GENERAL NOTES

A) THE STRUCTURE UNDER THIS CONTRACT HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS STIPULATED IN THE CONTRACT AND SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CIVIL OR STRUCTURAL ENGINEER. THE BUILDING MANUFACTURER WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED.

B) THIS METAL BUILDING IS DESIGNED WITH THE BUILDING MANUFACTURER'S STANDARD PRACTICES WHICH ARE BASED ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES.

1. AMERICAN INSTITUTE OF STEEL CONSTRUCTION: " AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS— ALLOWABLE STRESS DESIGN" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
2. AMERICAN IRON AND STEEL INSTITUTE: "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
3. AMERICAN WELDING SOCIETY: "STRUCTURAL WELDING CODE" AWS D1.1. AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAL" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.

C) 1) MATERIAL PROPERTIES OF STEEL PLATE USED IN THE FABRICATION OF PRIMARY RIGID FRAMES, AND OTHER PRIMARY STRUCTURAL EXCLUSIVE OF COLD-FORMED SECTIONS, CONFORM TO ASTM-A529 OR A572 . FLANGES AND WEB MATERIAL CONFORMS TO ASTM-A529 OR A572 GRADE 55 WITH A MINIMUM YIELD POINT OF 55,000 psi.

2) MATERIAL PROPERTIES OF HSS ROUND SECTIONS CONFORM TO ASTM-A500, GRADE B OR C WITH A MINIMUM YIELD POINT OF 42,000 psi.

3) MATERIAL PROPERTIES OF HSS RECT. OR SQUARE SECTIONS CONFORM TO ASTM-A500, GRADE B OR C WITH A MINIMUM YIELD POINT OF 46,000 psi.

4) MATERIAL PROPERTIES OF HOT ROLLED CHANNEL AND ANGLE MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992 WITHMINIMUM YIELD POINT OF 50,000 PSI. HOT ROLLED W-SHAPED MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992WITH MINIMUM YIELD POINT OF 50,000 PSI.

5) MATERIAL PROPERTIES OF COLD FORMED LIGHT GAGE STEEL MEMBERS CONFORM TO EITHER ASTM A653-06 GR 55 OR A1011-04 HSLAS GRADE 55 WITH YIELD OF 55,000 psi.

6) MATERIAL PROPERTIES OF ROOF/WALL SHEETING, BASE METAL CONFORM TO ASTM-A792 GRADES 80 CLASS 1, 2 OR 3 WITH A MINIMUM YIELD STRENGTH OF 80,000 PSI. COATING OF BASE MATERIAL IS 55% ALUMINUM-ZINC ALLOY IN ACCORDANCE WITH AZ55 SPECIFICATIONS.

7) CABLE UTILIZED FOR BRACING CONFORMS TO ASTM A475. CABLE BRACING IS TO BE INSTALLED TO A TAUT CONDITION.

8) ROD UTILIZED FOR BRACING MEMBERS CONFORM TO ASTM-A36 WITH MINIMUM YIELD POINT OF 36,000 PSI.

9) IT IS THE RESPONSIBILITY OF ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE "RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A-325 OR A-490 BOLTS". ALL A-325 BOLTS IN PRIMARY FRAMING MUST BE "SNUG-TIGHT", EXCEPT AS FOLLOWS:
"FULLY-PRETENSION" A-325 BOLTS IF:
   a) BUILDING LOCATED IN A HIGH SEISMIC AREA. FOR IBC-BASED CODE, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF "D", "E" OR "F".
   b) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5.00 TONS.
   c) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS – REVERSALS ON THE CONNECTIONS.
   d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A-325 – SC".

10) SECONDARY MEMBERS AND FLANGE BRACE CONNECTIONS SHALL ALWAYS BE SNUG TIGHT, UNO.

11) ANCHOR BOLTS 3/4" IN DIAMETER THRU 1 1/4" IN DIAMETER CONFORM TO A.S.T.M. F1554 GR. 36. ANCHOR BOLTS 1/2" IN DIAMETER CONFORM TO A.S.T.M. A-307.

D) UNLESS NOTED OTHERWISE ON FRAMING COLOR CHART: ALL STEEL MEMBERS EXCEPT BOLTS, FASTENERS, CABLE AND RODS SHALL RECEIVE ONE COAT OF STANDARD RED OXIDE SHOP PRIMER.

E) SHOP AND FIELD INSPECTIONS AND ASSOCIATED FEES ARE THE RESPONSIBILITY OF THE CONTRACTOR, UNLESS STIPULATED OTHERWISE IN THE CONTRACT.

APPROVAL NOTES

THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS:

A) IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS:
   1) BE MADE IN CONTRASTING INK.
   2) HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED.
   3) BE LEGIBLE AND UNAMBIGUOUS.

B) DATED SIGNATURE IS REQUIRED ON ALL PAGES.

C) MANUFACTURER RESERVES THE RIGHT TO RESUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE DELIVERY SCHEDULE.

D) APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN, OR AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED BY MANUFACTURER.

E) ANY CHANGES NOTED ON THE DRAWINGS NOT IN CONFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION. MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILAR INDICATION OF APPROVAL, DISAPPROVAL ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER.

SAFETY COMMITMENT

A) THE BUILDING MANUFACTURER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF THE BUILDING MANUFACTURER.

B) IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE.

C) LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKER SAFETY.

D) MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES.

E) DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.

ERECTOR / CONTRACTOR RESPONSIBILITIES

A) IT IS THE RESPONSIBILITY OF THE ERECTOR/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE BUILDING MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.

B) THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS REQUIRED.

C) APPROVAL OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BUILDING MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.4.1 AISC CODE OF STANDARD PRACTICES, LATEST ED.)

D) WHERE DISCREPANCIES EXIST BETWEEN THE MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE LATEST ED.)

E) DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE BUILDING MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE BUILDING MANUFACTURER'S ENGINEERS UNLESS SPECIFICALLY INDICATED.

F) THE ERECTOR/CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH THE BUILDING MANUFACTURER'S "FOR CONSTRUCTION" DRAWINGS.

G) PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR IMMEDIATELY UPON ARRIVAL CLAIMS FOR SHORTAGES OR DEFECTIVE MATERIAL IF NOT PACKAGED MUST BE SENT TO THE MANUFACTURER IN WRITING WITHIN FIVE (5) DAYS AFTER RECEIPT OF THE SHIPMENT. HOWEVER, IF A DEFECT IS OF SUCH A NATURE THAT REASONABLE VISUAL INSPECTION WOULD FAIL TO DISCLOSE IT, THEN THE CLAIM MUST BE MADE WITHIN FIVE (5) DAYS AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LIABLE FOR ANY DEFECT UNLESS CLAIM IS MADE WITHIN ONE (1) YEAR AFTER DATE OF THE ORIGINAL SHIPMENT BY THE MANUFACTURER TO CONTRACTOR OR HIS CUSTOMER. THE MANUFACTURER WILL BE GIVEN A REASONABLE OPPORTUNITY TO INSPECT DEFECTIVE MATERIALS UPON RECEIPT OF CLAIM BY CONTRACTOR.
IF A DEFECT IS OF SUCH NATURE THAT IT CAN BE REMEDIED BY A FIELD OPERATION AT THE JOB SITE WITHOUT THE NECESSITY OF RETURNING THE MATERIAL TO THE MANUFACTURER, THEN UPON WRITTEN AUTHORIZATION OF THE MANUFACTURER THE CONTRACTOR MAY REPAIR OR CAUSE THE MATERIAL TO BE REPAIRED AND THE MANUFACTURER WILL REIMBURSE THE CONTRACTOR FOR THE COST OF THE REPAIR IN ACCORDANCE WITH THE WRITTEN AUTHORIZATION.

THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS IN TO LINE, MODERATE AMOUNTS OF REAMING,CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.

H) ALL BRACING AS SHOWN AND PROVIDED BY THE MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.

I) TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION OR COLLISION. (SECT. 7.10.3 AISC CODE OF STANDARD PRACTICE, LATEST ED.)

J) METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DESIGN,MATERIAL AND WORKMANSHIP OF FOUNDATION. ANCHOR BOLT PLANS PREPARED BY MBM ARE INTENDED TO SHOW ONLY LOCATION, DIAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED TO ATTACH THE METAL BUILDING SYSTEM TO FOUNDATION. IT IS RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE PROVISIONS ARE MADE FOR SPECIFYING ROD EMBEDMENT, BEARING VALUES, TIE RODS AND OTHER ASSOCIATED ITEMS EMBEDDED IN THE CONCRETE FOUNDATION, AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY MB SYSTEM, OTHER IMPOSED LOAD, AND THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE (MBMA 06 SECTIONS 3.2.2 AND A3)

K) METAL BUILDING MANUFACTURER DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION, NOR DOES MBM PERFORM ANY INSPECTIONS DURING OR AFTER ERECTION.

Columbia County Building Department
Plans Reviewed for Code Compliance
State of Florida

FLORIDA PRODUCT APPROVAL NUMBER	
APEC PC216	42382.11
PBR WALL PANEL	36876.1
26 GA. 7.2	17662.1

IT IS THE RESPONSIBILITY OF THE CUSTOMER TO PROVIDE ALL DOCUMENTATION REQUIRED FOR ANY ACCESSORIES NOT PROVIDED BY MBM TO THEIR LOCAL PERMITTING OFFICE. ALL ACCESSORIES MUST COMPLY AND MEET ALL DESIGN REQUIREMENTS PER LOCAL CODES.

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

*ADDITIONAL WALL WEIGHT CONSIDERED IN SEISMIC DESIGN.

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

Richard T Smith

Digitally signed by Richard T Smith
DN: c=US, serialNumber=AATL20240726359737, cn=Richard T Smith, email=rtsmith@smitheng.net
Date: 2025.10.29 13:04:42 -04'00'

This item has been digitally signed and sealed by Richard T Smith on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

BAM BUILDINGS AND MORE

DRAWING INDEX

IAS ACCREDITED Metal Building Systems AC 472

BUILDING DESIGNED & MANUFACTURED BY AN IAS ACCREDITED FACILITY.

FRAMING COLORS

Rigid Frame: GP
Flange brace: GP
Angle: GP

RO – Red Oxide
GP – Grey Primer
GZ – Galvanized

	Grt	Purl	EvSt	Jmb	BB	Endwall Col	Endwall Raf
U SECTION:	GP	GP	GP	GP	GP	GP	GP
C SECTION:	GP	GP	GP	GP	GP	GP	GP
D SECTION:	GP	GP	GP	GP	GP	GP	GP
Z SECTION:	GP	GP	GP	GP	GP	GP	GP
E SECTION:	GP	GP	GP	GP	GP	GP	GP
R SECTION:	GP	GP	GP	GP	GP	GP	GP
W SECTION:	GP	GP	GP	GP	GP	GP	GP

WHEN GALVANIZED PROVIDED: ALL FINISHED PRIMARY BUILT-UP AND HOT ROLL MEMBERS ARE HOT DIPPED GALVANIZED. ALL SECONDARY COLD FORMED MEMBERS ARE PRE-GALVANIZED.

DRAWING STATUS

☐ FOR APPROVAL:
THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.

☒ FOR PERMIT:
THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL IN THAT, AS A MINIMUM, PIECE MARKINGS ARE NOT IDENTIFIED. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.

☐ FOR CONSTRUCTION: THESE DRAWINGS ARE FINAL AND ISSUED FOR FIELD USE FOR BUILDING ERECTION

ISSUE	DET	CHK	DATE

FOR: PINNACLE SITE SOLUTIONS
19816 S US HWY 441
HIGH SPRINGS, FL 32643
JOBSITE: HIGH SPRINGS, FL 32643

FROM: BUILDINGS AND MORE
792 SW BASCOM NORRIS DR.
LAKE CITY, FL 32025

JOB NO : 9249R1
DATE : 10/13/25
BY : DJH
SCALE : NONE
TITLE : COVER PAGE
NUMBER : PAGE 0A

BUILDING "A" PROFILE

A

Width (ft)	= 50	Eave Height (ft)	= 21
Length (ft)	= 251.33	Roof Slope (Rise/12)	= 3.0:12

BUILDING LOADS

A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY FBC 23 / 8TH EDITION

B) THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE LOCAL BUILDING DEPT.

OCCUPANCY/RISK CATEGORY	II – Normal	Is 1.0000	Ie 1.00		
WIND LOAD	ULTIMATE 122 MPH	NOMINAL 94.50 MPH	WIND EXPOSURE C		
CLOSURE TYPE	Enclosed	INTERNAL WIND COFF.	−0.18 / 0.18		
GROUND SNOW LOAD	0.00 PSF	ROOF SNOW LOAD 0.00 PSF	Ce 1.0000	Ct 1.2000	
SNOW BANKING LOADS	PER CODE				
COLLATERAL DEAD LOAD	3.00 PSF				
ROOF LIVE LOAD	20.00 PSF	(REDUCIBLE Yes)			
DEAD LOAD	3.75 PSF	(FOR ROOF PANELS AND PURLINS)			
SEISMIC					
SPECTRAL RESPONSE	Ss 0.1100	S1 0.0550	Sds 0.1000	Sd1 0.0800	
SITE CLASS D	DESIGN RISK CATEGORY B		Cs 0.0334		
RESPONSE MODIFICATION FACTOR, R	3.000* FRAMES	3.000* BRACING			

BASIC SEISMIC FORCE RESISTING SYSTEM (LATERAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES
BASIC SEISMIC FORCE RESISTING SYSTEM (ENDWALLS) = ORDINARY STEEL MOMENT BRACED FRAMES
BASIC SEISMIC FORCE RESISTING SYSTEM (LONGITUDINAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES
ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE

SERVICEABILITY CRITERIA

* STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

MINIMUM DESIGN DEFLECTIONS			
Endwall Column	= 180	Roof Panel (Live)	= 60
Endwall Rafter (Live)	= 180	Roof Panel (Wind)	= 60
Endwall Rafter (Wind)	= 180	Rigid Frame (Horz)	= 100
Wall Girt	= 90	Rigid Frame (Vert)	= 180
Roof Purlin (Live)	= 150	Rigid Frame (Seismic)	= 50
Roof Purlin (Wind)	= 150	MEZZANINE (Dead + Live)	= 240
Wall Panel	= 60	MEZZANINE (Live)	= 360

COMPONENTS & CLADDING (unfactored)				
Wall Field Values	= 35.583 psf	/	−38.599 psf	
Wall Edge Values	= 35.583 psf	/	−47.645 psf	

THIS PROJECT IS DESIGNED AS AN ENCLOSED BUILDING. ACCESSORIES (DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF REFERENCED BUILDING CODE. [3]

THE MBM DOES NOT CONSIDER VIBRATION ANALYSIS OF MEZZANINE SYSTEMS. [18]

MEZZANINE LOADINGS

DEAD LOAD = 50 psf. (4" CONC., DECK & JOIST)
LIVE LOAD = 100 psf. (UNREDUCIBLE)
COLL. LOAD = 1 psf.

BUILDING "B" PROFILE

B

Width (ft)	= 25	Eave Height (ft)	= 16.13 H/S
Length (ft)	= 76	Roof Slope (Rise/12)	= 1.5:12.0

BUILDING LOADS

A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY FBC 23 / 8TH EDITION

B) THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE LOCAL BUILDING DEPT.

OCCUPANCY/RISK CATEGORY	II – Normal	Is 1.0000	Ie 1.00		
WIND LOAD	ULTIMATE 122 MPH	NOMINAL 94.50 MPH	WIND EXPOSURE C		
CLOSURE TYPE	Partially Enclosed	INTERNAL WIND COFF.	−0.55 / 0.55		
GROUND SNOW LOAD	0.00 PSF	ROOF SNOW LOAD 0.00 PSF	Ce 1.0000	Ct 1.2000	
SNOW BANKING LOADS	PER CODE				
COLLATERAL DEAD LOAD	3.00 PSF				
ROOF LIVE LOAD	20.00 PSF	(REDUCIBLE Yes)			
DEAD LOAD	4.00 PSF	(FOR ROOF PANELS AND PURLINS)			
SEISMIC					
SPECTRAL RESPONSE	Ss 0.1100	S1 0.0550	Sds 0.1000	Sd1 0.0800	
SITE CLASS D	DESIGN RISK CATEGORY B		Cs 0.0334		
RESPONSE MODIFICATION FACTOR, R	3.000* FRAMES	3.000* BRACING			

BASIC SEISMIC FORCE RESISTING SYSTEM (LATERAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES
BASIC SEISMIC FORCE RESISTING SYSTEM (ENDWALLS) = ORDINARY STEEL MOMENT BRACED FRAMES
BASIC SEISMIC FORCE RESISTING SYSTEM (LONGITUDINAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES
ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE

SERVICEABILITY CRITERIA

* STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

MINIMUM DESIGN DEFLECTIONS			
Endwall Column	= 180	Roof Panel (Live)	= 60
Endwall Rafter (Live)	= 180	Roof Panel (Wind)	= 60
Endwall Rafter (Wind)	= 180	Rigid Frame (Horz)	= 100
Wall Girt	= 90	Rigid Frame (Vert)	= 180
Roof Purlin (Live)	= 150	Rigid Frame (Seismic)	= 50
Roof Purlin (Wind)	= 150		
Wall Panel	= 60		

COMPONENTS & CLADDING (unfactored)				
Wall Field Values	= 39.835 psf	/	−42.308 psf	
Wall Edge Values	= 39.835 psf	/	−49.726 psf	

THIS PROJECT IS DESIGNED AS A PARTIALLY ENCLOSED BUILDING AS DEFINED BY THE REFERENCED BUILDING CODE. [5]

THE MBM DOES NOT CONSIDER VIBRATION ANALYSIS OF MEZZANINE SYSTEMS. [18]

COLORS:

ROOF:	COLOR
WALLS:	COLOR
GABLE:	COLOR
EAVE:	COLOR
CORNER:	COLOR
FRAMED OPENINGS:	COLOR
GUTTER:	COLOR
DOWNSPOUTS:	COLOR
BASE:	COLOR

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS
JOB NO: 9249R1 | DATE: 10/13/25
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: COVER PAGE
DRAWING NO: PAGE 0B | DRAWN BY: DJH | CHECKED BY: SPW | SCALE: NONE

BUILDING "C" PROFILE

C

Width (ft) = 25	Eave Height (ft) = 16.13 H/S
Length (ft) = 76	Roof Slope (Rise/12) = 1.5:12.0

BUILDING LOADS

A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY FBC 23 / 8TH EDITION

B) THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE LOCAL BUILDING DEPT.

OCCUPANCY/RISK CATEGORY: II – Normal Is 1.0000 Ie 1.00

WIND LOAD: ULTIMATE 122 MPH NOMINAL 94.50 MPH WIND EXPOSURE C

CLOSURE TYPE: Enclosed INTERNAL WIND COEF. −0.18 / 0.18

GROUND SNOW LOAD: 0.00 PSF ROOF SNOW LOAD 0.00 PSF Ce 1.0000 Ct 1.2000

SNOW BANKING LOADS: PER CODE

COLLATERAL DEAD LOAD: 3.00 PSF

ROOF LIVE LOAD: 20.00 PSF (REDUCIBLE Yes)

DEAD LOAD: 2.75 PSF (FOR ROOF PANELS AND PURLINS)

SEISMIC

SPECTRAL RESPONSE Ss 0.1100 S1 0.0550 Sds 0.1000 Sd1 0.0800

SITE CLASS D DESIGN RISK CATEGORY B Cs 0.0334

RESPONSE MODIFICATION FACTOR, R 3.000* FRAMES 3.000* BRACING

BASIC SEISMIC FORCE RESISTING SYSTEM (LATERAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES

BASIC SEISMIC FORCE RESISTING SYSTEM (ENDWALLS) = ORDINARY STEEL MOMENT BRACED FRAMES

BASIC SEISMIC FORCE RESISTING SYSTEM (LONGITUDINAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES

ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE

SERVICEABILITY CRITERIA

* STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

MINIMUM DESIGN DEFLECTIONS			
Endwall Column	= 180	Roof Panel (Live)	= 60
Endwall Rafter (Live)	= 180	Roof Panel (Wind)	= 60
Endwall Rafter (Wind)	= 180	Rigid Frame (Horz)	= 100
Wall Girt	= 90	Rigid Frame (Vert)	= 180
Roof Purlin (Live)	= 150	Rigid Frame (Seismic)	= 50
Roof Purlin (Wind)	= 150		
Wall Panel	= 60		

COMPONENTS & CLADDING (unfactored)				
Wall Field Values	= 29.670 psf	/	−32.143 psf	
Wall Edge Values	= 29.670 psf	/	−39.561 psf	

THIS PROJECT IS DESIGNED AS AN ENCLOSED BUILDING. ACCESSORIES (DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF REFERENCED BUILDING CODE. [3]

THE MBM DOES NOT CONSIDER VIBRATION ANALYSIS OF MEZZANINE SYSTEMS. [18]

BUILDING "D" PROFILE

D

Width (ft) = 28	Eave Height (ft) = 10
Length (ft) = 12	Roof Slope (Rise/12) = 6.0:12

BUILDING LOADS

A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY FBC 23 / 8TH EDITION

B) THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE LOCAL BUILDING DEPT.

OCCUPANCY/RISK CATEGORY: II – Normal Is 1.0000 Ie 1.00

WIND LOAD: ULTIMATE 122 MPH NOMINAL 94.50 MPH WIND EXPOSURE C

CLOSURE TYPE: Enclosed INTERNAL WIND COEF. −0.18 / 0.18

GROUND SNOW LOAD: 0.00 PSF ROOF SNOW LOAD 0.00 PSF Ce 1.0000 Ct 1.2000

SNOW BANKING LOADS: PER CODE

COLLATERAL DEAD LOAD: 1.00 PSF

ROOF LIVE LOAD: 20.00 PSF (REDUCIBLE Yes)

DEAD LOAD: 2.50 PSF (FOR ROOF PANELS AND PURLINS)

SEISMIC

SPECTRAL RESPONSE Ss 0.1100 S1 0.0550 Sds 0.1000 Sd1 0.0800

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Endwall Rafter (Wind)	= 180	Rigid Frame (Horz)	= 100
Wall Girt	= 90	Rigid Frame (Vert)	= 180
Roof Purlin (Live)	= 150	Rigid Frame (Seismic)	= 50
Roof Purlin (Wind)	= 150		
Wall Panel	= 60		

COMPONENTS & CLADDING (unfactored)				
Wall Field Values	= 29.670 psf	/	−29.670 psf	
Wall Edge Values	= 29.670 psf	/	−54.396 psf	

1.0 PSF COLL ONLY ALLOW LIGHTING AND HVAC DUCT TO HANG FROM ROOF SYSTEMS SUSPENSION OF ANY LOAD INDUCING SYSTEM IS EXPLICITLY PROHIBITED, UNLESS A CORRESPONDING REDUCTION IN CERTIFIED LIVE/SNOW LOADS CAN BE PERMITTED BY CODE. [8M]

THE MBM DOES NOT CONSIDER VIBRATION ANALYSIS OF MEZZANINE SYSTEMS. [18]

COLORS:

ROOF:	COLOR
WALLS:	COLOR
GABLE:	COLOR
EAVE:	COLOR
CORNER:	COLOR
FRAMED OPENINGS:	COLOR
GUTTER:	COLOR
DOWNSPOUTS:	COLOR
BASE:	COLOR

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

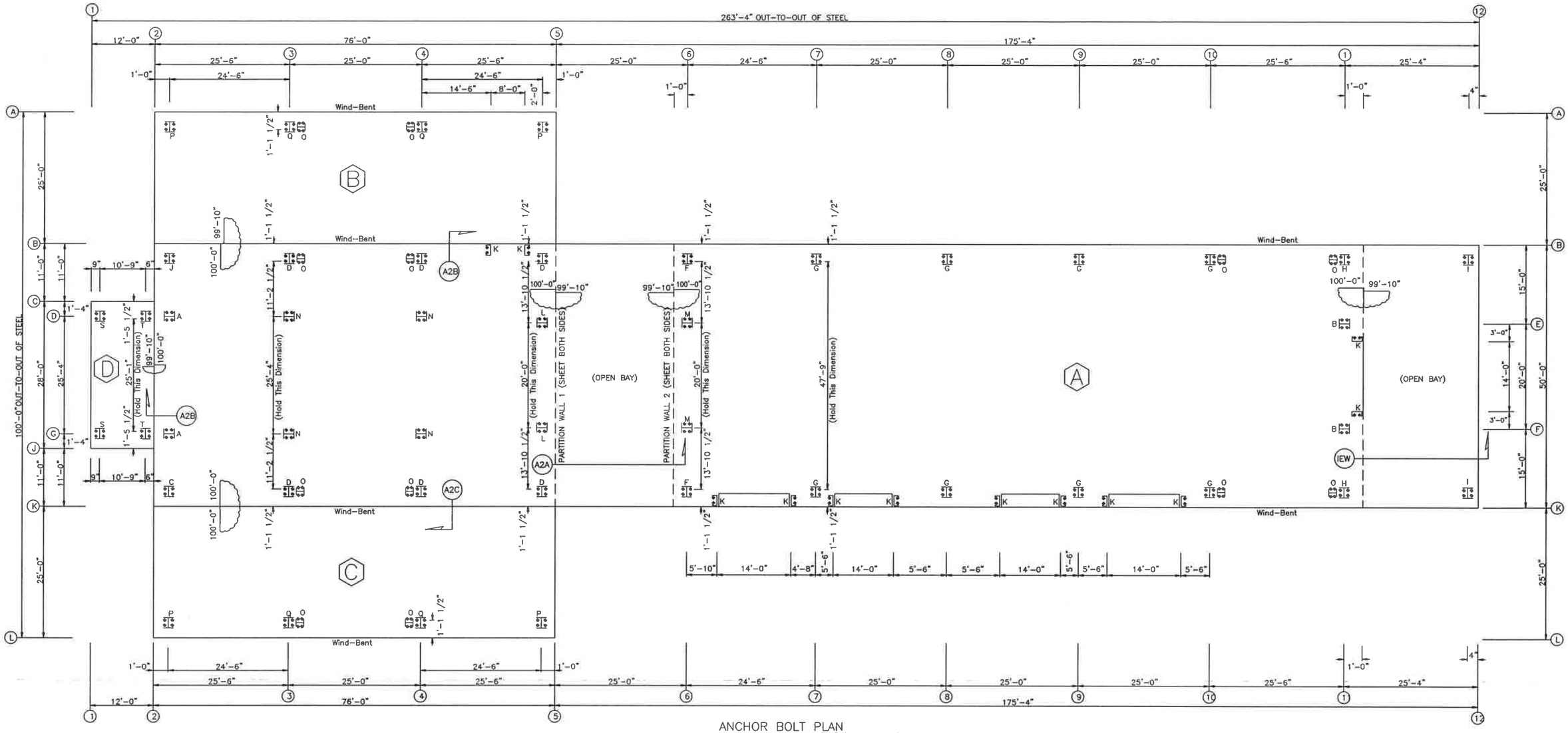
JOB NO: 9249R1 DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: COVER PAGE

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 0C	DJH	SPW	NONE

⊕ Dia= 5/8"
⊗ Dia= 3/4"



ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (Unless Noted)

NOTE: ALL FIELD LOCATED FRAMED OPENING LOCATIONS SHALL BE AT THE DISCRETION OF THE ERECTOR/CUSTOMER. IT IS RECOMMENDED THAT THESE ANCHORS BE LOCATED AT TIME OF ERECTION.

FIELD LOCATE:
(18) 3W x 7L FRAMED OPENINGS
(1) 8'-0" x 8'-0" FRAMED OPENING
(7) 3'-4" x 7'-2" FRAMED OPENINGS
(3) 6'-4" x 7'-2" FRAMED OPENINGS

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS
JOB NO: 9249R1
DATE: 10/13/25
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: ANCHOR BOLT LAYOUT
DRAWING NO: PAGE 1
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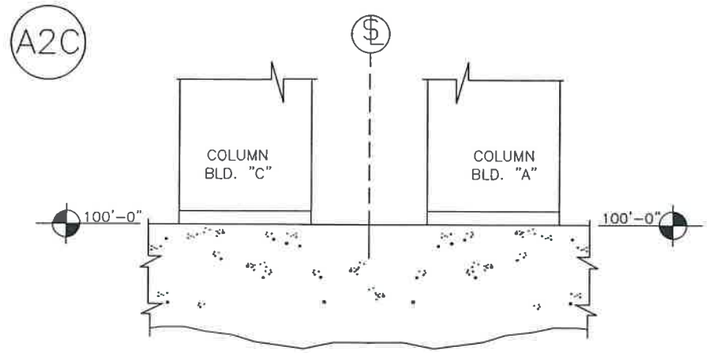

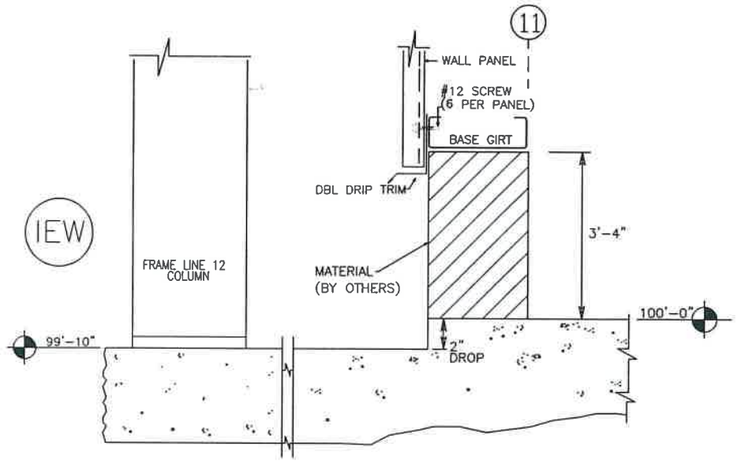

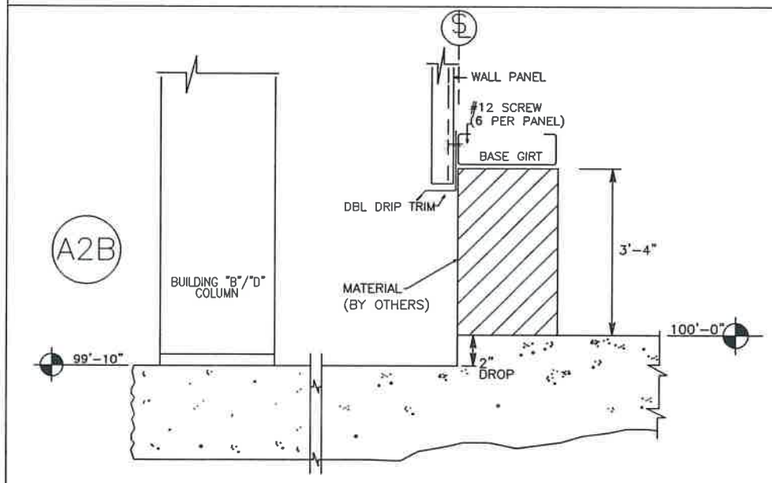

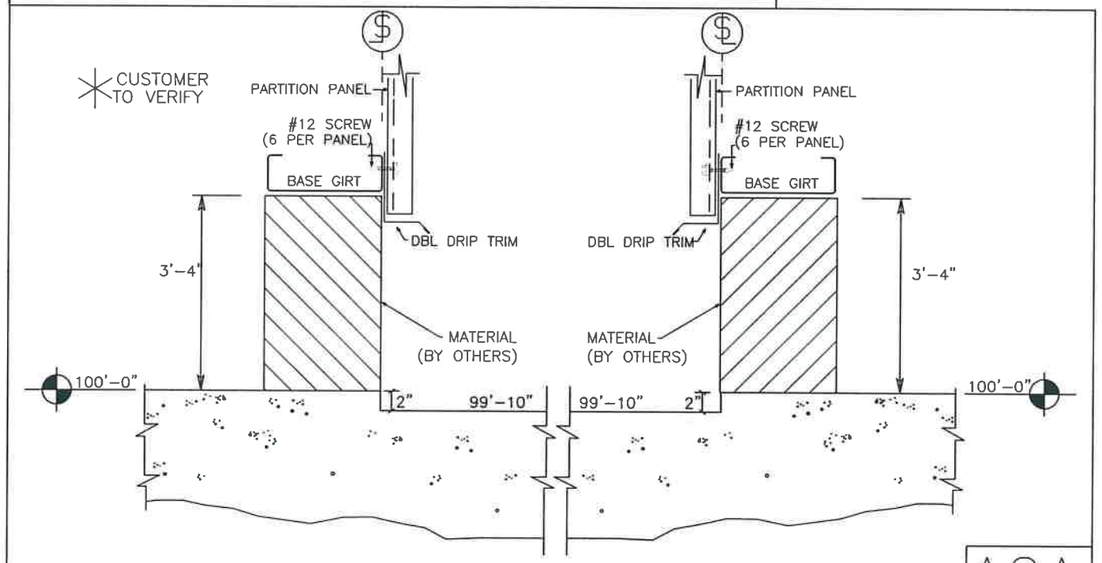

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: ANCHOR BOLT DETAILS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 1.1	DJH	SPW	NONE

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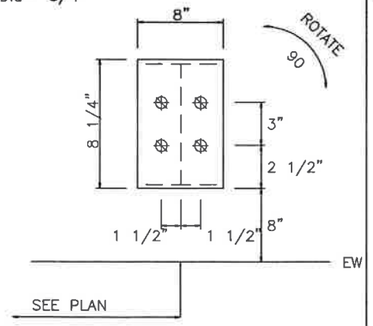

DETAIL A

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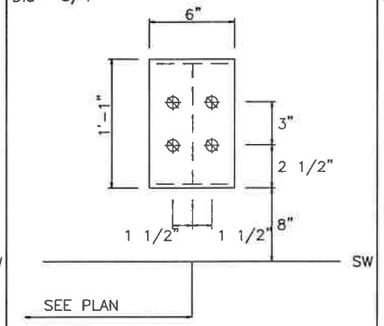

DETAIL D

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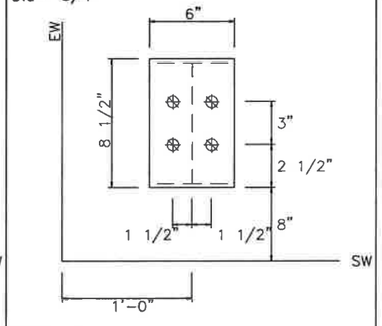
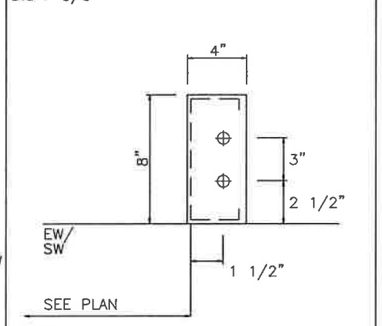


DETAIL H

Dia= 5/8"



DETAIL K

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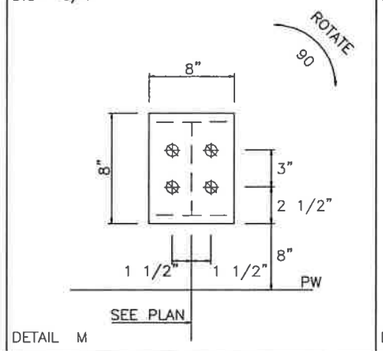

DETAIL M

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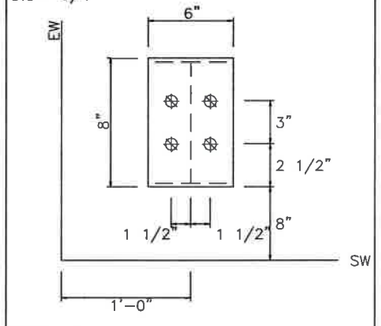

DETAIL P

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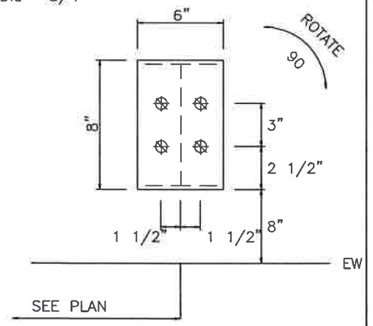

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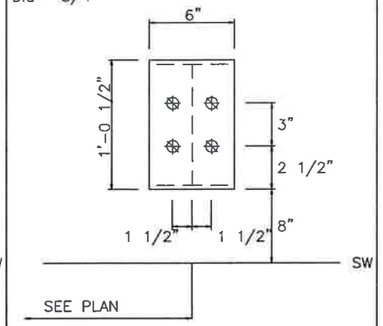

DETAIL F

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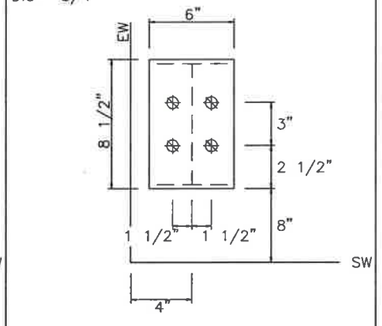

DETAIL I

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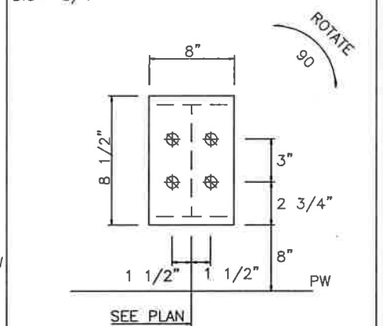

DETAIL L

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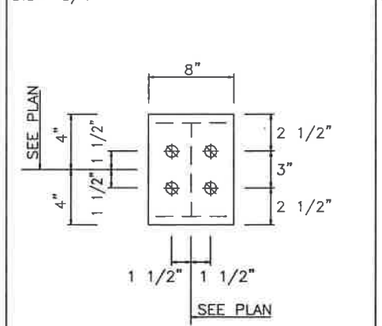

DETAIL N

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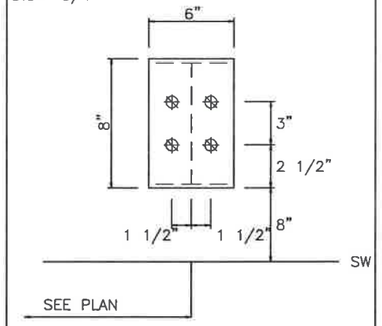

DETAIL Q

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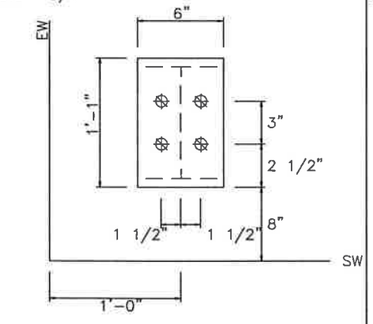

DETAIL C

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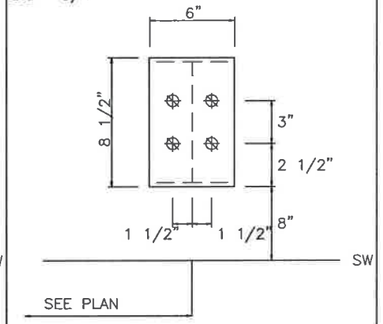

DETAIL G

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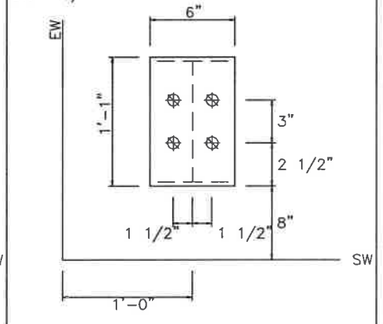

DETAIL J

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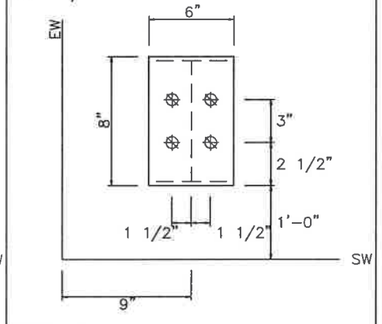

DETAIL S

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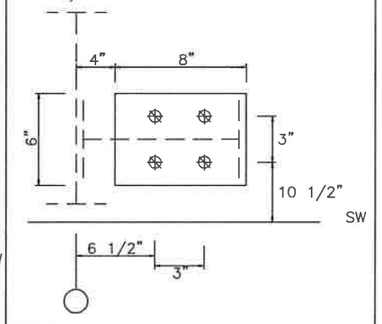

DETAIL O

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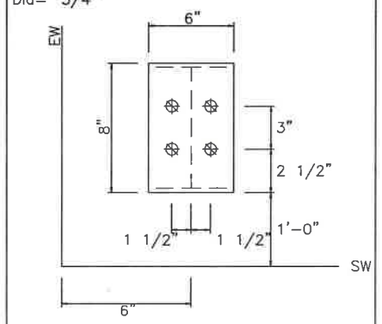
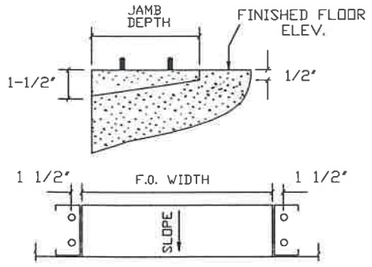
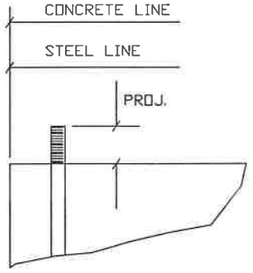


DETAIL T



TYP. O.H. DOOR RECESS DETAIL



ANCHOR BOLT PROJECTION

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

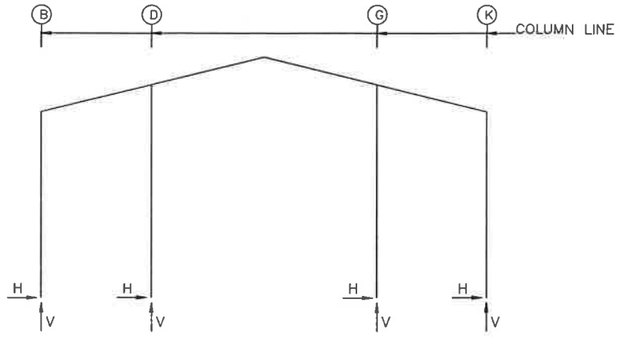
JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: ANCHOR BOLT DETAILS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 1.2	DJH	SPW	NONE

FRAME LINES: 2



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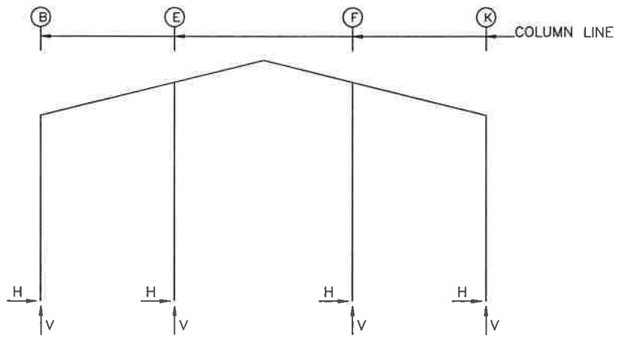

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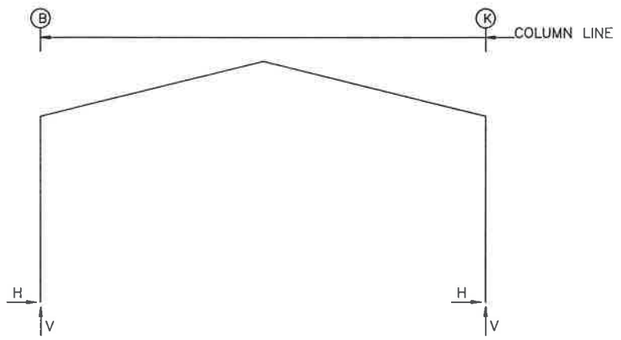


NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width	(ft)	=	50.0
Length	(ft)	=	251.3
Eave Height	(ft)	=	21.0/ 21.0
Roof Slope	(rise/12)	=	3.00/ 3.00
Roof Dead Load	(psf)	=	3.8
Wall Dead Load			
Left Endwall	(psf)	=	13.0
Right Endwall	(psf)	=	13.0
Front Sidewall	(psf)	=	8.0
Back Sidewall	(psf)	=	8.0
Roof Live Load	(psf)	=	20.0
Collateral Load	(psf)	=	3.0
Wind Speed	(mph)	=	122.0
Wind Code		=	FBC 23 (8th Edition)
Exposure		=	C
Closure		=	Enclosed
Internal Wind Coeff		=	−0.18, +0.18
Risk Category		=	II − Normal
Importance − Wind		=	1.00
Importance − Seismic		=	1.00
Seismic Design Category		=	B
Seismic Coeff	(Sms)	=	0.15

GENERAL NOTES

1. FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
2. ALL REACTIONS ARE UNFACTORED.
3. ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
4. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/− 1/8" IN BOTH ELEVATION AND LOCATION.
5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Seis	Note
L_EW	2								(h)
F_SW	K	3,4							(a)
		10,11							(a)
R_EW	11								(h)
B_SW	B	10,11							(a)
		3,4							(a)

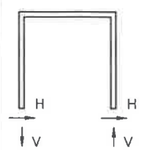
(a)Wind bent in bay
(h)Rigid frame at endwall

FLOOR BRACING REACTIONS

Orient	Offset	Location Start	Location End	± Reactions(k) Seismic Horz	Seismic Vert
Tran	@76.0	B	E	1.1	1.1

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

WIND BENT REACTIONS



Wall Loc	Wall Line	Col Line	Wind(k) Horz	Wind(k) Vert	Seismic(k) Horz	Seismic(k) Vert	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Thick
F_SW	K	3	2.4	3.7	1.6	2.5	4	0.750	6.000	12.000	0.375
F_SW	K	4	2.4	3.7	1.6	2.5	4	0.750	6.000	12.000	0.375
F_SW	K	10	2.4	3.6	1.6	2.5	4	0.750	6.000	12.000	0.375
F_SW	K	11	2.4	3.6	1.6	2.5	4	0.750	6.000	12.000	0.375
B_SW	B	11	2.4	3.6	1.6	2.5	4	0.750	6.000	12.000	0.375
B_SW	B	10	2.4	3.6	1.6	2.5	4	0.750	6.000	12.000	0.375
B_SW	B	4	2.4	3.7	1.6	2.5	4	0.750	6.000	12.000	0.375
B_SW	B	3	2.4	3.7	1.6	2.5	4	0.750	6.000	12.000	0.375

FLOOR COLUMN REACTIONS

Frame Line	Col Line	Max_Vert Ld	(k)	Dead Vert (k)	Coll Vert (k)	Live Vert (k)	Anc._Bolt Qty	Dia	Base Plate (in) Width	Length	Thick	Grout (in)
3	D	2	69.5	23.6	0.5	45.4	4	0.750	8.000	8.000	0.375	0.0
3	G	2	69.5	23.6	0.5	45.4	4	0.750	8.000	8.000	0.375	0.0
4	D	2	69.5	23.6	0.5	45.4	4	0.750	8.000	8.000	0.375	0.0
4	G	2	69.5	23.6	0.5	45.4	4	0.750	8.000	8.000	0.375	0.0

MEZZANINE LOADINGS

DEAD LOAD = 50 psf. (4" CONC., DECK & JOIST)
LIVE LOAD = 100 psf. (UNREDUCIBLE)
COLL. LOAD = 1 psf.

*ADDITIONAL WALL WEIGHT CONSIDERED IN SEISMIC DESIGN.

