DATE 07/03/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024704

APPLICANT SUSAN FAIR PHONE 386.752.1711

ADDRESS 180 NW AMENITY COURT LAKE CITY FL 32055

OWNER CORNERSTONE DEVELOPERS PHONE 386.752.1711

ADDRESS 139 SW TIMBERLAND COURT LAKE CITY FL 32055

CONTRACTOR BRYAN ZECHER PHONE 386.752.8653

LOCATION OF PROPERTY 90-W TO HEATHRIDGE DR,TL TO TIMBERLAND CT, TR AND IT'S THE 2ND LOT ON L.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 68500.00

HEATED FLOOR AREA 1370.00 TOTAL AREA 2049.00 HEIGHT 17.11 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 33-3S-16-02438-171 SUBDIVISION EMERALD COVE

LOT 71 BLOCK PHASE 1 UNIT TOTAL ACRES 0.50

000001142 Susan Fair

Culvert Permit No. | Culvert Waiver | Contractor's License Number | Applicant/Owner/Contractor

18"X32'MITERED 06-0594-N BLK JTH

Driveway Connection | Septic Tank Number | LU & Zoning checked by | Approved for Issuance | New Resident

COMMENTS: 1 FOOT ABOVE ROAD.

Check # or Cash 2398

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by | Foundation date/app. by | Monolithic date/app. by

Under slab rough-in plumbing date/app. by | Slab date/app. by | Sheathing/Nailing date/app. by

Framing date/app. by | Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by | Heat & Air Duct date/app. by | Peri. beam (Lintel) date/app. by

Permanent power date/app. by | C.O. Final date/app. by | Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by | Pool date/app. by

Reconnection date/app. by | Pump pole date/app. by | Utility Pole date/app. by

M/H Pole date/app. by | Travel Trailer date/app. by | Re-roof date/app. by

BUILDING PERMIT FEE $ 345.00 CERTIFICATION FEE $ 10.25 SURCHARGE FEE $ 10.25

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 465.50

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

PREPARED BY AND RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Inst:2005026450 Date:10/24/2005 Time:13:06
Doc Stamp-Deed : 3628.80
DC,P.DeWitt Cason,Columbia County B:1062 P:2214

Property Appraiser's 02438-000
Identification Number 02421-000

TM File No: 05-652

WARRANTY DEED

This Warranty Deed, made this 18th day of October, 2005, BETWEEN D D P CORPORATION, a Florida corporation, whose post office address is 4158 US Highway 90 West, Lake City, Florida 32055, of the County of Columbia, State of Florida, grantor, and CORNERSTONE DEVELOPERS, LLC, a Florida Limited Liability Company, whose post office address is P.O. Box 815, Lake City, Florida 32056, grantee.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth: that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

Lots 65,66,67,68,71,72,73,74,93,94,95,96,97 & 98, Emerald Cove, Phase 1, a subdivision according to the plat thereof recorded in Plat Book 8, Pages 35-36, public records, Columbia County, Florida.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

(Signature of First Witness)

(Typed Name of First Witness)

(Signature of Second Witness)

(Typed Name of Second Witness)

D D P CORPORATION

BY: _____ (SEAL)
O. P. Daughtry, III,
President

(Corporate Seal)

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 18th day of October, 2005, by O. P. Daughtry, III, President of D D P Corporation, a Florida corporation, on behalf of said corporation, who is/are personally known to me or who has/have produced _____ as identification and who did not take an oath.

My Commission Expires:

Notary Public
Printed, typed, or stamped name:

KAREN M. WRIGHT
Notary Public - State of Florida
My Commission Expires Jan 23, 2008
Commission # DD086234
Bonded By National Notary Assn.

Inst:2005026450 Date:10/24/2005 Time:13:06
Doc Stamp-Deed : 3628.80
_____DC,P.DeWitt Cason,Columbia County B:1062 P:2215

Columbia County Building Permit Application

Rev#2370 465.50

Revised 9-23-04

For Office Use Only Application # 06-06-90 Date Received 6-23-06 By CF Permit # 1142 / 24704

Application Approved by - Zoning Official BLK Date 29.06.06 Plans Examiner OKJTH Date 6-29-06

Flood Zone XP plat Development Permit N/A Zoning RSF-2 Land Use Plan Map Category RES. Low Den

Comments NOC

Applicants Name SUSAN FAIR Phone 752-1711 Fax: 487-0396

Address 180 NW AMENITY CT. LAKE CITY FL. 32055

Owners Name CORNERSTONE DEVELOPERS Phone 752-1711

911 Address 139 SW TIMBERLAND CT. LAKE CITY, FL. 32055

Contractors Name BRYAN ZECHER Phone 752-8653

Address PO BOX 815 LAKE CITY FL 32056

Fee Simple Owner Name & Address NA

Bonding Co. Name & Address NA

Architect/Engineer Name & Address MARK DISOSWAY PO BOX 848 LAKE CITY FL 32056

Mortgage Lenders Name & Address NA

Circle the correct power company – (FL Power & Light) – Clay Elec. – Suwannee Valley Elec. – Progressive Energy

Property ID Number 33-3S-16-02438-171 Estimated Cost of Construction 110,000.00

Subdivision Name EMERALD COVE PHASE 1 Lot 71 Block ____ Unit ____ Phase ____

Driving Directions TAKE 90 TO SW HEATHRIDGE DRIVE & TURN LEFT. GO TO SOUTHWEST TIMBERLAND CT. TURN RIGHT. 2ND LOT ON LEFT.

Type of Construction FRAME & BRICK Number of Existing Dwellings on Property 0

Total Acreage 36 acres Lot Size 1/4 acre Do you need a (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 35' Side 34' Side 34' Rear 104'

Total Building Height 17'11" Number of Stories 1 Heated Floor Area ~~2049~~ 1370 Roof Pitch 6/12

Porch 218 GARAGE 416 2049

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

Contractor Signature

Contractors License Number CBC054575

Competency Card Number

NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 22 day of June 20 06.

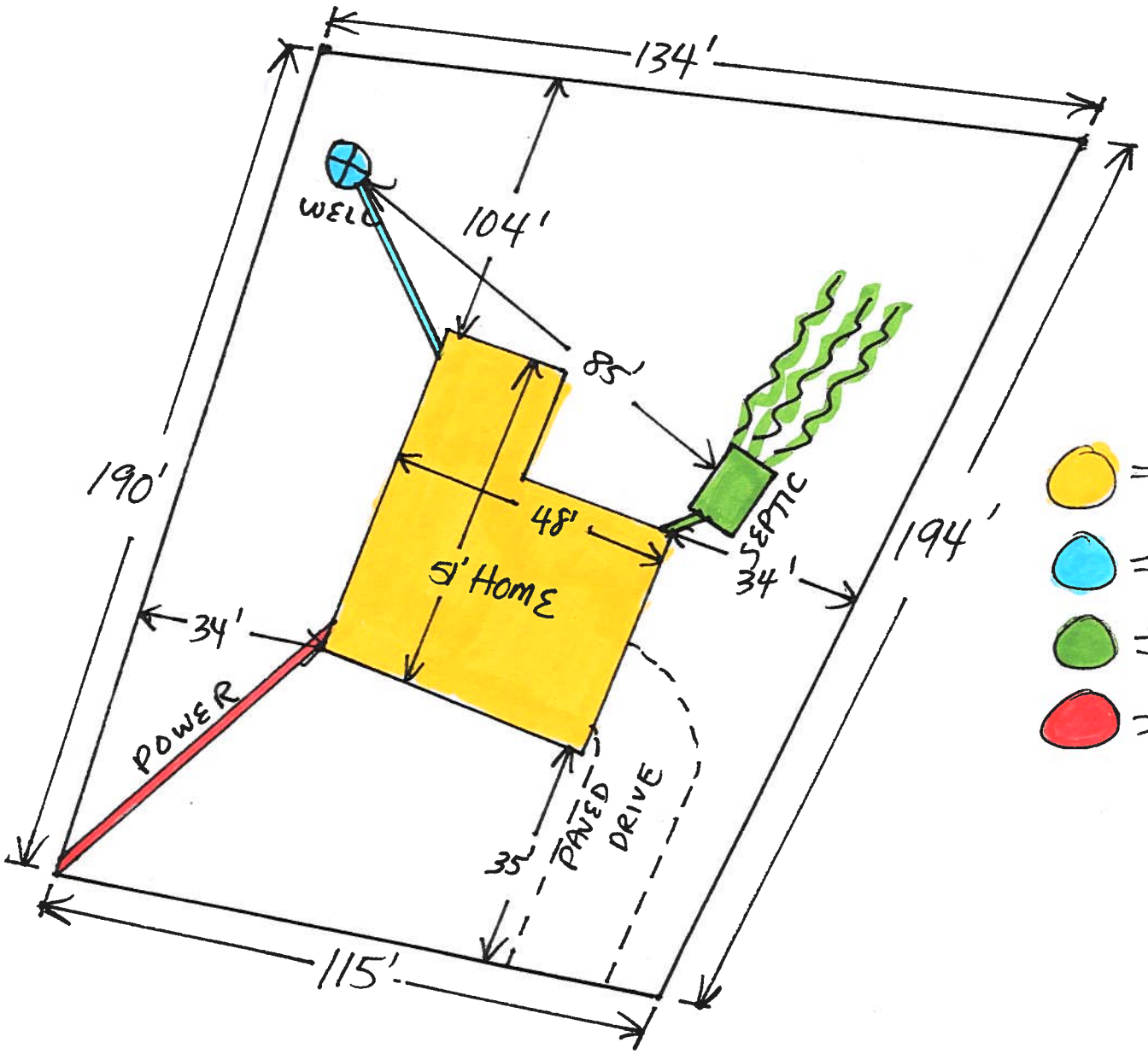
Personally known ✓ or Produced Identification ____

Notary Signature

LEIGH ANN CANNON
Notary Public - State of Florida
My Commission Expires Dec 16, 2006
Commission # DD172314
Bonded By National Notary Assn.

FROM :COLUMBIA CO BUILDING + ZONING FAX NO. :386-758-2160 Jun. 16 2006 09:00AM P2

EMERALD COVE PHASE 1
LOT 71 SITE PLAN



CODE

= HOME

= WATER

= SEPTIC

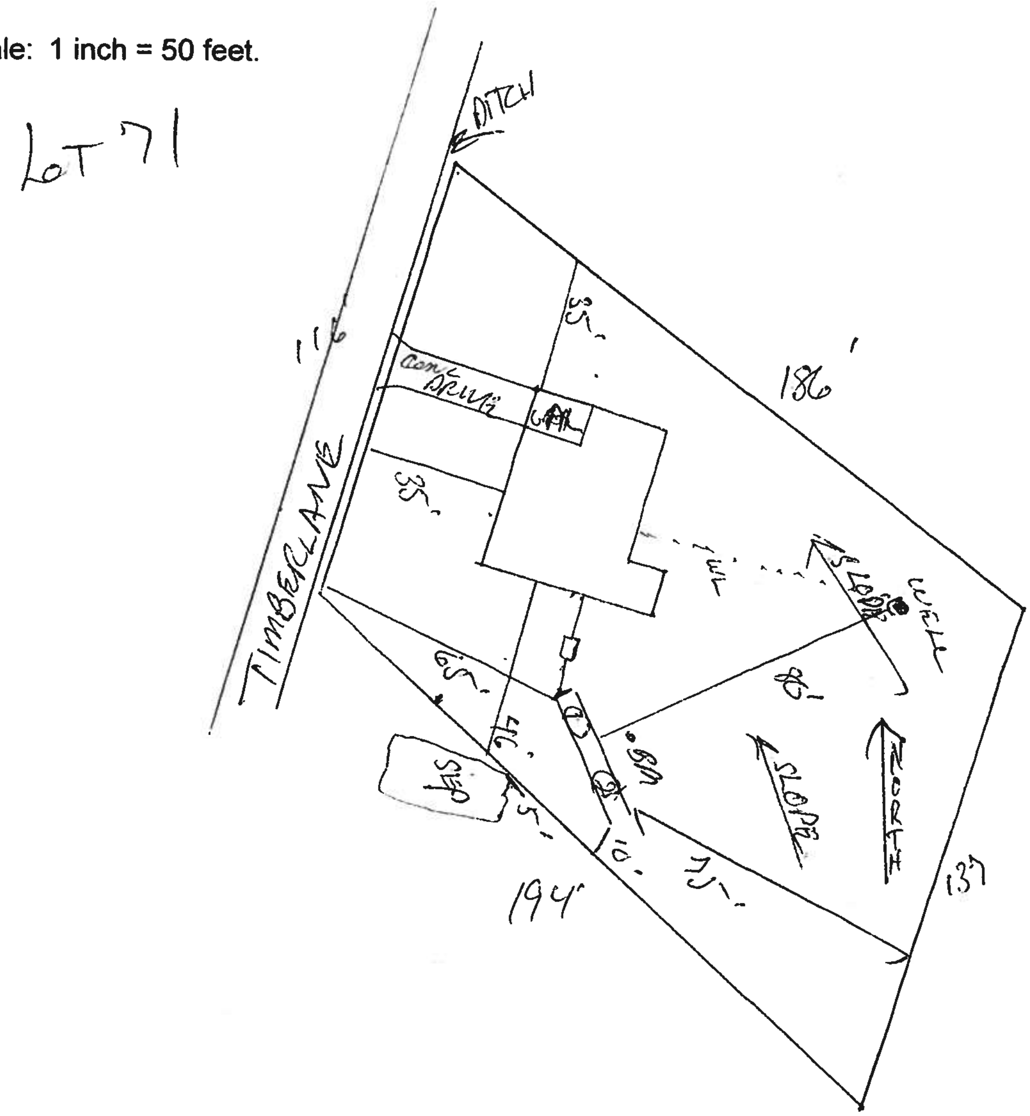
= POWER

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION F0R ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-0594N

------------------------------ PART II - SITEPLAN ------------------------------

Scale: 1 inch = 50 feet.



Notes: ____

Site Plan submitted by: Rocky D. Ford MASTER CONTRACTOR

Plan Approved ✓ Not Approved_____ Date 6/26/06

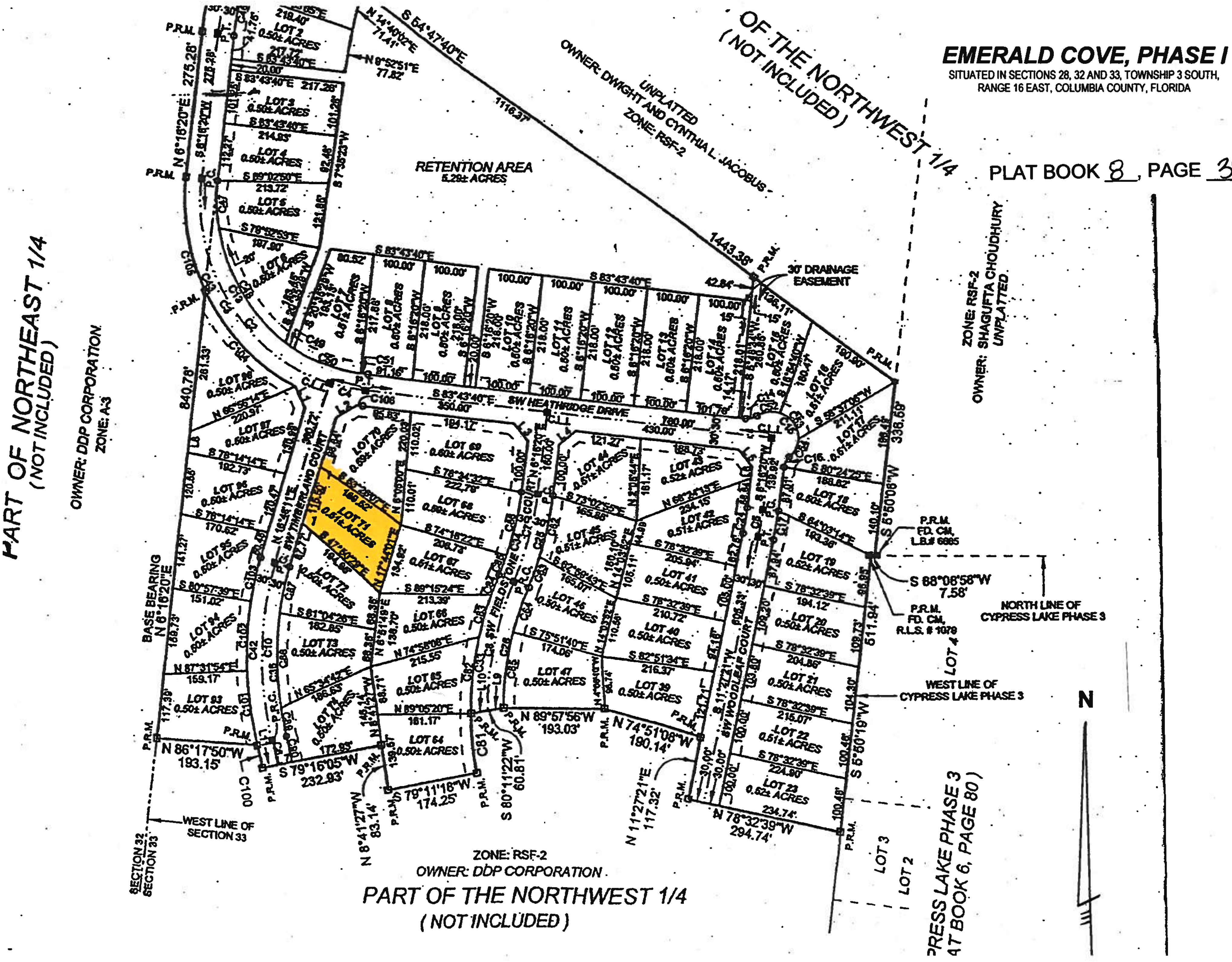
By ____________ Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

EMERALD COVE, PHASE I

SITUATED IN SECTIONS 28, 32 AND 33, TOWNSHIP 3 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA

PLAT BOOK 8, PAGE 36



1850/360 24704

Prepared by and after
recording return to:

William L. Joel
Stoneburner Berry & Simmons, P.A.
841 Prudential Drive, Suite 1400
Jacksonville, FL 32207

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing
is a true copy of the original filed in this office.
P. DeWitt CASON, CLERK OF COURTS
By _____ Deputy Clerk
Date 7/10/06

Permit No. _____
Tax Folio No. _____

Inst:200616325 Date:07/10/2006 Time:11:13
DC,P.DeWitt Cason,Columbia County B:1089 P:468

NOTICE OF COMMENCEMENT

STATE OF FLORIDA

COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvements will be made to certain real property, and in accordance with section 713 Florida Statutes, the following information is provided in this notice of commencement.

