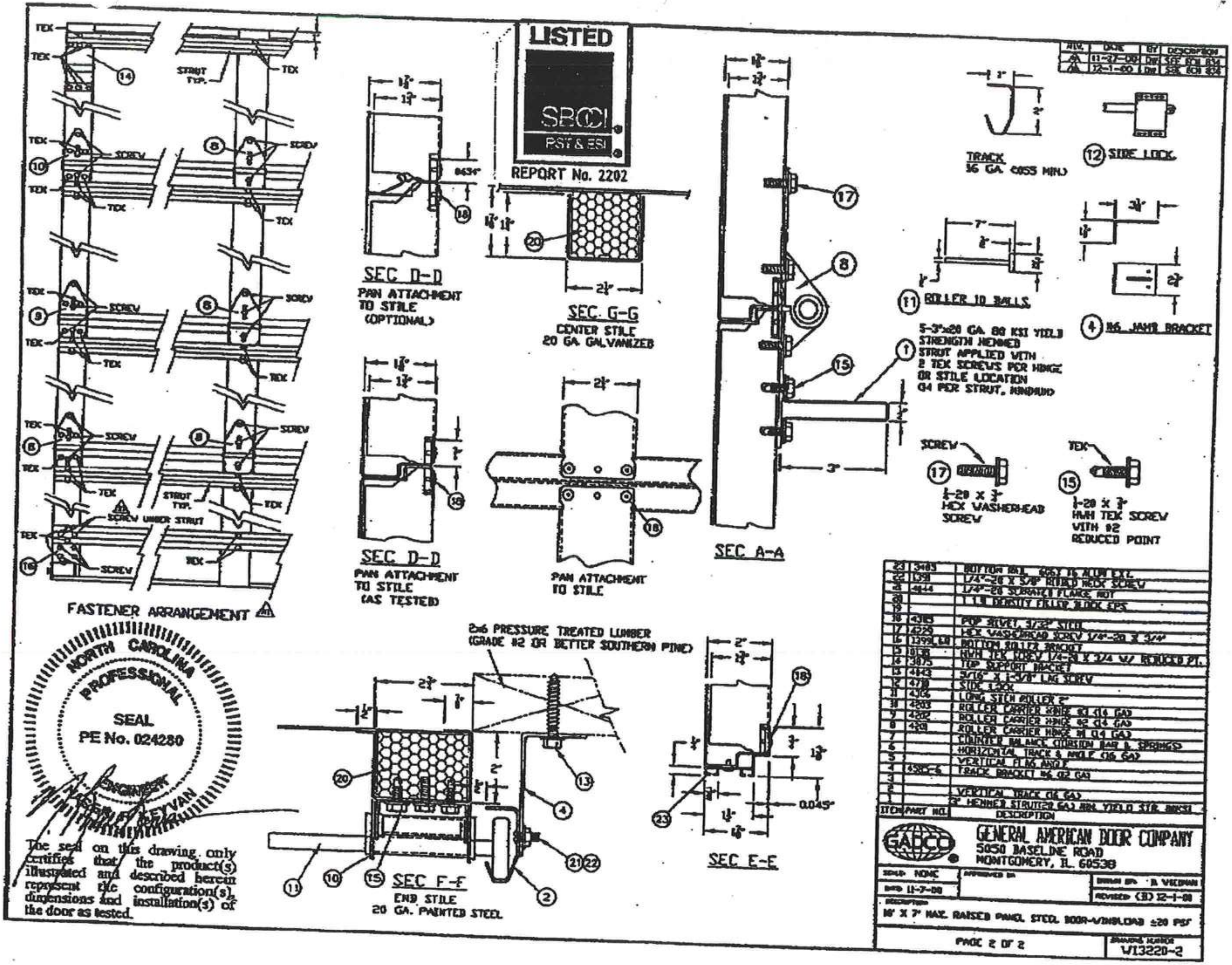
DESIGN LOAD +20.0 PSF & -20.0 PSF
TEST LOAD +30.0 PSF & -30.0 PSF

GADCO DOORS
SERIES 7480, EXTERIOR STEEL =.017 MIN (AS TESTED)
SERIES 7825, EXTERIOR STEEL =.019" MIN △
SERIES 7324, EXTERIOR STEEL =.024" MIN △
(TESTED WITH WINDOWS

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CTR. STILE SPACING	STRUTS 80 KSI SIZE	STRUTS 80 KSI QTY.	VERTICAL TRACK
16'	7'	23"	3"	5	2 IN.

GADCO GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: NTS	APPROVED BY:	DRAWN BY: D. WICHMAN
DATE: 10-20-00		REVISED: (A) 11-10-00
DESCRIPTION: 16' X 7' MAX RAISED PANEL STEEL DOOR -WINDLOAD ±20 PSF		
PART NUMBER:	PAGE 1 OF 2	DRAWING NUMBER: W13220-1

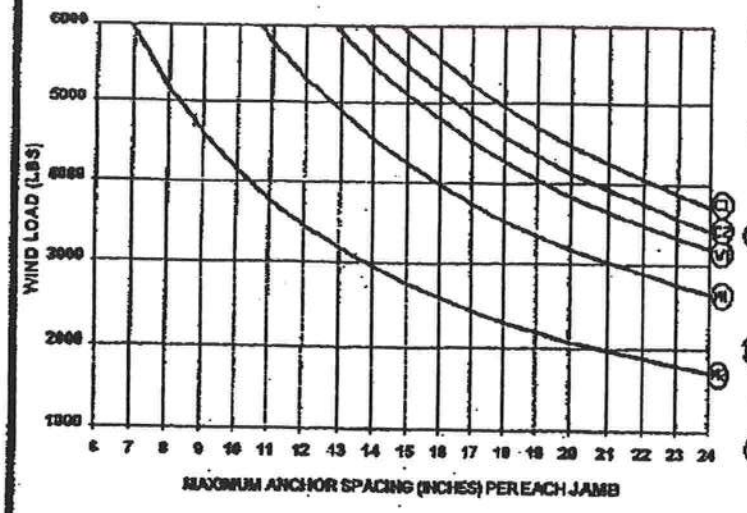


ITEM	PART NO.	DESCRIPTION
23	3485	BOTTOM RAIL, 6063 T6 ALUM EXT.
22	1398	1/4"-20 X 5/8" ROUND NECK SCREW
21	4944	1/4"-20 SERRATED FLANGE NUT
20		1 LB DENSITY FILLER BLOCK EPS
19		
18	4385	POP RIVET, 5/32" STEEL
17	4229	HEX WASHERHEAD SCREW 1/4"-20 X 3/4"
16	1399L [illegible]	BOTTOM ROLLER BRACKET
15	10138	HWH TEK SCREW 1/4-20 X 3/4 W/ REDUCED PT.
14	3875	TOP SUPPORT BRACKET
13	4843	5/16" X 1-5/8" LAG SCREW
12	4738	SIDE LOCK
11	4316	LONG STEM ROLLER 2"
10	4203	ROLLER CARRIER HINGE #3 (14 GA)
9	4202	ROLLER CARRIER HINGE #2 (14 GA)
8	4201	ROLLER CARRIER HINGE #1 (14 GA)
7		COUNTER BALANCE (TORSION BAR & SPRINGS)
6		HORIZONTAL TRACK & ANGLE (16 GA)
5		VERTICAL FLAG ANGLE
4	4580-6	TRACK BRACKET #6 (12 GA)
3		
2		VERTICAL TRACK (16 GA)
1		3" HEMMED STRUT (20 GA) MIN. YIELD STR. 80KSI

GADCO
GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE NONE | DATE 11-7-00 | DRAWN BY: B. VIEDMAN | REVISED (B) 12-1-00
DESCRIPTION: 16' X 7' MAX. RAISED PANEL STEEL DOOR-WINDLOAD ±20 PSF
PAGE 2 OF 2 | DRAWING NUMBER W13220-2

WIND LOAD vs ANCHOR SPACING



- (C1) CONCRETE BACKUP HILTI KWIK BOLT II EXPANSION ANCHOR 3/8" DIA. 1-5/8" EMBEDMENT
- (C2) CONCRETE BACKUP RAWL LOK/BOLT SLEEVE ANCHOR 3/8" DIA. 1-5/8" EMBEDMENT
- (M1) MASONRY/CMU BACKUP RAWL LOK/BOLT SLEEVE ANCHOR 3/8" DIA. 1-3/8" EMBEDMENT
- (M2) MASONRY/CMU BACKUP ITW BUILDEX TAPCON MASONRY ANCHOR 1/4" DIA. 1-3/4" EMBEDMENT
- (V1) WOOD STUD BACKUP LAG SCREWS 5/16" DIA. 1-1/2" EMBEDMENT

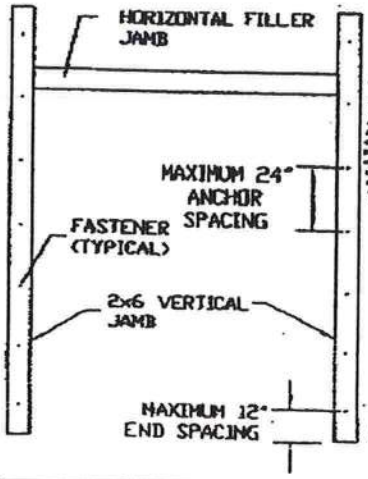
DESIGN LOAD (LBS/FT²) X GARAGE DOOR AREA(WIDTH-FT X HEIGHT-FT) = WIND LOAD(LBS)

EXAMPLE

30 LBS/FT² X 16 FT WIDE X 8 FT HIGH = 3840 LBS

- (C1) USE 22" SPACING
- (C2) USE 21" SPACING
- (V1) USE 19" SPACING
- (M1) USE 16" SPACING
- (M2) USE 10" SPACING

SEE NOTE 10 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS



NORTH CAROLINA PROFESSIONAL SEAL PE No. 024280 ENGINEER NASER R KEYVAN

NASER R KEYVAN LICENSE NO. 53774 STATE OF FLORIDA PROFESSIONAL ENGINEER

3/8/2002

2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION" SSTD 10," CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE (#2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.

5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2150 PSI; GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI; REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4"

8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.

9) WASHERS ARE REQUIRED ON ALL FASTENERS.

10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.

11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

GADCO

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: NONE	APPROVED BY:	DRAWN BY: DV
DATE: 8-30-99		REVISED
DESCRIPTION: JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS		
	DRAWING NUMBER: A10560	REVISION NUMBER

ELK

PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE™

Prestique Plus *High Definition* and Prestique Gallery Collection™

Product size	13¼"x 39⅜"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	11

50-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Raised Profile

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique I *High Definition*

Product size	13¼"x 39⅜"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	14

40-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique *High Definition*

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/*FLX*™

Size: 12"x 12"
Exposure: 6⅝"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors: Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood**, Forest Green, Wedgewood**, Birchwood**, Sandalwood. Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge roofing products contain Elk WindGuard® sealant. WindGuard activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae. Not available in Sablewood.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles meet the latest Metro Dade building code requirements.

***See actual limited warranty for conditions and limitations.**
****Check for product availability.**

SPECIFICATIONS

SCOPE: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color).

MATERIALS: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater: apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For low-

warranties are contingent upon the correct installatic as shown on the instructions. These instructions are th

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 24-4S-16-03113-166 **Building permit No.** 000023821

Use Classification SFD,UTILITY **Fire:** 23.68

Permit Holder HUGO ESCALANTE **Waste:** 49.00

Owner of Building HBM CONSTRUCTION CORPORATION **Total:** 72.68

Location: 268 SW PLATEAU GLEN(WISE ESTAES, LOT 36)

Date: 06/19/2006

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Project Information for:	L135123		
Builder:	Hugo Escalante	Date:	1/19/2006
Lot:	Lot 36	Start Number:	**1091**
Subdivision:	Wise Estates		
County or City:	Columbia County		
Truss Page Count:	41		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		**Wind**		**Building Code:**	**FBC2004**
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**120**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ESCALANTE, HUGO CRC 1326967
Address: P.O. BOX 280
FORT WHITE, FL. 32038 Designer: **32**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	119061091	1/19/2006	41	T29G	119061131	1/19/2006
2	CJ3	119061092	1/19/2006				
3	CJ5	119061093	1/19/2006				
4	EJ7	119061094	1/19/2006				
5	EJ7A	119061095	1/19/2006				
6	EJ7B	119061096	1/19/2006				
7	EJ7G	119061097	1/19/2006				
8	EJ7GA	119061098	1/19/2006				
9	HJ9	119061099	1/19/2006				
10	T01	119061100	1/19/2006				
11	T01G	119061101	1/19/2006				
12	T02	119061102	1/19/2006				
13	T03	119061103	1/19/2006				
14	T04	119061104	1/19/2006				
15	T05	119061105	1/19/2006				
16	T06	119061106	1/19/2006				
17	T07	119061107	1/19/2006				
18	T08	119061108	1/19/2006				
19	T09	119061109	1/19/2006				
20	T10	119061110	1/19/2006				
21	T11	119061111	1/19/2006				
22	T12	119061112	1/19/2006				
23	T13	119061113	1/19/2006				
24	T14	119061114	1/19/2006				
25	T15	119061115	1/19/2006				
26	T16	119061116	1/19/2006				
27	T17	119061117					
28	T18	119061118					
29	T19	119061119					
30	T20	119061120					
31	T21	119061121					
32	T22	119061122					
33	T23	119061123					
34	T24	119061124					
35	T24A	119061125					
36	T25	119061126					
37	T26	119061127					
38	T27	119061128	1/19/2006				
39	T28	119061129	1/19/2006				
40	T29	119061130	1/19/2006				

REVISED

JAN 19 2006

2382

36 WISE

02:00:39 PM 10/6/2004

Licensee Details

Licensee Information

Name:	**ESCALANTE, HUGO (Primary Name)**
	EWPL INC (DBA Name)
Main Address:	**P.O. BOX 280** **FORT WHITE, Florida 32038**

License Information

License Type:	**Certified Residential Contractor**
Rank:	**Cert Residental**
License Number:	**CRC1326967**
Status:	**Current, Active**
Licensure Date:	**11/24/2003**
Expires:	**08/31/2006**

Special Qualifications	Effective Date
Qualified Business License Required	11/24/2003

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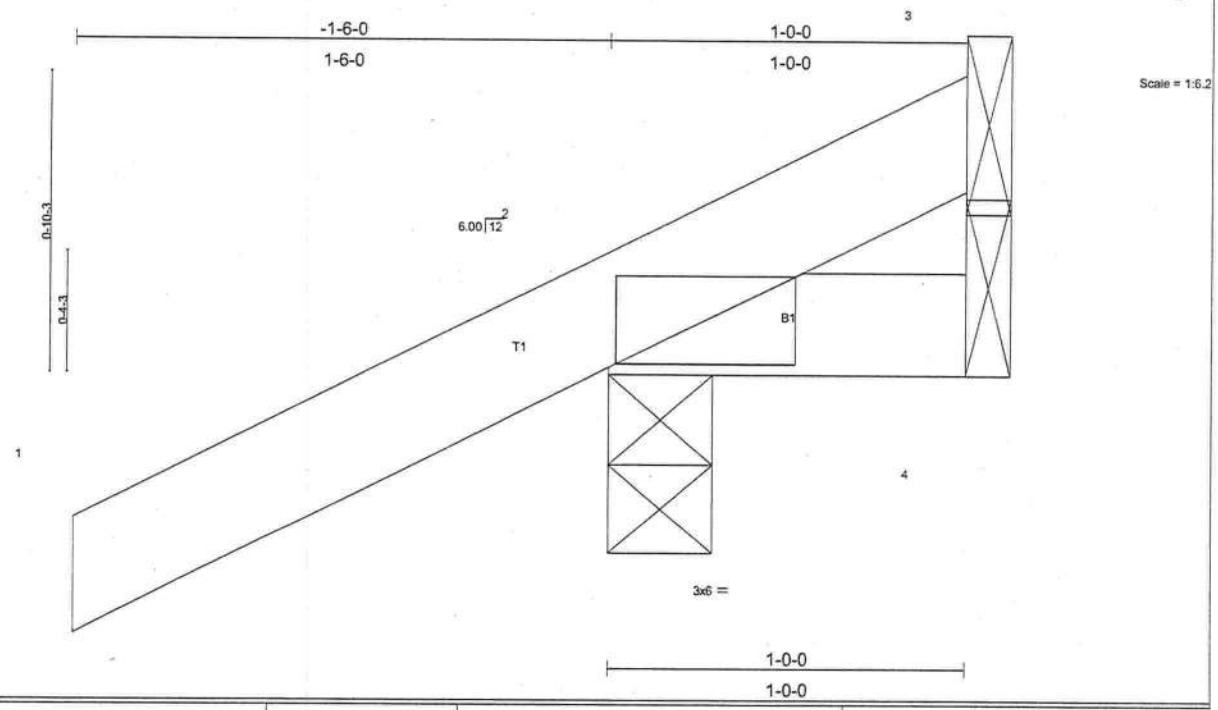
JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061091

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	CJ1	MONO TRUSS	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:10 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=189/0-3-8, 4=14/Mechanical, 3=-40/Mechanical
Max Horz 2=84(load case 5)
Max Uplift2=-220(load case 5), 3=-40(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=73(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/41
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.12

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 220 lb uplift at joint 2 and 40 lb uplift at joint 3.

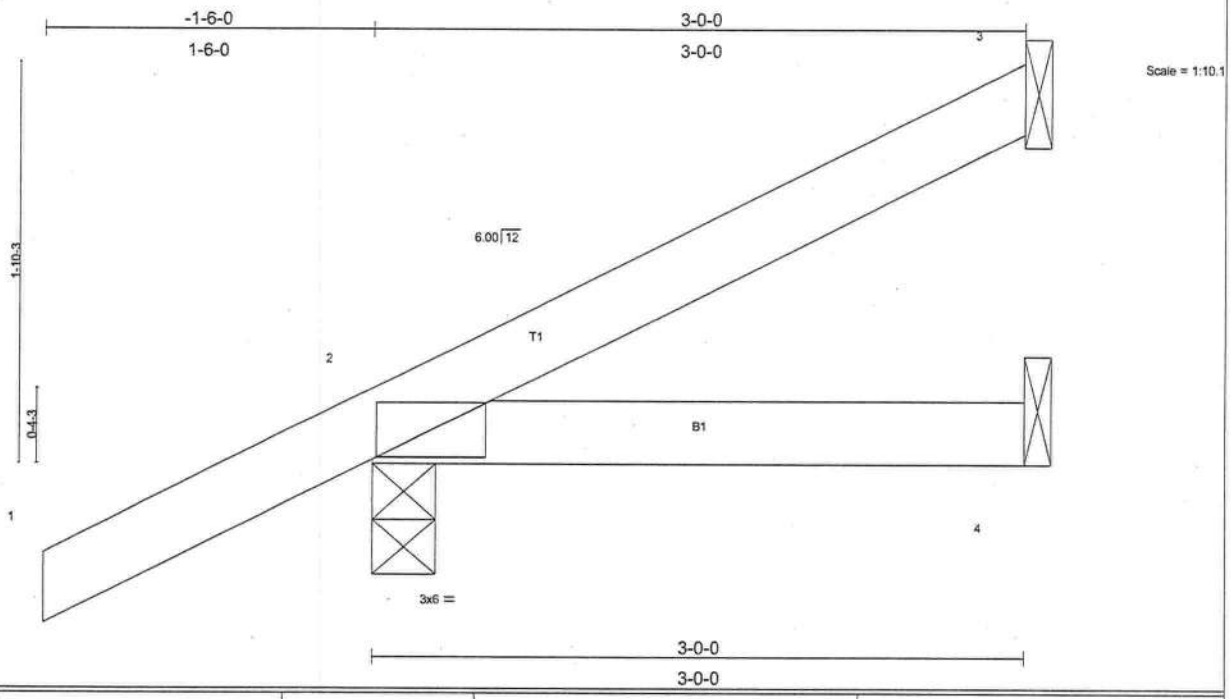
LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061092

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	CJ3	MONO TRUSS	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:10 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical
Max Horz 2=137(load case 5)
Max Uplift3=-47(load case 5), 2=-187(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-52/16
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 3 and 187 lb uplift at joint 2.