PARTITION REACTIONS

Part Id	Orient	Max(k) Dead Vert	Max(k) Wind Horz
1	Trans.	0.4	3.9
2	Trans.	0.6	3.9
3	Trans.	0.4	3.9

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

1

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
2	B	4	0.750	6.000	13.00	0.375	0.0
2	K	4	0.750	6.000	13.00	0.375	0.0
2	D	4	0.750	8.000	8.250	0.500	0.0
2	G	4	0.750	8.000	8.250	0.500	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
3*	B	4	0.750	6.000	13.00	0.375	0.0
3*	K	4	0.750	6.000	13.00	0.375	0.0

3* Frame lines: 3 4

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
5	B	4	0.750	6.000	13.00	0.375	0.0
5	K	4	0.750	6.000	13.00	0.375	0.0
5	E	4	0.750	8.000	8.500	0.375	0.0
5	F	4	0.750	8.000	8.500	0.375	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
6	B	4	0.750	6.000	12.50	0.375	0.0
6	K	4	0.750	6.000	12.50	0.375	0.0
6	E	4	0.750	8.000	8.000	0.375	0.0
6	F	4	0.750	8.000	8.000	0.375	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
7*	B	4	0.750	6.000	8.500	0.500	0.0
7*	K	4	0.750	6.000	8.500	0.500	0.0

7* Frame lines: 7 8 9 10

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
11	B	4	0.750	6.000	8.500	0.375	0.0
11	K	4	0.750	6.000	8.500	0.375	0.0
11	E	4	0.750	6.000	8.000	0.375	0.0
11	F	4	0.750	6.000	8.000	0.375	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
12	B	4	0.750	6.000	8.500	0.375	0.0
12	K	4	0.750	6.000	8.500	0.375	0.0

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Projection (in)
⊕ 24	Jamb	5/8"	A307	1.50
⊕ 16	Endwall	3/4"	GR36	1.50
⊕ 104	Frame	3/4"	GR36	2.50
⊕ 32	WindCol	3/4"	GR36	2.50
⊕ 16	Floor	3/4"	GR36	2.50

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Floor Horz	Floor Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert
2	B	−0.1	7.1	−0.1	0.9	−0.3	4.6	0.0	10.2	−1.9	−14.6	4.9	−2.3
2	K	0.1	6.9	0.1	0.9	0.3	4.6	0.0	10.2	−1.6	−3.7	2.6	−15.7
2	D	0.0	14.8	0.0	1.4	0.0	4.6	0.1	24.0	−0.1	−8.8	0.2	−14.5
2	G	0.0	14.8	0.0	1.4	0.0	4.6	−0.1	24.0	−0.1	−10.9	0.2	−5.5

Frame Line	Column Line	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Press Horz	Wind_Press Vert	Wind_Suct Horz	Wind_Suct Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert
2	B	−4.8	−11.5	2.0	0.7	0.0	0.0	0.0	0.0	4.0	−4.4	3.0	−6.1
2	K	−3.6	2.6	0.6	−9.3	0.0	0.0	0.0	0.0	2.2	−10.4	1.2	−8.7
2	D	−0.2	−1.4	0.1	−7.0	−6.0a	0.0	6.6a	0.0	0.2	−13.3	0.1	−7.9
2	G	−0.3	−11.3	0.1	−5.9	−6.0a	0.0	6.6a	0.0	0.2	−1.0	0.1	−6.5

Frame Line	Column Line	Seismic_Left Horz	Seismic_Left Vert	Seismic_Right Horz	Seismic_Right Vert	Seismic_Long1 Horz	Seismic_Long1 Vert	Seismic_Long2 Horz	Seismic_Long2 Vert	F1PAT_LL_1 Horz	F1PAT_LL_1 Vert	F1PAT_LL_2 Horz	F1PAT_LL_2 Vert
2	B	−0.9	−1.6	0.9	1.6	0.0	−1.7	0.0	1.7	−0.2	0.9	−0.3	−0.3
2	K	−0.9	1.6	0.9	−1.5	0.0	−1.7	0.0	1.7	0.3	−0.3	0.2	0.9
2	D	−0.1	1.8	0.1	−1.8	0.1a	−0.1	0.0	0.0	0.0	4.5	0.0	3.0
2	G	−0.1	−1.8	0.1	1.8	0.1a	−0.1	0.0	0.0	0.0	3.0	0.0	4.5

Frame Line	Column Line	F1PAT_LL_3 Horz	F1PAT_LL_3 Vert	F1PAT_LL_4 Horz	F1PAT_LL_4 Vert
2	B	0.0	1.6	−0.3	−0.5
2	K	0.0	1.6	0.3	−0.5
2	D	0.0	1.1	0.0	3.2
2	G	0.0	1.1	0.0	3.2

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Floor Horz	Floor Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert
3*	B	−0.1	14.9	−0.2	3.0	−0.9	12.1	0.4	19.0	−2.3	−28.7	9.2	−16.9
3*	K	0.1	14.5	0.2	3.0	0.9	12.1	−0.4	19.0	−3.2	−16.3	4.6	−28.1

Frame Line	Column Line	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert	Seismic_Right Horz	Seismic_Right Vert
3*	B	−8.2	−16.1	3.3	−4.3	8.0	−21.5	6.5	−19.9	−1.6	1.0	1.6	1.0
3*	K	−7.0	−4.4	0.8	−16.2	3.8	−21.3	2.3	−23.0	−1.6	1.0	1.6	−1.0

Frame Line	Column Line	Seismic_Long1 Horz	Seismic_Long1 Vert	Seismic_Long2 Horz	Seismic_Long2 Vert
3*	B	0.0	−1.8	0.0	1.8
3*	K	0.0	−1.8	0.0	1.8

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Floor Horz	Floor Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert
5	B	−0.1	8.7	0.0	1.3	−0.2	6.3	0.0	12.0	−5.2	−20.2	8.0	−2.6
5	K	0.1	8.4	0.0	1.3	0.2	6.3	0.0	12.0	−6.6	−2.7	6.7	−20.4
5	E	0.0	13.9	0.0	1.6	0.0	5.3	0.0	22.2	−0.2	−3.8	0.3	−16.5
5	F	0.0	13.9	0.0	1.6	0.0	5.3	0.0	22.2	−0.2	−14.2	0.3	−1.5

Frame Line	Column Line	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert	Seismic_Right Horz	Seismic_Right Vert
5	B	−8.2	−14.2	5.1	3.3	4.2	−9.5	2.7	−10.8	−1.5	−2.0	1.5	2.0
5	K	−6.5	3.5	6.8	−14.1	−1.3	−10.9	−2.8	−9.6	−1.5	2.1	1.5	−2.0
5	E	−0.3	2.6	0.2	−10.1	0.1	−12.9	0.0	−5.9	−0.1	2.5	0.1	−2.5
5	F	−0.3	−12.4	0.2	0.3	0.1	−3.6	0.0	−10.6	−0.1	−2.5	0.1	2.5

Frame Line	Column Line	F3PAT_LL_1 Horz	F3PAT_LL_1 Vert	F3PAT_LL_2 Horz	F3PAT_LL_2 Vert	F3PAT_LL_3 Horz	F3PAT_LL_3 Vert	F3PAT_LL_4 Horz	F3PAT_LL_4 Vert
5	B	−0.2	2.2	−0.2	0.0	0.0	2.8	−0.2	−0.3
5	K	0.2	0.0	0.2	2.2	0.0	2.8	0.2	−0.3
5	E	0.0	5.6	0.0	2.9	0.0	1.7	0.0	3.3
5	F	0.0	2.9	0.0	5.6	0.0	1.7	0.0	3.3

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert
6	B	0.0	1.1	0.0	0.5	0.1	3.0	−5.4	−12.6	6.6	0.0	−7.5	−8.8
6	K	0.0	1.1	0.0	0.5	−0.1	3.0	−6.6	0.0	5.4	−12.6	−4.5	3.9
6	E	0.0	2.6	0.0	1.5	0.0	5.6	−4.4	0.0	−14.0	0.0	0.0	−1.4
6	F	0.0	2.6	0.0	1.5	0.0	5.6	0.0	−14.0	0.0	−4.4	0.0	−11.0

Frame Line	Column Line	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert	Seismic_Right Horz	Seismic_Right Vert	F4PAT_LL_1 Horz	F4PAT_LL_1 Vert
6	B	4.5	3.9	4.4	−6.5	2.9	−7.8	−0.3	0.4	0.3	−0.4	0.0	1.6
6	K	7.5	−8.8	−2.9	−7.8	−4.4	−6.5	−0.3	0.4	0.3	−0.4	0.0	−0.5
6	E	0.0	−11.0	0.0	−9.9	0.0	−2.8	0.0	0.6	0.0	−0.6	0.0	6.1
6	F	0.0	−1.4	0.0	−2.8	0.0	−9.9	0.0	−0.6	0.0	0.6	0.0	3.3

Frame Line	Column Line	F4PAT_LL_2 Horz	F4PAT_LL_2 Vert	F4PAT_LL_3 Horz	F4PAT_LL_3 Vert	F4PAT_LL_4 Horz	F4PAT_LL_4 Vert
6	B	0.0	−0.5	0.2	2.5	−0.1	−0.7
6	K	0.0	1.6	−0.2	2.5	0.1	−0.7
6	E	0.0	6.1	0.0	2.0	0.0	3.7
6	F	0.0	6.1	0.0	2.0	0.0	3.7

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert
7*	B	1.1	3.4	0.8	2.0	2.9	8.6	−10.1	−19.8	2.0	−11.2	−10.7	−12.9
7*	K	−1.1	3.4	−0.8	2.0	−2.9	8.6	−2.0	−11.2	10.1	−19.8	−1.4	−4.4

Frame Line	Column Line	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert	Seismic_Right Horz	Seismic_Right Vert
7*	B	1.4	−4.4	1.3	−14.3	−0.3	−12.7	−0.3	−0.2	0.3	0.2
7*	K	10.7	−12.9	0.3	−12.7	−1.3	−14.3	−0.3	0.2	0.3	−0.2

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert
11	B	0.0	1.1	0.0	0.5	0.1	3.0	−4.4	−11.6	5.1	−2.0	−5.8	−9.2
11	K	0.0	1.1	0.0	0.5	−0.1	3.0	−5.1	−2.0	4.4	−11.7	−3.7	0.2
11	E	0.0	2.4	0.0	1.5	0.0	5.6	0.0	−7.3	0.0	−14.1	0.0	−2.1
11	F	0.0	2.4	0.0	1.5	0.0	5.6	0.0	−14.1	0.0	−7.3	0.0	−17.5

Frame Line	Column Line	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Press Horz	Wind_Press Vert	Wind_Suct Horz	Wind_Suct Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert
11	B	3.7	0.2	0.0	0.0	0.0	0.0	2.1	−5.7	1.3	−6.4	−0.3	−0.4
11	K	5.8	−9.2	0.0	0.0	0.0	0.0	−1.3	−6.4	−2.1	−5.7	−0.3	0.4
11	E	0.0	−17.5	−5.8a	0.0	6.4a	0.0	0.0	−10.9	0.0	−7.2	0.0	0.6
11	F	0.0	−2.1	−5.8a	0.0	6.4a	0.0	0.0	−7.2	0.0	−10.9	0.0	−0.6

Frame Line	Column Line	Seismic_Right Horz	Seismic_Right Vert	Seismic_Long1 Horz	Seismic_Long1 Vert	F6PAT_LL_1 Horz	F6PAT_LL_1 Vert	F6PAT_LL_2 Horz	F6PAT_LL_2 Vert	F6PAT_LL_3 Horz	F6PAT_LL_3 Vert	F6PAT_LL_4 Horz	F6PAT_LL_4 Vert
11	B	0.3	0.4	0.0	0.0	0.0	1.6	0.0	−0.5	0.2	2.6	−0.1	−0.7
11	K	0.3	−0.4	0.0	0.0	0.0	−0.5	0.0	1.6	−0.2	2.6	0.1	−0.7
11	E	0.0	−0.6	0.1a	0.0	0.0	5.1	0.0	3.3	0.0	2.0	0.0	3.7
11	F	0.0	0.6	0.1a	0.0	0.0	3.3	0.0	6.1	0.0	2.0	0.0	3.7

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert
12	B	0.7	2.3	0.4	1.2	1.6	5.2	−3.5	−12.2	−4.1	−11.3	−5.0	−10.7
12	K	−0.7	2.3	−0.4	1.2	−1.6	5.2	4.1	−11.3	3.5	−12.2	2.8	−13.2

Frame Line	Column Line	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert	Seismic_Right Horz	Seismic_Right Vert
12	B	−2.8	−13.2	−4.2	−11.7	1.8	4.9	−0.2	−0.1	0.2	0.1
12	K	5.0	−10.7	4.2	−11.7	−1.8	4.9	−0.2	0.1	0.2	−0.1

3* Frame lines: 3 4
7* Frame lines: 7 8 9 10

a − Out−Of−Plane Horizontal Load

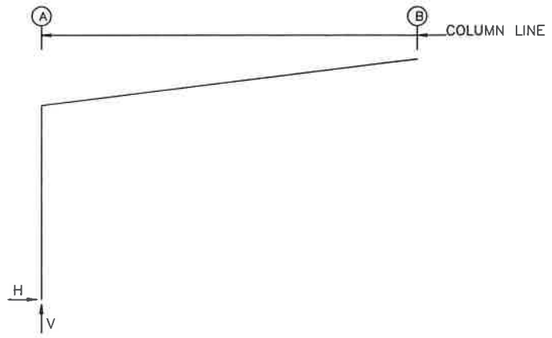
BUILDING "A" REACTIONS

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS
JOB NO: 9249R1
DATE: 10/13/25
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: ANCHOR BOLT REACTIONS
DRAWING NO: PAGE 1.3
DRAWN BY: DJH
CHECKED BY: SPW
SCALE: NONE

FRAME LINES: 2 3 4 5



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
2	A	4	0.750	6.000	8.000	0.375	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
3*	A	4	0.750	6.000	8.000	0.375	0.0

3* Frame lines: 3 4

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
5	A	4	0.750	6.000	8.000	0.375	0.0

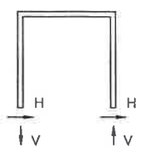
ANCHOR BOLT SUMMARY

	Qty	Locate	Dia (in)	Type	Projection (in)
⊕	16	Frame	3/4"	GR36	2.50
⊕	8	WindCol	3/4"	GR36	2.50

GENERAL NOTES

1. FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
2. ALL REACTIONS ARE UNFACTORED.
3 ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
4. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

WIND BENT REACTIONS



Wall Loc	Wall Line	Col Line	± Reactions Wind(k) Horz	Wind(k) Vert	Seismic(k) Horz	Seismic(k) Vert	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Length	Thick
B_SW	A	4	1.0	0.9	0.2	0.2	4	0.750	6.000	12.000	0.375
B_SW	A	3	1.0	0.9	0.2	0.2	4	0.750	6.000	12.000	0.375

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert
2	A	0.0	1.1	0.0	0.6	0.0	3.9	0.2	−9.3	2.0	−6.2	−2.1	−2.8
3*	A	0.0	1.7	0.0	1.0	−0.1	4.9	0.6	−13.3	3.4	−9.6	−3.7	−2.6
5	A	0.0	1.1	0.0	0.6	0.0	3.9	0.2	−9.3	2.0	−6.2	−2.1	−2.8

Frame Line	Column Line	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert
2	A	−0.3	0.2	2.1	−7.2	2.1	−5.4
3*	A	−0.9	1.1	3.9	−11.9	3.9	−9.0
5	A	−0.3	0.2	2.1	−7.2	2.1	−5.4

3* Frame lines: 3 4

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width	(ft)	=	25.0
Length	(ft)	=	76.0
Eave Height	(ft)	=	13.0/ 16.1
Roof Slope	(rise/12)	=	1.50
Roof Dead Load	(psf)	=	4.0
Wall Dead Load			
Left Endwall	(psf)	=	18.0
Right Endwall	(psf)	=	2.0
Front Sidewall	(psf)	=	0.0
Back Sidewall	(psf)	=	2.0
Roof Live Load	(psf)	=	20.0
Collateral Load	(psf)	=	3.0
Wind Speed	(mph)	=	122.0
Wind Code		=	FBC 23 (8th Edition)
Exposure		=	C
Closure		=	Partially Enclosed
Internal Wind Coeff		=	−0.55, +0.55
Risk Category		=	II − Normal
Importance − Wind		=	1.00
Importance − Seismic		=	1.00
Seismic Design Category		=	B
Seismic Coeff	(Sms)	=	0.15

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Seis	Note
L_EW	2								(h)
F_SW	B								(e)
R_EW	5								(h)
B_SW	A	3,4							(a)

(a)Wind bent in bay
(e)Bracing loads applied to supporting building
(h)Rigid frame at endwall

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE. 1

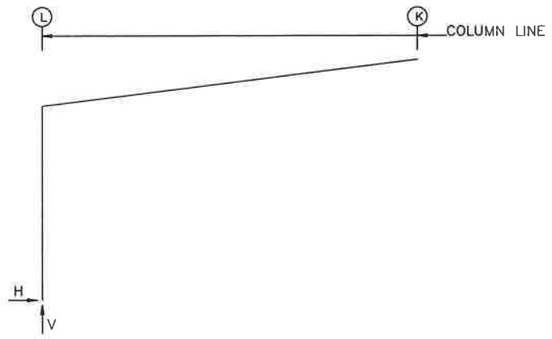
BUILDING "B" REACTIONS

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS
JOB NO: 9249R1
DATE: 10/13/25
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: ANCHOR BOLT REACTIONS
DRAWING NO: PAGE 1.4
DRAWN BY: DJH
CHECKED BY: SPW
SCALE: NONE

FRAME LINES: 5 4 3 2



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
5	L	4	0.750	6.000	8.000	0.375	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
4*	L	4	0.750	6.000	8.000	0.375	0.0

4* Frame lines: 4 3

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
2	L	4	0.750	6.000	8.000	0.375	0.0

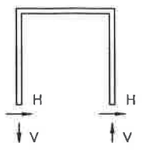
ANCHOR BOLT SUMMARY

	Qty	Locate	Dia (in)	Type	Projection (in)
⊕	16	Frame	3/4"	GR36	2.50
⊕	8	WindCol	3/4"	GR36	2.50

GENERAL NOTES

1. FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
2. ALL REACTIONS ARE UNFACTORED.
3. ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
4. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/− 1/8" IN BOTH ELEVATION AND LOCATION.
5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

WIND BENT REACTIONS



Wall Loc	Wall Line	Col Line	± Reactions Wind(k) Horz	Wind Vert	Seismic(k) Horz	Seismic Vert	Bolt(in) Qty	Bolt Dia	Base_Plate(in) Width	Length	Thick
B_SW	L	3	1.0	0.9	0.4	0.3	4	0.750	6.000	12.000	0.375
B_SW	L	4	1.0	0.9	0.4	0.3	4	0.750	6.000	12.000	0.375

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert
5	L	0.0	0.9	0.0	0.6	0.0	3.9	−0.6	−7.1	1.2	−4.0	−1.4	−5.0
4*	L	0.0	1.3	0.0	1.0	−0.1	4.9	−0.8	−9.7	2.0	−6.0	−2.2	−6.2
2	L	0.0	0.9	0.0	0.6	0.0	3.9	−0.6	−7.1	1.2	−4.0	−1.4	−5.0

Frame Line	Column Line	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert
5	L	0.4	−2.0	1.3	−5.0	1.3	−3.2
4*	L	0.6	−2.5	2.4	−8.3	2.5	−5.4
2	L	0.4	−2.0	1.3	−5.0	1.3	−3.2

4* Frame lines: 4 3

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width	(ft)	=	25.0
Length	(ft)	=	76.0
Eave Height	(ft)	=	13.0/ 16.1
Roof Slope	(rise/12)	=	1.50
Roof Dead Load	(psf)	=	2.8
Wall Dead Load			
Left Endwall	(psf)	=	18.0
Right Endwall	(psf)	=	18.0
Front Sidewall	(psf)	=	0.0
Back Sidewall	(psf)	=	21.0
Roof Live Load	(psf)	=	20.0
Collateral Load	(psf)	=	3.0
Wind Speed	(mph)	=	122.0
Wind Code		=	FBC 23 (8th Edition)
Exposure		=	C
Closure		=	Enclosed
Internal Wind Coeff		=	−0.18, +0.18
Risk Category		=	II − Normal
Importance − Wind		=	1.00
Importance − Seismic		=	1.00
Seismic Design Category		=	B
Seismic Coeff	(Sms)	=	0.15

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Seis	Note
L_EW	5								(h)
F_SW	K								(e)
R_EW	2								(h)
B_SW	L	4,3							(a)

(a)Wind bent in bay
(e)Bracing loads applied to supporting building
(h)Rigid frame at endwall

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE. 1

BUILDING "C" REACTIONS

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

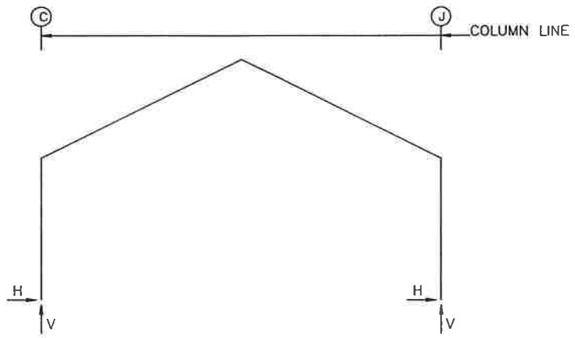
JOB NO: 9249R1 DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: ANCHOR BOLT REACTIONS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 1.5	DJH	SPW	NONE

FRAME LINES: 1 2



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
1	C	4	0.750	6.000	8.000	0.375	0.0
1	J	4	0.750	6.000	8.000	0.375	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
2	C	4	0.750	6.000	8.000	0.375	0.0
2	J	4	0.750	6.000	8.000	0.375	0.0

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Projection (in)
16	Frame	3/4"	GR36	2.50

GENERAL NOTES

1. FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
2. ALL REACTIONS ARE UNFACTORED.
3. ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
4. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel Shear (lb/ft) Wind	Seis	Note
L_EW	1								(h)
F_SW	J								(1)
R_EW	2								(h)
B_SW	C								(1)

(h)Rigid frame at endwall
(1)Building "A" Tie-in

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

1

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert
1	C	0.1	1.1	0.1	0.6	0.3	2.4	−1.2	−2.4	0.8	−1.7	−1.4	−1.3
1	J	−0.1	1.1	−0.1	0.6	−0.3	2.4	−0.8	−1.7	1.2	−2.4	−0.6	−0.5
2	C	0.1	0.5	0.0	0.1	0.4	1.8	−1.2	−1.8	0.6	−1.1	−1.3	−1.0
2	J	−0.1	0.5	0.0	0.1	−0.4	1.8	−0.6	−1.1	1.2	−1.8	−0.5	−0.3

Frame Line	Column Line	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert	Seismic_Right Horz	Seismic_Right Vert	Wind_Left3 Horz	Wind_Left3 Vert
1	C	0.6	−0.5	0.1	−3.9	0.4	0.1	0.0	0.0	0.0	0.0	−1.5	−1.4
1	J	1.4	−1.3	−0.1	−3.9	−0.4	0.1	0.0	0.0	0.0	0.0	−1.2	−0.8
2	C	0.5	−0.3	0.1	−2.8	0.4	0.1	0.0	0.0	0.0	0.0	−1.4	−1.1
2	J	1.3	−1.0	−0.1	−2.8	−0.4	0.1	0.0	0.0	0.0	0.0	−0.9	−0.5

Frame Line	Column Line	Wind_Right3 Horz	Wind_Right3 Vert	Wind_Left4 Horz	Wind_Left4 Vert	Wind_Right4 Horz	Wind_Right4 Vert
1	C	1.2	−0.8	−1.7	−0.2	1.0	0.3
1	J	1.5	−1.4	−1.0	0.3	1.7	−0.2
2	C	0.9	−0.5	−1.5	−0.2	0.8	0.3
2	J	1.4	−1.1	−0.8	0.3	1.5	−0.2

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width	(ft)	=	28.0
Length	(ft)	=	12.0
Eave Height	(ft)	=	10.0/ 10.0
Roof Slope	(rise/12)	=	6.00/ 6.00
Roof Dead Load	(psf)	=	2.5
Wall Dead Load			
Left Endwall	(psf)	=	2.0
Right Endwall	(psf)	=	0.0
Front Sidewall	(psf)	=	2.0
Back Sidewall	(psf)	=	2.0
Roof Live Load	(psf)	=	20.0
Collateral Load	(psf)	=	1.0
Wind Speed	(mph)	=	122.0
Wind Code		=	FBC 23 (8th Edition)
Exposure		=	C
Closure		=	Enclosed
Internal Wind Coeff		=	−0.18, +0.18
Risk Category		=	II − Normal
Importance − Wind		=	1.00
Importance − Seismic		=	1.00
Seismic Design Category		=	B
Seismic Coeff	(Sms)	=	0.15

BUILDING "D" REACTIONS

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

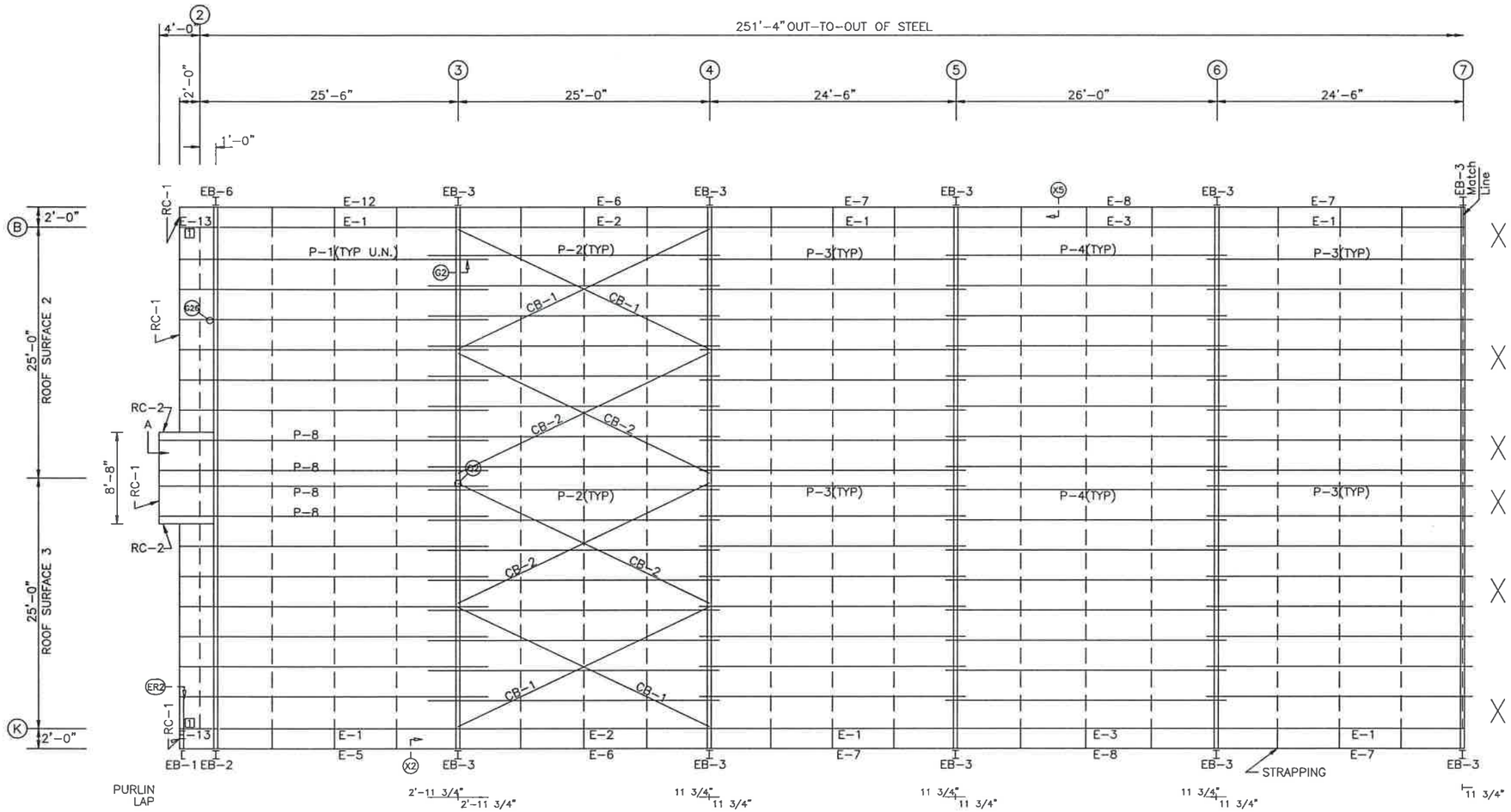
CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: ANCHOR BOLT REACTIONS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 1.6	DJH	SPW	NONE

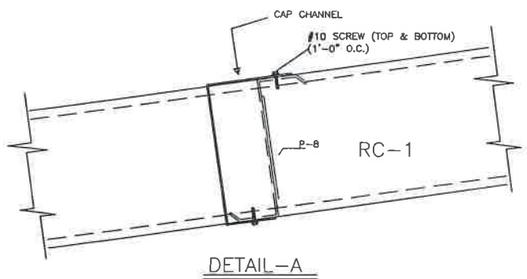


ROOF FRAMING PLAN

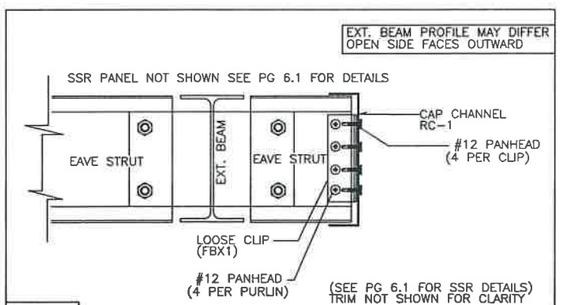
EXTENSION/CANOPY BOLTS ROOF PLAN				
MARK	QUAN	TYPE	DIA	LENGTH
EB-2	4	A325	5/8"	2"
EB-3	4	A325	5/8"	2"
EB-4	4	A325	5/8"	2"
EB-5	4	A325	5/8"	2"
EB-6	4	A325	5/8"	2"

MEMBER TABLE ROOF PLAN		
MARK	PART	LENGTH
EB-1	10X2CH16	5'-7 7/8"
EB-2	W10X12	3'-5 7/8"
EB-3	W10X12	3'-6 3/4"
EB-6	W10X12	3'-5 7/8"
P-1	10x25Z16	32'-5 1/2"
P-2	10x25Z16	28'-11 1/2"
P-3	10x25Z16	26'-5 1/2"
P-4	10x25Z16	27'-11 1/2"
P-8	10x25Z16	32'-5 1/2"
E-1	10LE14@3	23'-9 1/2"
E-2	10LE14@3	24'-3 1/2"
E-3	10LE14@3	25'-3 1/2"
E-5	10x35C14	27'-5 1/2"
E-6	10x35C14	24'-11 1/2"
E-7	10x35C14	24'-5 1/2"
E-8	10x35C14	25'-11 1/2"
E-12	10x35C14	27'-5 1/2"
E-13	10LE14@3	2'-7 1/2"
CB-1	1/4 CBL	27'-9"
CB-2	1/4 CBL	28'-5"

CONNECTION PLATES ROOF PLAN	
ID	MARK/PART
1	FBX1



DETAIL-A



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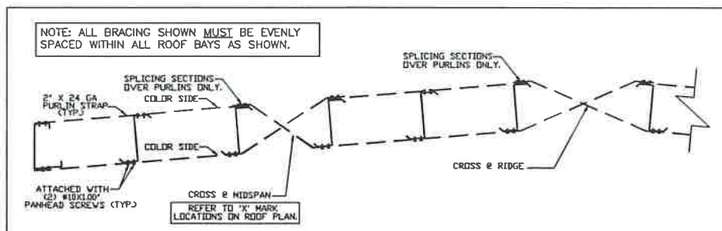


PURLIN BRACING DETAIL

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

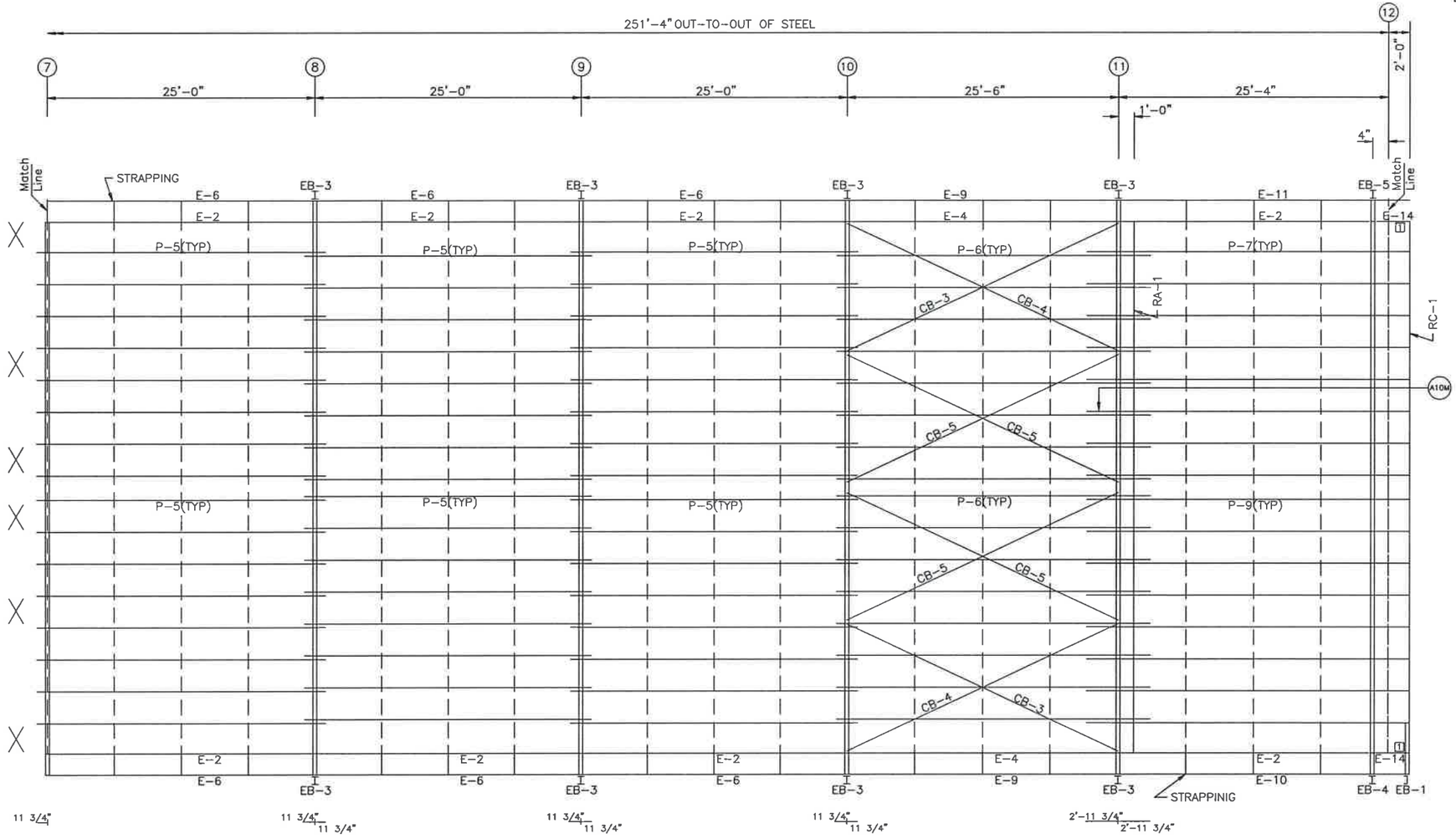
DRAWING NAME: ROOF FRAMING LAYOUT

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2A	DJH	GTL	NONE

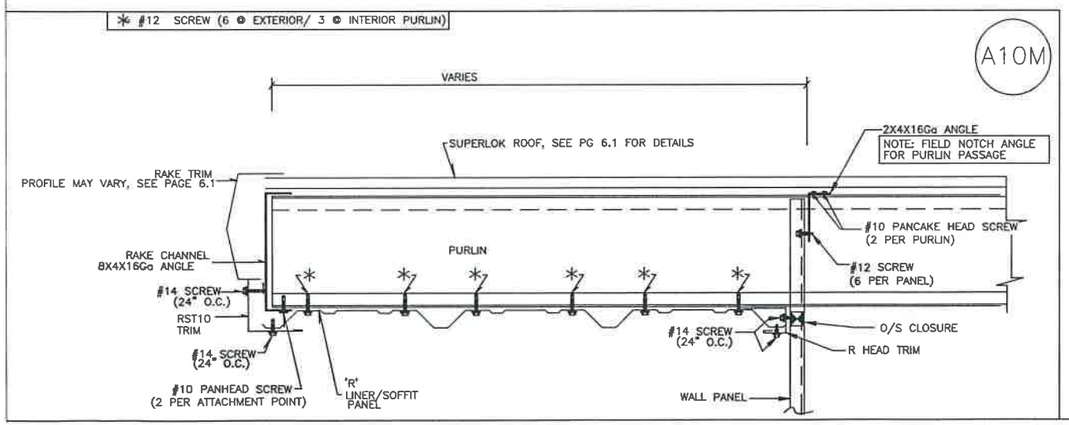
EXTENSION/CANOPY BOLTS ROOF PLAN				
MARK	QUAN	TYPE	DIA	LENGTH
EB-2	4	A325	5/8"	2"
EB-3	4	A325	5/8"	2"
EB-4	4	A325	5/8"	2"
EB-5	4	A325	5/8"	2"
EB-6	4	A325	5/8"	2"

MEMBER TABLE ROOF PLAN		
MARK	PART	LENGTH
EB-1	10X2CH16	5'-7 7/8"
EB-3	W10X12	3'-6 3/4"
EB-4	W10X12	3'-6 3/4"
EB-5	W10X12	3'-6 3/4"
P-5	10x25Z16	26'-11 1/2"
P-6	10x25Z16	29'-5 1/2"
P-7	10x25Z14	30'-3 1/2"
P-9	10x25Z14	30'-3 1/2"
E-2	10LE14@3	24'-3 1/2"
E-4	10LE14@3	24'-9 1/2"
E-6	10x35C14	24'-11 1/2"
E-9	10x35C14	25'-5 1/2"
E-10	10x35C14	27'-3 1/2"
E-11	10x35C14	27'-3 1/2"
E-14	10LE14@3	1'-11 1/2"
CB-3	1/4 CBL	27'-11"
CB-4	1/4 CBL	28'-3"
CB-5	1/4 CBL	28'-10"

CONNECTION PLATES ROOF PLAN	
☐ID	MARK/PART
1	FBX1



ROOF FRAMING PLAN



BUILDING "A"			
ISSUE	DET	CHK	DATE

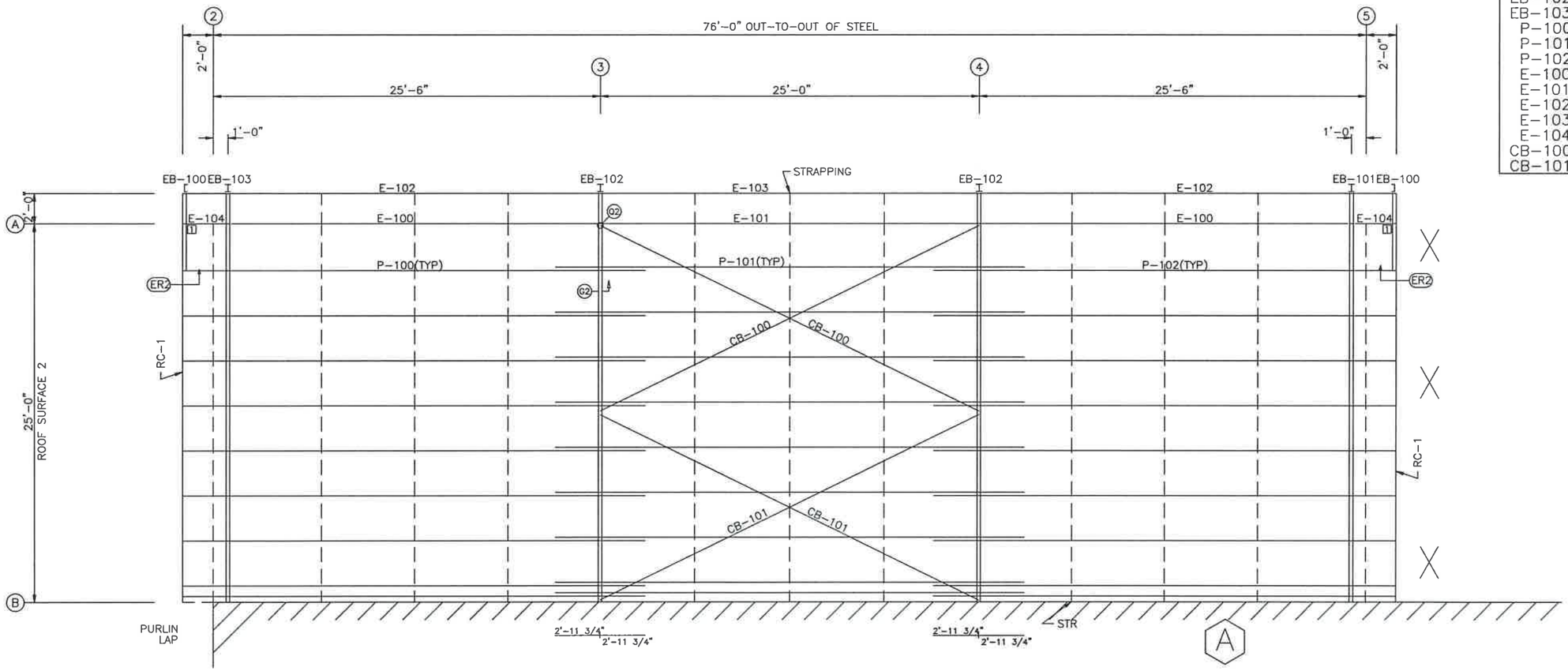
BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS			
JOB NO: 9249R1	DATE: 10/13/25		
LOCATION: HIGH SPRINGS, FL 32643			
DRAWING NAME: ROOF FRAMING LAYOUT			
DRAWING NO: PAGE 2A.2	DRAWN BY: DJH	CHECKED BY: GTL	SCALE: NONE

EXTENSION/CANOPY BOLTS ROOF PLAN				
MARK	QUAN	TYPE	DIA	LENGTH
EB-101	4	A325	5/8"	2"
EB-102	4	A325	5/8"	2"
EB-103	4	A325	5/8"	2"

MEMBER TABLE ROOF PLAN		
MARK	PART	LENGTH
EB-100	8X2CH16	5'-6 3/16"
EB-101	W8X10	3'-6 3/16"
EB-102	W8X10	3'-6 3/16"
EB-103	W8X10	3'-6 3/16"
P-100	8x25Z14	30'-5 1/2"
P-101	8x25Z16	30'-11 1/2"
P-102	8x25Z14	30'-5 1/2"
E-100	8LE12@1	23'-9 1/2"
E-101	8LE12@1	24'-3 1/2"
E-102	8x35C14	27'-5 1/2"
E-103	8x35C14	24'-11 1/2"
E-104	8LE12@1	2'-7 1/2"
CB-100	1/4 CBL	27'-8"
CB-101	1/4 CBL	28'-3"

CONNECTION PLATES ROOF PLAN	
☐ID	MARK/PART
1	FBX1



ROOF FRAMING PLAN

BUILDING "B"			
ISSUE	DET	CHK	DATE

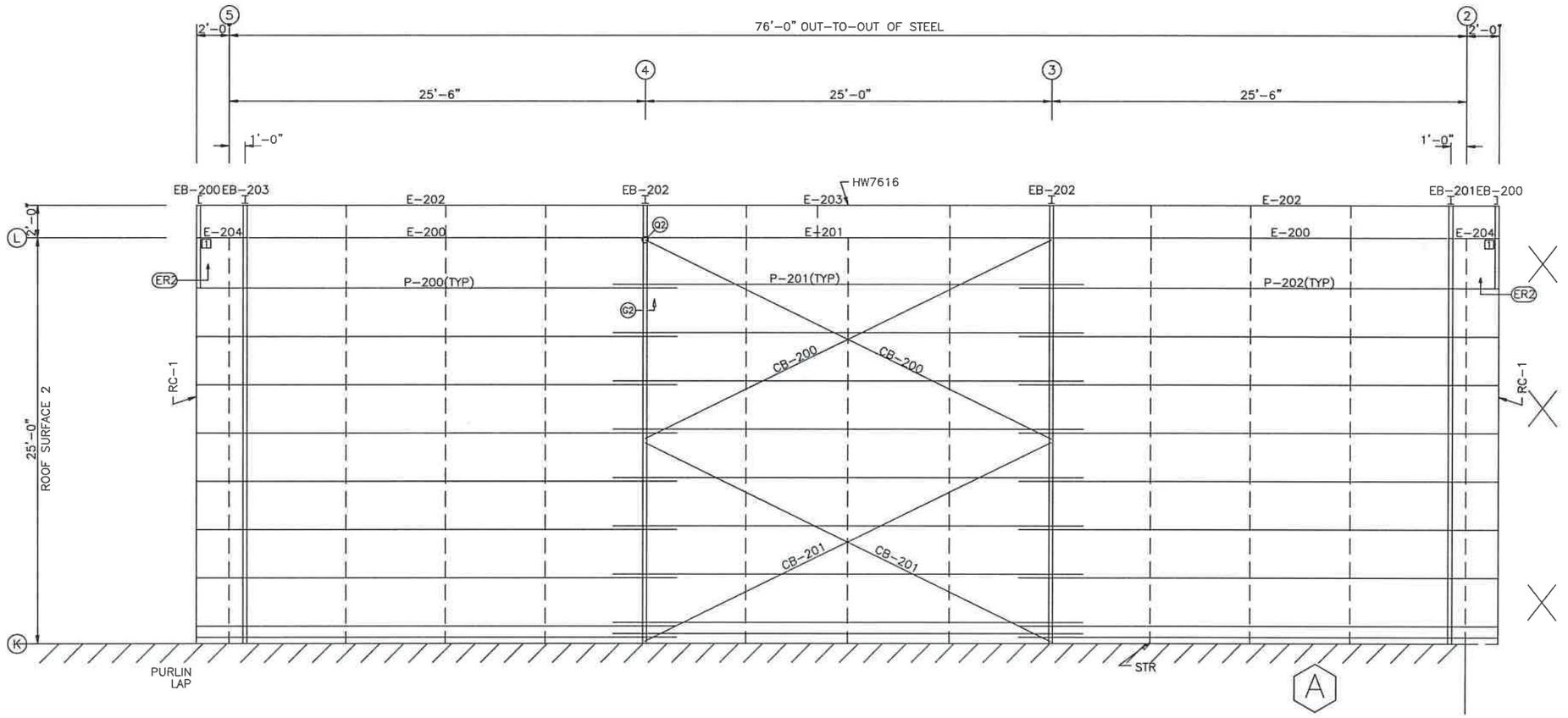
BUILDINGS AND MORE

CUSTOMER: PINNICAL SITE SOLUTIONS			
JOB NO: 9249R1		DATE: 10/13/25	
LOCATION: HIGH SPRINGS, FL 32643			
DRAWING NAME: ROOF FRAMING LAYOUT			
DRAWING NO: PAGE 2B	DRAWN BY: DJH	CHECKED BY: GTL	SCALE: NONE

EXTENSION/CANOPY BOLTS ROOF PLAN				
MARK	QUAN	TYPE	DIA	LENGTH
EB-201	4	A325	5/8"	2"
EB-202	4	A325	5/8"	2"
EB-203	4	A325	5/8"	2"

MEMBER TABLE ROOF PLAN		
MARK	PART	LENGTH
EB-200	8X2CH16	5'-6 3/16"
EB-201	W8X10	3'-6 3/16"
EB-202	W8X10	3'-6 3/16"
EB-203	W8X10	3'-6 3/16"
P-200	8x25Z16	29'-5 1/2"
P-201	8x25Z16	28'-11 1/2"
P-202	8x25Z16	29'-5 1/2"
E-200	8LE14@1	23'-9 1/2"
E-201	8LE14@1	24'-3 1/2"
E-202	8x35C14	27'-5 1/2"
E-203	8x35C14	24'-11 1/2"
E-204	8LE14@1	2'-7 1/2"
CB-200	1/4 CBL	27'-8"
CB-201	1/4 CBL	28'-3"

CONNECTION PLATES ROOF PLAN	
☐ID	MARK/PART
1	FBX1



1'-11 3/4" 1'-11 3/4"

1'-11 3/4" 1'-11 3/4"

ROOF FRAMING PLAN

BUILDING "C"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

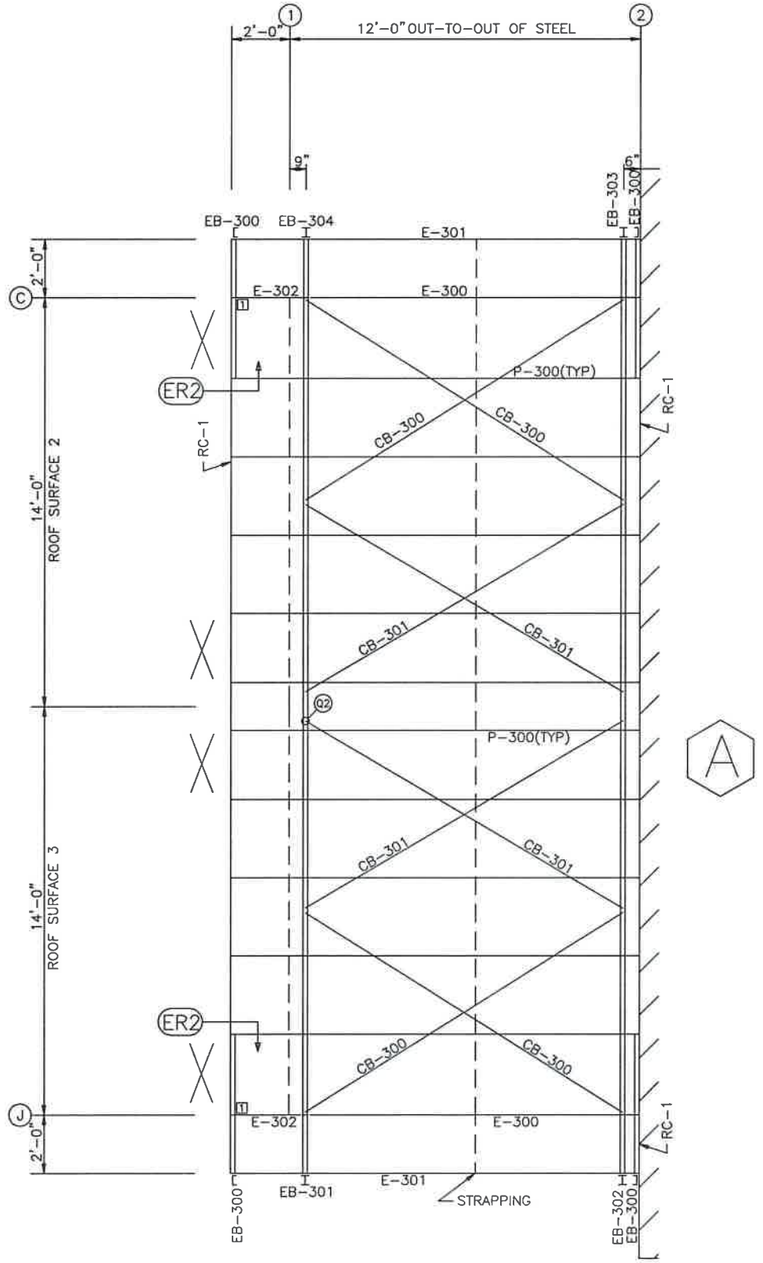
CUSTOMER: PINNICAL SITE SOLUTIONS

JOB NO: 9249R1 DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: ROOF FRAMING LAYOUT

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2C	DJH	GTL	NONE



ROOF FRAMING PLAN

EXTENSION/CANOPY BOLTS ROOF PLAN				
MARK	QUAN	TYPE	DIA	LENGTH
EB-301	4	A325	5/8"	2"
EB-302	4	A325	5/8"	2"
EB-303	4	A325	5/8"	2"
EB-304	4	A325	5/8"	2"