1. Description of property (legal description and address, if available):

Lot 71, Emerald Cove, Phase I, according to map or plat thereof as recorded in Plat Book 8, Pages 35 and 36, Public Records of Columbia County, Florida.

2. General description of improvements: Construction of residential dwellings

3. Owner Information:

(a) Name and Address: Cornerstone Developers, LLC, a Florida limited liability company
180 NW Amenity Court
Lake City, Florida 32025

(b) Owner's interest in the site of the improvements (if other than fee simple title holder):

(c) Name and Address of fee simple title holder (if other than owner):

4. Contractor:

(a) Name and Address: Bryan Zecher Construction Inc.
465 NW Orange Street
Lake City, FL 32055

(b) Phone No. 386-752-8653 Fax No. _____ (Optional, if service by fax is acceptable)

5. Surety on any payment bond: N/A

(a) Name and Address:
(b) Phone No. _____ Fax No. _____ (Optional, if service by fax is acceptable)
(c) Amount of bond $_____

6. Lender making loan for the construction of the improvements:

(a) Name and Address: First Horizon Home Loan Corporation
1051 Deerwood Park Boulevard
Building 200, Suite 115
Jacksonville, FL 32256
Attn: James J. O'Connor, Jr.

(b) Phone No. 904-998-5300 Fax No. _____ (Optional, if service by fax is acceptable)

7. Persons within the State of Florida designated by owner upon whom notices may be served as provided by Section 713.13(1)(a)7, Florida Statutes:

(a) Name and Address: Cornerstone Developers, LLC a Florida limited liability company
180 NW Amenity Court
Lake City, Florida 32025

(b) Phone No. 386- 752-1711 Fax No. _____ (Optional, if service by fax is acceptable)

8. In addition to himself, Owner designates the following person to receive a copy of the lienor's notice as provided in Section 713.13(1)(b), Florida Statute:

(a) Name and Address: James J. O'Connor, Jr.
First Horizon Home Loan Corporation
1051 Deerwood Park Boulevard
Building 200, Suite 115
Jacksonville, FL 32256

(b) Phone No. 904-998-5300 Fax No. _____ (Optional, if service by fax is acceptable)

9. Expiration date of notice of commencement (the expiration date is one (1) year from the date of recording unless a different date is specified):

CORNERSTONE DEVELOPERS, LLC

By: _____
Frank Soucinek, its sole Manager (SEAL)

(OWNER)

Sworn to and Subscribed before me this 21 day of June, 2006, by Frank Soucinek, the Manager of Cornerstone Developers, LLC. He is personally known or has produced _____ as identification.

Susan Ann Fair
Notary Public, State and County Aforesaid
Print Name:
My Commission Expires:
My Commission No.:
(NOTARIAL SEAL)

SUSAN ANN FAIR
MY COMMISSION # DD 342845
EXPIRES: August 14, 2008
Bonded Thru Notary Public Underwriters

POST A CERTIFIED COPY OF THE RECORDED NOTICE ON CONSTRUCTION SITE

Notice of Treatment 12170

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: BAYA Ave
City Lake City Phone 752 1703

Site Location: Subdivision Emerald Cove
Lot # 71 Block# _____ Permit # 24704
Address 139 SW Timberland CT

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☑ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☑ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling	2049	~~399~~ 616	4.5

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

10/16/06 Date | 0850 Time | F254 Gunny Print Technician's Name

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink
10/05 ©

FORM 600A-2004 EnergyGauge® 4.1

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**606025TheVictoriaModel**	Builder:	**Bryan Zecher**
Address:	**Lot: 71, Sub: Emerald Cove, Plat:**	Permitting Office:	Columbia
City, State:	**Lake City, FL**	Permit Number:	24704
Owner:	**Model Home**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	
2. Single family or multi-family	Single family	
3. Number of units, if multi-family	1	
4. Number of Bedrooms	4	
5. Is this a worst case?	Yes	
6. Conditioned floor area (ft²)	1370 ft²	
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
a. U-factor: (or Single or Double DEFAULT) 7a.	Description Area (Dble Default) 185.3 ft²	
b. SHGC: (or Clear or Tint DEFAULT) 7b.	(Clear) 185.3 ft²	
8. Floor types		
a. Slab-On-Grade Edge Insulation	R=0.0, 197.0(p) ft	
b. N/A		
c. N/A		
9. Wall types		
a. Frame, Wood, Adjacent	R=13.0, 308.0 ft²	
b. Frame, Wood, Exterior	R=13.0, 997.7 ft²	
c. N/A		
d. N/A		
e. N/A		
10. Ceiling types		
a. Under Attic	R=30.0, 1414.0 ft²	
b. N/A		
c. N/A		
11. Ducts		
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 140.0 ft	
b. N/A		
12. Cooling systems		
a. Central Unit	Cap: 33.0 kBtu/hr SEER: 11.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 33.0 kBtu/hr HSPF: 7.30	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 40.0 gallons EF: 0.92	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.14 Total as-built points: 24267 Total base points: 25065 **PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Ben Zecher

DATE: 6-5-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Chris W. Cr

DATE: 6-20-06

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 71, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1370.0	20.04	4941.9	Double, Clear	W	1.5	1.5	3.3	38.52	0.53	67.4	
				Double, Clear	W	1.5	5.5	30.0	38.52	0.90	1036.6	
				Double, Clear	W	12.5	7.5	20.0	38.52	0.43	330.8	
				Double, Clear	E	1.5	5.5	45.0	42.06	0.90	1696.4	
				Double, Clear	E	4.0	5.5	30.0	42.06	0.60	759.5	
				Double, Clear	S	1.5	5.5	15.0	35.87	0.83	447.7	
				Double, Clear	S	1.5	1.2	32.0	35.87	0.49	559.1	
				Double, Clear	W	1.5	5.5	10.0	38.52	0.90	345.5	
				As-Built Total:				**185.3**			**5243.1**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	308.0	0.70	215.6	Frame, Wood, Adjacent			13.0	308.0	0.60		184.8	
Exterior	997.7	1.70	1696.1	Frame, Wood, Exterior			13.0	997.7	1.50		1496.6	
Base Total:	**1305.7**		**1911.7**	**As-Built Total:**				**1305.7**			**1681.4**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	20.0	1.60	32.0	Exterior Insulated				20.0	4.10		82.0	
Exterior	40.0	4.10	164.0	Exterior Insulated				20.0	4.10		82.0	
				Adjacent Insulated				20.0	1.60		32.0	
Base Total:	**60.0**		**196.0**	**As-Built Total:**				**60.0**			**196.0**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1370.0	1.73	2370.1	Under Attic			30.0	1414.0	1.73 X 1.00		2446.2	
Base Total:	**1370.0**		**2370.1**	**As-Built Total:**				**1414.0**			**2446.2**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	197.0(p)	-37.0	-7289.0	Slab-On-Grade Edge Insulation			0.0	197.0(p	-41.20		-8116.4	
Raised	0.0	0.00	0.0									
Base Total:			**-7289.0**	**As-Built Total:**				**197.0**			**-8116.4**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1370.0	10.21	13987.7					1370.0	10.21		13987.7	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 71, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT						
Summer Base Points: 16118.4			**Summer As-Built Points:**						**15438.0**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points	
			(sys 1: Central Unit 33000 btuh ,SEER/EFF(11.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)						
			15438	1.00	(1.09 x 1.147 x 0.91)	0.310	1.000	5449.6	
16118.4	**0.4266**	**6876.1**	**15438.0**	**1.00**	**1.138**	**0.310**	**1.000**	**5449.6**	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 71, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1370.0	12.74	3141.7	Double, Clear	W	1.5	1.5	3.3	20.73	1.17	79.8	
				Double, Clear	W	1.5	5.5	30.0	20.73	1.03	639.3	
				Double, Clear	W	12.5	7.5	20.0	20.73	1.21	503.0	
				Double, Clear	E	1.5	5.5	45.0	18.79	1.04	880.6	
				Double, Clear	E	4.0	5.5	30.0	18.79	1.20	678.6	
				Double, Clear	S	1.5	5.5	15.0	13.30	1.15	228.8	
				Double, Clear	S	1.5	1.2	32.0	13.30	3.11	1324.8	
				Double, Clear	W	1.5	5.5	10.0	20.73	1.03	213.1	
				As-Built Total:				**185.3**			**4548.0**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	308.0	3.60	1108.8	Frame, Wood, Adjacent			13.0	308.0	3.30		1016.4	
Exterior	997.7	3.70	3691.5	Frame, Wood, Exterior			13.0	997.7	3.40		3392.2	
Base Total:	**1305.7**		**4800.3**	**As-Built Total:**				**1305.7**			**4408.6**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	20.0	8.00	160.0	Exterior Insulated				20.0	8.40		168.0	
Exterior	40.0	8.40	336.0	Exterior Insulated				20.0	8.40		168.0	
				Adjacent Insulated				20.0	8.00		160.0	
Base Total:	**60.0**		**496.0**	**As-Built Total:**				**60.0**			**496.0**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X WCM =		Points	
Under Attic	1370.0	2.05	2808.5	Under Attic			30.0	1414.0	2.05 X 1.00		2898.7	
Base Total:	**1370.0**		**2808.5**	**As-Built Total:**				**1414.0**			**2898.7**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	197.0(p)	8.9	1753.3	Slab-On-Grade Edge Insulation			0.0	197.0(p	18.80		3703.6	
Raised	0.0	0.00	0.0									
Base Total:			**1753.3**	**As-Built Total:**				**197.0**			**3703.6**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	1370.0	-0.59	-808.3					1370.0	-0.59		-808.3	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 71, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT						
Winter Base Points:		**12191.5**	**Winter As-Built Points:**						**15246.6**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
			(sys 1: Electric Heat Pump 33000 btuh ,EFF(7.3) Ducts:Unc(S),Unc(R),Int(AH),R6.0						
			15246.6	1.000	(1.069 x 1.169 x 0.93)	0.467	1.000	8277.1	
12191.5	**0.6274**	**7648.9**	**15246.6**	**1.00**	**1.162**	**0.467**	**1.000**	**8277.1**	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 71, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
4		2635.00	10540.0	40.0	0.92	4		1.00	2635.00	1.00	10540.0
				As-Built Total:							**10540.0**

CODE COMPLIANCE STATUS

BASE					AS-BUILT				
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points		Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	
6876	**7649**	**10540**	**25065**		**5450**	**8277**	**10540**	**24267**	

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 71, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 83.5

The higher the score, the more efficient the home.

Model Home, Lot: 71, Sub: Emerald Cove, Plat: , Lake City, FL,

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family		a. Central Unit	Cap: 33.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 11.00
4.	Number of Bedrooms	4		b. N/A	
5.	Is this a worst case?	Yes			
6.	Conditioned floor area (ft²)	1370 ft²		c. N/A	
7.	Glass type [1] and area: (Label reqd. by 13-104.4.5 if not default)				
	a. U-factor:	Description Area	13.	Heating systems	
	(or Single or Double DEFAULT) 7a.	(Dble Default) 185.3 ft²		a. Electric Heat Pump	Cap: 33.0 kBtu/hr
	b. SHGC:				HSPF: 7.30
	(or Clear or Tint DEFAULT) 7b.	(Clear) 185.3 ft²		b. N/A	
8.	Floor types				
	a. Slab-On-Grade Edge Insulation	R=0.0, 197.0(p) ft		c. N/A	
	b. N/A				
	c. N/A		14.	Hot water systems	
9.	Wall types			a. Electric Resistance	Cap: 40.0 gallons
	a. Frame, Wood, Adjacent	R=13.0, 308.0 ft²			EF: 0.92
	b. Frame, Wood, Exterior	R=13.0, 997.7 ft²		b. N/A	
	c. N/A				
	d. N/A			c. Conservation credits	
	e. N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
	a. Under Attic	R=30.0, 1414.0 ft²	15.	HVAC credits	
	b. N/A			(CF-Ceiling fan, CV-Cross ventilation,	
	c. N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
	a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 140.0 ft		MZ-C-Multizone cooling,	
	b. N/A			MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: Chris W. G Date: 6-20-06

Address of New Home: 139 SW TIMBERLAND CT. City/FL Zip: LAKE CITY, FL. 32055

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001142

DATE 07/03/2006 PARCEL ID # 33-3S-16-02438-171

APPLICANT SUSAN FAIR PHONE 386.752.1711

ADDRESS 180 NW AMENITY COURT LAKE CITY FL 32055

OWNER CORNERSTONE DEVELOPERS PHONE 386.752.1711

ADDRESS 139 SW TIMBERLAND COURT LAKE CITY FL 32055

CONTRACTOR BRYAN ZECHER PHONE 386.752.8653

LOCATION OF PROPERTY 90-W TO HEATHRIDGE DR,TL TO TIMBERLAND CT, TR AND IT'S THE 2ND LOT ON L.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT EMERALD COVE 71 1

SIGNATURE V Susan Fair

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
904 NW Main Blvd.
LAKE CITY, FLORIDA 32055

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0606-90

Contractor: Bryan Zecher Owners Cornerstone Developers Lot 71 Phase 1 of Emerald Cove Subdivision

On the date of June 28, 2006 application 0606-90 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0606-90 when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

To help ensure compliance with the Florida Residential Code 2004 the comments below need to be addressed on the plans.

1. Please provide a copy of a signed released site plan from the Columbia County Environmental Health Department which confirms approval of the waste water disposal system.
2. The electrical plan shows the location of the electrical service, Please indicate on the electrical plan that an overcurrent protection device will be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.
3. Please indicate on the floor plans that the Garage area will comply with the FRC-2004 Sections R309 R309.1 Opening protection: Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors.
4. In the garage area show compliance with the FRC-2004 sections R309.2 Separation required: The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling

assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent.

5. Please submit a recorded (with the Columbia County Clerk Office) notice of commencement before any inspections can be preformed by the Columbia County Building Department.