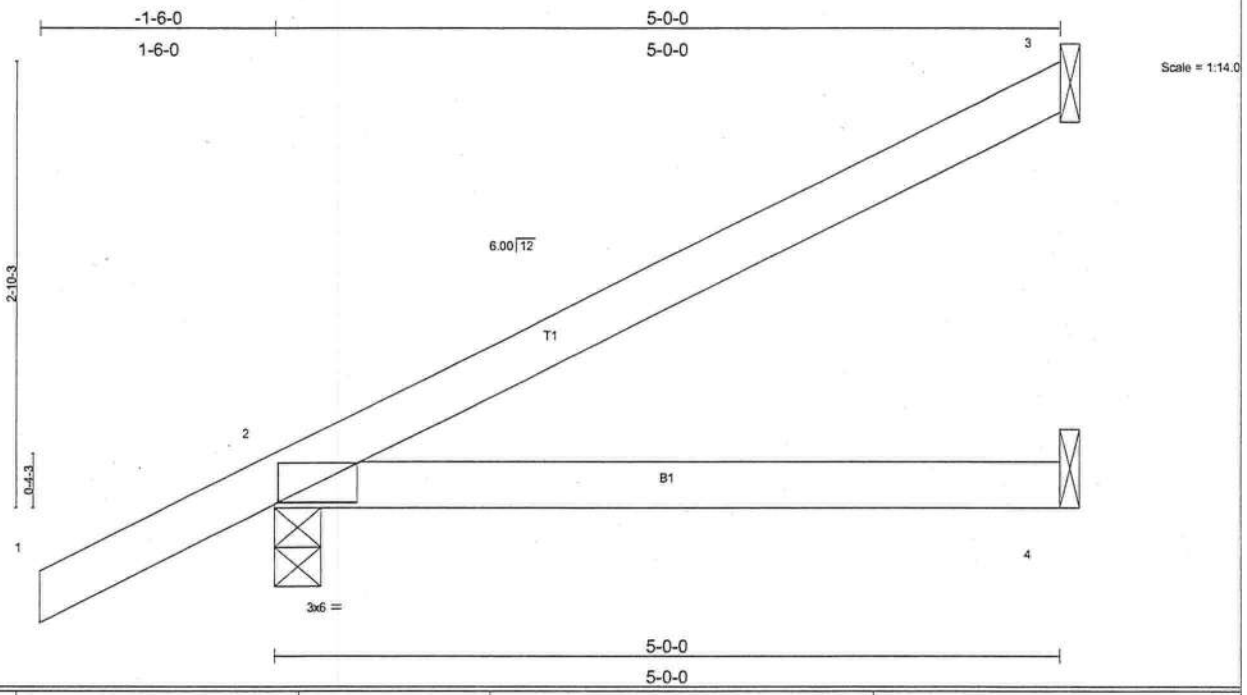
LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061093

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	CJ5	MONO TRUSS	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:11 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=192(load case 5)
Max Uplift3=-124(load case 5), 2=-197(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-112/41
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 3 and 197 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061094

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	EJ7	MONO TRUSS	29	1	Job Reference (optional)

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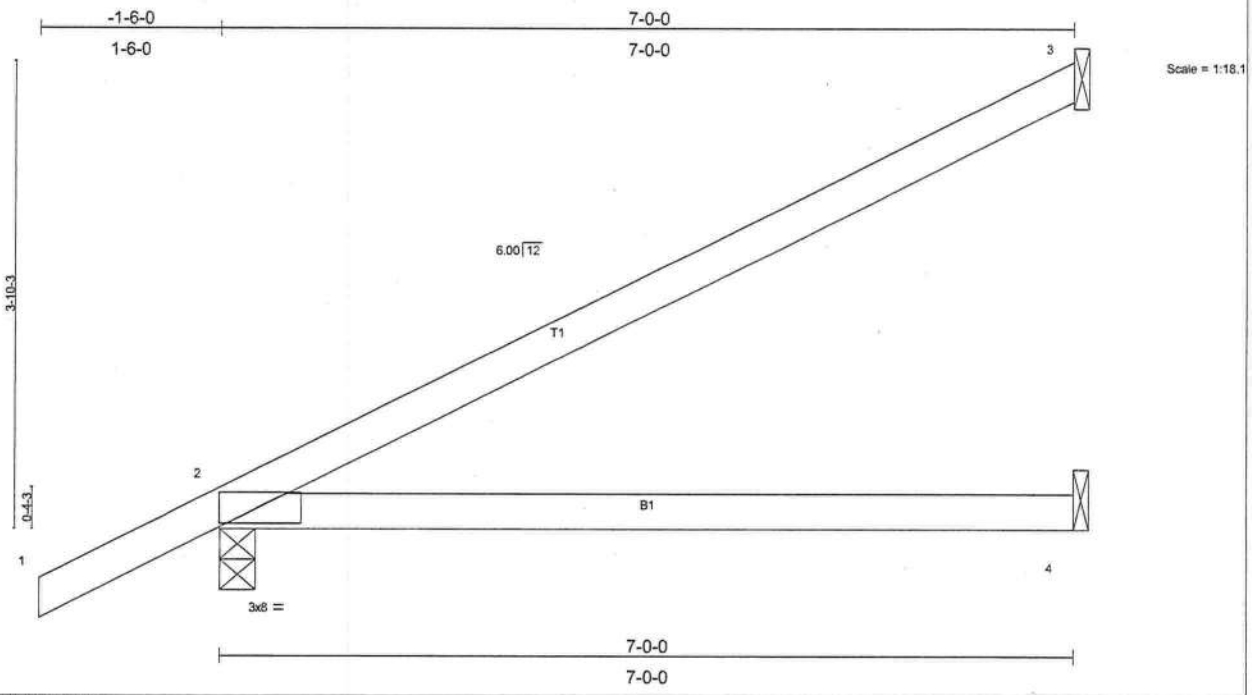

Plate Offsets (X,Y): [2:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.13	2-4	>606	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.22	2-4	>365	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=166/Mechanical, 2=385/0-3-8, 4=108/Mechanical
Max Horz 2=247(load case 5)
Max Uplift3=-170(load case 5), 2=-217(load case 5), 4=-1(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-142/59
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.76

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 3, 217 lb uplift at joint 2 and 1 lb uplift at joint 4.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061095

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	EJ7A	MONO HIP	1	1	Job Reference (optional)

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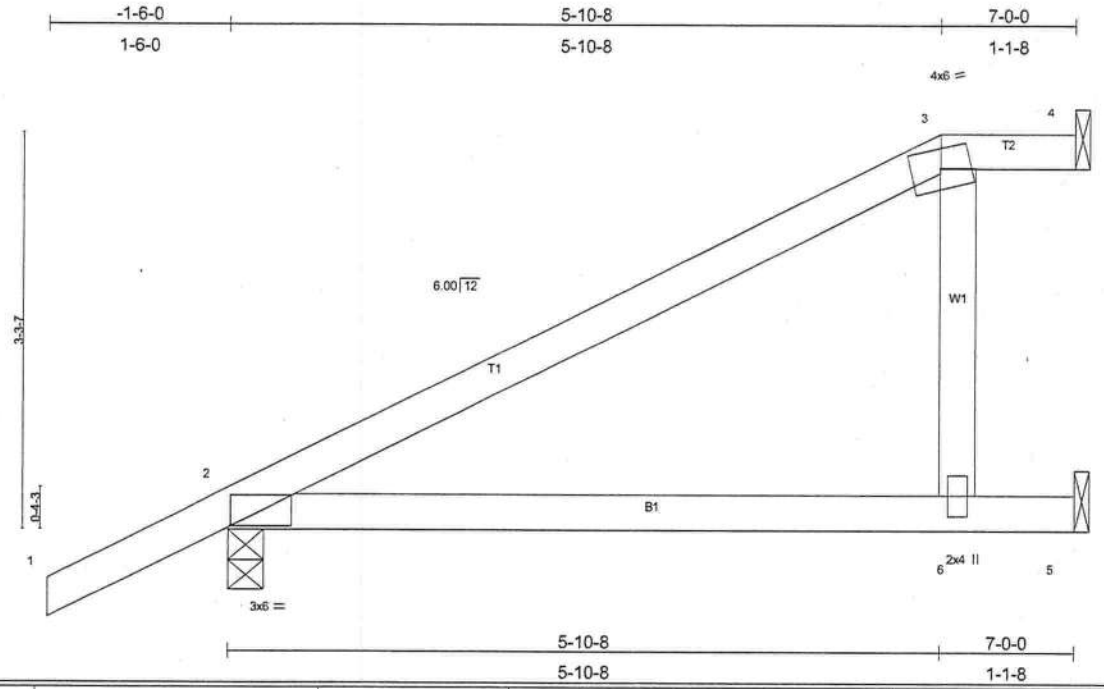

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.20	2-6	>404	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.25	2-6	>323	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.06	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=77/Mechanical, 2=385/0-3-8, 5=197/Mechanical
Max Horz 2=219(load case 5)
Max Uplift4=-13(load case 3), 2=-229(load case 5), 5=-160(load case 5)
Max Grav 4=94(load case 10), 2=385(load case 1), 5=197(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-113/42, 3-4=-0/1
BOT CHORD 2-6=-15/5, 5-6=0/0
WEBS 3-6=-112/302

JOINT STRESS INDEX
2 = 0.44, 3 = 0.42 and 6 = 0.17

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 4, 229 lb uplift at joint 2 and 160 lb uplift at joint 5.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061096

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	EJ7B	MONO HIP	1	1	Job Reference (optional)

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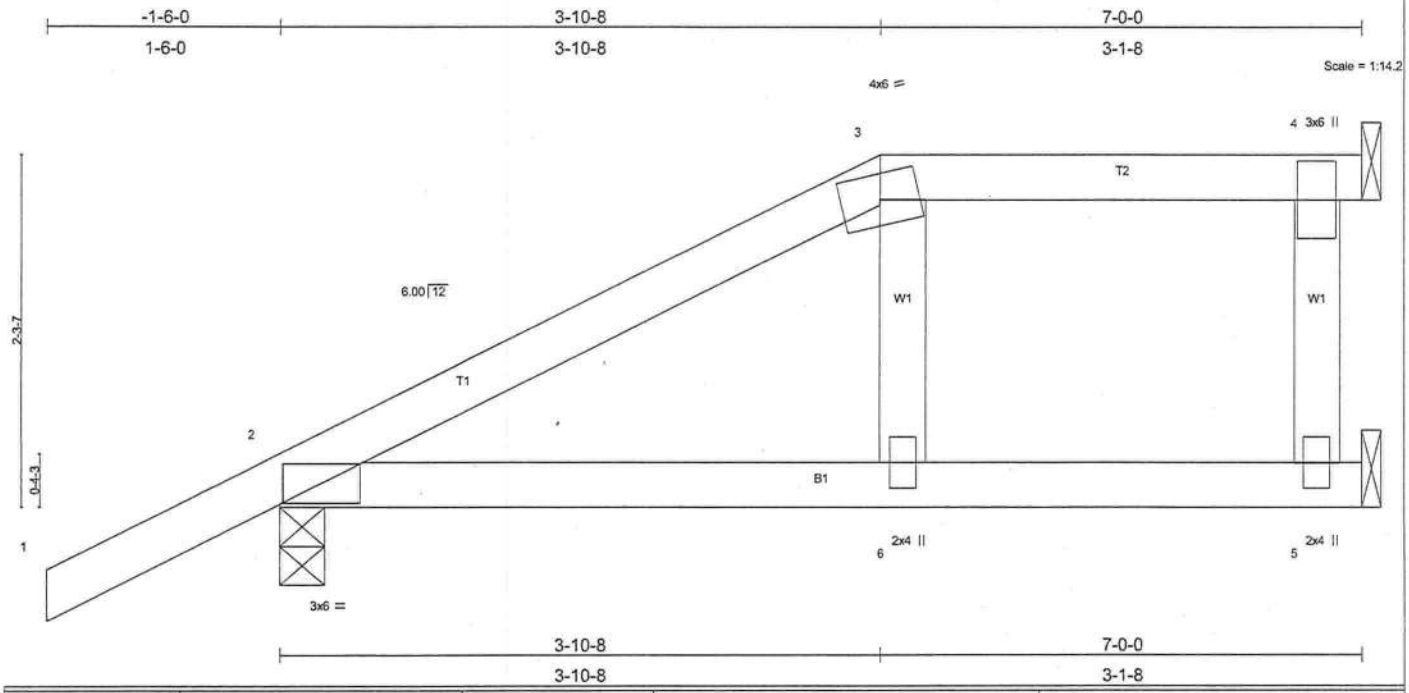

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.19	2-6	>422	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.22	2-6	>355	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.11	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=376/0-3-8, 4=138/Mechanical, 5=127/Mechanical
Max Horz 2=164(load case 5)
Max Uplift2=-236(load case 5), 4=-83(load case 4), 5=-36(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-80/6, 3-4=-0/0
BOT CHORD 2-6=-12/4, 5-6=0/0
WEBS 3-6=-55/167, 4-5=0/0

JOINT STRESS INDEX
2 = 0.25, 3 = 0.78, 4 = 0.00, 5 = 0.00 and 6 = 0.09

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 236 lb uplift at joint 2, 83 lb uplift at joint 4 and 36 lb uplift at joint 5.
6) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061097

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	EJ7G	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:13 2006 Page 1

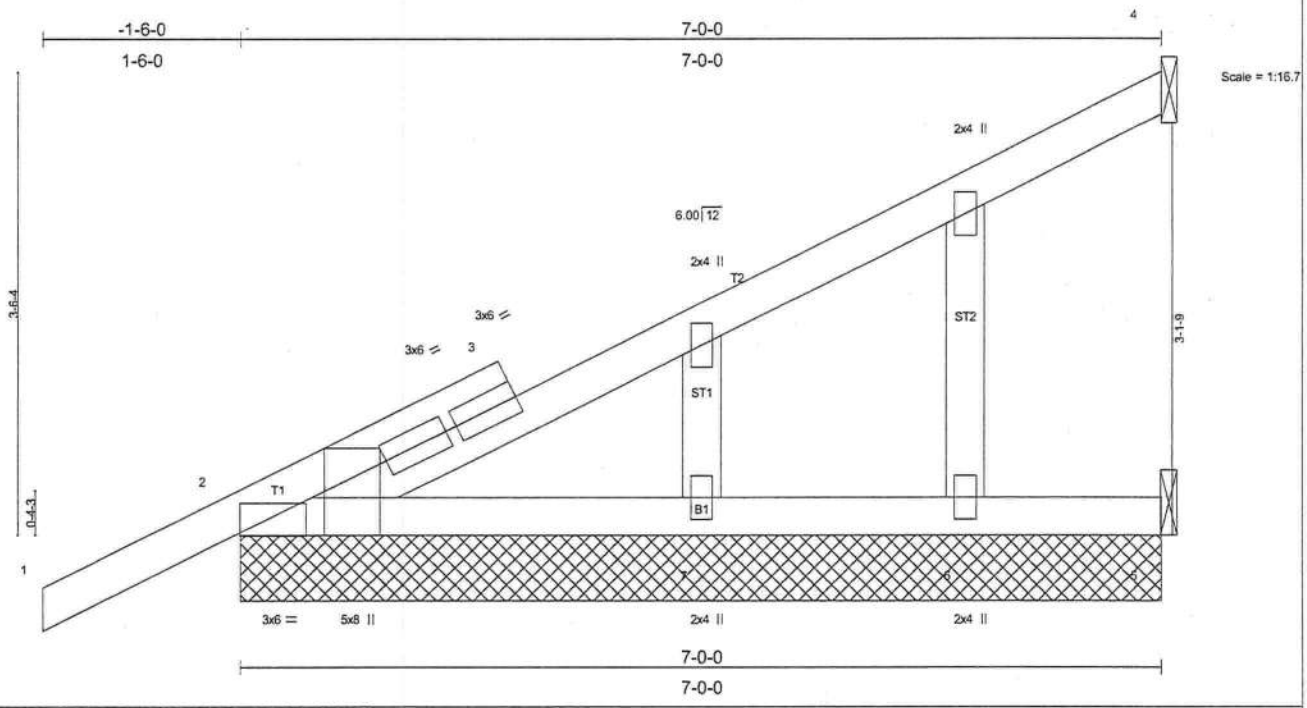

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [3:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.04	2-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	0.03	2-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except* T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/7-0-0, 4=146/Mechanical, 5=35/Mechanical, 7=243/7-0-0, 6=-27/7-0-0
Max Horz 2=229(load case 5)
Max Uplift2=-175(load case 5), 4=-154(load case 5), 5=-12(load case 5), 7=-116(load case 5), 6=-27(load case 1)
Max Grav 2=266(load case 1), 4=146(load case 1), 5=35(load case 1), 7=243(load case 1), 6=73(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-146/0, 3-4=-112/49
BOT CHORD 2-7=0/0, 6-7=0/0, 5-6=0/0

JOINT STRESS INDEX
2 = 0.74, 2 = 0.00, 3 = 0.00, 3 = 0.57, 3 = 0.57, 6 = 0.00, 7 = 0.00, 8 = 0.00 and 9 = 0.00

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Gable studs spaced at 2-0-0 oc.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 175 lb uplift at joint 2, 154 lb uplift at joint 4, 12 lb uplift at joint 5, 116 lb uplift at joint 7 and 27 lb uplift at joint 6.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061098

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	EJ7GA	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:14 2006 Page 1

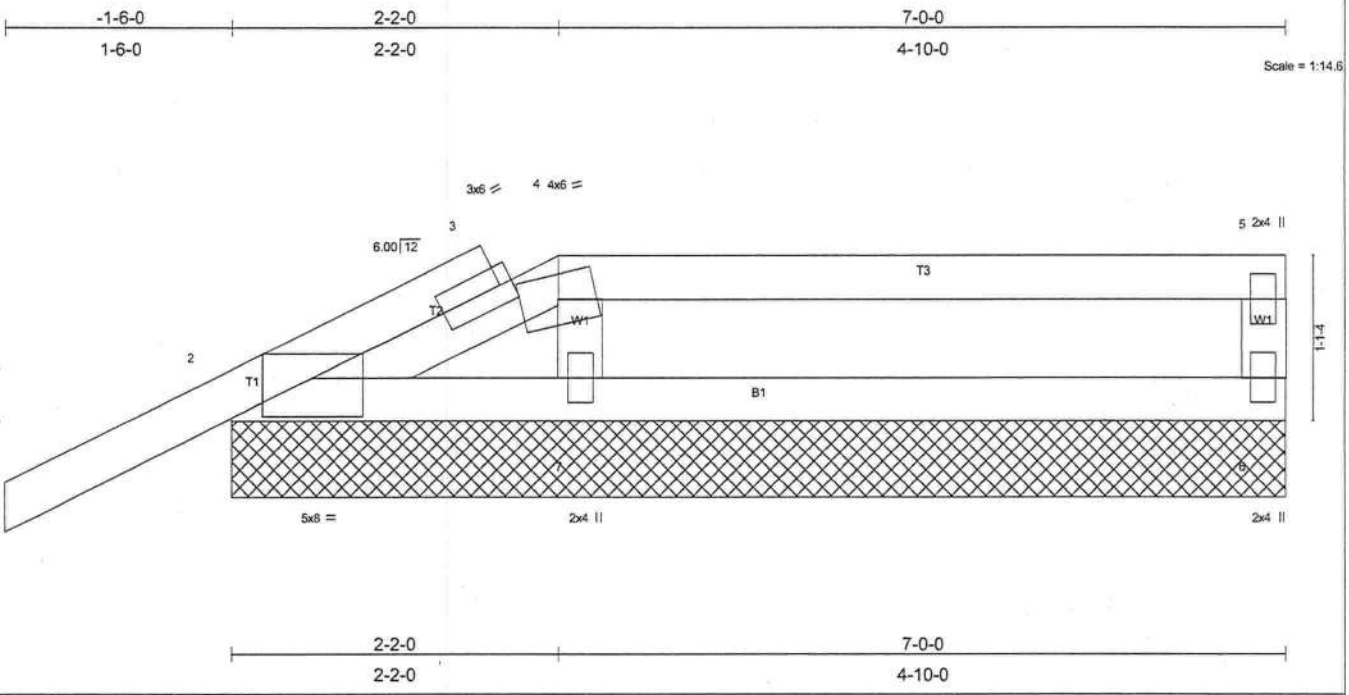

Plate Offsets (X,Y): [2:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.01	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.10	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 27 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=198/7-0-0, 6=204/7-0-0, 7=464/7-0-0
Max Horz 2=99(load case 5)
Max Uplift2=-196(load case 5), 6=-100(load case 3), 7=-206(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8/51, 2-3=-77/28, 3-4=-10/10, 4-5=-0/0
BOT CHORD 2-7=-70/60, 6-7=0/0
WEBS 4-7=-338/393, 5-6=-148/173

JOINT STRESS INDEX
2 = 0.55, 3 = 0.00, 3 = 0.23, 4 = 0.74, 5 = 0.10, 6 = 0.10 and 7 = 0.22

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 196 lb uplift at joint 2, 100 lb uplift at joint 6 and 206 lb uplift at joint 7.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-79(F=-25), 4-5=-79(F=-25), 2-6=-30

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061099

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	HJ9	MONO TRUSS	4	1	Job Reference (optional)

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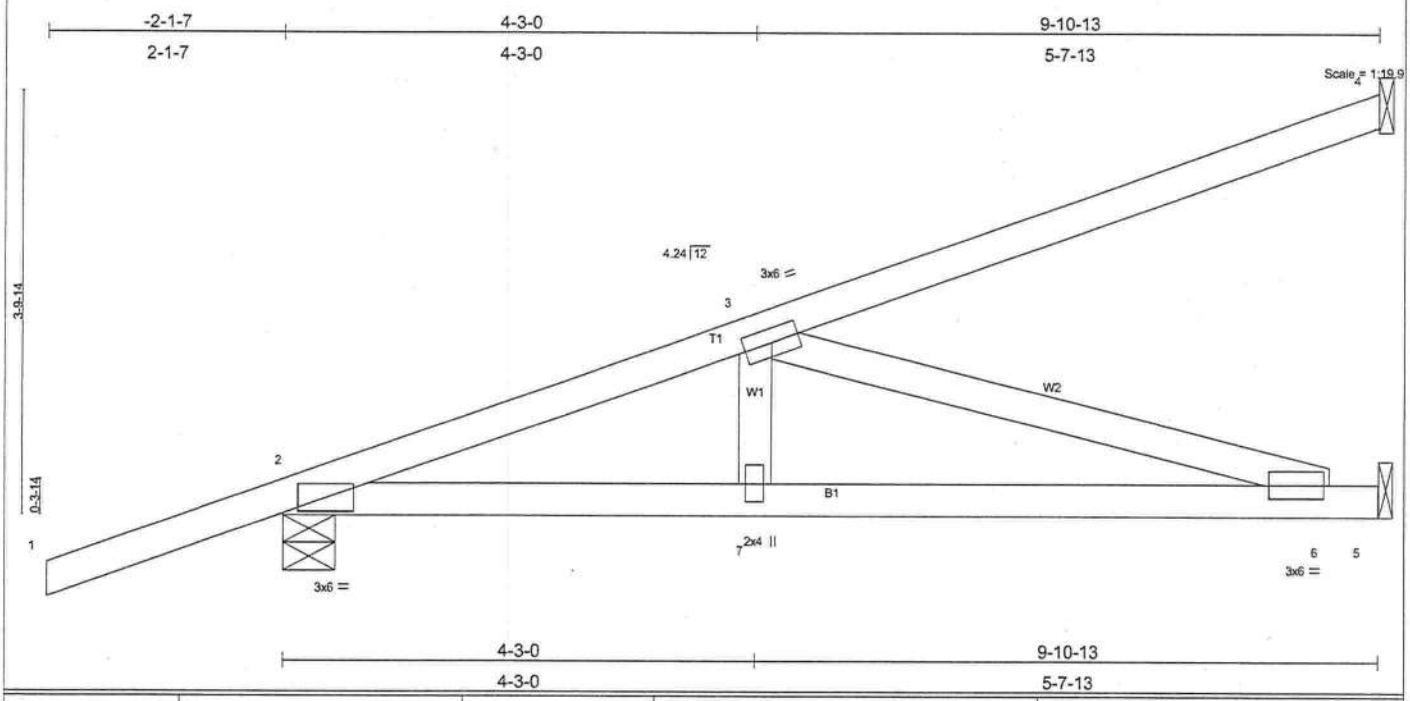

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.18	6-7	>626	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.50	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-6-11 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=486/0-5-11, 5=386/Mechanical
Max Horz 2=303(load case 2)
Max Uplift4=-289(load case 2), 2=-285(load case 2), 5=-107(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-934/246, 3-4=-124/65
BOT CHORD 2-7=-475/869, 6-7=-475/869, 5-6=0/0
WEBS 3-7=0/208, 3-6=-906/495

JOINT STRESS INDEX
2 = 0.72, 3 = 0.24, 6 = 0.26 and 7 = 0.15

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 289 lb uplift at joint 4, 285 lb uplift at joint 2 and 107 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

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Dwg.#0119061100

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T01	COMMON	2	1	Job Reference (optional)

