

- 1) This drawing is for anchor rod placement only and is not foundation design.
- 2) Foundation must be square and level with all anchor rods true in size, location, and projection.
- 3) Projection shown must be held to keep threads clear of finished concrete.
- 4) This structural design data includes magnitude and location of design loads and support conditions, material properties, and type and size of major structural members necessary to show compliance with the Order Documents at the time of this issue. Any change to building loads or dimensions may change structural member sizes and locations shown. This structural design data will be superseded and voided by any future mailing.
- 5) Anchor rod size is determined by shear and tension at the bottom of the base plate. The length of the anchor rod and method of load transfer to the foundation are to be determined by the foundation engineer, and are not provided by the manufacturer.
- 6) Anchor rods are ASTM F1554 Gr. 36 material unless noted otherwise.
- 7) 3000 psi concrete compressive strength (f'_c) is assumed for the purpose of column base plate design unless otherwise noted.

SWC

EWB

BLD'G "B"

SWC

EWB

BLD'G "A"

SWA

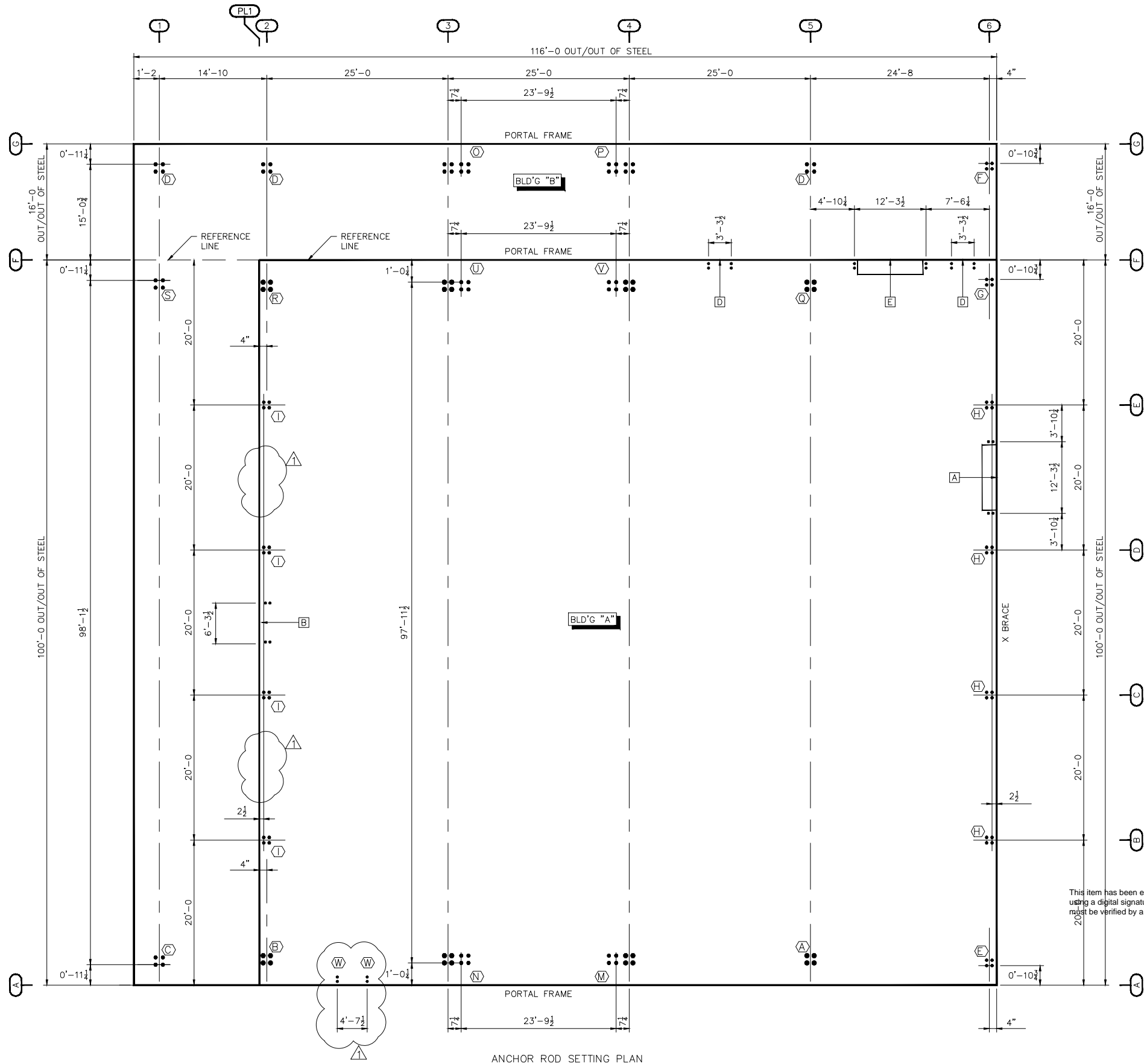
EWD

EWD

KEY PLAN

ACCESSORY SCHEDULE			
MARK	DESCRIPTION	DETAIL	QUAN.
<u>A</u>	12'-0" X 12'-0" FRAMED OPENINGS	<u>K</u>	1
<u>B</u>	6'-0" X 7'-0" FRAMED OPENINGS	<u>J</u>	1
<u>D</u>	3'-0" X 4'-0" FRAMED OPENINGS	<u>L</u>	2
<u>E</u>	12'-0" X 12'-0" FRAMED OPENINGS	<u>T</u>	1


ANCHOR BOLTS TO BE DESIGNED BY FOUNDATION ENGINEER USING DIAMETERS SHOWN IN THIS TABLE.	
ANCHOR ROD DESCRIPTION	QUANTITY
$\frac{5}{8}$ " \emptyset DIAMETER X	68
$\frac{3}{4}$ " \emptyset DIAMETER X	52
1" \emptyset DIAMETER X	32



Aug 06, 2024

DETAILS SUBJECT TO CHANGE BASED ON FINAL DESIGN.
ISSUED FOR CONSTRUCTION DRAWINGS WILL REPLACE THE
PERMIT DRAWINGS.

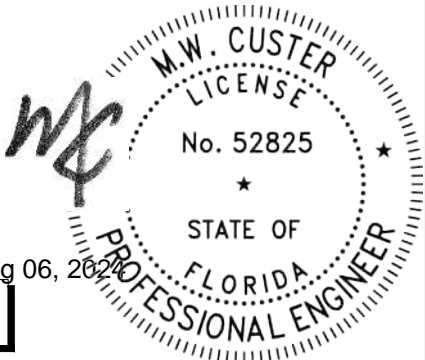
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		4704 W. Commercial Dr. Ste. B North Little Rock, AR 72116-8040 800-643-5555	
Customer: DOUG MOSLEY 362 SW MCCLINTON DR FORT WHITE FL 32038 DOUG MOSLEY		Project Name & Location: DOUG MOSLEY FORT WHITE FL 32038	
Drawing Status: <div> <input type="checkbox"/> Preliminary <input type="checkbox"/> (Not For Construction) </div>		<div> <input checked="" type="checkbox"/> For Construction Permit <input type="checkbox"/> For Erector Installation </div>	
<div> <div>REVISED</div> <div> <input type="checkbox"/> For Approval <input type="checkbox"/> (Not For Construction) </div> </div>			

Scale: NOT TO SCALE
 Drawn by: FEM 4/28/23
 Checked by: JAQ 5/9/23
 Project Engineer: MTS
 Job Number: 19-B-33112-1
 Sheet Number: F1 of 6

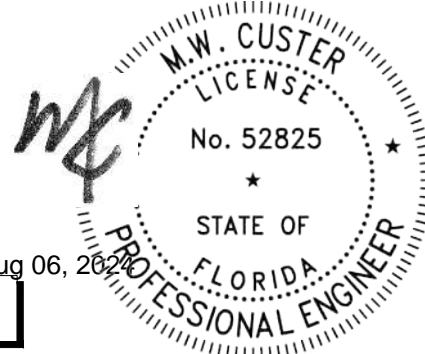
The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

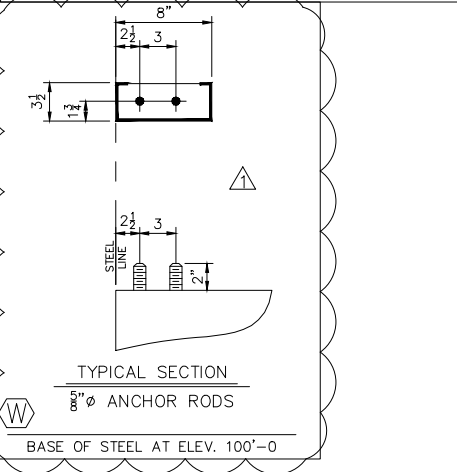
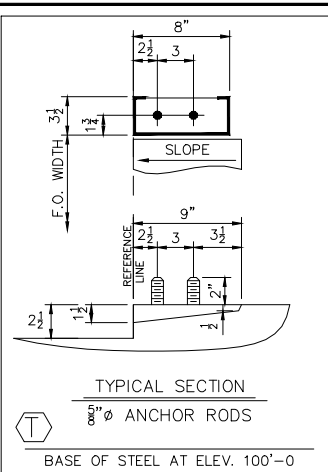
M.W. CUSTER, P.E.
FLORIDA P.E. 0052825
E on the date and/or time stamp shown




[illegible]

Aug 06, 2024





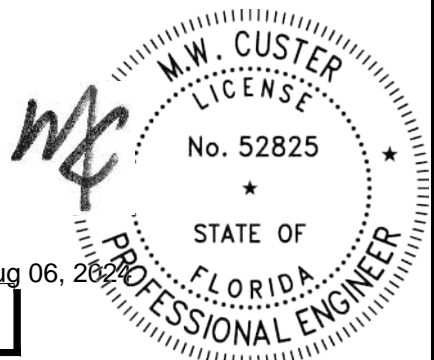
		4704 W. Commercial Dr. Ste. B North Little Rock, AR 72116-8040 800-643-5555	
Customer: DOUG MOSLEY 362 SW MCCLINTON DR FORT WHITE FL 32038 DOUG MOSLEY		Project Name & Location: DOUG MOSLEY FORT WHITE FL 32038	
Drawing Status: <div> <input type="checkbox"/> Preliminary (Not For Construction) </div> <div> <input type="checkbox"/> For Approval (Not For Construction) </div>		<div> <input checked="" type="checkbox"/> For Construction Permit </div> <div> <input type="checkbox"/> For Erector/Installation </div>	
REVISED			

Sheet Number: F3 of 6

M.W. CUSTER, P.E.
FLORIDA P.E. 0052825

Aug 06, 2024

DETAILS SUBJECT TO CHANGE BASED ON FINAL DESIGN.
ISSUED FOR CONSTRUCTION DRAWINGS WILL REPLACE THE
PERMIT DRAWINGS.

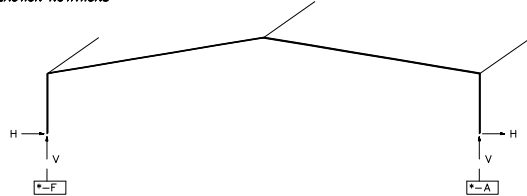
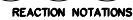


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FRAME ID: #6                                USER NAME:mtsanchez
cs 100./14./8.583    20./15./              JOB  NAME:33121A
DATE: 07/12/24  PAGE:6-3
FILE:frame_1.fra

SUPPORT REACTIONS FOR EACH LOAD GROUP
*LOCATION: Gridlines:      1
NOTES: (1) All reactions are in kips and kip-ft.
        (2) Primary wind load cases are not concurrent.
        (3) X-bracing reactions (RBPULLW and RBPUEQ) are combined withWLW and LEO groups only.

```



LOAD GROUP REACTION TABLE GRIDLINES * = 1						
COLUMN	*F=			*A=		
LOAD GROUP	H	V	L	H	V	L
DL	2.2	2.7	-0.0	-2.2	2.5	-0.0
LL	8.8	9.8	-0.0	-8.8	8.6	-0.0
COLL	0.4	0.5	-0.0	-0.4	0.4	-0.0
WL1	-18.5	-25.1	-0.0	19.7	-19.2	-0.0
WL2	-5.2	-5.0	-0.0	0.4	-0.5	-0.0
WL3	-15.6	-23.8	-0.0	19.6	-18.8	-0.0
WL4	-2.8	-20.9	-0.0	20.8	-23.3	-0.0
LWL3	0.3	-3.6	-0.0	0.6	-0.0	-0.0
LWL4	-2.5	-0.8	-0.0	1.3	-4.5	-0.0
WL3	-19.0	-21.7	-0.0	21.9	-23.7	-0.0
WL4	-2.9	-1.6	-0.0	2.5	-4.9	-0.0

LOAD GROUP DESCRIPTION

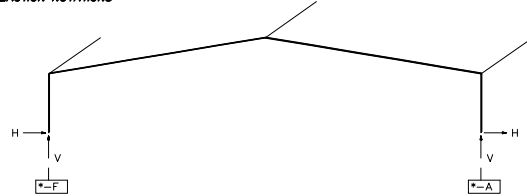
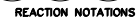
DL	:	Roof Dead Load
LL	:	Roof Live Load
COLL	:	Roof Collateral Load
WL1	:	Wind from Left to Right with +Gcpi
WL2	:	Wind from Left to Right with -Gcpi
LWL1	:	Windward Corner Left with +Gcpi
LWL2	:	Windward Corner Right with +Gcpi
LWL3	:	Windward Corner Left with -Gcpi
LWL4	:	Windward Corner Right with -Gcpi
WL3	:	Wind from Right to Left with +Gcpi
WL4	:	Wind from Right to Left with -Gcpi