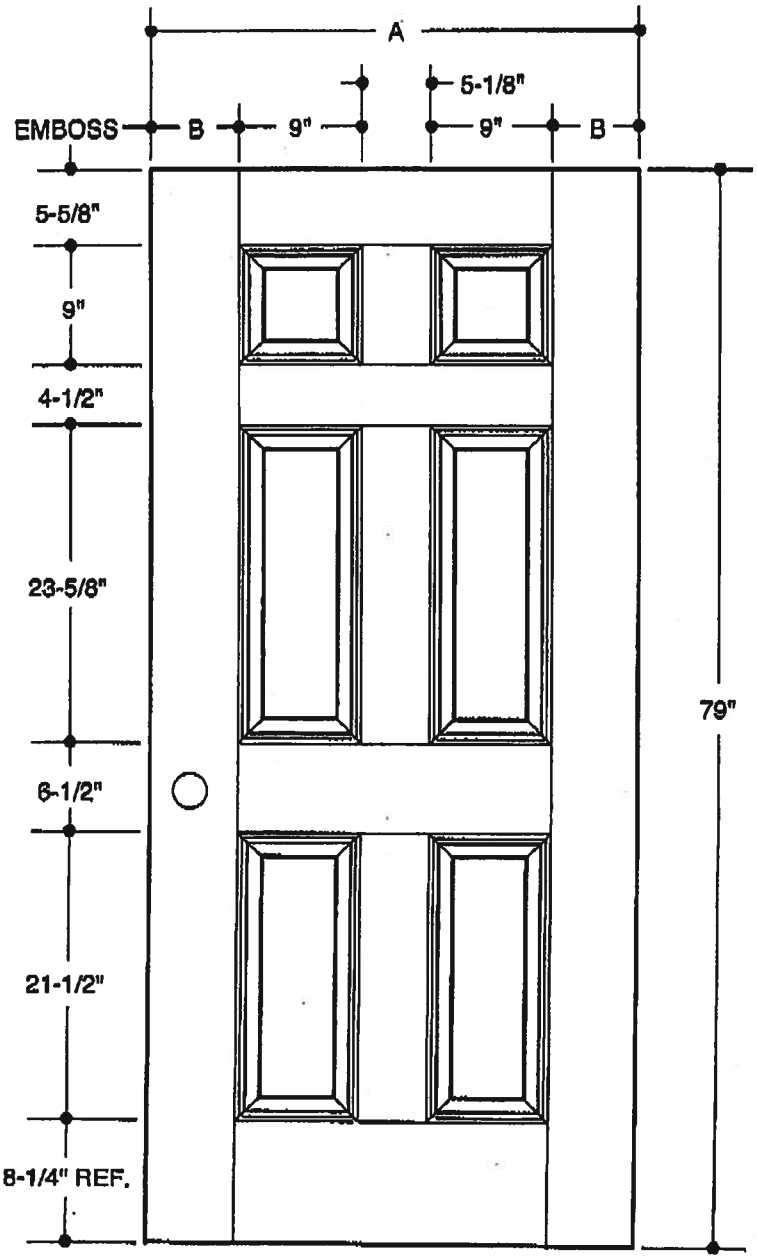
6. Please show the method to which the roof ventilation as required in section R806.2 of the FRC-2004 will be accomplished: Minimum area: the total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents. As an alternative, the net free cross-ventilation area may be reduced to 1 to 300 when a vapor barrier having a transmission rate not exceeding 1 perm (57.4 mg/s · m2 · Pa) is installed on the warm side of the ceiling.

Joe Haltiwanger

Plan Examiner
Columbia County Building Department

Masonite

6-PANEL



A	31-3/4"
B	4-5/16"

2'8"

A	33-3/4"
B	5-5/16"

2'10"

A	35-3/4"
B	6-5/16"

3'0"

Please Note:

1. Available as 20-Minute Fire Rated - Warnock Hersey Labeled.

PART NO.	QTY.	DESCRIPTION
		ARTEK® SMOOTH FIBERGLASS

Our continuing program of product improvement makes specifications, design and product details subject to change without notice.

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ——— 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ---------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** Plans or specifications must state compliance with FBC Section 1609. The following information must be shown as per section 1603.1.4 FBC a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m^2) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.
		Elevations including:
☑	☐	a) All sides
☐	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation

☑ ☐ d) Location, size and height above roof of chimneys.
☑ ☐ e) Location and size of skylights
☑ ☐ f) Building height
☑ ☐ e) Number of stories

Floor Plan including:

☑ ☐ a) Rooms labeled and dimensioned.
☑ ☐ b) Shear walls identified.
☑ ☐ c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
☑ ☐ d) Show safety glazing of glass, where required by code.
☑ ☐ e) Identify egress windows in bedrooms, and size.
☑ ☐ f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, (Please circle applicable type).
☑ ☐ g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
☑ ☐ h) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

☑ ☐ a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
☑ ☐ b) All posts and/or column footing including size and reinforcing
☑ ☐ c) Any special support required by soil analysis such as piling
☑ ☐ d) Location of any vertical steel.

Roof System:

☑ ☐ a) Truss package including:
1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

☐ ☐ b) Conventional Framing Layout including:
1. Rafter size, species and spacing
2. Attachment to wall and uplift
3. Ridge beam sized and valley framing and support details
4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

NA

Wall Sections including:

☐ ☐ a) Masonry wall
1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termiticide or alternative method)
10. Slab on grade
    a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:

NA

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

☑ ☐ b) Wood frame wall
1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ NA ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☐ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ ☐ b) Floor joist size and spacing

☐ NA ☐ c) Girder size and spacing

☐ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

☐ ☐ **Plumbing Fixture layout**

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

☑ ☐ h) Exhaust fans in bathroom

HVAC information

☑ ☐ a) Energy Calculations (dimensions shall match plans)

☑ ☐ b) Manual J sizing equipment or equivalent computation

☑ ☐ c)Gas System Type (LP or Natural) Location and BTU demand of equipment

☑ ☐ **Disclosure Statement for Owner Builders**

☑ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done**

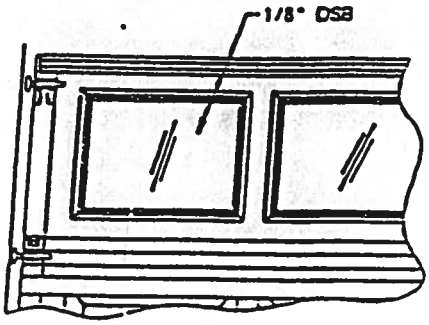
☑ ☐ **Private Potable Water**

GARAGE DOORS

REVISIONS			
LETTER	DESCRIPTION	DATE	
A	REV PER EN 10130	3/06/98	
B	REV PER EN 10141	3/13/98	
C	REV PER EN 10132	3/18/98	

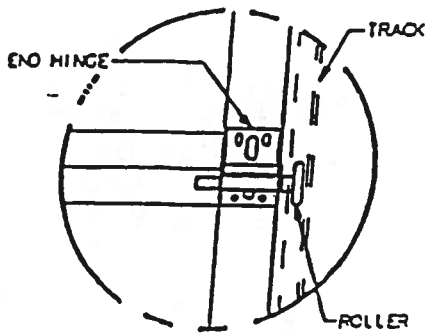
...UCTED WITH 5 OR MORE SECTIONS MUST BE
...E STRUTS FOR THE ADDITIONAL INTERMEDIATE
ON THE THIRD SECTION.
...L SECTIONS LESS THAN 20.812" MUST BE
THE ACTUAL SECTION HEIGHT & 20.812".
...X 5/8" LONG TYPE AB HEX HEAD METAL SCREW
...WIND WARNINGS ISSUED.

2.



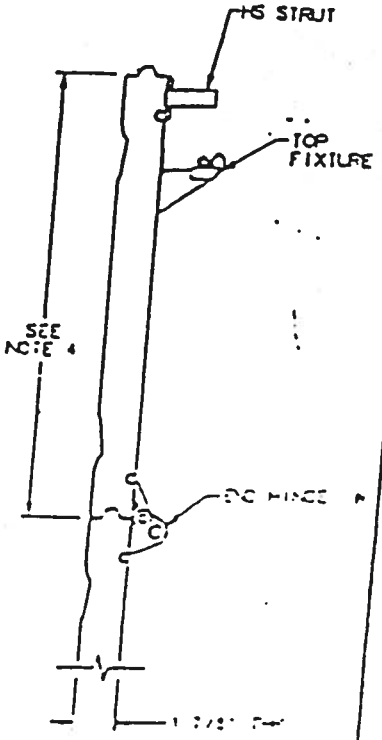
OPTIONAL WINDOW DETAIL

SCALE: 3/32

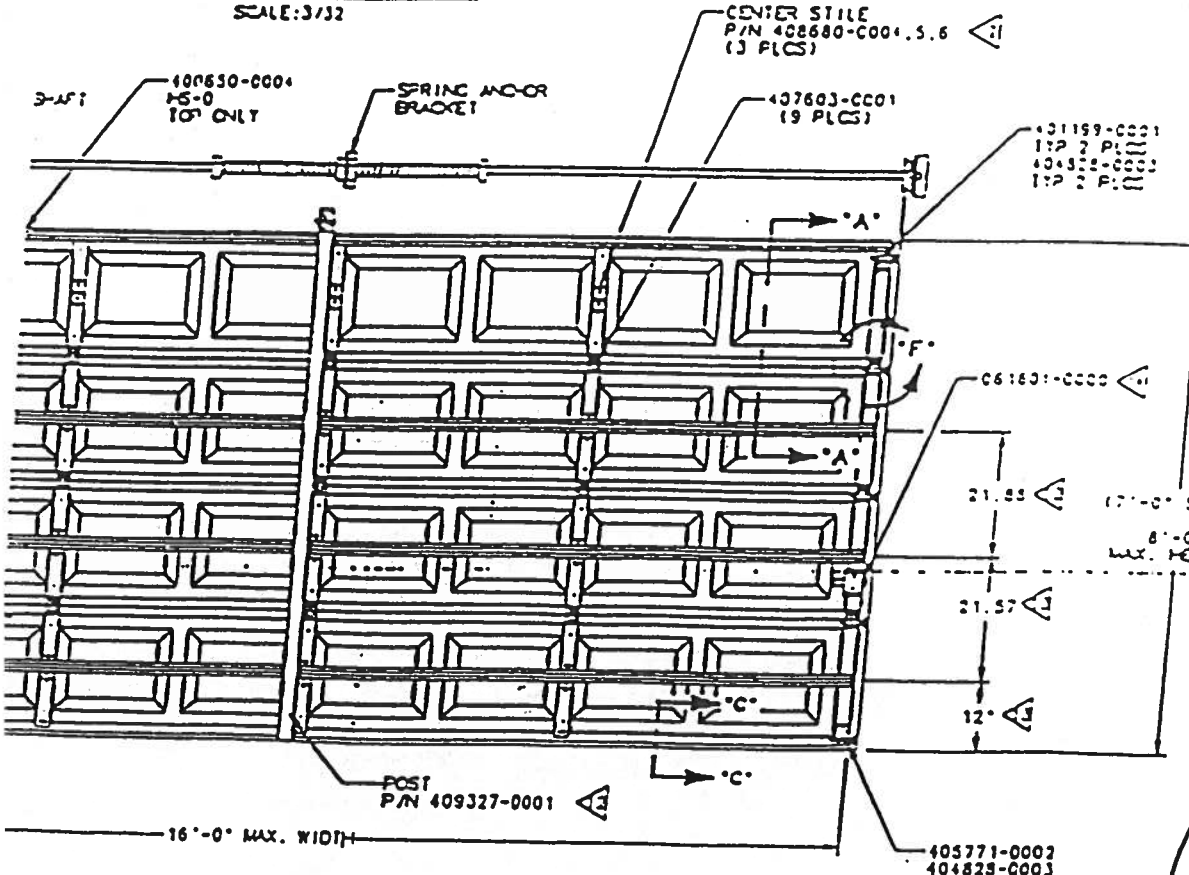


DETAIL "F"

SCALE: 1/4
(16" WIDE)



SECTION "A-A"



DESIGN LOAD
25 PSF =
TEST LOAD
37.5 PSF =

21.93 (7'-0" SHOWN)

8'-0" MAX. HEIGHT

21.57

12"

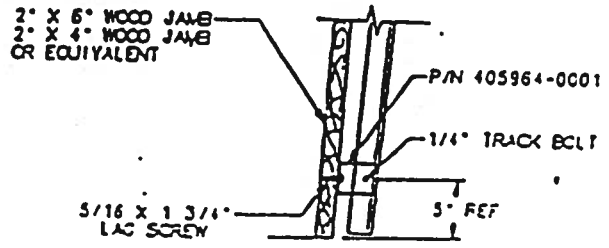
SCALE: 1/16"=1"

INTERIOR ELEVATION

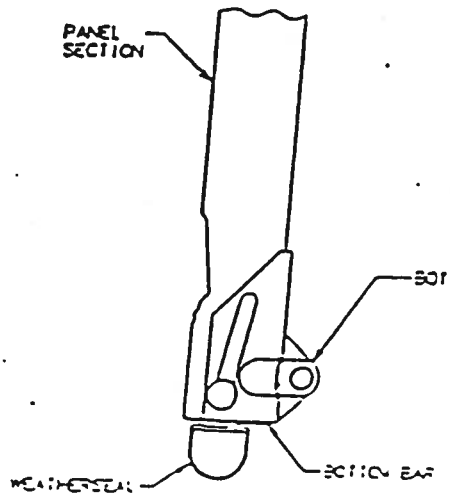
	M.TOLNIS	01/13/98	SERIES 280 & 180 RESI STL DR, 16'-0" MAX WIDTH, WINDLOAD
N/A	DAVID FAW	02/19/98	
NONE / EACH	DAVID FAW	02/19/98	D-409335

NOTES:

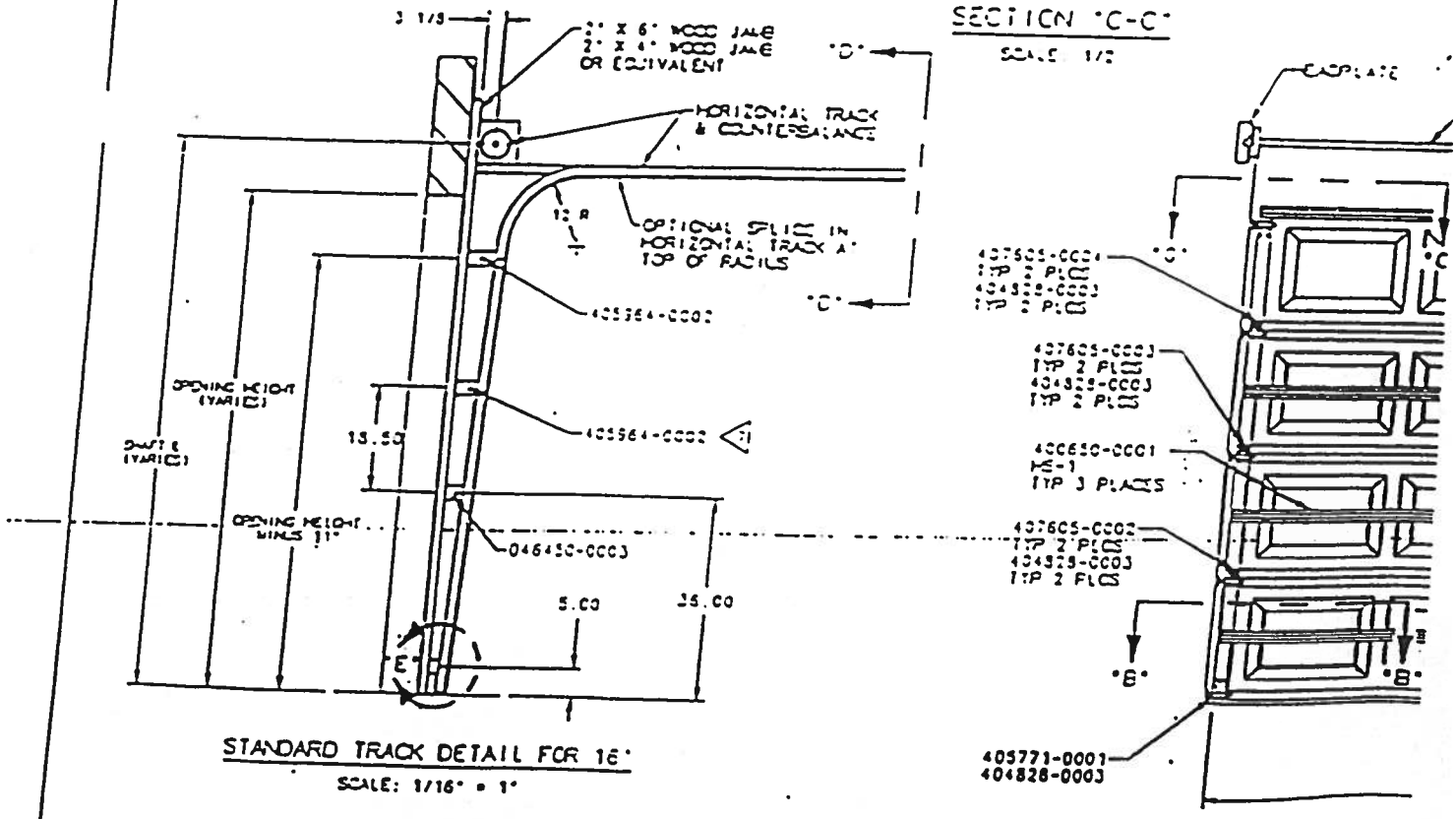
1. TESTED IN ACCORDANCE WITH STANDARD BUILDING CODE, CHAPTER 17, TO A POSITIVE AND NEGATIVE 37.5 PSF.
2. DASH NUMBERS REPRESENT VARIOUS SECTION HEIGHTS.
3. FOUR SECTION 7' HIGH DOOR SHOWN. 8' HIGH DOORS HAVE FIVE SECTIONS.
4. SECTION HEIGHT OF 20.312, 19.00 & 16.75 ARE AVAILABLE AND MAY BE USED IN COMBINATION TO ACHIEVE VARIOUS HEIGHT DOORS.
5. EMBOSSMENT PATTERN OF 14.50 X 20.375 SHOWN. ALTERNATE PATTERNS OF 12.50 X 43.375 AND 12.50 X 20.375 MAY BE USED.
6. TORSION SPRINGS SHOWN. EXTENSION SPRINGS AVAILABLE.
7. USE THIS BRACKET, REF. P/N 405964-0002, ON 8' HIGH DOORS ONLY.
8. WINDOW MAY BE INSTALLED IN THE TOP SECTION OR THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
9. THE STRUT PLACEMENT ON [illegible] CONSISTENT WITH THE DOOR SECTIONS ARE TO BE PLACE
10. THE STRUT PLACEMENT DIME REDUCED BY THE DIFFERENC
11. SCREW P/N 605911-0001 IS
12. POST TO BE INSTALLED ONLY
13. STRUT PLACEMENTS CAN VARY
14. QUANTITY FOR LOCKS CAN BE