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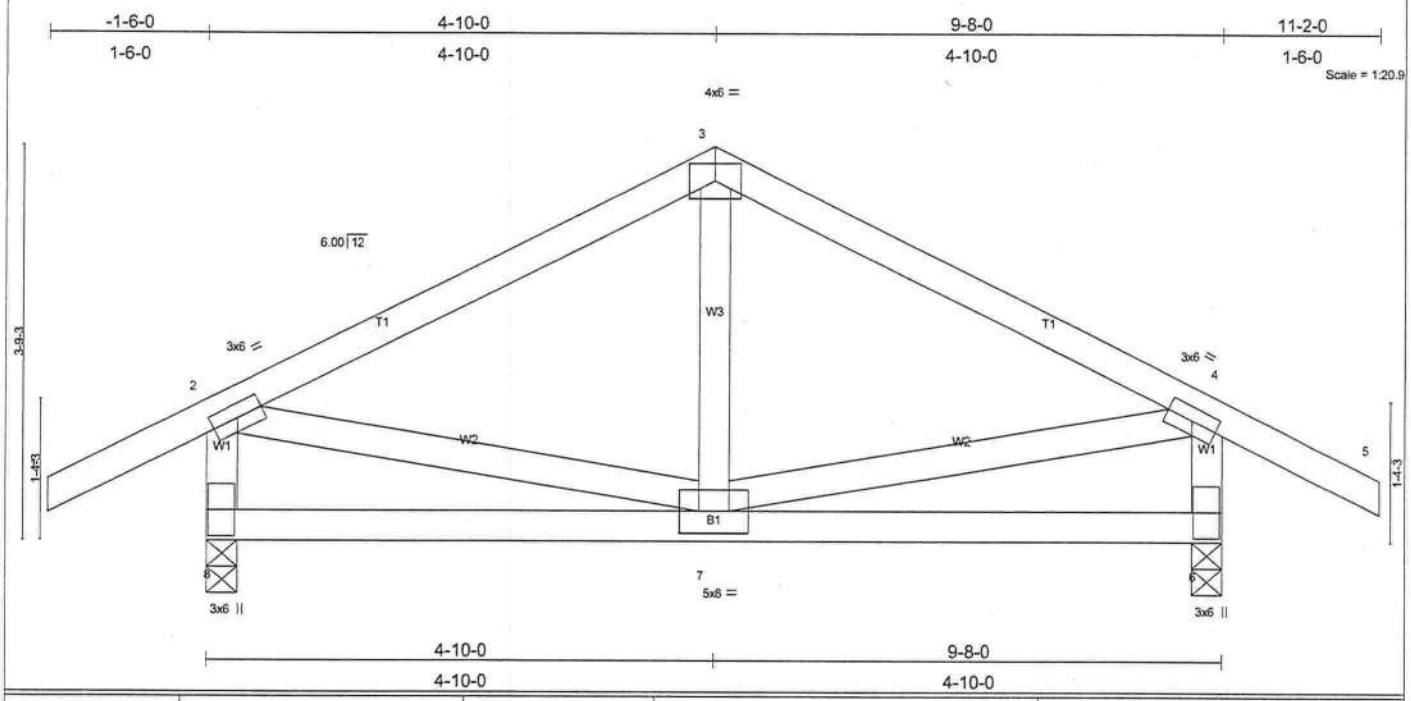

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.03	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	0.03	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 56 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=483/0-3-8, 6=483/0-3-8
Max Horz 8=87(load case 4)
Max Uplift8=-417(load case 5), 6=-417(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-380/578, 3-4=-380/578, 4-5=0/40, 2-8=-413/604, 4-6=-413/604
BOT CHORD 7-8=-134/83, 6-7=-71/83
WEBS 3-7=-223/67, 2-7=-219/222, 4-7=-219/222

JOINT STRESS INDEX
2 = 0.40, 3 = 0.56, 4 = 0.40, 6 = 0.41, 7 = 0.11 and 8 = 0.41

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 417 lb uplift at joint 8 and 417 lb uplift at joint 6.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
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Dwg.#0119061101

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T01G	COMMON	1	1	Job Reference (optional)

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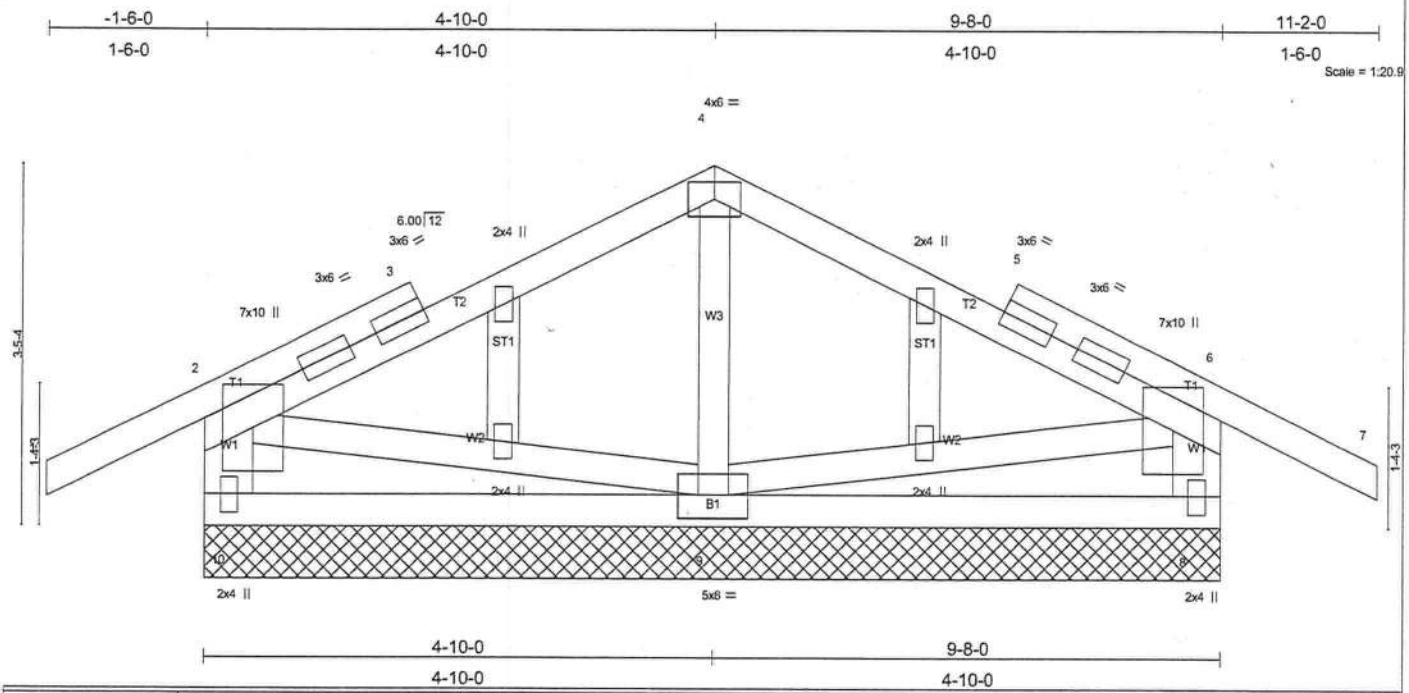

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.01	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.07	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 67 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 6 SYP No.1D, W1 2 X 6 SYP No.1D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 10=401/9-8-0, 8=401/9-8-0, 9=474/9-8-0
Max Horz 10=79(load case 4)
Max Uplift10=-266(load case 5), 8=-269(load case 6), 9=-186(load case 5)
Max Grav 10=405(load case 9), 8=405(load case 10), 9=474(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/51, 2-3=-118/80, 3-4=-41/76, 4-5=-41/76, 5-6=-118/80, 6-7=-6/51, 2-10=-338/386, 6-8=-338/386
BOT CHORD 9-10=-109/151, 8-9=-36/151
WEBS 4-9=-292/263, 2-9=-120/163, 6-9=-120/163

JOINT STRESS INDEX
2 = 0.66, 3 = 0.00, 3 = 0.37, 3 = 0.37, 4 = 0.58, 5 = 0.00, 5 = 0.37, 5 = 0.37, 6 = 0.66, 8 = 0.52, 9 = 0.06, 10 = 0.52, 11 = 0.00, 12 = 0.00, 13 = 0.00 and 14 = 0.00

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 10, 269 lb uplift at joint 8 and 186 lb uplift at joint 9.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-79(F=-25), 2-4=-79(F=-25), 4-6=-79(F=-25), 6-7=-79(F=-25), 8-10=-30

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
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Dwg.#0119061102

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T02	HIP	1	1	Job Reference (optional)

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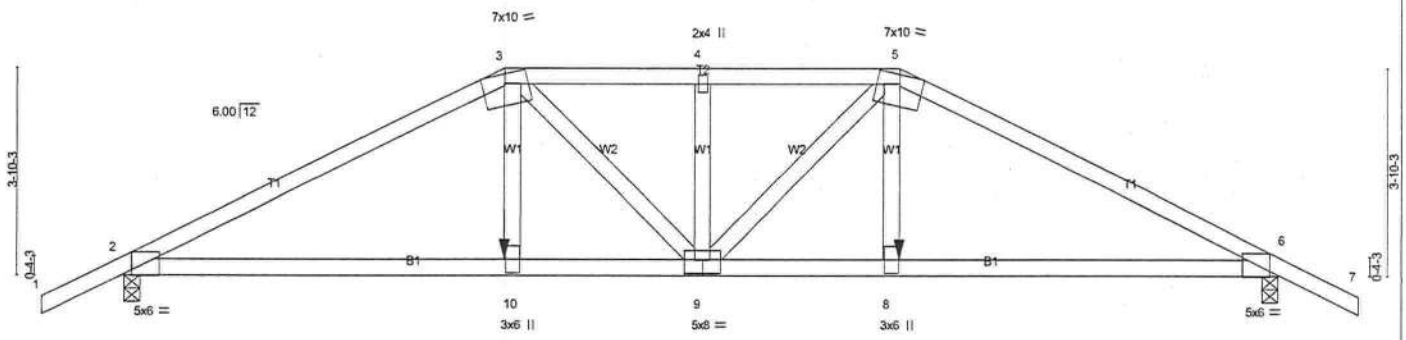
-1-6-0	3-9-4	7-0-0	10-8-0	14-4-0	17-6-12	21-4-0	22-10-0
1-6-0	3-9-4	3-2-12	3-8-0	3-8-0	3-2-12	3-9-4	1-6-0

Scale = 1:40.8



3-9-4	7-0-0	10-8-0	14-4-0	17-6-12	21-4-0
3-9-4	3-2-12	3-8-0	3-8-0	3-2-12	3-9-4

Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-1-11,Edge], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.16	6-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.26	6-8	>955	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 100 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-3-8 oc bracing.

REACTIONS (lb/size) 2=1841/0-3-8, 6=1841/0-3-8
Max Horz 2=-92(load case 5)
Max Uplift2=-978(load case 4), 6=-978(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-3339/1619, 3-4=-3176/1615, 4-5=-3176/1615, 5-6=-3339/1619, 6-7=0/35
BOT CHORD 2-10=-1368/2897, 9-10=-1380/2930, 8-9=-1327/2930, 6-8=-1316/2897
WEBS 3-10=-291/803, 3-9=-315/472, 4-9=-365/397, 5-9=-316/472, 5-8=-291/803

JOINT STRESS INDEX
2 = 0.83, 3 = 0.71, 4 = 0.34, 5 = 0.71, 6 = 0.83, 8 = 0.26, 9 = 0.64 and 10 = 0.26

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 978 lb uplift at joint 2 and 978 lb uplift at joint 6.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 321 lb up at 14-4-0, and 539 lb down and 321 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-113(F=-58), 5-7=-54, 2-10=-30, 8-10=-62(F=-33), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-539(F) 8=-539(F)

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
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Dwg.#0119061103

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T03	HIP	1	1	Job Reference (optional)

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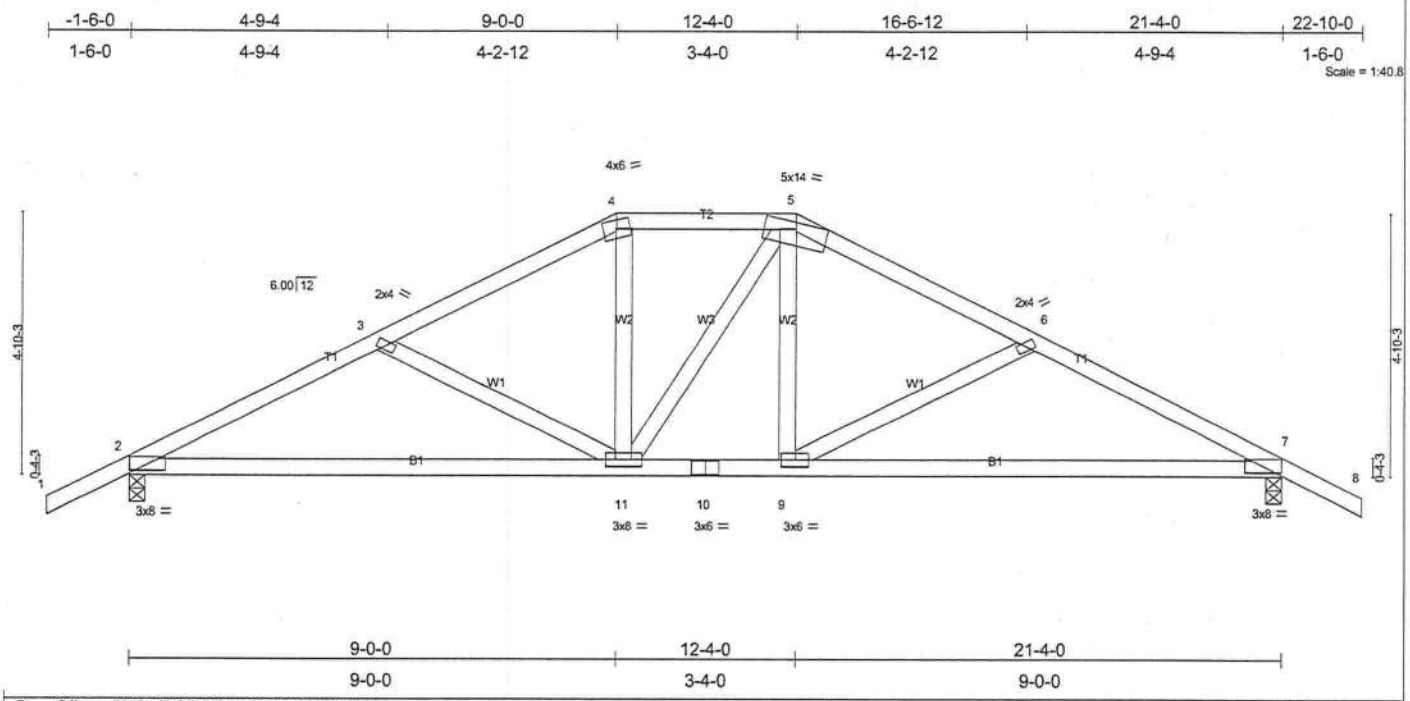

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [7:0-8-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.18	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.30	7-9	>829	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 107 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-7 oc bracing.

REACTIONS (lb/size) 2=973/0-3-8, 7=973/0-3-8
Max Horz 2=-109(load case 6)
Max Uplift2=-475(load case 5), 7=-475(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1468/815, 3-4=-1199/657, 4-5=-1030/648, 5-6=-1198/657, 6-7=-1468/815, 7-8=0/35
BOT CHORD 2-11=-568/1277, 10-11=-304/1028, 9-10=-304/1028, 7-9=-568/1277
WEBS 3-11=-291/300, 4-11=-100/314, 5-11=-102/106, 5-9=-100/315, 6-9=-293/300

JOINT STRESS INDEX
2 = 0.73, 3 = 0.34, 4 = 0.44, 5 = 0.32, 6 = 0.34, 7 = 0.73, 9 = 0.35, 10 = 0.60 and 11 = 0.59

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 475 lb uplift at joint 2 and 475 lb uplift at joint 7.

LOAD CASE(S) Standard

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Dwg.#0119061104

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T04	COMMON	2	1	Job Reference (optional)

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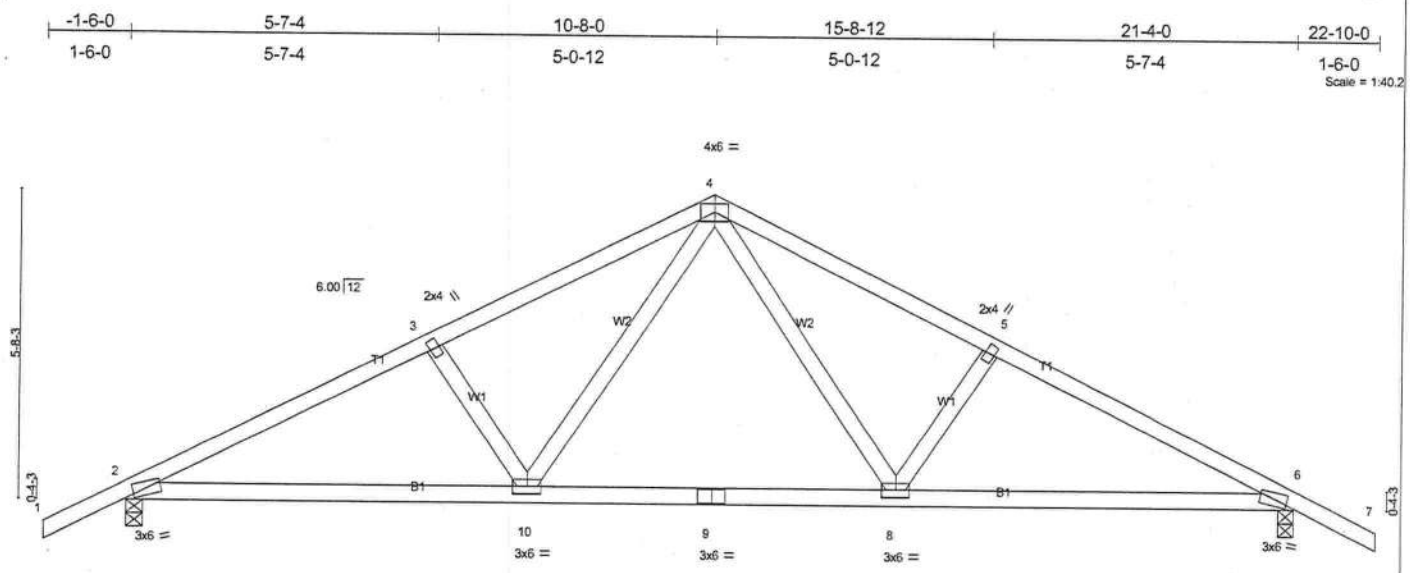

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.19	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.31	8-10	>821	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 100 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-10 oc bracing.

REACTIONS (lb/size) 2=1141/0-3-8, 6=1141/0-3-8
Max Horz 2=-123(load case 6)
Max Uplift2=-568(load case 5), 6=-568(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1888/1014, 3-4=-1732/1007, 4-5=-1732/1007, 5-6=-1888/1014, 6-7=0/35
BOT CHORD 2-10=-732/1619, 9-10=-383/1112, 8-9=-383/1112, 6-8=-732/1619
WEBS 3-10=-250/291, 4-10=-365/734, 4-8=-364/734, 5-8=-250/291

JOINT STRESS INDEX
2 = 0.79, 3 = 0.34, 4 = 0.66, 5 = 0.34, 6 = 0.79, 8 = 0.56, 9 = 0.83 and 10 = 0.56

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 568 lb uplift at joint 2 and 568 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061105

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T05	COMMON	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:19 2006 Page 1

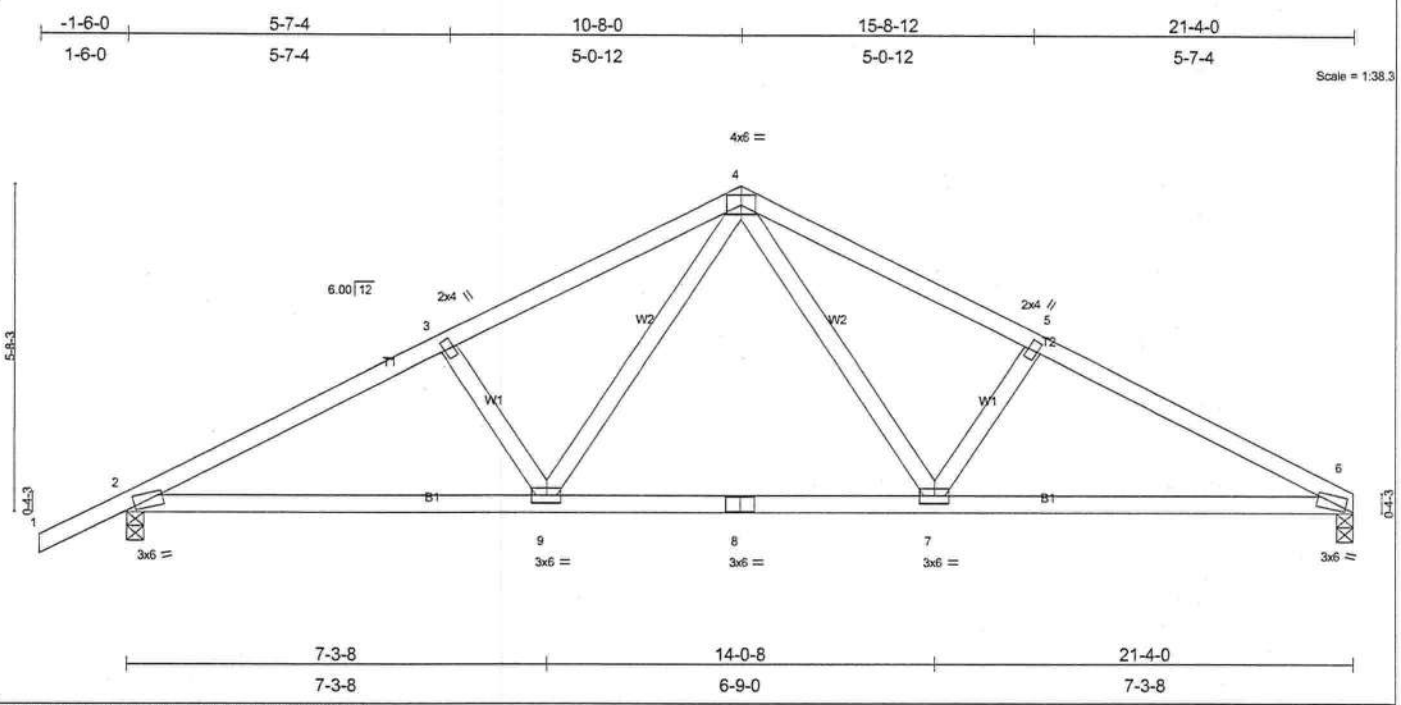

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.19	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.30	7-9	>829	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.29	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 97 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-7-13 oc bracing.