MEMBER TABLE ROOF PLAN		
MARK	PART	LENGTH
EB-300	8X2CH16	5'-8 13/16"
EB-301	W8X10	3'-7 11/16"
EB-302	W8X10	3'-7 11/16"
EB-303	W8X10	3'-7 11/16"
EB-304	W8X10	3'-7 11/16"
P-300	8x25Z16	13'-11 1/2"
E-300	8LE14@6	10'-0 1/2"
E-301	8x35C14	13'-11 1/2"
E-302	8LE14@6	2'-4 1/2"
CB-300	1/4 CBL	12'-6"
CB-301	1/4 CBL	13'-5"

CONNECTION PLATES ROOF PLAN	
☐ID	MARK/PART
1	FBX1

BUILDING "D"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNICAL SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: ROOF FRAMING LAYOUT

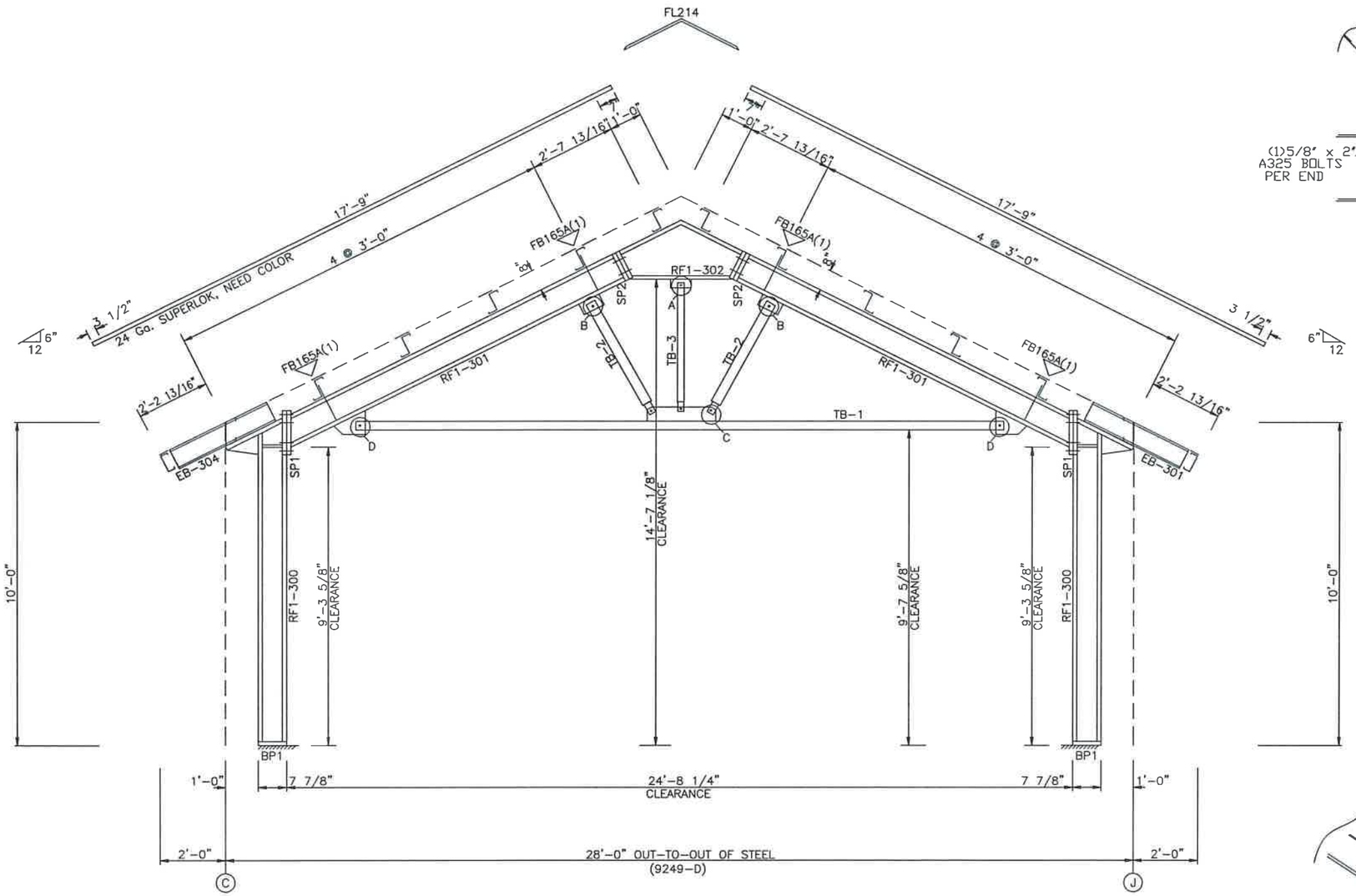
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2D	DJH	GTL	NONE

SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP1	4	4	0	A325	7/8"	2 1/2"
SP2	2	2	0	A325	7/8"	2 1/2"

BASE PLATE TABLE			
COL MARK	PLATE SIZE Width	THICK	Length
BP1	6"	3/8"	8"

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A – L2x2x14

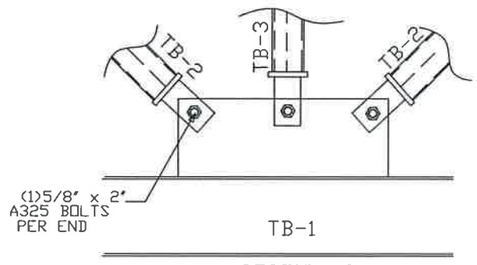
MEMBER TABLE						
MARK	Weight	Web Depth Start/End	Web PLATE THICK	Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-300	138	W8X10				
RF1-301	586	8.0/ 8.0	0.188	11'-9 1/2"	12 x 1/2" x 11'-5 1/4"	12 x 1/2" x 11'-9 1/2"
RF1-302	229	8.3/19.2	0.188	3'-10 15/16"	12 x 1/2" x 2'-2 1/4"	12 x 1/2" x 3'-3"
					12 x 1/2" x 2'-2 1/4"	
EB-301	38	W8X10				
EB-304	38	W8X10				
TB-1	TBD	TS4X4X.1875	TBD			
TB-2	TBD	TS4X4X.1875	TBD			
TB-3	TBD	TS4X4X.1875	TBD			



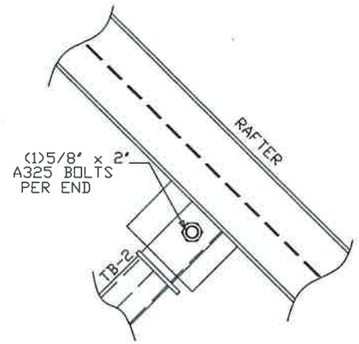
RIGID FRAME ELEVATION: FRAME LINE 1

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

6

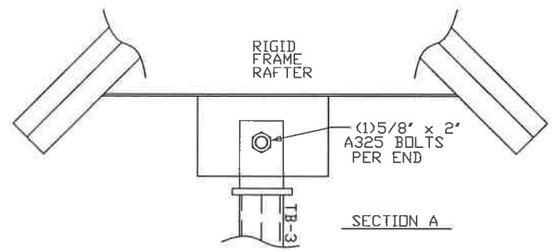


SECTION C



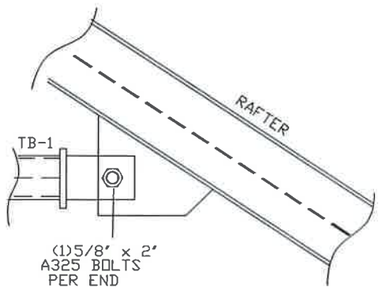
ANGLE BRACING @ RAFTER

SECTION B



SECTION A

ANGLE BRACING @ TOP OF RIGID FRAME



SECTION D

BUILDING "D"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

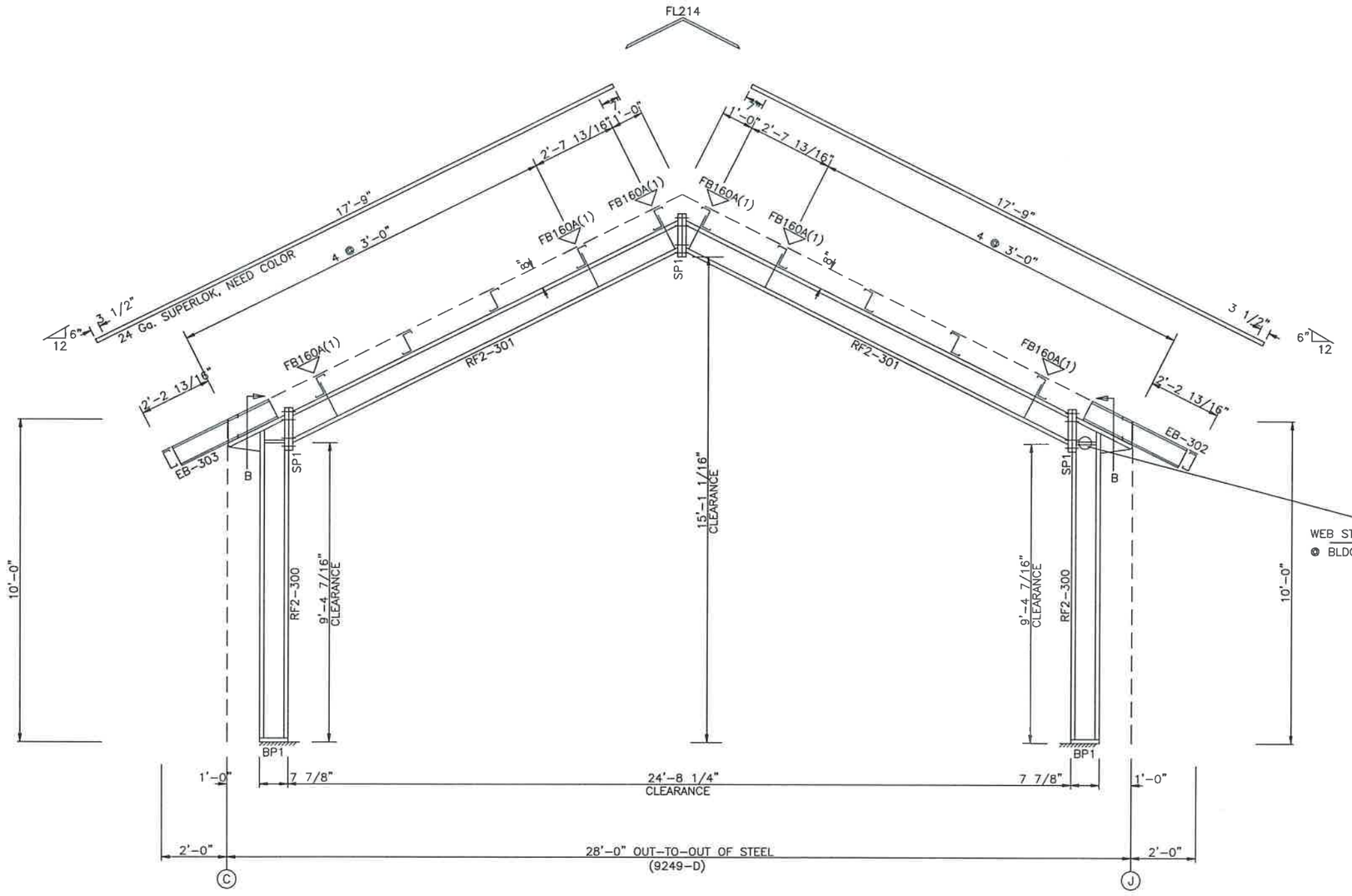
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.1	DJH	SPW	NONE

SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP1	4	4	0	A325	5/8"	2"

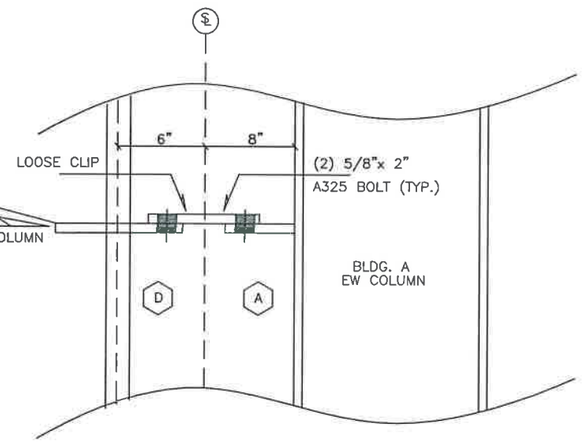
BASE PLATE TABLE			
COL MARK	PLATE SIZE Width	THICK	Length
BP1	6"	3/8"	8"

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A – L2x2x14

MEMBER SIZE TABLE			
MARK	MEMBER	LENGTH	WEIGHT
RF2–300	W8X10	9'–9 1/16"	124
RF2–301	W8X10	13'–9 3/8"	172
EB–302	W8X10	3'–7 11/16"	37
EB–303	W8X10	3'–7 11/16"	37



RIGID FRAME ELEVATION: FRAME LINE 2



DETAIL–B (TYP.)

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.2	WTH	SPW	NONE

SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP1	4	4	0	A325	5/8"	2"
SP2	4	0	0	A325	5/8"	2"
SP3	4	0	0	A325	5/8"	2"

CAP PLATE BOLTS			
MARK	Qty	TYPE DIA	Length
EC-1	4	A325T 5/8"	2"
EC-2	4	A325T 5/8"	2"

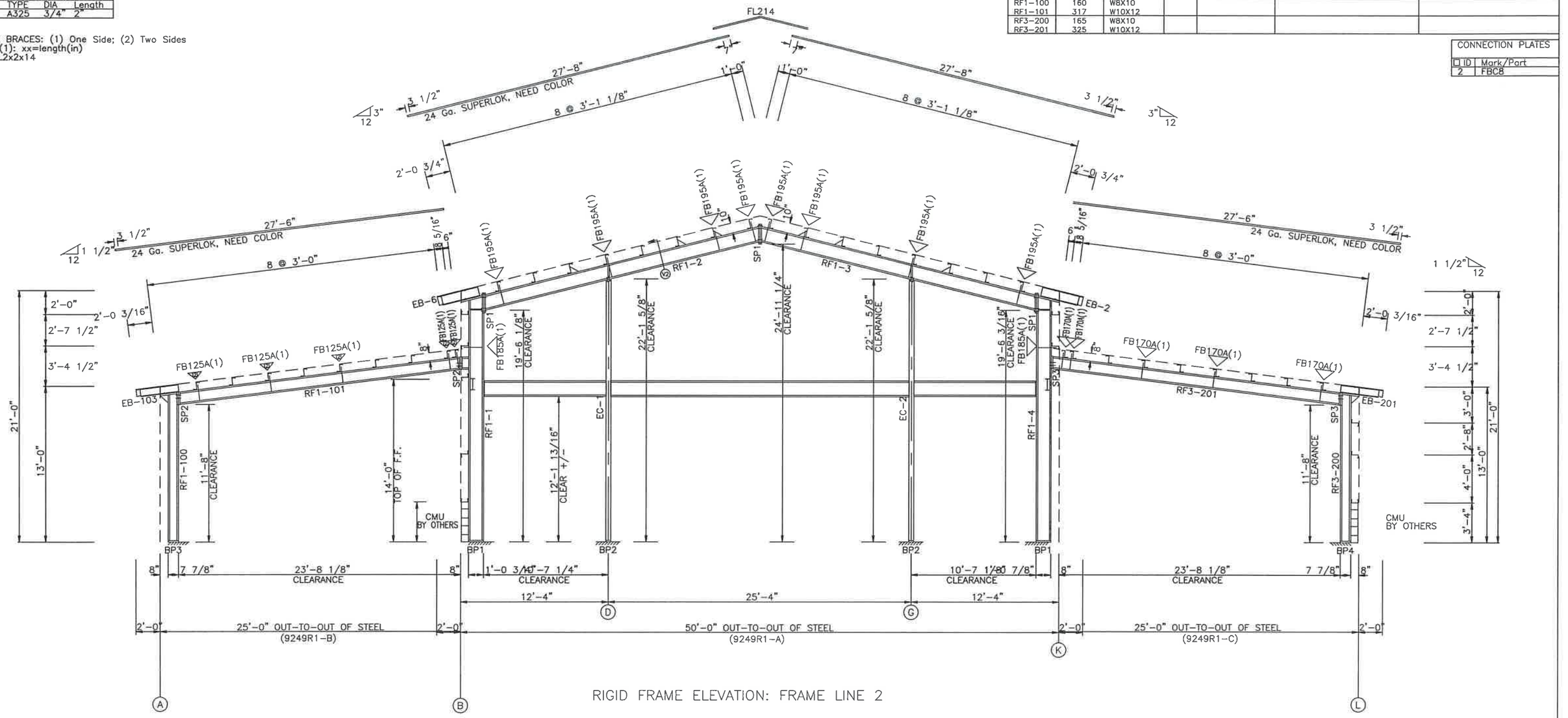
BASE PLATE TABLE			
COL MARK	PLATE SIZE Width	THICK	Length
BP1	6"	3/8"	1'-1"
BP2	8"	1/2"	8 1/4"
BP3	6"	3/8"	8"
BP4	6"	3/8"	8"

SUPPORT BEAM BOLT TABLE				
☐ ID	Qty	TYPE	DIA	Length
S1	4	A325	3/4"	2"

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A – L2x2x14

MEMBER TABLE						
MARK	Weight	Web Depth Start/End	Web PLATE THICK	Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1	481	12.0/12.0	0.188	13'-11 5/8"	5 x 1/4" x 18'-3 1/16"	5 x 1/2" x 14'-5 5/8"
		12.0/12.0	0.135	6'-6 1/2"	5 x 1/4" x 2'-0"	5 x 3/8" x 4'-7 7/16"
					5 x 1/4" x 1'-8 13/16"	
RF1-2	398	12.0/12.0	0.135	14'-11"	5 x 1/4" x 20'-0"	5 x 1/4" x 10'-7 7/16"
		12.0/12.0	0.135	9'-2 15/16"	5 x 1/4" x 3'-10 7/8"	5 x 1/4" x 11'-11 7/8"
RF1-3	398	12.0/12.0	0.135	9'-2 13/16"	5 x 1/4" x 3'-10 3/4"	5 x 1/4" x 11'-11 7/8"
		12.0/12.0	0.135	14'-11"	5 x 1/4" x 20'-0"	5 x 1/4" x 10'-7 5/16"
RF1-4	465	12.0/12.0	0.135	6'-6 1/2"	5 x 1/4" x 1'-8 15/16"	5 x 3/8" x 4'-7 7/16"
		12.0/12.0	0.188	13'-11 5/8"	5 x 1/4" x 4'-9"	5 x 1/2" x 14'-5 5/8"
					5 x 1/4" x 14'-8 1/16"	
EC-1	431	W8X18				
EC-2	431	W8X18				
EB-2	43	W10X12				
EB-6	43	W10X12				
RF1-100	160	W8X10				
RF1-101	317	W10X12				
RF3-200	165	W8X10				
RF3-201	325	W10X12				

CONNECTION PLATES	
☐ ID	Mark/Part
2	FBC8



RIGID FRAME ELEVATION: FRAME LINE 2

SEE PAGE 12 FOR MEZZANINE INFORMATION

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.3	DJH	SPW	NONE

SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP1	4	4	0	A325	5/8"	2"
SP3	4	0	0	A325	5/8"	2"

BASE PLATE TABLE			
COL MARK	PLATE SIZE Width	THICK	Length
BP1	6"	3/8"	1'-1"
BP2	6"	3/8"	8"

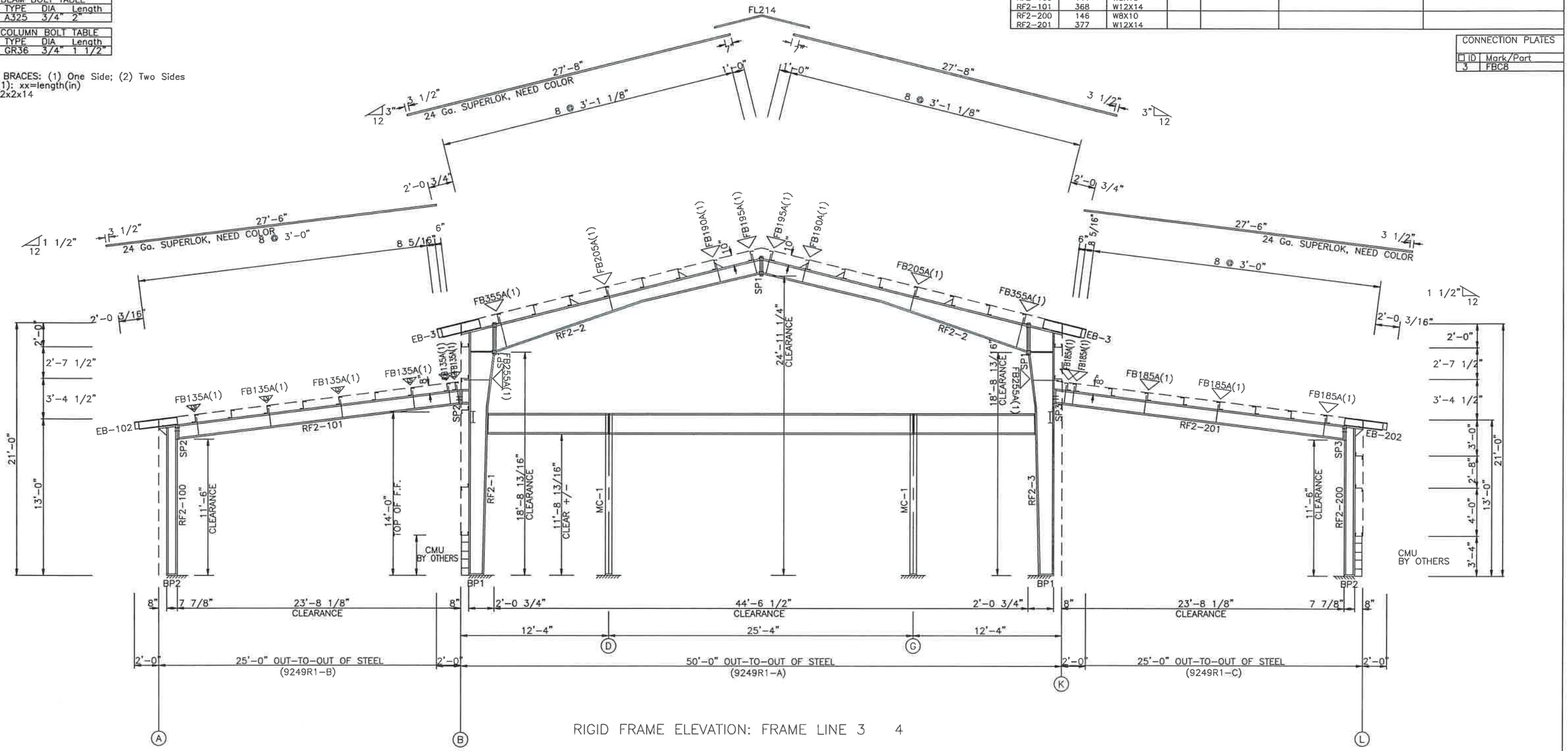
SUPPORT BEAM BOLT TABLE				
☐ ID	Qty	TYPE	DIA	Length
S1	5	A325	3/4"	2"

SUPPORT COLUMN BOLT TABLE				
☐ ID	Qty	TYPE	DIA	Length
C1	4	GR36	3/4"	1 1/2"

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A – L2x2x14

MEMBER TABLE						
MARK	Weight	Web Depth Start/End	Web PLATE THICK	Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF2-1	620	12.0/16.0	0.188	13'-11 5/8"	6 x 3/8" x 14'-5 5/8"	6 x 1/2" x 13'-11 5/8"
		16.0/24.0	0.135	4'-4 1/4"	6 x 5/16" x 5'-9 7/16"	6 x 3/8" x 4'-4 13/16"
		24.0/24.0	0.188	2'-5 1/4"	6 x 1/4" x 2'-9 5/16"	
RF2-2	414	24.0/10.5	0.135	12'-10 3/16"	5 x 1/4" x 20'-0"	5 x 1/4" x 12'-10 3/4"
		10.5/12.0	0.135	10'-6 5/16"	5 x 1/4" x 2'-10 3/8"	5 x 1/4" x 10'-3 1/4"
RF2-3	599	24.0/24.0	0.188	2'-5 1/4"	6 x 1/4" x 2'-9 5/16"	6 x 3/8" x 4'-4 13/16"
		24.0/16.0	0.135	4'-4 1/4"	6 x 5/16" x 4'-9"	6 x 1/2" x 13'-11 5/8"
		16.0/12.0	0.188	13'-11 5/8"	6 x 5/16" x 7/16"	
					6 x 3/8" x 14'-5 5/8"	
EB-3	46	W10X12				
RF2-100	141	W8X10				
RF2-101	368	W12X14				
RF2-200	146	W8X10				
RF2-201	377	W12X14				

CONNECTION PLATES	
☐ ID	Mark/Part
3	FBC8



RIGID FRAME ELEVATION: FRAME LINE 3 4

SEE PAGE 12 FOR MEZZANINE INFORMATION

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.4	DJH	SPW	NONE

SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP1	4	4	0	A325	5/8"	2"
SP2	4	0	0	A325	5/8"	2"
SP3	4	0	0	A325	5/8"	2"

CAP PLATE BOLTS				
MARK	Qty	TYPE	DIA	Length
RF3-5	4	A325T	5/8"	2"
RF3-6	4	A325T	5/8"	2"

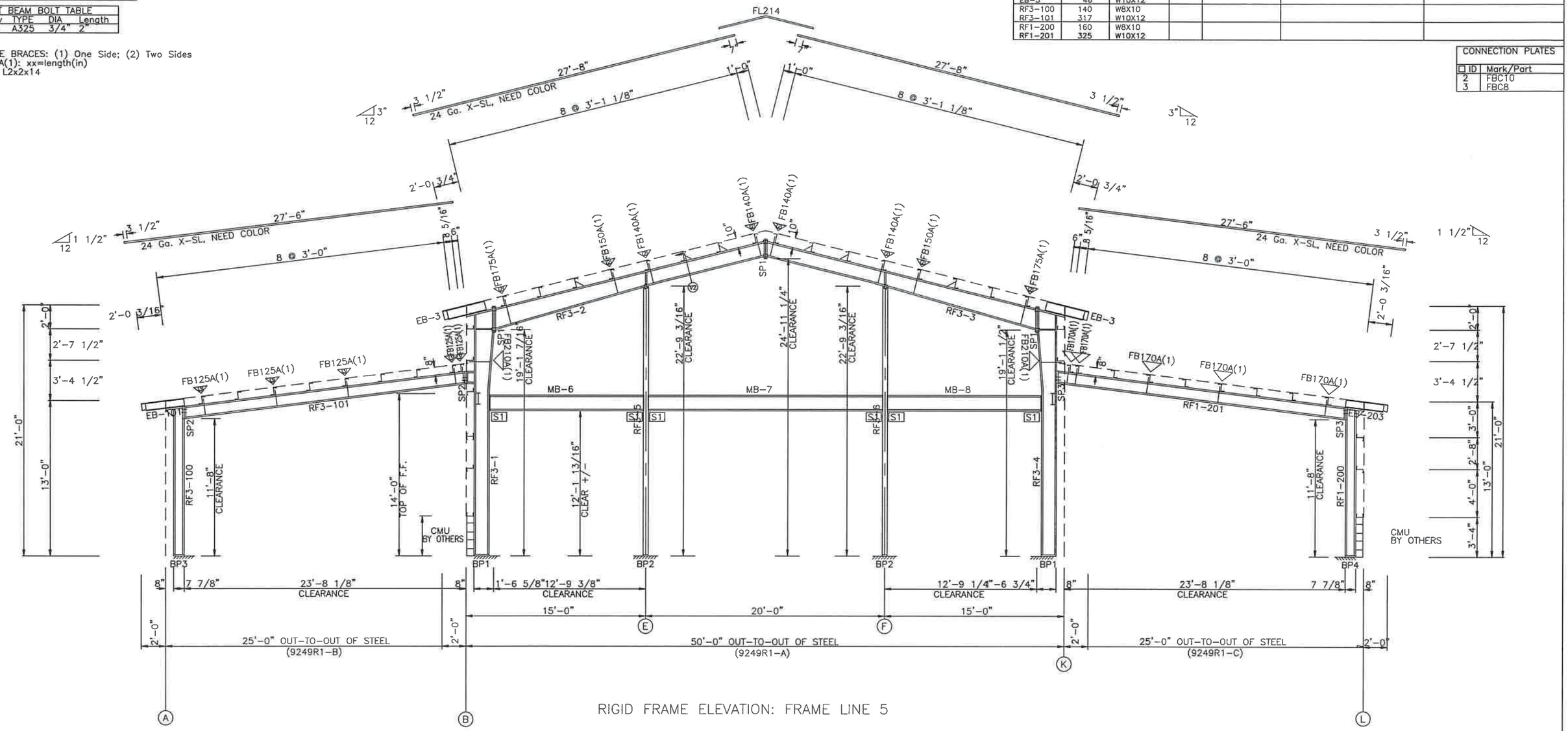
BASE PLATE TABLE			
COL MARK	PLATE SIZE Width	THICK	Length
BP1	6"	3/8"	1'-1"
BP2	8"	3/8"	8 1/2"
BP3	6"	3/8"	8"
BP4	6"	3/8"	8"

SUPPORT BEAM BOLT TABLE				
☐ ID	Qty	TYPE	DIA	Length
S1	4	A325	3/4"	2"

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A – L2x2x14

MEMBER TABLE						
MARK	Weight	Web Depth Start/End	Web PLATE THICK	Web PLATE Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF3-1	553	12.0/12.0 12.0/18.0	0.188 0.188	13'-11 5/8" 6'-8"	6 x 1/4" x 18'-3 1/16" 6 x 1/4" x 2'-0" 6 x 1/4" x 2'-3"	6 x 1/2" x 13'-11 5/8" 6 x 3/8" x 4'-9 1/8"
RF3-2	416	18.0/12.0 12.0/12.0	0.135 0.135	14'-3 5/16" 9'-5 15/16"	5 x 1/4" x 20'-0" 5 x 1/4" x 3'-4 11/16"	5 x 1/4" x 12'-11 3/4" 5 x 1/4" x 9'-2 7/8"
RF3-3	416	12.0/12.0 12.0/18.0	0.135 0.135	9'-5 15/16" 14'-3 3/16"	5 x 1/4" x 3'-4 9/16" 5 x 1/4" x 20'-0"	5 x 1/4" x 9'-2 7/8" 5 x 1/4" x 12'-11 5/8"
RF3-4	537	18.0/12.0 12.0/12.0	0.188 0.188	6'-8" 13'-11 5/8"	6 x 1/4" x 2'-3 1/8" 6 x 1/4" x 4'-9" 6 x 1/4" x 14'-8 1/16"	6 x 3/8" x 4'-9 3/16" 6 x 1/2" x 13'-11 5/8"
RF3-5	448	W8X18				
RF3-6	448	W8X18				
EB-3	46	W10X12				
RF3-100	140	W8X10				
RF3-101	317	W10X12				
RF1-200	160	W8X10				
RF1-201	325	W10X12				

CONNECTION PLATES	
☐ ID	Mark/Part
2	FBC10
3	FBC8



RIGID FRAME ELEVATION: FRAME LINE 5

SEE PAGE 12 FOR MEZZANINE INFORMATION

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.5	DJH	SPW	NONE

SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP1	4	4	0	A325	5/8"	2"

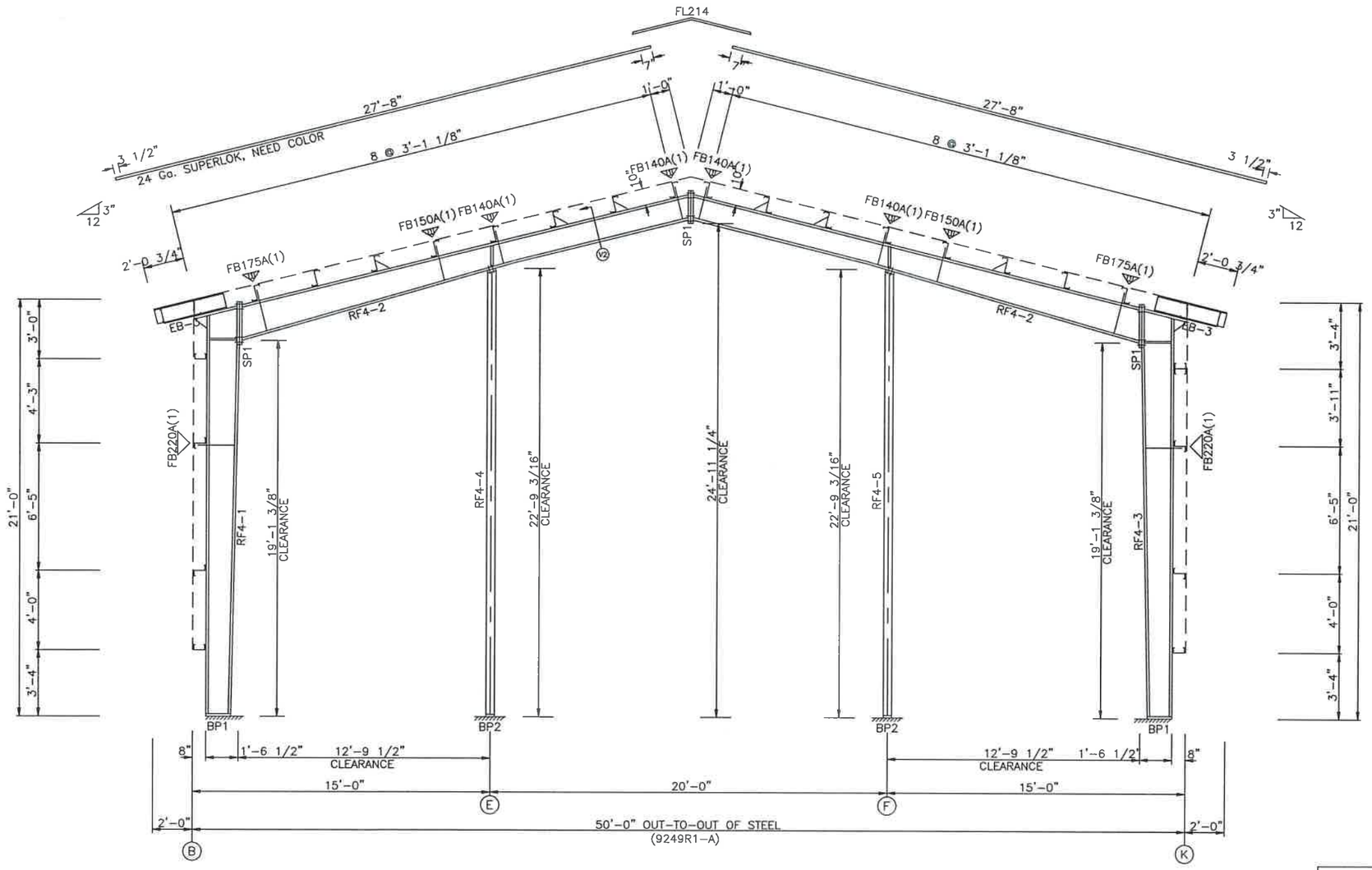
CAP PLATE BOLTS				
MARK	Qty	TYPE	DIA	Length
RF4-4	4	A325T	5/8"	2"
RF4-5	4	A325T	5/8"	2"

BASE PLATE TABLE			
COL MARK	PLATE SIZE Width	THICK	Length
BP1	6"	3/8"	1'-0 1/2"
BP2	8"	3/8"	8"

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A – L2x2x14

MEMBER TABLE						
MARK	Weight	Web Depth Start/End	Web PLATE THICK	Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF4-1	359	12.0/13.8 13.8/18.0	0.135 0.135	5'-8 5/8" 14'-11"	5 x 1/4" x 18'-3 1/16" 5 x 1/4" x 2'-0" 5 x 1/4" x 2'-3"	5 x 1/4" x 18'-8 1/2"
RF4-2	415	18.0/12.0 12.0/12.0	0.135 0.135	14'-3 9/16" 9'-5 15/16"	5 x 1/4" x 20'-0" 5 x 1/4" x 3'-4 15/16"	5 x 1/4" x 12'-11" 5 x 1/4" x 9'-2 7/8"
RF4-3	358	18.0/13.8 13.8/12.0	0.135 0.135	14'-11" 5'-8 5/8"	5 x 1/4" x 2'-3" 5 x 1/4" x 2'-0" 5 x 1/4" x 18'-3 1/16"	5 x 1/4" x 18'-8 1/2"
RF4-4	573	W8X24				
RF4-5	573	W8X24				
EB-3	46	W10X12				

CONNECTION PLATES		
☐ ID	Mark/Part	DETAIL
1	FBC10	G25



RIGID FRAME ELEVATION: FRAME LINE 6

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.6	DJH	SPW	NONE

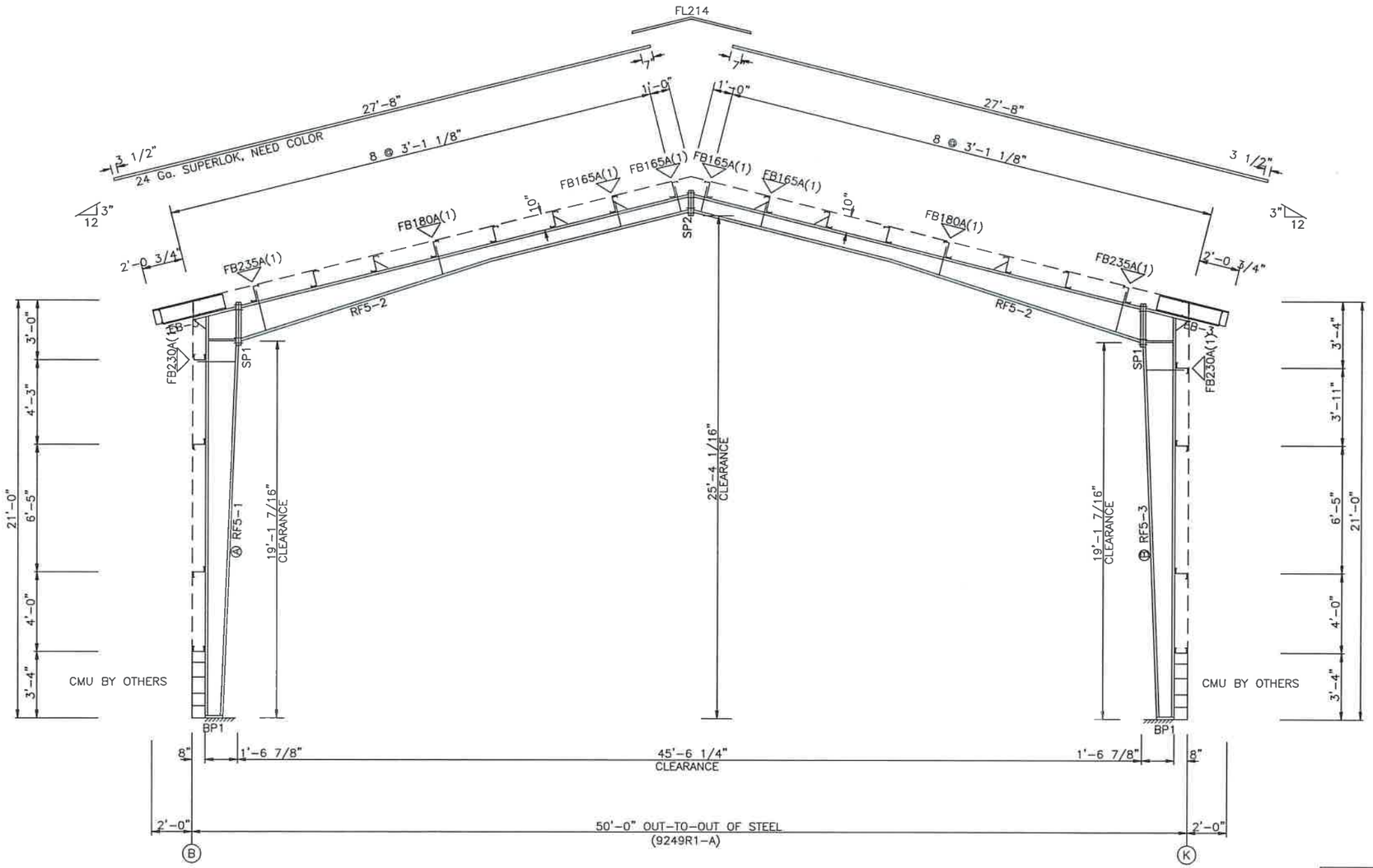
SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP1	4	4	0	A325	5/8"	2 1/2"
SP2	4	4	0	A325	5/8"	2"

BASE PLATE TABLE				
COL MARK	PLATE SIZE Width	THICK	Length	
BP1	6"	1/2"	8 1/2"	

ALTERNATE MEMBER		
Frame Line	OID	MARK
10	A	RF5-4
	B	RF5-5

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A – L2x2x14

MEMBER TABLE						
MARK	Weight	Web Depth Start/End	Web PLATE THICK	Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF5-1	578	7.5/10.7	0.188	5'-8 1/2"	6 x 1/4" x 18'-2 15/16"	6 x 5/8" x 18'-8 9/16"
		10.7/18.0	0.188	14'-11"	6 x 1/4" x 2'-0"	
					6 x 1/4" x 2'-3"	
RF5-2	389	18.0/ 7.5	0.135	13'-2 11/16"	5 x 1/4" x 20'-0"	5 x 5/16" x 13'-3 1/16"
		7.5/ 7.5	0.135	10'-6 5/16"	5 x 1/4" x 3'-4 7/16"	5 x 1/4" x 10'-4 5/16"
RF5-3	576	18.0/10.7	0.188	14'-11"	6 x 1/4" x 2'-3"	6 x 5/8" x 18'-8 9/16"
		10.7/ 7.5	0.188	5'-8 1/2"	6 x 1/4" x 2'-0"	
					6 x 1/4" x 18'-2 15/16"	
EB-3	46	W10X12				



RIGID FRAME ELEVATION: FRAME LINE 7 8 9 10

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.7	DJH	SPW	NONE

SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP1	4	4	0	A325	5/8"	2"

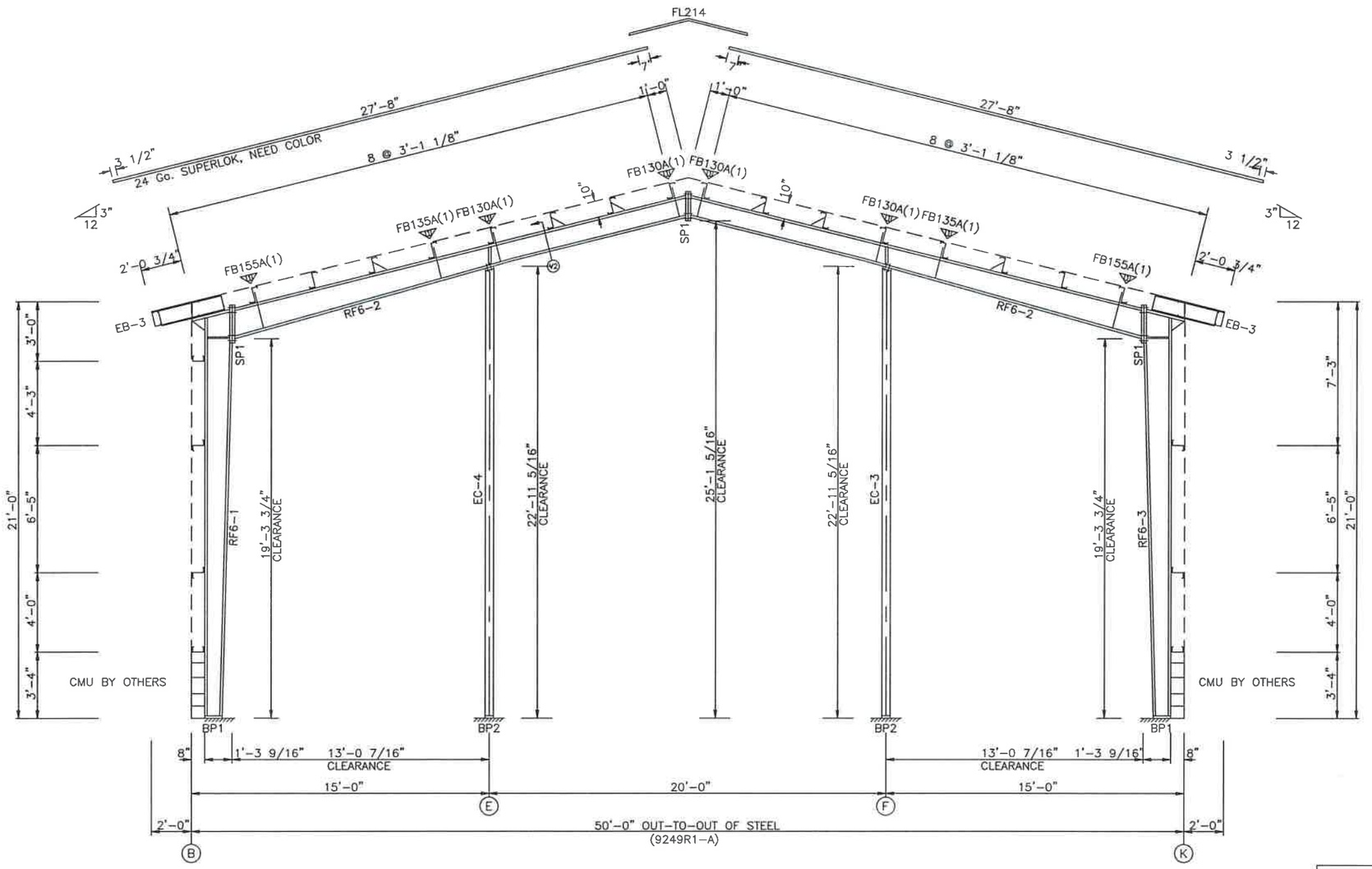
CAP PLATE BOLTS				
MARK	Qty	TYPE	DIA	Length
EC-4	4	A325T	5/8"	2"
EC-3	4	A325T	5/8"	2"

BASE PLATE TABLE			
COL MARK	PLATE SIZE Width	THICK	Length
BP1	6"	3/8"	8 1/2"
BP2	6"	3/8"	8"

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A – L2x2x14

MEMBER TABLE						
MARK	Weight	Web Depth Start/End	Web PLATE THICK	Web PLATE Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF6-1	393	7.5/ 9.7 9.7/15.0	0.135 0.135	5'-7 7/8" 14'-11"	6 x 1/4" x 18'-3 1/16" 6 x 1/4" x 2'-0" 6 x 1/4" x 1'-11 7/8"	6 x 5/16" x 18'-10 13/16"
RF6-2	385	15.0/10.0 10.0/10.0	0.135 0.135	14'-6 7/16" 9'-5 7/16"	5 x 1/4" x 20'-0" 5 x 1/4" x 3'-8 1/16"	5 x 1/4" x 13'-2 13/16" 5 x 1/4" x 9'-2 7/8"
RF6-3	391	15.0/ 9.7 9.7/ 7.5	0.135 0.135	14'-11" 5'-7 7/8"	6 x 1/4" x 1'-11 7/8" 6 x 1/4" x 2'-0" 6 x 1/4" x 18'-3 1/16"	6 x 5/16" x 18'-10 13/16"
EC-4	318	W8X13				
EC-3	318	W8X13				
EB-3	46	W10X12				

CONNECTION PLATES		
☐ ID	Mark/Part	DETAIL
1	FBC10	G25



RIGID FRAME ELEVATION: FRAME LINE 11

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO: PAGE 2.8 | DRAWN BY: DJH | CHECKED BY: SPW | SCALE: NONE

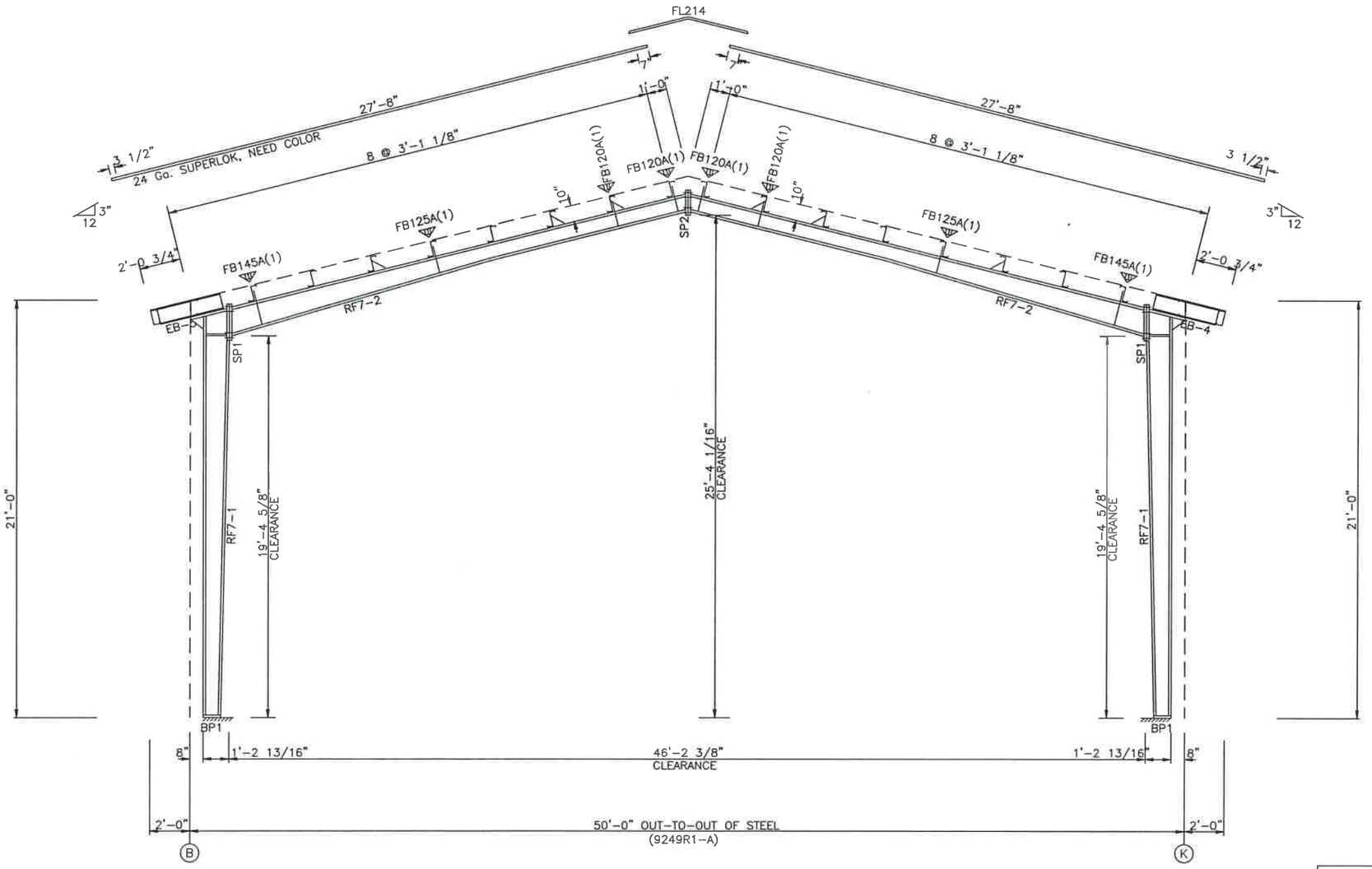
SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP1	4	4	0	A325	5/8"	2 1/2"
SP2	4	4	0	A325	5/8"	2"