```

FRAME ID #5                                USER NAME: mtsanchez                DATE: 07/12/24   PAGE: 5-3
cs 100/14./19.917   20./145.              JOB  NAME: 33112A                  FILE: frame_2.fro

SUPPORT REACTIONS FOR EACH LOAD GROUP
*LOCATION: Gridlines:      2
NOTES: (1) All reactions are in kips and kip-ft.
        (2) Primary wind load cases are not concurrent.
        (3) X-bracing reactions (RBPULV and RBPUEQ) are combined with WL and LEQ groups only.

```

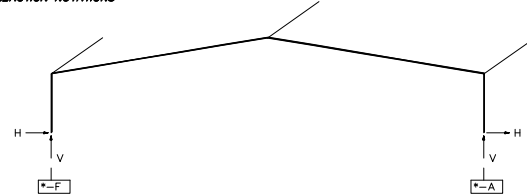
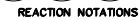


COLUMN LOAD GROUP	*=-F			*=-A		
	H	V	L	H	V	L
DL	4.1	4.6	-0.0	-4.1	4.1	-0.0
LL	21.5	22.8	-0.0	-21.6	19.9	-0.0
COLL	1.1	1.1	-0.0	-1.1	1.0	-0.0
LWL1	-39.1	-37.5	-0.0	33.5	-36.9	-0.0
LWL2	3.7	3.7	-0.0	-4.1	5.9	-0.0
LWL3	-35.5	-35.5	-0.0	32.8	-36.2	-0.0
LWL4	-39.4	-41.3	-0.0	33.9	-42.4	-0.0
LWL5	7.2	2.5	-0.0	-5.6	6.6	-0.0
LWL6	3.4	6.4	-0.0	-4.5	0.4	-0.0
LWL7	-38.7	-42.0	-0.0	42.7	-43.6	-0.0
LWL8	4.1	5.8	-0.0	4.1	-0.8	-0.0

LOAD GROUP DESCRIPTION

DL	:	Roof Dead Load
LL	:	Roof Live Load
COLL	:	Roof Collateral Load
WL1	:	Wind from Left to Right with +Gcpi
WL2	:	Wind from Left to Right with -Gcpi
LWL1	:	Windward Corner Left with +Gcpi
LWL2	:	Windward Corner Right with +Gcpi
LWL3	:	Windward Corner Left with -Gcpi
LWL4	:	Windward Corner Right with -Gcpi
WL3	:	Wind from Right to Left with +Gcpi
WL4	:	Wind from Right to Left with -Gcpi

FRAME ID #4	USER NAME: mtsanchez	DATE: 07/12/24	PAGE: 4-3
cs 100./14./25. 20./121./0.	JOB NAME: 33112A	FILE: frames_3-5.fra	
SUPPORT REACTIONS FOR EACH LOCAL GROUP			
*LOCATION: Gridlines: 3 4 5			
NOTES: (1) All reactions are in kips and kip-ft.			
(2) Primary and local loads are not concurrent.			
(3) X-bracing reactions (RBPJULW and RBUPCEQ) are combined withWL and LEQ groups only.			
			TIME: 17:48:59

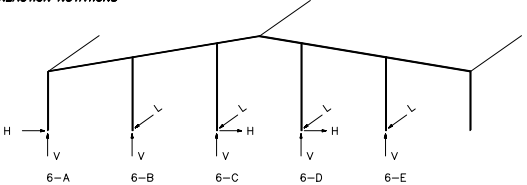


LOAD GROUP		REACTION		TABLE		GRIDLINES * =		3 4 5	
COLUMN		*-F		*-A					
LOAD GROUP	H	V	L	H	V	L			
DL	4.9	5.4	0.0	-4.9	4.9	0.0			
LL	27.1	28.6	-0.0	-27.2	25.0	0.0			
COLL	1.4	1.4	-0.0	-1.4	1.2	-0.0			
WL1	-21.7	-27.2	-0.0	18.0	-19.7	-0.0			
WL2	-9.4	-3.6	-0.0	7.0	-7.4	-0.0			
WL3	-18.5	-25.5	-0.0	14.7	-19.1	-0.0			
LWL2	-2.9	-22.1	-0.0	18.4	-24.5	-0.0			
LWL3	-6.3	-11.9	-0.0	6.4	-6.8	-0.0			
LWL4	-9.6	-8.4	-0.0	7.3	-12.3	-0.0			
WL4	-21.1	-22.6	-0.0	25.5	-25.5	-0.0			
WL4	-9.0	-9.0	-0.0	14.9	-13.3	-0.0			

LOAD GROUP DESCRIPTION

DL	:	Roof Dead Load
LL	:	Roof Live Load
COLL	:	Roof Collateral Load
WL1	:	Wind from Left to Right with +Gcpi
WL2	:	Wind from Left to Right with -Gcpi
LWL1	:	Windward Corner Left with +Gcpi
LWL2	:	Windward Corner Right with +Gcpi
LWL3	:	Windward Corner Left with -Gcpi
LWL4	:	Windward Corner Right with -Gcpi
WL3	:	Wind from Right to Left with +Gcpi
WL4	:	Wind from Right to Left with -Gcpi

FRAME DESCRIPTION: Endwall EWD		USER NAME: mjmenez JOB NAME: 33112A	DATE: 04/17/23 FILE: REW4BLDG1	PAGE: EW-2
PATH: R:\Jobs\Active\Eng\19-B-33112\ver02-mjmenez\BLDG-A\run01\				
SUPPORT REACTIONS FOR EACH LOAD GROUP NOTE: All reactions are in kips and kip-ft.				
				TIME: 17:13:17

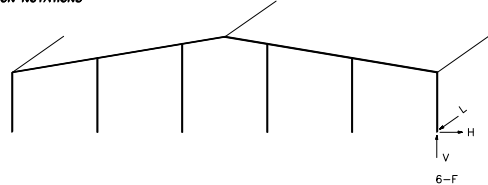


LOAD GROUP REACTION TABLE															
COLUMN	6-A			6-B			6-C			6-D			6-E		
LOAD GROUP	H	V	L	H	V	L	H	V	L	H	V	L	H	V	L
D	0.0	0.0	0.5	0.0	1.1	0.	0.0	1.0	0.	0.0	1.1	0.	0.0	1.2	0.
C	0.0	0.0	0.1	0.0	0.3	0.	0.0	0.2	0.	0.0	0.2	0.	0.0	0.3	0.
L	0.1	2.2	0.	0.5	0.0	0.	0.0	0.0	0.	0.0	5.0	0.	0.0	0.6	0.
W+	-0.1	-2.7	0.	0.6	3.7	0.	-8.2	4.4	0.	-8.2	4.4	0.	-8.3	3.7	0.
W-	-0.1	-2.7	0.	-6.3	-4.1	0.	-8.2	-4.9	0.	-8.2	-4.9	0.	-8.3	-4.1	0.
WR	-0.1	-2.7	0.	-6.3	0.0	0.	-4.9	0.0	3.2	-11.6	0.0	0.	-8.3	0.0	0.
WL	-0.1	-2.7	0.	-6.3	0.0	-4.4	-12.8	0.0	0.	-3.7	0.0	0.	-8.3	0.0	0.

LOAD GROUP DESCRIPTION

D	:	Dead load
C	:	Collateral load
L	:	Live load
W+	:	Wind load as an inward acting pressure
W-	:	Wind load as an outward acting suction
WR	:	Wind force from the right
WL	:	Wind force from the left

FRAME DESCRIPTION: Endwall EMD		USER NAME: mjimenez JOB NAME: 33112A	DATE: 04/17/23 FILE: REW4BLDG1	PAGE: EW-3
PATH: R: \jobs\Active\Eng\19-B-33112\ver02-mjimenez\BLDG-A\run01\				
SUPPORT REACTIONS FOR EACH LOAD GROUP NOTE: All reactions are in kips and kip-ft.				
				TIME: 17:13:17



COLUMN	6-F		
LOAD GROUP	H	V	L
D	0.0	0.9	0.
C	0.0	0.2	0.
L	0.1	4.0	0.
W+	-0.3	-7.0	3.4
W-	-0.3	-7.0	-3.9
WR	-0.3	-7.0	0.
WL	-0.3	-7.0	0.

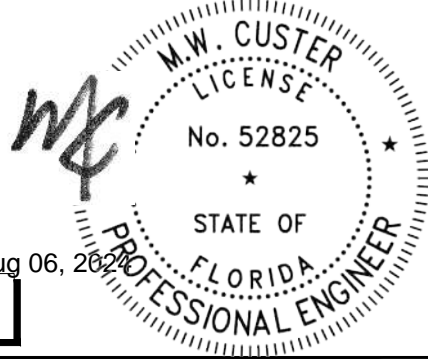
LOAD GROUP DESCRIPTION

D	:	Dead load
C	:	Collateral load
L	:	Live load
W+	:	Wind load as an inward acting pressure
W-	:	Wind load as an outward acting suction
WR	:	Wind force from the right
WL	:	Wind force from the left

NOTES

- 1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
 - 2) THE REACTION PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTED OTHERWISE).
 - a) A REACTION TABLE IS PROVIDED WITH THE REACTIONS FOR EACH LOAD GROUP.
 - b) RIGID FIELDS
 - (1) GABLED BUILDINGS
 - a) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE LEFT SIDE OF THE BUILDING, AS SHOWN ON THE ANCHOR ROD DRAWING, FROM THE OUTSIDE OF THE BUILDING.
 - b) INTERIOR COLUMNS ARE SPACED FROM LEFT SIDE TO RIGHT SIDE.
 - (2) SINGLE SLOPE BUILDINGS
 - a) LEFT COLUMN IS THE LOW SIDE COLUMN.
 - b) RIGHT COLUMN IS THE HIGH SIDE COLUMN.
 - c) INTERIOR COLUMNS ARE SPACED FROM LOW SIDE TO HIGH SIDE.
 - c) ENDWALLS
 - (1) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE WALL FROM THE OUTSIDE.
 - (2) INTERIOR COLUMNS ARE SPACED FROM LEFT TO RIGHT.