REACTIONS (lb/size) 6=1049/0-3-8, 2=1145/0-3-8
Max Horz 2=144(load case 5)
Max Uplift6=-450(load case 6), 2=-570(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1895/1028, 3-4=-1739/1020, 4-5=-1753/1046, 5-6=-1895/1055
BOT CHORD 2-9=-813/1625, 8-9=-465/1119, 7-8=-465/1119, 6-7=-845/1642
WEBS 3-9=-250/292, 4-9=-361/734, 4-7=-402/753, 5-7=-260/311

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.64, 5 = 0.34, 6 = 0.80, 7 = 0.57, 8 = 0.83 and 9 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 450 lb uplift at joint 6 and 570 lb uplift at joint 2.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-9=-30, 7-9=-80(F=-50), 6-7=-30

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
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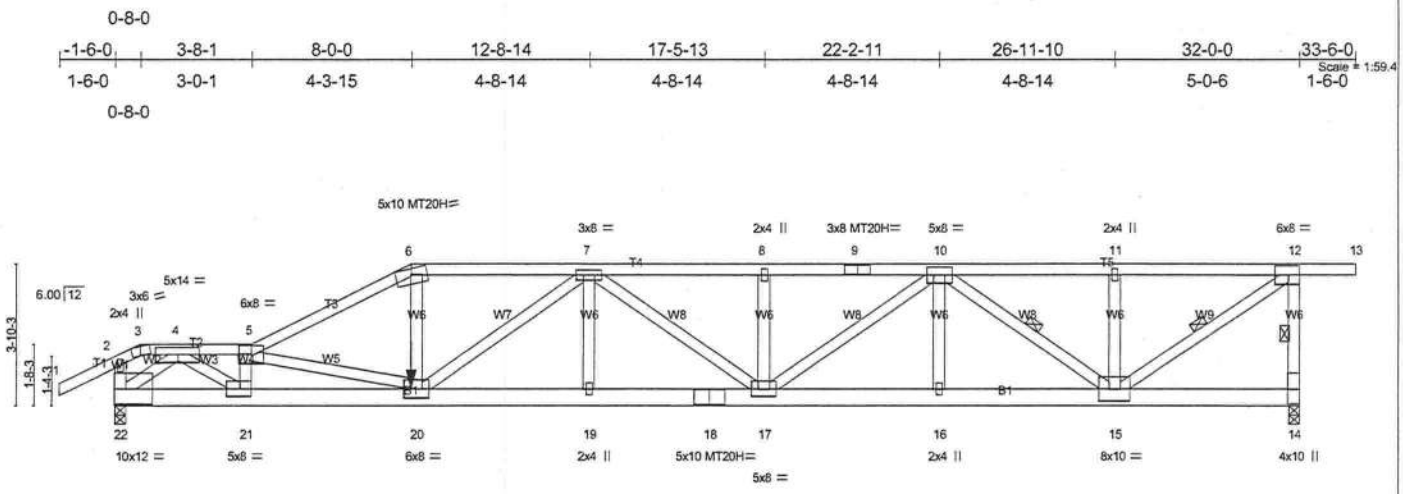
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061106

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T06	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:20 2006 Page 1



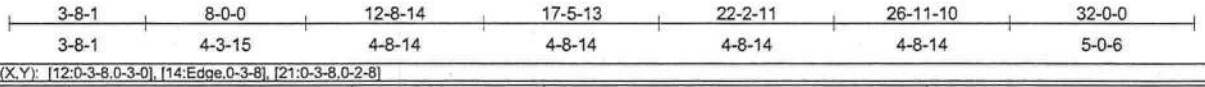


Plate Offsets (X,Y): [12:0-3-8,0-3-0], [14:Edge,0-3-8], [21:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	0.47	17-19	>815	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.74	17-19	>516	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.83	Horz(TL)	0.14	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 211 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2, W9 2 X 4 SYP No.2, W1 2 X 4 SYP No.1D, W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-4-6 oc bracing.
WEBS 1 Row at midpt 12-14, 10-15, 12-15

REACTIONS (lb/size) 14=2929/0-3-8, 22=2694/0-3-8
Max Horz 22=212(load case 3)
Max Uplift14=-1721(load case 3), 22=-1425(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-258/153, 3-4=-213/141, 4-5=-5971/3094, 5-6=-5822/3158, 6-7=-5286/2918, 7-8=-6561/3661, 8-9=-6561/3661, 9-10=-6561/3661, 10-11=-3428/1957, 11-12=-3428/1957, 12-13=0/0, 12-14=-2746/1707, 2-22=-237/227
BOT CHORD 21-22=-1622/2810, 20-21=-3393/6238, 19-20=-3685/6468, 18-19=-3685/6468, 17-18=-3685/6468, 16-17=-3192/5592, 15-16=-3192/5592, 14-15=-93/83
WEBS 4-21=-1986/3865, 5-21=-2399/1313, 5-20=-1087/597, 6-20=-1077/2146, 7-20=-1483/917, 7-19=0/295, 7-17=-98/115, 8-17=-521/519, 10-17=-671/1201, 10-16=0/334, 10-15=-2683/1530, 11-15=-535/546, 12-15=-2289/4104, 4-22=-3424/1799

JOINT STRESS INDEX
2 = 0.44, 3 = 0.32, 4 = 0.95, 5 = 0.98, 6 = 0.99, 7 = 0.72, 8 = 0.34, 9 = 0.93, 10 = 0.68, 11 = 0.34, 12 = 0.79, 14 = 0.57, 15 = 0.97, 16 = 0.34, 17 = 0.56, 18 = 0.90, 19 = 0.34, 20 = 0.54, 21 = 0.88 and 22 = 0.58

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1721 lb uplift at joint 14 and 1425 lb uplift at joint 22.
6) Girder carries hip end with 0-0-0 right side setback, 8-0-0 left side setback, and 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 616 lb down and 397 lb up at 8-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 3-5=-54, 5-6=-54, 6-12=-113(F=-58), 12-13=-54, 20-22=-30, 14-20=-62(F=-33)
Concentrated Loads (lb)
Vert: 20=-616(F)

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

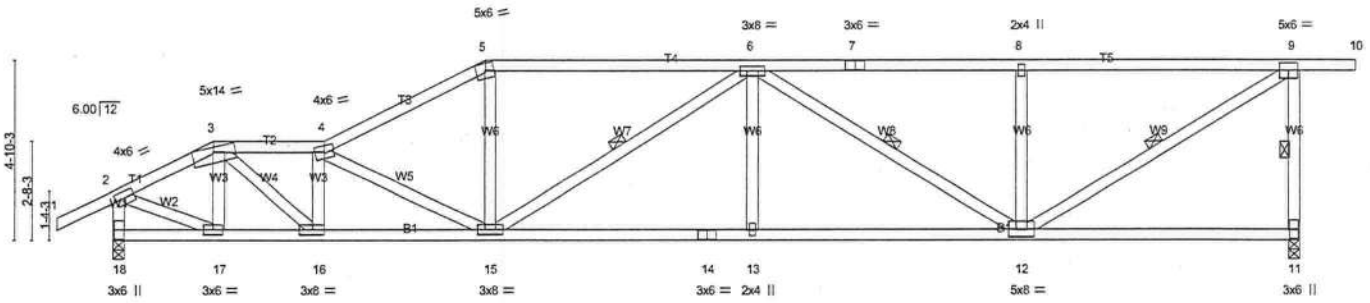
Dwg.#0119061107

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T07	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:21 2006 Page 1

-1-6-0	2-8-0	5-8-1	10-0-0	17-2-13	24-5-11	32-0-0	33-6-0
1-6-0	2-8-0	3-0-1	4-3-15	7-2-13	7-2-13	7-6-5	1-6-0

Scale = 1:59.4



2-8-0	5-8-1	10-0-0	17-2-13	24-5-11	32-0-0
2-8-0	3-0-1	4-3-15	7-2-13	7-2-13	7-6-5

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [16:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.21	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.34	13-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.08	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 186 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-7-8 oc bracing.
WEBS 1 Row at midpt 9-11, 6-15, 6-12, 9-12

REACTIONS (lb/size) 11=1421/0-3-8, 18=1421/0-3-8
Max Horz 18=271(load case 4)
Max Uplift11=-742(load case 4), 18=-621(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1512/787, 3-4=-2456/1310, 4-5=-2287/1211, 5-6=-2047/1162, 6-7=-1719/966, 7-8=-1719/966, 8-9=-1719/966, 9-10=0/0, 9-11=-1311/768, 2-18=-1371/836
BOT CHORD 17-18=-252/81, 16-17=-654/1289, 15-16=-1273/2510, 14-15=-1138/2363, 13-14=-1138/2363, 12-13=-1138/2363, 11-12=-69/76
WEBS 3-17=-457/260, 3-16=-781/1518, 4-16=-898/533, 4-15=-549/362, 5-15=-229/659, 6-15=-376/285, 6-13=0/216, 6-12=-762/399, 8-12=-399/351, 9-12=-987/1953, 2-17=-655/1391

JOINT STRESS INDEX
2 = 0.71, 3 = 0.53, 4 = 0.56, 5 = 0.65, 6 = 0.57, 7 = 0.49, 8 = 0.34, 9 = 0.63, 11 = 0.40, 12 = 0.90, 13 = 0.34, 14 = 0.79, 15 = 0.57, 16 = 0.83, 17 = 0.79 and 18 = 0.28

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 742 lb uplift at joint 11 and 621 lb uplift at joint 18.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

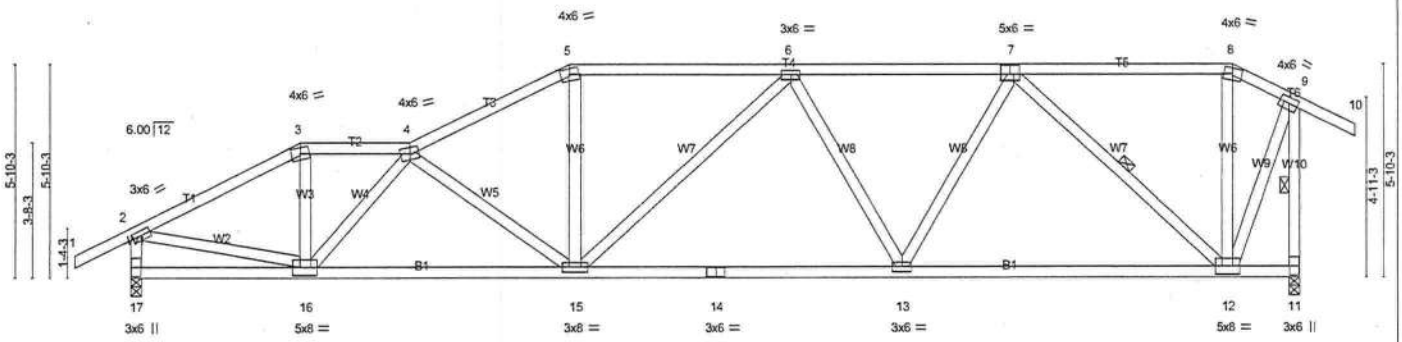
Dwg.#0119061108

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T08	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:21 2006 Page 1

-1-6-0	4-8-0	7-8-1	12-0-0	18-0-11	24-1-5	30-2-0	32-0-0	33-6-0
1-6-0	4-8-0	3-0-1	4-3-15	6-0-11	6-0-11	6-0-11	1-10-0	1-6-0

Scale = 1:60.2



4-8-0	7-8-1	12-0-0	21-1-0	30-2-0	32-0-0
4-8-0	3-0-1	4-3-15	9-1-0	9-1-0	1-10-0

Plate Offsets (X,Y): [7:0-2-12,0-3-0], [9:0-2-15,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.20	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.33	13-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.48	Horz(TL)	0.08	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 200 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 7-12, 9-11

REACTIONS (lb/size) 17=1421/0-3-8, 11=1421/0-3-8
Max Horz 17=262(load case 4)
Max Uplift17=-643(load case 5), 11=-597(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1802/939, 3-4=-1580/903, 4-5=-2057/1142, 5-6=-1831/1095, 6-7=-1715/1011, 7-8=-446/392, 8-9=-504/434, 9-10=0/40, 2-17=-1351/849, 9-11=-1473/843
BOT CHORD 16-17=-276/111, 15-16=-1060/2219, 14-15=-893/1871, 13-14=-893/1871, 12-13=-700/1406, 11-12=-51/113
WEBS 3-16=-235/579, 4-16=-990/566, 4-15=-507/389, 5-15=-230/597, 6-15=-193/189, 6-13=-327/274, 7-13=-210/645, 7-12=-1318/711, 8-12=-55/115, 2-16=-660/1485, 9-12=-512/1234

JOINT STRESS INDEX
2 = 0.76, 3 = 0.43, 4 = 0.52, 5 = 0.79, 6 = 0.45, 7 = 0.63, 8 = 0.52, 9 = 0.74, 11 = 0.42, 12 = 0.71, 13 = 0.52, 14 = 0.84, 15 = 0.57, 16 = 0.71 and 17 = 0.30

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 643 lb uplift at joint 17 and 597 lb uplift at joint 11.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061109

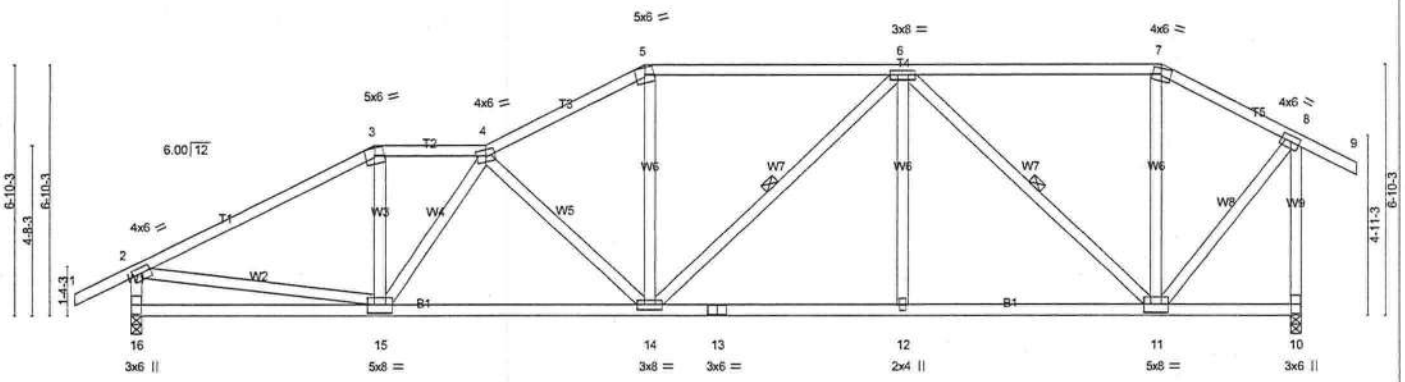
Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:22 2006 Page 1

-1-6-0 | 6-8-0 | 9-8-1 | 14-0-0 | 21-1-0 | 28-2-0 | 32-0-0 | 33-6-0

1-6-0 | 6-8-0 | 3-0-1 | 4-3-15 | 7-1-0 | 7-1-0 | 3-10-0 | 1-6-0

Scale = 1:60.2



6-8-0 | 9-8-1 | 14-0-0 | 21-1-0 | 28-2-0 | 32-0-0

6-8-0 | 3-0-1 | 4-3-15 | 7-1-0 | 7-1-0 | 3-10-0

Plate Offsets (X,Y): [2:0-3-0,0-1-12], [8:0-2-15,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.96	Vert(LL)	-0.15	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.24	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.06	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 207 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-14, 6-11

REACTIONS (lb/size) 16=1421/0-3-8, 10=1421/0-3-8
Max Horz 16=279(load case 4)
Max Uplift16=-660(load case 5), 10=-540(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1894/998, 3-4=-1634/980, 4-5=-1825/1071, 5-6=-1622/1034, 6-7=-709/569, 7-8=-824/584, 8-9=0/40, 2-16=-1317/863, 8-10=-1385/856
BOT CHORD 15-16=-335/254, 14-15=-912/1990, 13-14=-673/1489, 12-13=-673/1489, 11-12=-673/1489, 10-11=-42/117
WEBS 3-15=-162/522, 4-15=-652/367, 4-14=-529/394, 5-14=-171/463, 6-14=-141/192, 6-12=0/210, 6-11=-1087/546, 7-11=-11/96, 2-15=-575/1382, 8-11=-453/1123

JOINT STRESS INDEX
2 = 0.80, 3 = 0.61, 4 = 0.46, 5 = 0.63, 6 = 0.57, 7 = 0.69, 8 = 0.69, 10 = 0.43, 11 = 0.55, 12 = 0.34, 13 = 0.52, 14 = 0.57, 15 = 0.63 and 16 = 0.47

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 660 lb uplift at joint 16 and 540 lb uplift at joint 10.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

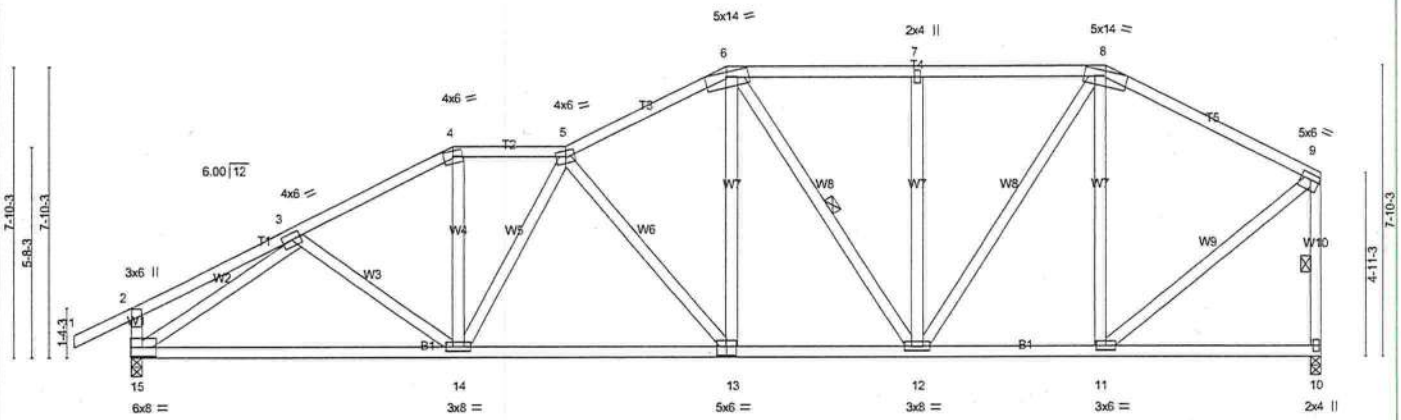
Dwg.#0119061110

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:23 2006 Page 1

-1-6-0	4-5-12	8-8-0	11-8-1	16-0-0	21-1-0	26-2-0	32-0-0
1-6-0	4-5-12	4-2-4	3-0-1	4-3-15	5-1-0	5-1-0	5-10-0

Scale = 1:59.1



8-8-0	11-8-1	16-0-0	21-1-0	26-2-0	32-0-0
8-8-0	3-0-1	4-3-15	5-1-0	5-1-0	5-10-0

Plate Offsets (X,Y): [9:Edge,0-1-12], [13:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.13	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.22	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.06	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 220 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-0 oc bracing.
WEBS 1 Row at midpt 6-12, 9-10

REACTIONS (lb/size) 15=1423/0-3-8, 10=1329/0-3-8
Max Horz 15=328(load case 4)
Max Uplift15=-672(load case 5), 10=-460(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-324/153, 3-4=-1812/1009, 4-5=-1591/967, 5-6=-1620/987, 6-7=-1270/867, 7-8=-1270/867, 8-9=-1030/652, 2-15=-343/328, 9-10=-1242/722
BOT CHORD 14-15=-785/1472, 13-14=-853/1801, 12-13=-583/1424, 11-12=-355/853, 10-11=-54/77
WEBS 3-14=-57/166, 4-14=-255/565, 5-14=-446/289, 5-13=-610/447, 6-13=-309/654, 6-12=-295/183, 7-12=-281/245, 8-12=-381/790, 8-11=-495/316, 3-15=-1558/899, 9-11=-457/1059

JOINT STRESS INDEX
2 = 0.26, 3 = 0.46, 4 = 0.50, 5 = 0.49, 6 = 0.58, 7 = 0.34, 8 = 0.73, 9 = 0.70, 10 = 0.87, 11 = 0.64, 12 = 0.83, 13 = 0.58, 14 = 0.61 and 15 = 0.32

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 672 lb uplift at joint 15 and 460 lb uplift at joint 10.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

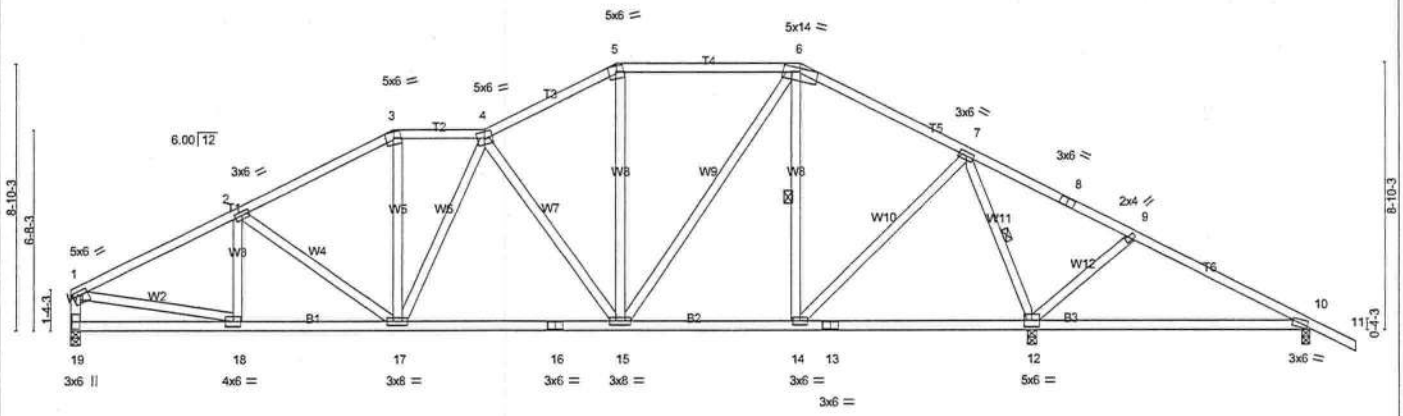
Dwg.#0119061111

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T11	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:24 2006 Page 1

5-5-12	10-8-0	13-8-1	18-0-0	24-2-0	29-7-13	35-1-11	41-2-0	42-8-0
5-5-12	5-2-4	3-0-1	4-3-15	6-2-0	5-5-13	5-5-13	6-0-5	1-6-0

Scale = 1:73.1



5-5-12	10-8-0	13-8-1	18-0-0	24-2-0	31-10-4	41-2-0
5-5-12	5-2-4	3-0-1	4-3-15	6-2-0	7-8-4	9-3-12

Plate Offsets (X,Y): [1:Edge,0-1-12], [10:0-1-5,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	0.41	10-12	>266	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	0.35	10-12	>311	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.04	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 251 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-14, 7-12

REACTIONS (lb/size) 19=1256/0-3-8, 12=2048/0-3-8, 10=219/0-3-8
Max Horz 19=-209(load case 3)
Max Uplift19=-538(load case 5), 12=-912(load case 6), 10=-345(load case 6)
Max Grav 19=1256(load case 1), 12=2048(load case 1), 10=281(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1738/945, 2-3=-1598/939, 3-4=-1382/910, 4-5=-1242/834, 5-6=-1080/814, 6-7=-883/613, 7-8=-159/591, 8-9=-178/469, 9-10=-80/361, 10-11=0/35, 1-19=-1162/677
BOT CHORD 18-19=-193/220, 17-18=-653/1495, 16-17=-515/1448, 15-16=-515/1448, 14-15=-114/725, 13-14=0/174, 12-13=0/174, 10-12=-291/161
WEBS 2-18=-134/163, 2-17=-159/210, 3-17=-197/442, 4-17=-163/131, 4-15=-648/488, 5-15=-96/226, 6-15=-342/659, 6-14=-460/306, 7-14=-301/899, 7-12=-1634/893, 9-12=-351/440, 1-18=-604/1303