BASE PLATE TABLE				
COL MARK	PLATE SIZE Width	THICK	Length	
BP1	6"	3/8"	8 1/2"	

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A – L2x2x14

MEMBER TABLE						
MARK	Weight	Web Depth Start/End	Web PLATE THICK	Web PLATE Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF7-1	513	7.5/ 9.4	0.188	5'-7 5/8"	6 x 5/16" x 18'-3 1/16"	6 x 1/2" x 18'-11 5/8"
		9.4/14.0	0.188	14'-11"	6 x 5/16" x 2'-0"	
					6 x 1/4" x 1'-10 15/16"	
RF7-2	363	14.0/ 7.5	0.135	13'-5 3/4"	5 x 1/4" x 20'-0"	5 x 1/4" x 13'-5 7/8"
		7.5/ 7.5	0.135	10'-6 5/16"	5 x 1/4" x 3'-8 1/2"	5 x 1/4" x 10'-4 5/16"
EB-4	44	W10X12				
EB-5	44	W10X12				

CONNECTION PLATES		
☐ ID	Mark/Part	DETAIL
1	FBC10	G25



RIGID FRAME ELEVATION: FRAME LINE 12

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

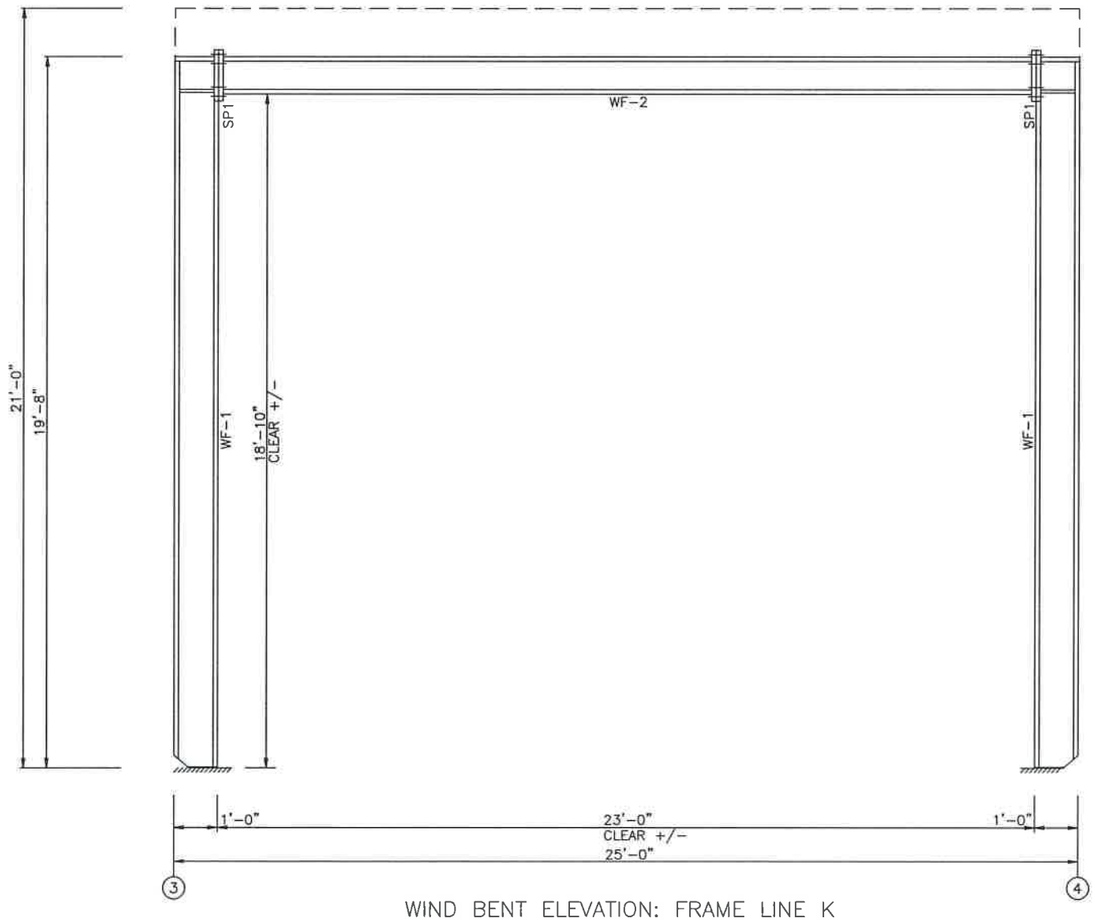
LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.9	DJH	SPW	NONE

SPLICE BOLTS					
Splice Mark	Quan Top	Quan Bot	Bolt Type	Bolt Dia	Bolt Length
SP1	4	4	A325	3/4"	2 1/2"

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF-2	B10651	22'-11"
WF-1	B12641	19'-8"



WIND BENT ELEVATION: FRAME LINE K

BLDG "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

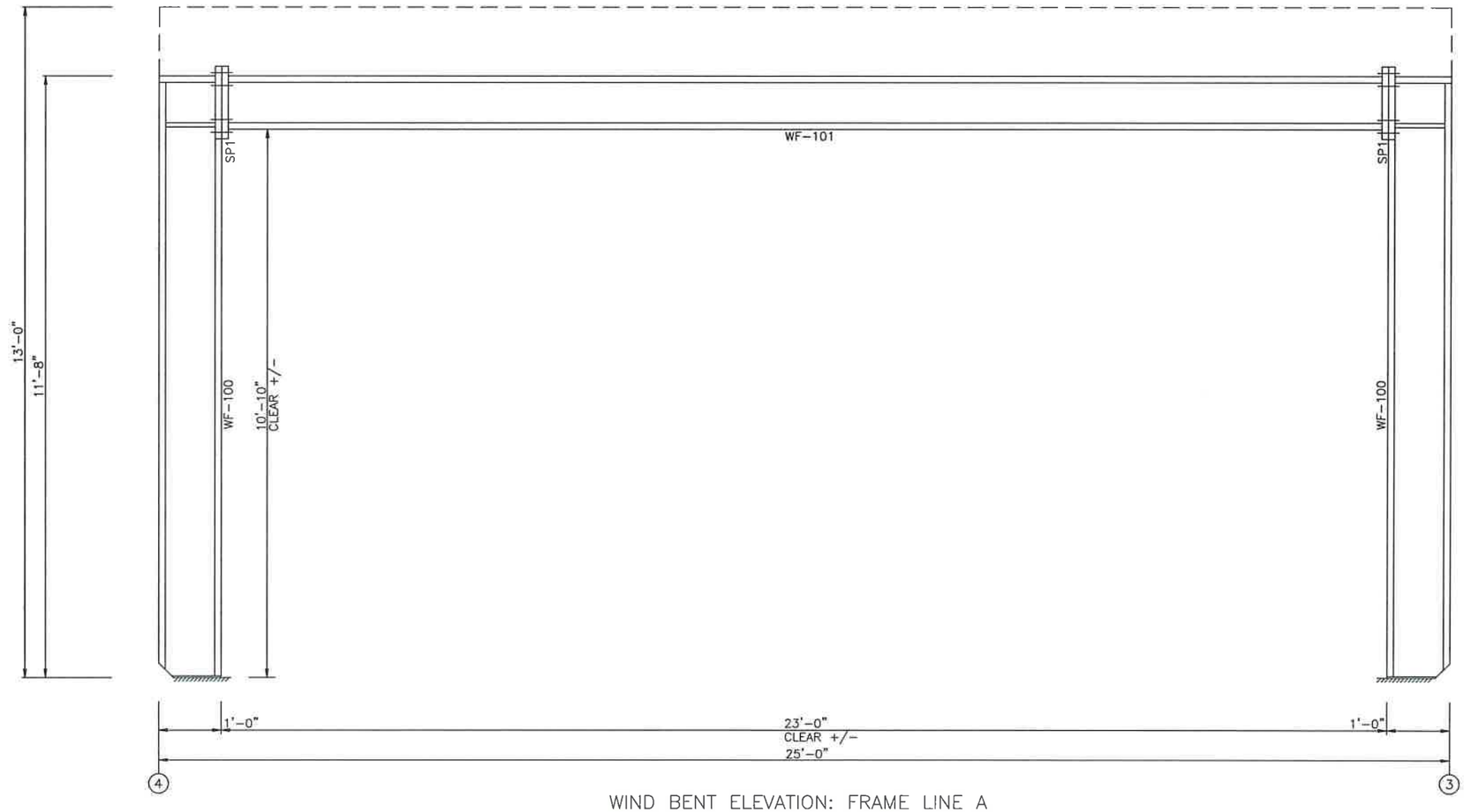
LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.10	DJH	SPW	NONE

SPLICE BOLTS					
Splice Mark	Quan Top	Quan Bot	Bolt Type	Bolt Dia	Bolt Length
SP1	4	4	A325	5/8"	2"

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF-101	B10651	22'-11 3/4"
WF-100	B12541	11'-8"



WIND BENT ELEVATION: FRAME LINE A

BLDG "B"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNICAL SITE SOLUTIONS

JOB NO: 9249R1 — DATE: 10/13/25

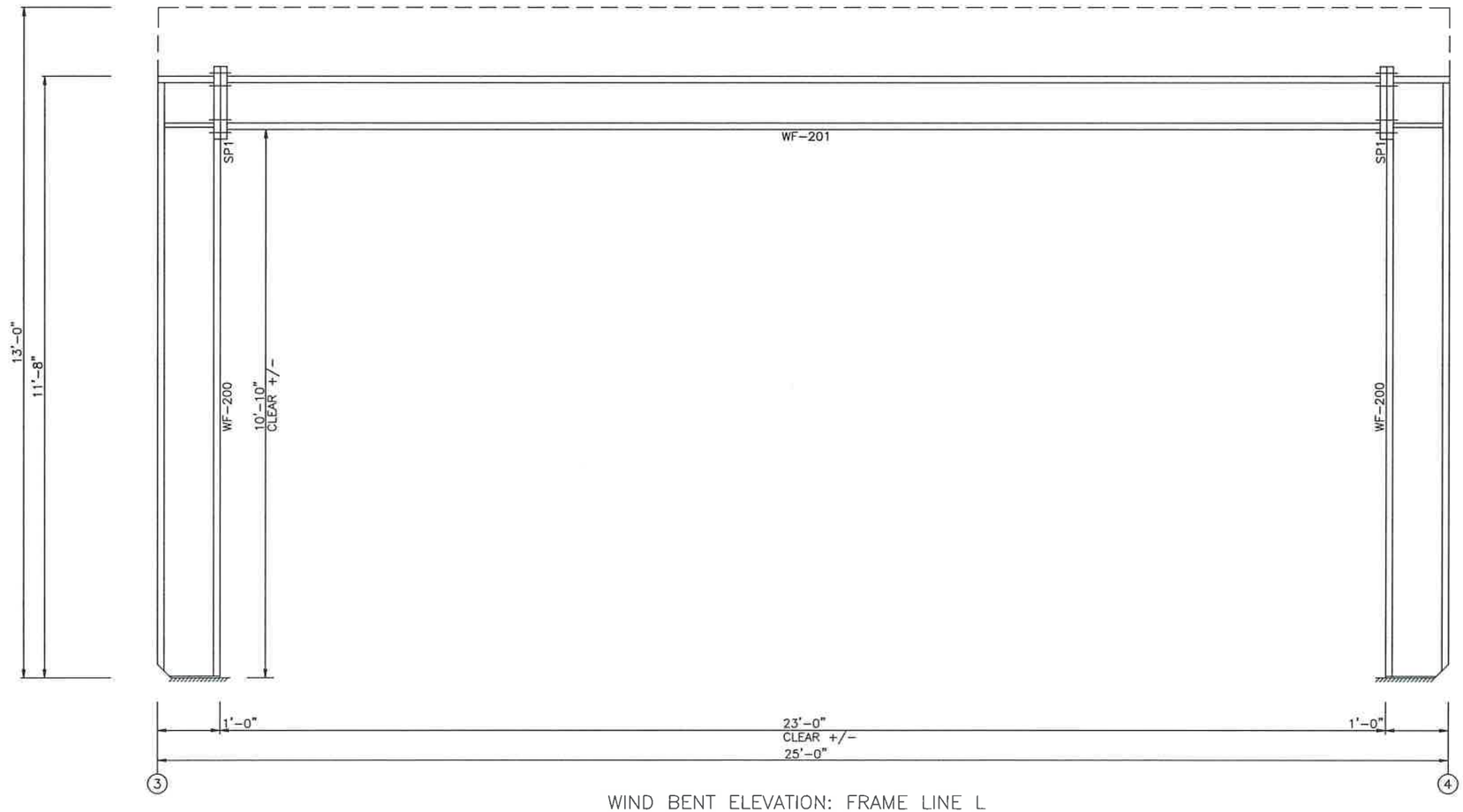
LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.11	DJH	SPW	NONE

SPLICE BOLTS					
Splice Mark	Quan Top/	Bot	Bolt Type	Dia	Length
SP1	4	4	A325	5/8"	2"

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF-201	B10651	22'-11 3/4"
WF-200	B12541	11'-8"



WIND BENT ELEVATION: FRAME LINE L

BLDG "C"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNICAL SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

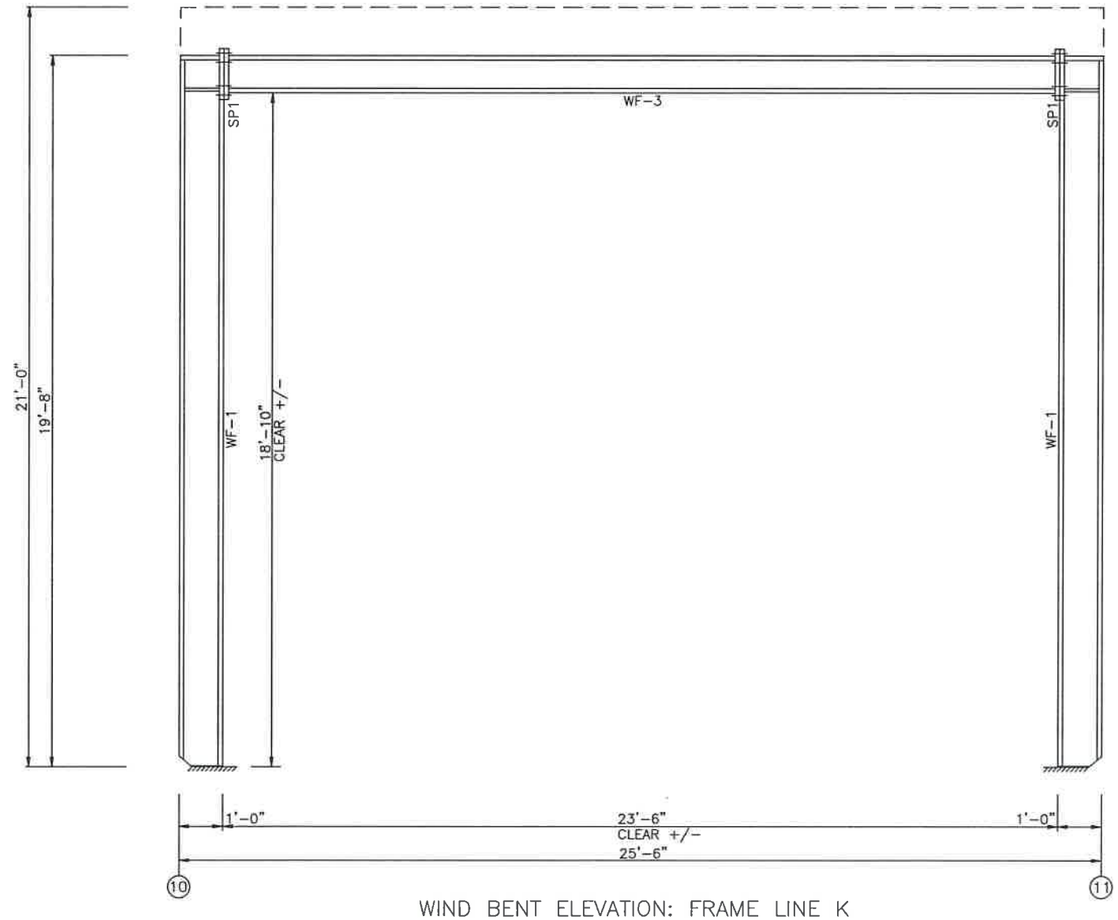
LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.12	DJH	SPW	NONE

SPLICE BOLTS						
Splice Mark	Quan Top/	Bot	Bolt Type	Dia	Length	
SP1	4	4	A325	3/4"	2	1/2"

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF-3	B10651	23'-5"
WF-1	B12641	19'-8"



WIND BENT ELEVATION: FRAME LINE K

BLDG "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

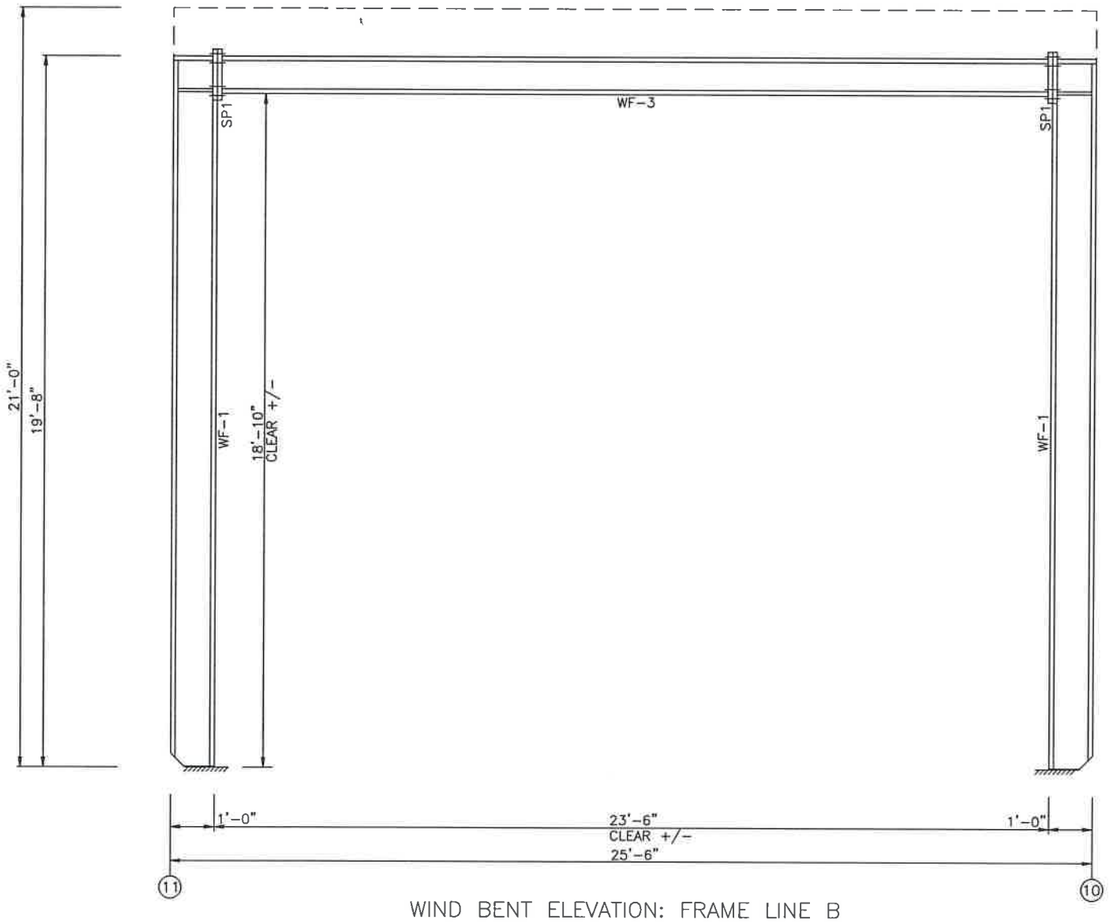
LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.13	DJH	SPW	NONE

SPLICE BOLTS					
Splice Mark	Quan Top/ Bot	Bolt Type	Dia	Length	
SP1	4 4	A325	3/4"	2	1/2"

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF-3	B10651	23'-5"
WF-1	B12641	19'-8"



WIND BENT ELEVATION: FRAME LINE B

BLDG "A

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

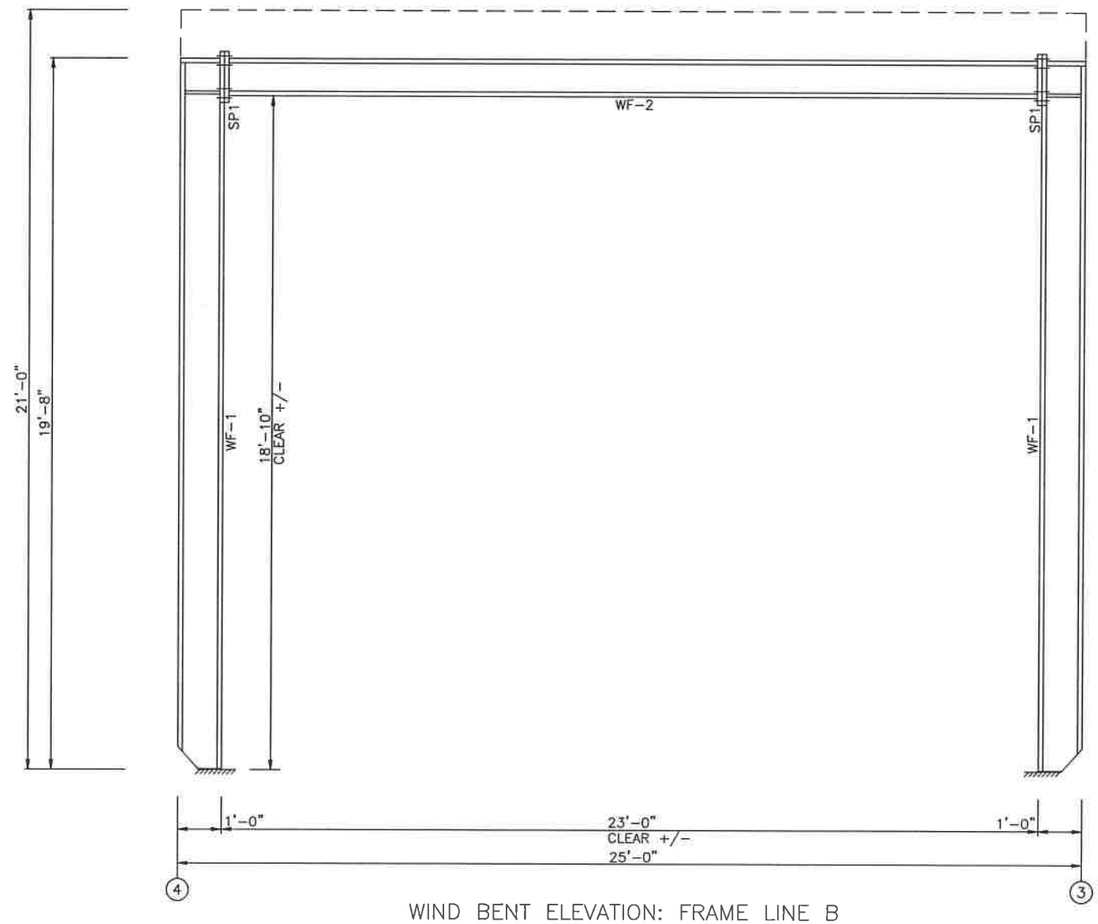
LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.14	DJH	SPW	NONE

SPLICE BOLTS					
Splice Mark	Quan Top/	Bot	Bolt Type	Dia	Length
SP1	4	4	A325	3/4"	2 1/2"

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF-2	B10651	22'-11"
WF-1	B12641	19'-8"



WIND BENT ELEVATION: FRAME LINE B

BLDG "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

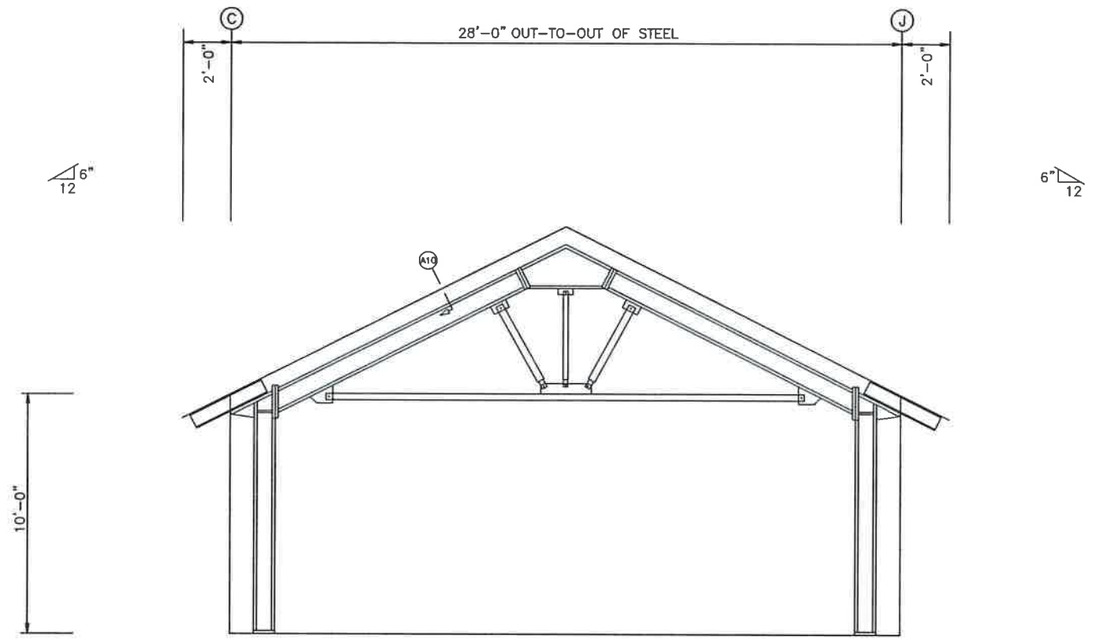
JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

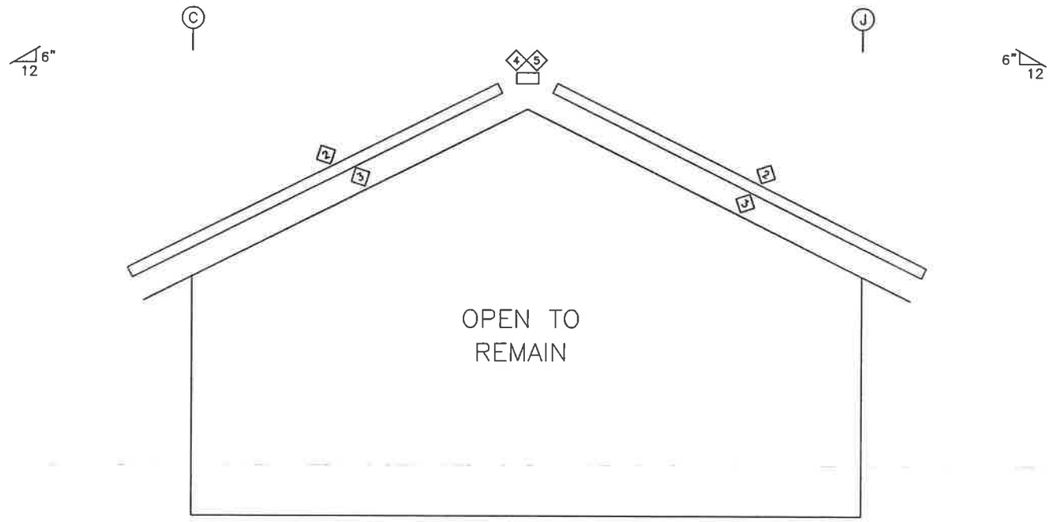
DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.15	DJH	SPW	NONE

TRIM TABLE FRAME LINE 1			
◇ID	PART	LENGTH	DETAIL
2	RAKE TRIM	15'-8"	RAKE2
3	RST8	15'-11"	RAKE2
4	PEAK BOX	1'-6"	
5	RID END CAP	1'-9 1/2"	



ENDWALL FRAMING: FRAME LINE 1



ENDWALL TRIM: FRAME LINE 1

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

6

BUILDING "D"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

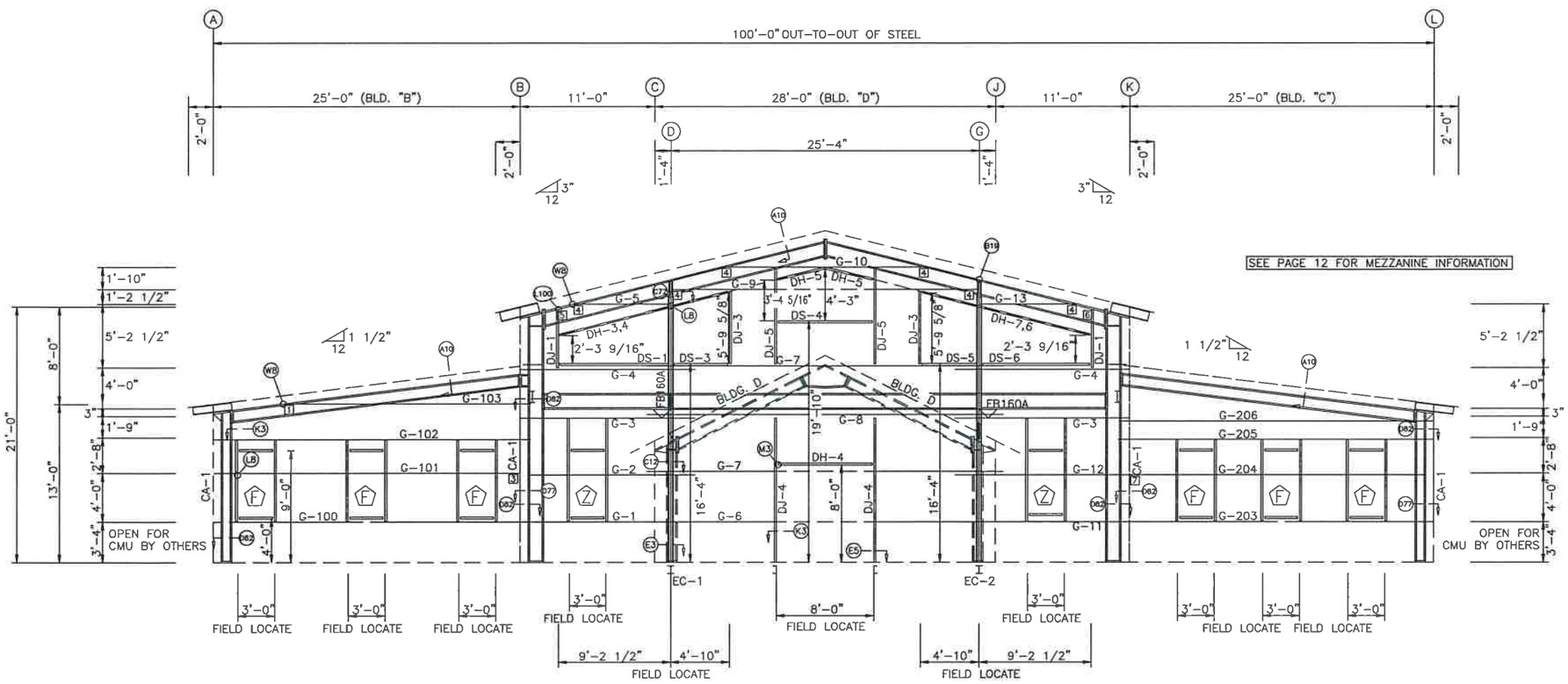
CUSTOMER: PINNACLE SITE SOLUTIONS

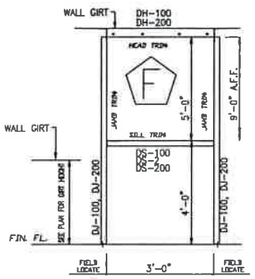
JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: ENDWALL FRAMING & SHEETING LAYOUT

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 3.0	DJH	SPW	NONE



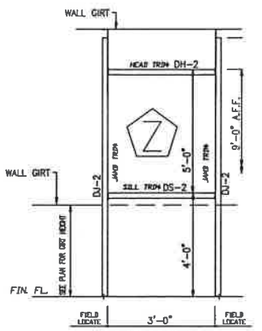


1. SEE FRAMING ELEVATIONS FOR GIRT QUANTITY AND ELEVATIONS
2. SEE DETAILS FOR BOLT AND FRAMING REQUIREMENTS.
3. FIELD CUT AND WORK GIRTS, PANELS, AND TRIM AS REQUIRED.

FRAMED OPENING
FIELD LOCATED 3'-0" X 5'-0"

BOLT TABLE FRAME LINE 2				
LOCATION	QUAN	TYPE	DIA	LENGTH
9249R1-A				
Columns/Raf	4	A325	5/8"	2"

TRIM TABLE FRAME LINE 2			
◇ID	PART	LENGTH	DETAIL
1	DBLBASTR	12'-9"	TRIM_22
2	R HEAD	12'-11"	A10M
3	DBLBASTR	16'-11"	TRIM_22
4	R HEAD	13'-2"	A10M
5	7.2 JAMB	5'-3"	7.2 WDJ
6	HEAD DRIP	3'-3"	7.2 HDD
7	7.2 HEAD	3'-3"	7.2 SDD
8	7.2 JAMB	4'-11"	7.2 WDJ
9	HEAD DRIP	8'-3"	7.2 HDD
10	7.2 JAMB	2'-7"	7.2 WDJ
11	7.2 JAMB	6'-1"	7.2 WDJ
12	7.2 HEAD	14'-4"	7.2 SDD
13	HEAD DRIP	14'-9"	7.2 HDD
14	7.2 JAMB	3'-8"	7.2 WDJ
15	7.2 HEAD	4'-5"	7.2 SDD
16	7.2 HEAD	8'-3"	7.2 SDD



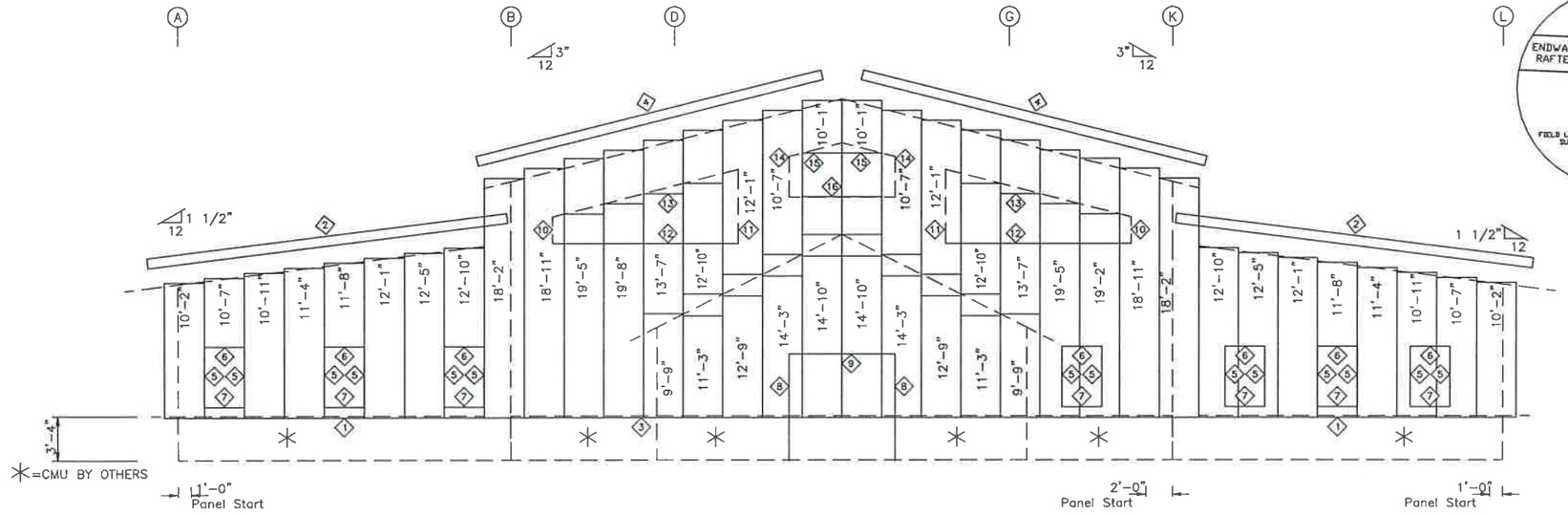
1. SEE FRAMING ELEVATIONS FOR GIRT QUANTITY AND ELEVATIONS
2. SEE DETAILS FOR BOLT AND FRAMING REQUIREMENTS.
3. FIELD CUT AND WORK GIRTS, PANELS, AND TRIM AS REQUIRED.

FRAMED OPENING
FIELD LOCATED 3'-0" X 5'-0"

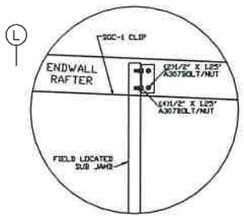
MEMBER TABLE FRAME LINE 2		
MARK	PART	LENGTH
9249-A		
EC-1	W8X18	22'-2 5/16"
EC-2	W8X18	22'-2 5/16"
DJ-1	8X25C16	5'-2 1/2"
DJ-2	8X25C16	6'-8"
DJ-3	8X25C16	6'-5"
DJ-4	8X25C16	12'-0"
DJ-5	8X25C16	8'-3"
DH-1	8.3.5CH6	9'-4 3/4"
DH-2	8X25C16	3'-0"
DH-3	8.3.5CH6	5'-0 1/4"
DH-4	8X25C16	8'-0"
DH-5	8.3.5CH6	8'-0"
DH-6	8.3.5CH6	5'-0 1/4"
DH-7	8.3.5CH6	9'-4 3/4"
DS-1	8.3.5CH6	9'-4 3/4"
DS-2	8X25C16	3'-0"
DS-3	8.3.5CH6	5'-0 1/4"
DS-4	8.3.5CH6	8'-0"
DS-5	8.3.5CH6	5'-0 1/4"
DS-6	8.3.5CH6	9'-4 3/4"
G-1	8x25C16	11'-7 1/2"
G-2	8x25Z16	13'-7 1/2"
G-3	8x25Z12	14'-3 1/2"
G-4	8x25Z14	14'-3 1/2"
G-5	8x25Z16	8'-0 1/2"
G-6	8x25C16	25'-3 1/2"
G-7	8x25Z16	29'-3 1/2"
G-8	8x25Z12	29'-3 1/2"
G-9	8x25Z16	25'-4"
G-10	8x25Z14	17'-1 1/2"
G-11	8x25C16	12'-3 1/2"
G-12	8x25Z16	14'-3 1/2"
G-13	8x25Z16	8'-0 1/2"
9249-B		
DJ-100	8X25C16	6'-8"
DH-100	8X25C16	3'-0"
DS-100	8X25C16	3'-0"
G-100	8X25C16	24'-11 3/4"
G-101	8X7DC14	24'-11 3/4"
G-102	8x25Z12	26'-3 1/2"
G-103	8x25Z16	19'-6 3/16"
9249-C		
DJ-200	8X25C16	6'-8"
DH-200	8X25C16	3'-0"
DS-200	8X25C16	3'-0"
G-203	8X25C16	24'-3 3/4"
G-204	8X25C16	24'-3 3/4"
G-205	8x25Z16	26'-3 1/2"
G-206	8x25Z12	26'-3 1/2"

GIRT LAPS

ENDWALL FRAMING: FRAME LINE 2

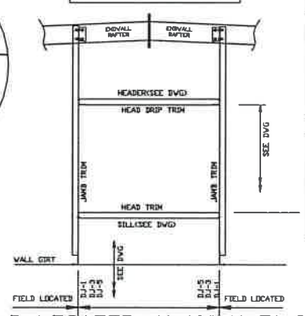


ENDWALL SHEETING & TRIM: FRAME LINE 2
PANELS: 26 Ga. 7.2 - NEED COLOR



SGC-1 DETAIL

NOTE: ERECTOR TO FIELD LOCATE HOLES AND ATTACH SGC-1 CLIP AS REQUIRED.
NOTE: ERECTOR TO FIELD CUT SUBJAMBS AND ATTACH SGC-1 CLIP AS REQUIRED.



FIELD LOCATED 4'-10" X 5'-9 5/8"
FIELD LOCATED 9'-2 1/2" X 5'-6 5/8"
FIELD LOCATED 8'-0" X 4'-3"

CONNECTION PLATES FRAME LINE 2	
ID	MARK/PART
1	SGC-1
4	SGC-1
5	FOC-1
6	FOC-1

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

15

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

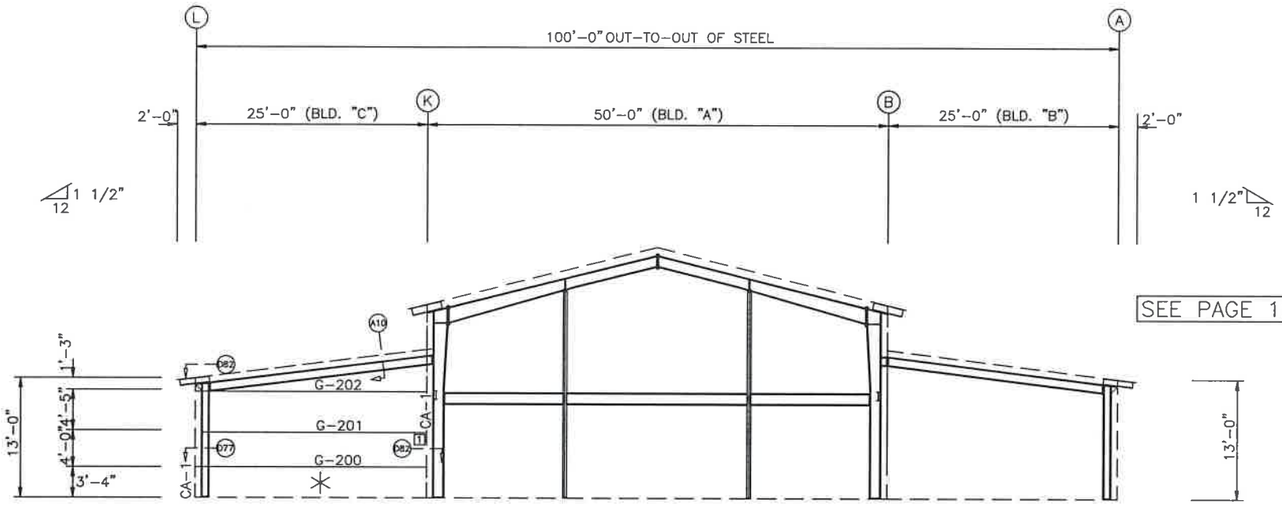
DRAWING NAME: ENDWALL FRAMING & SHEETING LAYOUT

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 3.1	DJH	SPW	NONE

TRIM TABLE FRAME LINE 5			
◇ID	PART	LENGTH	DETAIL
1	O/S CORN	9'-10"	TRIM_5
2	DBLBASTR	12'-9"	TRIM_22
3	R HEAD	12'-11"	A10M
5	RAKE	12'-9"	RAKE2

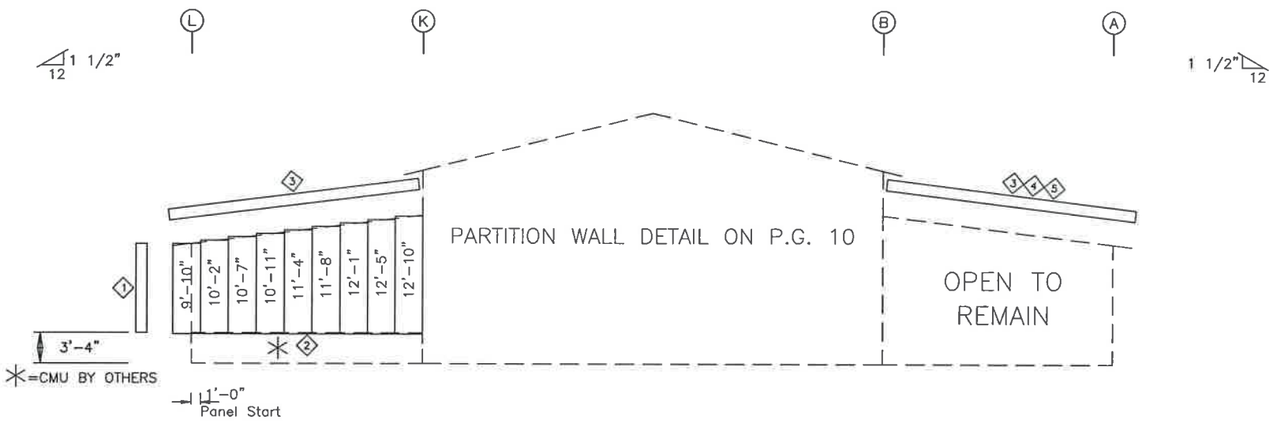
MEMBER TABLE FRAME LINE 5		
MARK	PART	LENGTH
9249R1-C		
G-200	8x25C16	24'-3 1/2"
G-201	8x25Z12	24'-3 1/2"
G-202	8x25Z12	24'-11 1/2"

CONNECTION PLATES FRAME LINE 5	
□ID	MARK/PART
1	CGC-3



SEE PAGE 12 FOR MEZZANINE INFORMATION

ENDWALL FRAMING: FRAME LINE 5



ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 Ga. R – NEED COLOR

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

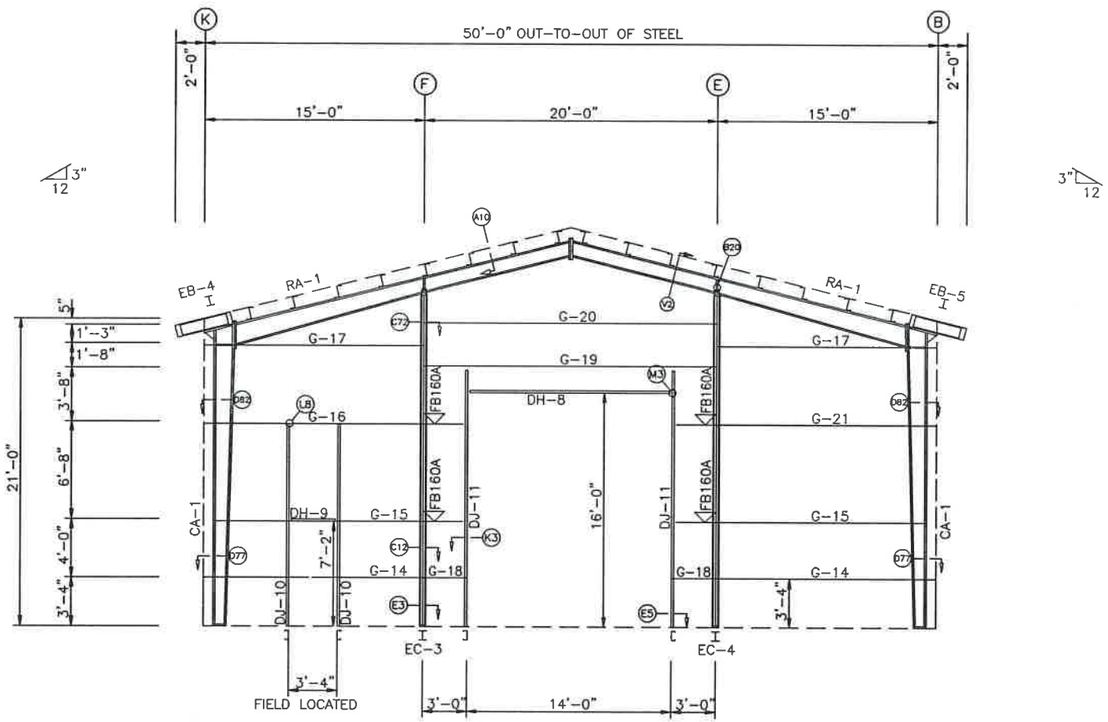
DRAWING NAME: ENDWALL FRAMING & SHEETING LAYOUT