DETAIL "E"
SCALE: 1/3



VIEW "D"
2" TRACK
.049 THK (VERT



SERIES 280 THRU 289 ARE EQUIVALENT CONSTRUCTION 25GA STEEL
SERIES 180 ARE SAME CONSTRUCTION AS SERIES 280 ONLY 24GA STEEL AND END CAPS

DOOR TESTED WAS 281 SERIES.

DOOR WIDTH	CENTER STILE	END STILE	ROLLER SHAFT BRACKET	STRUTS/SECT.	ROLLER	VERTICAL TRACK GAGE	JAMB LOAD (# PER FT-HT)	HARDWARE
16'	3	SINGLE		HS1 HS0	2" X 7/16"	.049"	100	STD.

Anthony Power Header®

GARAGE HEADER (84) $26F_b$ - 1.9E

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS (F_b) =	2600
COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) =	740
HORIZONTAL SHEAR (F_v) =	225
MODULUS OF ELASTICITY (MOE) =	1.9×10^6

	7.7	9.0	10.4	11.7	12.9	14.2	15.5
	326	514	789	1115	1521	2014	2604
	8865	12015	15996	20145	24772	29877	35460
	3908	4550	5250	5892	6533	7175	7817

NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b, shall be modified by the Volume Factor, C_v, as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1997.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for Power Header®.

GARAGE HEADER COMPARISONS

	810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
	990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-5/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
	610 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-1/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"*
	765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"*
	750 / 490	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"*
	900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	—

For more information on Power Header®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

Anthony Forest Products Company
Post Office Box 1877 • El Dorado, Arkansas 71731
Internet address: http:// www.anthonyforest.com
e-mail: info@anthonyforest.com

Distributed by:

WOODFORD PLYWOOD, INC.
"Structural Wood Products"

11980 West Beaver Street
Jacksonville, Florida 32220
(904) 695-9080
(800) 447-6568
FAX (904) 695-9160

Anthony POWER HEADER®

$26F_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

844	896		1216		1573						
161	207	254	330	390	510	552	669	752	824		
114	145	180	231	277	359	391	510	534	653	707	789

844	975		1322								
161	207	254	330	390	510	552	724	732	897		
114	145	180	231	277	359	391	510	534	699	693	

562	778	888	1056	1363	1367		1582						
107	153	169	245	260	380	368	540	501	715	664	864	840	
76	107	120	171	185	267	261	380	356	521	471	684	609	813

NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

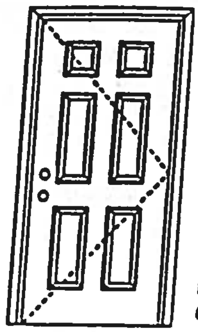
To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

X

Opaque Inswing Unit

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door

Maximum unit size = 3'0" x 6'8"

Design Pressure

+66.0/-66.0

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

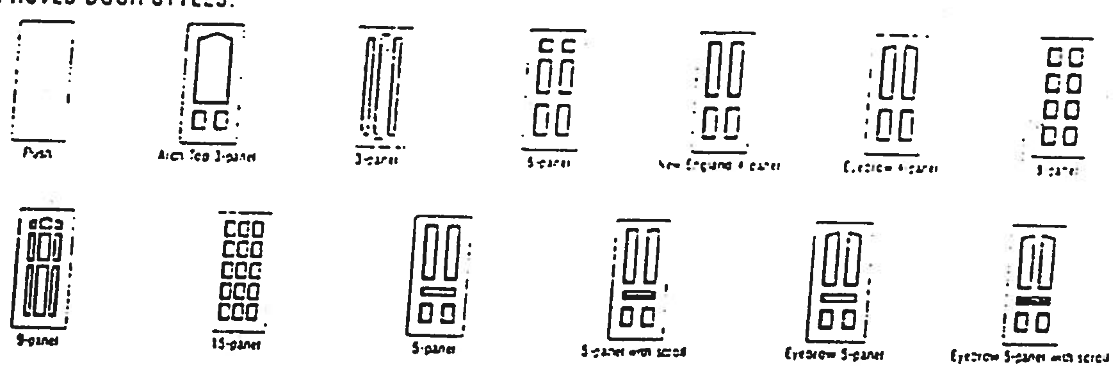
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02

APPROVED DOOR STYLES:



Johnson EntrySystems

June 17, 2002

PREMDOOR Collection

Exclusively from Masonite

Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Vindicon Matrix #3026447A-001 provides additional information - available from the ITS WH website (www.itscwh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson EntrySystems

June 17, 2002

PREMDOR Premium Quality Doors

Masonite International Corporation

WINDOWS

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft²
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:nlb

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allen H. Reeves
1 APRIL 2002

REEVES
No. 18334
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

ALLEN N. REEVES CERTIFICATE NO. 11??? STATE OF FLORIDA PROFESSIONAL ENGINEER

Allen N. Reeves
1 APRIL 2002

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

ALLEN N. REEVES, CERTIFICATE NO. 13??4, STATE OF FLORIDA, PROFESSIONAL ENGINEER

Allen N. Reeves
1 APRIL 2002

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft²	0.3 cfm/ft² max

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.42"*	0.26" max.
	@ 34.7 psf (negative)	0.43"*	0.26" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.02"	0.18" max.
	@ 52.1 psf (negative)	0.02"	0.18" max.

*Exceeds L/175 for deflection, but passes all other test requirements.

Allen H. Reeves
1 APRIL 2002

ALLEN H. REEVES
CERTIFICATE
NO. 1837
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

MAH:rlb
01-41134.01

ALLEN N. REEVES ☆ PROFESSIONAL ENGINEER ☆
CERTIFICATE NO. [illegible]
☆
STATE OF FLORIDA

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	**Deglazing Test (ASTM E 987)**		
	In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test:	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test: A7	No entry	No entry
	Lock Manipulation Test:	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.05"	0.15" max.
	@ 70.8 psf (negative)	0.05"	0.15" max.

Exceeds L/175 for deflection, but passes all other test requirements.

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
No. 18334
STATE OF FLORIDA
PROFESSIONAL ENGINEER

SHINGLES

ELK Premium Roofing

March 6, 2002

Subject: Elk Product Approval Information

All Prestique® and Capstone® products manufactured in Tuscaloosa, AL are certified under the Miami – Dade County Building Code Office (BCCO). These products also meet the requirements for the Florida Building Code since they are MD approved. The following test protocols must be passed by each of the products in order for MD product certification:

ASTM D3462
PA 100 (110 mph uplift and wind driven rain resistance)
PA 107 (Modified ASTM D3161 - 110 mph wind uplift resistance)

The nailing patterns that were used during the PA 100 and PA 107 wind test protocols for the Prestique and Capstone products are listed below. Also listed below are the Miami – Dade Notice of Acceptance Numbers (NOA)

Raised Profile, Prestique High Definition, Prestique 35, or Prestique 30 –
PA 100 = 4 nails
PA 107 = 5 nails
MD NOA# = 01-1226.04

Prestique I 35 or Prestique I* –
PA 100 = 4 nails
PA 107 = 5 nails
MD NOA# = 01-1226.05

Prestique Plus or Prestique Gallery Collection* –
PA 100 = 4 nails
PA 107 = 4 nails
MD NOA# = 01-1226.03

Capstone*
PA 100 = 4 Nails
PA 107 = 4 Nails
MD NOA# = 01-0523.01

* As per the Elk Limited Warranty, six nails are required for the Elk high wind warranty

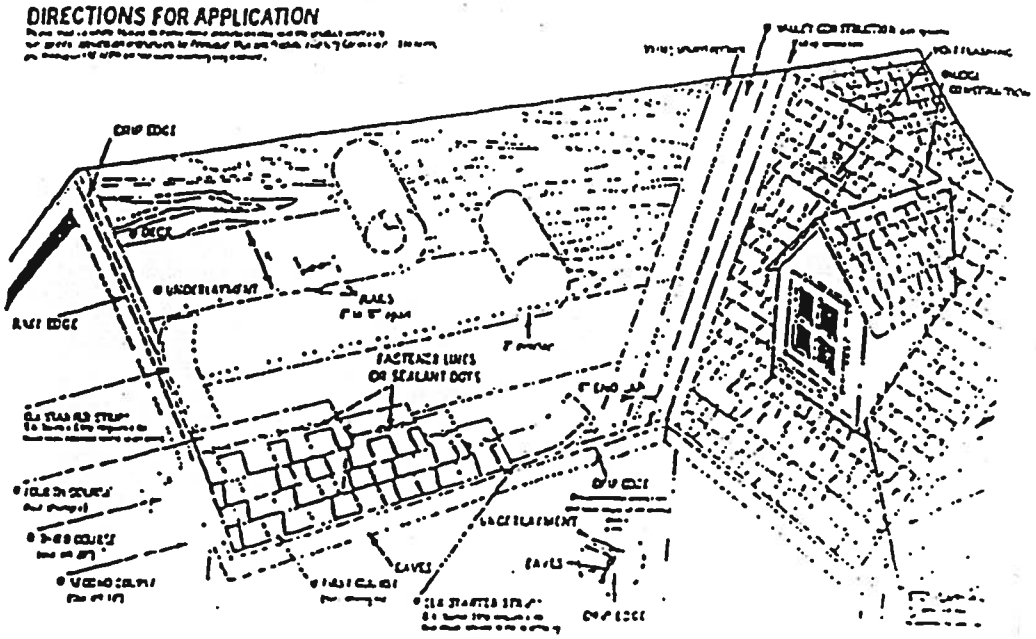
If there are any questions please contact:

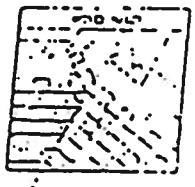
Mike Reed – Technical Manager (205) 342-0287

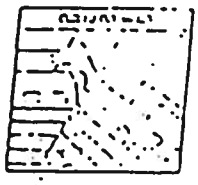
or

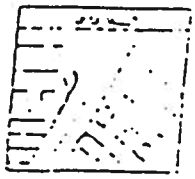
Daniel Delarmette – QA Engineer (205) 342-0293

DIRECTIONS FOR APPLICATION









DIRECTIONS FOR APPLICATION

STARTER COURSE

FIRST COURSE

SECOND COURSE

CAUTION TO WHOLESALER

ELK

www.elkcorp.com

ROOFING PRODUCTS SPECIFICATIONS – TUSCALOOSA, AL

PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE™

HIP AND RIDGE SHINGLES

AS Prestique and Raised Profile shingles meet UL Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790), and ASTM Specifications D 3018, Type I, D 3161, Type I, E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles meet the latest ... Code building code requirements.

SOUTHEAST & ATLANTIC OFFICE:
800.945.5551

CORPORATE HEADQUARTERS:
800.354.7732

PLANT LOCATION:
800.945.5545

ELK
www.elkcorp.com

Residential System Sizing Calculation

Summary

Model Home

Lake City, FL

Project Title:
606025TheVictoriaModel

Class 3 Rating
Registration No. 0
Climate: North

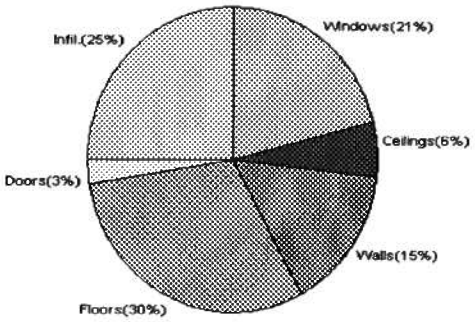
6/5/2006

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)					
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	**28252**	**Btuh**	**Total cooling load calculation**	**21525**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	116.8	33000	Sensible (SHR = 0.75)	145.1	24750
Heat Pump + Auxiliary(0.0kW)	116.8	33000	Latent	184.5	8250
			Total (Electric Heat Pump)	153.3	33000

WINTER CALCULATIONS

Winter Heating Load (for 1370 sqft)

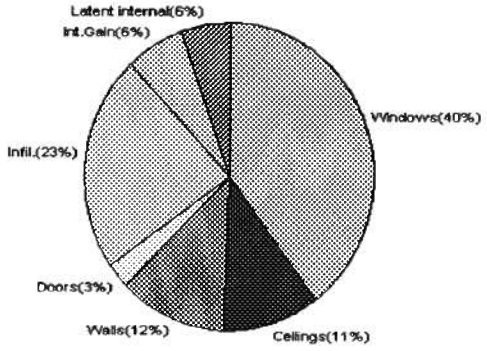
Load component			Load	
Window total	185	sqft	5965	Btuh
Wall total	1306	sqft	4288	Btuh
Door total	60	sqft	777	Btuh
Ceiling total	1414	sqft	1666	Btuh
Floor total	197	sqft	8601	Btuh
Infiltration	172	cfm	6955	Btuh
Duct loss			0	Btuh
Subtotal			**28252**	**Btuh**
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			**28252**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1370 sqft)

Load component			Load	
Window total	185	sqft	8533	Btuh
Wall total	1306	sqft	2546	Btuh
Door total	60	sqft	588	Btuh
Ceiling total	1414	sqft	2342	Btuh
Floor total			0	Btuh
Infiltration	90	cfm	1666	Btuh
Internal gain			1380	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			**17054**	**Btuh**
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			3271	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1200	Btuh
Total latent gain			**4471**	**Btuh**
TOTAL HEAT GAIN			**21525**	**Btuh**