JOINT STRESS INDEX
1 = 0.64, 2 = 0.41, 3 = 0.42, 4 = 0.31, 5 = 0.51, 6 = 0.81, 7 = 0.68, 8 = 0.24, 9 = 0.34, 10 = 0.91, 12 = 0.38, 13 = 0.15, 14 = 0.57, 15 = 0.69, 16 = 0.59, 17 = 0.63, 18 = 0.57 and 19 = 0.39

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 538 lb uplift at joint 19, 912 lb uplift at joint 12 and 345 lb uplift at joint 10.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

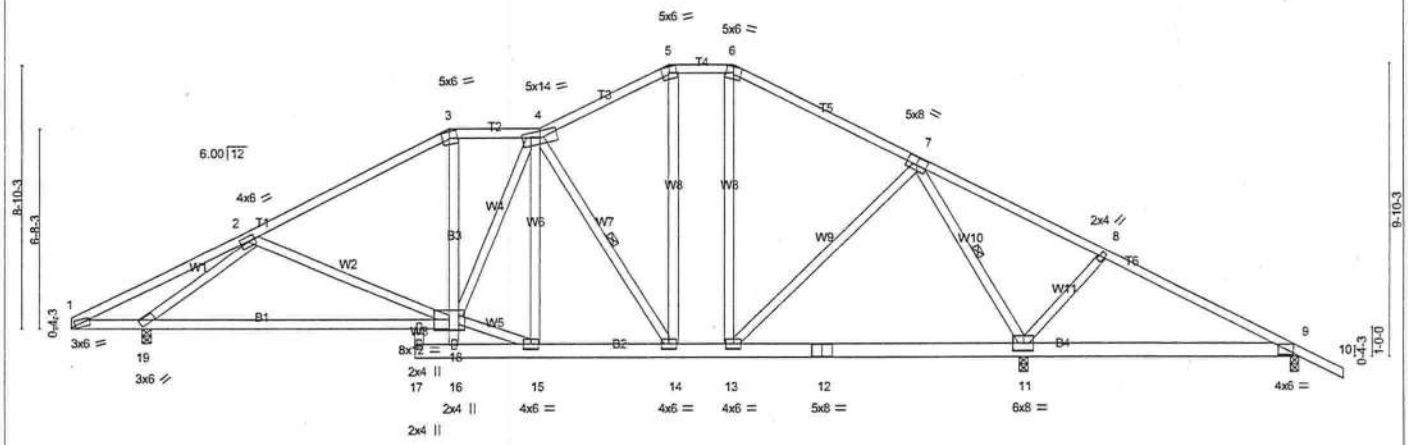
Dwg.#0119061112

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:25 2006 Page 1

6-0-4	11-6-0	12-8-0	15-8-1	20-0-0	22-2-0	28-3-13	34-5-11	41-2-0	42-8-0
6-0-4	5-5-12	1-2-0	3-0-1	4-3-15	2-2-0	6-1-13	6-1-13	6-8-5	1-6-0

Scale = 1:73.8



2-5-12	11-6-0	12-8-0	15-8-1	20-0-0	22-2-0	31-10-4	41-2-0
2-5-12	9-0-4	1-2-0	3-0-1	4-3-15	2-2-0	9-8-4	9-3-12

Plate Offsets (X,Y): [1:0-1-8,0-0-7], [7:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.14	9-11	>768	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	0.12	9-11	>888	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 280 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B1 2 X 4 SYP No.2, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-14, 7-11

REACTIONS (lb/size) 11=1806/0-3-8, 9=343/0-3-8, 19=1424/0-3-8
Max Horz 19=-254(load case 6)
Max Uplift11=-872(load case 6), 9=-373(load case 6), 19=-686(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-303/85, 2-3=-1566/836, 3-4=-1310/825, 4-5=-977/724, 5-6=-827/705, 6-7=-998/697, 7-8=-14/329, 8-9=-56/103, 9-10=0/39
BOT CHORD 1-19=-10/341, 18-19=-575/1252, 16-18=0/59, 3-18=-99/383, 16-17=0/0, 15-16=-231/0, 14-15=-380/1204, 13-14=-135/827, 12-13=-69/454, 11-12=-69/454, 9-11=-57/93
WEBS 2-18=-16/179, 15-18=-256/1446, 4-18=-46/282, 4-15=-230/28, 4-14=-758/515, 5-14=-238/318, 6-13=-56/215, 7-13=-92/529, 7-11=-1409/760, 8-11=-335/419, 2-19=-1640/1232

JOINT STRESS INDEX
1 = 0.91, 2 = 0.48, 3 = 0.62, 4 = 0.27, 5 = 0.21, 6 = 0.66, 7 = 0.45, 8 = 0.34, 9 = 0.26, 11 = 0.22, 12 = 0.13, 13 = 0.25, 14 = 0.30, 15 = 0.58, 16 = 0.34, 17 = 0.34, 18 = 0.39, 19 = 0.62 and 20 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 872 lb uplift at joint 11, 373 lb uplift at joint 9 and 686 lb uplift at joint 19.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061113

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T13	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:26 2006 Page 1

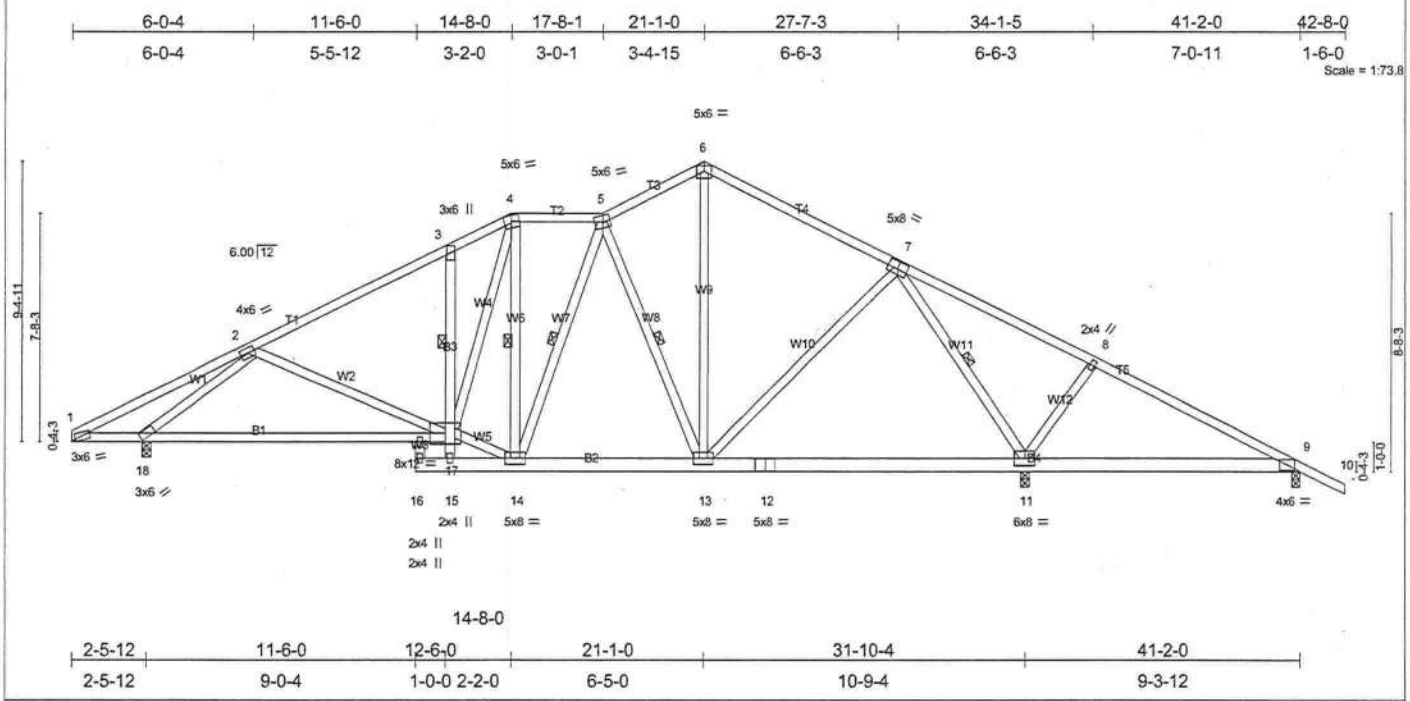


Plate Offsets (X,Y): [1:0-1-8,0-0-7], [7:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	0.14	9-11	>777	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	0.13	9-11	>876	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.65	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 283 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B1 2 X 4 SYP No.2, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 3-17
WEBS 1 Row at midpt 4-14, 5-14, 5-13, 7-11

REACTIONS (lb/size) 11=1888/0-3-8, 9=279/0-3-8, 18=1401/0-3-8
Max Horz 18=-263(load case 6)
Max Uplift11=-916(load case 6), 9=-347(load case 6), 18=-669(load case 5)
Max Grav 11=1888(load case 1), 9=337(load case 10), 18=1401(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-300/91, 2-3=-1525/808, 3-4=-1445/959, 4-5=-1007/756, 5-6=-883/678, 6-7=-943/659, 7-8=-120/451, 8-9=-112/273, 9-10=0/39
BOT CHORD 1-18=-15/339, 17-18=-563/1226, 15-17=0/41, 3-17=-254/349, 15-16=0/0, 14-15=-220/0, 13-14=-252/994, 12-13=-17/449, 11-12=-17/449, 9-11=-221/238
WEBS 2-17=0/185, 14-17=-134/1252, 4-17=-406/934, 4-14=-406/113, 5-14=-57/97, 5-13=-622/468, 6-13=-293/482, 7-13=-100/494, 7-11=-1467/835, 8-11=-343/431, 2-18=-1611/1204

JOINT STRESS INDEX
1 = 0.90, 2 = 0.47, 3 = 0.31, 4 = 0.54, 5 = 0.29, 6 = 0.55, 7 = 0.51, 8 = 0.34, 9 = 0.37, 11 = 0.22, 12 = 0.13, 13 = 0.30, 14 = 0.57, 15 = 0.39, 16 = 0.34, 17 = 0.73, 18 = 0.61 and 19 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 916 lb uplift at joint 11, 347 lb uplift at joint 9 and 669 lb uplift at joint 18.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061114

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T14	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:26 2006 Page 1

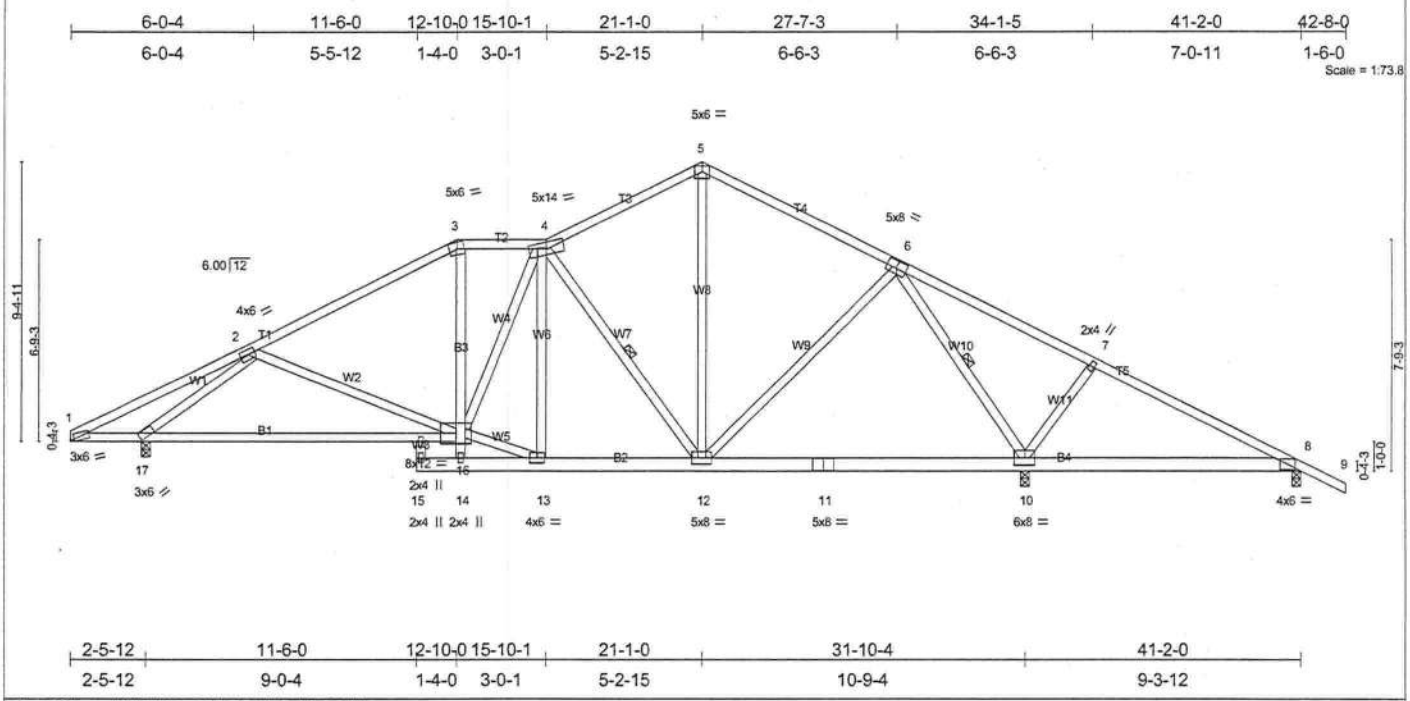


Plate Offsets (X,Y): [1:0-1-8,0-0-7], [6:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.14	8-10	>778	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	0.13	8-10	>877	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 270 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except* B1 2 X 4 SYP No.2, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-12, 6-10

REACTIONS (lb/size) 10=1903/0-3-8, 8=271/0-3-8, 17=1405/0-3-8
Max Horz 17=-263(load case 6)
Max Uplift10=-914(load case 6), 8=-348(load case 6), 17=-668(load case 5)
Max Grav 10=1903(load case 1), 8=329(load case 10), 17=1405(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-306/78, 2-3=-1527/801, 3-4=-1272/796, 4-5=-916/666, 5-6=-943/661, 6-7=-116/469, 7-8=-107/287, 8-9=0/39
BOT CHORD 1-17=-3/343, 16-17=-565/1231, 14-16=0/98, 3-16=-71/354, 14-15=0/0, 13-14=-239/0, 12-13=-344/1140, 11-12=-18/441, 10-11=-18/441, 8-10=-233/234
WEBS 2-16=-4/195, 13-16=-205/1367, 4-16=-73/364, 4-13=-360/77, 4-12=-657/484, 5-12=-253/433, 6-12=-105/510, 6-10=-1482/831, 7-10=-345/433, 2-17=-1602/1211

JOINT STRESS INDEX
1 = 0.90, 2 = 0.47, 3 = 0.66, 4 = 0.25, 5 = 0.64, 6 = 0.50, 7 = 0.34, 8 = 0.37, 10 = 0.22, 11 = 0.21, 12 = 0.28, 13 = 0.55, 14 = 0.38, 15 = 0.34, 16 = 0.39, 17 = 0.60 and 18 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 914 lb uplift at joint 10, 348 lb uplift at joint 8 and 668 lb uplift at joint 17.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

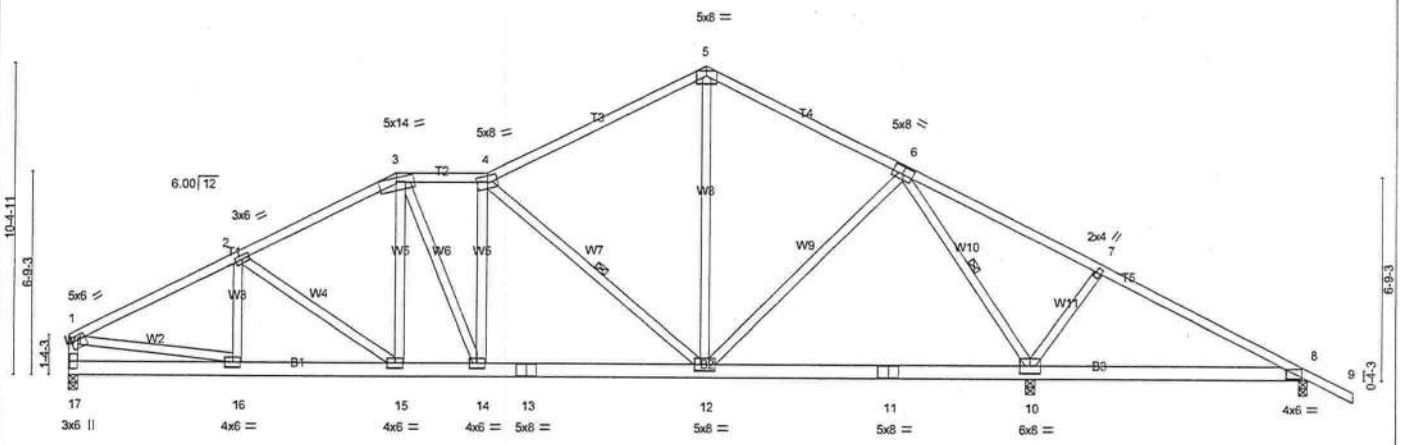
Dwg.#0119061115

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T15	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:27 2006 Page 1

5-6-12	10-10-0	13-10-1	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
5-6-12	5-3-4	3-0-1	7-2-15	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:73.1



5-6-12	10-10-0	13-10-1	21-1-0	31-10-4	41-2-0
5-6-12	5-3-4	3-0-1	7-2-15	10-9-4	9-3-12

Plate Offsets (X,Y): [1:Edge,0-1-12], [6:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.14	8-10	>781	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.12	8-10	>882	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.03	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 276 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-12, 6-10

REACTIONS (lb/size) 17=1264/0-3-8, 10=2012/0-3-8, 8=247/0-3-8
Max Horz 17=-239(load case 6)
Max Uplift17=-549(load case 5), 10=-964(load case 6), 8=-328(load case 6)
Max Grav 17=1264(load case 1), 10=2012(load case 1), 8=315(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1782/963, 2-3=-1613/956, 3-4=-1504/993, 4-5=-1033/702, 5-6=-1024/712, 6-7=-166/524, 7-8=-140/334, 8-9=0/39, 1-17=-1153/666
BOT CHORD 16-17=-274/293, 15-16=-700/1534, 14-15=-533/1387, 13-14=-580/1507, 12-13=-580/1507, 11-12=-28/460, 10-11=-28/460, 8-10=-275/280
WEBS 2-16=-115/173, 2-15=-207/216, 3-15=-111/258, 3-14=-176/287, 4-14=-141/173, 4-12=-885/648, 5-12=-241/456, 6-12=-164/591, 6-10=-1612/929, 7-10=-349/438, 1-16=-549/1263

JOINT STRESS INDEX
1 = 0.68, 2 = 0.41, 3 = 0.40, 4 = 0.70, 5 = 0.70, 6 = 0.49, 7 = 0.34, 8 = 0.36, 10 = 0.24, 11 = 0.19, 12 = 0.28, 13 = 0.34, 14 = 0.34, 15 = 0.25, 16 = 0.54 and 17 = 0.54

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 549 lb uplift at joint 17, 964 lb uplift at joint 10 and 328 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

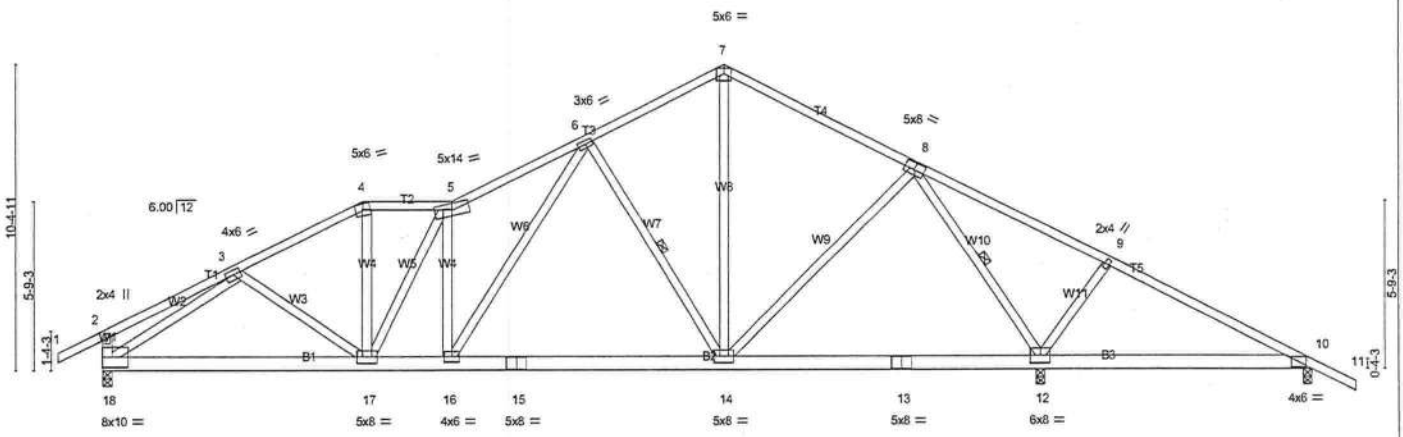
Dwg.#0119061116

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T16	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:28 2006 Page 1

1-6-0	4-6-12	8-10-0	11-10-1	16-5-8	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	4-6-12	4-3-4	3-0-1	4-7-8	4-7-8	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:75.0



8-10-0	11-10-1	21-1-0	25-2-3	31-10-4	41-2-0
8-10-0	3-0-1	9-2-15	4-1-3	6-8-1	9-3-12

Plate Offsets (X,Y): [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	0.14	10-12	>775	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	0.13	10-12	>880	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.03	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 279 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 10-12.
WEBS 1 Row at midpt 6-14, 8-12

REACTIONS (lb/size) 12=2029/0-3-8, 18=1351/0-3-8, 10=231/0-3-8
Max Horz 18=-214(load case 6)
Max Uplift12=-969(load case 6), 18=-660(load case 5), 10=-324(load case 6)
Max Grav 12=2029(load case 1), 18=1351(load case 1), 10=309(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-359/180, 3-4=-1690/955, 4-5=-1478/918, 5-6=-1954/1252, 6-7=-984/718, 7-8=-1014/701, 8-9=-191/561, 9-10=-169/373, 10-11=0/39, 2-18=-357/341
BOT CHORD 17-18=-692/1411, 16-17=-662/1688, 15-16=-403/1214, 14-15=-403/1214, 13-14=-16/445, 12-13=-16/445, 10-12=-310/305
WEBS 3-17=-5/134, 4-17=-233/514, 5-17=-447/250, 5-16=-631/507, 6-16=-589/939, 6-14=-742/589, 8-14=-153/587, 8-12=-1644/950, 9-12=-344/432, 3-18=-1425/833, 7-14=-325/524