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 3.2	DJH	SPW	NONE

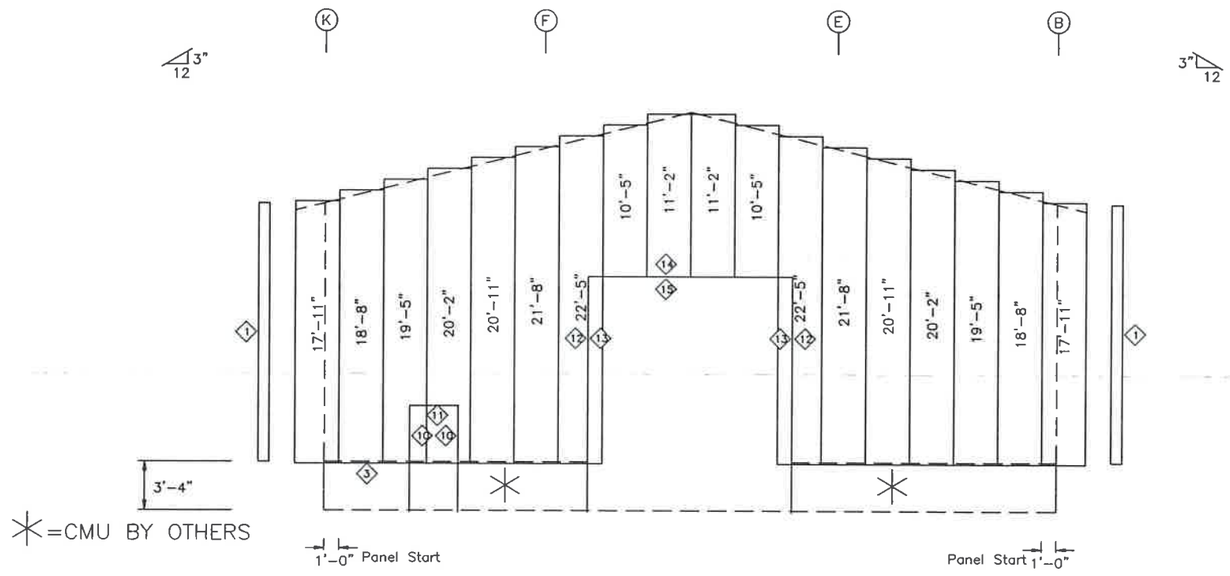
BOLT TABLE FRAME LINE 11				
LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	2	A325	5/8"	2"

TRIM TABLE FRAME LINE 11			
◇ID	PART	LENGTH	DETAIL
1	O/S CORN	17'-10"	TRIM_5
3	DBLBASTR	18'-3"	TRIM_22
10	R JAMB	4'-1"	TRIM_8
11	R HEAD	3'-7"	TRIM_61
12	CT8	16'-1"	TRIM_11
13	R JAMB	12'-11"	TRIM_8
14	CT8	14'-0"	TRIM_10
15	R HEAD	14'-3"	TRIM_61

MEMBER TABLE FRAME LINE 11		
MARK	PART	LENGTH
EB-4	W10X12	3'-6 3/4"
EB-5	W10X12	3'-6 3/4"
EC-3	W8X13	22'-11 15/16"
EC-4	W8X13	22'-11 15/16"
DJ-10	8X25C16	14'-0"
DJ-11	8X35C12	17'-8"
DH-8	8X35C16	14'-0"
DH-9	8X25C16	3'-4"
G-14	8x25C16	14'-3 1/2"
G-15	8x25Z16	16'-11 1/2"
G-16	8x25Z14	17'-7 1/2"
G-17	8x25Z16	15'-3 1/2"
G-18	8x25C16	2'-7 1/2"
G-19	8x25Z12	20'-7 1/2"
G-20	8x25Z12	20'-7 1/2"
G-21	8x25Z16	17'-7 1/2"



ENDWALL FRAMING: FRAME LINE 11



ENDWALL SHEETING & TRIM: FRAME LINE 11

PANELS: 26 Ga. R – NEED COLOR

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

15

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

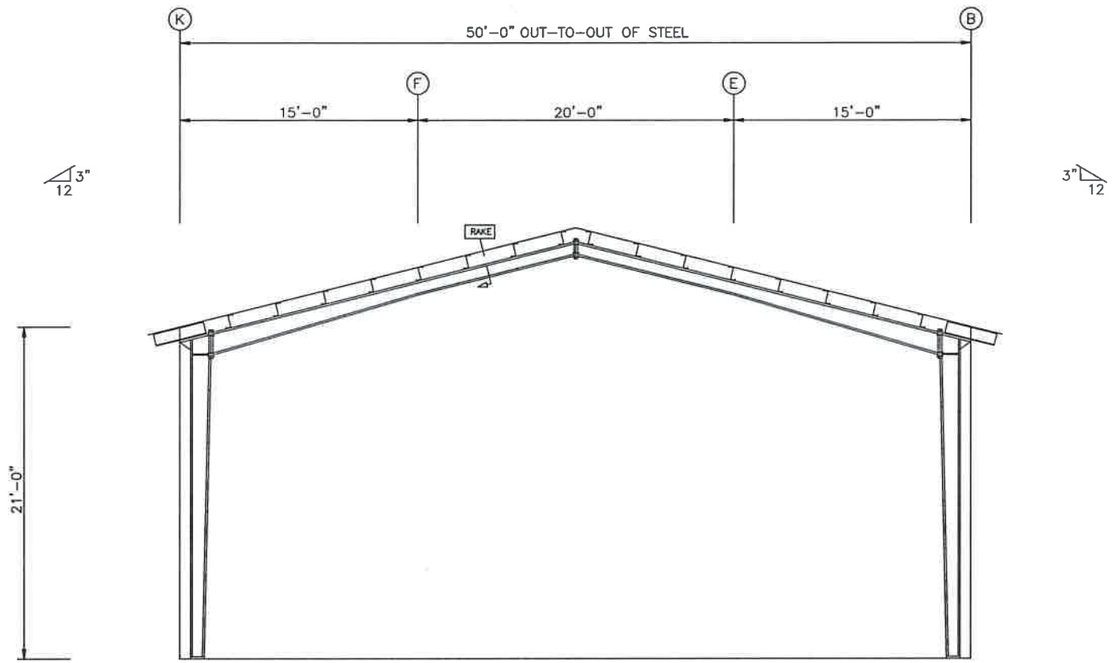
JOB NO: 9249R1 DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

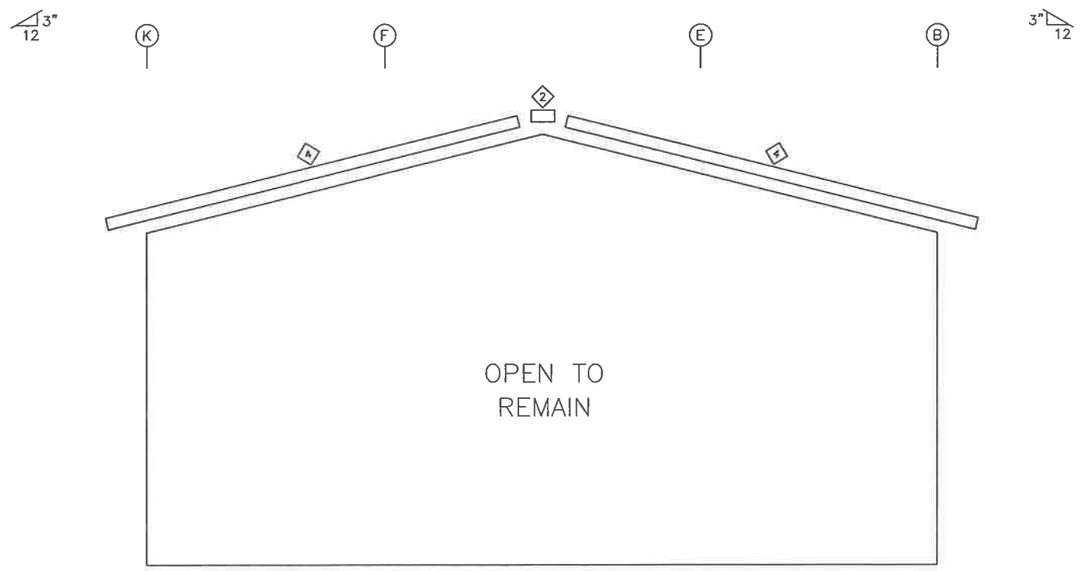
DRAWING NAME: ENDWALL FRAMING & SHEETING LAYOUT

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 3.3	DJH	SPW	NONE

TRIM TABLE FRAME LINE 2			
◇ID	PART	LENGTH	DETAIL
1	RAKE TRM	12'-9"	RAKE1
2	PEAK BOX	1'-0"	



ENDWALL FRAMING: FRAME LINE 12



ENDWALL TRIM: FRAME LINE 12

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

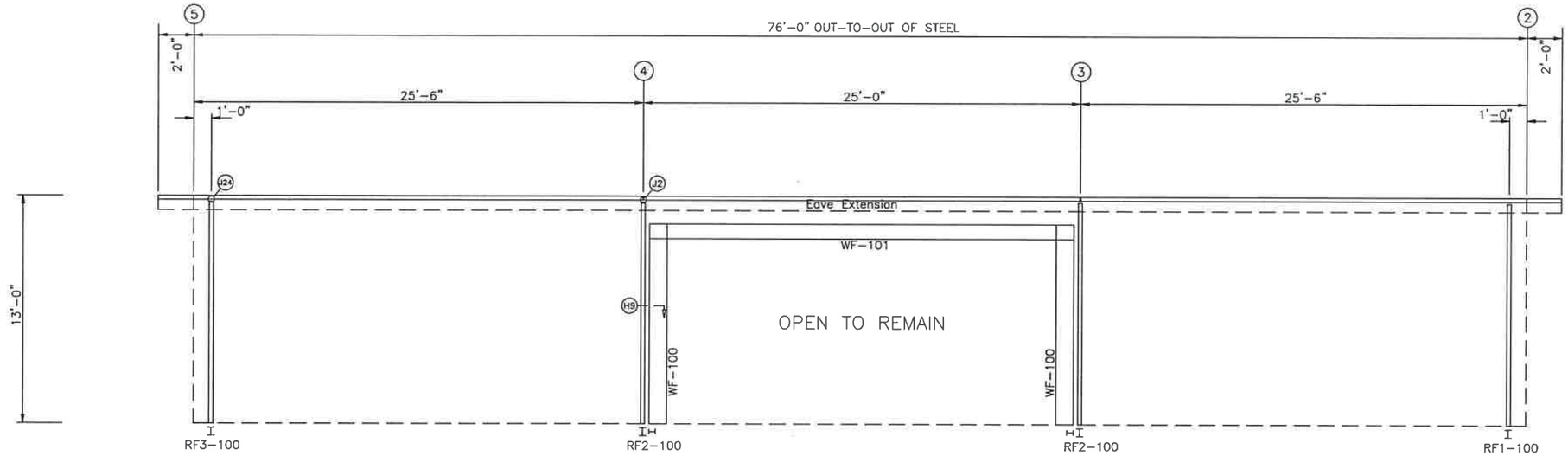
LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: ENDWALL FRAMING & SHEETING LAYOUT

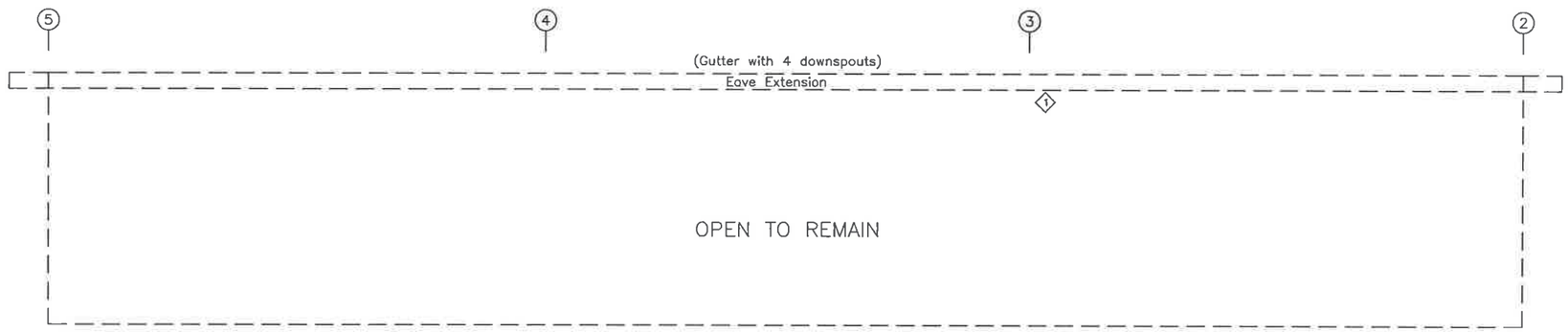
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 3.4	DJH	GTL	NONE

BOLT TABLE FRAME LINE A				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-100 – WF-101	8	A325	5/8"	2"
WF-100 – RF2-100	6	A325	5/8"	2"

MEMBER TABLE FRAME LINE A		
MARK	PART	LENGTH
WF-100	B12541	11'-8"
WF-101	B10651	22'-11 3/4"



SIDEWALL FRAMING: FRAME LINE A

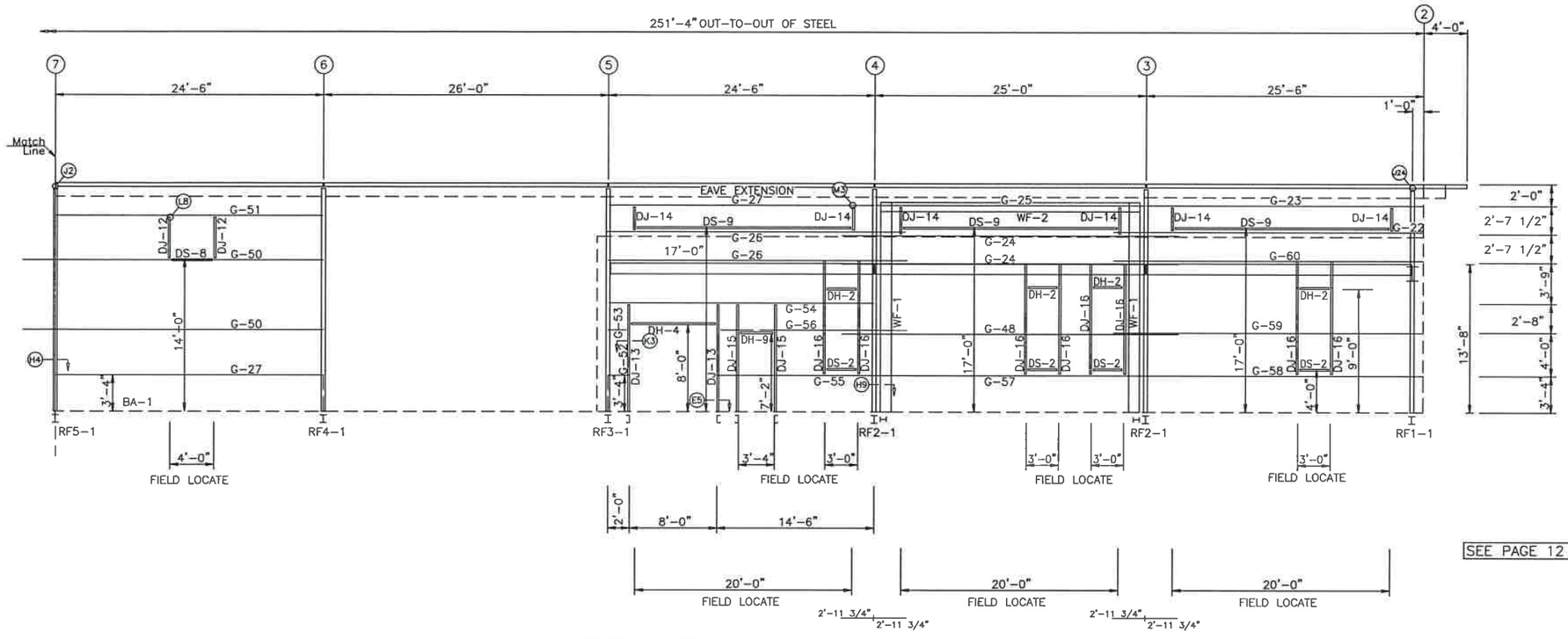


SIDEWALL TRIM: FRAME LINE A

BUILDING "B"			
ISSUE	DET	CHK	DATE

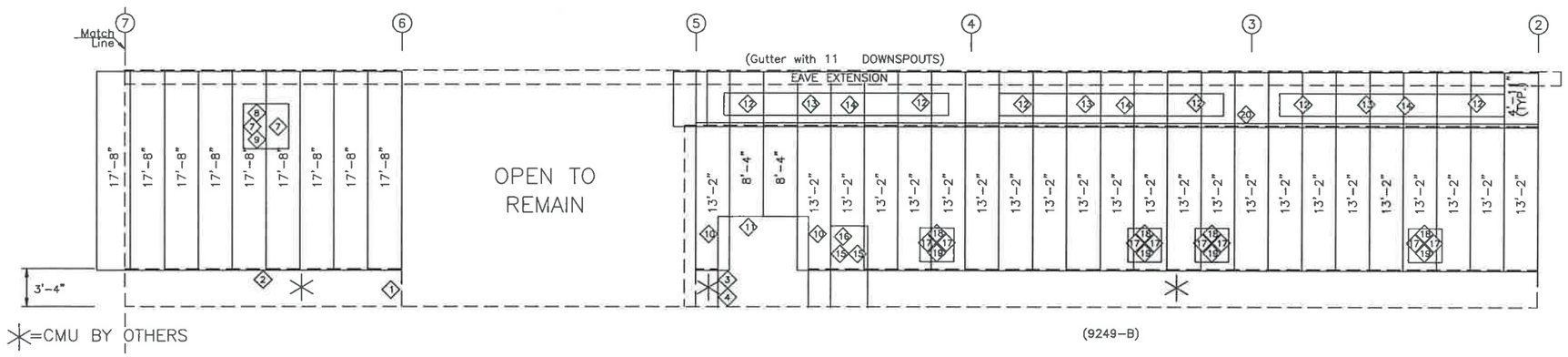
BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS			
JOB NO: 9249R1			DATE: 10/13/25
LOCATION: HIGH SPRINGS, FL 32643			
DRAWING NAME: SIDEWALL FRAMING & SHEETING LAYOUT			
DRAWING NO: PAGE 4	DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE



GIRT LAPS

SIDEWALL FRAMING: FRAME LINE B



SIDEWALL SHEETING & TRIM: FRAME LINE B

PANELS: 26 GA. PBR – COLOR
(9249-B) PANELS: 26 GA. PBR – COLOR

SEE PAGE 12 FOR MEZZANINE INFORMATION

BOLT TABLE
FRAME LINE B

LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 – WF-3	8	A325	3/4"	2 1/2"
WF-1 – RF6-1	4	A325	5/8"	2"
WF-1 – RF5-4	4	A325	5/8"	2"
WF-1 – WF-2	8	A325	3/4"	2 1/2"
WF-1 – RF2-1	4	A325	5/8"	2"

TRIM TABLE
FRAME LINE B

◇ID	PART	LENGTH	DETAIL
1	R JAMB	3'-7"	TRIM_8
2	DBLBASTR	18'-2"	TRIM_22
3	DBLBASTR	2'-3"	TRIM_22
4	DBLBASTR	16'-6"	TRIM_22
7	R JAMB	4'-3"	TRIM_8
8	R HEAD	4'-3"	TRIM_61
9	R HEAD	4'-3"	TRIM_7
10	R JAMB	4'-11"	TRIM_8
11	R HEAD	8'-3"	TRIM_61
12	R JAMB	2'-3"	TRIM_8
13	R HEAD	20'-3"	TRIM_61
14	R HEAD	20'-3"	TRIM_7
15	R JAMB	4'-1"	TRIM_8
16	R HEAD	3'-7"	TRIM_61
17	R JAMB	3'-3"	TRIM_8
18	R HEAD	3'-3"	TRIM_61
19	R HEAD	3'-3"	TRIM_7
20	STEP F	19'-0"	TRIM_65

MEMBER TABLE
FRAME LINE B

MARK	PART	LENGTH
WF-1	B12641	19'-8"
WF-2	B10651	22'-11"
DJ-12	8X25C16	4'-3"
DJ-13	8X25C16	10'-0"
DJ-14	8X25C16	2'-7 1/2"
DJ-15	8X25C16	10'-0"
DJ-16	8X25C16	10'-5"
DH-2	8.3.5CH6	3'-0"
DH-4	8X25C16	8'-0"
DH-9	8X25C16	3'-4"
DS-2	8.3.5CH6	3'-0"
DS-8	8.3.5CH6	4'-0"
DS-9	8.3.5CH6	20'-0"
G-22	8X25Z16	28'-5 1/2"
G-23	8X25C16	25'-5 1/2"
G-24	8X25Z16	30'-11 1/2"
G-25	8X25C16	24'-11 1/2"
G-26	8X25Z16	27'-9 1/2"
G-27	8X25C16	24'-5 1/2"
G-48	8X25Z14	30'-11 1/2"
G-50	8X25Z12	27'-9 1/2"
G-51	8X25C12	24'-5 1/2"
G-52	8X25C16	1'-7 1/2"
G-53	8X25Z16	1'-11 1/2"
G-54	8X7DC14	24'-5 1/2"
G-55	8X25C16	14'-1 1/2"
G-56	8X25Z14	17'-1 1/2"
G-57	8X25C14	24'-11 1/2"
G-58	8X25C16	25'-5 1/2"
G-59	8X25Z12	28'-5 1/2"
G-60	8X25Z14	28'-5 1/2"

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

BUILDING "A"

ISSUE	DET	CHK	DATE

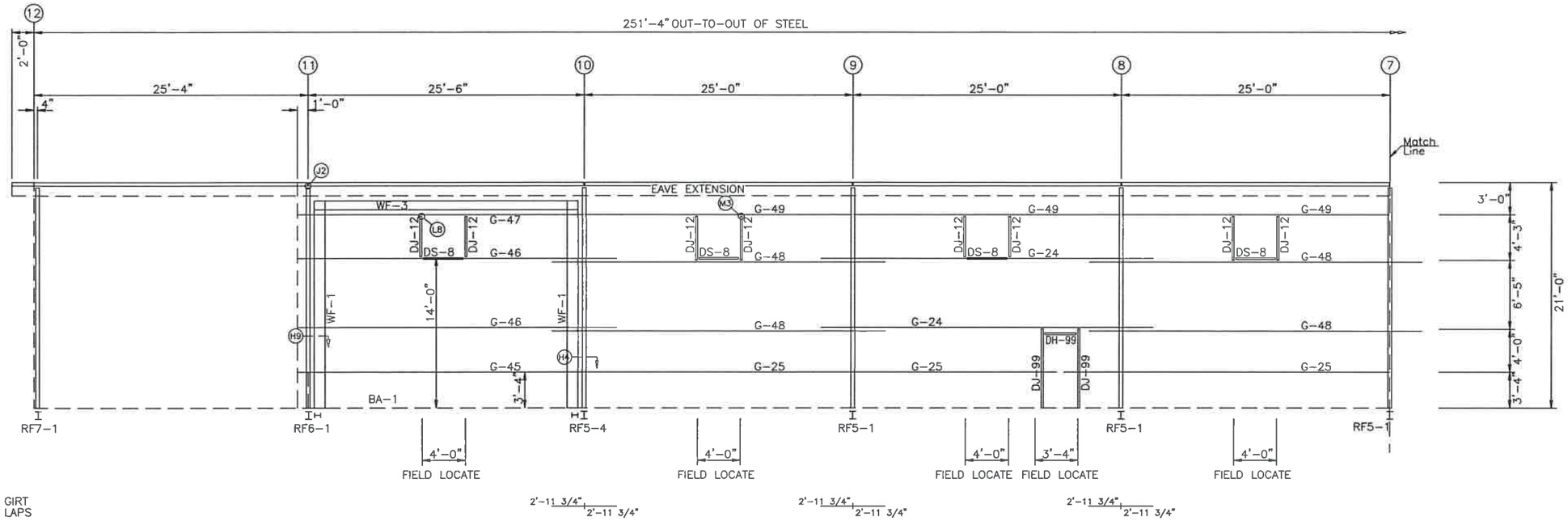
BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS
JOB NO: 9249R1
DATE: 10/13/25
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: SIDEWALL FRAMING & SHEETING LAYOUT
DRAWING NO: PAGE 4.1
DRAWN BY: DJH
CHECKED BY: GTL
SCALE: NONE

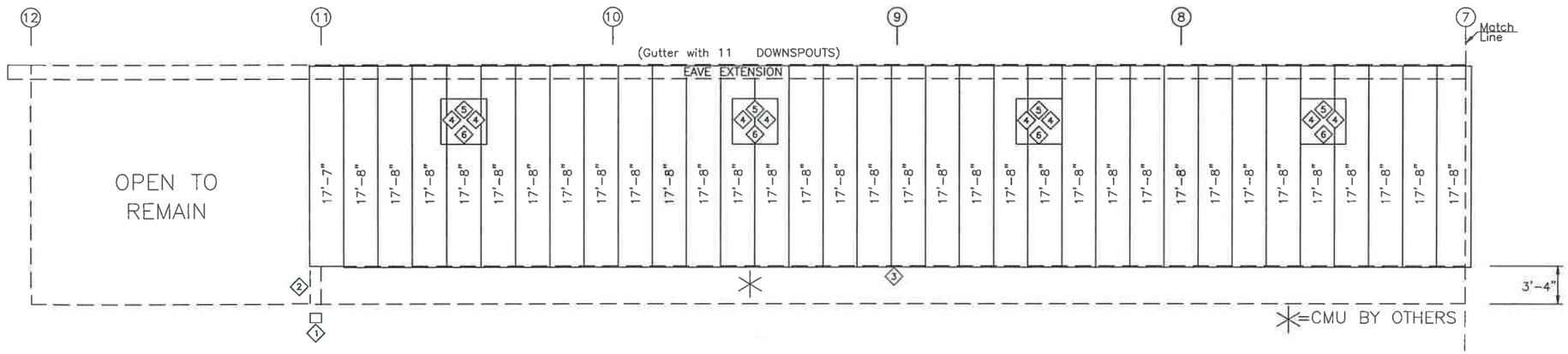
BOLT TABLE FRAME LINE B				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 – WF-3	8	A325	3/4"	2 1/2"
WF-1 – RF6-1	4	A325	5/8"	2"
WF-1 – RF5-4	4	A325	5/8"	2"
WF-1 – WF-2	8	A325	3/4"	2 1/2"
WF-1 – RF2-1	4	A325	5/8"	2"

TRIM TABLE FRAME LINE B			
◇ID	PART	LENGTH	DETAIL
1	DRIP BASE	1'-3"	TRIM_16
2	O/S CORN	3'-6"	TRIM_5
3	DBLBASTR	18'-2"	TRIM_22
4	R JAMB	4'-3"	TRIM_8
5	R HEAD	4'-3"	TRIM_61
6	R HEAD	4'-3"	TRIM_7

MEMBER TABLE FRAME LINE B		
MARK	PART	LENGTH
WF-1	B12641	19'-8"
WF-3	B10651	23'-5"
DJ-12	8X25C16	4'-3"
DJ-99	8X25C16	7'-4"
DH-99	8X25C16	3'-4"
DS-8	8.3.5CH6	4'-0"
G-24	8x25Z16	30'-11 1/2"
G-25	8x25C16	24'-11 1/2"
G-45	8x25C14	26'-5 1/2"
G-46	8x25Z12	29'-5 1/2"
G-47	8x25C12	26'-5 1/2"
G-48	8x25Z14	30'-11 1/2"
G-49	8x25C12	24'-11 1/2"



SIDEWALL FRAMING: FRAME LINE B



SIDEWALL SHEETING & TRIM: FRAME LINE B

PANELS: 26 GA. PBR – COLOR
(9249-B) PANELS: 26 GA. PBR – COLOR

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

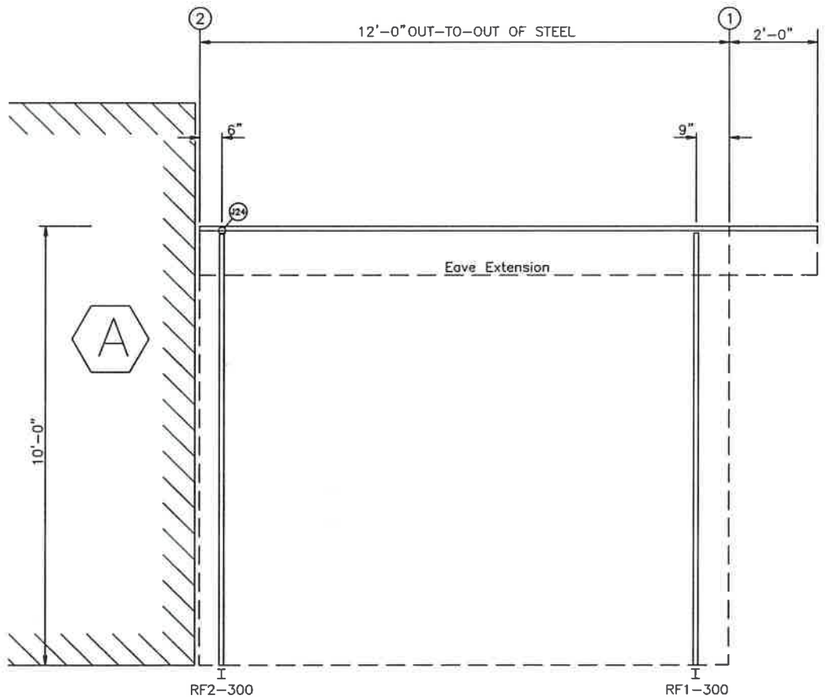
CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

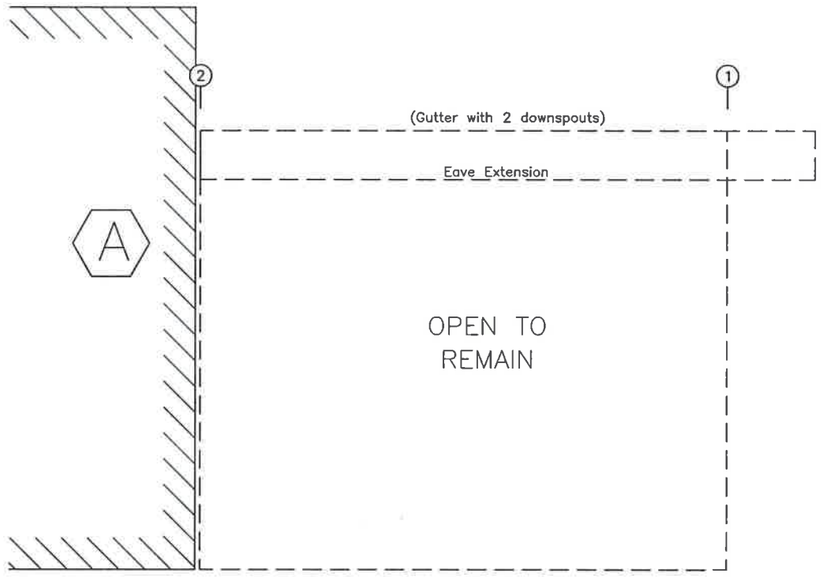
LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: SIDEWALL FRAMING & SHEETING LAYOUT

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 4.2	DJH	GTL	NONE

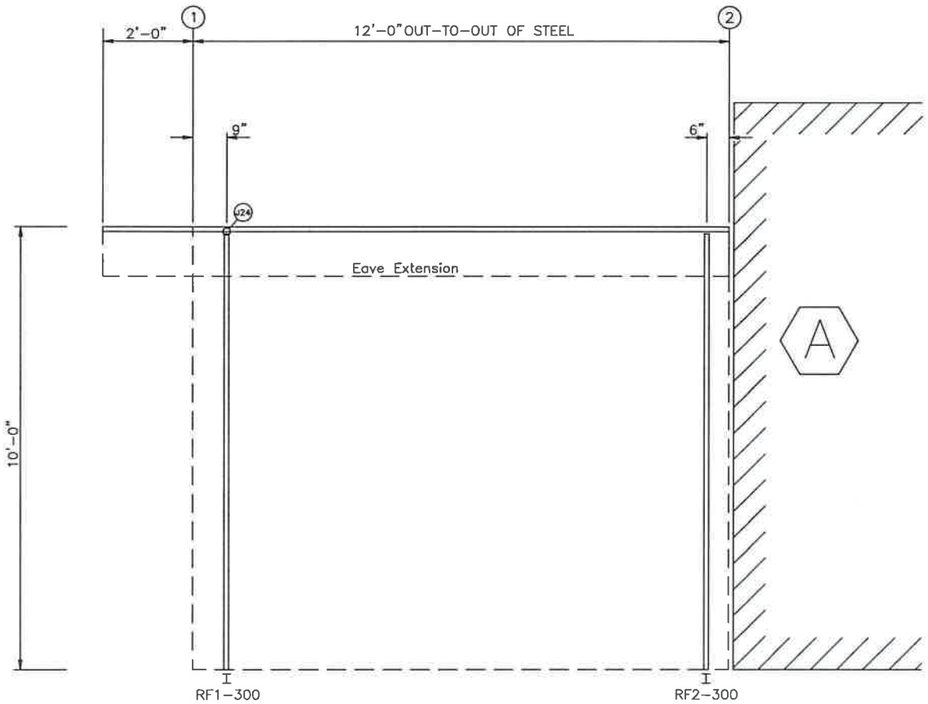


SIDEWALL FRAMING: FRAME LINE C

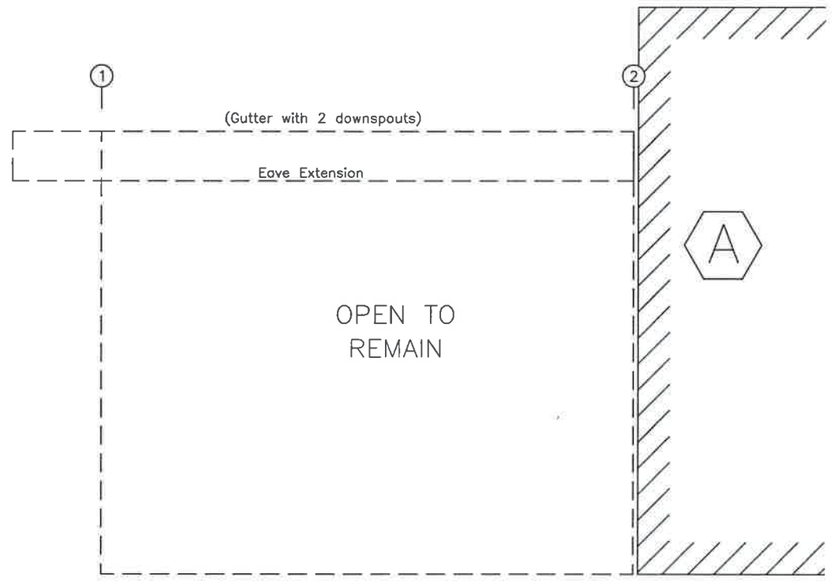


SIDEWALL TRIM: FRAME LINE C

BUILDING "D"			
ISSUE	DET	CHK	DATE
BUILDINGS AND MORE			
CUSTOMER: PINNACLE SITE SOLUTIONS			
JOB NO: 9249R1		DATE: 10/13/25	
LOCATION: HIGH SPRINGS, FL 32643			
DRAWING NAME: SIDEWALL FRAMING & SHEETING LAYOUT			
DRAWING NO: PAGE 4.3	DRAWN BY: DJH	CHECKED BY: GTL	SCALE: NONE



SIDEWALL FRAMING: FRAME LINE J



SIDEWALL TRIM: FRAME LINE J

BUILDING "D"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

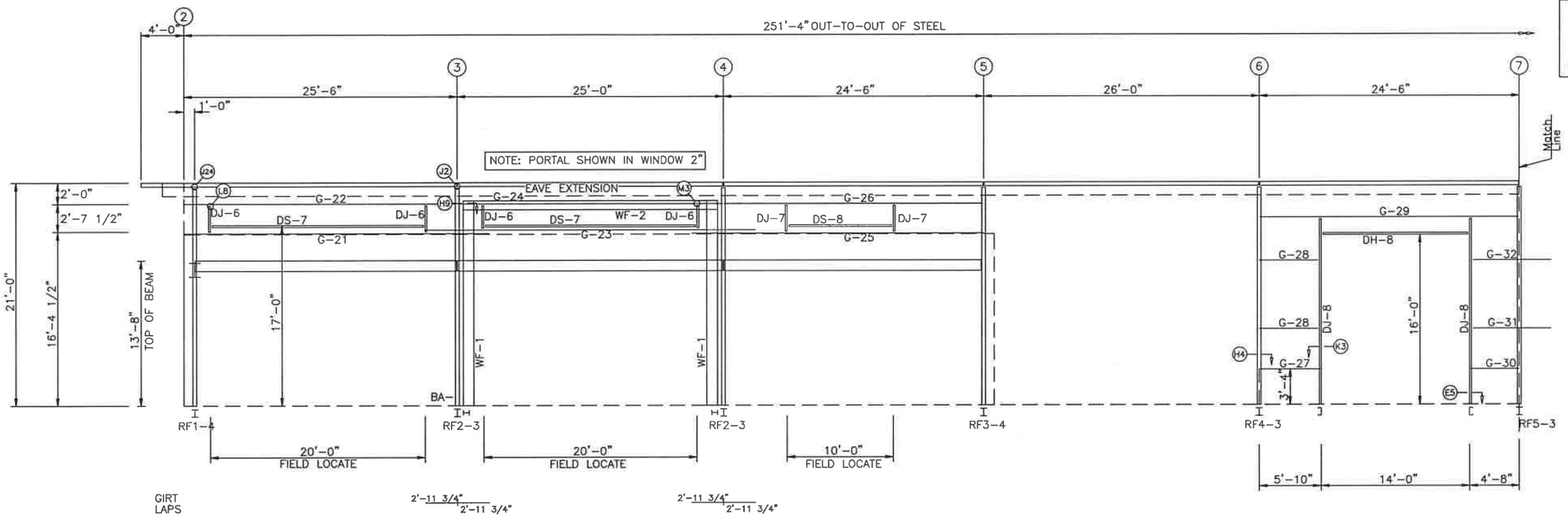
CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

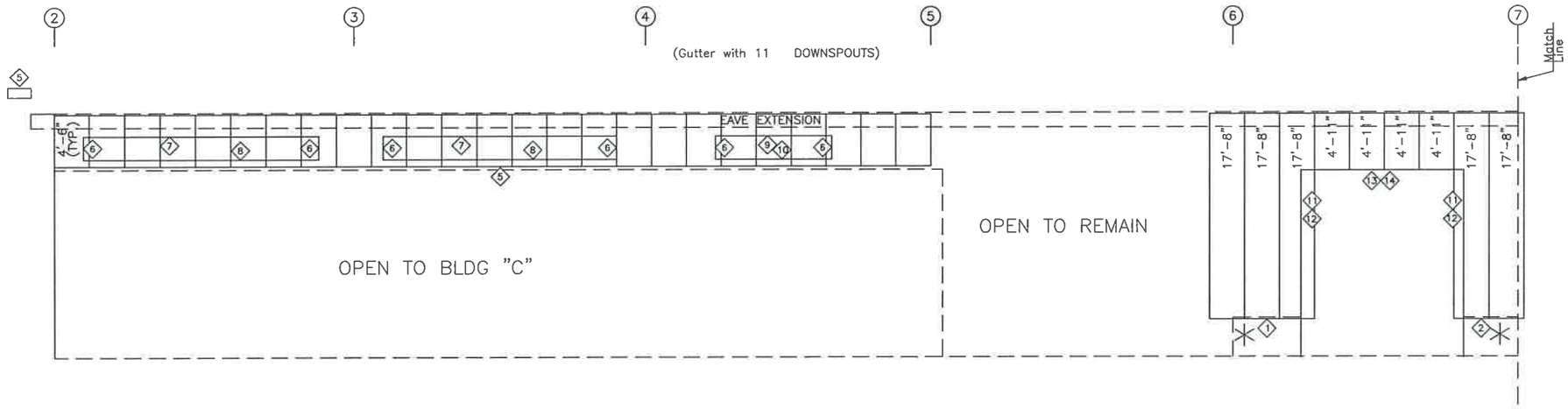
LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: SIDEWALL FRAMING & SHEETING LAYOUT

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 4.4	DJH	GTL	NONE



SIDEWALL FRAMING: FRAME LINE K



SIDEWALL SHEETING & TRIM: FRAME LINE K

PANELS: 26 GA. PBR – COLOR

=CMU BY OTHERS

SEE PAGE 12 FOR MEZZANINE INFORMATION

BOLT TABLE
FRAME LINE K

LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 – WF-2	8	A325	3/4"	2 1/2"
WF-1 – RF2-3	4	A325	5/8"	2"
WF-1 – WF-3	8	A325	3/4"	2 1/2"
WF-1 – RF5-5	4	A325	5/8"	2"
WF-1 – RF6-3	4	A325	5/8"	2"

TRIM TABLE
FRAME LINE K

◇ID	PART	LENGTH	DETAIL
1	DBLBASTR	6'-1"	TRIM_22
2	DBLBASTR	10'-5"	TRIM_22
5	STEP F	19'-0"	TRIM_65
6	R JAMB	2'-3"	TRIM_8
7	R HEAD	20'-3"	TRIM_61
8	R HEAD	20'-3"	TRIM_7
9	R HEAD	10'-3"	TRIM_61
10	R HEAD	10'-3"	TRIM_7
11	CT8	16'-1"	TRIM_11
12	R JAMB	12'-11"	TRIM_8
13	CT8	14'-0"	TRIM_10
14	R HEAD	14'-3"	TRIM_61

MEMBER TABLE
FRAME LINE K

MARK	PART	LENGTH
WF-1	B12641	19'-8"
WF-2	B10651	22'-11"
DJ-6	8X25C16	2'-7 1/2"
DJ-7	8X25C16	2'-7 1/2"
DJ-8	8X35C12	17'-8"
DH-8	8X35C16	14'-0"
DS-7	8X35C14	20'-0"
DS-8	8.3.5CH6	10'-0"
G-21	8x25Z16	28'-5 1/2"
G-22	8x25C16	25'-5 1/2"
G-23	8x25Z16	30'-11 1/2"
G-24	8x25C16	24'-11 1/2"
G-25	8x25Z16	27'-9 1/2"
G-26	8x25C16	24'-5 1/2"
G-27	8x25C16	5'-5 1/2"
G-28	8x25Z16	5'-9 1/2"
G-29	8X7DC14	24'-5 1/2"
G-30	8x25C16	4'-3 1/2"
G-31	8x25Z16	7'-3 1/2"
G-32	8x25Z12	7'-3 1/2"
MB-9	w12X14	24'-4 3/4"
MB-10	w12X14	24'-10 3/4"
MB-11	w12X14	24'-2 5/8"

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

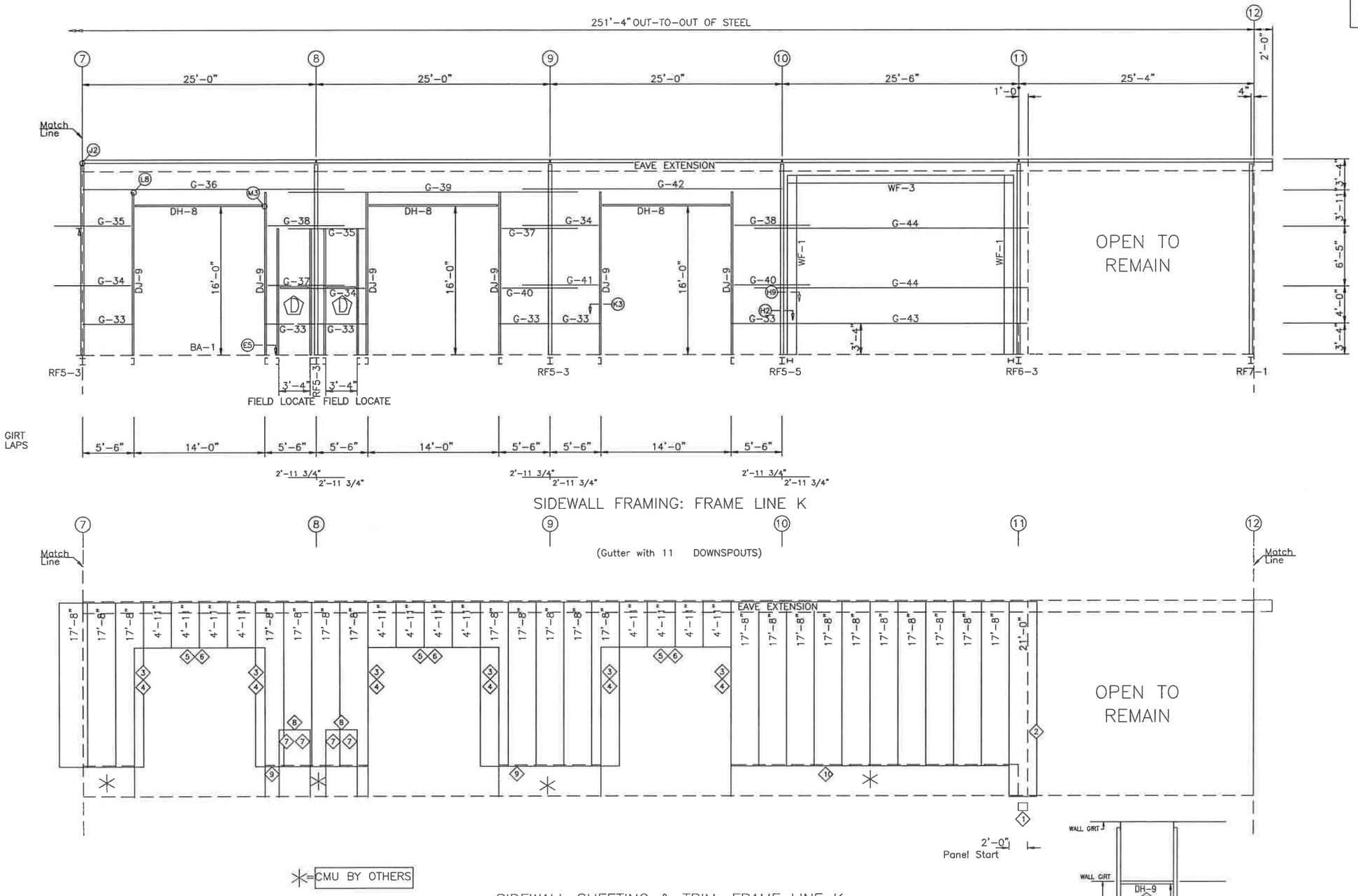
DRAWING NAME: SIDEWALL FRAMING & SHEETING LAYOUT

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 4.5	DJH	SPW	NONE

BOLT TABLE FRAME LINE K				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 – WF-2	8	A325	3/4"	2 1/2"
WF-1 – RF2-3	4	A325	5/8"	2"
WF-1 – WF-3	8	A325	3/4"	2 1/2"
WF-1 – RF5-5	4	A325	5/8"	2"
WF-1 – RF6-3	4	A325	5/8"	2"

TRIM TABLE FRAME LINE K			
◇ID	PART	LENGTH	DETAIL
1	DRIP BASE	1'-3"	TRIM_16
2	O/S CORN	3'-6"	TRIM_5
3	CT8	16'-1"	TRIM_11
4	R JAMB	12'-11"	TRIM_8
5	CT8	14'-0"	TRIM_10
6	R HEAD	14'-3"	TRIM_61
7	R JAMB	4'-1"	TRIM_8
8	R HEAD	3'-7"	TRIM_61
9	DBLBASTR	11'-3"	TRIM_22
10	DBLBASTR	15'-9"	TRIM_22

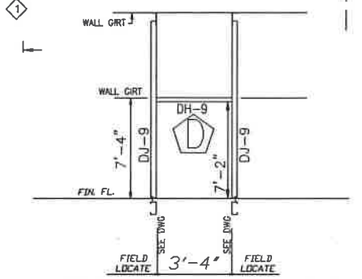
MEMBER TABLE FRAME LINE K		
MARK	PART	LENGTH
WF-1	B12641	19'-8"
WF-3	B10651	23'-5"
DJ-9	8X35C12	17'-8"
DJ-10	8X25C16	13'-9"
DH-8	8X35C16	14'-0"
DH-9	8X25C16	3'-4"
G-33	8x25C16	5'-1 1/2"
G-34	8x25Z16	8'-1 1/2"
G-35	8x25Z12	8'-1 1/2"
G-36	8x25Z16	27'-11 1/2"
G-37	8x25Z16	8'-1 1/2"
G-38	8x25Z12	8'-1 1/2"
G-39	8x25Z16	30'-11 1/2"
G-40	8x25Z14	8'-1 1/2"
G-41	8x25Z14	8'-1 1/2"
G-42	8x25Z16	28'-3 1/2"
G-43	8x25C14	26'-5 1/2"
G-44	8x25Z12	29'-5 1/2"



SIDEWALL FRAMING: FRAME LINE K

SIDEWALL SHEETING & TRIM: FRAME LINE K

PANELS: 26 GA. PBR – COLOR



FIELD LOCATED 3'-4" X 7'-2" F.O.