For Florida residences only

EnergyGauge® System Sizing
PREPARED BY:
DATE: 6-5-06

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Model Home

Lake City, FL

Project Title:
606025TheVictoriaModel

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

6/5/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	3.3	32.2	106 Btuh
2	2, Clear, Metal, 0.87	NW	30.0	32.2	966 Btuh
3	2, Clear, Metal, 0.87	NW	20.0	32.2	644 Btuh
4	2, Clear, Metal, 0.87	SE	45.0	32.2	1449 Btuh
5	2, Clear, Metal, 0.87	SE	30.0	32.2	966 Btuh
6	2, Clear, Metal, 0.87	SW	15.0	32.2	483 Btuh
7	2, Clear, Metal, 0.87	SW	32.0	32.2	1030 Btuh
8	2, Clear, Metal, 0.87	NW	10.0	32.2	322 Btuh
	Window Total		185(sqft)		5965 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Adj(0.09)	13.0	308	3.3	1011 Btuh
2	Frame - Wood - Ext(0.09)	13.0	998	3.3	3277 Btuh
	Wall Total		1306		4288 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
3	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		60		777Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1414	1.2	1666 Btuh
	Ceiling Total		1414		1666Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	197.0 ft(p)	43.7	8601 Btuh
	Floor Total		197		8601 Btuh
			Zone Envelope Subtotal:		21297 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.94	10960	171.7	6955 Btuh
Ductload	Average sealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**28252 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

Model Home

Lake City, FL

Project Title:
606025TheVictoriaModel

Class 3 Rating
Registration No. 0
Climate: North

6/5/2006

WHOLE HOUSE TOTALS

	Subtotal Sensible	28252 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	28252 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Model Home | Project Title: 606025TheVictoriaModel | Class 3 Rating

Lake City, FL | Registration No. 0 | Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F | 6/5/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	3.3	32.2	106 Btuh
2	2, Clear, Metal, 0.87	NW	30.0	32.2	966 Btuh
3	2, Clear, Metal, 0.87	NW	20.0	32.2	644 Btuh
4	2, Clear, Metal, 0.87	SE	45.0	32.2	1449 Btuh
5	2, Clear, Metal, 0.87	SE	30.0	32.2	966 Btuh
6	2, Clear, Metal, 0.87	SW	15.0	32.2	483 Btuh
7	2, Clear, Metal, 0.87	SW	32.0	32.2	1030 Btuh
8	2, Clear, Metal, 0.87	NW	10.0	32.2	322 Btuh
	Window Total		185(sqft)		5965 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Adj(0.09)	13.0	308	3.3	1011 Btuh
2	Frame - Wood - Ext(0.09)	13.0	998	3.3	3277 Btuh
	Wall Total		1306		4288 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
3	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		60		777Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1414	1.2	1666 Btuh
	Ceiling Total		1414		1666Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	197.0 ft(p)	43.7	8601 Btuh
	Floor Total		197		8601 Btuh
			Zone Envelope Subtotal:		21297 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.94	10960	171.7	6955 Btuh
Ductload	Average sealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**28252 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

Model Home

Lake City, FL

Project Title:
606025TheVictoriaModel

Class 3 Rating
Registration No. 0
Climate: North

6/5/2006

WHOLE HOUSE TOTALS

	Subtotal Sensible	28252 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	28252 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Model Home

Lake City, FL

Project Title:
606025TheVictoriaModel

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 6/5/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	1.5ft.	3.3	0.0	3.3	29	60	198 Btuh
2	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	30.0	0.0	30.0	29	60	1801 Btuh
3	2, Clear, 0.87, None,N,N	NW	12.5f	7.5ft.	20.0	0.0	20.0	29	60	1201 Btuh
4	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	45.0	18.2	26.8	29	63	2203 Btuh
5	2, Clear, 0.87, None,N,N	SE	4ft.	5.5ft.	30.0	30.0	0.0	29	63	869 Btuh
6	2, Clear, 0.87, None,N,N	SW	1.5ft.	5.5ft.	15.0	6.1	8.9	29	63	734 Btuh
7	2, Clear, 0.87, None,N,N	SW	1.5ft.	1.16	32.0	32.0	0.0	29	63	927 Btuh
8	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	10.0	0.0	10.0	29	60	600 Btuh
	Window Total				185 (sqft)					8533 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Adj	13.0/0.09	308.0	1.5	465 Btuh
2	Frame - Wood - Ext	13.0/0.09	997.7	2.1	2081 Btuh
	Wall Total		1306 (sqft)		2546 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	20.0	9.8	196 Btuh
2	Insulated - Exterior	20.0	9.8	196 Btuh
3	Insulated - Exterior	20.0	9.8	196 Btuh
	Door Total	60 (sqft)		588 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1414.0	1.7	2342 Btuh
	Ceiling Total		1414 (sqft)		2342 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	197 (ft(p))	0.0	0 Btuh
	Floor Total		197.0 (sqft)		0 Btuh

	Zone Envelope Subtotal:	14008 Btuh

Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.49	10960	89.5	1666 Btuh

Internal gain	Occupants	Btuh/occupant	Appliance	Load
	6	X 230 +	0	1380 Btuh

Duct load	Average sealed, R6.0, Supply(Attic), Return(Attic) DGM = 0.00	0.0 Btuh
	Sensible Zone Load	**17054 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Model Home

Lake City, FL

Project Title:
606025TheVictoriaModel

Class 3 Rating
Registration No. 0
Climate: North

6/5/2006

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	**Sensible Envelope Load All Zones**	**17054 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**17054 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	**17054 Btuh**
	Latent infiltration gain (for 54 gr. humidity difference)	3271 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**4471 Btuh**
	TOTAL GAIN	**21525 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Model Home
Lake City, FL

Project Title:
606025TheVictoriaModel

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 6/5/2006
This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Zone #1: Main

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	1.5ft.	3.3	0.0	3.3	29	60	198 Btuh
2	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	30.0	0.0	30.0	29	60	1801 Btuh
3	2, Clear, 0.87, None,N,N	NW	12.5f	7.5ft.	20.0	0.0	20.0	29	60	1201 Btuh
4	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	45.0	18.2	26.8	29	63	2203 Btuh
5	2, Clear, 0.87, None,N,N	SE	4ft.	5.5ft.	30.0	30.0	0.0	29	63	869 Btuh
6	2, Clear, 0.87, None,N,N	SW	1.5ft.	5.5ft.	15.0	6.1	8.9	29	63	734 Btuh
7	2, Clear, 0.87, None,N,N	SW	1.5ft.	1.16	32.0	32.0	0.0	29	63	927 Btuh
8	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	10.0	0.0	10.0	29	60	600 Btuh
	Window Total				185 (sqft)					8533 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Adj	13.0/0.09	308.0	1.5	465 Btuh
2	Frame - Wood - Ext	13.0/0.09	997.7	2.1	2081 Btuh
	Wall Total		1306 (sqft)		2546 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	20.0	9.8	196 Btuh
2	Insulated - Exterior	20.0	9.8	196 Btuh
3	Insulated - Exterior	20.0	9.8	196 Btuh
	Door Total	60 (sqft)		588 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1414.0	1.7	2342 Btuh
	Ceiling Total		1414 (sqft)		2342 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	197 (ft(p))	0.0	0 Btuh
	Floor Total		197.0 (sqft)		0 Btuh

	Zone Envelope Subtotal:	14008 Btuh

Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.49	10960	89.5	1666 Btuh

Internal gain	Occupants	Btuh/occupant	Appliance	Load
	6	X 230 +	0	1380 Btuh

Duct load	Average sealed, R6.0, Supply(Attic), Return(Attic)	DGM = 0.00	0.0 Btuh
		Sensible Zone Load	**17054 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Model Home

Lake City, FL

Project Title:
606025TheVictoriaModel

Class 3 Rating
Registration No. 0
Climate: North

6/5/2006

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	**Sensible Envelope Load All Zones**	**17054 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**17054 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	**17054 Btuh**
	Latent infiltration gain (for 54 gr. humidity difference)	3271 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**4471 Btuh**
	TOTAL GAIN	**21525 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

Residential Window Diversity

MidSummer

Model Home

Lake City, FL

Project Title:
606025TheVictoriaModel

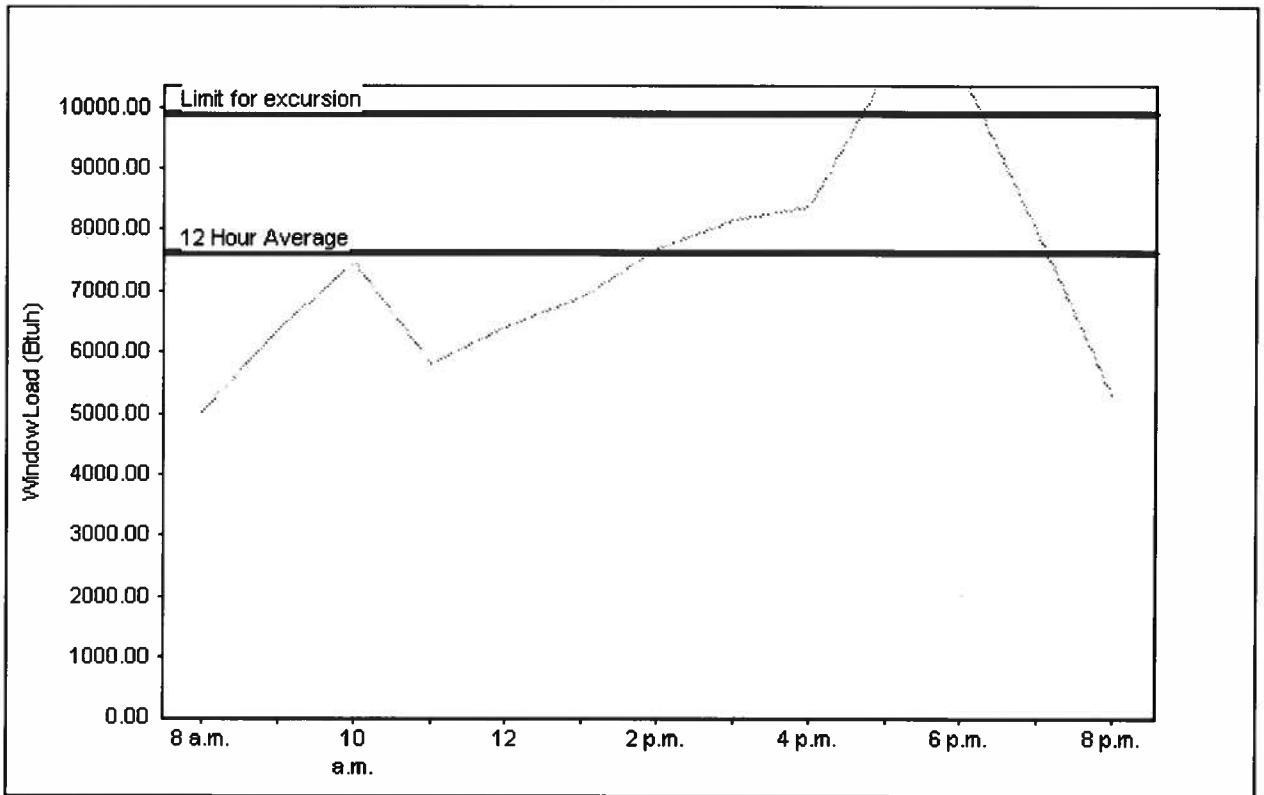
Class 3 Rating
Registration No. 0
Climate: North

6/5/2006

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	7617 Btuh
Summer setpoint	75 F	Peak window load for July	10695 Btu
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	9903 Btuh
Latitude	29 North	Window excursion (July)	792 Btuh

WINDOW Average and Peak Loads



Total July Window Load(Radiation and conduction)

Warning: This application has glass areas that produce relatively large heat gains for part of the day. Variable air volume devices may be required to overcome spikes in solar gain for one or more rooms. A zoned system may be required or some rooms may require zone control.

EnergyGauge® System Sizing for Florida residences only
PREPARED BY: Ben Gaude
DATE: 6-5-06

Project Information for:	L166538		
Builder:	CORNERSTONE	Date:	5/31/2006
Lot:	LOT 71 EMERALD COVE	Start Number:	**1212**
Subdivision:	N/A		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	43		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ZECHER, BRYAN C. CBC 054575
Address: PO BOX 815
LAKE CITY,FLORIDA 32056 Designer: **163**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0531061212	5/31/2006	41	T20P	0531061252	5/31/2006
2	CJ3	0531061213	5/31/2006	42	T21	0531061253	5/31/2006
3	CJ5	0531061214	5/31/2006	43	T21G	0531061254	5/31/2006
4	EJ3	0531061215	5/31/2006				
5	EJ5	0531061216	5/31/2006				
6	EJ7	0531061217	5/31/2006				
7	EJ7A	0531061218	5/31/2006				
8	HJ3	0531061219	5/31/2006				
9	HJ5	0531061220	5/31/2006				
10	HJ7	0531061221	5/31/2006				
11	T01	0531061222	5/31/2006				
12	T02	0531061223	5/31/2006				
13	T02T	0531061224	5/31/2006				
14	T03	0531061225	5/31/2006				
15	T03T	0531061226	5/31/2006				
16	T04	0531061227	5/31/2006				
17	T04T	0531061228	5/31/2006				
18	T05	0531061229	5/31/2006				
19	T05T	0531061230	5/31/2006				
20	T06	0531061231	5/31/2006				
21	T06T	0531061232	5/31/2006				
22	T07	0531061233	5/31/2006				
23	T07P	0531061234	5/31/2006				
24	T08	0531061235	5/31/2006				
25	T08P	0531061236	5/31/2006				
26	T09	0531061237	5/31/2006				
27	T09P	0531061238	5/31/2006				
28	T10	0531061239	5/31/2006				
29	T11	0531061240	5/31/2006				
30	T12	0531061241	5/31/2006				
31	T13	0531061242	5/31/2006				
32	T14	0531061243	5/31/2006				
33	T15	0531061244	5/31/2006				
34	T15P	0531061245	5/31/2006				
35	T16	0531061246	5/31/2006				
36	T16P	0531061247	5/31/2006				
37	T17P	0531061248	5/31/2006				
38	T18H	0531061249	5/31/2006				
39	T19P	0531061250	5/31/2006				
40	T20	0531061251	5/31/2006				

MAY 31 2006

10:00:13 AM

Licensee Details

Licensee Information

Name:	**ZECHER, BRYAN CHRISTIAN (Primary Name)** **BRYAN ZECHER CONSTRUCTION INC (DBA**
Main Address:	**P O BOX 815** **LAKE CITY, Florida 32056**
Lic. Location:	**465 NW ORANGE ST** **LAKE CITY, FL 32055 United States** **Columbia**

License Information

License Type:	**Certified Building Contractor**
Rank:	**Cert Building**
License Number:	**CBC054575**
Status:	**Current, Active**
Licensure Date:	**12/05/1991**
Expires:	**08/31/2006**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
Qualified Business License Required	04/13/2004

View Related License Information

View License Complaint

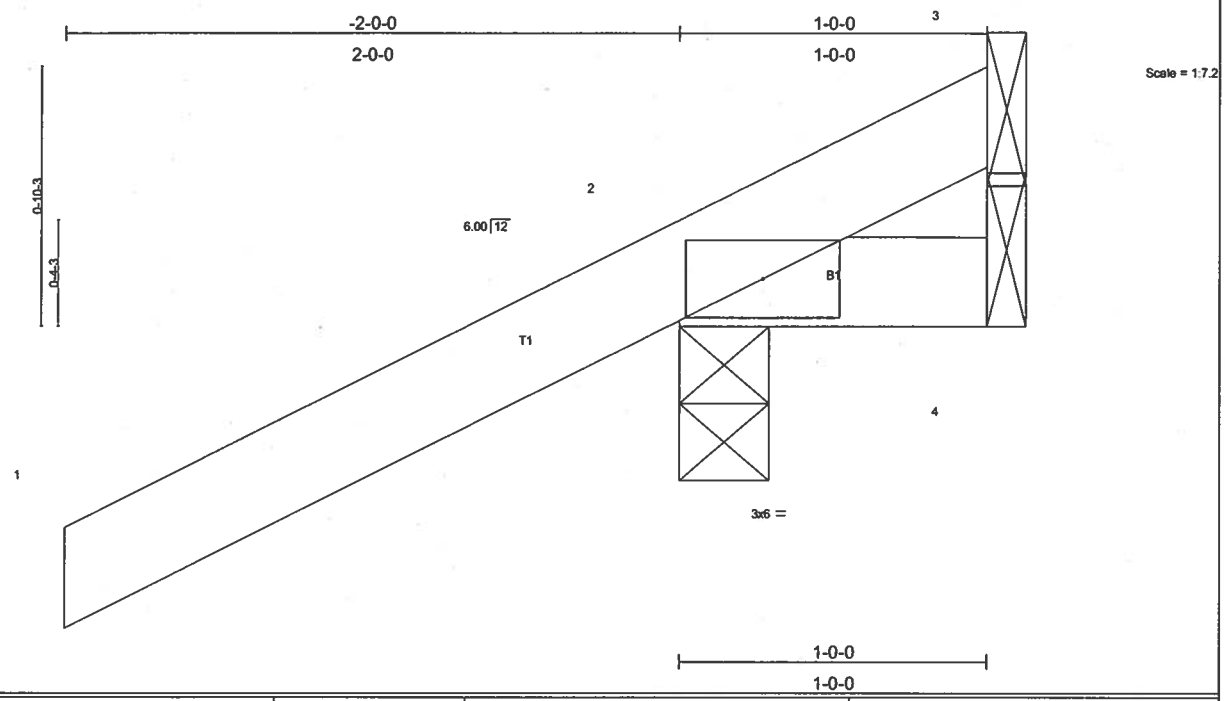
New Search

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061212

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	CJ1	MONO TRUSS	14	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:18 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-286(load case 5), 4=-9(load case 3), 3=-90(load case 1)
Max Grav 2=266(load case 1), 4=14(load case 1), 3=127(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearings are assumed to be:
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3.