JOINT STRESS INDEX
2 = 0.80, 3 = 0.42, 4 = 0.35, 5 = 0.47, 6 = 0.53, 7 = 0.56, 8 = 0.51, 9 = 0.34, 10 = 0.36, 12 = 0.25, 13 = 0.18, 14 = 0.28, 15 = 0.32, 16 = 0.50, 17 = 0.29 and 18 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 969 lb uplift at joint 12, 660 lb uplift at joint 18 and 324 lb uplift at joint 10.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

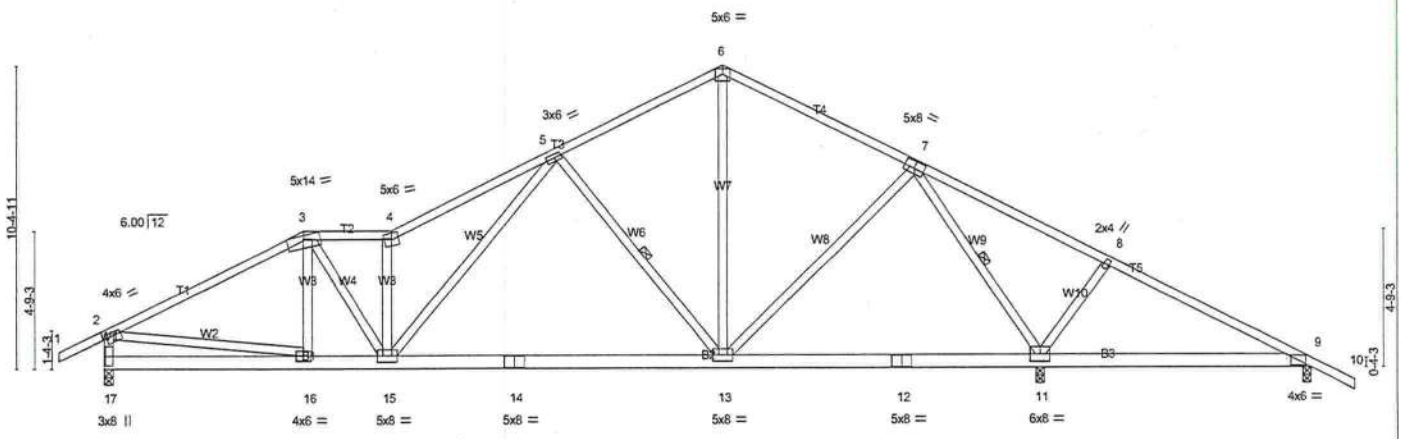
Dwg.#0119061117

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T17	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:29 2006 Page 1

1-6-0	6-10-0	9-10-1	15-5-8	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	6-10-0	3-0-1	5-7-8	5-7-8	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:75.0



6-10-0	9-10-1	21-1-0	31-10-4	41-2-0
6-10-0	3-0-1	11-2-15	10-9-4	9-3-12

Plate Offsets (X,Y): [2:0-3-0,0-1-8], [7:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.69	Vert(LL)	0.14	9-11	>774	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	0.12	9-11	>884	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.84	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 270 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 9-11.
WEBS 1 Row at midpt 5-13, 7-11

REACTIONS (lb/size) 17=1338/0-3-8, 11=2086/0-3-8, 9=188/0-3-8
Max Horz 17=-214(load case 6)
Max Uplift17=-656(load case 5), 11=-989(load case 6), 9=-308(load case 6)
Max Grav 17=1338(load case 1), 11=2086(load case 1), 9=281(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1760/942, 3-4=-1884/1090, 4-5=-2205/1341, 5-6=-980/697, 6-7=-991/688, 7-8=-240/662, 8-9=-209/466, 9-10=0/39, 2-17=-1206/817
BOT CHORD 16-17=-316/356, 15-16=-655/1491, 14-15=-465/1280, 13-14=-465/1280, 12-13=0/395, 11-12=0/395, 9-11=-393/348
WEBS 3-16=-130/105, 3-15=-307/720, 4-15=-1139/761, 5-15=-590/1032, 5-13=-755/619, 6-13=-290/486, 7-13=-170/630, 7-11=-1720/984, 8-11=-346/434, 2-16=-456/1152

JOINT STRESS INDEX
2 = 0.81, 3 = 0.67, 4 = 0.53, 5 = 0.55, 6 = 0.61, 7 = 0.51, 8 = 0.34, 9 = 0.34, 11 = 0.26, 12 = 0.16, 13 = 0.30, 14 = 0.42, 15 = 0.50, 16 = 0.48 and 17 = 0.46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 656 lb uplift at joint 17, 989 lb uplift at joint 11 and 308 lb uplift at joint 9.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

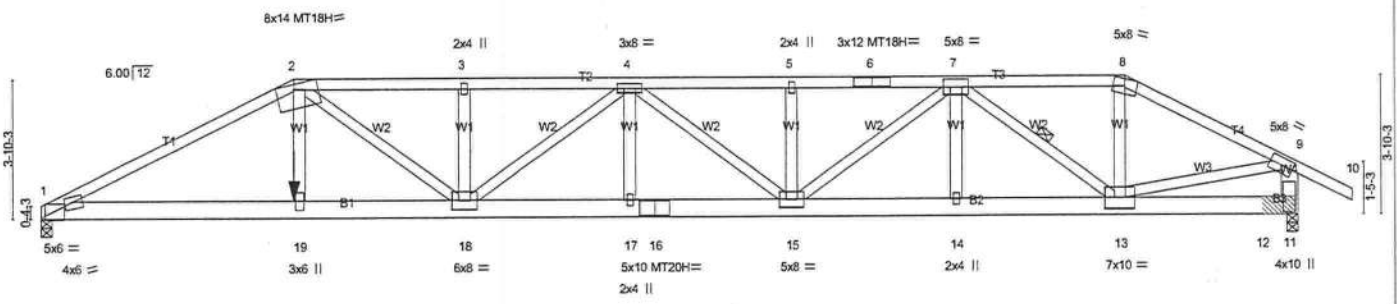
Dwg.#0119061118

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T18	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:30 2006 Page 1

3-9-4	7-0-0	11-8-4	16-2-12	20-9-4	25-3-12	30-0-0	34-10-0	36-4-0
3-9-4	3-2-12	4-8-4	4-6-8	4-6-8	4-6-8	4-8-4	4-10-0	1-6-0

Scale = 1:61.0



3-9-4	7-0-0	11-8-4	16-2-12	20-9-4	25-3-12	30-0-0	34-10-0
3-9-4	3-2-12	4-8-4	4-6-8	4-6-8	4-6-8	4-8-4	4-10-0

Plate Offsets (X,Y): [1:0-1-10,Edge], [1:0-8-10,0-0-7], [2:0-6-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	0.56	15-17	>736	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.89	15-17	>463	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.73	Horz(TL)	0.17	11	n/a	n/a	MT18H	244/190
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 216 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 6 SYP No.1D, W3 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-9-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-1-2 oc bracing.
WEBS 1 Row at midpt 7-13

REACTIONS (lb/size) 1=2877/0-3-8, 11=3157/0-3-12 (0-3-8 + bearing block)
Max Horz 1=-265(load case 5)
Max Uplift1=-1488(load case 3), 11=-1638(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5907/3197, 2-3=-6981/3942, 3-4=-6981/3942, 4-5=-7374/4176, 5-6=-7374/4176, 6-7=-7374/4176, 7-8=-3637/2134, 8-9=-4131/2243, 9-10=0/42, 9-11=-2920/1615
BOT CHORD 1-19=-2688/5202, 18-19=-2701/5238, 17-18=-4165/7745, 16-17=-4165/7745, 15-16=-4165/7745, 14-15=-3235/6069, 13-14=-3235/6069, 12-13=-268/478, 11-12=-268/478
WEBS 2-19=-346/854, 2-18=-1347/2276, 3-18=-517/528, 4-18=-990/587, 4-17=0/340, 4-15=-484/260, 5-15=-494/497, 7-15=-911/1658, 7-14=0/337, 7-13=-3091/1741, 8-13=-546/1326, 9-13=-1637/3195

JOINT STRESS INDEX
1 = 0.82, 1 = 0.81, 2 = 0.82, 3 = 0.34, 4 = 0.57, 5 = 0.34, 6 = 0.89, 7 = 0.82, 8 = 0.81, 9 = 0.91, 11 = 0.55, 11 = 0.00, 12 = 0.00, 12 = 0.00, 13 = 0.97, 14 = 0.34, 15 = 0.77, 16 = 0.98, 17 = 0.34, 18 = 0.86 and 19 = 0.28

NOTES
1) 2 X 6 SYP No.1D bearing block 12" long at jt. 11 attached to front face with 3 rows of 0.131"x3" Nails spaced 3" o.c. 12 Total fasteners. Bearing is assumed to be SYP.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1488 lb uplift at joint 1 and 1638 lb uplift at joint 11.
8) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 348 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-8=-113(F=-58), 8-9=-112(F=-58), 9-10=-54, 1-19=-30, 11-19=-62(F=-33)
Concentrated Loads (lb)
Vert: 19=-539(F)

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

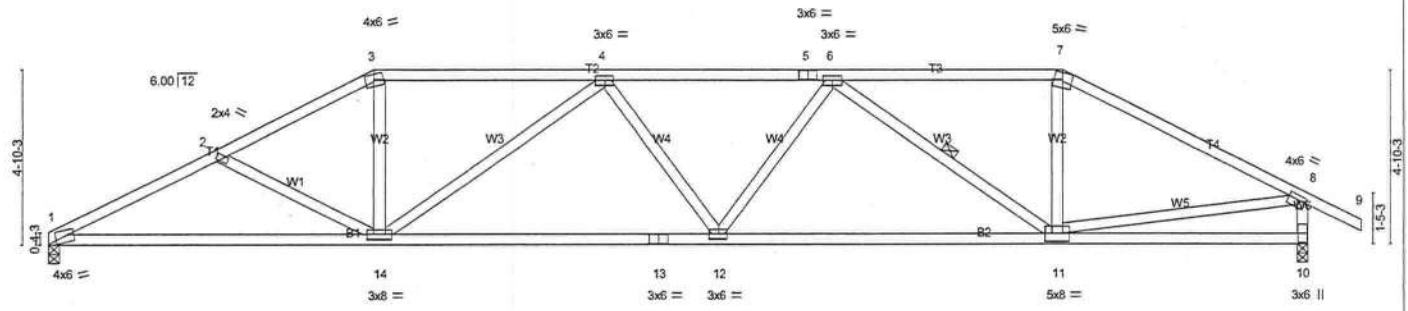
Dwg.#0119061119

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T19	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:31 2006 Page 1

4-9-4	9-0-0	15-4-0	21-8-0	28-0-0	34-10-0	36-4-0
4-9-4	4-2-12	6-4-0	6-4-0	6-4-0	6-10-0	1-6-0

Scale = 1:60.9



9-0-0	18-6-0	28-0-0	34-10-0
9-0-0	9-6-0	9-6-0	6-10-0

Plate Offsets (X,Y): [1:0-2-12,0-0-11], [8:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.30	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.50	11-12	>831	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.12	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 178 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-9-1 oc bracing.
WEBS 1 Row at midpt 6-11

REACTIONS (lb/size) 1=1449/0-3-8, 10=1542/0-3-8
Max Horz 1=-107(load case 6)
Max Uplift1=-534(load case 5), 10=-621(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2706/1459, 2-3=-2473/1306, 7-8=-2084/1073, 8-9=0/40, 8-10=-1447/913, 3-4=-2193/1241, 4-5=-2755/1491, 5-6=-2755/1491, 6-7=-1807/1051
BOT CHORD 1-14=-1155/2371, 13-14=-1213/2724, 12-13=-1213/2724, 11-12=-1139/2587, 10-11=-85/217
WEBS 2-14=-230/292, 3-14=-310/790, 4-14=-748/448, 4-12=0/108, 6-12=-29/317, 6-11=-1038/551, 7-11=-161/599, 8-11=-647/1596

JOINT STRESS INDEX
1 = 0.87, 2 = 0.34, 3 = 0.79, 4 = 0.41, 5 = 0.59, 6 = 0.41, 7 = 0.73, 8 = 0.73, 10 = 0.42, 11 = 0.74, 12 = 0.41, 13 = 0.94 and 14 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 534 lb uplift at joint 1 and 621 lb uplift at joint 10.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

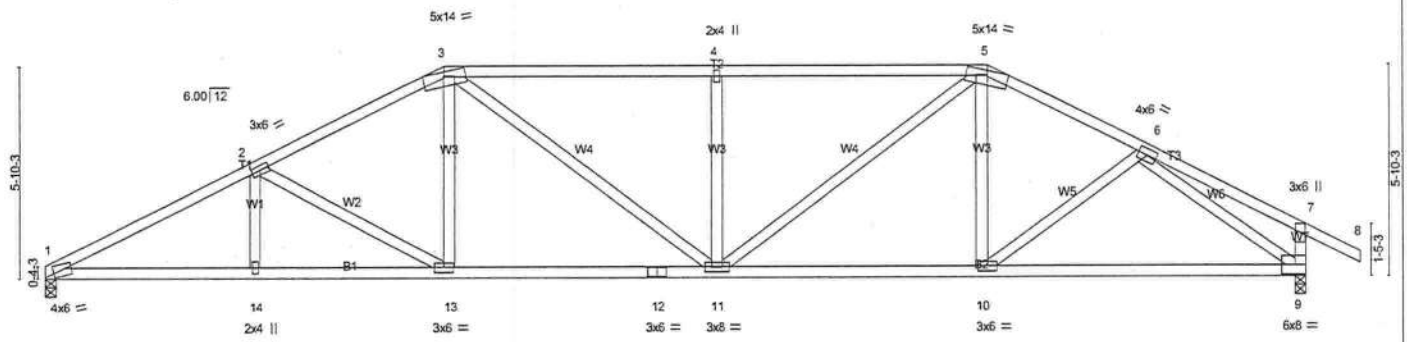
Dwg.#0119061120

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T20	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:31 2006 Page 1

5-9-4	11-0-0	18-6-0	26-0-0	30-3-4	34-10-0	36-4-0
5-9-4	5-2-12	7-6-0	7-6-0	4-3-4	4-6-12	1-6-0

Scale = 1:60.9



5-9-4	11-0-0	18-6-0	26-0-0	34-10-0
5-9-4	5-2-12	7-6-0	7-6-0	8-10-0

Plate Offsets (X,Y): [1:0-3-0,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.22	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.35	11-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.11	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 189 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-9-2 oc bracing.

REACTIONS (lb/size) 1=1449/0-3-8, 9=1542/0-3-8
Max Horz 1=-123(load case 6)
Max Uplift1=-556(load case 5), 9=-645(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2760/1443, 2-3=-2280/1260, 3-4=-2310/1353, 4-5=-2310/1353, 5-6=-1998/1113, 6-7=-336/173, 7-8=0/40, 7-9=-358/345
BOT CHORD 1-14=-1127/2392, 13-14=-1127/2392, 12-13=-807/1998, 11-12=-807/1998, 10-11=-669/1757, 9-10=-701/1585
WEBS 2-14=0/176, 2-13=-464/368, 3-13=-135/418, 3-11=-289/512, 4-11=-429/376, 5-11=-379/766, 5-10=-10/134, 6-10=-133/318, 6-9=-1703/960

JOINT STRESS INDEX
1 = 0.88, 2 = 0.41, 3 = 0.92, 4 = 0.34, 5 = 0.93, 6 = 0.51, 7 = 0.31, 9 = 0.35, 10 = 0.35, 11 = 0.74, 12 = 0.70, 13 = 0.35 and 14 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 556 lb uplift at joint 1 and 645 lb uplift at joint 9.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

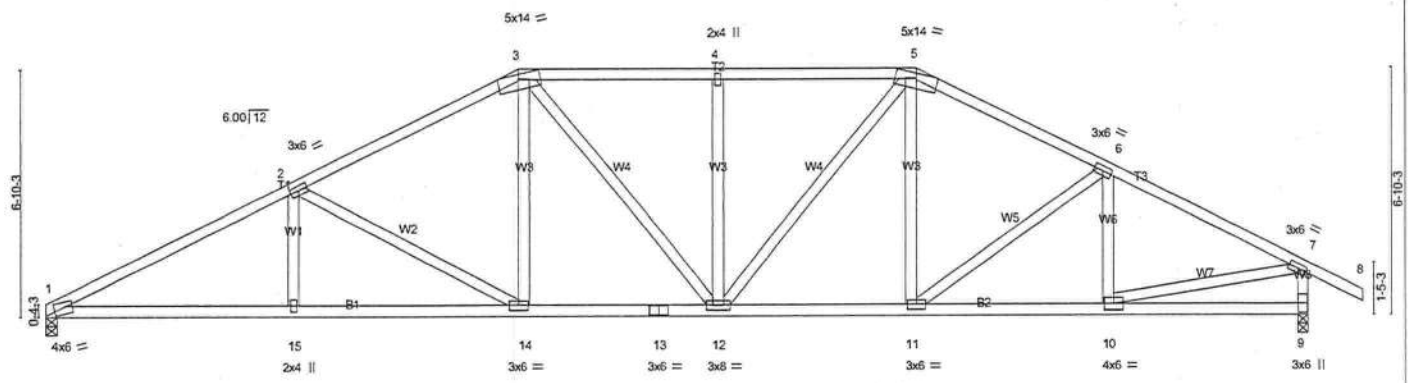
Dwg.#0119061121

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T21	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:32 2006 Page 1

6-9-4	13-0-0	18-6-0	24-0-0	29-3-4	34-10-0	36-4-0
6-9-4	6-2-12	5-6-0	5-6-0	5-3-4	5-6-12	1-6-0

Scale = 1:60.9



6-9-4	13-0-0	18-6-0	24-0-0	29-3-4	34-10-0
6-9-4	6-2-12	5-6-0	5-6-0	5-3-4	5-6-12

Plate Offsets (X,Y): [1:0-3-0,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.16	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.26	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.10	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 202 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-8-7 oc bracing.

REACTIONS (lb/size) 1=1449/0-3-8, 9=1542/0-3-8
Max Horz 1=-140(load case 6)
Max Uplift1=-575(load case 5), 9=-666(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2728/1439, 2-3=-2104/1200, 3-4=-1925/1198, 4-5=-1925/1198, 5-6=-1915/1114, 6-7=-1993/1074, 7-8=0/40, 7-9=-1450/928
BOT CHORD 1-15=-1111/2359, 14-15=-1111/2359, 13-14=-699/1821, 12-13=-699/1821, 11-12=-612/1662, 10-11=-736/1719, 9-10=-34/162
WEBS 2-15=0/229, 2-14=-627/473, 3-14=-184/468, 3-12=-188/306, 4-12=-303/269, 5-12=-246/508, 5-11=-78/233, 6-11=-109/183, 6-10=-197/204, 7-10=-748/1595

JOINT STRESS INDEX
1 = 0.87, 2 = 0.41, 3 = 0.73, 4 = 0.34, 5 = 0.67, 6 = 0.41, 7 = 0.89, 9 = 0.38, 10 = 0.71, 11 = 0.35, 12 = 0.57, 13 = 0.65, 14 = 0.35 and 15 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 575 lb uplift at joint 1 and 666 lb uplift at joint 9.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

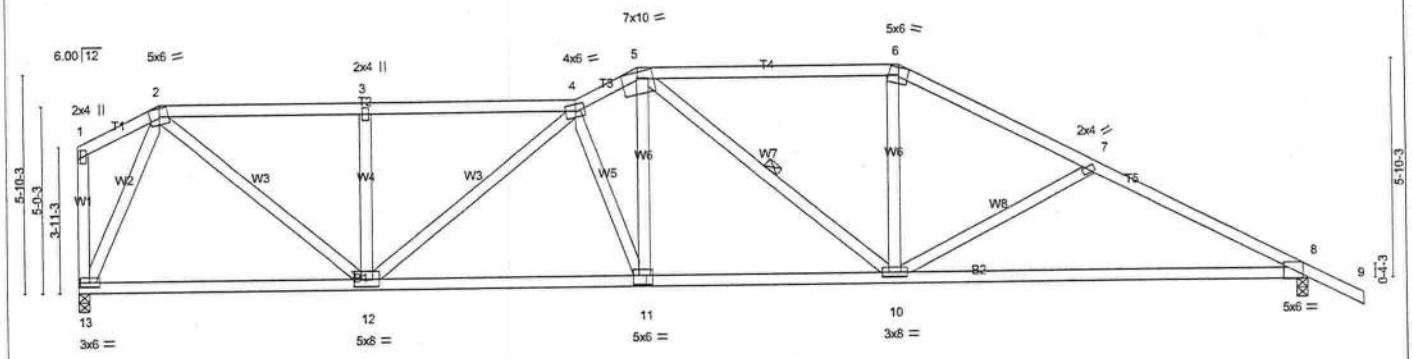
Dwg.#0119061122

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T22	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:33 2006 Page 1

2-2-0	7-9-0	13-4-0	15-0-0	22-0-0	27-2-12	33-0-0	34-6-0
2-2-0	5-7-0	5-7-0	1-8-0	7-0-0	5-2-12	5-9-4	1-6-0

Scale = 1:59.1



2-2-0	7-9-0	13-4-0	15-0-0	22-0-0	27-2-12	33-0-0
2-2-0	5-7-0	5-7-0	1-8-0	7-0-0	5-2-12	5-9-4

Plate Offsets (X,Y): [8:0-1-11,Edge], [11:0-2-4,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.40	8-10	>983	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.68	8-10	>578	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.09	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 187 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-4-5 oc bracing.
WEBS 1 Row at midpt 5-10

REACTIONS (lb/size) 13=1372/0-3-8, 8=1465/0-3-8
Max Horz 13=-288(load case 6)
Max Uplift13=-583(load case 5), 8=-640(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-58/18, 2-3=-1762/956, 3-4=-1762/956, 4-5=-2242/1245, 5-6=-1874/1083, 6-7=-2141/1123, 7-8=-2450/1304, 8-9=0/35, 1-13=-51/25
BOT CHORD 12-13=-117/560, 11-12=-913/2284, 10-11=-767/2052, 8-10=-985/2143
WEBS 2-12=-775/1573, 3-12=-331/289, 4-12=-683/409, 5-11=-371/830, 5-10=-355/164, 6-10=-183/602, 7-10=-332/354, 2-13=-1320/768, 4-11=-708/435

JOINT STRESS INDEX
1 = 0.44, 2 = 0.57, 3 = 0.34, 4 = 0.53, 5 = 0.66, 6 = 0.68, 7 = 0.34, 8 = 0.72, 10 = 0.57, 11 = 0.76, 12 = 0.74 and 13 = 0.58

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 583 lb uplift at joint 13 and 640 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061123

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T23	SPECIAL	1	1	Job Reference (optional)