1. FIELD CUT AND WORK GIRTS, PANELS, AND TRIM AS REQUIRED.
2. REFER TO DETAIL PAGES FOR APPLICABLE TRIM DETAILS.

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

15

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

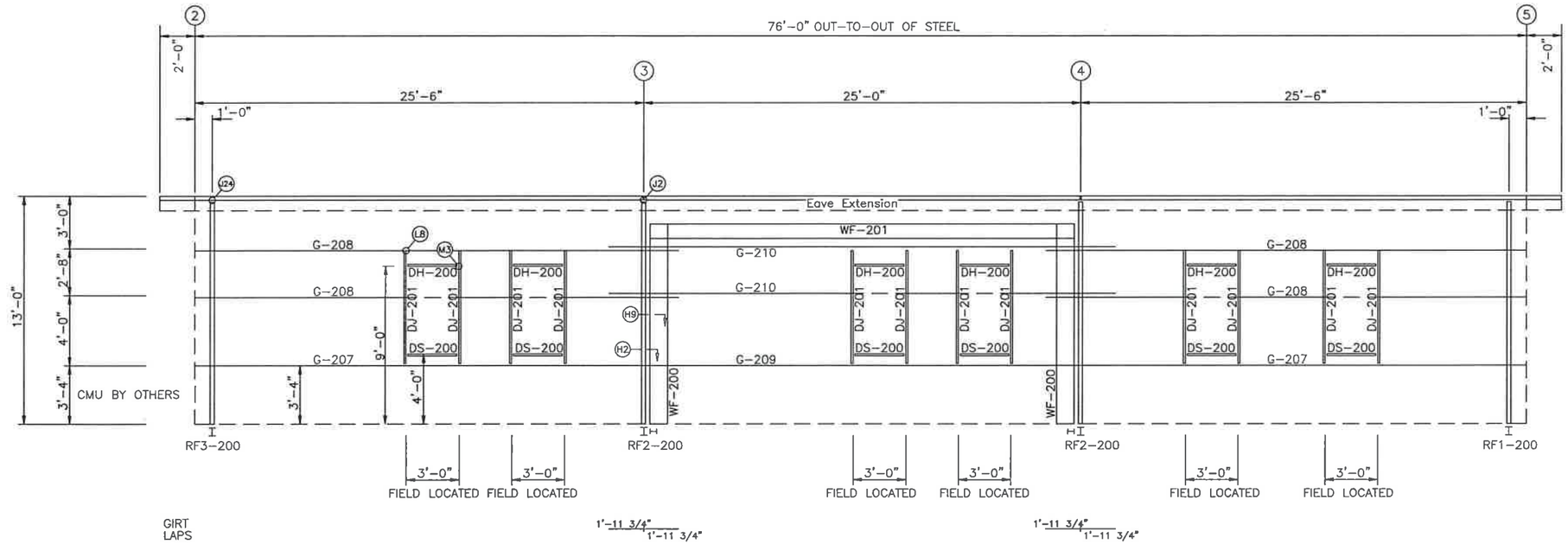
DRAWING NAME: SIDEWALL FRAMING & SHEETING LAYOUT

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 4.6	DJH	SPW	NONE

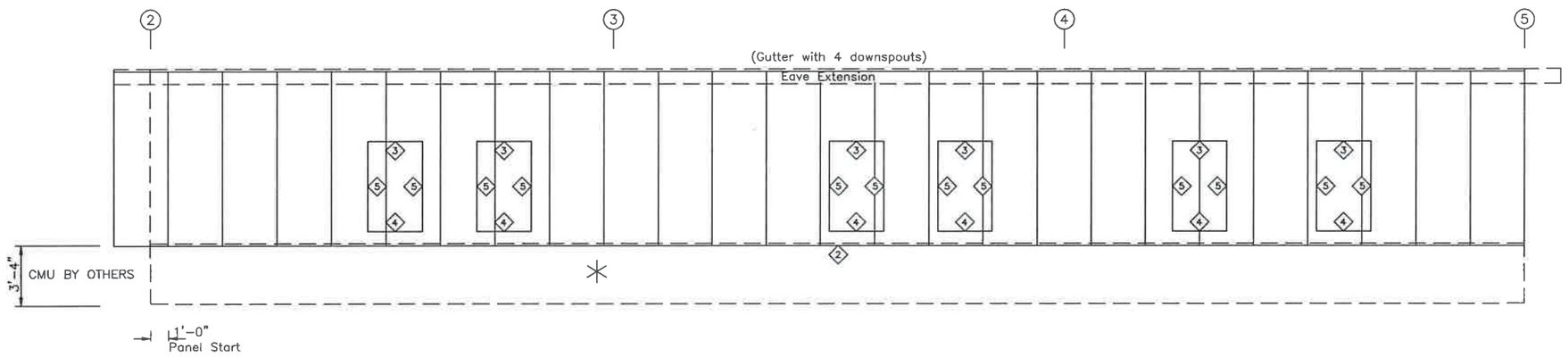
BOLT TABLE FRAME LINE L				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-200 – WF-201	8	A325	5/8"	2"
WF-200 – RF2-200	6	A325	5/8"	2"

TRIM TABLE FRAME LINE L			
◇ID	PART	LENGTH	DETAIL
2	DBLBASTR	19'-3"	TRIM_22
3	HEAD DRIP	3'-3"	7.2 HDD
4	7.2 HEAD	3'-3"	7.2 SDD
5	7.2 JAMB	3'-3"	7.2 WDJ

MEMBER TABLE FRAME LINE L		
MARK	PART	LENGTH
WF-200	B12541	11'-8"
WF-201	B10651	22'-11 3/4"
DJ-201	8X25C16	6'-8"
DH-200	8X25C16	3'-0"
DS-200	8X25C16	3'-0"
G-207	8x25C14	25'-9 1/2"
G-208	8x25Z16	27'-5 1/2"
G-209	8x25C16	24'-11 1/2"
G-210	8x25Z16	28'-11 1/2"



SIDEWALL FRAMING: FRAME LINE L



SIDEWALL SHEETING & TRIM: FRAME LINE L

PANELS: 26 Ga. 7.2 – NEED COLOR

✳=CMU BY OTHERS

BUILDING "C"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

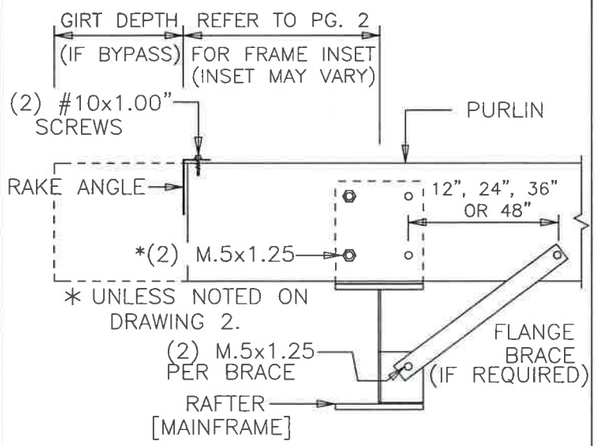
CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: SIDEWALL FRAMING & SHEETING LAYOUT

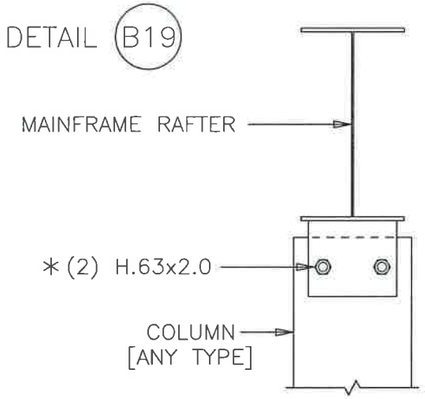
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 4.7	DJH	SPW	NONE



DETAIL A10

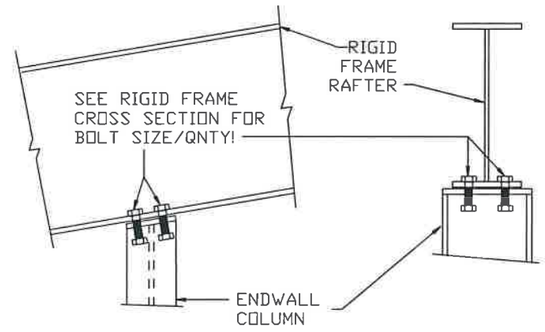
PURLIN TO ENDWALL RAFTER

NOTE: HAND TIGHTEN EW COLUMN/RAFTER CONNECTION BOLTS, THEN INTERRUPT THREADS.

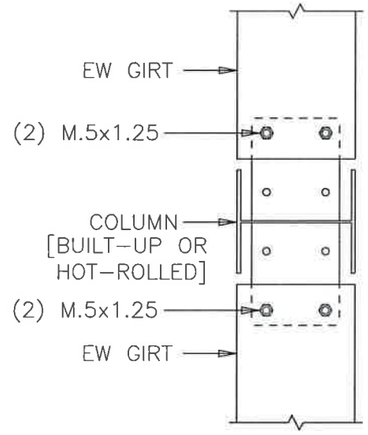


* UNLESS NOTED EW FRAMING OR PARTITION FRAMING PLAN(S)

MAINFRAME RAFTER / COLUMN CONNECTION



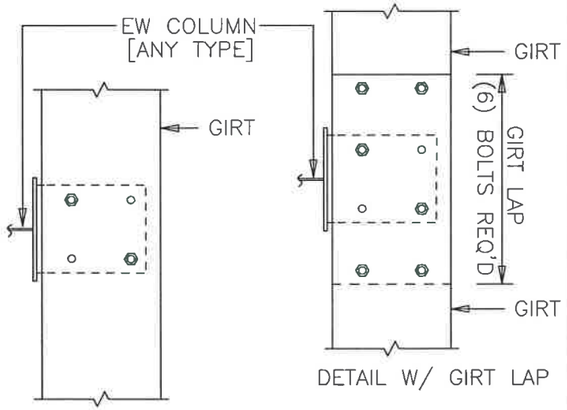
B20 RIGID FRAME RAFTER TO ENDWALL COLUMN



ENDWALL GIRTS TO INTERIOR COLUMN

C12 ENDWALL GIRT TO COLUMN

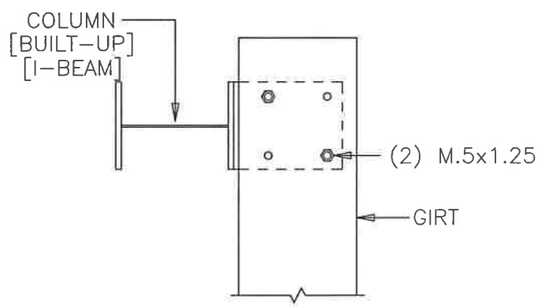
ALL BOLTS SHOWN ARE: 1/2"x1.25" A307



DETAIL FOR NO GIRT LAP

DETAIL W/ GIRT LAP

DETAIL C72



* (4) M.5x1.25 – IF (2) GIRTS / NO LAP.

BYPASS ENDWALL GIRT TERMINATION AT COLUMN

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

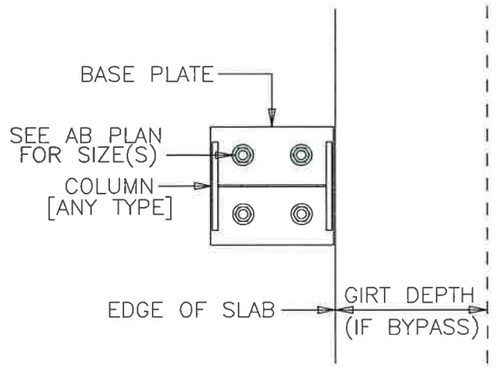
JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: FRAMING DETAILS

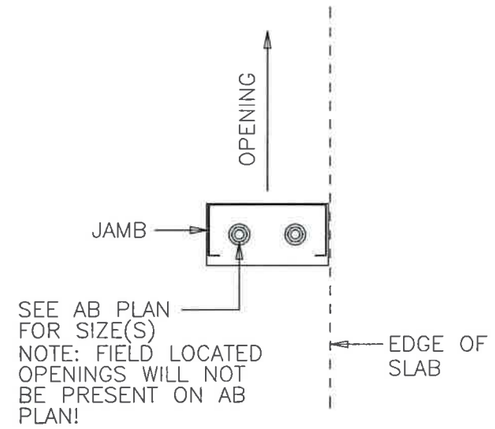
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 5	DJH	SPW	NONE

DETAIL E3



ENDWALL COLUMN BASE DETAIL

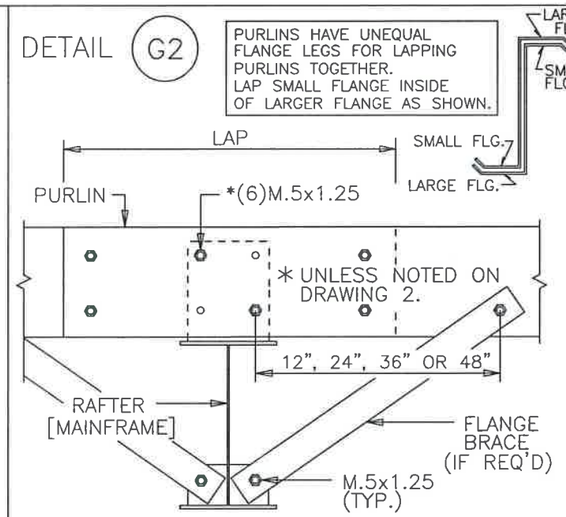
DETAIL E5



FRAMED OPENING JAMB BASE DETAIL

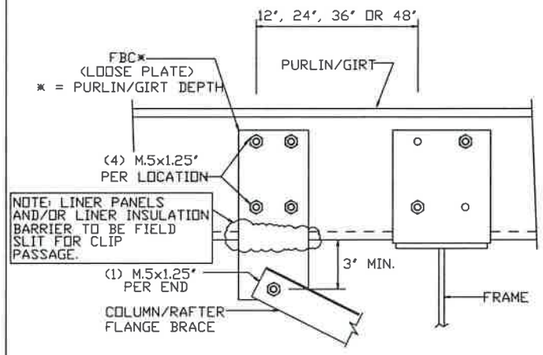
DETAIL G2

PURLINS HAVE UNEQUAL FLANGE LEGS FOR LAPPING PURLINS TOGETHER. LAP SMALL FLANGE INSIDE OF LARGER FLANGE AS SHOWN.

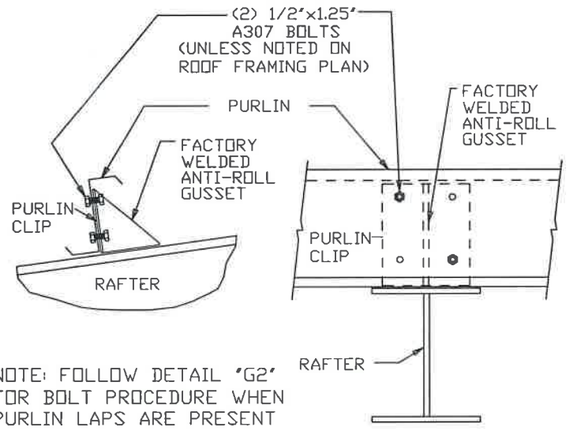


PURLIN TO MAINFRAME RAFTER

NOTE: FBC* CLIP MUST BE INSTALLED PRIOR TO ANY LINER SYSTEM!



G25 FLANGE BRACE TO PURLIN/GIRT DURING LINER TYPE INSTANCES

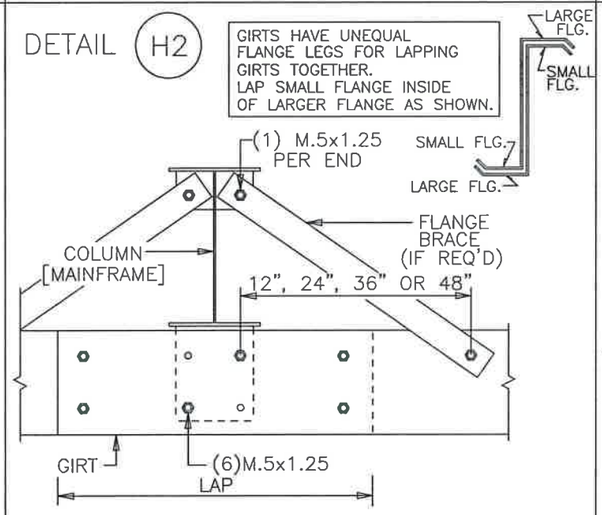


NOTE: FOLLOW DETAIL 'G2' FOR BOLT PROCEDURE WHEN PURLIN LAPS ARE PRESENT

G26 WELDED ANTI-ROLL CLIP

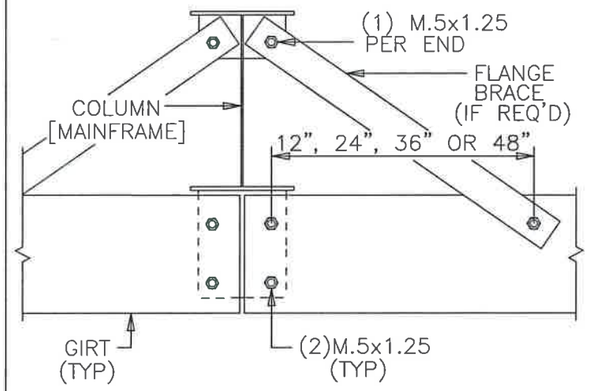
DETAIL H2

GIRTS HAVE UNEQUAL FLANGE LEGS FOR LAPPING GIRTS TOGETHER. LAP SMALL FLANGE INSIDE OF LARGER FLANGE AS SHOWN.



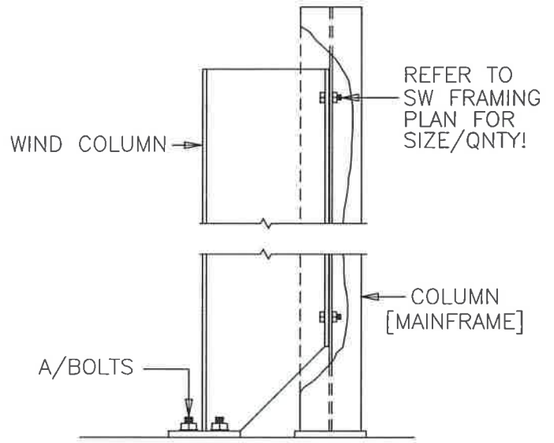
GIRT TO MAINFRAME COLUMN

DETAIL H4



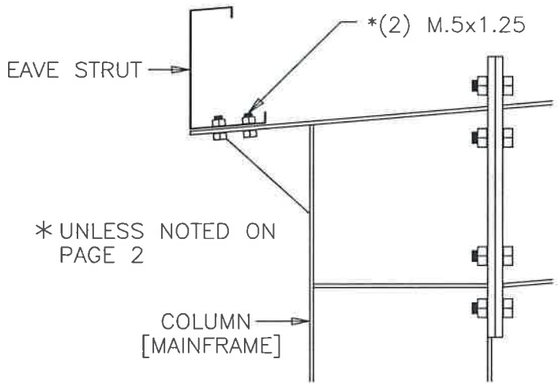
GIRT TO MAINFRAME COLUMN

DETAIL H9

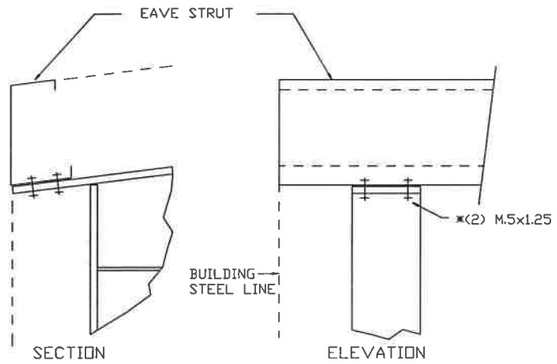


WIND COLUMN DETAIL

ISSUE	DET	CHK	DATE
BUILDINGS AND MORE			
CUSTOMER: PINNACLE SITE SOLUTIONS			
JOB NO: 9249R1	DATE: 10/13/25		
LOCATION: HIGH SPRINGS, FL 32643			
DRAWING NAME: FRAMING DETAILS			
DRAWING NO: PAGE 5.1	DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE

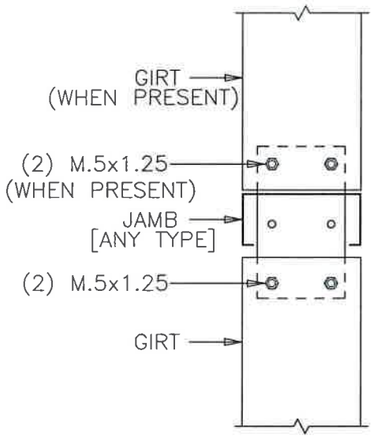


J2 EAVE STRUT CONNECTION AT MAINFRAME

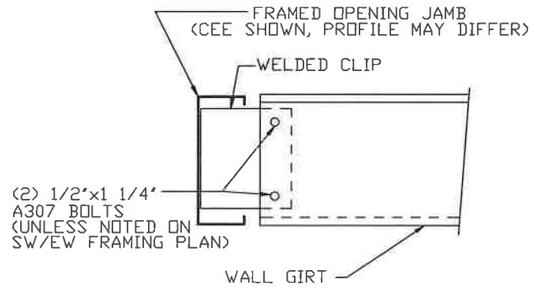


J24 EAVE STRUT TO RIGID FRAME

DETAIL K3

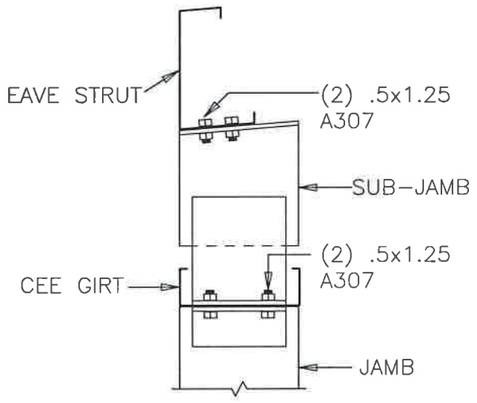


GIRTS TO JAMB



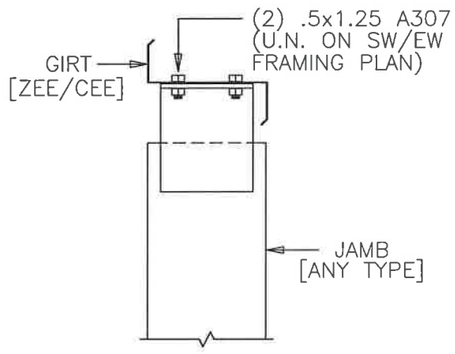
K9 WALL GIRT TO JAMB

DETAIL L2



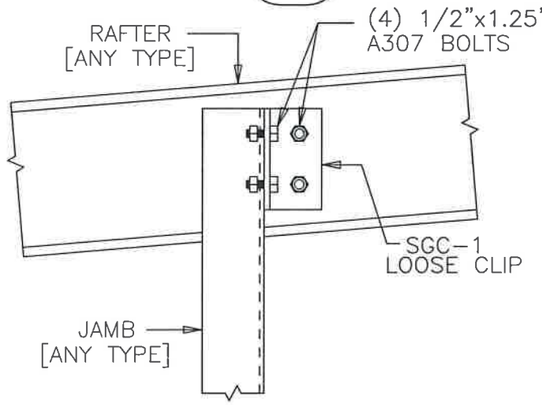
FRAMED OPENING JAMB TO CEE GIRT TO SUBJAMB TO EAVE STRUT

DETAIL L8



FRAMED OPENING JAMB TO GIRT

DETAIL L100



* UNLESS NOTED ON EW FRAMING PLAN

JAMB/COLUMN TO BYPASS EW RAFTER

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

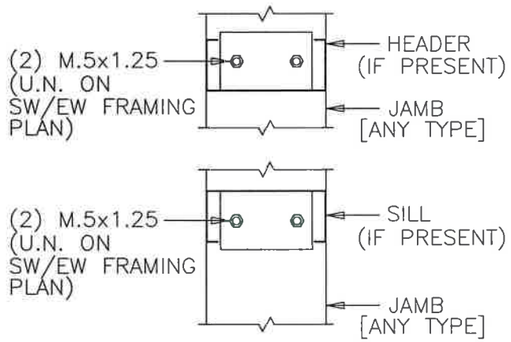
JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

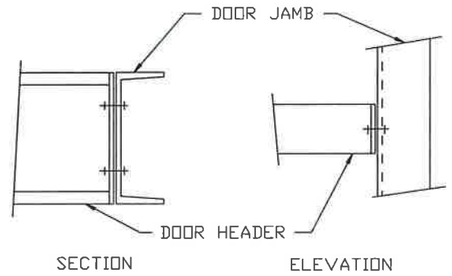
DRAWING NAME: FRAMING DETAILS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 5.2	DJH	SPW	NONE

DETAIL M3

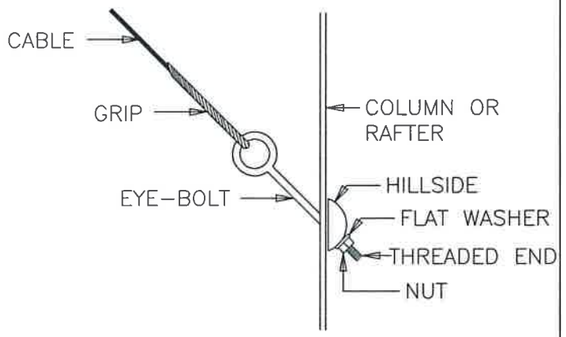


FRAMED OPENING HEADER/SILL TO JAMB



M7 DOOR HEADER TO DOOR JAMB

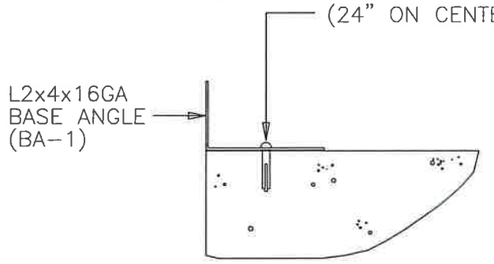
Q2 CABLE INSTALLATION DETAIL



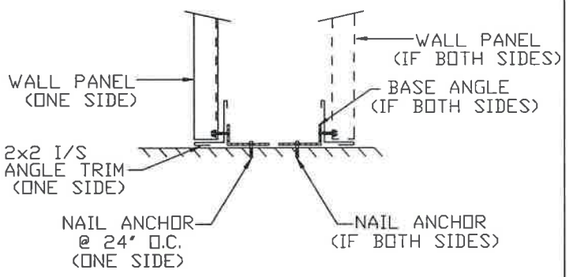
NOTE: WHEN FLUSH GIRTS/PURLINS ARE USED, FIELD SLOT GIRT OR PURLIN AS REQ'D FOR CABLE/ROD PASSAGE THROUGH PURLIN/GIRT.

DETAIL T1

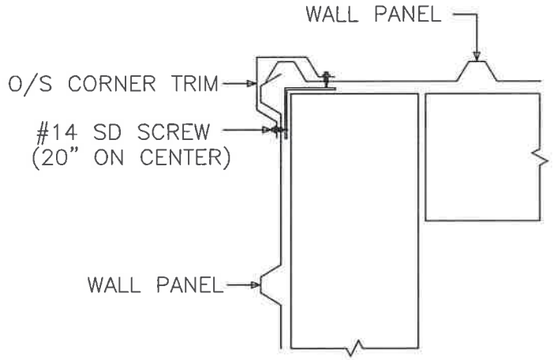
1/4" x 1 1/4" ZINC HAMMER DRIVES
ZAMAK ALLOY (ASTM B633, SC1, TYPE III)
(24" ON CENTER)



BASE ANGLE DETAIL



T10 SECTION THRU PARTITION BASE AND CONCRETE FLOOR



TRIM_5

O/S CORNER DETAIL

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

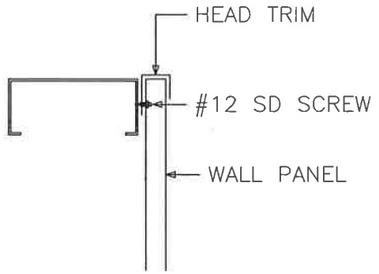
CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

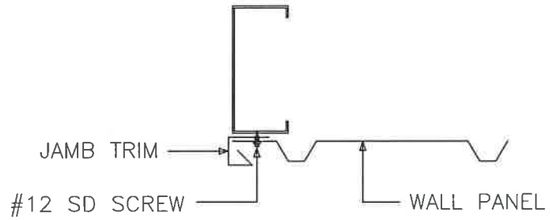
DRAWING NAME: FRAMING DETAILS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 5.3	DJH	SPW	NONE



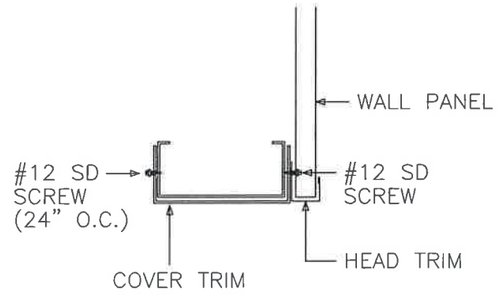
TRIM_7

HEAD TRIM DETAIL AT SILL



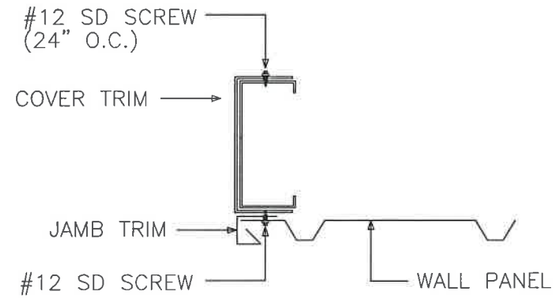
TRIM_8

JAMB TRIM DETAIL AT JAMB



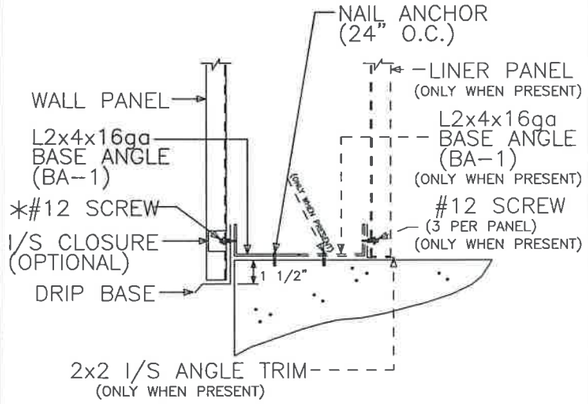
TRIM_10

COVER TRIM DETAIL AT HEADER



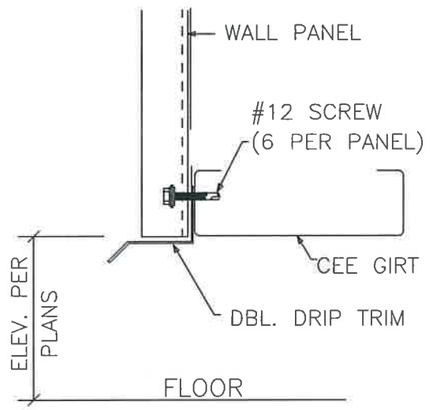
TRIM_11

COVER TRIM DETAIL AT JAMB



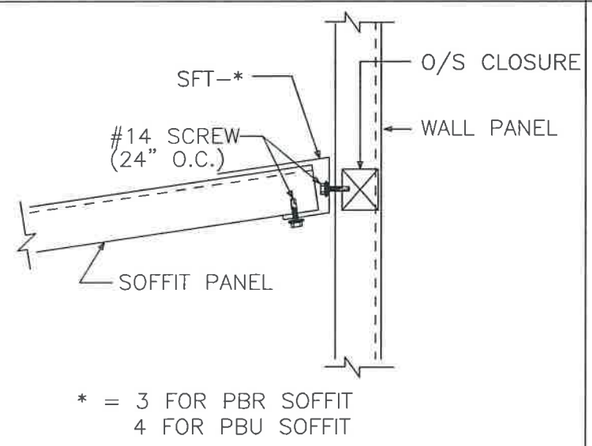
*= 6 PER PANEL FOR STANDARD PBR
3 PER PANEL FOR REV. ROLLED PBR

TRIM_16 BASE TRIM DETAIL



TRIM_22

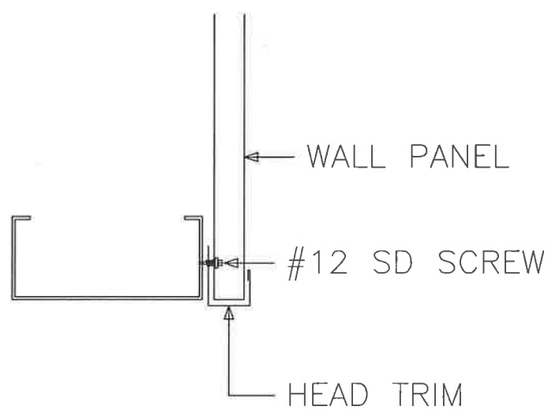
DOUBLE DRIP TRIM DETAIL
(PARTIAL WALL; CLEAR OPENING)



* = 3 FOR PBR SOFFIT
4 FOR PBU SOFFIT

TRIM_23

SOFFIT TRIM DETAIL AT
WALL PANEL

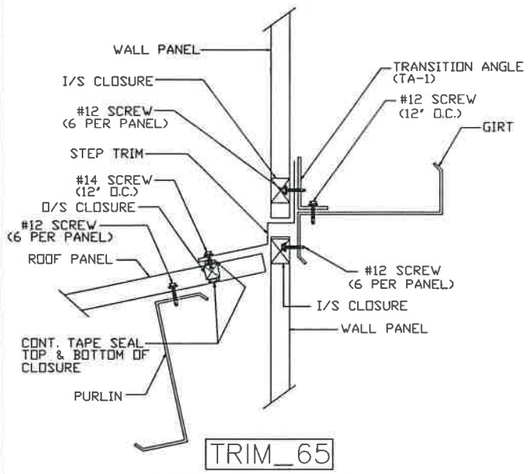


TRIM_61

HEAD TRIM DETAIL AT HEADER

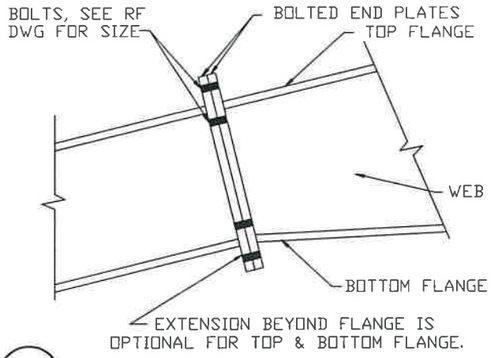
ISSUE	DET	CHK	DATE
BUILDINGS AND MORE			

CUSTOMER: PINNACLE SITE SOLUTIONS			
JOB NO: 9249R1			DATE: 10/13/25
LOCATION: HIGH SPRINGS, FL 32643			
DRAWING NAME: FRAMING DETAILS			
DRAWING NO: PAGE 5.4	DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE

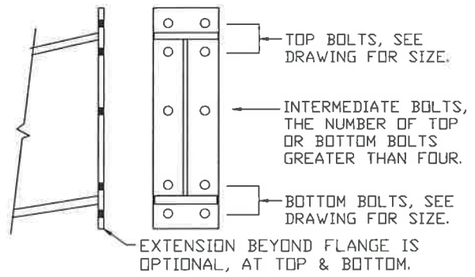


TRIM_65

STEP TRIM DETAIL

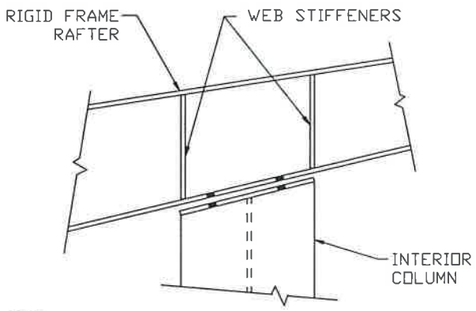


U1 BOLTED END PLATE RAFTER SPLICE

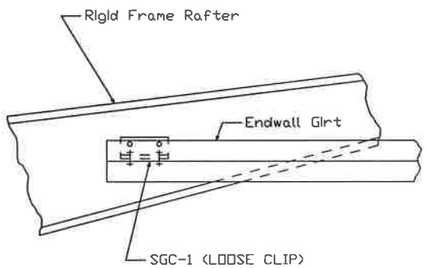


BOLTED END PLATE CONNECTION

NOTE: REFER TO RIGID FRAME CROSS SECTION(S) FOR BOLT SIZE/QNTY!

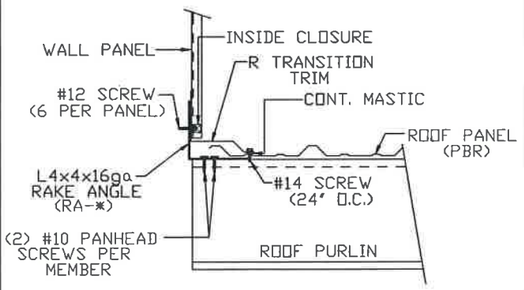


V2 INTERIOR COLUMN TO RAFTER

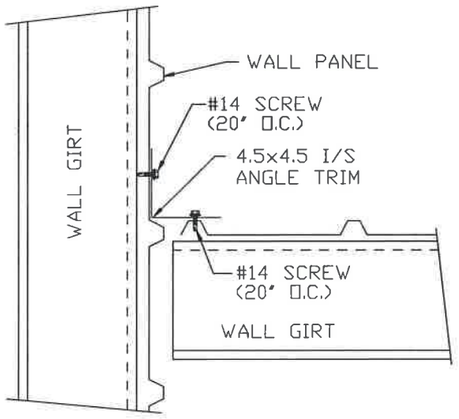


ATTACH WITH:
CLIP TO RAFTER (2) 1/2"x1.25" A307 BOLT/NUT
CLIP TO GIRT (2) 1/2"x1.25" A307 BOLT/NUT

W8 SECTION OF ENDWALL GIRT TO RAFTER



X1 SECTION THROUGH ROOF/WALL (ROOF & WALL= BOTH PBR PANELS) TRANSITION (COMMON WALL OPEN)



X2 I/S CORNER TRIM DETAIL

STRUCTURAL BOLTED CONNNECTIONS

REFER TO COVER PAGE "GENERAL NOTES" PARAGRAPH "C", SECTION "9" FOR INSTRUCTIONS ON TIGHTENING ALL A325 AND A490 CONNECTION BOLTS.

TRIM NOTES:

[1] SEAL TRIM SPLICES WITH TUBE CAULK.
[2] SECURE GUTTER SPLICES AND END PLUGS WITH RIVETS.
[3] SECURE ALL OTHER ROOF TRIM SPLICES WITH TRIM SCREWS UNLESS NOTED OTHERWISE.
[4] TRIM SCREWS ARE LOCATED 24" ON CENTER UNLESS NOTED OTHERWISE.
[5] STD. TRIM SPLICES ARE 3" TOTAL UNLESS NOTED OTHERWISE.

MORTISE PREPPED PERSONNEL DOORS

ALL MORTISE PREPPED PERSONNEL DOORS COME AS RIGHTHAND REVERSED SWING.

(i.e. STANDING ON THE OUTSIDE OF THE BUILDING FACING THE DOOR, THE LOCK WILL BE ON THE LEFTHAND SIDE OF THE DOOR AND THE DOOR WILL SWING OUTWARD FROM THE BUILDING.)

ANY FIELD MODIFICATIONS ARE THE RE-SPONSIBILITY OF THE ERECTOR AND MBM IS NOT LIABLE FOR LABOR CHARGES NOR DAMAGES DUE TO ERROR.

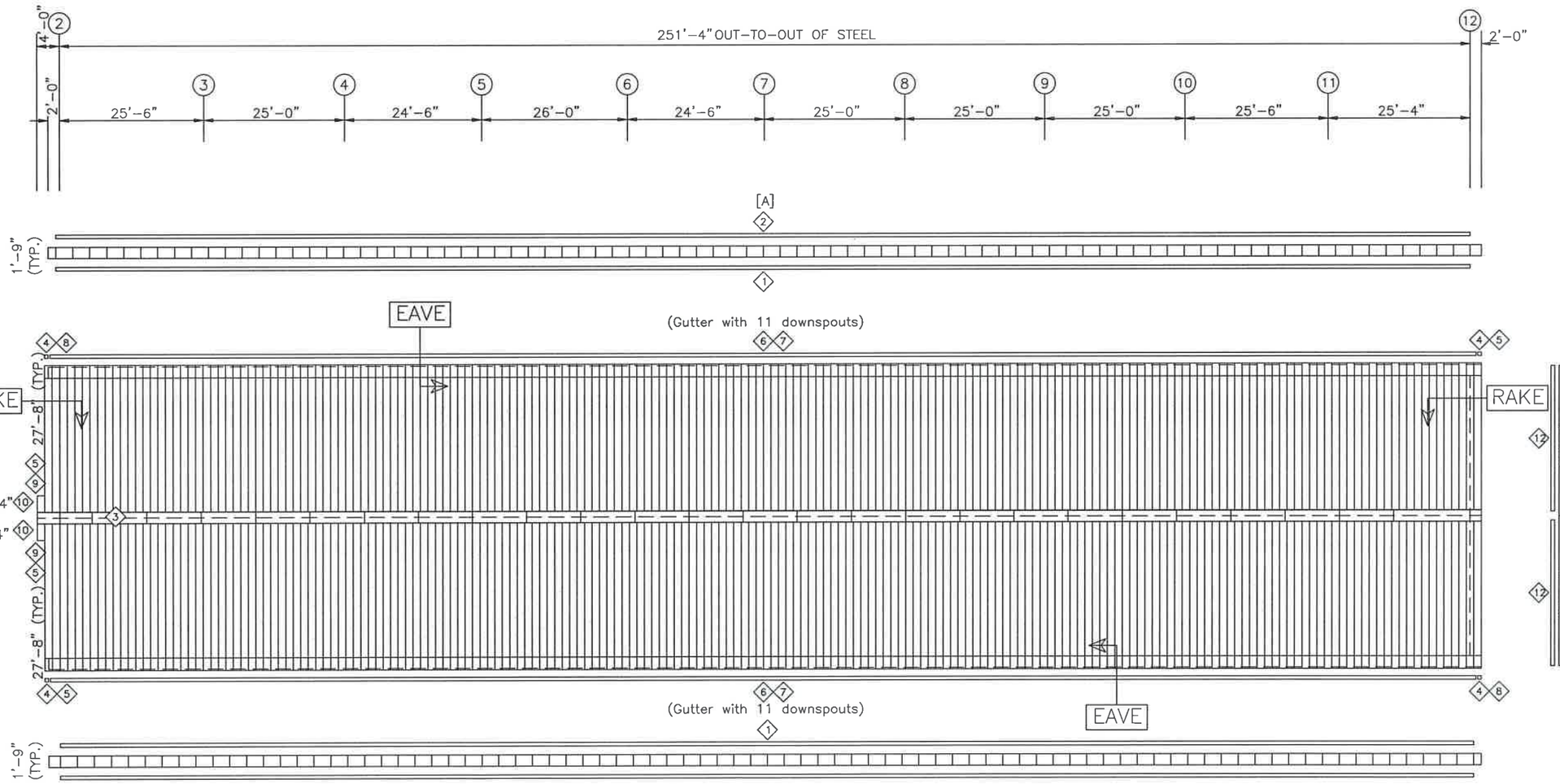
BUILT-UP MEMBER LEGEND

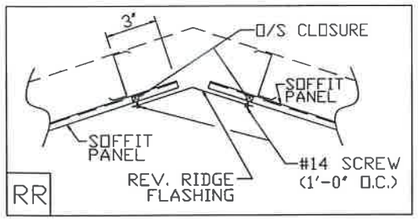
BEAM TYPE	BEAM DEPTH	FLANGE WIDTH	FLANGE THK.	WEB THK.
B	08	5	4	1
B= BUILT-UP	08= 8" 10= 10" 12= 12" 14= 14" ETC.	5,6,8,10 OR 12 (INCHES)	MEASURED IN 16ths. (4= 1/4", 5= 5/16" ETC.)	1= 10ga 3= 3/16" ETC.