 - 3) ANCHOR ROD SIZE IS DETERMINED BY SHEAR AND TENSION AT THE BOTTOM OF THE BASE PLATE. THE LENGTH OF THE ANCHOR ROD AND METHOD OF LOAD TRANSFER TO THE FOUNDATION ARE TO BE DETERMINED BY THE FOUNDATION ENGINEER.
 - e) ANCHOR RODS ARE ASTM F1554 GR. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
 - f) X-BRACING
 - (1) ROD BRACING REACTIONS HAVE BEEN INCLUDED IN VALUES SHOWN IN THE REACTION TABLES.
 - (2) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (R_{BUPEO} AND R_{BWDOW}) DO NOT INCLUDE THE AMPLIFICATION FACTOR, Q_s .
 - (3) FOR CANADA BUILDING CODE (NBC), WHEN X-BRACING IS PRESENT IN THE SIDEWALL OR ENDWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (R_{BUPEO} AND R_{BWDOW}) ARE MULTIPLIED BY FORCE REDUCTION FACTOR, R_d , WHEN SPECIFIED. SHORT-PERIOD SPECTRAL ACCELERATION RATIO ($f_s \leq 0.2$) IS GREATER THAN 0.45.
 - 3) REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.
 - a) FOR PROJECTS USING SEISMIC DESIGN WIND SPEEDS SUCH AS 2012 IBC, 2015 IBC, OR FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH VALUE WITH A LOAD FACTOR OF 1.0.
 - b) FOR IBC CODES, THE SEISMIC REACTIONS PROVIDED ARE AT A STRENGTH LEVEL AND DO NOT CONTAIN THE RHO FACTOR.
 - c) FOR UBC CODES, THE SEISMIC REACTIONS PROVIDED DO NOT CONTAIN THE $R_w R_s$ FACTOR.
- THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMIC FOUNDATION DESIGN.

This item has been electronically signed and sealed by M.W. Custer, P.E. on the date and time stamp shown using a digital signature. Printed copies of this document are not considered signed and sealed and the signature must be verified by a 3rd Party Certificate Authority on any electronic copy.



DETAILS SUBJECT TO CHANGE BASED ON FINAL DESIGN.
ISSUED FOR CONSTRUCTION DRAWINGS WILL REPLACE THE
PERMIT DRAWINGS.

[illegible]

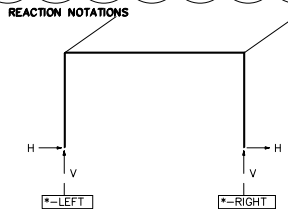
HERITAGE BUILDING SYSTEMS.		4704 W. Commercial Dr. Ste. B North Little Rock, AR 72116-8040 800-643-5555	
Customer: DOUG MOSLEY 362 SW MCCLINTON DR FORT WHITE FL 32038 DOUG MOSLEY		Project Name & Location: DOUG MOSLEY FORT WHITE FL 32038	
Drawing Status: <input type="checkbox"/> Preliminary <input checked="" type="checkbox"/> For Construction Permit REVISED		<input type="checkbox"/> For Approval <input checked="" type="checkbox"/> For Ejector Installation <input type="checkbox"/> (Not For Construction) <input type="checkbox"/> (Not For Construction)	

Scale:	NOT TO SCALE	
Drawn by:	FEM	4/28/23
Checked by:	JAQ	5/9/23
Project Engineer:	MTS	
Job Number:	19-B-33112-1	
Sheet Number:	F4 of 6	

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

M.W. CUSTER, P.E.
FLORIDA P.E. 0052825

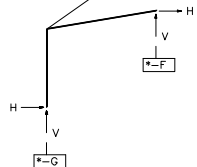
FRAME ID #8 pf 25/14, main building at p	USER NAME:mtsanchez JOB NAME:33112A	DATE:07/12/24 PAGE:8-2 FILE:pframe_gridf_bay3.fra
SUPPORT REACTIONS FOR EACH LOAD GROUP		
LOCATION:boys 3--(Grdline F)		
NOTES(1) All reactions are in kips and kip-ft. (2) Primary wind load cases are not concurrent. (3) X-bracing reactions (RBPULU and RBUPCO) are combined withLWL and LEQ groups only.		TIME:17:42:47



LOAD GROUP REACTION TABLE GRIDLINES * = F						
COLUMN	*-LEFT			*-RIGHT		
LOAD GROUP	H	V	L	H	V	L
DL	0.0	0.5	-0.0	-0.0	0.5	-0.0
LWL1	-4.7	-5.3	-0.0	-4.9	5.3	-0.0
LWL2	4.9	5.3	-0.0	4.7	-5.3	-0.0

LOAD GROUP DESCRIPTION

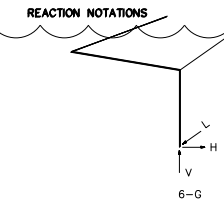
DL	:	Roof Dead Load
LWL1	:	Wind from Left to Right with +GCp
LWL2	:	Wind from Right to Left with -GCp



LOAD GROUP		REACTION TABLE		GRIDLINES * = 3 4 5			
COLUMN	* = G			SUPPORT(*=F)			
LOAD GROUP	H	V	L	H	V	L	
DL	0.0	0.8	-0.0	-0.0	0.5	-0.0	
LL	0.1	4.4	-0.0	0.1	3.6	-0.0	
COLL.	0.0	0.2	-0.0	-0.0	0.2	-0.0	
WL1	-1.1	-5.1	-0.0	0.6	-4.3	-0.0	
WL2	-2.3	-2.7	-0.0	1.6	-2.8	-0.0	
LWL1	2.1	-5.7	-0.0	4.3	-6.7	-0.0	
LWL2	2.1	-3.8	-0.0	3.7	-2.2	-0.0	
LWL3	0.9	-3.3	-0.0	2.1	-2.2	-0.0	
LWL4	0.9	-1.3	-0.0	1.5	-0.7	-0.0	
WL3	1.7	-3.9	-0.0	3.2	-2.4	-0.0	
WL4	0.5	-1.4	-0.0	1.1	-0.9	-0.0	

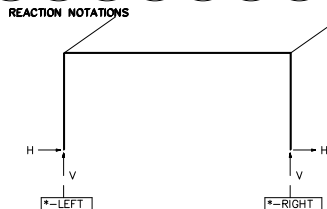
LOAD GROUP DESCRIPTION	
DL	: Roof Dead Load
LL	: Roof Live Load
COLL	: Roof Collateral Load
WL1	: Wind from Left to Right with +GCpi
WL2	: Wind from Left to Right with -GCpi
LWL1	: Windward Corner Left with +GCpi
LWL2	: Windward Corner Right with +GCpi
LWL3	: Windward Corner Left with -GCpi
LWL4	: Windward Corner Right with -GCpi
WL3	: Wind from Right to Left with +GCpi
WL4	: Wind from Right to Left with -GCpi

FRAME ID #9 pf 25/11.333 leanto plane SW	USER NAME: mtsanchez JOB NAME: 33112A	DATE: 07/12/24 PAGE: 9-2 FILE: prframe_grid3_b0y3.fra
SUPPORT REACTIONS FOR EACH LOAD GROUP LOCATION: boys 3-(Gridline G) NOTES: (1) All reactions are in kips and kip-ft. (2) Primary wind load cases are not concurrent. (3) X-bracing reactions (RBPULV and RBPUEQ) are combined with LM and LEQ groups only.		
		TIME: 17:44:49



COLUMN	6-G		
LOAD GROUP	H	V	L
D	0.0	0.5	0.
C	0.0	0.1	0.
L	-0.1	2.2	0.
W+	0.1	-4.1	1.5
W-	0.1	-4.1	-1.7
WR	2.4	-4.1	0.
WL	-2.5	-4.1	0.

LOAD GROUP DESCRIPTION	
D	: Dead load
C	: Collateral load
L	: Live load
W+	: Wind load as an inward acting pressure
W-	: Wind load as an outward acting suction
WR	: Wind force from the right
WL	: Wind force from the left



LOAD GROUP REACTION TABLE GRIDLINES * = G						
COLUMN	*-LEFT			*-RIGHT		
LOAD GROUP	H	V	L	H	V	L
DL	0.0	0.3	-0.0	-0.0	0.3	-0.0
LWL1	-0.4	-0.4	-0.0	-0.4	0.4	-0.0
LWL2	0.4	0.4	-0.0	0.4	-0.4	-0.0

LOAD GROUP DESCRIPTION


DL	:	Roof Dead Load
LWL1	:	Wind from Left to Right with +GCp
LWL2	:	Wind from Right to Left with -GCp

NOTES