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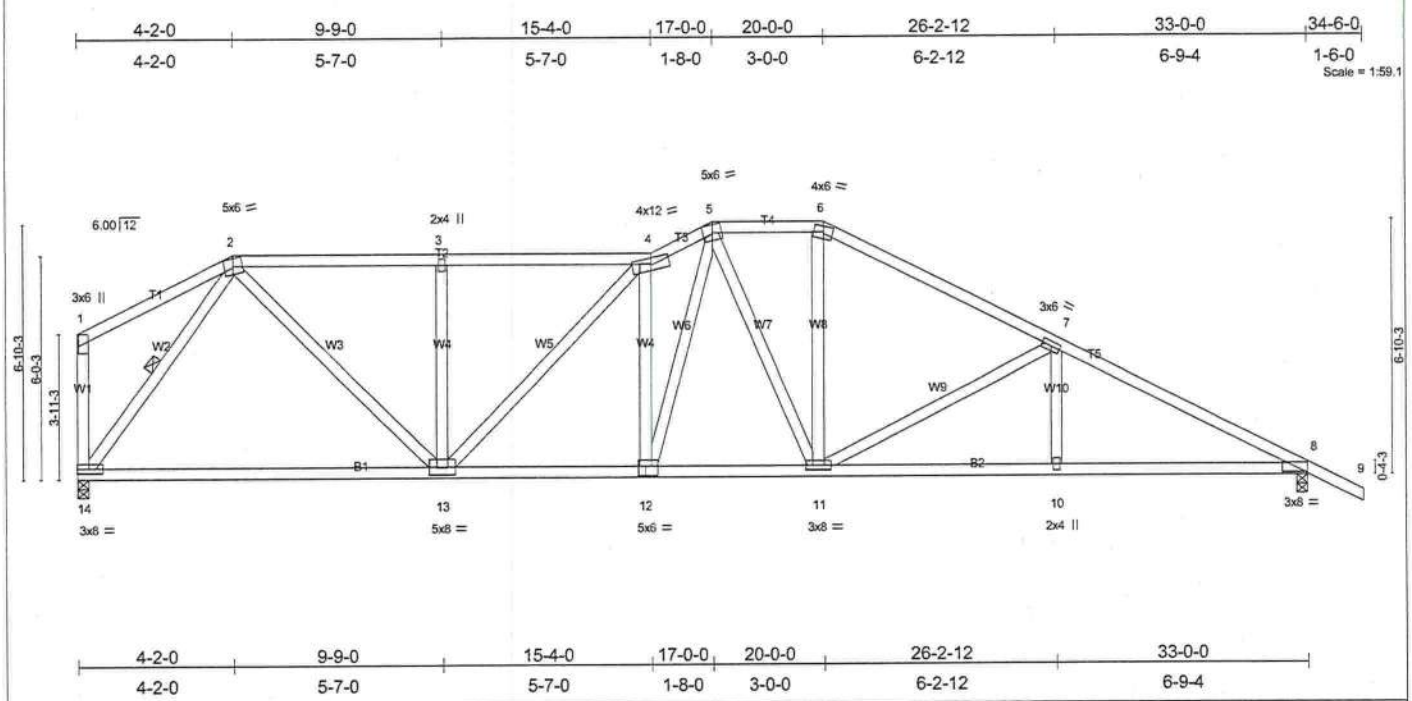


Plate Offsets (X,Y): [8:0-8-0,0-0-6], [12:0-2-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.73	Vert(LL)	-0.21	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.36	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 202 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-7 oc bracing.
WEBS 1 Row at midpt 2-14

REACTIONS (lb/size) 14=1372/0-3-8, 8=1465/0-3-8
Max Horz 14=-305(load case 6)
Max Uplift14=-597(load case 5), 8=-659(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-134/74, 2-3=-1686/969, 3-4=-1686/969, 4-5=-2254/1355, 5-6=-1667/1040, 6-7=-1932/1070, 7-8=-2546/1283, 8-9=0/35, 1-14=-145/119
BOT CHORD 13-14=-254/838, 12-13=-734/1964, 11-12=-585/1707, 10-11=-958/2195, 8-10=-958/2195
WEBS 2-13=-569/1213, 3-13=-326/283, 4-13=-404/258, 4-12=-910/620, 5-11=-245/109, 6-11=-200/558, 7-11=-623/441, 7-10=0/221, 2-14=-1323/770, 5-12=-621/1053

JOINT STRESS INDEX
1 = 0.27, 2 = 0.47, 3 = 0.34, 4 = 0.60, 5 = 0.61, 6 = 0.85, 7 = 0.41, 8 = 0.80, 10 = 0.34, 11 = 0.64, 12 = 0.80, 13 = 0.58 and 14 = 0.63

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 597 lb uplift at joint 14 and 659 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

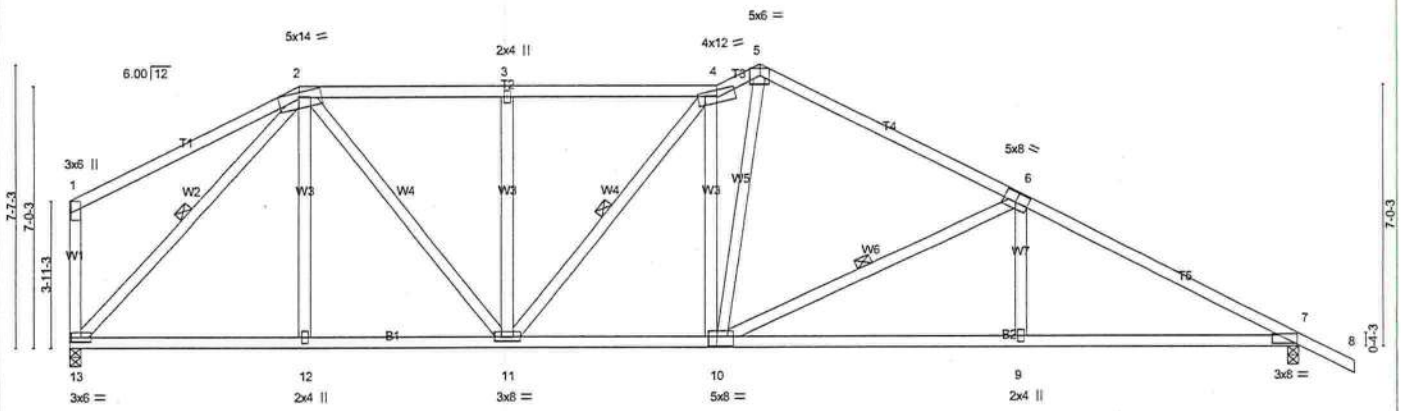
Dwg.#0119061124

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T24	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:34 2006 Page 1

6-2-0	11-9-0	17-4-0	18-6-0	25-5-12	33-0-0	34-6-0
6-2-0	5-7-0	5-7-0	1-2-0	6-11-12	7-6-4	1-6-0

Scale = 1:59.1



6-2-0	11-9-0	17-4-0	25-5-12	33-0-0
6-2-0	5-7-0	5-7-0	8-1-12	7-6-4

Plate Offsets (X,Y): [6:0-4-0,0-3-0], [7:0-8-8,0-0-10], [10:0-2-8,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.20	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.32	9-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 205 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-4-0 oc bracing.
WEBS 1 Row at midpt 4-11, 6-10, 2-13

REACTIONS (lb/size) 13=1372/0-3-8, 7=1465/0-3-8
Max Horz 13=-317(load case 6)
Max Uplift13=-606(load case 5), 7=-671(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-142/161, 2-3=-1557/969, 3-4=-1557/969, 4-5=-1828/1119, 5-6=-1753/997, 6-7=-2512/1269, 7-8=0/35, 1-13=-191/218
BOT CHORD 12-13=-316/1036, 11-12=-315/1040, 10-11=-568/1687, 9-10=-934/2153, 7-9=-934/2159
WEBS 2-12=0/183, 2-11=-426/808, 3-11=-318/285, 4-11=-275/138, 4-10=-783/470, 5-10=-619/1238, 6-10=-746/514, 6-9=0/270, 2-13=-1458/703

JOINT STRESS INDEX
1 = 0.47, 2 = 0.34, 3 = 0.34, 4 = 0.41, 5 = 0.67, 6 = 0.68, 7 = 0.79, 9 = 0.34, 10 = 0.75, 11 = 0.82, 12 = 0.34 and 13 = 0.58

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 606 lb uplift at joint 13 and 671 lb uplift at joint 7.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

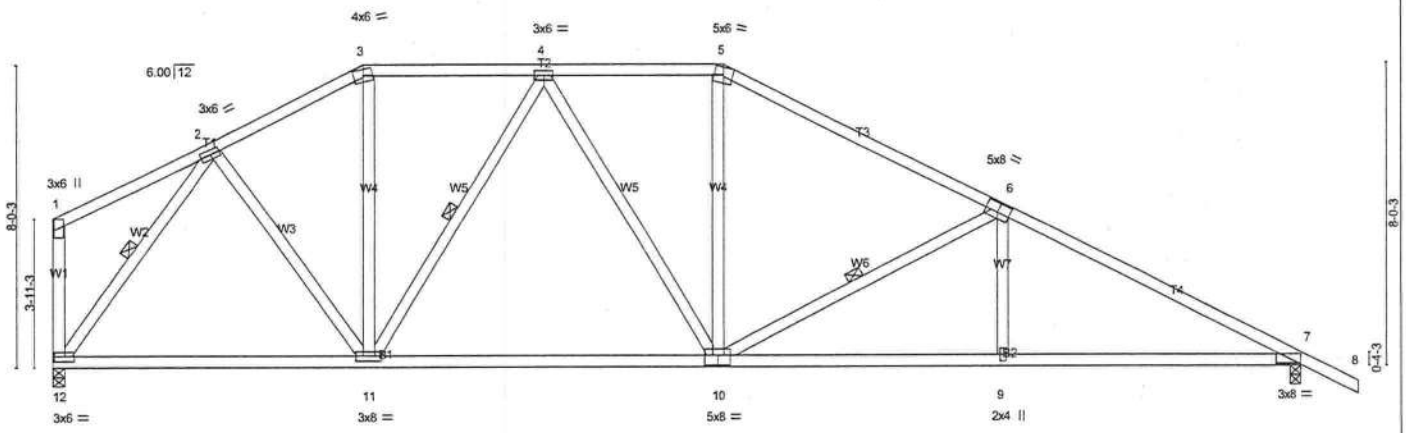
Dwg.#0119061125

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T24A	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:35 2006 Page 1

4-2-12	8-2-0	12-11-0	17-8-0	25-0-12	33-0-0	34-6-0
4-2-12	3-11-4	4-9-0	4-9-0	7-4-12	7-11-4	1-6-0

Scale = 1:58.3



8-2-0	17-8-0	25-0-12	33-0-0
8-2-0	9-6-0	7-4-12	7-11-4

Plate Offsets (X,Y): [6:0-4-0,0-3-0], [7:0-8-0,0-0-6], [10:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.20	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.34	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.36	Horz(TL)	0.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 196 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-3-9 oc bracing.
WEBS 1 Row at midpt 4-11, 6-10, 2-12

REACTIONS (lb/size) 12=1372/0-3-8, 7=1465/0-3-8
Max Horz 12=-325(load case 6)
Max Uplift12=-502(load case 5), 7=-677(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-111/84, 2-3=-1297/793, 3-4=-1127/767, 4-5=-1488/983, 5-6=-1750/995, 6-7=-2485/1267, 7-8=0/35, 1-12=-142/130
BOT CHORD 11-12=-211/839, 10-11=-393/1374, 9-10=-927/2128, 7-9=-927/2133
WEBS 2-11=-163/520, 3-11=-121/320, 4-11=-538/317, 4-10=-148/265, 5-10=-117/409, 6-10=-740/549, 6-9=0/255, 2-12=-1366/767

JOINT STRESS INDEX
1 = 0.25, 2 = 0.38, 3 = 0.56, 4 = 0.44, 5 = 0.73, 6 = 0.78, 7 = 0.78, 9 = 0.34, 10 = 0.61, 11 = 0.59 and 12 = 0.61

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 502 lb uplift at joint 12 and 677 lb uplift at joint 7.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

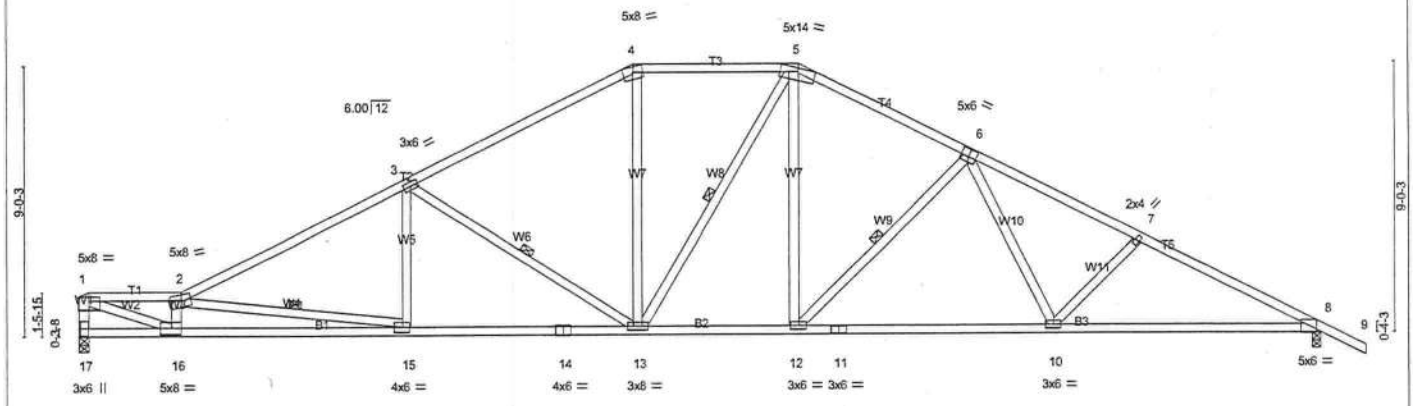
Dwg.#0119061126

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T25	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:36 2006 Page 1

0-3-8 3-3-9 | 10-9-12 | 18-4-0 | 23-10-0 | 29-5-3 | 35-0-5 | 41-2-0 | 42-8-0
0-3-8 3-0-1 | 7-6-4 | 7-6-4 | 5-6-0 | 5-7-3 | 5-7-3 | 6-1-11 | 1-6-0

Scale = 1:73.1



3-3-9 | 10-9-12 | 18-4-0 | 23-10-0 | 32-2-12 | 41-2-0
3-3-9 | 7-6-4 | 7-6-4 | 5-6-0 | 8-4-12 | 8-11-4

Plate Offsets (X,Y): [1:0-4-8,0-1-12], [6:0-3-0,0-3-0], [8:0-1-11,Edge], [16:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.35	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.57	15-16	>866	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.16	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 236 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D *Except*
B2 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-5-11 oc bracing.
WEBS 1 Row at midpt 2-15, 3-13, 5-13, 6-12

REACTIONS (lb/size) 17=1715/0-3-8, 8=1807/0-3-8
Max Horz 17=-246(load case 6)
Max Uplift17=-560(load case 6), 8=-812(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4004/2182, 2-3=-3243/1755, 3-4=-2343/1393, 4-5=-2018/1343, 5-6=-2278/1388, 6-7=-3030/1682, 7-8=-3239/1756, 8-9=0/35, 1-17=-1616/867
BOT CHORD 16-17=-109/191, 15-16=-2155/4182, 14-15=-1314/2833, 13-14=-1314/2833, 12-13=-720/1986, 11-12=-1068/2417, 10-11=-1068/2417, 8-10=-1386/2833
WEBS 1-16=-2263/4124, 2-16=-1468/950, 2-15=-1366/851, 3-15=-99/488, 3-13=-983/682, 4-13=-288/656, 5-13=-156/242, 5-12=-325/654, 6-12=-633/506, 6-10=-206/544, 7-10=-278/334

JOINT STRESS INDEX
1 = 0.76, 2 = 0.72, 3 = 0.41, 4 = 0.66, 5 = 0.73, 6 = 0.71, 7 = 0.34, 8 = 0.81, 10 = 0.46, 11 = 0.84, 12 = 0.43, 13 = 0.60, 14 = 0.92, 15 = 0.39, 16 = 0.92 and 17 = 0.44

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 560 lb uplift at joint 17 and 812 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

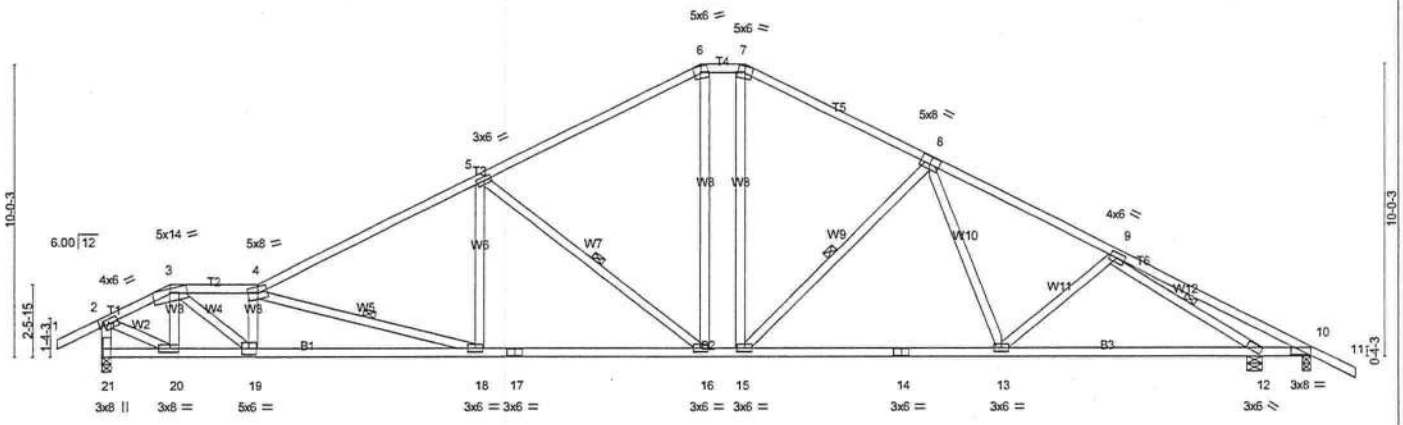
Dwg.#0119061127

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T26	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:37 2006 Page 1

1-6-0	2-3-8	5-3-9	12-9-12	20-4-0	21-10-0	28-1-3	34-4-5	41-2-0	42-8-0
1-6-0	2-3-8	3-0-1	7-6-4	7-6-4	1-6-0	6-3-3	6-3-3	6-9-11	1-6-0

Scale = 1:75.0



2-3-8	5-3-9	12-9-12	20-4-0	21-10-0	30-6-6	39-2-12	41-2-0
2-3-8	3-0-1	7-6-4	7-6-4	1-6-0	8-8-6	8-8-6	1-11-4

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [8:0-4-0,0-3-0], [10:0-8-0,0-0-6], [20:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.31	16-18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.49	16-18	>959	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.74	Horz(TL)	0.14	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 251 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-9-3 oc bracing.
WEBS 1 Row at midpt 4-18, 5-16, 8-15, 9-12

REACTIONS (lb/size) 21=1739/0-3-8, 12=1760/0-6-7, 10=113/0-3-8
Max Horz 21=-204(load case 6)
Max Uplift21=-814(load case 5), 12=-627(load case 6), 10=-269(load case 6)
Max Grav 21=1739(load case 1), 12=1760(load case 1), 10=118(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1763/976, 3-4=-3327/1878, 4-5=-2818/1590, 5-6=-1970/1251, 6-7=-1682/1217, 7-8=-1948/1262, 8-9=-2450/1437, 9-10=-242/163, 10-11=0/35, 2-21=-1682/1052
BOT CHORD 20-21=-139/209, 19-20=-688/1488, 18-19=-1733/3410, 17-18=-1103/2449, 16-17=-1103/2449, 15-16=-560/1682, 14-15=-877/2032, 13-14=-877/2032, 12-13=-1076/2109, 10-12=-1/138
WEBS 3-20=-687/349, 3-19=-1294/2318, 4-19=-1256/851, 4-18=-1001/656, 5-18=-149/547, 5-16=-1000/694, 6-16=-328/609, 7-15=-376/635, 8-15=-574/463, 8-13=-96/315, 9-13=-27/230, 9-12=-2382/1347, 2-20=-829/1643

JOINT STRESS INDEX
2 = 0.64, 3 = 0.78, 4 = 0.79, 5 = 0.41, 6 = 0.77, 7 = 0.72, 8 = 0.53, 9 = 0.69, 10 = 0.77, 12 = 0.86, 13 = 0.49, 14 = 0.87, 15 = 0.42, 16 = 0.40, 17 = 0.90, 18 = 0.36, 19 = 0.75, 20 = 0.66 and 21 = 0.35

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 814 lb uplift at joint 21, 627 lb uplift at joint 12 and 269 lb uplift at joint 10.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

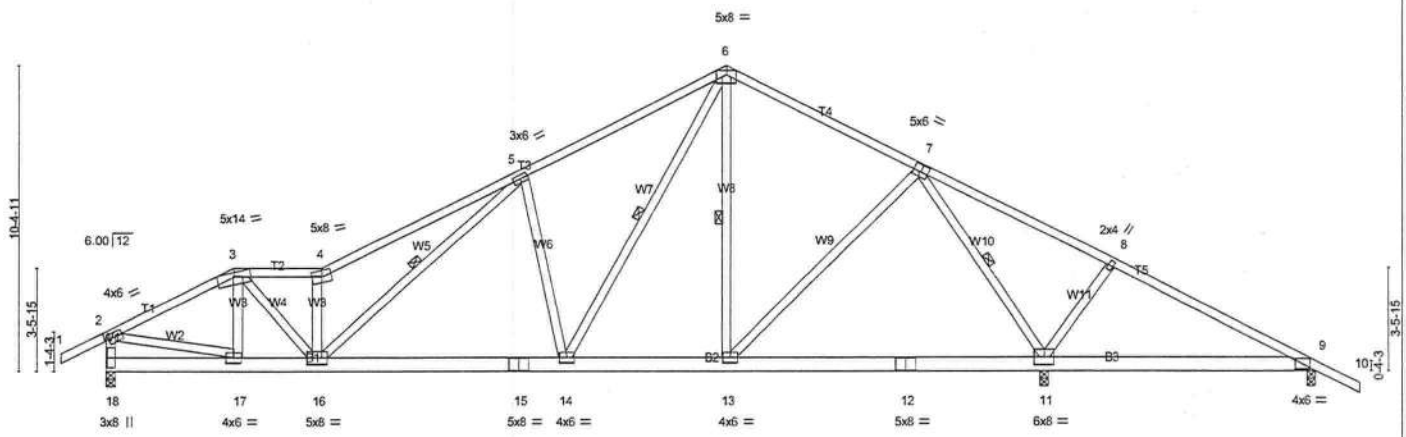
Dwg.#0119061128

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T27	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:38 2006 Page 1

1-6-0	4-3-8	7-3-9	14-2-4	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	4-3-8	3-0-1	6-10-12	6-10-12	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:75.0



4-3-8	7-3-9	15-5-13	21-1-0	31-10-4	41-2-0
4-3-8	3-0-1	8-2-4	5-7-3	10-9-4	9-3-12

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [7:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.16	14-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	0.13	9-11	>880	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 274 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 9-11.
WEBS 1 Row at midpt 5-16, 6-14, 6-13, 7-11