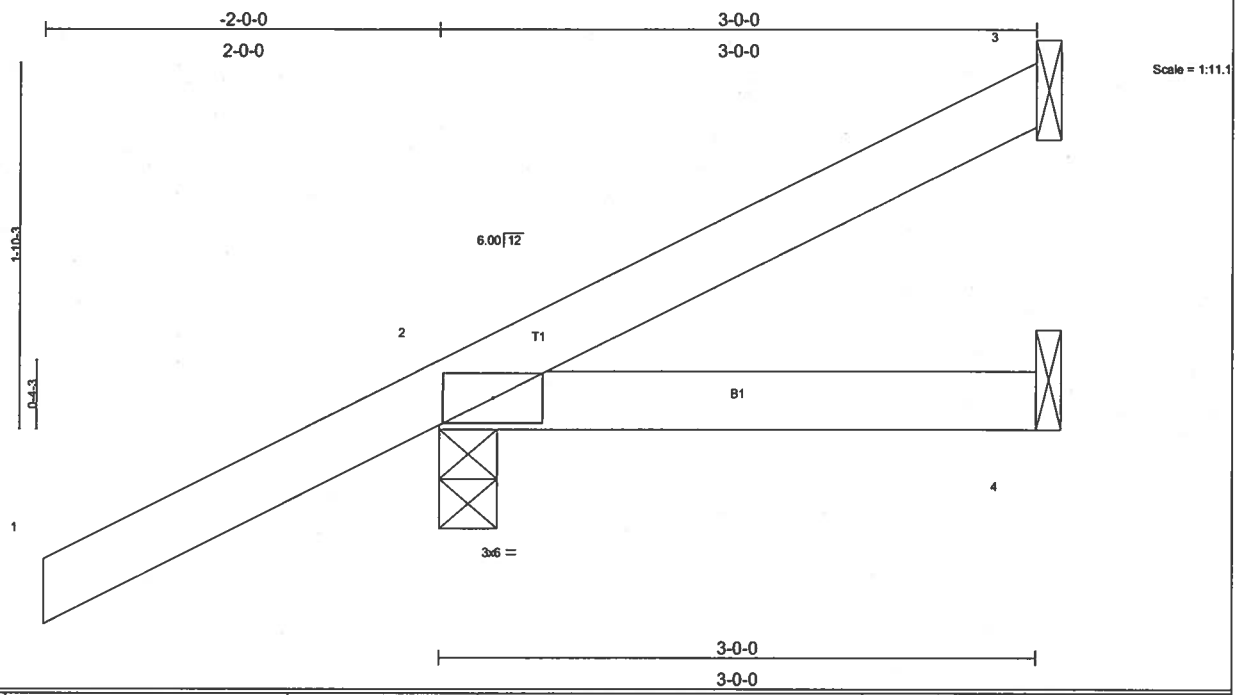
LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061213

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	CJ3	MONO TRUSS	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:19 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-28(load case 6), 2=-238(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearings are assumed to be:
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061214

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	CJ5	MONO TRUSS	6	1	Job Reference (optional)

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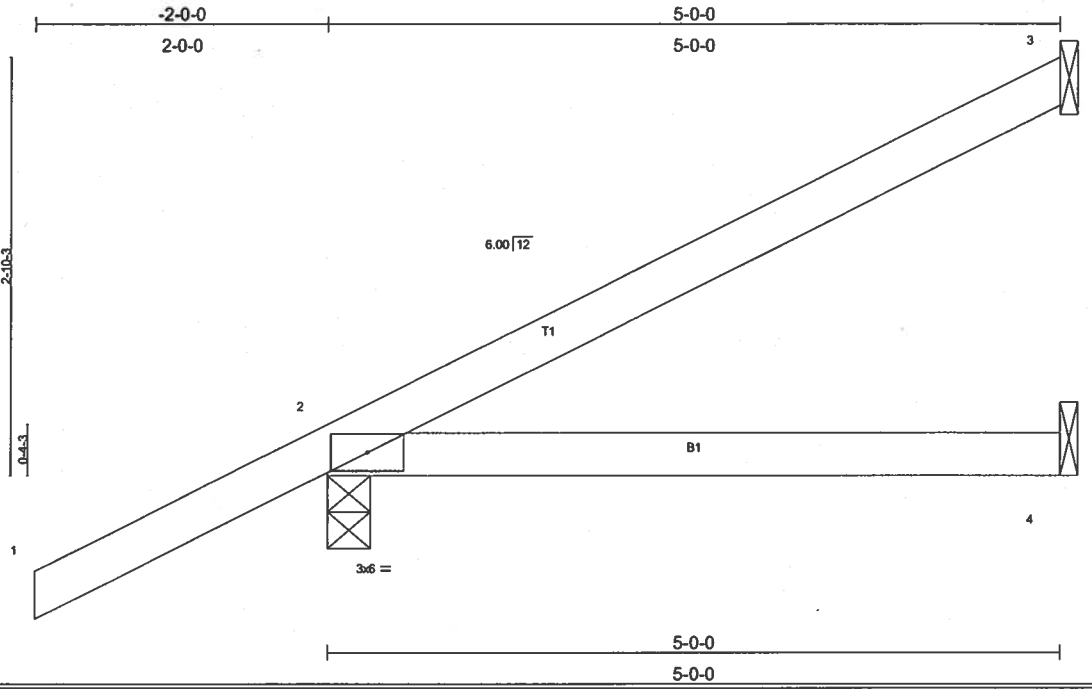

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-199(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearings are assumed to be:
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3 and 199 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061215

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	EJ3	MONO TRUSS	7	1	Job Reference (optional)

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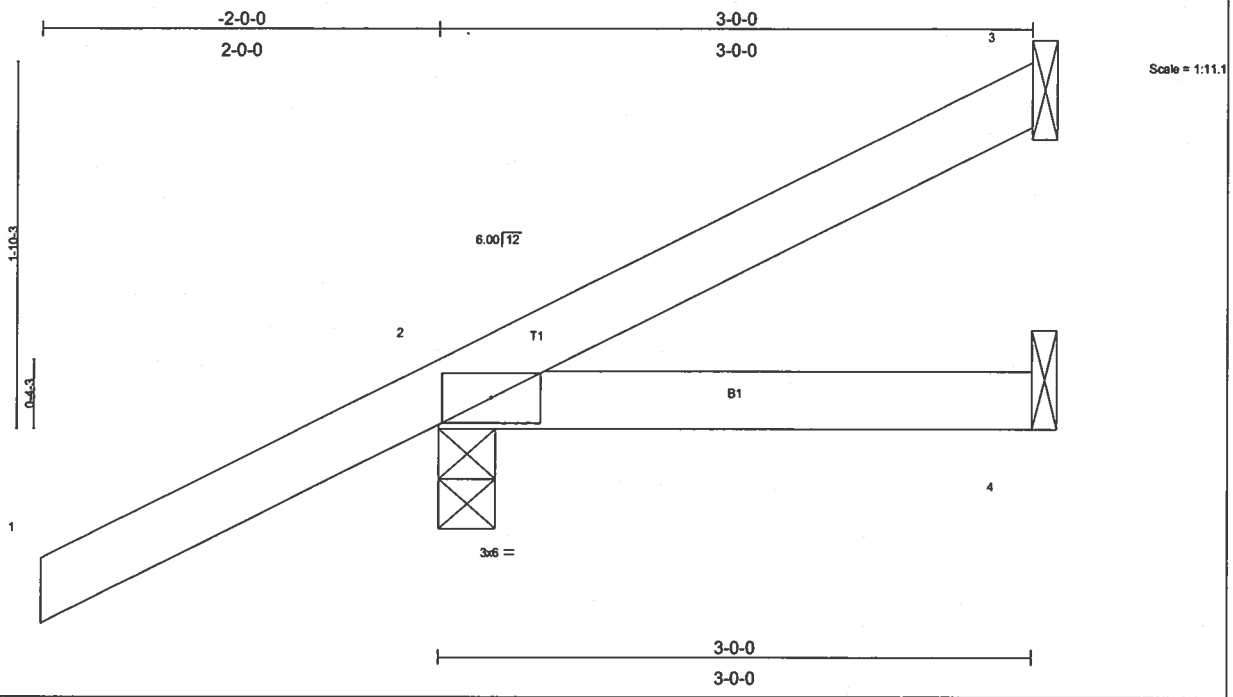

Plate Offsets (X,Y): [2:0-0-0,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-28(load case 6), 2=-238(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061216

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	EJ5	MONO TRUSS	3	1	Job Reference (optional)

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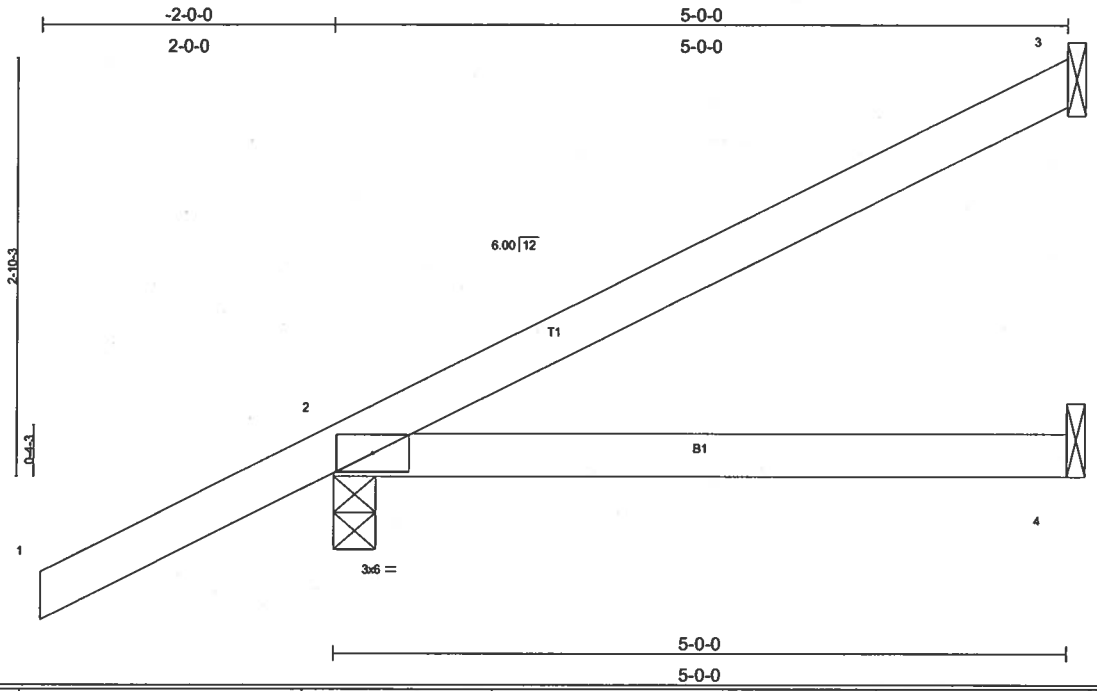

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.09	2-4	>663	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>774	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-260(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearings are assumed to be:
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061217

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	EJ7	MONO TRUSS	24	1	Job Reference (optional)

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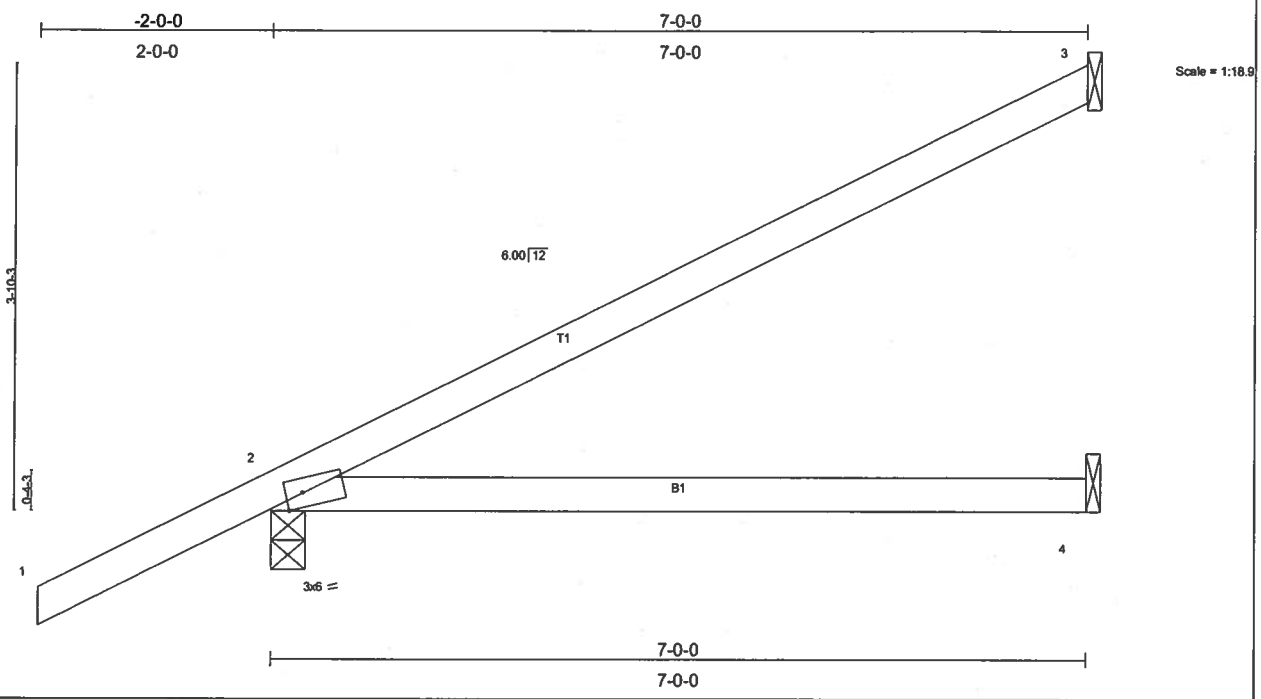

Plate Offsets (X,Y): [2:0-1-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.12	2-4	>664	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.21	2-4	>397	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=419/0-3-8, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-134(load case 5), 2=-210(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-119/58
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.79

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearings are assumed to be:
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 3 and 210 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061218

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	EJ7A	COMMON	3	1	Job Reference (optional)