- 1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE ORDER DOCUMENTS.
- 2) THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTED OTHERWISE).
 - a) A REACTION TABLE IS PROVIDED WITH THE REACTIONS FOR EACH LOAD GROUP.
 - b) RIGID FRAMES
 - (1) GABLED BUILDINGS
 - (a) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE LEFT SIDE OF THE BUILDING, AS SHOWN ON THE ANCHOR ROD DRAWING, FROM THE OUTSIDE OF THE BUILDING.
 - (b) INTERIOR COLUMNS ARE SPACED FROM LEFT SIDE TO RIGHT SIDE.
 - (2) SINGLE SLOPE BUILDINGS
 - (a) LEFT COLUMN IS THE LOW SIDE COLUMN.
 - (b) RIGHT COLUMN IS THE HIGH SIDE COLUMN.
 - (c) INTERIOR COLUMNS ARE SPACED FROM LOW SIDE TO HIGH SIDE.
 - c) ENDWALLS
 - (1) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE WALL FROM THE OUTSIDE.
 - (2) INTERIOR COLUMNS ARE SPACED FROM LEFT TO RIGHT.
 - d) ANCHOR ROD SIZE IS DETERMINED BY SHEAR AND TENSION AT THE BOTTOM OF THE BASE PLATE. THE LENGTH OF THE ANCHOR ROD AND METHOD OF LOAD TRANSFER TO THE FOUNDATION ARE TO BE DETERMINED BY THE FOUNDATION ENGINEER.
 - e) ANCHOR RODS ARE ASTM F1554 OR 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.

- (1) X-BRACING
- (2) IF BRACING REACTIONS HAVE BEEN INCLUDED IN VALUES SHOWN IN THE REACTION TABLES.
- (3) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUEQF AND RBWQEQ) DO NOT INCLUDE THE AMPLIFICATION FACTOR, R_d .
- (4) FOR CANADA BUILDING CODE (NBC), WHEN X-BRACING IS PRESENT IN THE SIDEWALL OR ENDWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUEQF & RBWQEQ) ARE MULTIPLIED BY FORCE REDUCTION FACTOR, R_d , WHEN SPECIFIED SHORT-PERIOD SPECTRAL DETERMINATION PERIOD T_d IS GREATER THAN 0.5 SECONDS.
- 3) REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE THE DESIGN PROPPING REACTIONS. THE LOAD FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.
 - a) FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC, THE REACTIONS ARE TO BE USED AS UN-FACTORED REACTIONS.
 - b) FOR IBC CODES, THE SEISMIC REACTIONS PROVIDED ARE AT A STRENGTH LEVEL AND DO NOT CONTAIN THE RHO FACTOR.
 - c) FOR NBCC CODES, THE SEISMIC REACTIONS PROVIDED DO NOT

THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

		4704 W. Commercial Dr. Ste. B North Little Rock, AR 72116-8040 800-643-5555	
Customer: DOUG MOSLEY 362 SW MCCLINTON DR FORT WHITE FL 32038 DOUG MOSLEY		Project Name & Location: DOUG MOSLEY FORT WHITE FL 32038	
Drawing Status:		<input type="checkbox"/> Preliminary (Not For Construction)	
REVISED		<input checked="" type="checkbox"/> For Construction Permit <input type="checkbox"/> For Approval <input type="checkbox"/> For Erector Installation	

HERITAGE
BUILDING SYSTEMS.

Customer:	Project Name & Location:
BOULDER COUNTY	BOULDER COUNTY

DOUG MOSLEY

FORT WHITE FL 32038

Drawing Status:	<input type="checkbox"/> Preliminary (Not For Construction)	<input checked="" type="checkbox"/> For Construction Permit
-----------------	--	---

REVISÉ ☐ For Approval ☐ For Frector Installation

Scale: NOT TO SCALE

Drawn by: FEM 4/28/23

Checked by: JAQ 5/9/23

Project Engineer: MTS

Job Number: 19-B-33112-1

Sheet Number: F5 of 6

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

M.W. CUSTER, P.E.

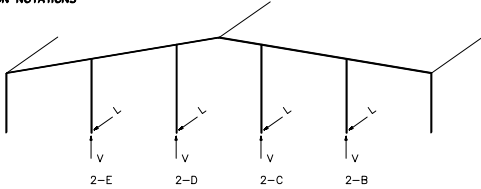
FLORIDA P.E. 0052825

This item has been electronically signed and sealed by M.W. Custer, P.E. on the date and/or time stamp shown using a digital signature. Printed copies of this document are not considered signed and sealed and the signature must be verified by a 3rd Party Certificate Authority on any electronic copy.

DETAILS SUBJECT TO CHANGE BASED ON FINAL DESIGN.
ISSUED FOR CONSTRUCTION DRAWINGS WILL REPLACE THE
PERMIT DRAWINGS.

Aug 06, 2021

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

COLUMN	2-E			2-D			2-C			2-B		
LOAD GROUP	H	V	L	H	V	L	H	V	L	H	V	L
D	0.	0.2	0.	0.	0.2	0.	0.	0.2	0.	0.	0.2	0.
W+	0.	0.	3.7	0.	0.	4.4	0.	0.	4.4	0.	0.	3.7
W-	0.	0.	-4.1	0.	0.	-4.9	0.	0.	-4.9	0.	0.	-4.1

LOAD GROUP DESCRIPTION

- D : Dead load
- W+ : Wind load as an inward acting pressure
- W- : Wind load as an outward acting suction

NOTES

- 1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
- 2) THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTED OTHERWISE).
- a) A REACTION TABLE IS PROVIDED WITH THE REACTIONS FOR EACH LOAD GROUP.
- b) RIGID FRAMES
- (1) GABLED BUILDINGS
- a) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE LEFT SIDE OF THE BUILDING, AS SHOWN ON THE ANCHOR ROD DRAWINGS, FROM THE OUTSIDE OF THE BUILDING.
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- a) LEFT COLUMN IS THE LOW SIDE COLUMN.
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- (1) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE WALL FROM THE OUTSIDE.
- (2) INTERIOR COLUMNS ARE SPACED FROM LEFT TO RIGHT.
- d) ANCHOR ROD SIZE IS DETERMINED BY SHEAR AND TENSION AT THE BOTTOM OF THE BASE PLATE. THE LENGTH OF THE ANCHOR ROD AND METHOD OF LOAD TRANSFER TO THE FOUNDATION ARE TO BE DETERMINED BY THE FOUNDATION ENGINEER.
- e) ANCHOR RODS ARE ASTM F1554 Gr. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
- f) X-BRACING
- (1) ROD BRACING REACTIONS HAVE BEEN INCLUDED IN VALUES SHOWN IN THE REACTION TABLES.
- (2) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ AND RBDWEQ) DO NOT INCLUDE THE AMPLIFICATION FACTOR, q_s .
- (3) FOR CANADA BUILDING CODE (NBC), WHEN X-BRACING IS PRESENT IN THE SIDEWALL OR ENDWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ & RBDWEQ) ARE MULTIPLIED BY FORCE REDUCTION FACTOR, R_d , WHEN SPECIFIED SHORT-PERIOD SPECTRAL ACCELERATION RATIO ($f_s \leq 0.2$) IS GREATER THAN 0.45.