ISSUE	DET	CHK	DATE
BUILDINGS AND MORE			

CUSTOMER: PINNACLE SITE SOLUTIONS
JOB NO: 9249R1 DATE: 10/13/25
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: FRAMING DETAILS
DRAWING NO: PAGE 5.5 DRAWN BY: DJH CHECKED BY: SPW SCALE: NONE

TRIM TABLE ROOF PLAN			
◇ID	PART	LENGTH	DETAIL
1	SFT-3	19'-7"	TRIM_23
2	SCT10	19'-11"	EAVE
3	RID FLSH	10'-3"	
4	GUTEND	1"	
5	CORBOX L	1'-0"	
6	EAVE TRM	19'-11"	EAVE
7	GUTTER	19'-11"	EAVE
8	CORBOX R	1'-0"	
9	EAVE TRM	2'-0"	EAVE
10	RAKE TRM	4'-7"	EAVE
11	RST10	14'-1"	RAKE
12	R HEAD	13'-2"	RAKE
13	REV RIDGE	4'-3"	RR





ROOF SHEETING PLAN

PANELS: 24 Ga. PC216 – NEED COLOR
[A] SOFFIT PANELS: 26 GA. PBR – NEED COLOR

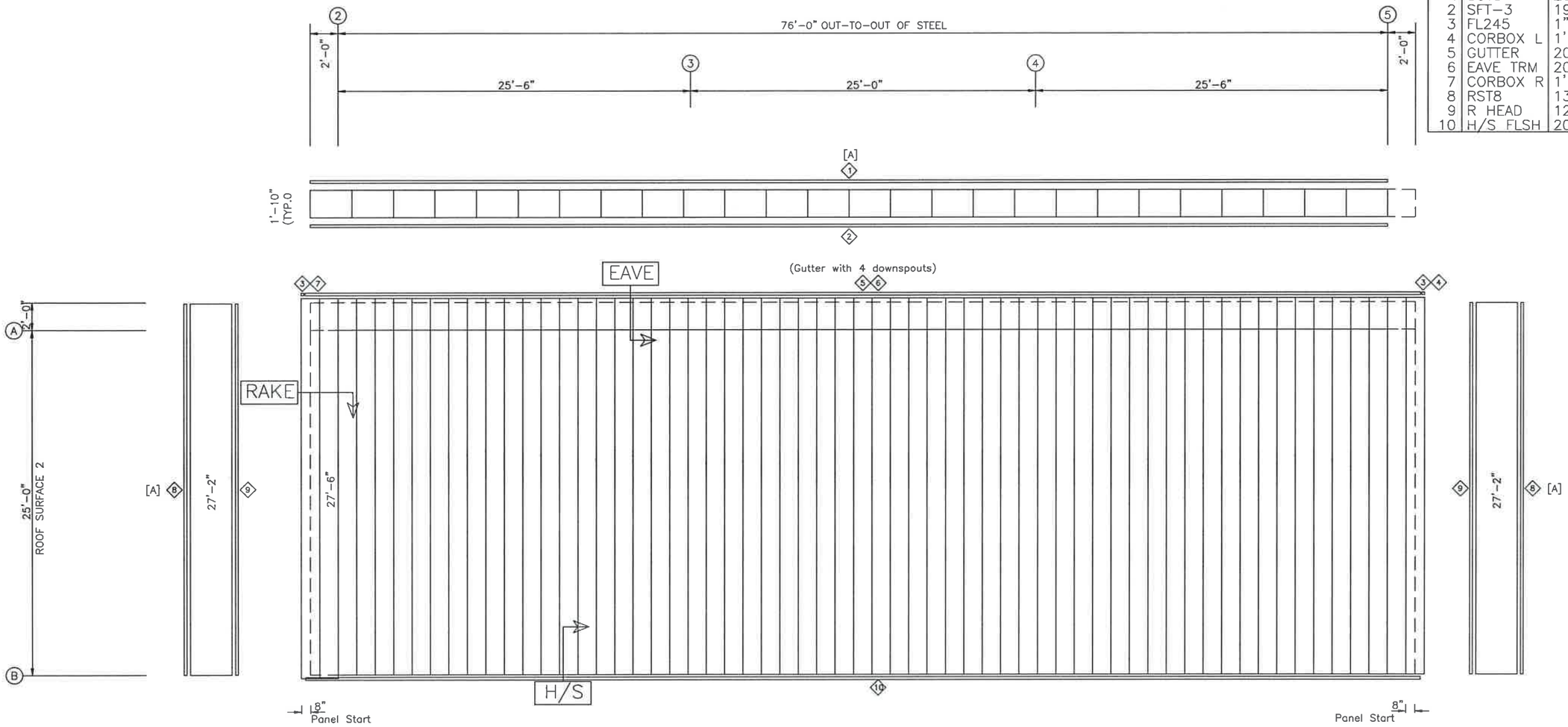
BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS
JOB NO: 9249R1
DATE: 10/13/25
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: ROOF PANELS & TRIM
DRAWING NO: PAGE 6A
DRAWN BY: DJH
CHECKED BY: GTL
SCALE: NONE

TRIM TABLE ROOF PLAN			
◇ID	PART	LENGTH	DETAIL
1	SCT8	20'-3"	EAVE2
2	SFT-3	19'-3"	TRIM_23
3	FL245	1"	
4	CORBOX L	1'-0"	
5	GUTTER	20'-3"	EAVE2
6	EAVE TRM	20'-3"	EAVE2
7	CORBOX R	1'-0"	
8	RST8	13'-9"	RAKE2
9	R HEAD	12'-11"	RAKE2
10	H/S FLSH	20'-3"	H/S



ROOF SHEETING PLAN

PANELS: 24 Ga. PC216 – NEED COLOR
[A] SOFFIT PANELS: 26 GA. PBR -- NEED COLOR

BUILDING "B"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNICAL SITE SOLUTIONS

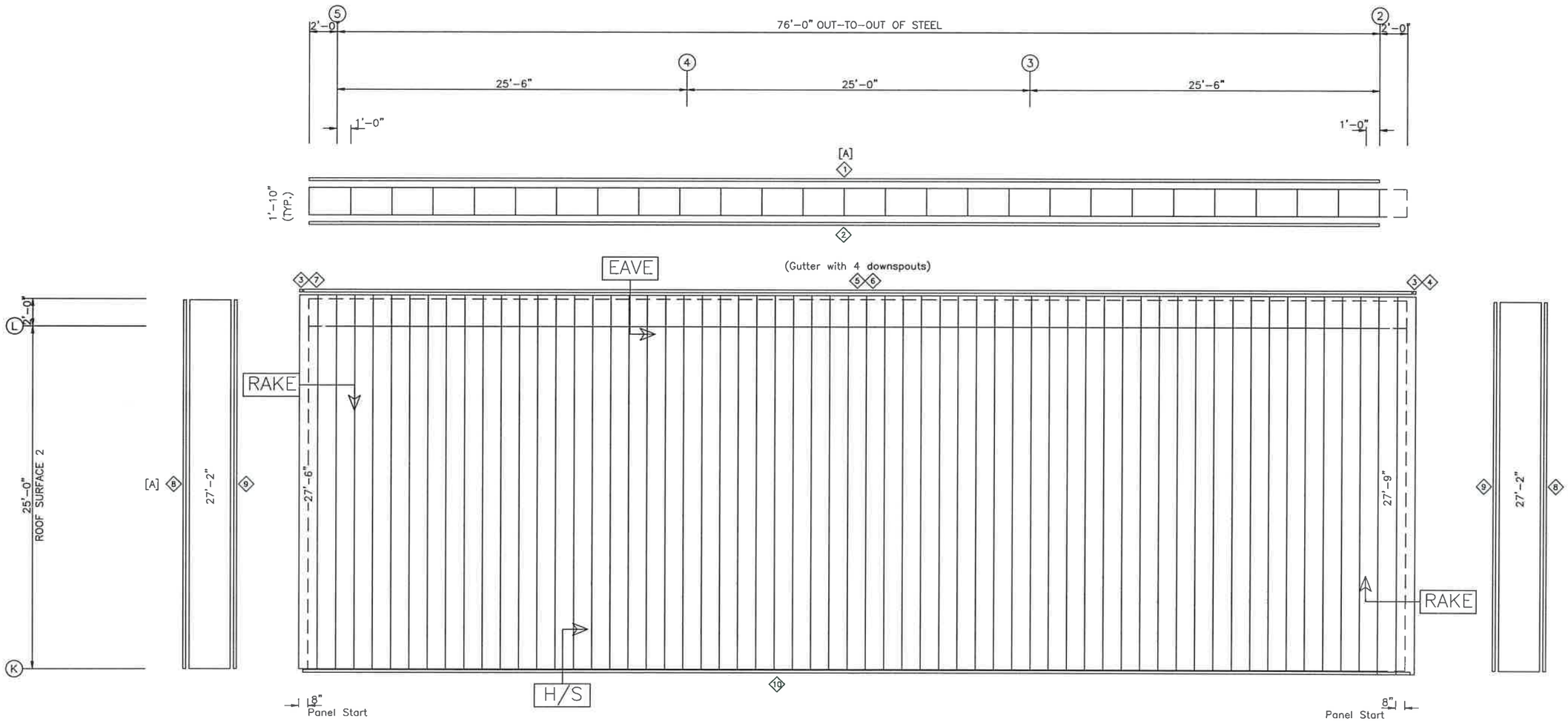
JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: ROOF PANELS & TRIM

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 6B	DJH	GTL	NONE

TRIM TABLE ROOF PLAN			
◇ID	PART	LENGTH	DETAIL
1	SCT8	20'-3"	EAVE2
2	SFT-3	19'-3"	TRIM_23
3	FL245	1"	
4	CORBOX L	1'-0"	
5	FL248A	20'-3"	EAVE2
6	FL272	20'-3"	EAVE2
7	CORBOX R	1'-0"	
8	RST8	13'-9"	RAKE2
9	R HEAD	12'-11"	RAKE2
10	H/S FLSH	19'-3"	H/S



ROOF SHEETING PLAN

PANELS: 24 Ga. PC216 – NEED COLOR
[A] SOFFIT PANELS: 26 GA. PBR – NEED COLOR

BULDING "C"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNICAL SITE SOLUTIONS

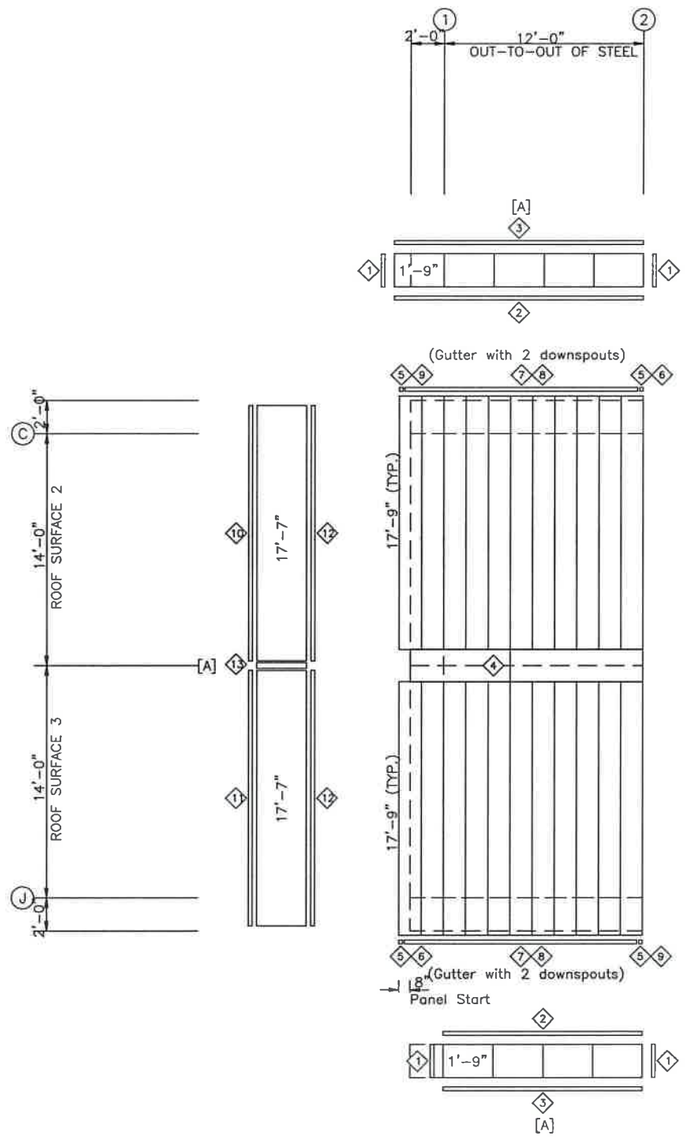
JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: ROOF PANELS & TRIM

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 6C	DJH	GTL	NONE

TRIM TABLE ROOF PLAN			
◇ID	PART	LENGTH	DETAIL
1	RST8	2'-0"	RAKE2
2	SFT-3	12'-3"	TRIM_23
3	SCT8	14'-0"	EAVE2
4	RIDG FLSH	7'-2"	
5	GUT END	1"	
6	CORBOX L	1'-0"	
7	GUTTER	14'-0"	EAVE2
8	EAVE TRM	14'-0"	EAVE2
9	CORBOX R	1'-0"	
10	RST8	15'-8"	RAKE2
11	RST8	17'-8"	RAKE2
12	R HEAD	15'-11"	
13	REV RIDGE	2'-3"	RR



ROOF SHEETING PLAN

PANELS: 24 Ga. PC216 – NEED COLOR
[A] SOFFIT PANELS: 26 GA. PBR – NEED COLOR

BUILDINGS "D"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNICAL SITE SOLUTIONS

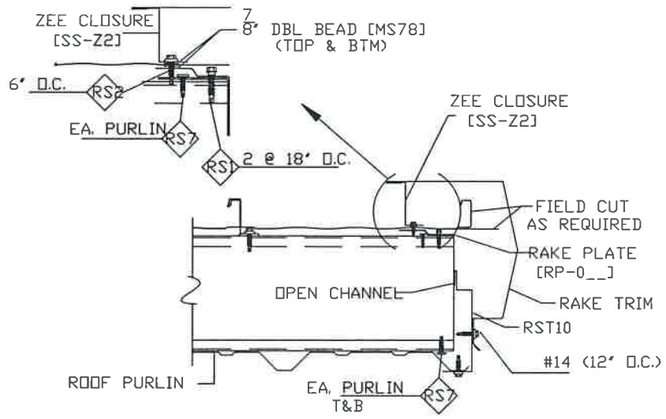
JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

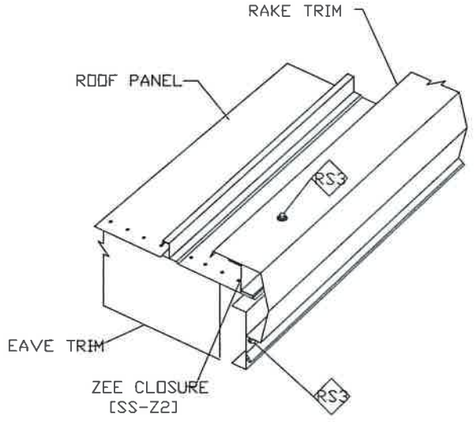
DRAWING NAME: ROOF PANELS & TRIM

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 6D	DJH	GTL	NONE

RAKE



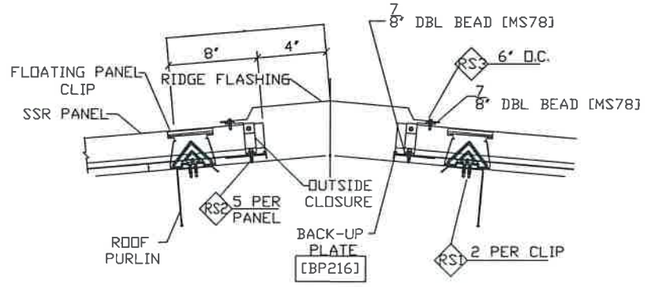
SECTION THRU RAKE
(WITHOUT WALL PANEL)



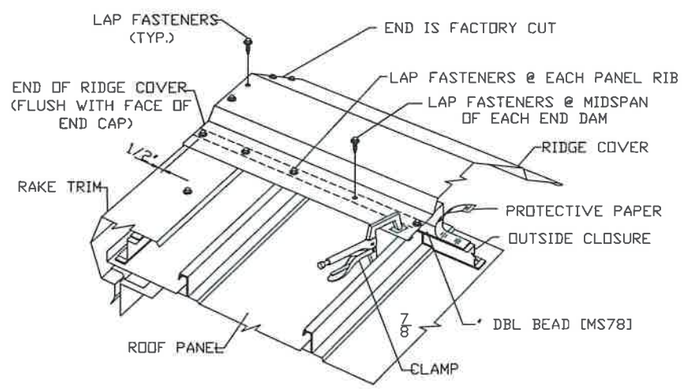
ISOMETRIC VIEW OF RAKE

PEAK

*= 3/8" OR 3/8" STANDOFF



SECTION THRU RIDGE
POSITION RIDGE COVER SO EXPOSED

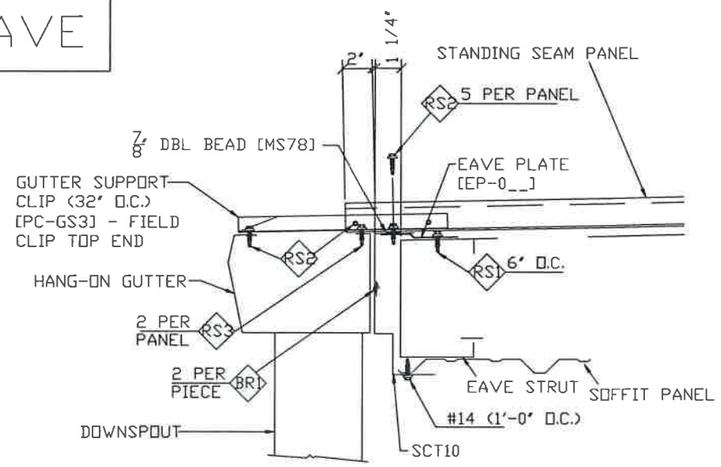


ISOMETRIC VIEW OF RIDGE

ROOF SCREW LEGEND

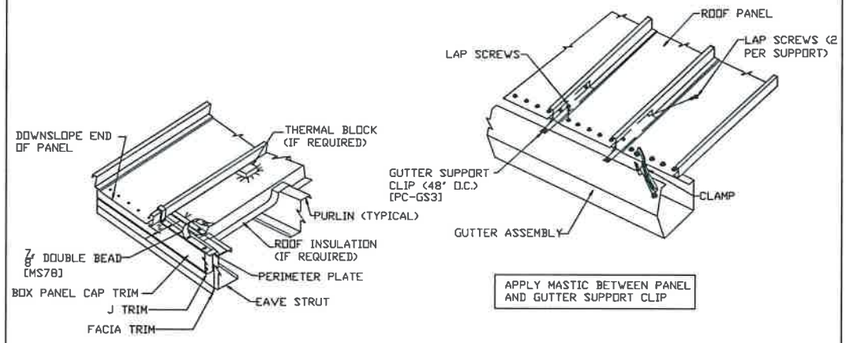
RS1	1/4"-14X1 1/4" DRILLER WOW	RS7	12x1" PANCAKE
RS2	1/4"-14x1 1/4" L.L. DRILLER	RS8	#17x1" SELF TAPPER
RS3	1/4"-14x7/8" L.L. LAPTEK	BR1	1/8"x3/16" RIVET
RS4	1/4"-14x1 1/4" SHOULDER TEK2	WS1	12-14x1 1/4" L.L. DRILLER
RS5	12-24x1 1/4" TEK4.5 DRILLER	WS2	1/4"-14x7/8" L.L. LAPTEK
RS6	1/4"-20x1 1/4" SHOULDER TEK 4		

EAVE



USE POP RIVETS AS REQ'D TO SUPPORT FLASHING UNTIL SCREWS CAN BE INSTALLED

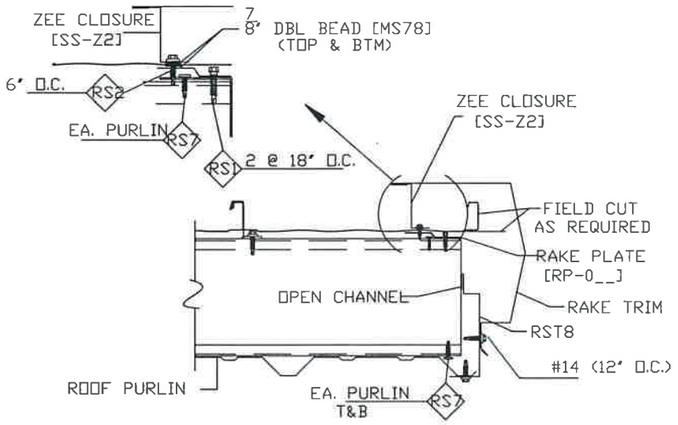
SECTION THRU EAVE WITH NORTHERN GUTTER
(WITHOUT WALL PANEL)



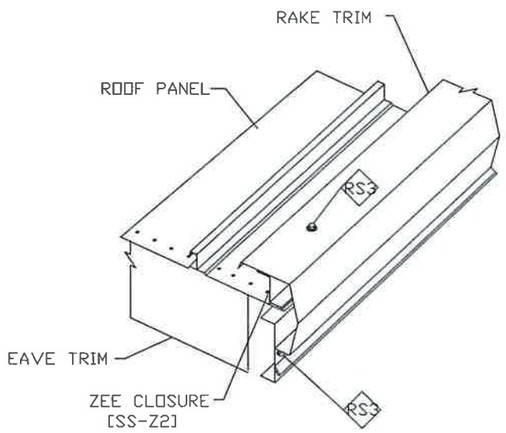
ISOMETRIC VIEW OF EAVE

BUILDING "A"			
ISSUE	DET	CHK	DATE
BUILDINGS AND MORE			
CUSTOMER: PINNACLE SITE SOLUTIONS			
JOB NO: 9249R1	DATE: 10/13/25		
LOCATION: HIGH SPRINGS, FL 32643			
DRAWING NAME: ROOF PANELS & TRIM			
DRAWING NO: PAGE 6.1	DRAWN BY: DJH	CHECKED BY: GTL	SCALE: NONE

RAKE2

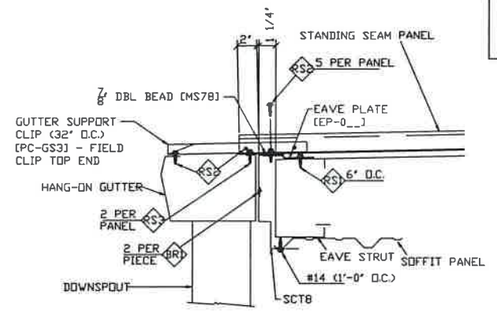


SECTION THRU RAKE
(WITHOUT WALL PANEL)



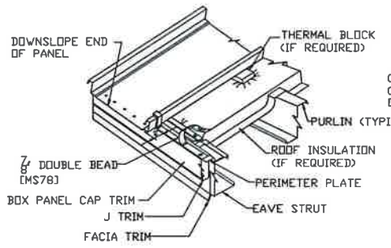
ISOMETRIC VIEW OF RAKE

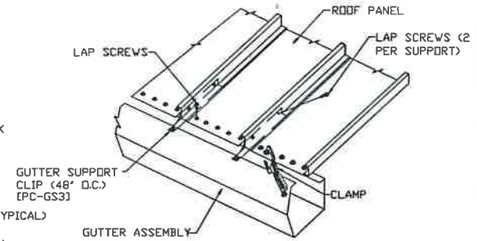
EAVE2



USE POP RIVETS AS REQ'D TO SUPPORT FLASHING UNTIL SCREWS CAN BE INSTALLED

SECTION THRU EAVE WITH NORTHERN GUTTER
(WITHOUT WALL PANEL)





APPLY MASTIC BETWEEN PANEL AND GUTTER SUPPORT CLIP

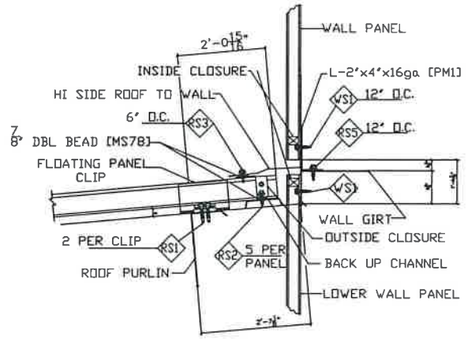
ISOMETRIC VIEW OF EAVE

ROOF SCREW LEGEND

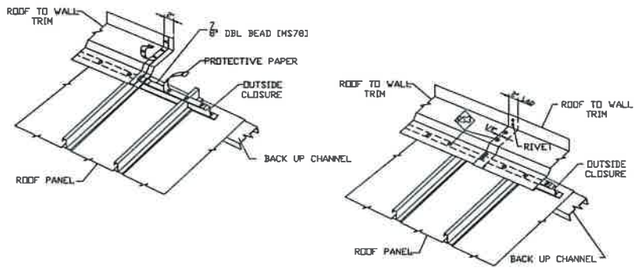
RS1	1/4"-14X1 1/4" DRILLER WOW	RS7	12x1" PANCAKE
RS2	1/4"-14x1 1/4" L.L. DRILLER	RS8	#17x1" SELF TAPPER
RS3	1/4"-14x7/8" L.L. LAPTEK	BR1	1/8"x3/16" RIVET
RS4	1/4"-14x1 1/4" SHOULDER TEK2	WS1	12-14x1 1/4" L.L. DRILLER
RS5	12-24x1 1/2" TEK4.5 DRILLER	WS2	1/4"-14x7/8" L.L. LAPTEK
RS6	1/4"-20x1 1/4" SHOULDER TEK 4		

H/S

*= 3/8" OR 3/16" STANDOFF



SECTION THRU HIGH EAVE ROOF TO WALL
(WITH WALL PANEL CONTINUATION)



ISOMETRIC VIEW OF HI EAVE ROOF TO WALL

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

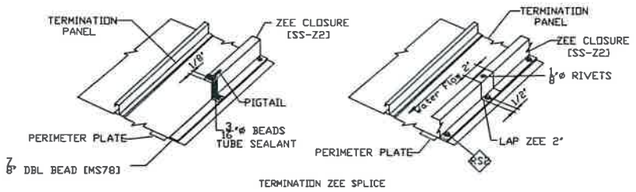
CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

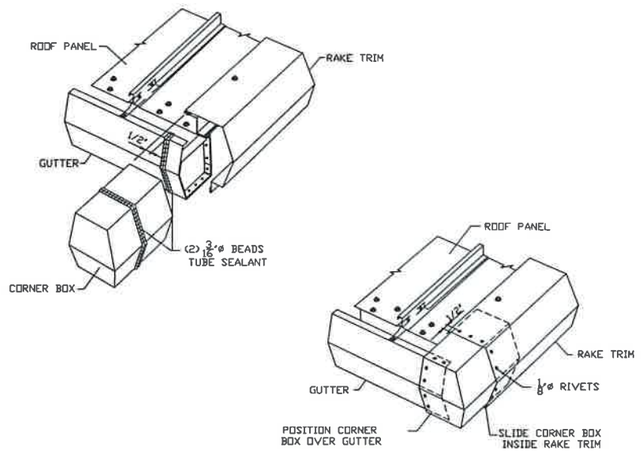
LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: ROOF PANELS & TRIM

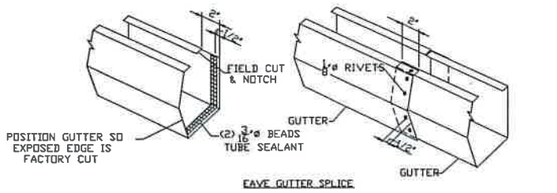
DRAWING NO	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 6.2	DJH	GTL	NONE



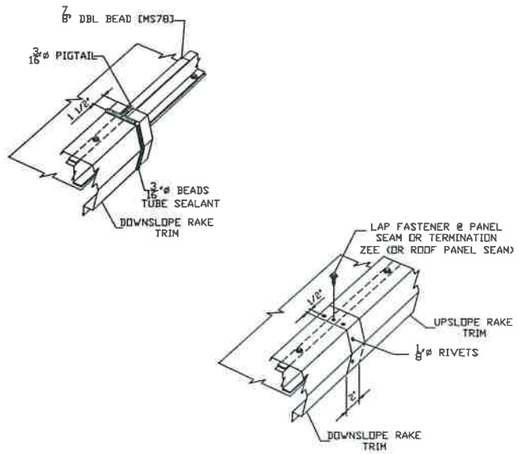
ISOMETRIC VIEW OF ZEE CLOSURE SPLICE AT PANEL



ISOMETRIC VIEW OF CORNER BOX INSTALLATION



ISOMETRIC VIEW OF GUTTER SPLICE



ISOMETRIC VIEW OF RAKE SPLICE

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

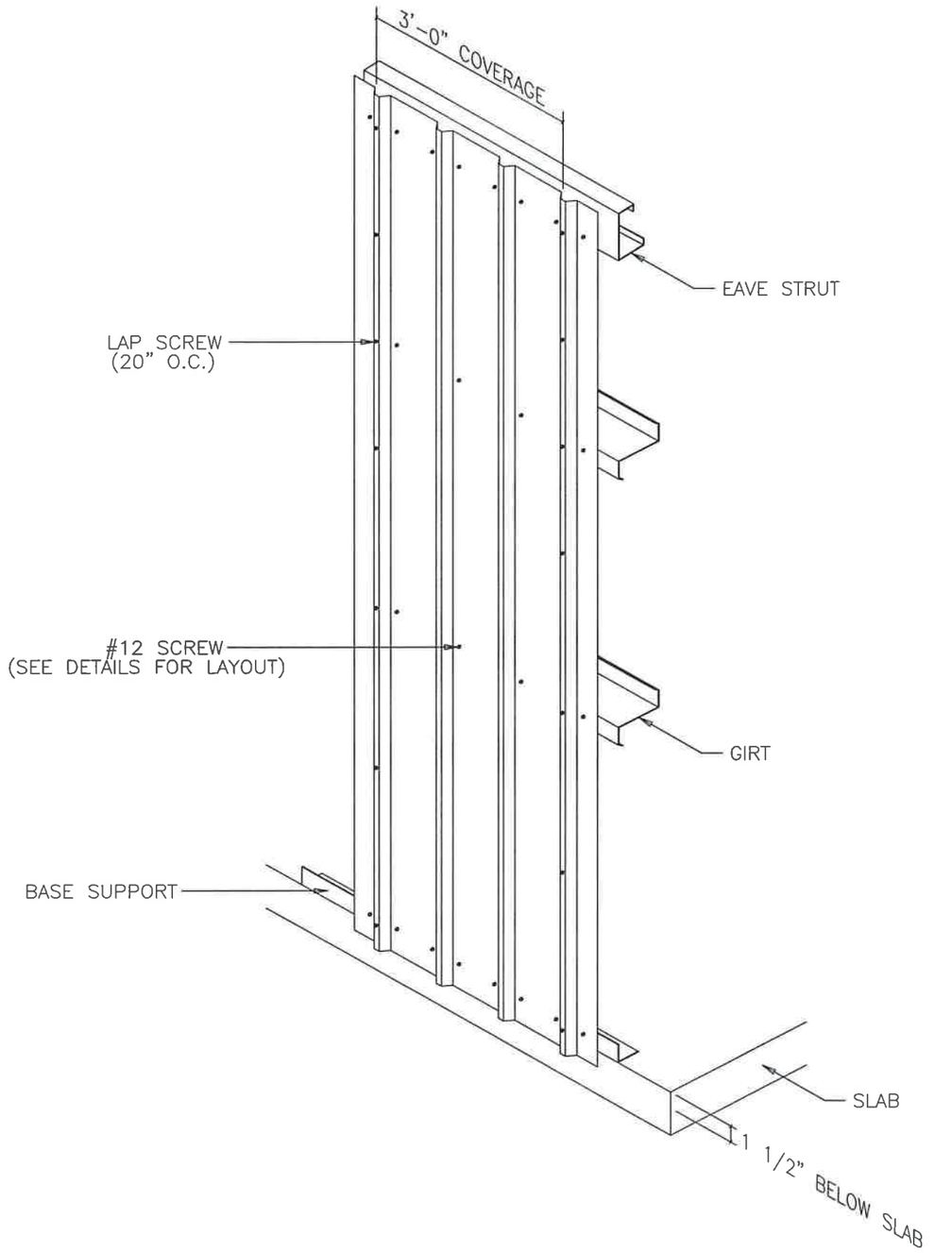
CUSTOMER: PINNACLE SITE SOLUTIONS

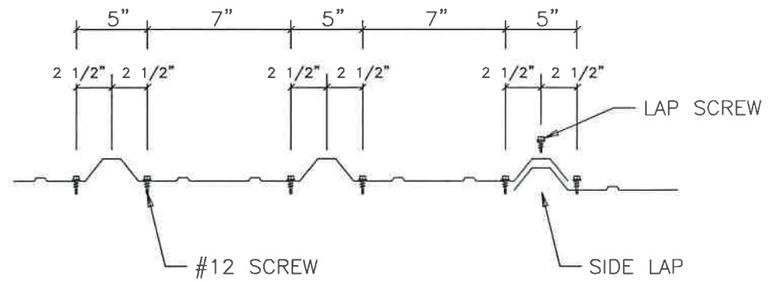
JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

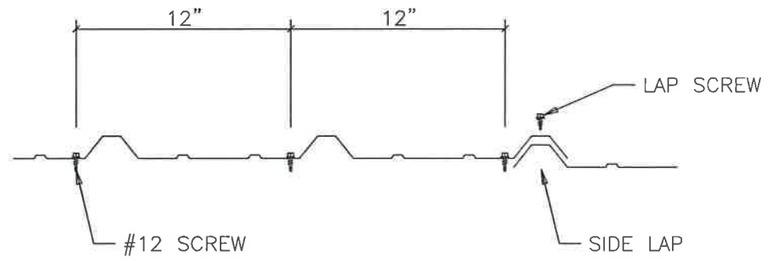
DRAWING NAME: ROOF PANELS & TRIM

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 6.3	DJH	GTL	NONE

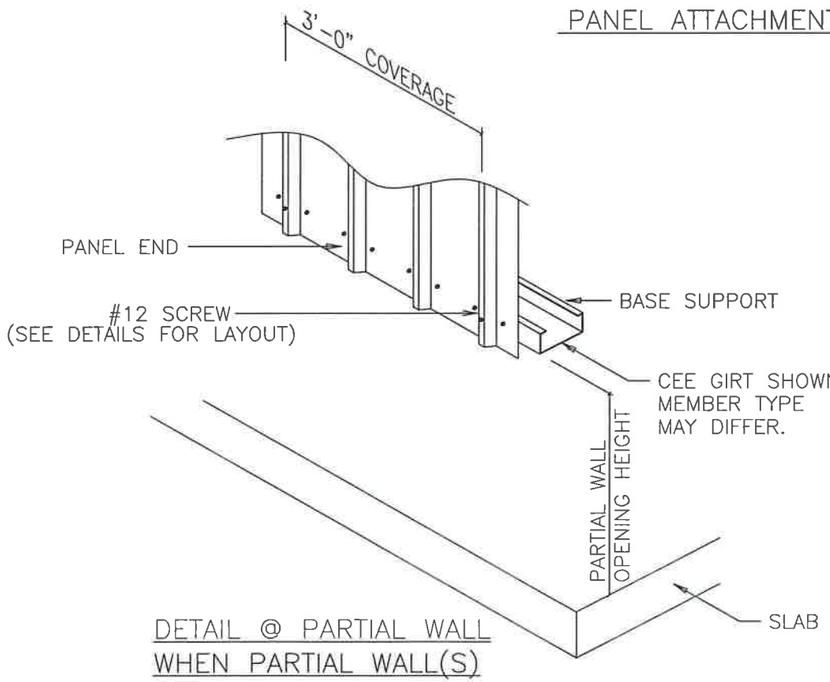




PANEL ATTACHMENT AT PANEL END
(BASE, EAVE STRUT, HEADER, SILL, AND PANEL END LAPS)



PANEL ATTACHMENT AT INTERMEDIATE MEMBERS



DETAIL @ PARTIAL WALL
WHEN PARTIAL WALL(S)
ARE PRESENT

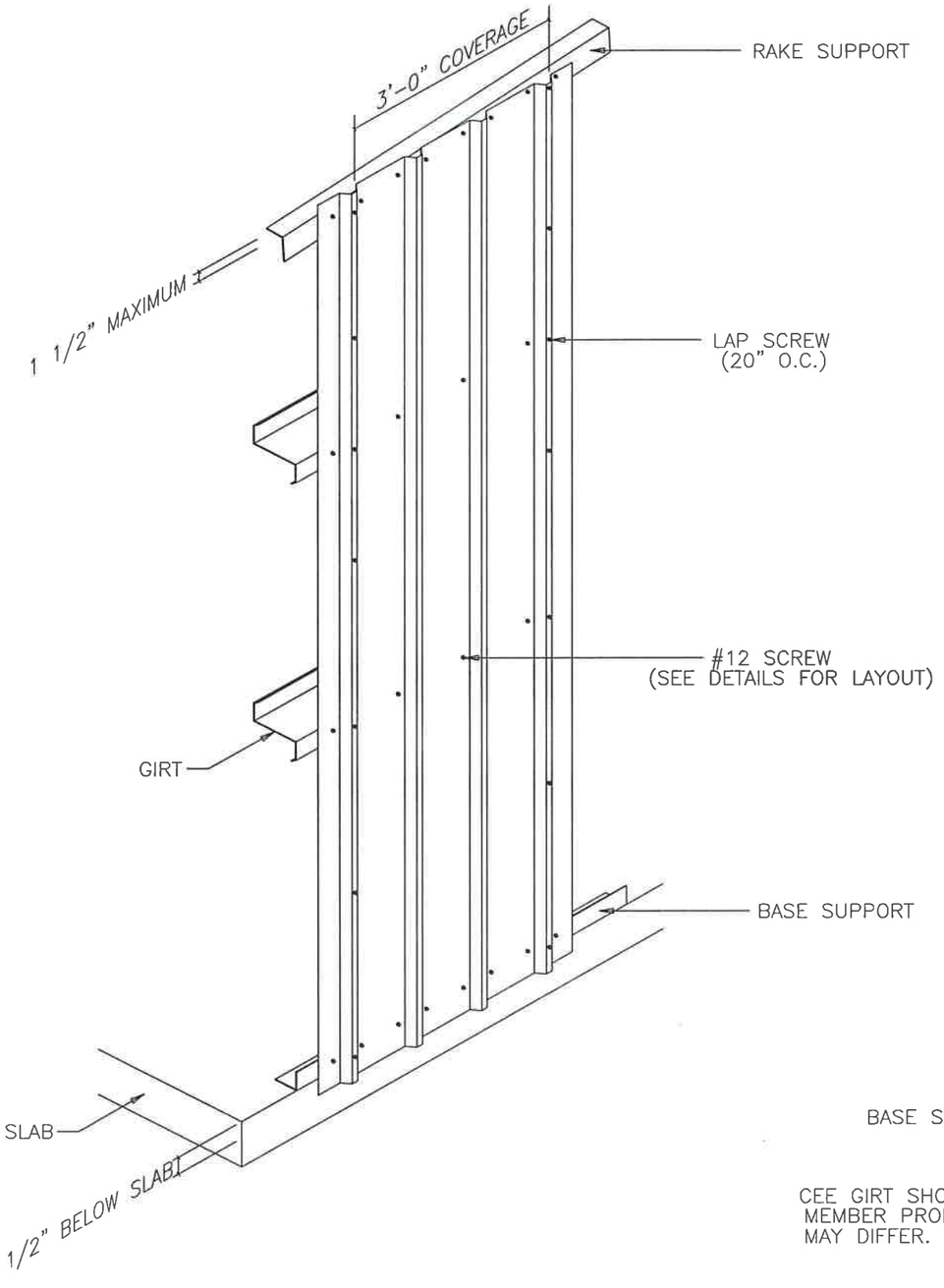
NOTES:

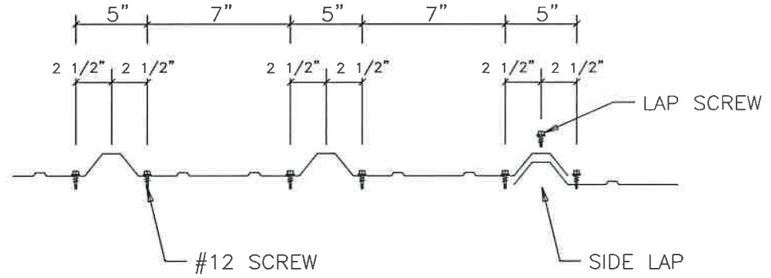
[1] METAL SHAVINGS MUST BE SWEPT FROM THE WALL EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.

[2] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE GIRTS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.

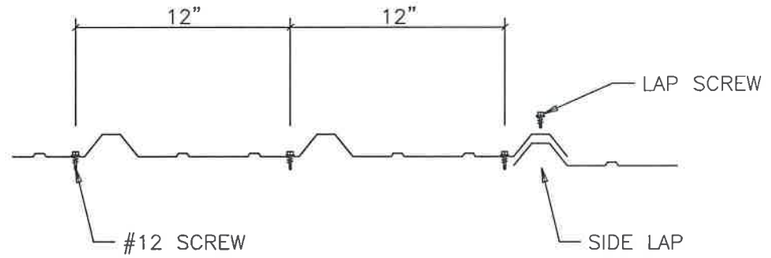
ISSUE	DET	CHK	DATE
BUILDINGS AND MORE			

CUSTOMER: PINNACLE SITE SOLUTIONS			
JOB NO: 9249R1		DATE: 10/13/25	
LOCATION: HIGH SPRINGS, FL 32643			
DRAWING NAME: SIDEWALL PANEL DETAILS			
DRAWING NO: PAGE 7	DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE

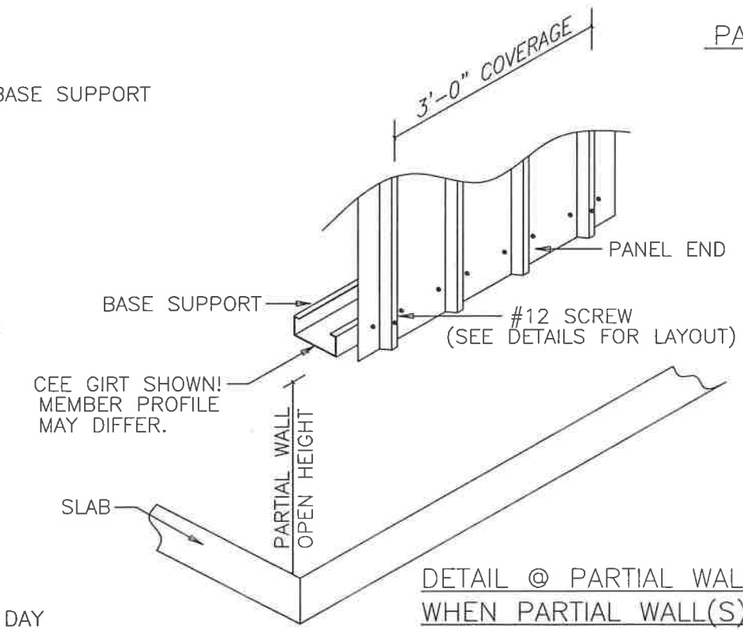




PANEL ATTACHMENT AT PANEL END
(BASE, EAVE STRUT, HEADER, SILL, AND PANEL END LAPS)



PANEL ATTACHMENT AT INTERMEDIATE MEMBERS



DETAIL @ PARTIAL WALL
WHEN PARTIAL WALL(S)
ARE PRESENT

NOTES:

[1] METAL SHAVINGS MUST BE SWEPT FROM THE WALL EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.

[2] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE GIRTS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

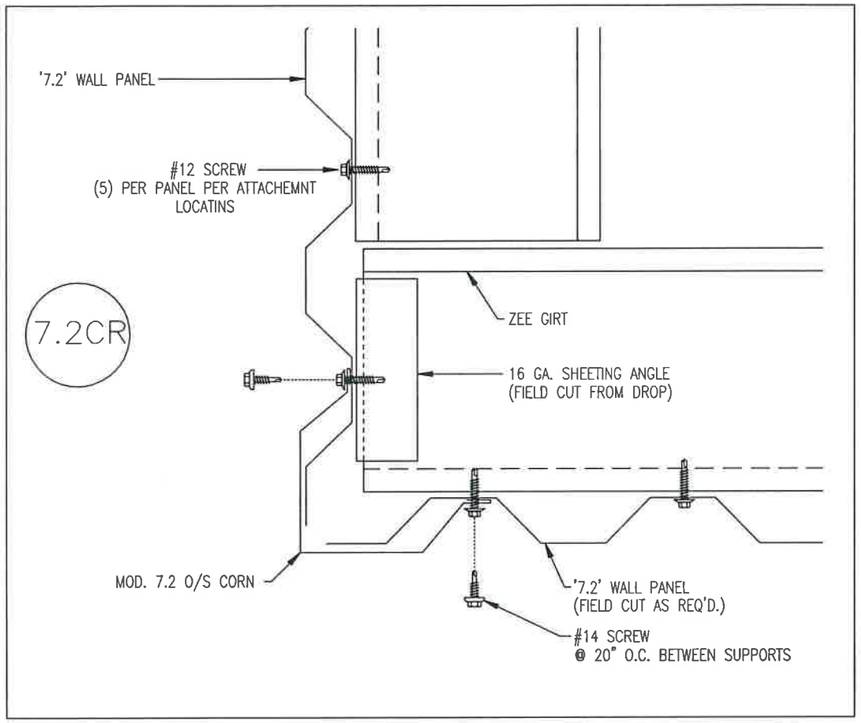
CUSTOMER: PINNACLE SITE SOLUTIONS

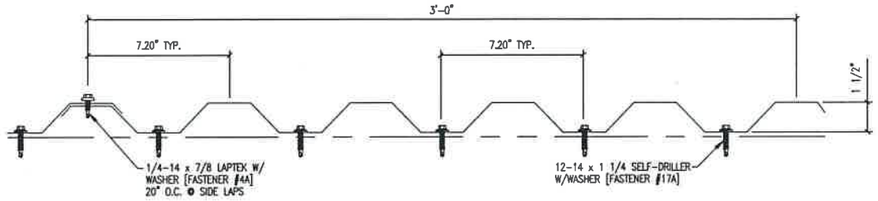
JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

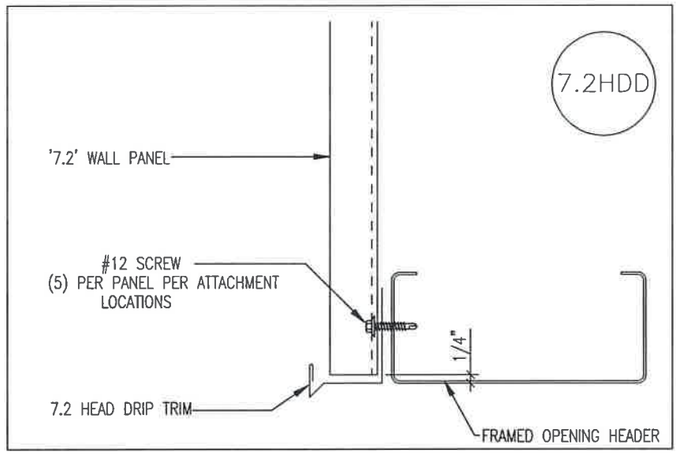
DRAWING NAME: ENDWALL PANEL DETAILS

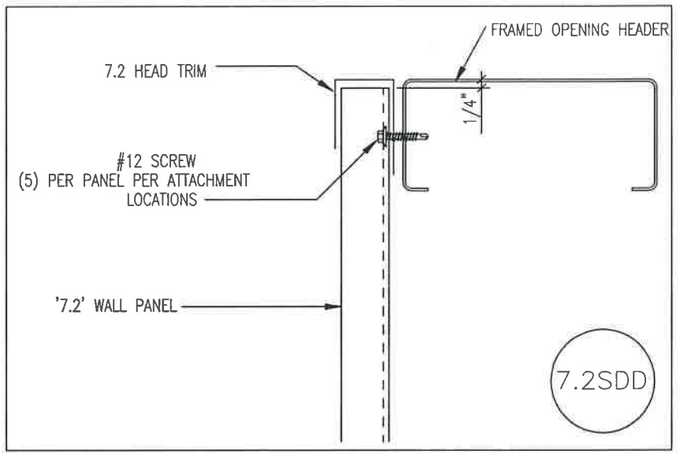
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 8	DJH	SPW	NONE

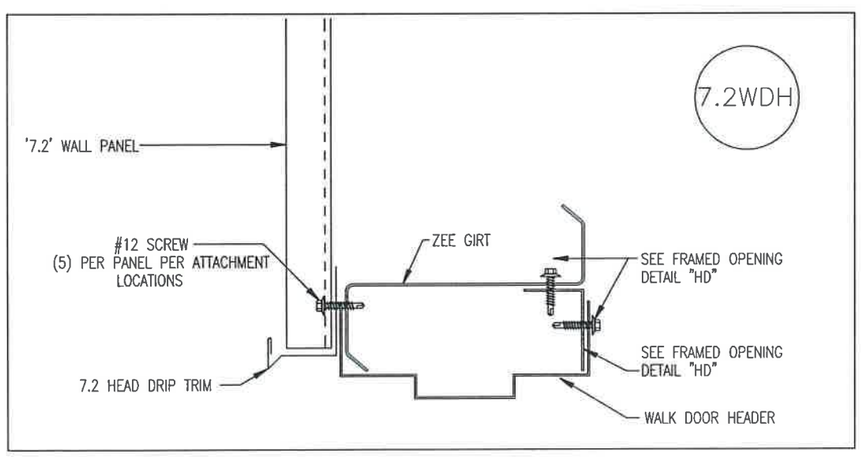


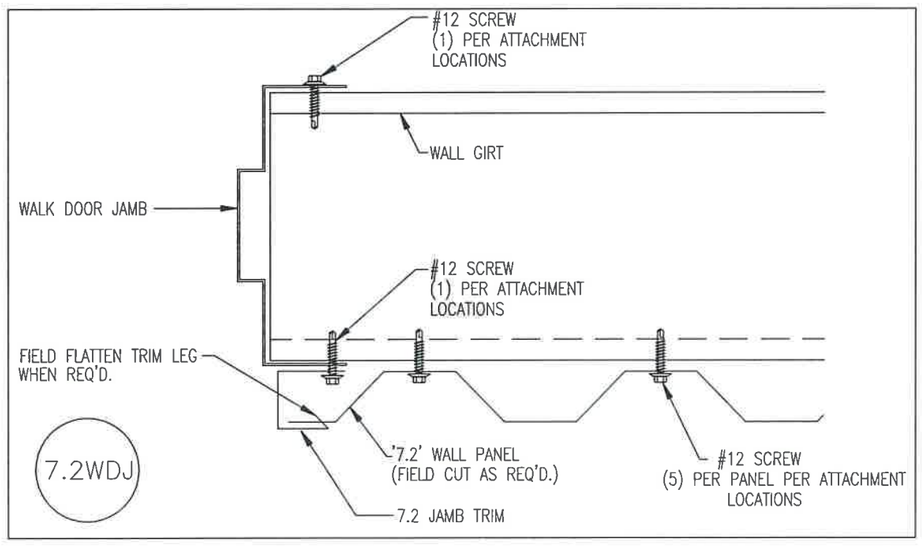


7.2 FASTENER SPACING @ PANEL ENDS & INTERMEDIATE SUPPORTS









ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

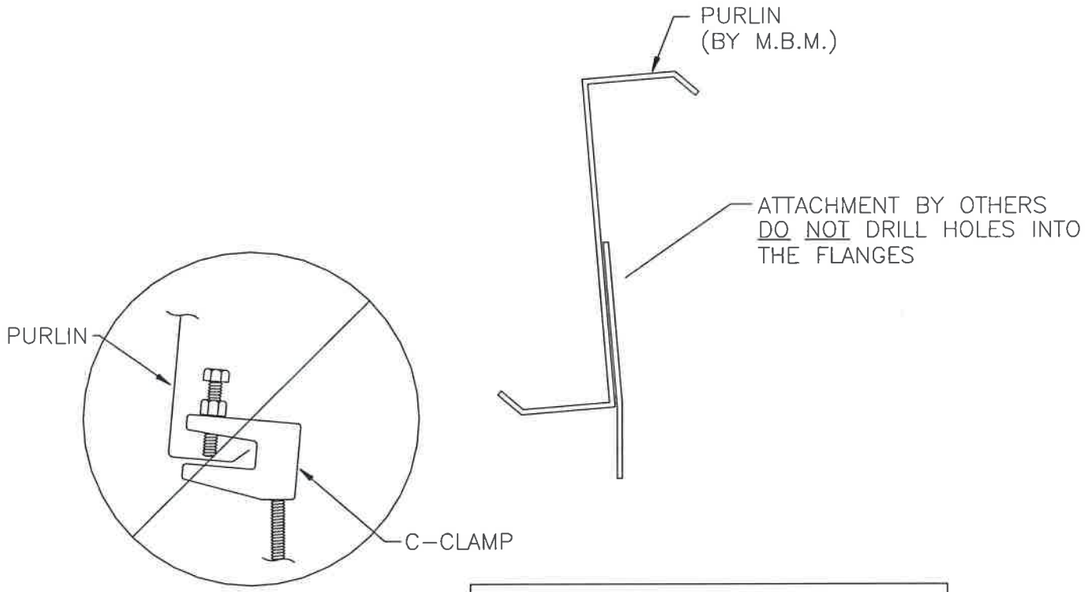
CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: ENDWALL PANEL DETAILS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 8.1	DJH	GTL	NONE



Flange C–Clamp is not an acceptable connection

NOTE: M.B.M. only provides the roof purlin. All other material and hardware is by others.

Recommended Connection Detail

NOTE

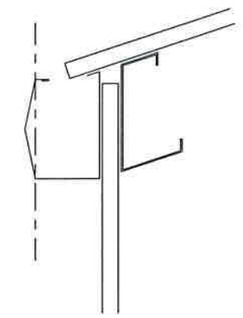
MANY FACTORS BEYOND THE CONTROL OF THE METAL BUILDING SUPPLIER AFFECT THE ABILITY OF A PURLIN TO SAFELY SUPPORT HANGING LOADS COMBINED WITH OTHER REQUIRED ROOF LOADS. DUE TO THE VARIABLES INVOLVED IN HANGING LOADS AND THEIR ATTACHMENTS TO THE PURLINS, THE METAL BUILDING SUPPLIER CANNOT ASSURE THAT THE PURLINS FOR A PARTICULAR BUILDING PROJECT CAN SAFELY SUPPORT THE MAXIMUM ALLOWABLE HANGING LOADS IN COMBINATION WITH OTHER ROOF LOADS.

IT IS THE RESPONSIBILITY OF THE HANGER SYSTEM INSTALLER TO COORDINATE WITH THE ENGINEER OF RECORD FOR THE OVERALL PROJECT TO ENSURE A SAFE HANGING LOAD INSTALLATION. THE METAL BUILDING ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT. WITHOUT SPECIFIC CERTIFICATION FOR INDIVIDUAL HANGING LOADS, THE NET EFFECTS OF APPLIED HANGER LOADS INSTALLED ON A PARTICULAR PURLIN SHALL NOT EXCEED THE NET EFFECTS OF THE CERTIFIED UNIFORMLY APPLIED DESIGN COLLATERAL LOAD.

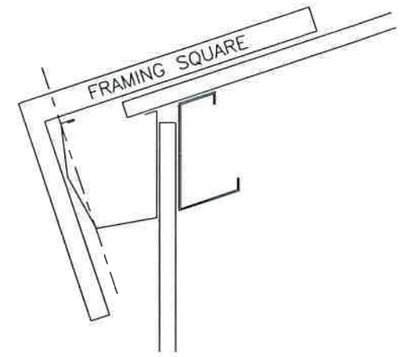
HANGING LOADS SHOULD NOT BE APPLIED TO THE PURLIN LIP. WHERE PERMISSIBLE, THE BEST PRACTICE FOR HANGING LOADS IS TO ATTACH TO THE PURLIN WEB USING A BOLT AND NUT, OR SELF–DRILLING SCREWS.