REACTIONS (lb/size) 18=1334/0-3-8, 11=2105/0-3-8, 9=173/0-3-8
Max Horz 18=-214(load case 6)
Max Uplift18=-654(load case 5), 11=-993(load case 6), 9=-305(load case 6)
Max Grav 18=1334(load case 1), 11=2105(load case 1), 9=271(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1650/889, 3-4=-2148/1207, 4-5=-2547/1519, 5-6=-1526/1077, 6-7=-967/686, 7-8=-254/685, 8-9=-222/487, 9-10=0/39, 2-18=-1238/813
BOT CHORD 17-18=-185/209, 16-17=-654/1409, 15-16=-548/1428, 14-15=-548/1428, 13-14=-117/796, 12-13=0/372, 11-12=0/372, 9-11=-411/360
WEBS 3-17=-234/125, 3-16=-553/1093, 4-16=-1381/954, 5-16=-660/1073, 5-14=-692/646, 6-14=-722/1019, 6-13=-217/217, 7-13=-183/636, 7-11=-1717/993, 8-11=-347/436, 2-17=-608/1292

JOINT STRESS INDEX
2 = 0.49, 3 = 0.43, 4 = 0.64, 5 = 0.65, 6 = 0.68, 7 = 0.75, 8 = 0.34, 9 = 0.36, 11 = 0.26, 12 = 0.18, 13 = 0.28, 14 = 0.52, 15 = 0.31, 16 = 0.53, 17 = 0.57 and 18 = 0.32

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 654 lb uplift at joint 18, 993 lb uplift at joint 11 and 305 lb uplift at joint 9.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

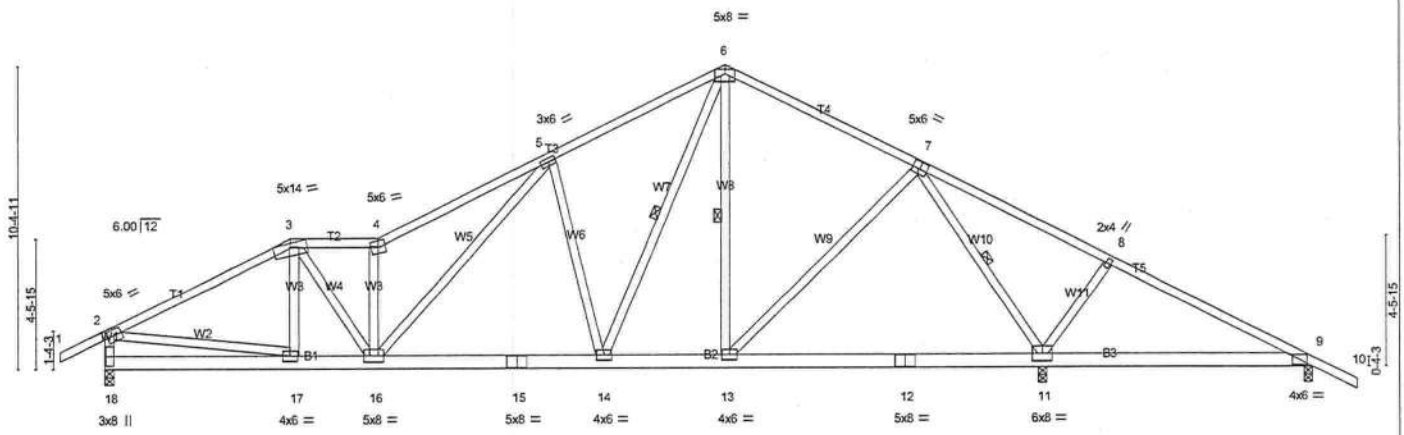
Dwg.#0119061129

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T28	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:38 2006 Page 1

1-6-0	6-3-8	9-3-9	15-2-4	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	6-3-8	3-0-1	5-10-12	5-10-12	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:75.0



6-3-8	9-3-9	16-9-13	21-1-0	31-10-4	41-2-0
6-3-8	3-0-1	7-6-4	4-3-3	10-9-4	9-3-12

Plate Offsets (X,Y): [2:0-2-11,0-2-8], [7:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	0.14	9-11	>781	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	0.13	9-11	>878	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 281 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 9-11.
WEBS 1 Row at midpt 6-14, 6-13, 7-11

REACTIONS (lb/size) 18=1342/0-3-8, 11=2066/0-3-8, 9=203/0-3-8
Max Horz 18=-214(load case 6)
Max Uplift18=-659(load case 5), 11=-980(load case 6), 9=-315(load case 6)
Max Grav 18=1342(load case 1), 11=2066(load case 1), 9=288(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1763/945, 3-4=-1919/1118, 4-5=-2254/1383, 5-6=-1377/992, 6-7=-987/696, 7-8=-213/620, 8-9=-181/424, 9-10=0/39, 2-18=-1221/820
BOT CHORD 17-18=-284/307, 16-17=-666/1499, 15-16=-486/1333, 14-15=-486/1333, 13-14=-123/812, 12-13=-10/409, 11-12=-10/409, 9-11=-356/324
WEBS 3-17=-95/101, 3-16=-353/736, 4-16=-1187/811, 5-16=-599/968, 5-14=-710/621, 6-14=-664/924, 6-13=-199/203, 7-13=-163/606, 7-11=-1675/965, 8-11=-345/434, 2-17=-505/1212

JOINT STRESS INDEX
2 = 0.75, 3 = 0.53, 4 = 0.61, 5 = 0.59, 6 = 0.63, 7 = 0.75, 8 = 0.34, 9 = 0.36, 11 = 0.25, 12 = 0.19, 13 = 0.27, 14 = 0.51, 15 = 0.37, 16 = 0.47, 17 = 0.50 and 18 = 0.42

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 659 lb uplift at joint 18, 980 lb uplift at joint 11 and 315 lb uplift at joint 9.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061130

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T29	MONO TRUSS	1	1	Job Reference (optional)

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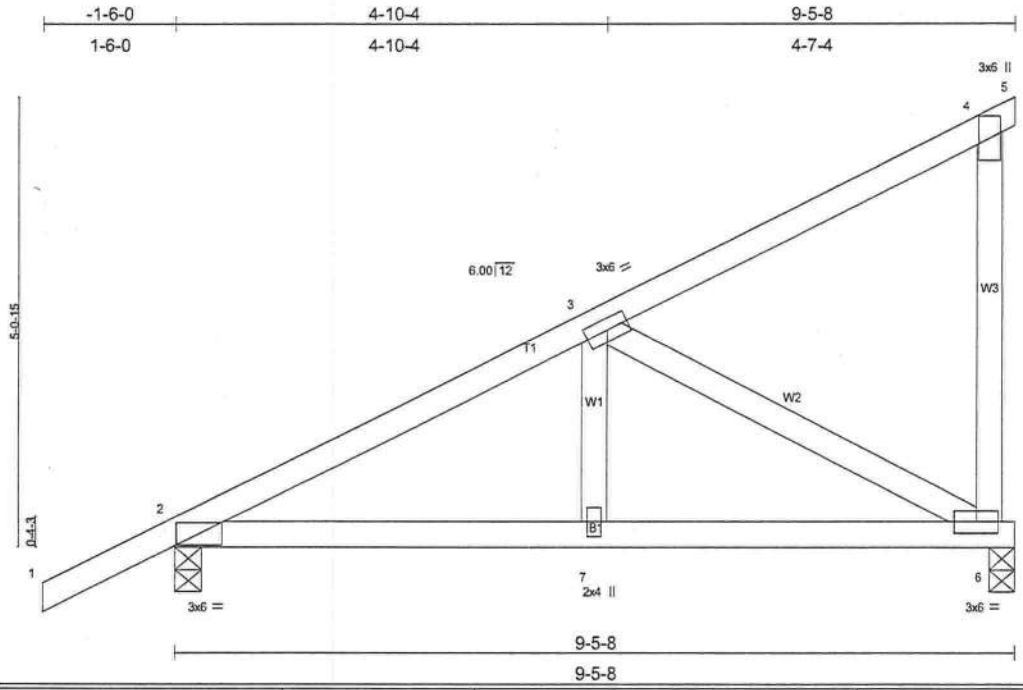


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	0.04	2-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	0.04	2-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.20	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-3-6 oc bracing.

REACTIONS (lb/size) 6=375/0-3-8, 2=476/0-3-8
Max Horz 2=315(load case 5)
Max Uplift6=-385(load case 5), 2=-374(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-512/540, 3-4=-81/27, 4-5=-2/0, 4-6=-104/147
BOT CHORD 2-7=-723/407, 6-7=-723/407
WEBS 3-6=-439/781, 3-7=-324/147

JOINT STRESS INDEX
2 = 0.41, 3 = 0.38, 4 = 0.30, 6 = 0.61 and 7 = 0.11

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 385 lb uplift at joint 6 and 374 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0119061131

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	T29G	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:40 2006 Page 1

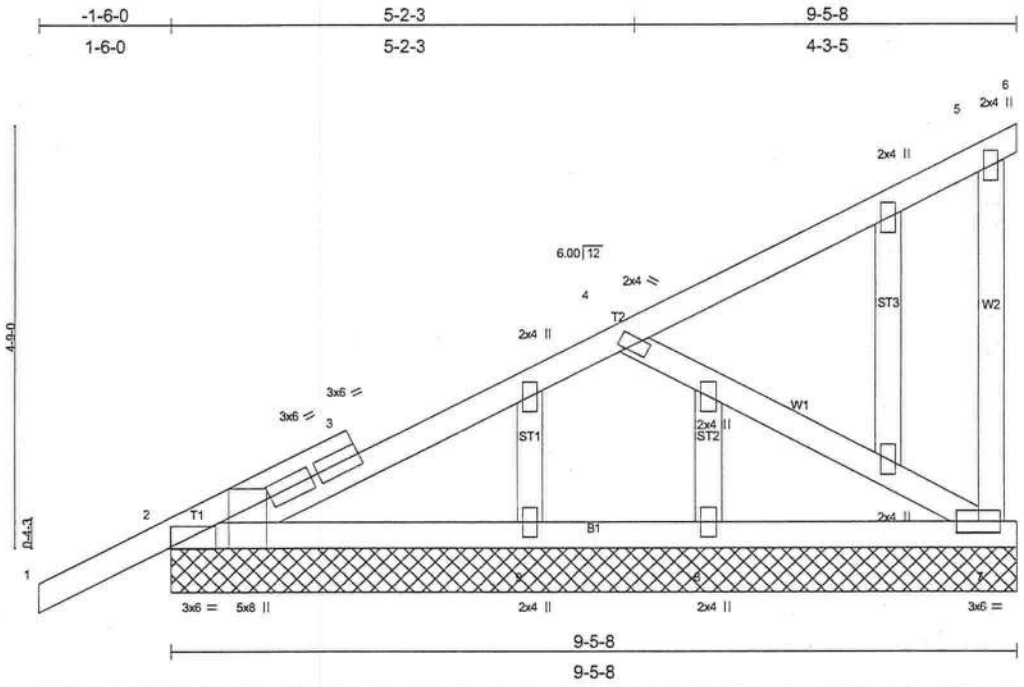


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [3:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.02	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	0.03	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.18	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 56 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-0-2 oc bracing.

REACTIONS (lb/size) 2=522/9-5-8, 6=-161/9-5-8, 7=582/9-5-8, 9=147/9-5-8, 8=52/9-5-8
Max Horz 2=298(load case 5)
Max Uplift2=-313(load case 5), 6=-161(load case 1), 7=-486(load case 5), 9=-12(load case 5)
Max Grav 2=522(load case 1), 6=161(load case 5), 7=582(load case 1), 9=147(load case 1), 8=52(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/51, 2-3=-506/240, 3-4=-450/238, 4-5=-92/54, 5-6=-82/100, 5-7=-323/433
BOT CHORD 2-9=-462/402, 8-9=-462/402, 7-8=-462/402
WEBS 4-7=-447/515

JOINT STRESS INDEX
2 = 0.37, 2 = 0.00, 3 = 0.00, 3 = 0.41, 3 = 0.41, 4 = 0.27, 5 = 0.42, 7 = 0.23, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00, 12 = 0.00 and 13 = 0.00

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Gable requires continuous bottom chord bearing.
4) Gable studs spaced at 2-0-0 oc.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 2, 161 lb uplift at joint 6, 486 lb uplift at joint 7 and 12 lb uplift at joint 9.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

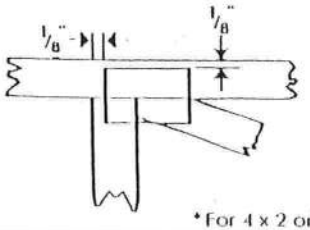
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-79(F=-25), 5-6=-79(F=-25), 2-7=-30

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

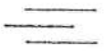
Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

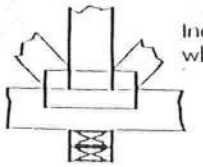
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

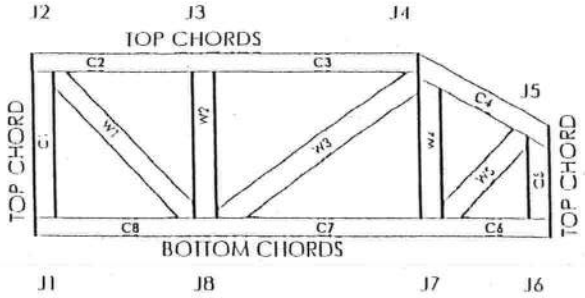
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

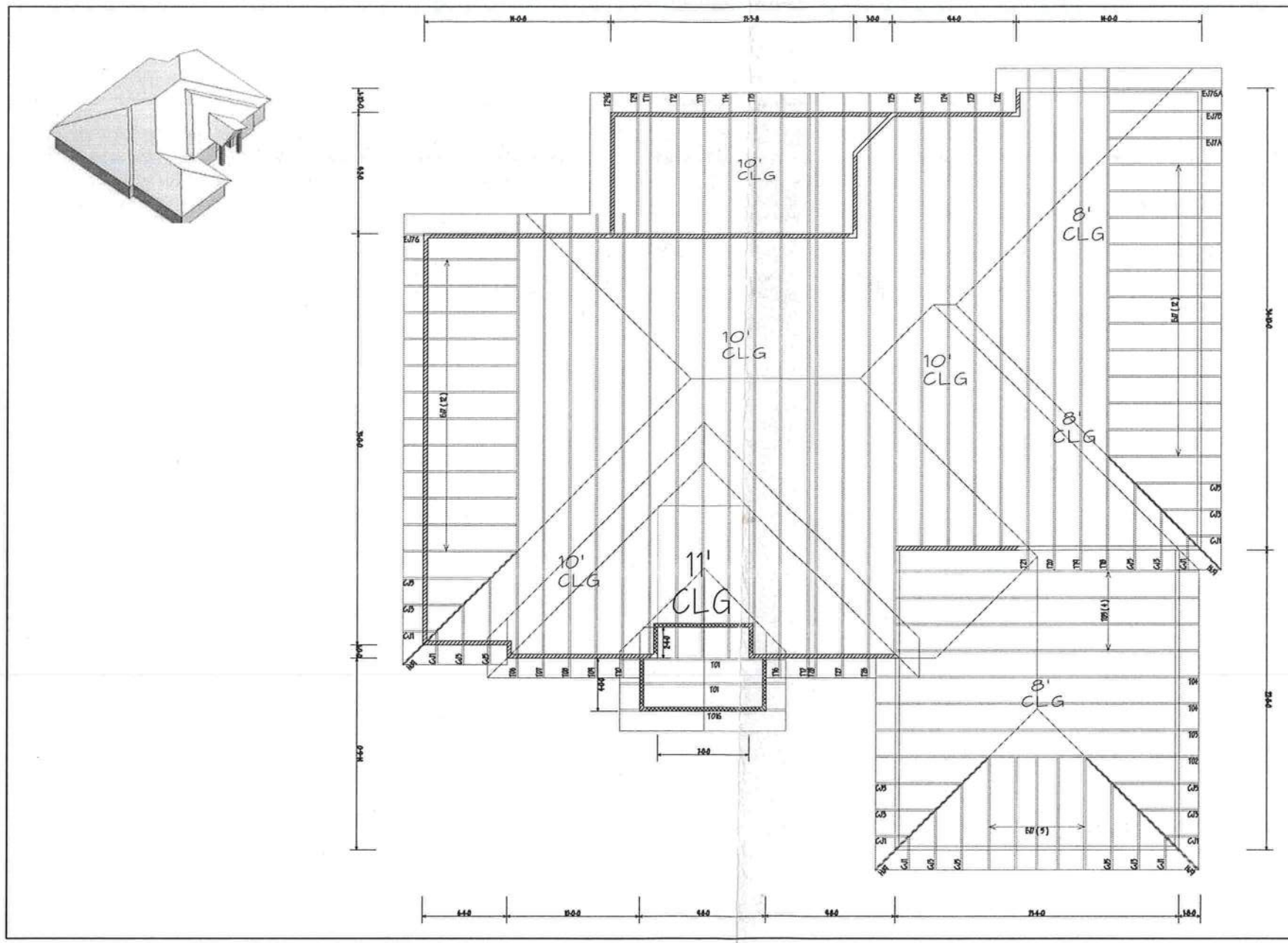
BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.



BEARING HEIGHT SCHEDULE

8'-0"

10'-0"

11'-0"

1'4" OH
6/12 PITCH

NOTES:

1) REFER TO HIB-91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL VIO5 FOR ALTERNATE BRACING REQUIREMENTS.

3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4) ALL TRUSSES ARE DESIGNED FOR 2 o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6) ST42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO ENSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved By: _____ Date: _____

Builders FirstSource

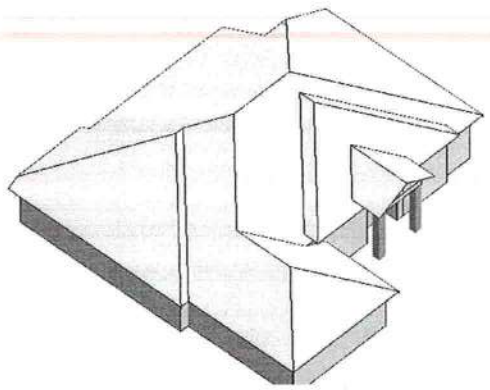
Bunnell
PHONE: 904-437-3348 FAX: 904-437-3984

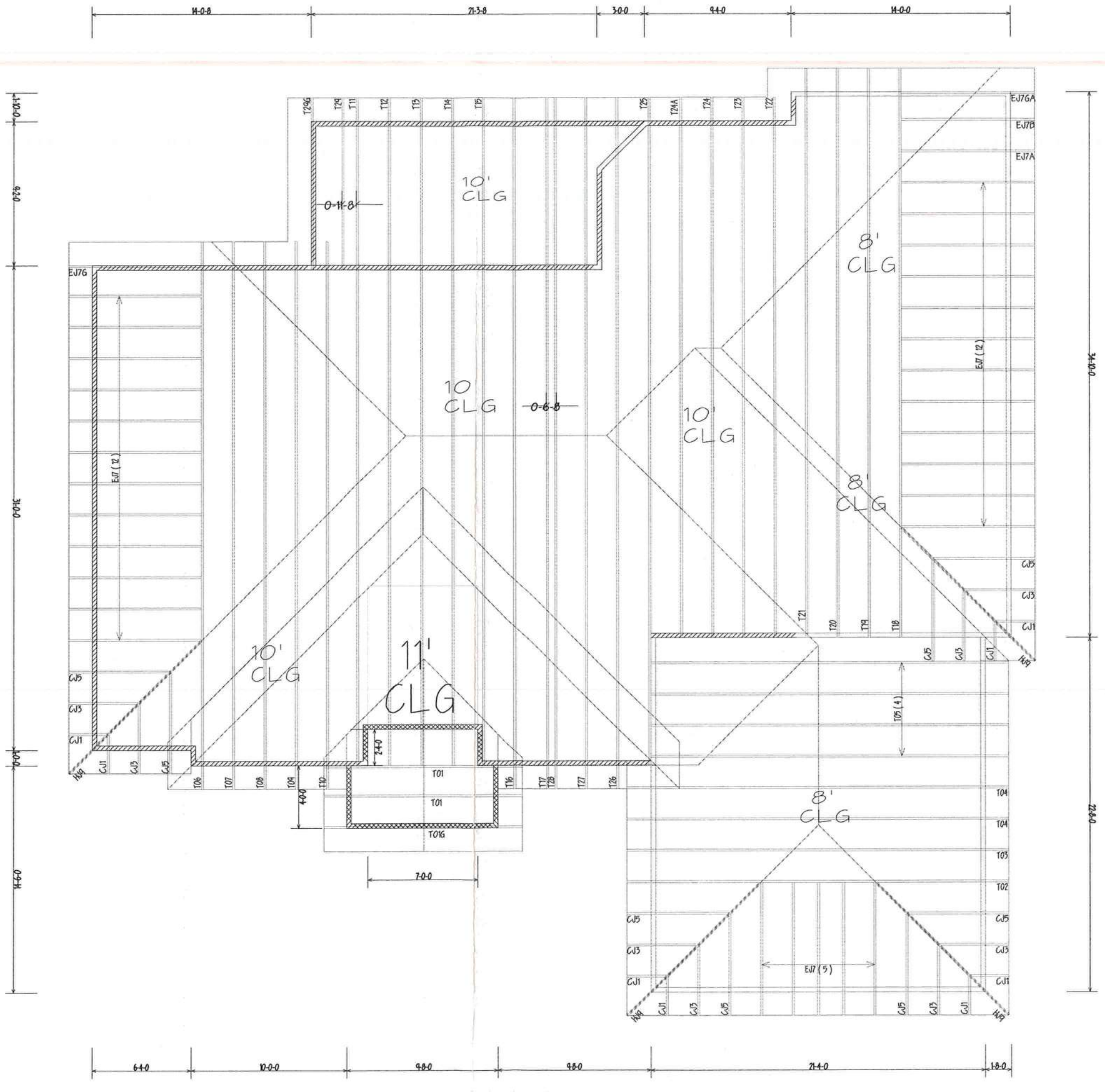
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7473

Sanford
PHONE: 407-322-0098 FAX: 407-322-9993

BUILDER:	HUGO ESCALANTE
LEGAL ADDRESS:	LOT 37 WISE ESTATES
MODEL: NATHAN	ELEVATION:
DATE: 10-25-05	DRAWN BY: JRD
SCALE: NTS	JOB #: L135114





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Requested Delivery Date : _____

Approved by: _____ Date: _____

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PHONE: 904-772-6100 FAX: 904-77

Lake City
PHONE: 904-755-6894 FAX: 904-75

Sanford
PHONE: 407-322-0059 FAX: 407-32

BUILDER:	HUGO ESCALANTE
LEGAL ADDRESS:	LOT 36 WISE ESTATES
MODEL:	NATHAN
REVISION:	
SCALE:	1
DATE:	10-25-05
DRAWN BY:	JRD
JOB #:	L135

Notice of Treatment 11823

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: BAYA Ave
City LC Phone 752-1703

Site Location: Subdivision Wise Estates
Lot # 36 Block# C Permit # 23821
Address 268 SW Plateau Gln

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☑ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☑ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling	2640	814	4.5

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

2-15-06 (Date) 1445 (Time) F254 Gunny (Print Technician's Name)

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink

10/05 ©