- 3) REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.
- a) FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC, 2015 IBC, OR FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH VALUE WITH A LOAD FACTOR OF 1.0.
- b) FOR IBC CODES, THE SEISMIC REACTIONS PROVIDED ARE AT A STRENGTH LEVEL AND DO NOT CONTAIN THE RHO FACTOR.
- c) FOR NBCC CODES, THE SEISMIC REACTIONS PROVIDED DO NOT CONTAIN THE $R_d \times R_o$ FACTOR.
- THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

By	Ck'd	Description											
MAC	JAQ	NO CHANGES											

4704 W. Commercial Dr. Ste. B
North Little Rock, AR 72116-8040
800-643-5555

HERITAGE
BUILDING SYSTEMS.

Customer:
DOUG MOSLEY
362 SW MCCLINTON DR
FORT WHITE FL 32038
DOUG MOSLEY

Project Name & Location:
DOUG MOSLEY
FORT WHITE FL 32038

Drawing Status:

☐ Preliminary
(Not For Construction)

☐ For Approval
(Not For Construction)

☒ For Construction Permit
(Not For Construction)

☐ For Erector Installation
(Not For Construction)

REVISED

Scale: NOT TO SCALE

Drawn by: FEM 4/28/23

Checked by: JAQ 5/9/23

Project Engineer: MTS

Job Number: 19-B-33112-1

Sheet Number: F6 of 6

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

M.W. CUSTER, P.E.
FLORIDA P.E. #0052825

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Aug 06, 2024

DETAILS SUBJECT TO CHANGE BASED ON FINAL DESIGN.
ISSUED FOR CONSTRUCTION DRAWINGS WILL REPLACE THE PERMIT DRAWINGS.

Builder/Contractor Responsibilities

Drawing Validity – These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

Builder Acceptance of Drawings – Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and quality criteria standards and tolerances. (AISC COSP April 2010 Section 4.4.1)

Code Official Approval – It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

Building Erection – The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector (AISC COSP April 2010 Section 7.10.3).

Discrepancies – Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (AISC COSP April 2010 Section 3.3)

Materials by Others – All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturers assumptions will govern.

Modification of the Metal Building from Plans – The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

Foundation Design
The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 06 Sections 3.2.2 and A3)

Shimming – "In accordance with Section 6.10 of Chapter 4 Common Industry Practices in the Metal Building Systems Manual, shimming is a normal part of erection and is not subject to claim."

HERITAGE
BUILDING SYSTEMS.

4704 W. Commercial Dr. Ste. B

North Little Rock, AR 72116-8040

800-643-5555

For questions or assistance
Concerning Erection call or Email:

1-844-840-4603

Monday - Friday 7:30am to 5:00pm

FIELD.SERVICES@CORNERSTONE-BB.COM

ENGINEERING DESIGN CRITERIA



Building Code FLORIDA BUILDING CODE, 8TH EDITION (2023)
Building Risk Category Normal (Risk Category II)
Roof Dead Load 2.18 psf (Bldg A)
Superimposed 2.33 psf (Bldg B)
Collateral 1.00 psf
(1.00 psf Other)
Roof Live Load 20.00 psf no reduction
Wind Ultimate Wind Speed (Vult) ... 120.00 mph
Nominal Wind Speed (Vaso) ... 92 mph (IBC section 1609.3.1)
Serviceability Wind Speed ... 76 mph
Ground Elevation Factor 1.00 (0 ft ASL)
Wind Exposure Category C
Exposure Coefficient (MWFRS) ... 0.849
Enclosure Classification ... Enclosed Building (Bldg A)
Partially Enclosed Building (Bldg B)
Internal Pressure Coef (GCp): 0.18/-0.18 (Bldg A From FL-2 to FL-6)
0.55/-0.55 (Bldg A From FL-1 to FL-2)
0.55/-0.55 (Bldg B)
Wall Loads for components not provided by building manufacturer
Zone 5 Areas (within 5.60' of corner) : 28.72 psf pressure -38.30 psf suction
Zone 4 Areas (away from corners) : 28.72 psf pressure -31.12 psf suction
These values are the maximum values required based on a 10 sq ft area.
Components with larger areas may have lower wind loads.

DEFLECTION CRITERIA

The material supplied by the manufacturer has been designed with the following minimum deflection criteria. The actual deflection may be less depending on actual load and actual member length.

BUILDING DEFLECTION LIMITS BLDG-A / BLDG-B				
Roof Limits		Rafters	Purlins	Panels
-----		-----	-----	-----
	Live: L/	180	180	60
Serviceability Wind:	L/	180	180	60
Total Gravity:	L/	120	120	60
Total Uplift:	L/	N/A	N/A	60
Frame Limits		Sidesway	Portal Frame Sidesway	
-----		-----	-----	
	Live: H/	60		
Serviceability Wind:	H/	60		
Portal Serviceability Wind:	H/	N/A	60	
Total Gravity:	H/	60		
Wall Limits		Limit		
-----		-----		
Total Wind Panels:	L/	60		
Total Wind Girts:	L/	90		
Total Wind EW Columns:	L/	120		

PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, or ASTM A1011 with 55 ksi min. yield, except flanges wider than 12" and thicker than 3/8", all flanges thicker than 1", and all webs thicker than 3/8" are 50 ksi min. yield. Rod X-bracing conforms to ASTM A529 or ASTM A572 with 50 ksi min. yield. Cable X-bracing conforms to ASTM A475 7 Strand Extra High-Strength grade. Hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with 50 ksi min. yield. Hot rolled angles, other than flange braces, conform to ASTM A36 minimum. Round and rectangular HSS conforms to ASTM A500 Grade B. Cold-Formed steel secondary framing Members conform to ASTM A1011 or ASTM A653 Grade 55 with 55 ksi min. yield. For Canada, material properties conform to CAN/CSA G40.20/G40.21 or equivalent.