HANGING UNIFORM LOADS SUCH AS SPRINKLER MAINS OR HVAC EQUIPMENT SHOULD BE DISTRIBUTED OVER SEVERAL PURLINS, AND SHOULD NEVER EXCEED THE COLLATERAL LOAD ALLOWANCE FOR THE ROOF SYSTEM. FOR UNIFORM LOADS THAT RUN PARALLEL TO THE PURLINS, IT MAY BE NECESSARY TO USE TRANSVERSE SUPPORT CHANNELS(A.KA. TRAPEZE BEAMS) ATTACHED TO THE WEBS OR FLANGES OF ADJACENT PURLINS TO SPREAD THE LOAD BETWEEN TWO OR MORE PURLINS. IN SUCH CASES, CONTACT THE BUILDING MANUFACTURER OR A LOCAL PROFESSIONAL ENGINEER PRIOR TO ATTEMPTING TO HANG LOADS FROM THE PURLINS

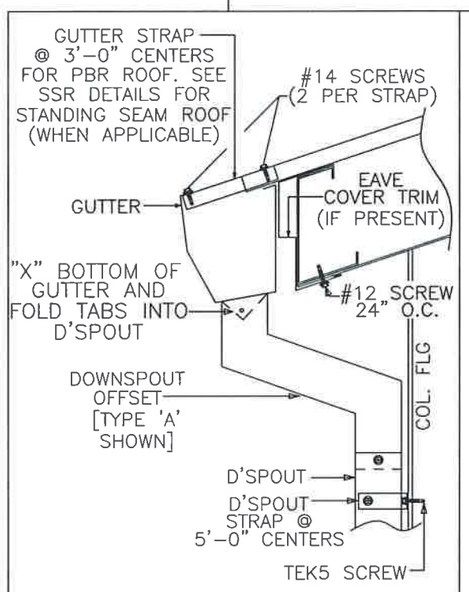
GUTTER INSTALLATION DETAIL (ONLY IF PROVIDED)



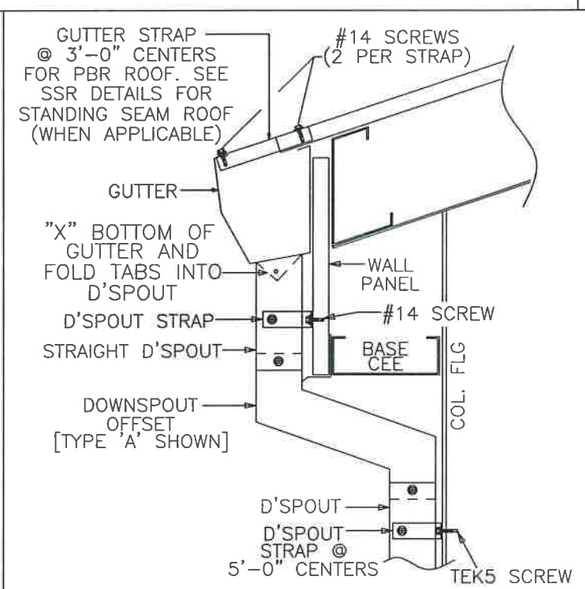
DO NOT INSTALL GUTTER WITH OUTSIDE FACE PERPENDICULAR TO THE GROUND.



INSTALL GUTTER WITH OUTSIDE FACE PERPENDICULAR TO THE ROOF.



@ OPEN WALLS W/O WALL PANELS



@ PARTIAL WALLS W/ WALL PANELS

NOTE: REGARDLESS OF DOWNSPOUT OFFSET SCENARIO, TEK5 SCREWS MUST BE USED TO ATTACH DOWNSPOUT STRAPS TO PEMB FRAMING. WHEN WALL PANELS SPAN FROM GROUND TO EAVE (FULL SPAN), #14 SCREWS WILL BE USED TO ATTACH DOWNSPOUT STRAPS TO WALL PANELS.

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

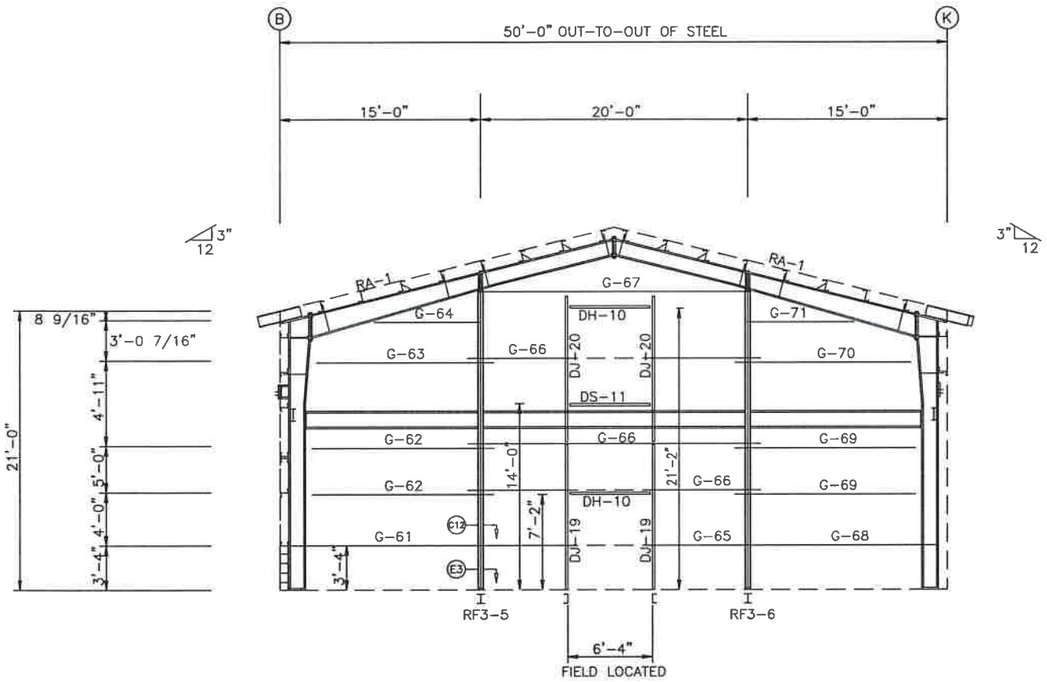
CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

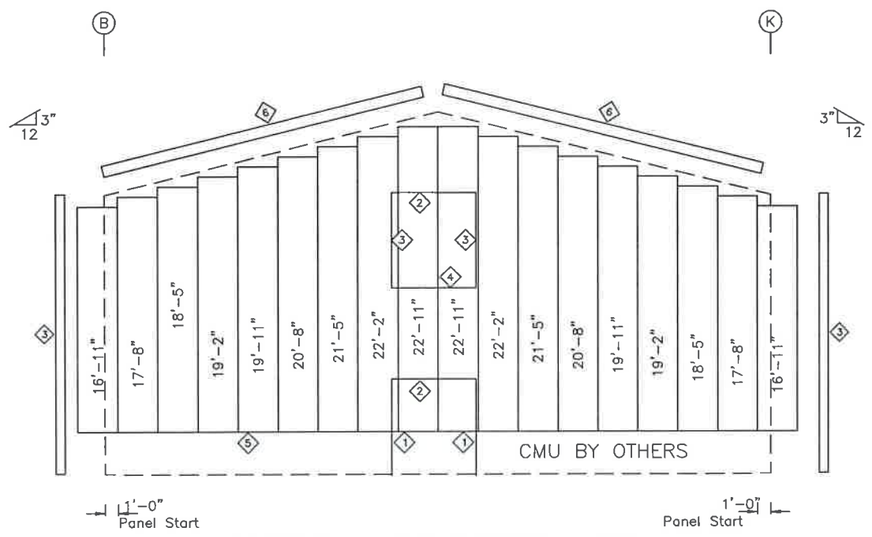
DRAWING NAME: SPECIAL DETAILS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 9	DJH	SPW	NONE

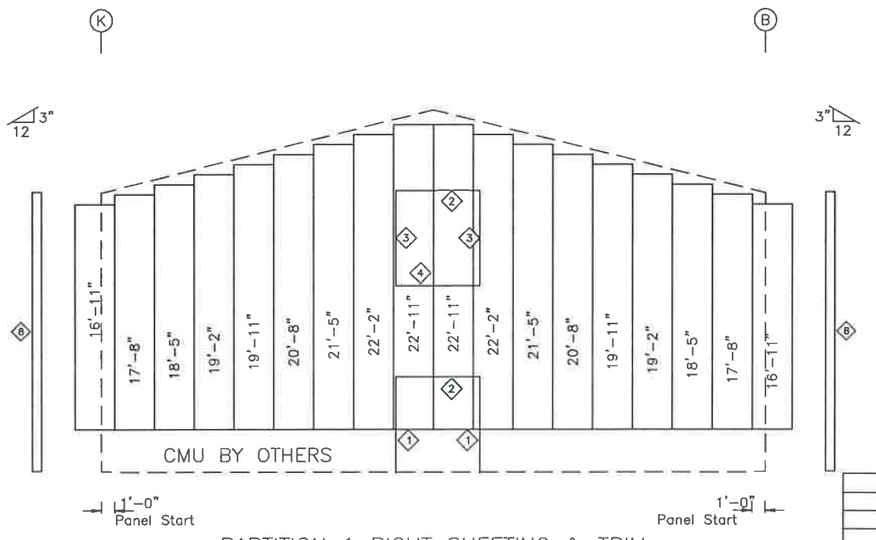


GIRT LAPS 11 3/4" / 11 3/4" 11 3/4" / 11 3/4"

PARTITION 1 FRAMING

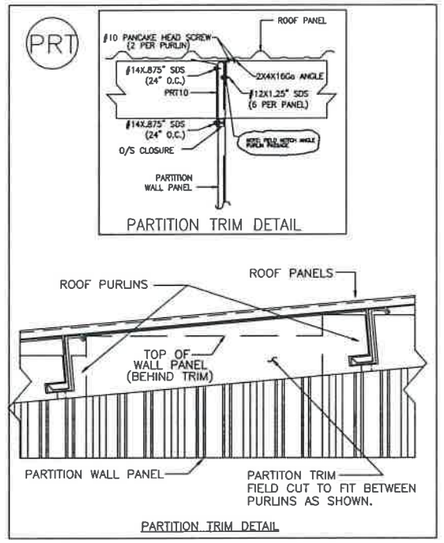


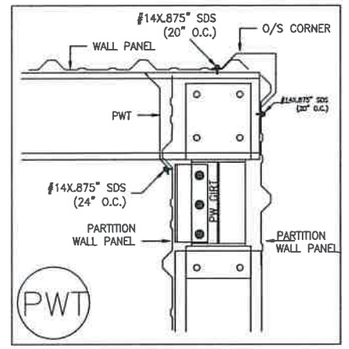
PARTITION 1 LEFT SHEETING & TRIM
PANELS: 26 Ga. R – NEED COLOR



PARTITION 1 RIGHT SHEETING & TRIM
PANELS: 26 Ga. R – NEED COLOR

SEE PAGE 12 FOR MEZZANINE INFORMATION





TRIM TABLE
PARTITION 1

◇ID	PART	LENGTH	DETAIL
1	R JAMB	7'-5"	TRIM_8
2	R HEAD	6'-7"	TRIM_61
3	R JAMB	7'-5"	TRIM_8
4	R HEAD	6'-7"	TRIM_7
5	DBLBASTR	16'-11"	TRIM_22
6	PRT10	4'-7"	PRT
7	PWT	4'-7"	PWT
8	O/S CORNER	10'-8"	PRT

MEMBER TABLE
PARTITION 1

MARK	PART	LENGTH
DJ-19	8X25C16	12'-1 13/16"
DJ-20	8X25C16	9'-11"
DH-10	8X25C16	6'-4"
DS-11	8X25C16	6'-4"
G-61	8x25C16	13'-2 3/4"
G-62	8x25Z16	14'-2 3/4"
G-63	8x25Z16	13'-10 3/4"
G-64	8x25Z16	8'-7 1/2"
G-65	8x25C16	19'-11 1/2"
G-66	8x25Z16	21'-11 1/2"
G-67	8x25Z14	20'-7 1/2"
G-68	8x25C16	13'-2 5/8"
G-69	8x25Z16	14'-2 5/8"
G-70	8x25Z16	13'-10 5/8"
G-71	8x25Z16	8'-7 1/2"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

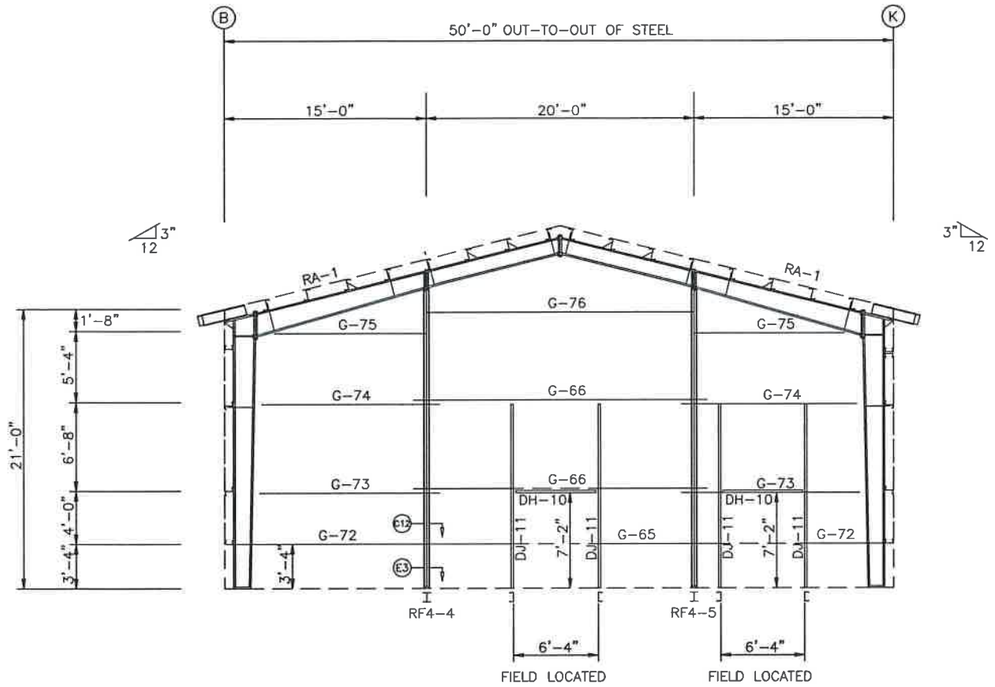
CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249R1-A | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: PARTITION FRAMING, SHEETING & TRIM

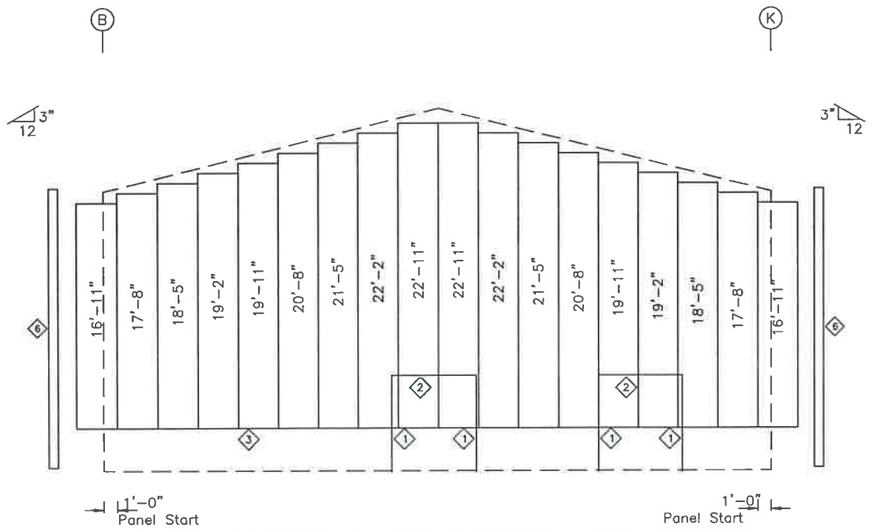
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 10	DJH	SPW	NONE



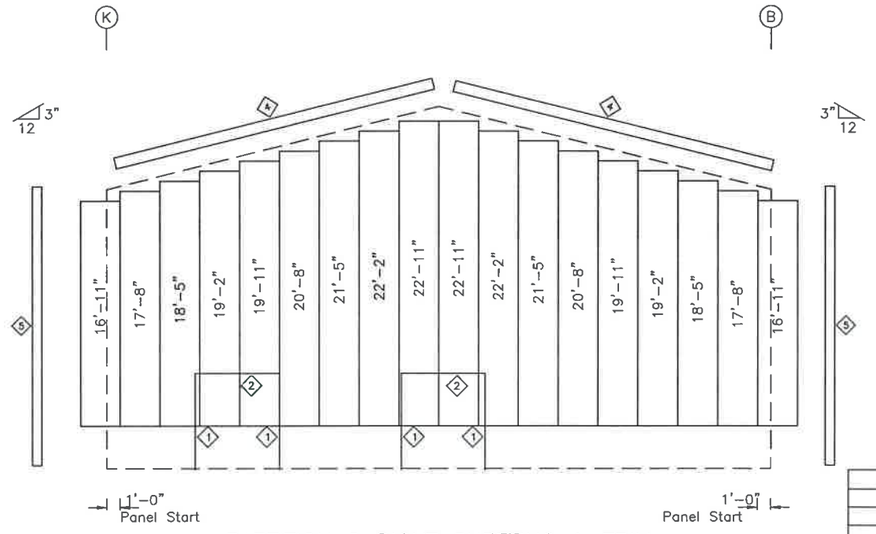
PARTITION 2 FRAMING

TRIM TABLE PARTITION 2			
◇ID	PART	LENGTH	DETAIL
1	R JAMB	7'-5"	TRIM_8
2	R HEAD	6'-7"	TRIM_61
3	DBLBASTR	16'-11"	TRIM_22
4	PRT10	4'-7"	PRT
5	PWT	4'-6"	PWT
6	O/S CORNER	10'-8"	PWT

MEMBER TABLE PARTITION 2		
MARK	PART	LENGTH
DJ-11	8X25C16	14'-0"
DH-10	8X25C16	6'-4"
G-65	8x25C16	19'-11 1/2"
G-66	8x25Z16	21'-11 1/2"
G-72	8x25C16	13'-1 15/16"
G-73	8x25Z16	14'-0 5/8"
G-74	8x25Z16	13'-10 1/2"
G-75	8x25Z16	11'-11 3/8"
G-76	8x25Z12	20'-7 1/2"



PARTITION 2 LEFT SHEETING & TRIM
PANELS: 26 Ga. R – NEED COLOR



PARTITION 2 RIGHT SHEETING & TRIM
PANELS: 26 Ga. R – NEED COLOR

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

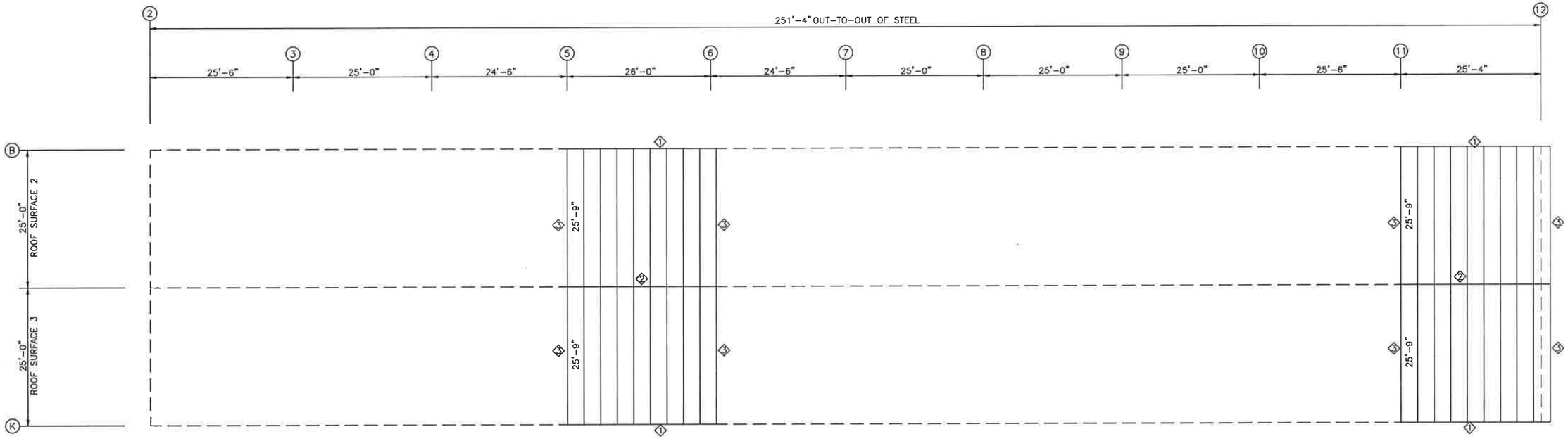
JOB NO: 9249R1-A | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: PARTITION FRAMING, SHEETING & TRIM

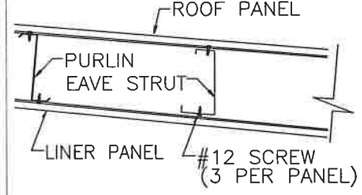
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 10.1	DJH	SPW	NONE

TRIM TABLE FRAME LINE 1			
◇ID	PART	LENGTH	DETAIL
1	R HEAD	12'-8"	ELA
2	RRIDFLSH	12'-6"	RR
3	R HEAD	12'-8"	WALL
4	R HEAD	12'-8"	LM

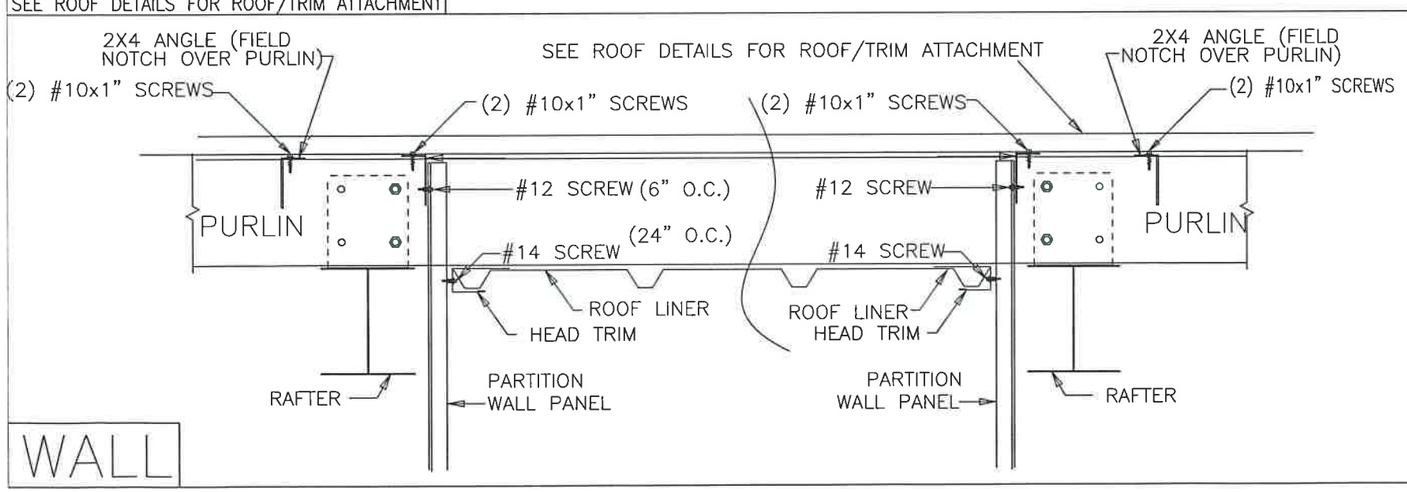


ROOF LINER SHEETING PLAN
PANELS: 26 Ga. R – NEED COLOR

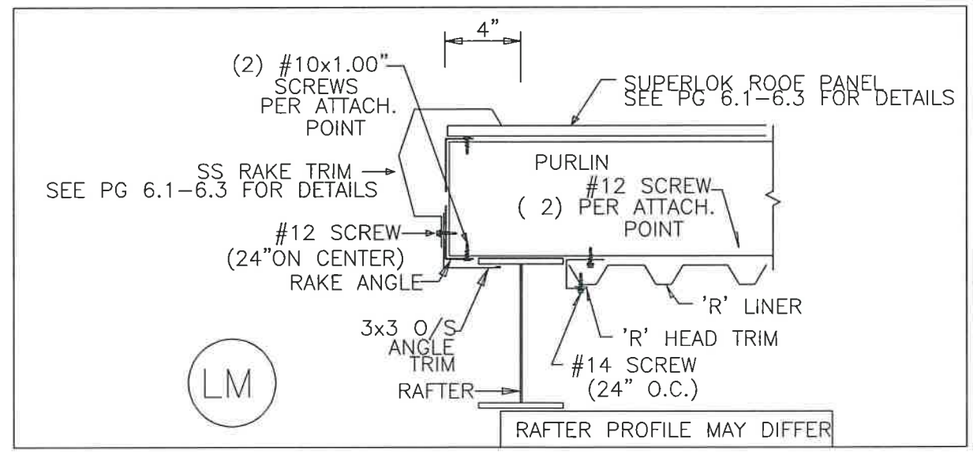
ELA



SEE ROOF DETAILS FOR ROOF/TRIM ATTACHMENT



WALL



RAFTER PROFILE MAY DIFFER

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

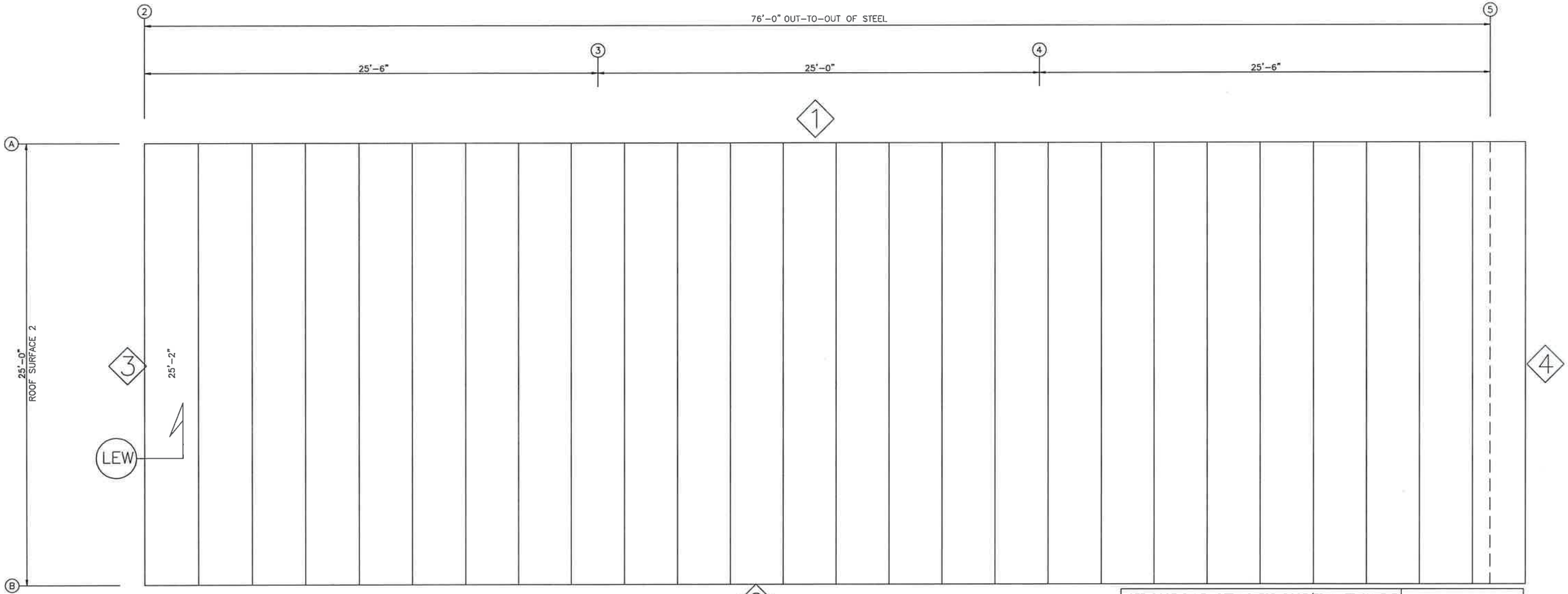
JOB NO: 9249 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: LINER SHEETING & TRIM

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 11	DJH	GTL	NONE

TRIM TABLE FRAME LINE 1			
◇ID	PART	LENGTH	DETAIL
1	R HEAD	19'-3"	ELA
2	3X3 I/S	19'-3"	WALL3
3	R HEAD	12'-8"	LEW
4	R HEAD	12'-8"	LM

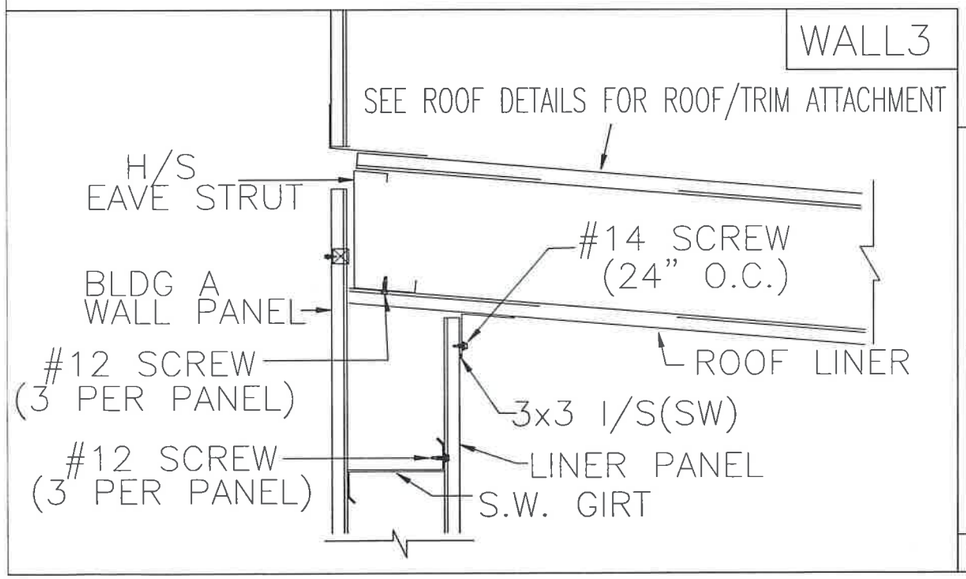


ROOF LINER SHEETING PLAN
PANELS: 26 Ga. R – NEED COLOR

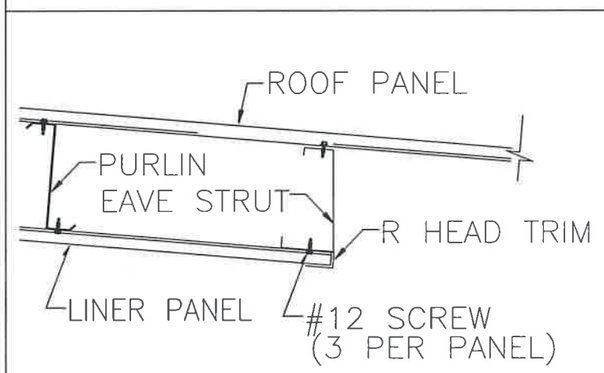
2

WALL3

SEE ROOF DETAILS FOR ROOF/TRIM ATTACHMENT

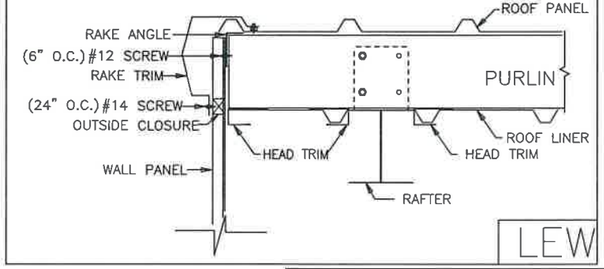


ELA



SEE ROOF DETAILS FOR ROOF/TRIM ATTACHMENT

SEE ROOF PANEL DETAILS FOR ROOF/TRIM ATTACHMENT



LEW

BUILDING "B"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNICAL SITE SOLUTIONS

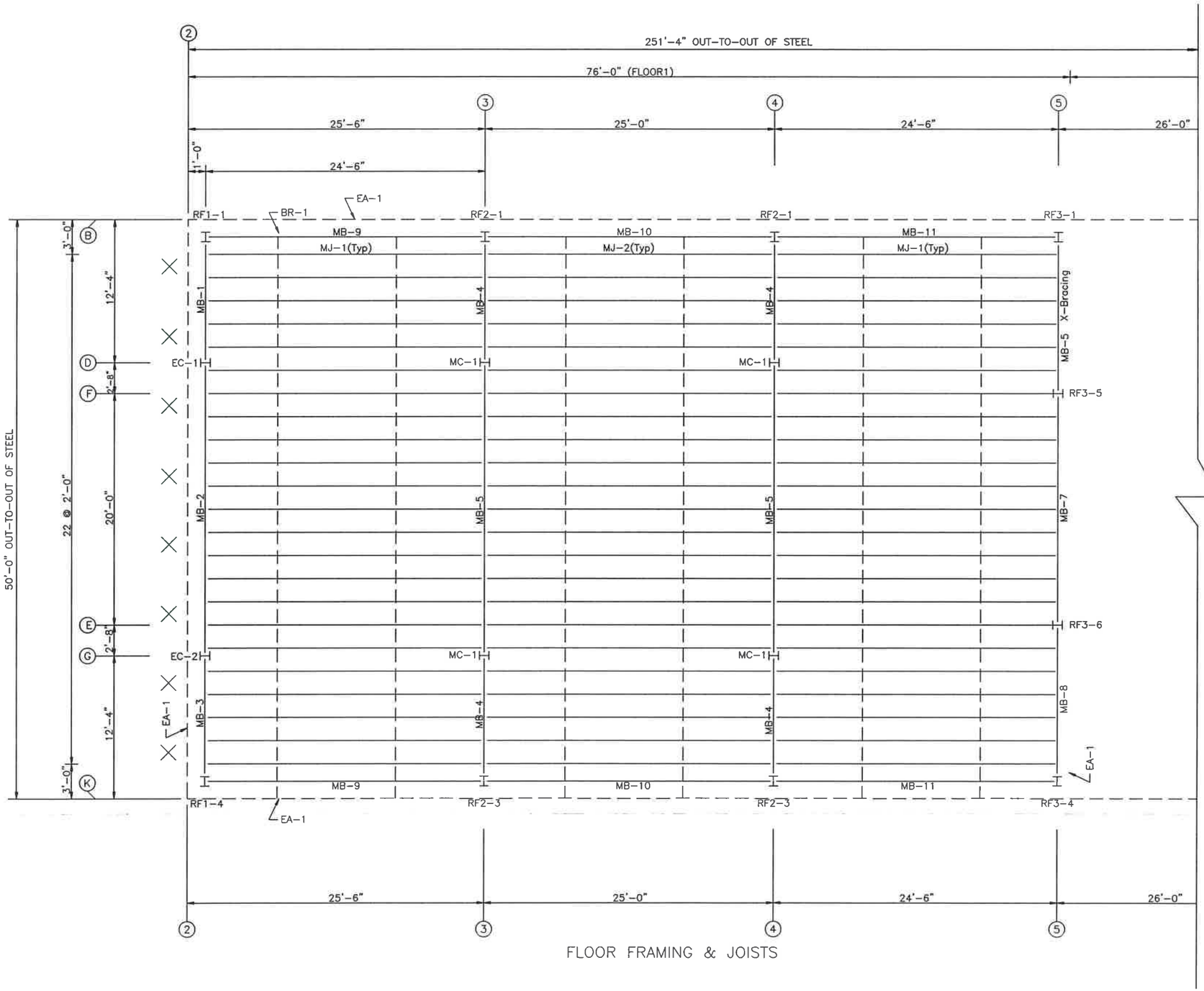
JOB NO: 9249 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: LINER SHEETING & TRIM

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 11.1	DJH	GTL	NONE

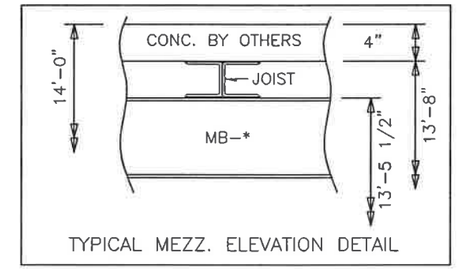
MEMBER TABLE				
MARK	PART	LENGTH	BOLT SIZE	QUANTITY
MB-1	w16X26	10'-6 1/8"	3/4"x2" A325	4
MB-2	w16X40	25'-2 3/4"	3/4"x2" A325	4
MB-3	w16X26	10'-6"	3/4"x2" A325	4
MB-4	w21X44	10'-2 1/8"	3/4"x2" A325	6
MB-5	w21X55	25'-2 3/4"	3/4"x2" A325	6
MB-6	w16X26	14'-11 3/4"	3/4"x2" A325	4
MB-7	w16X26	19'-11 1/2"	3/4"x2" A325	4
MB-8	w16X26	14'-11 3/4"	3/4"x2" A325	3
MB-9	w12X14	24'-4 3/4"	3/4"x2" A325	3
MB-10	w12X14	24'-10 3/4"	3/4"x2" A325	3
MB-11	w12X14	24'-2 5/8"	3/4"x2" A325	3
MC-1	w8X24	13'-5 1/2"		
MJ-1	18K3	25'-5 1/2"		
MJ-2	20K3	24'-11 1/2"		



FLOOR FRAMING & JOISTS

MEZZANINE LOADINGS

DEAD LOAD = 50 psf. (4" CONC., DECK & JOIST)
LIVE LOAD = 100 psf. (UNREDUCIBLE)
COLL. LOAD = 1 psf.



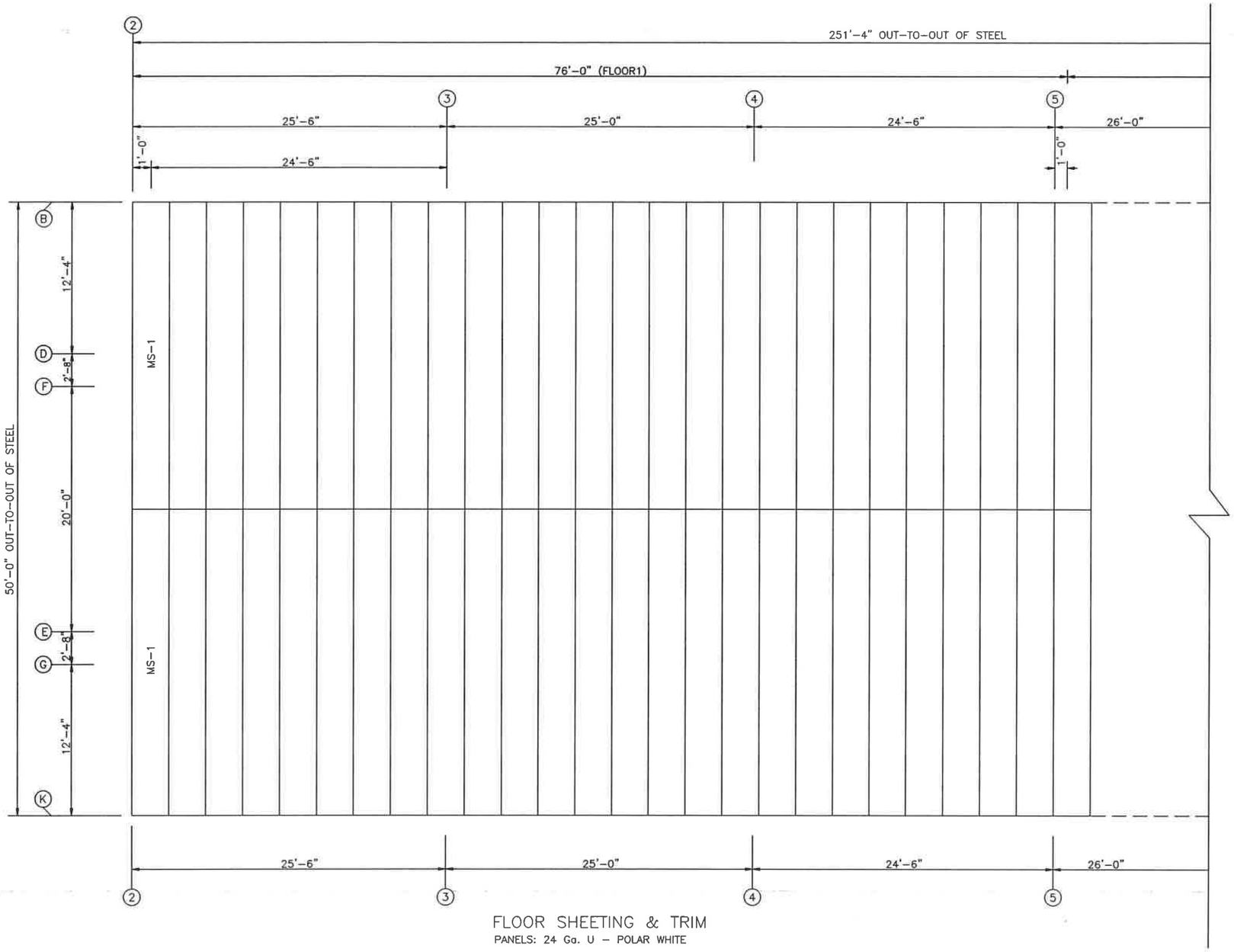
TYPICAL MEZZ. ELEVATION DETAIL

BUILDING "A"

ISSUE	DET	CHK	DATE

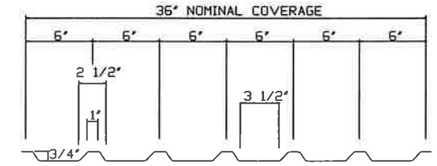
BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS
JOB NO: 9249
DATE: 10/13/25
LOCATION: HIGH SPRINGS, FL 32643
DRAWING NAME: MEZZANINE FRAMING LAYOUT
DRAWING NO: PAGE 12
DRAWN BY: DJH
CHECKED BY: GTL
SCALE: NONE

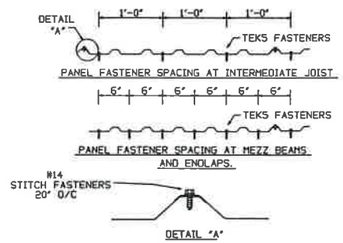


FLOOR SHEETING & TRIM

PANELS: 24 Ga. U – POLAR WHITE



PBU DECK PANEL



PBU DECK PANEL ATTACHMENT DETAIL

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

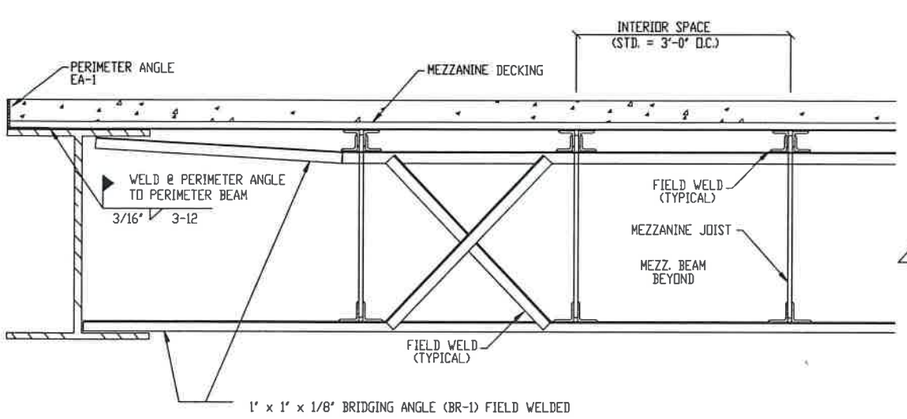
CUSTOMER: PINNACLE SITE SOLUTIONS

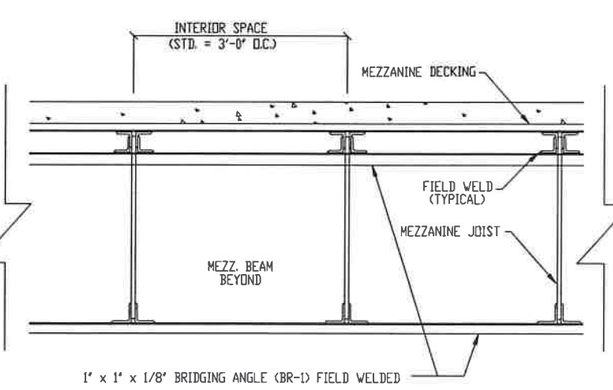
JOB NO: 9249 | DATE: 10/13/25

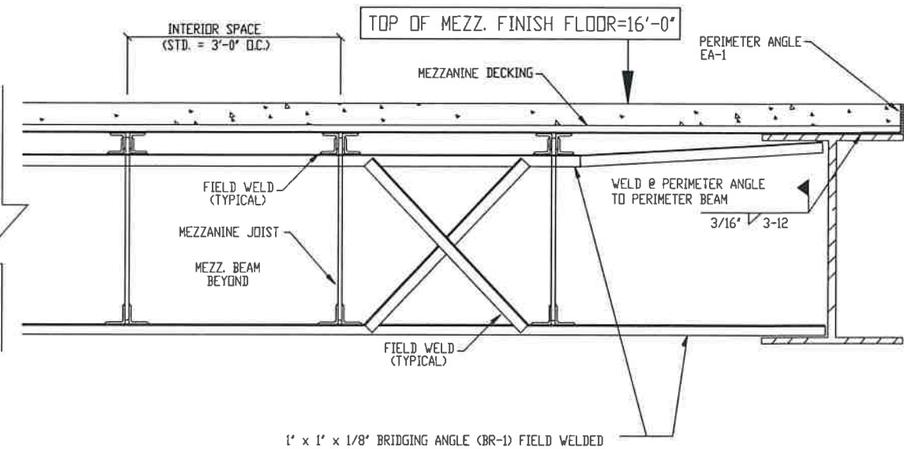
LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: MEZZANINE SHEETING LAYOUT

DRAWING NO.	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 12.1	DJH	GTL	NONE







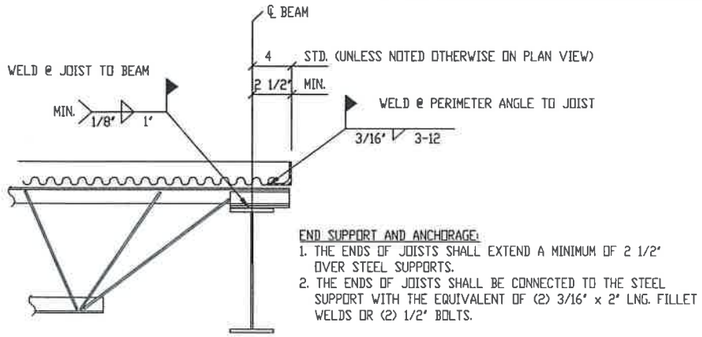
TYPICAL BRIDGING DETAILS

NOTE: BRIDGING ANGLE MAY CROSS IN MULTIPLE LOCATIONS. REFER TO DWG. ABOVE OR JOIST LAYOUT PAGE IF NOTED. LOCATIONS ARE MANDATORY.

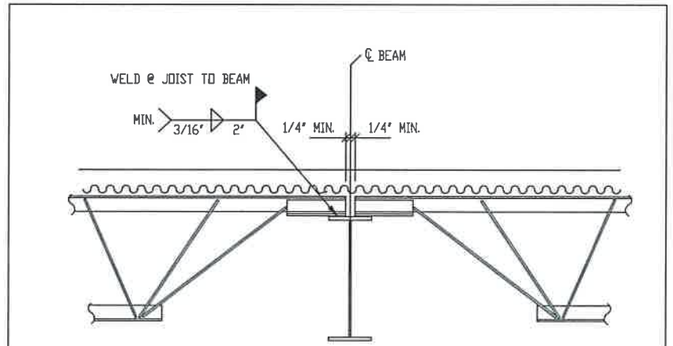
NOTE: ALL WELDING PER SJI & AWS SPECIFICATIONS.

HANDLING AND ERECTION OF JOISTS:

1. CARE SHALL BE EXERCISED AT ALL TIMES TO AVOID DAMAGE THROUGH CARELESS HANDLING DURING UNLOADING, STORAGE, AND ERECTION.
2. AS SOON AS JOISTS ARE ERECTED, ALL BRIDGING SHALL BE COMPLETELY INSTALLED AND THE JOISTS PERMANENTLY FASTENED INTO PLACE BEFORE THE APPLICATION OF ANY LOADS EXCEPT THE WEIGHT OF THE ERECTORS.
3. WHERE FIVE ROWS OF BRIDGING ARE REQUIRED IN SPANS OVER 40 FEET, EACH JOIST SHALL BE ADEQUATELY BRACED LATERALLY BEFORE THE NEXT JOIST IS ERECTED AND BEFORE ANY LOADS ARE APPLIED. HOISTING CABLES SHALL NOT BE RELEASED UNTIL SUPPORT HAS BEEN PROVIDED BY THE CENTER ROW OF DIAGONAL BRIDGING AND THE BRIDGING LINE HAS BEEN ANCHORED TO PREVENT LATERAL MOVEMENT.
4. DURING THE CONSTRUCTION PERIOD, THE CONTRACTOR SHALL PROVIDE MEANS FOR ADEQUATE DISTRIBUTION OF CONCENTRATED LOADS SO THAT THE CARRYING CAPACITY OF ANY JOIST IS NOT EXCEDED.
5. FIELD WELDING SHALL NOT DAMAGE THE JOISTS.



TYPICAL JOIST END DETAIL



TYPICAL JOIST END DETAIL @ INTERIOR BEAM

BUILDING "A"

ISSUE	DET	CHK	DATE

BUILDINGS AND MORE

CUSTOMER: PINNACLE SITE SOLUTIONS

JOB NO: 9249 | DATE: 10/13/25

LOCATION: HIGH SPRINGS, FL 32643

DRAWING NAME: MEZZANINE DETAILS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 12.2	DJH	GTL	NONE