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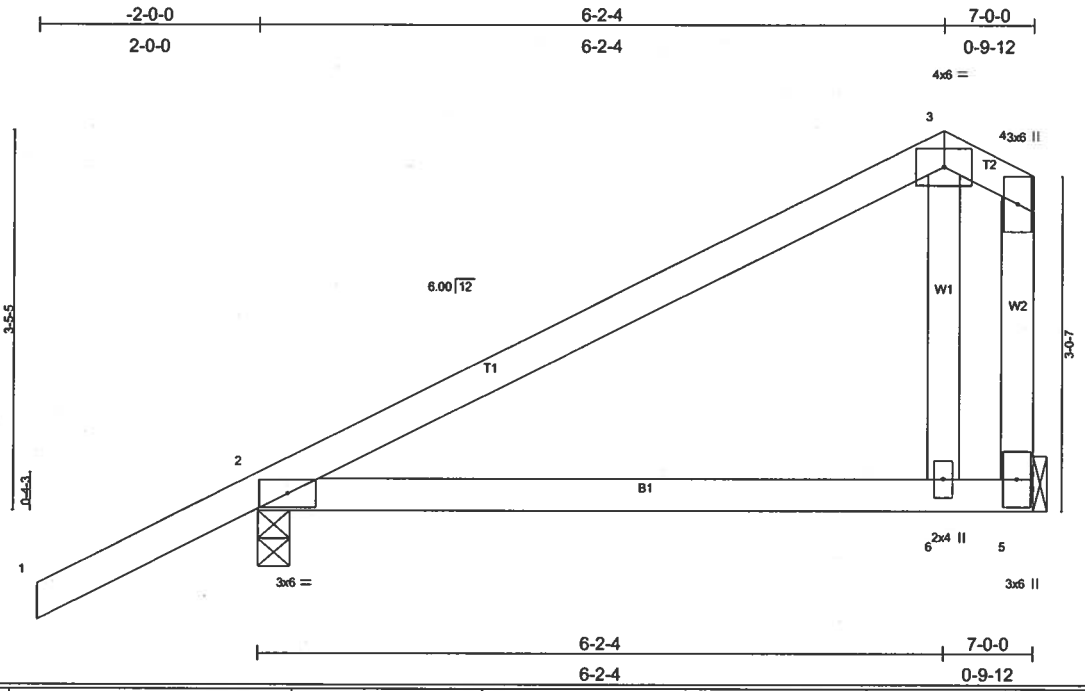

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.05	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.09	2-6	>896	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 34 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=416/0-3-8, 5=263/Mechanical
Max Horz 2=196(load case 5)
Max Uplift2=-221(load case 5), 5=-103(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-194/8, 3-4=-143/46, 4-5=-129/0
BOT CHORD 2-6=-71/107, 5-6=-71/107
WEBS 3-6=-41/249

JOINT STRESS INDEX
2 = 0.29, 3 = 0.65, 4 = 0.43, 5 = 0.45 and 6 = 0.14

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Bearings are assumed to be:
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint 2 and 103 lb uplift at joint 5.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061219

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	HJ3	MONO TRUSS	2	1	Job Reference (optional)

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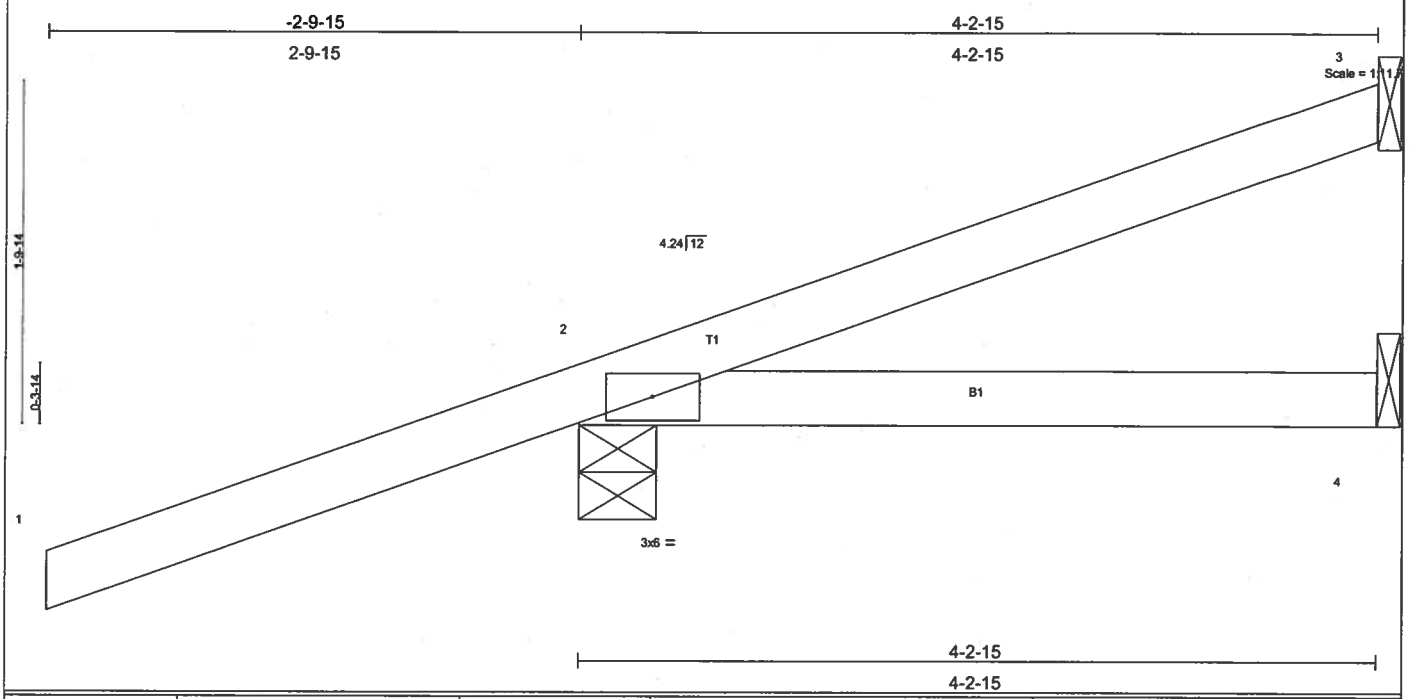

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	0.02	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=15/Mechanical, 2=289/0-4-15, 4=42/Mechanical
Max Horz 2=98(load case 2)
Max Uplift3=-6(load case 4), 2=-302(load case 2), 4=-41(load case 2)
Max Grav 3=32(load case 5), 2=289(load case 1), 4=42(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-37/10
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Bearings are assumed to be:
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 6 lb uplift at joint 3, 302 lb uplift at joint 2 and 41 lb uplift at joint 4.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-3=-57(F=-2, B=-2), 2=-0(F=15, B=15)-to-4=-32(F=-1, B=-1)

MAY 31, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061220

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	HJ5	MONO TRUSS	2	1	Job Reference (optional)

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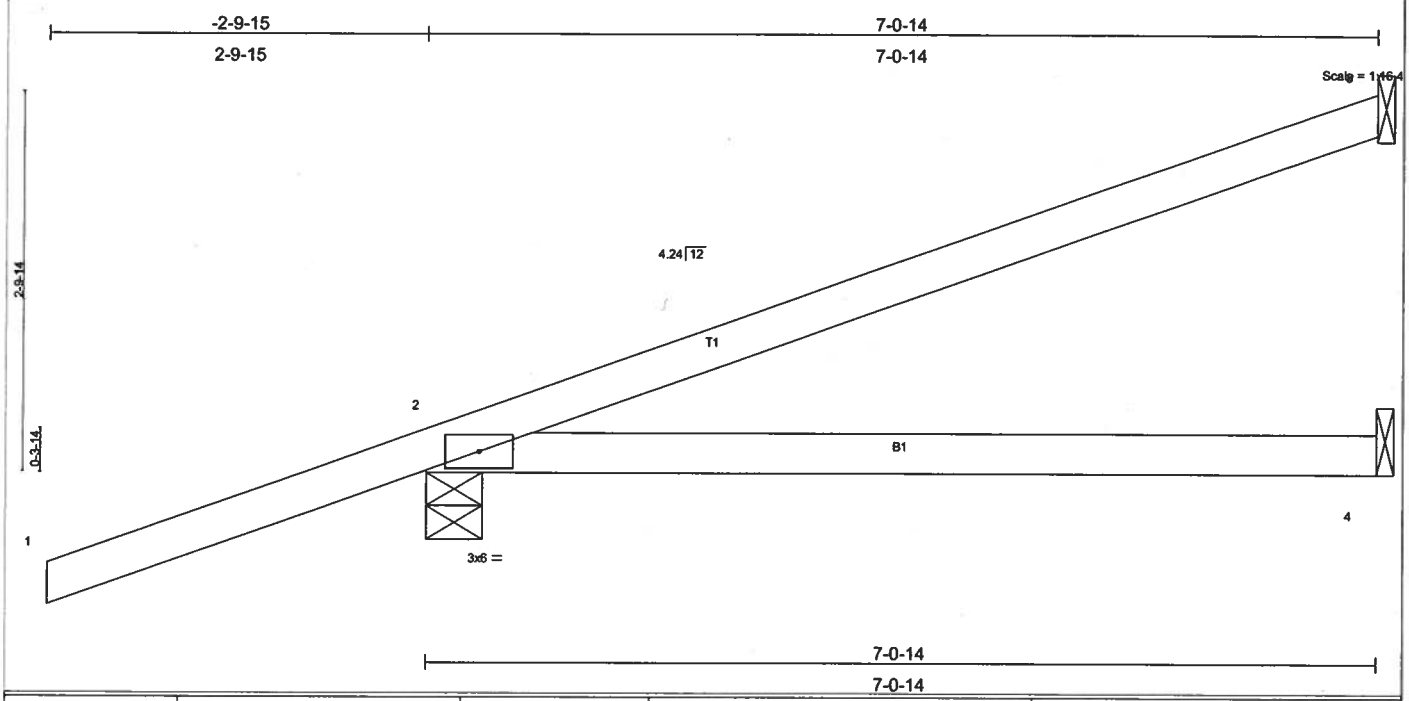

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	0.11	2-4	>744	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.13	2-4	>609	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-0-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=192/Mechanical, 2=375/0-4-15, 4=111/Mechanical
Max Horz 2=167(load case 2)
Max Uplift3=-155(load case 2), 2=-333(load case 2), 4=-55(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-71/45
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.52

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 155 lb uplift at joint 3, 333 lb uplift at joint 2 and 55 lb uplift at joint 4.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-3=-95(F=-21, B=-21), 2=0(F=15, B=15)-to-4=-53(F=-12, B=-12)

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061221

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	HJ7	MONO TRUSS	3	1	Job Reference (optional)

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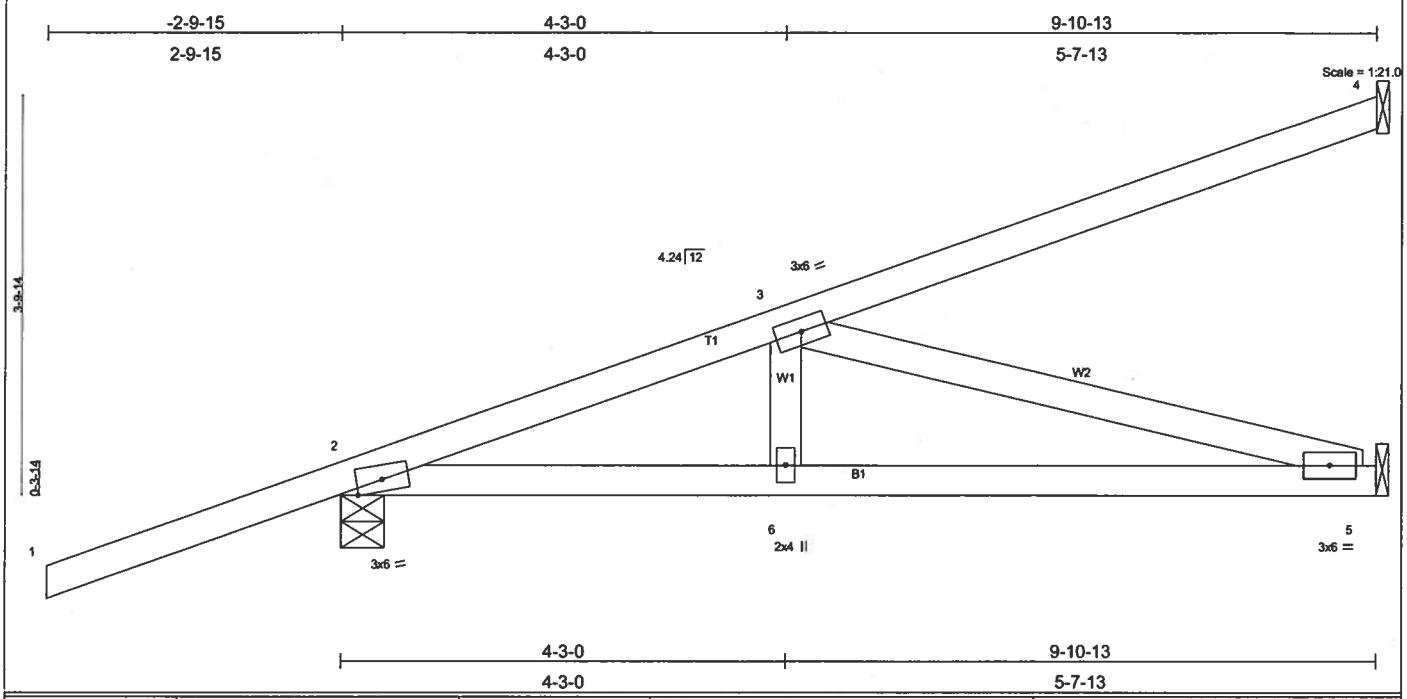


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.08	5-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.14	5-6	>818	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.51	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=531/0-4-15, 5=377/Mechanical
Max Horz 2=269(load case 2)
Max Uplift4=-231(load case 2), 2=-278(load case 2), 5=-64(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-877/116, 3-4=-105/65
BOT CHORD 2-6=-304/814, 5-6=-304/814
WEBS 3-6=0/163, 3-5=-844/316

JOINT STRESS INDEX
2 = 0.76, 3 = 0.22, 5 = 0.24 and 6 = 0.12

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Bearings are assumed to be:
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 4, 278 lb uplift at joint 2 and 64 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

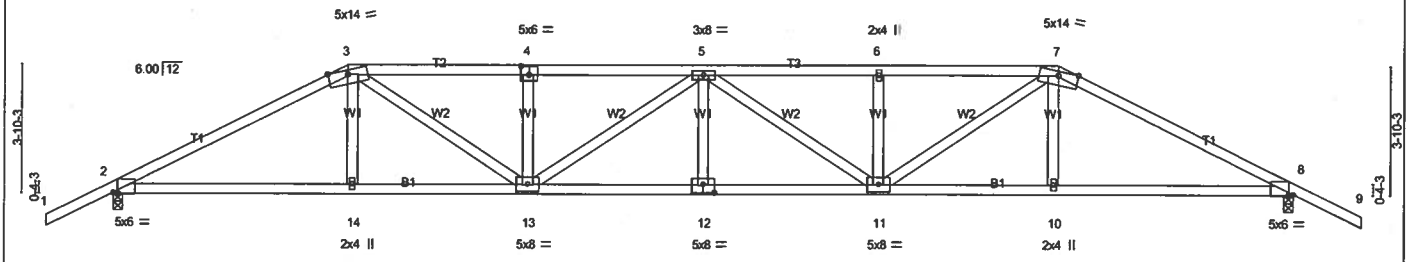
Dwg.#0531061222

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T01	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:27 2006 Page 1

-2-0-0	3-9-4	7-0-0	12-4-10	17-7-8	22-10-6	28-3-0	31-5-12	35-3-0	37-3-0
2-0-0	3-9-4	3-2-12	5-4-10	5-2-14	5-2-14	5-4-10	3-2-12	3-9-4	2-0-0

Scale = 1:65.8



3-9-4	7-0-0	12-4-10	17-7-8	22-10-6	28-3-0	31-5-12	35-3-0
3-9-4	3-2-12	5-4-10	5-2-14	5-2-14	5-4-10	3-2-12	3-9-4

Plate Offsets (X,Y): [2:0-1-11,Edge], [4:0-3-0,0-3-0], [8:0-1-11,Edge], [12:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.40	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.63	11-12	>662	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.43	Horz(TL)	0.18	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 348 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-6 oc bracing.