Unless otherwise noted, special inspection of fabricated items is not required. Per IBC section 1704.2.5.1, fabricator is approved to perform such work without special inspection through maintenance of IAS AC 472 certification MB-136.

All bolted joints with A325 Type 1 bolts are specified as snug-tightened joints in accordance with the most recent edition of the RCSC Specification for Structural Joints Using ASTM A325 or A490 Bolts. Pre-tensioning methods, including turn-of-nut, calibrated wrench, twist-off-type tension-control bolts or direct-tension-indicator are NOT required. Installation inspection requirements for Snug Tight Bolts (Specification for Structural Joints Section 9.1) is suggested.

Design criteria as noted is as given within order documents and is applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the metal building manufacturer nor the certifying engineer declares or attests that the loads as designated are proper for local provisions that may apply or for site specific parameters. The design criteria is supplied by the builder, project owner, or an Architect and/or Engineer of Record for the overall construction project.

Building A is designed as an Enclosed Building. Exterior and/or operable components including, but not limited to, doors, windows, vents, etc. ("Components") must be designed to withstand the required component and cladding wind pressures specified by the building code. In order to maintain the metal building system's Enclosed Building condition, all Components shall be closed when wind velocities reach half the designed wind load for the metal building system as shown on the drawings and design criteria documentation. Failure to maintain the metal building system's Enclosed Building condition will violate and void all warranties and certifications applicable to the material supplied by the metal building manufacturer.

The framing at building A, gridline 1 and building B, gridline 1 is NOT designed to receive a future bay addition. Corresponding frame reactions are calculated based upon actual tributary area.

Framed openings, walk doors, and open areas shall be located in the bay and elevation as shown in the erection drawings. The cutting or removal of girts shown on the erection drawings due to the addition of framed openings, walk doors, or open areas not shown may void the design certifications supplied by the metal building manufacturer.

Roof and wall panels have been designed in accordance with section 2222.4 of the Florida Building Code, 7th Edition. Product approval numbers for the State of Florida, Department of Community Affairs per Product Rule 9B-72:

- Panel Walls
FL11917 PBR 26 gauge walls
- Roofing Products
FL11819 PBR 26 gauge roofs
- Walk doors
FL17900.1 Telstar 3070, Wind-rated to +/- 50 psf, Impact-rate

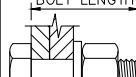


Download panel installation manuals from:
www.CBRmanuals.com

Descargue los manuales de instalación del panel desde:
www.CBRmanuals.com

BUILDING DESCRIPTIONS				
Building ID	Width	Length	Height	Slope
Building A	100'-0	116'-0	14'-0	2:12
Building B	16'-0	116'-0	11'-4	2:12

1"Ø A325 BOLT GRIP TABLE (UNLESS NOTED)			
GRIP		LENGTH	
0 TO 9/16"		1 1/4" F.T.	
Over 9/16"	TO 1 1/16"	1 3/4" F.T.	
Over 1 1/16"	TO 1 5/16"	2"	
Over 1 5/16"	TO 1 9/16"	2 1/4"	
Over 1 9/16"	TO 1 13/16"	2 1/2"	
Over 1 13/16"	TO 2 1/16"	2 3/4"	
LOCATIONS OF BOLTS LONGER THAN 2 3/4" NOTED ON ERECTION DRAWINGS			
F.T. DENOTES FULLY THREADED			

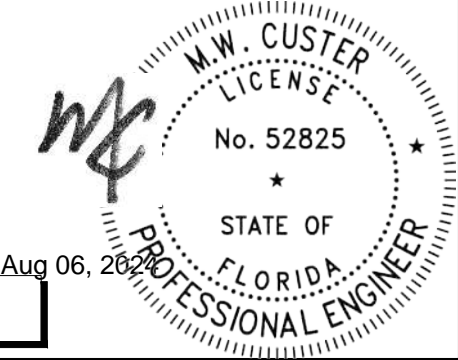


NOTE: FULL THREAD ENGAGEMENT IS DEEMED TO HAVE BEEN MET WHEN THE END OF THE BOLT IS FLUSH WITH THE FACE OF THE NUT.

WASHER REQUIRED ONLY WHEN SPECIFIED. WASHER MAY BE LOCATED UNDER HEAD OF BOLT, UNDER NUT, OR AT BOTH AT LOCATIONS NOTED ON ERECTION DRAWINGS. ADD 5/32" FOR EACH WASHER TO MATERIAL THICKNESS TO DETERMINE GRIP.



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Drawing Index		Ck'd	By	Description	JAQ	MAC	07/30/24	UPDATED	THRU	CO#6	Date
Page	Description										
F1	Anchor Rod										
F2	Anchor Rod Details										
F3-F5	Reaction Drawings										
E1	Cover Sheet										
E2	Primary Steel BLDGA										
E3	Roof Framing BLDGA										
E4	Roof Sheeting										
E5	Sidewall BLDGA WALLSWA										
E6	Sidewall BLDGA WALLSWC BLDGB WALLSWC										
E7	Endwall BLDG A&B WALLEWB										
E8	Endwall BLDGA&B WALLEWD										
E9	Partition BLDGA WALLPL1										
E10-E15	Main Frame Cross Sections										
E16	Portal Frame Cross Section 13 FRAMELINEA-SWA										
E17	Portal Frame Cross Section 13 FRAMELINEB-SWC										
E18	Portal Frame Cross Section 23 FRAMELINEA-SWC										

4704 W. Commercial Dr. Ste. B
North Little Rock, AR 72116-8040
800-643-5555

Project Name & Location:
DOUG MOSLEY
FORT WHITE FL 32038

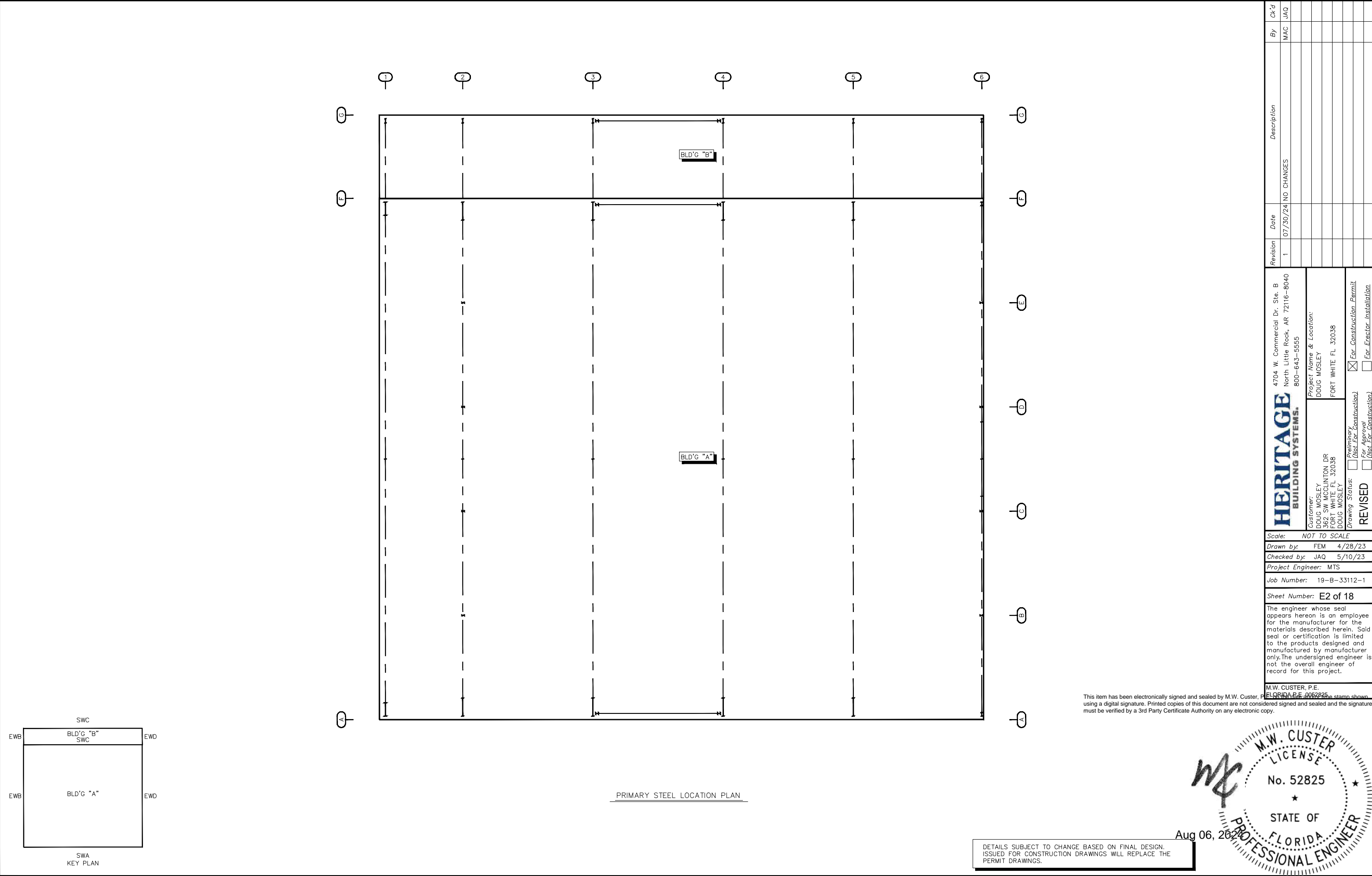
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DOUG MOSLEY

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☐ For Erector Installation

REVISED

Scale: NOT TO SCALE
Drawn by: FEM 4/28/23
Checked by: JAQ 5/10/23
Project Engineer: MTS
Job Number: 19-B-33112-1
Sheet Number: E1 of 18
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M.W. CUSTER, P.E.
FLORIDA P.E. #052825



Revision	Date	Description	By	Ch'd
1	07/30/24	NO CHANGES	MAC	JAQ

HERITAGE
BUILDING SYSTEMS.

4704 W. Commercial Dr. Ste. B
North Little Rock, AR 72116-8040
800-643-5555

Customer:
DOUG MOSLEY
362 SW MCCLINTON DR
FORT WHITE FL 32038
DOUG MOSLEY

Project Name & Location:
DOUG MOSLEY
FORT WHITE FL 32038

Drawing Status:
☐ Preliminary
☐ For Approval
☒ For Construction
☐ For Erector Installation
REVISED

Scale: NOT TO SCALE

Drawn by: FEM 4/28/23

Checked by: JAQ 5/10/23

Project Engineer: MTS

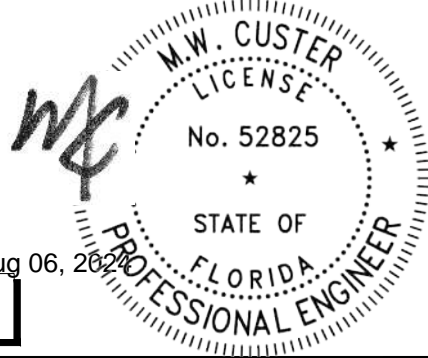
Job Number: 19-B-33112-1

Sheet Number: E2 of 18

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FLORIDA P.E. #052825






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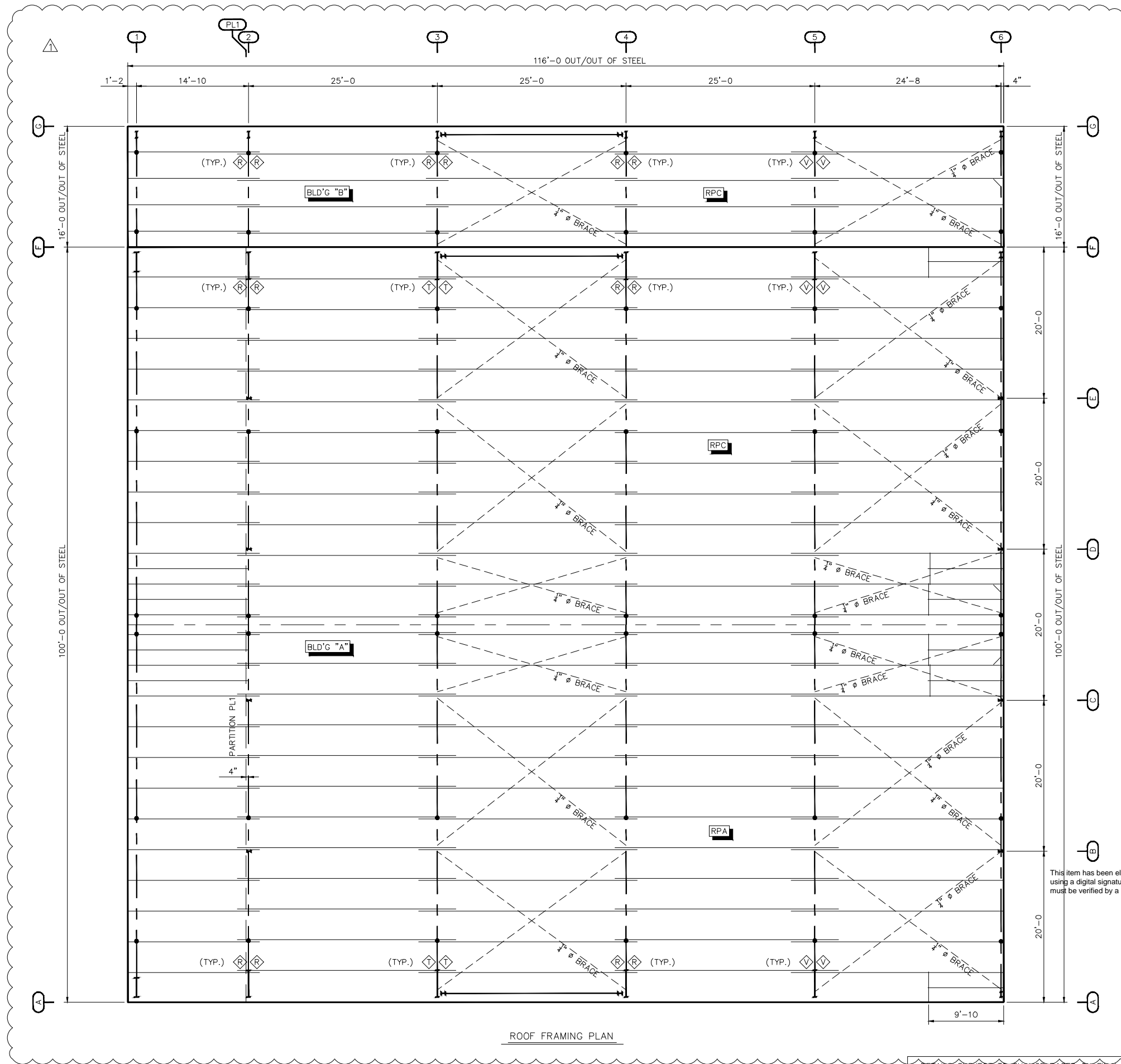
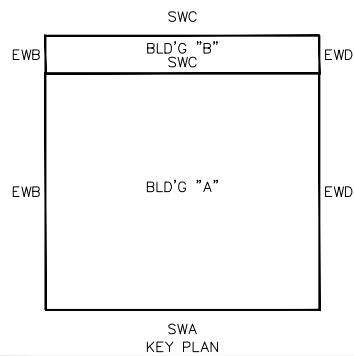


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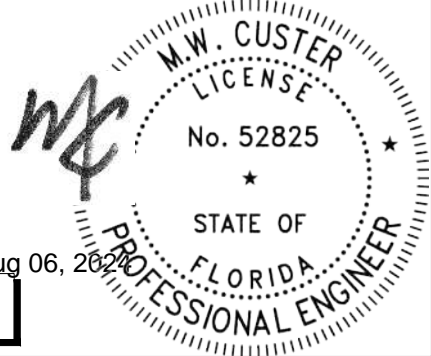
DETAILS SUBJECT TO CHANGE BASED ON FINAL DESIGN.
ISSUED FOR CONSTRUCTION DRAWINGS WILL REPLACE THE
PERMIT DRAWINGS.

- - DENOTES: CLIP LOCATION
SC90 AT 8" PURLINS
SC92 AT 10" PURLINS
SC94 AT 12" PURLINS

ZEE SECTION LAP TABLE			
SYMBOL	LAP LENGTH	SYMBOL	LAP LENGTH
	0'-0 1/4"		2'-5 1/2"
	0'-3 3/4"		3'-1 1/2"
	1'-5 1/2"	REFER TO CF01122	



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[illegible]

HERITAGE BUILDING SYSTEMS.		4704 W. Commercial Dr. Ste. B North Little Rock, AR 72116-8040 800-643-5555	
Customer: DOUG MOSLEY 362 SW MCCLINTON DR FORT WHITE FL 32038 DOUG MOSLEY		Project Name & Location: DOUG MOSLEY FORT WHITE FL 32038	
Drawing Status:		<input type="checkbox"/> Preliminary <input checked="" type="checkbox"/> Not For Construction.	
REVISED		<input type="checkbox"/> For Approval <input type="checkbox"/> Not For Construction.	
		<input checked="" type="checkbox"/> For Construction Permit <input type="checkbox"/> For Erector Installation.	

Scale:	NOT TO SCALE	
Drawn by:	FEM	4/28/23
Checked by:	JAQ	5/10/23
Project Engineer:	MTS	
Job Number:	19-B-33112-1	
Sheet Number:	E3 of 18	

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FLORIDA P.E. 0052825

#58 member fasteners are to be used for panel to secondary attachment in lieu of #3 shown on the R Drawings

1

ROOF SHEETING PLAN

1'-0 BEYOND

Aug 06, 2024

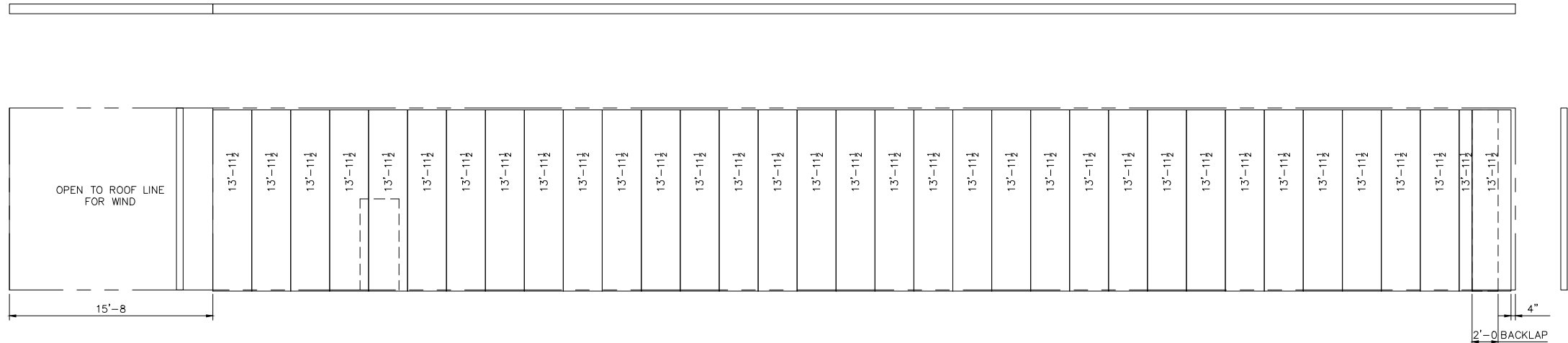
DETAILS SUBJECT TO CHANGE BASED ON FINAL DESIGN.
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#58 member fasteners are to be used for panel to secondary attachment in lieu of #17A shown on the R Drawings

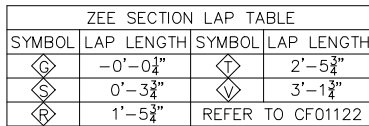
#4 lap fasteners are to be used for panel to panel and panel to trim attachment in lieu of #4A shown on the R Drawings

NO. REQD	DESCRIPTION
1	6'-0 X 7'-0 FIELD LOCATED FRAMED OPENING
2	5'-0 X 3'-0 FACTORY LOCATED FRAMED OPENINGS
2	12'-0 X 12'-0 FACTORY LOCATED FRAMED OPENINGS
2	3'-0 X 4'-0 FIELD LOCATED FRAMED OPENINGS
2	3070 KNOCK-DOWN WALK DOORS

REFER TO DETAILS ON INSTALLATION OF WALK DOORS.
REFER TO DETAILS ON INSTALLATION OF FRAMED OPENINGS.
USE STANDARD WALL PROCEDURES TO ERECT THE SIDEWALL AND ENDWALL PANELS.



WALL SHEETING ELEVATION "SWA"
BLDG "A"



2'-0 BACKLAP

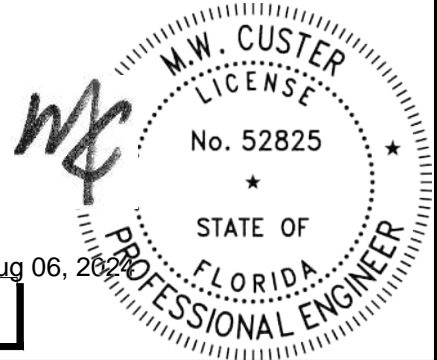
M.W. CUSTER, P.E.

FLORIDA P.E. NO. 528235

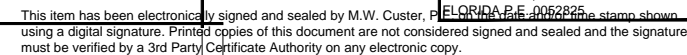
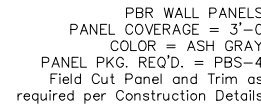
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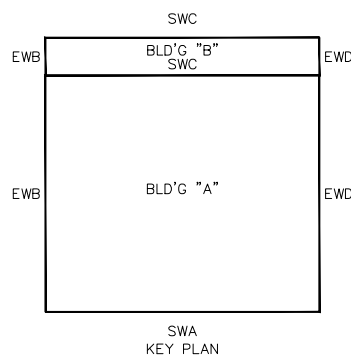
Aug 06, 2024



#4 lap fasteners are to be used for panel to panel and panel to trim attachment in lieu of #4A shown on the R Drawings

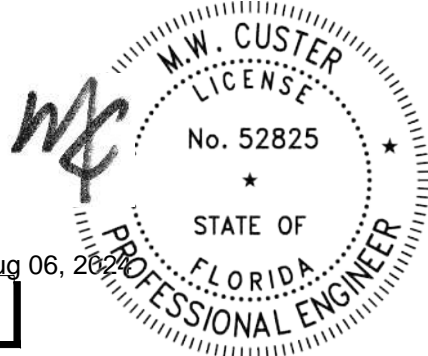


ZEE SECTION LAP TABLE			
SYMBOL	LAP LENGTH	SYMBOL	LAP LENGTH
	0'-0 1/4"		2'-5 3/4"
	0'-3 1/4"		3'-1 3/4"
	1'-5 1/2"	REFER TO CF01122	

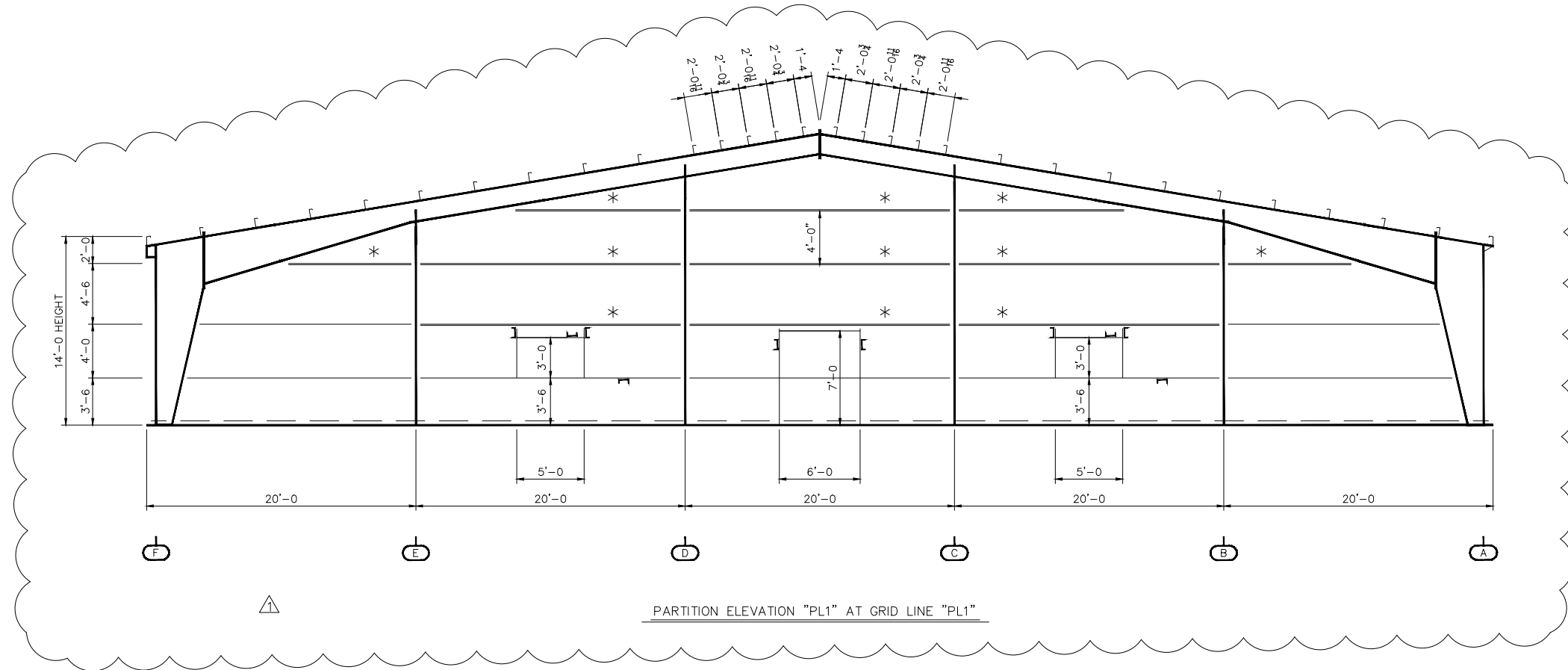


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Aug 06, 2022



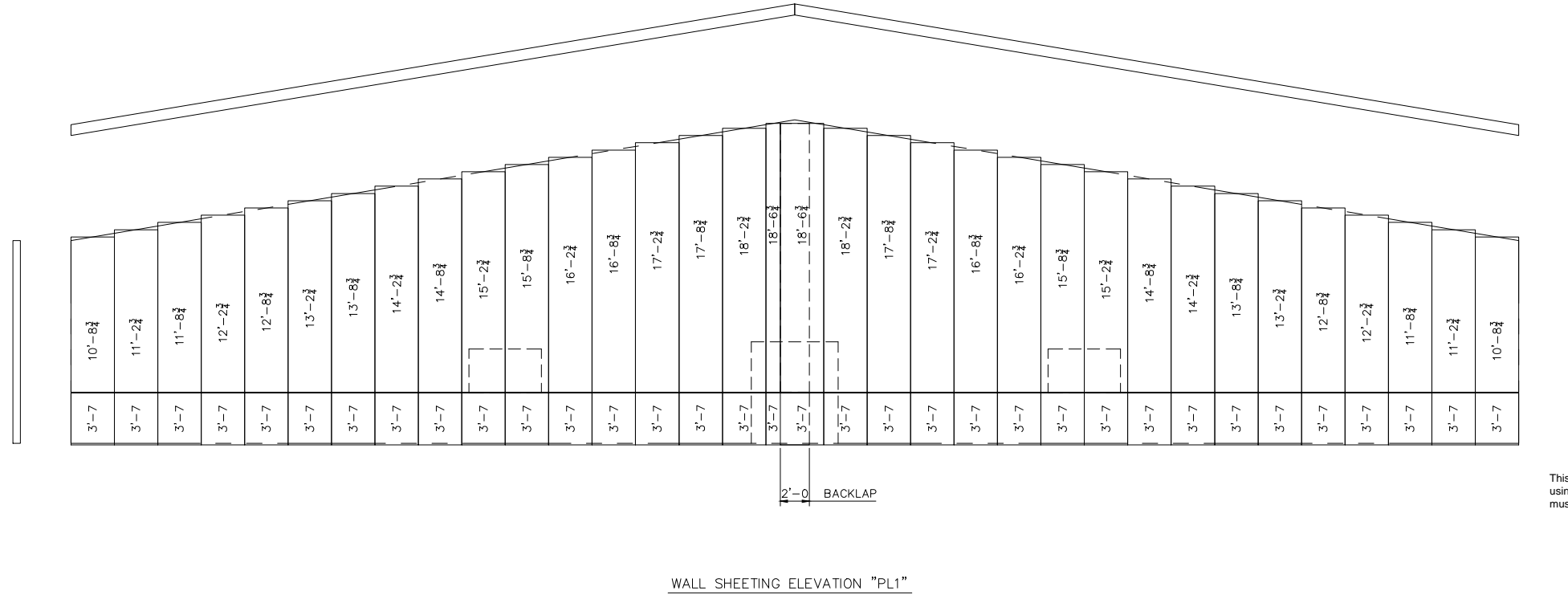
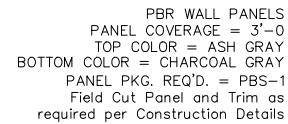
* DOUBLE GIRT



Non-Standard PBR Wall Panel Fasteners

#58 member fasteners are to be used for panel to secondary attachment in lieu of #17A shown on the R Drawings

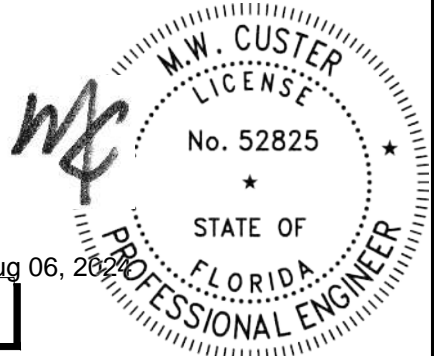
#4 lap fasteners are to be used for panel to panel and panel to trim attachment in lieu of #4A shown on the R Drawings




NOTE: FIELD BEVEL CUT WALL PANELS AT
ENDWALLS TO FOLLOW ROOF SLOPE.

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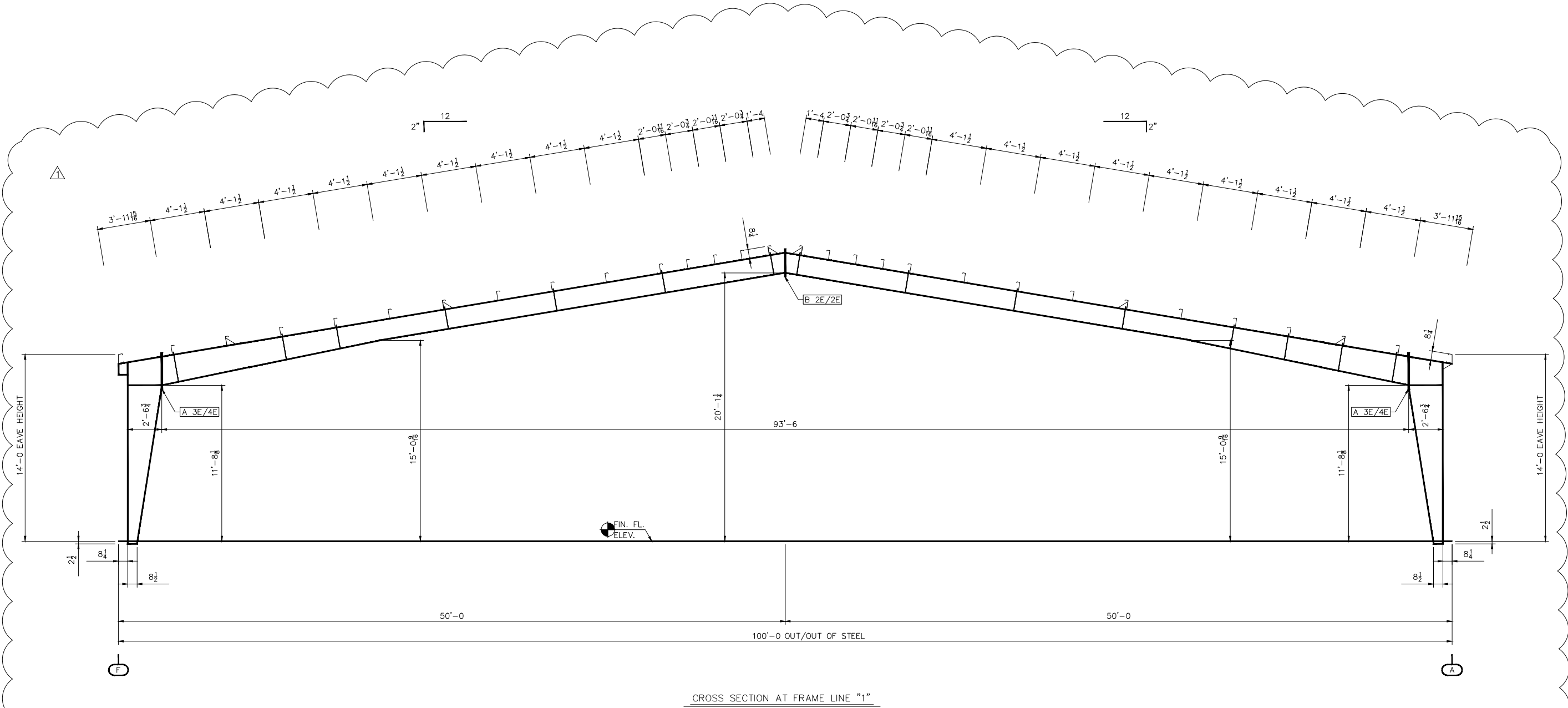
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	Customer: DOUG MOSLEY 362 SW MCCLINTON DR FORT WHITE FL 32038 DOUG MOSLEY		Project Name & Location: DOUG MOSLEY FORT WHITE FL 32038		4704 W. Commercial Dr. Ste. B North Little Rock, AR 72116-8040 800-643-5555		Revision	Date	Description	By	Ck'd
							1	07/30/24	REDRAWN THRU CO#6	MAC	JAQ
Drawing Status: <input type="checkbox"/> Preliminary <input type="checkbox"/> For Approval <input type="checkbox"/> For Approval (Not For Construction)		<input checked="" type="checkbox"/> For Construction Permit <input type="checkbox"/> For Erector Installation		REVISED							

GENERAL NOTES
FRAME CLEARANCES SHOWN ARE APPROXIMATE AND
MAY VARY DUE TO CONDITIONS (DEFLECTION).

VERTICAL CLEARANCE DIMENSIONS ARE FROM
FINISHED FLOOR REFERENCE ELEVATION.



SPlice Bolt Table				
CONN.	QTY.	SIZE	TYPE	HARDENED BEVELED WASHERS WASHERS
A	(14)	3/4" X 2 1/4"	A325 B&N	0 0
B	(8)	3/4" X 1 1/2"	A325 B&N	0 0

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Revision	Date	Description	By	Ch'd
1	07/30/24	UPDATED REACTION & REDRAWN THRU CO#6	MAC	JAQ

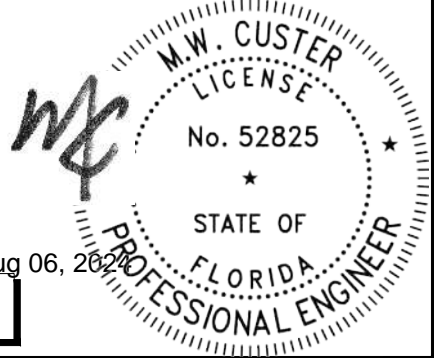
4704 W. Commercial Dr. Ste. B North Little Rock, AR 72116-8040 800-643-5555	
HERITAGE BUILDING SYSTEMS.	Project Name & Location: DOUG MOSLEY FORT WHITE FL 32038
Customer: DOUG MOSLEY 362 SW MCCLINTON DR FORT WHITE FL 32038 DOUG MOSLEY	Drawing Status: <input type="checkbox"/> Preliminary <input type="checkbox"/> For Construction <input checked="" type="checkbox"/> For Construction Permit <input type="checkbox"/> For Erector Installation
REVISED	

Scale: NOT TO SCALE
Drawn by: FEM 4/28/23
Checked by: JAQ 5/10/23
Project Engineer: MTS
Job Number: 19-B-33112-1
Sheet Number: E10 of 18

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FLORIDA P.E. #0052825

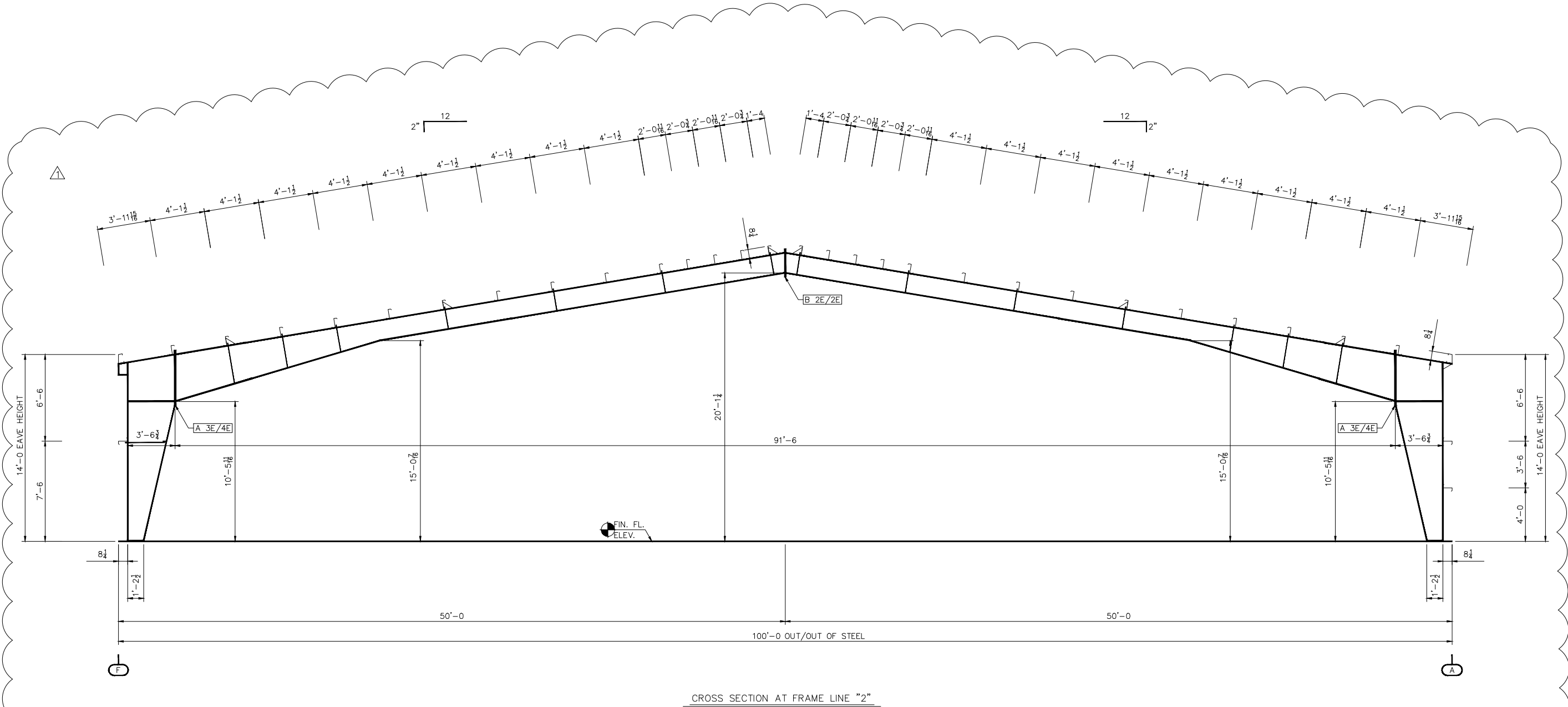
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SPlice Bolt Table				
CONN.	QTY.	SIZE	TYPE	HARDENED BEVELED WASHERS
A	(16)	3/4 X 2 1/4	A325 B&N	0
B	(8)	3/4 X 1 3/4	A325 B&N	0

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Revision	Date	Description	By	Ch'd
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HERITAGE
BUILDING SYSTEMS.

4704 W. Commercial Dr. Ste. B
North Little Rock, AR 72116-8040
800-643-5555

Customer:
DOUG MOSLEY
362 SW MCCLINTON DR
FORT WHITE FL 32038
DOUG MOSLEY

Project Name & Location:
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FORT WHITE FL 32038

Drawing Status:
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☒ For Construction Permit
☐ For Erector Installation

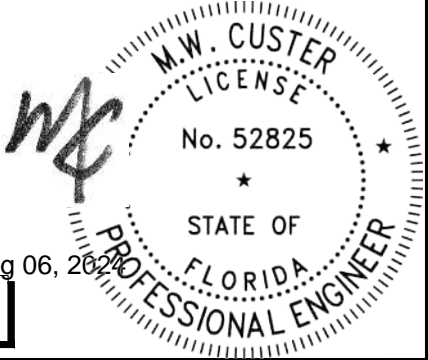
REVISED

Scale: NOT TO SCALE
Drawn by: FEM 4/28/23
Checked by: JQA 5/10/23
Project Engineer: MTS
Job Number: 19-B-33112-1
Sheet Number: E11 of 18

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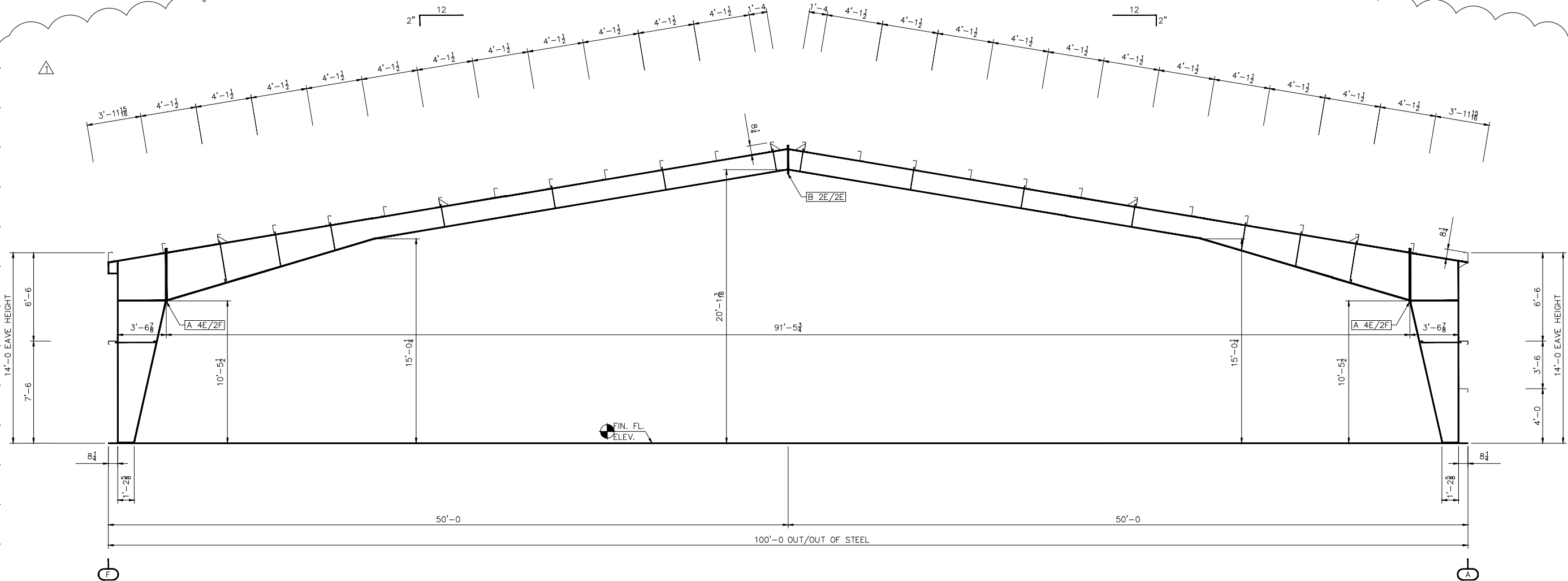
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CROSS SECTION AT FRAME LINE "3"

SPlice Bolt Table				
CONN.	QTY.	SIZE	TYPE	HARDENED BEVELED WASHERS
A	(14)	3/4 X 2 1/4	A325 B&N	0
B	(8)	3/4 X 1 1/2	A325 B&N	0

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Revision	Date	Description
1	07/30/24	UPDATED REACTION & REDRAWN THRU CO#6

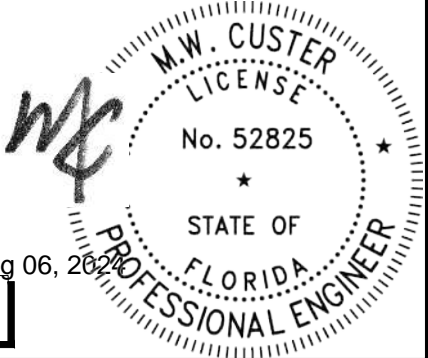
4704 W. Commercial Dr. Ste. B North Little Rock, AR 72116-8040 800-643-5555	Project Name & Location: DOUG MOSLEY FORT WHITE FL 32038	Customer: DOUG MOSLEY 362 SW MCCLINTON DR FORT WHITE FL 32038 DOUG MOSLEY	Drawing Status: <input type="checkbox"/> Preliminary <input type="checkbox"/> For Approval <input checked="" type="checkbox"/> For Construction Permit <input type="checkbox"/> For Erector Installation
---	--	---	--

Scale:	NOT TO SCALE
Drawn by:	FEM 4/28/23
Checked by:	JAQ 5/10/23
Project Engineer:	MTS
Job Number:	19-B-33112-1
Sheet Number:	E12 of 18

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M.W. CUSTER, P.E.
FLORIDA P.E. 52825

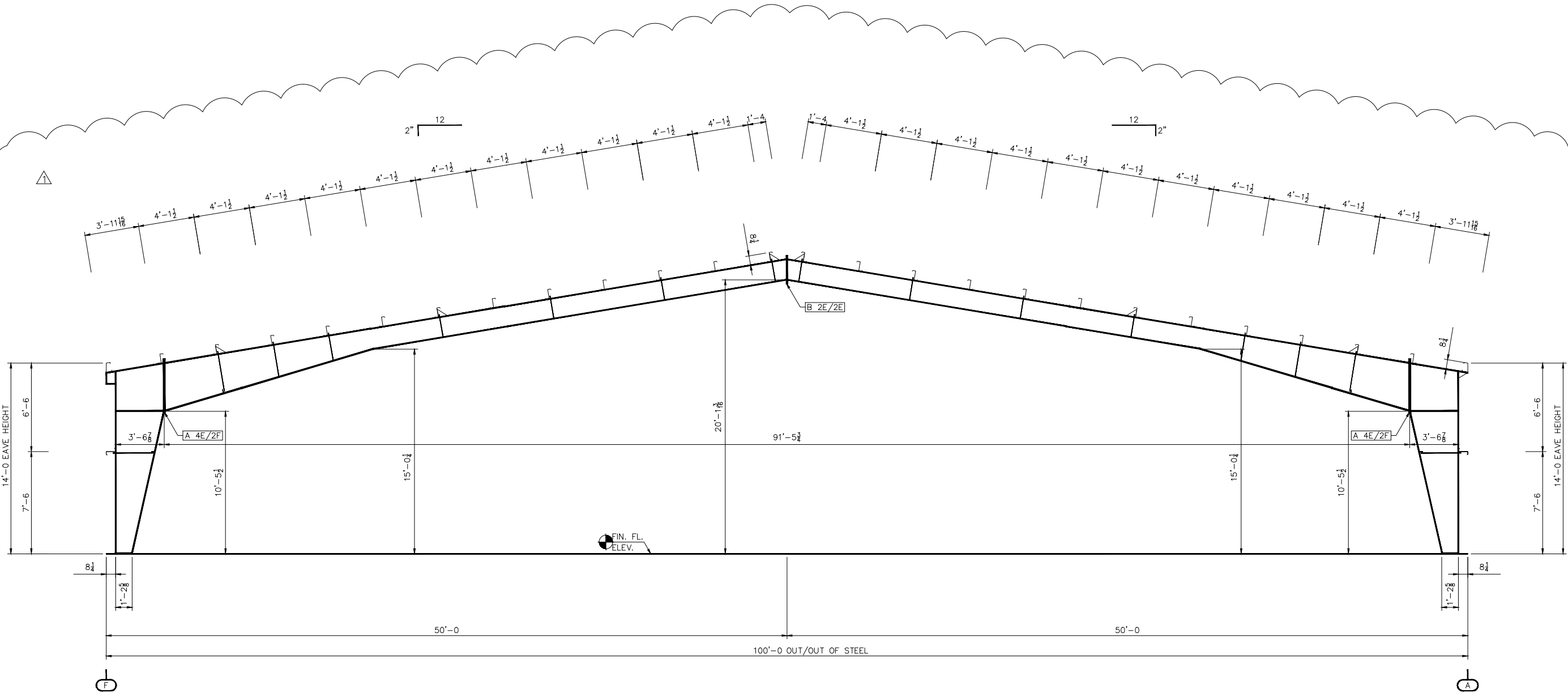
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GENERAL NOTES
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CROSS SECTION AT FRAME LINE "4"

SPlice Bolt Table				
CONN.	QTY.	SIZE	TYPE	HARDENED BEVELED WASHERS
A	(14)	3/4 X 2 1/4	A325 B&N	0
B	(8)	3/4 X 1 1/2	A325 B&N	0

DETAILS SUBJECT TO CHANGE BASED ON FINAL DESIGN.
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Revision	Date	Description	By	Ch'd
1	07/30/24	UPDATED REACTION & REDRAWN THRU CO#6	MAC	JAQ

4704 W. Commercial Dr. Ste. B
North Little Rock, AR 72116-8040
800-643-5555

HERITAGE
BUILDING SYSTEMS.

Customer:
DOUG MOSLEY
362 SW MCCLINTON DR
FORT WHITE FL 32038
DOUG MOSLEY

Project Name & Location:
DOUG MOSLEY
FORT WHITE FL 32038

Drawing Status:
☐ Preliminary
☐ For Approval
☒ For Construction Permit
☐ For Erector Installation

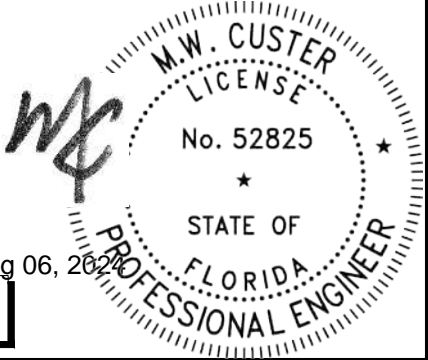
REVISED

Scale: NOT TO SCALE
Drawn by: FEM 4/28/23
Checked by: JAQ 5/10/23
Project Engineer: MTS
Job Number: 19-B-33112-1
Sheet Number: E13 of 18

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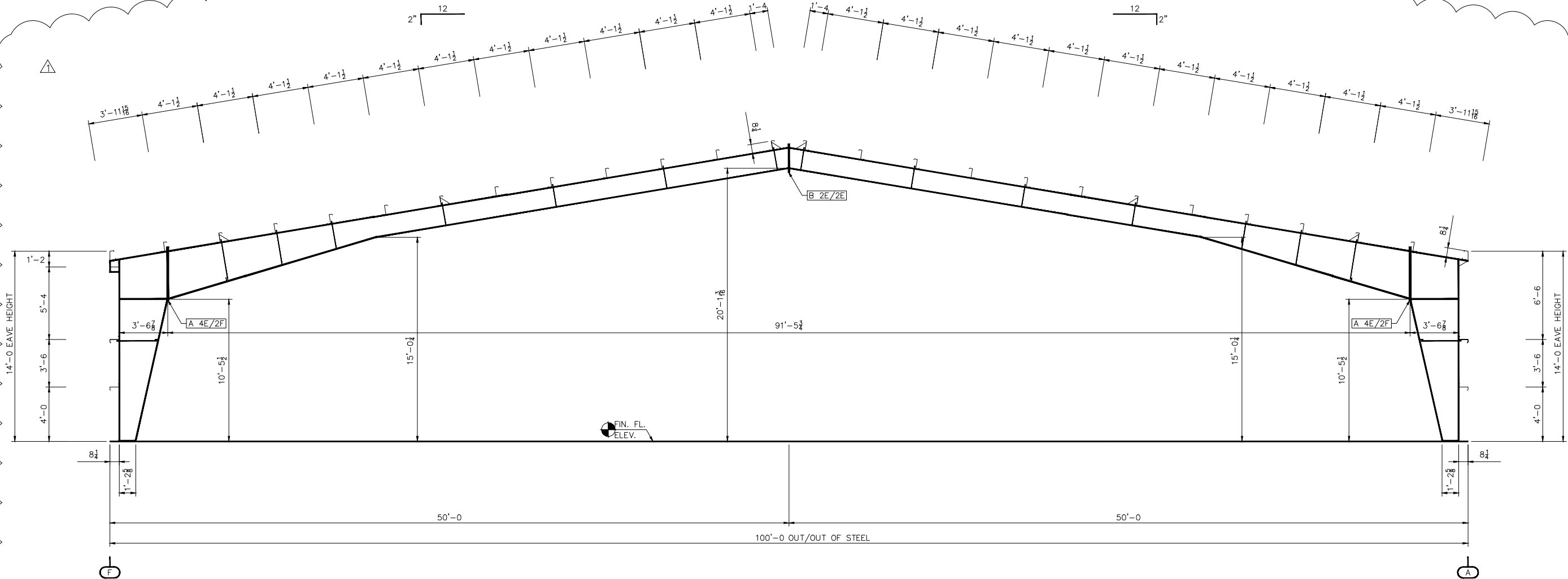
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GENERAL NOTES
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CROSS SECTION AT FRAME LINE "5"

SPlice Bolt Table				
CONN.	QTY.	SIZE	TYPE	HARDENED BEVELED WASHERS WASHERS
A	(14)	3/4 X 2 1/4	A325 B&N	0 0
B	(8)	3/4 X 1 3/4	A325 B&N	0 0

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Revision	Date	Description	By	Ch'd
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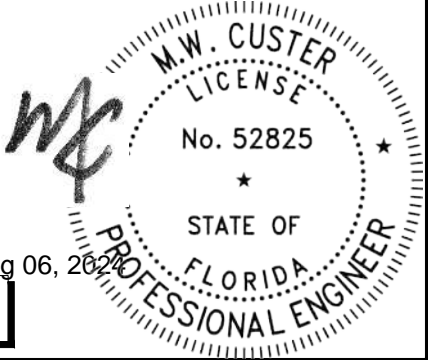
4704 W. Commercial Dr. Ste. B North Little Rock, AR 72116-8040 800-643-5555	
HERITAGE BUILDING SYSTEMS.	Project Name & Location: DOUG MOSLEY FORT WHITE FL 32038
Customer: DOUG MOSLEY 362 SW MCCLINTON DR FORT WHITE FL 32038 DOUG MOSLEY	Drawing Status: <input type="checkbox"/> Preliminary <input type="checkbox"/> For Approval <input checked="" type="checkbox"/> For Construction Permit <input type="checkbox"/> For Erector Installation
REVISED	

Scale: NOT TO SCALE
Drawn by: FEM 4/28/23
Checked by: JQA 5/10/23
Project Engineer: MTS
Job Number: 19-B-33112-1
Sheet Number: E14 of 18

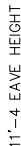
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FLORIDA P.E. #0052825

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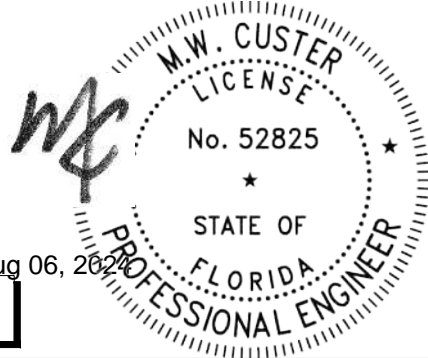
Aug 06, 2024



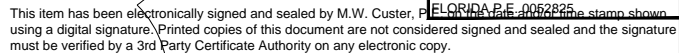
FIN. FL.
ELEV.

DETAILS SUBJECT TO CHANGE BASED ON FINAL DESIGN.
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Aug 06, 2021



VERTICAL CLEARANCE DIMENSIONS ARE FROM
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VERTICAL CLEARANCE DIMENSIONS ARE FROM
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Aug 06, 2024