REACTIONS (lb/size) 2=3169/0-3-8, 8=3169/0-3-8
Max Horz 2=87(load case 4)
Max Uplift2=-1336(load case 4), 8=-1336(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-6163/2622, 3-4=-7612/3367, 4-5=-7617/3372, 5-6=-7615/3369, 6-7=-7615/3369, 7-8=-6163/2622, 8-9=0/47
BOT CHORD 2-14=-2293/5407, 13-14=-2302/5441, 12-13=-3577/8314, 11-12=-3577/8314, 10-11=-2261/5441, 8-10=-2251/5407
WEBS 3-14=-225/831, 3-13=-1257/2692, 4-13=-645/539, 5-13=-872/386, 5-12=0/340, 5-11=-876/389, 6-11=-645/540, 7-11=-1261/2696, 7-10=-224/831

JOINT STRESS INDEX
2 = 0.76, 3 = 0.72, 4 = 0.52, 5 = 0.57, 6 = 0.34, 7 = 0.72, 8 = 0.76, 10 = 0.34, 11 = 0.62, 12 = 0.91, 13 = 0.62 and 14 = 0.34

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1336 lb uplift at joint 2 and 1336 lb uplift at joint 8.
7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 28-3-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-118(F=-64), 7-9=-54, 2-14=-30, 10-14=-65(F=-35), 8-10=-30
Concentrated Loads (lb)
Vert: 14=-539(F) 10=-539(F)

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

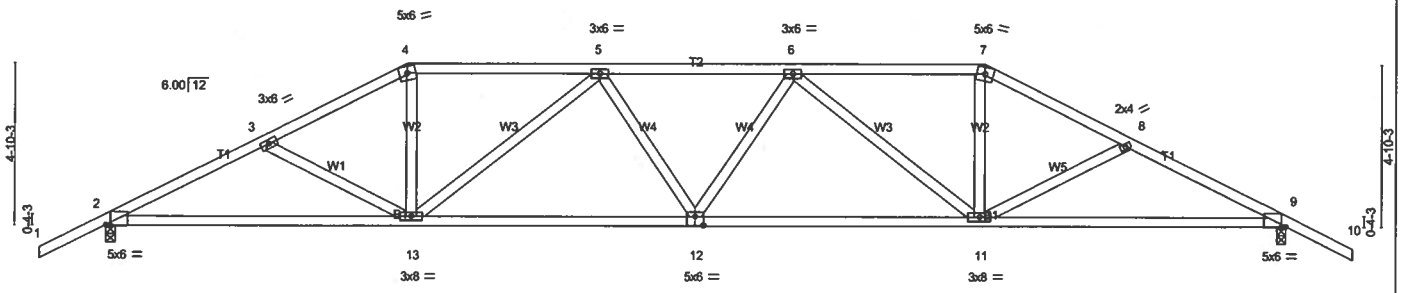
Dwg.#0531061223

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T02	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:28 2006 Page 1

-2-0-0	4-9-4	9-0-0	14-9-0	20-6-0	26-3-0	30-5-12	35-3-0	37-3-0
2-0-0	4-9-4	4-2-12	5-9-0	5-9-0	5-9-0	4-2-12	4-9-4	2-0-0

Scale = 1:65.8



0-3-8	9-0-0	17-7-8	26-3-0	35-3-0
0-3-8	8-8-8	8-7-8	8-7-8	9-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [9:0-1-11,Edge], [12:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.28	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.45	11-12	>927	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.14	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 178 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-7-14 oc bracing.

REACTIONS (lb/size) 2=1584/0-3-8, 9=1584/0-3-8
Max Horz 2=101(load case 5)
Max Uplift2=-537(load case 5), 9=-537(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2695/1092, 3-4=-2475/998, 4-5=-2197/953, 5-6=-2805/1184, 6-7=-2197/953, 7-8=-2477/998, 8-9=-2698/1095, 9-10=0/47
BOT CHORD 2-13=-807/2353, 12-13=-888/2729, 11-12=-888/2728, 9-11=-810/2356
WEBS 3-13=-210/194, 4-13=-228/807, 5-13=-762/349, 5-12=0/191, 6-12=0/191, 6-11=-762/349, 7-11=-225/803, 8-11=-211/195

JOINT STRESS INDEX
2 = 0.68, 3 = 0.29, 4 = 0.50, 5 = 0.43, 6 = 0.43, 7 = 0.50, 8 = 0.34, 9 = 0.68, 11 = 0.57, 12 = 0.80 and 13 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 537 lb uplift at joint 2 and 537 lb uplift at joint 9.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061224

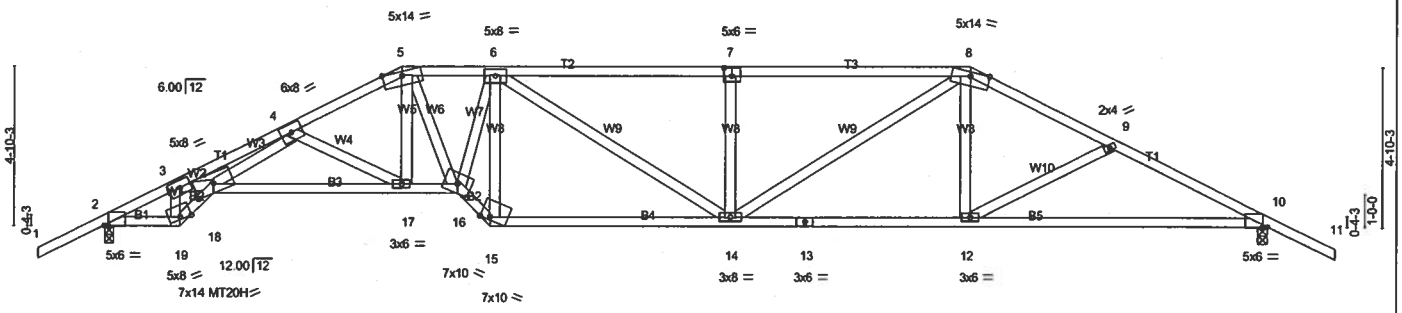
Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T02T	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:30 2006 Page 1

-2-0-0 | 2-3-8 | 5-7-12 | 9-0-0 | 11-8-0 | 18-11-8 | 26-3-0 | 30-5-12 | 35-3-0 | 37-3-0

2-0-0 | 2-3-8 | 3-4-4 | 3-4-4 | 2-8-0 | 7-3-8 | 7-3-8 | 4-2-12 | 4-9-4 | 2-0-0

Scale = 1:66.6



11-8-0

2-3-8 | 3-3-8 | 9-0-0 | 10-8-0 | 18-11-8 | 26-3-0 | 35-3-0

2-3-8 | 1-0-0 | 5-8-8 | 1-8-01-0-0 | 7-3-8 | 7-3-8 | 9-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-3-0,0-3-0], [10:0-1-11,Edge], [15:0-3-11,Edge], [16:0-5-0,0-3-4], [18:0-7-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.39	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-0.63	14-15	>666	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.31	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 195 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.1D, B2 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 2=1584/0-3-8, 10=1584/0-3-8
Max Horz 2=101(load case 5)
Max Uplift2=-537(load case 5), 10=-537(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2702/962, 3-4=-6137/2072, 4-5=-3103/1210, 5-6=-3000/1241, 6-7=-2845/1227, 7-8=-2849/1230, 8-9=-2479/1004, 9-10=-2691/1093, 10-11=0/47
BOT CHORD 2-19=-696/2282, 18-19=-818/2749, 17-18=-1198/3595, 16-17=-813/2755, 15-16=-1074/3291, 14-15=-803/2566, 13-14=-647/2190, 12-13=-647/2190, 10-12=-807/2349
WEBS 3-19=-1774/594, 3-18=-1071/3456, 4-18=-625/2435, 4-17=-959/435, 5-17=-207/756, 5-16=-306/616, 6-16=-490/1599, 6-15=-1996/756, 6-14=-174/412, 7-14=-417/292, 8-14=-361/865, 8-12=-29/350, 9-12=-195/184

JOINT STRESS INDEX
2 = 0.69, 3 = 0.81, 4 = 0.78, 5 = 0.73, 6 = 0.98, 7 = 0.65, 8 = 0.84, 9 = 0.34, 10 = 0.68, 12 = 0.35, 13 = 0.83, 14 = 0.83, 15 = 0.95, 16 = 0.78, 17 = 0.49, 18 = 0.73 and 19 = 0.89

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 537 lb uplift at joint 2 and 537 lb uplift at joint 10.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

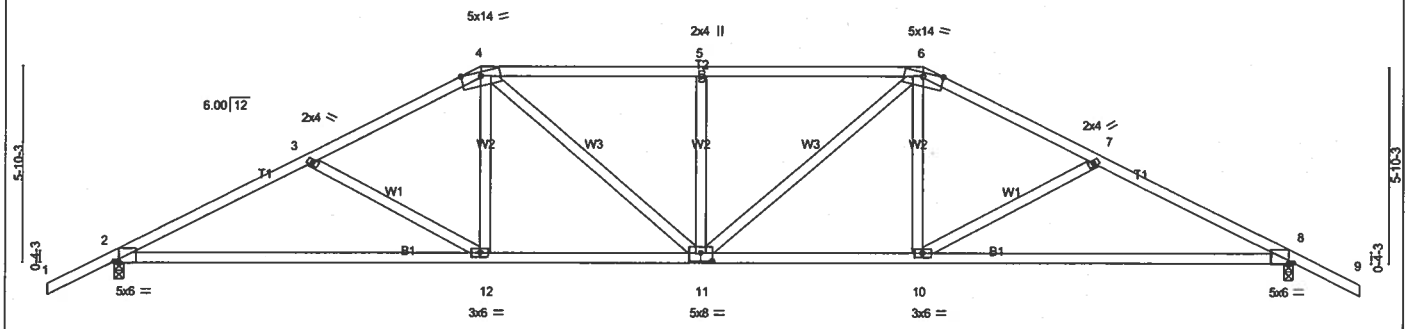
Dwg.#0531061225

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T03	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:31 2006 Page 1

-2-0-0	5-9-4	11-0-0	17-7-8	24-3-0	29-5-12	35-3-0	37-3-0
2-0-0	5-9-4	5-2-12	6-7-8	6-7-8	5-2-12	5-9-4	2-0-0

Scale = 1:65.8



0-3-8	11-0-0	17-7-8	24-3-0	29-5-12	35-3-0
0-3-8	10-8-8	6-7-8	6-7-8	5-2-12	5-9-4

Plate Offsets (X,Y): [2:0-1-11,Edge], [8:0-1-11,Edge], [11:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.40	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.67	2-12	>622	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.30	Horz(TL)	0.13	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 181 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-11-9 oc bracing.

REACTIONS (lb/size) 2=1584/0-3-8, 8=1584/0-3-8
Max Horz 2=-115(load case 6)
Max Uplift2=-555(load case 5), 8=-555(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2635/1107, 3-4=-2329/971, 4-5=-2328/1070, 5-6=-2328/1070, 6-7=-2329/971, 7-8=-2635/1107, 8-9=0/47
BOT CHORD 2-12=-811/2304, 11-12=-570/2037, 10-11=-570/2037, 8-10=-811/2304
WEBS 3-12=-322/275, 4-12=-66/461, 4-11=-241/492, 5-11=-381/266, 6-11=-241/492, 6-10=-66/461, 7-10=-322/275

JOINT STRESS INDEX
2 = 0.79, 3 = 0.34, 4 = 0.72, 5 = 0.34, 6 = 0.72, 7 = 0.34, 8 = 0.79, 10 = 0.35, 11 = 0.44 and 12 = 0.35

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 555 lb uplift at joint 2 and 555 lb uplift at joint 8.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

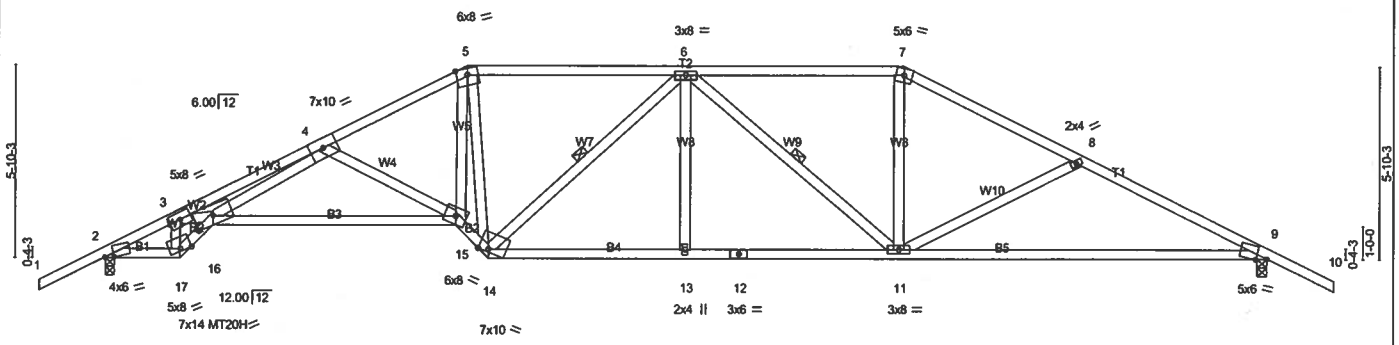
Dwg.#0531061226

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T03T	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:32 2006 Page 1

-2-0-0	2-3-8	6-7-12	11-0-0	17-7-8	24-3-0	29-5-12	35-3-0	37-3-0
2-0-0	2-3-8	4-4-4	4-4-4	6-7-8	6-7-8	5-2-12	5-9-4	2-0-0

Scale = 1:66.6



2-3-8	3-3-8	10-8-0	11-8-0	17-7-8	24-3-0	35-3-0
2-3-8	1-0-0	7-4-8	1-0-0	5-11-8	6-7-8	11-0-0

Plate Offsets (X,Y): [2:0-3-5,0-0-11], [9:0-3-13,Edge], [14:0-3-11,Edge], [16:0-7-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	-0.53	15-16	>789	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.86	15-16	>487	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.99	Horz(TL)	0.36	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 196 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-9-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 6-14, 6-11

REACTIONS (lb/size) 2=1584/0-3-8, 9=1584/0-3-8
Max Horz 2=115(load case 5)
Max Uplift2=-555(load case 5), 9=-555(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2679/993, 3-4=-6258/2111, 4-5=-2748/1118, 5-6=-2060/952, 6-7=-2044/935, 7-8=-2331/970, 8-9=-2641/1112, 9-10=0/47
BOT CHORD 2-17=-729/2262, 16-17=-861/2788, 15-16=-1075/3183, 14-15=-920/3268, 13-14=-708/2330, 12-13=-708/2330, 11-12=-708/2330, 9-11=-817/2311
WEBS 3-17=-1876/631, 3-16=-1062/3589, 4-16=-770/2951, 4-15=-880/438, 5-15=-823/2854, 5-14=-1951/552, 6-14=-477/216, 6-13=0/125, 6-11=-490/236, 7-11=-179/698, 8-11=-325/278

JOINT STRESS INDEX
2 = 0.88, 3 = 0.84, 4 = 0.92, 5 = 0.87, 6 = 0.57, 7 = 0.56, 8 = 0.34, 9 = 0.87, 11 = 0.57, 12 = 0.84, 13 = 0.34, 14 = 0.94, 15 = 0.73, 16 = 0.70 and 17 = 0.88

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 555 lb uplift at joint 2 and 555 lb uplift at joint 9.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

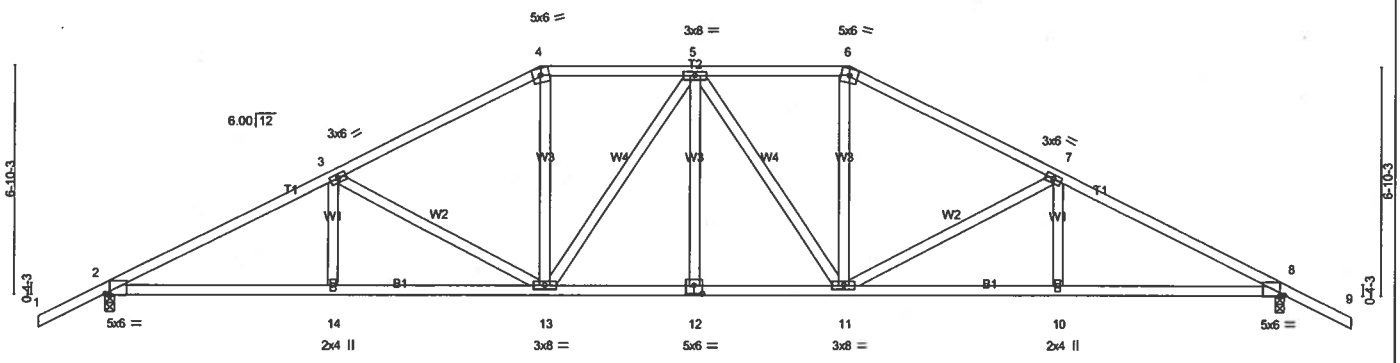
Dwg.#0531061227

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T04	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:34 2006 Page 1

-2-0-0 | 6-9-4 | 13-0-0 | 17-7-8 | 22-3-0 | 28-5-12 | 35-3-0 | 37-3-0
2-0-0 | 6-9-4 | 6-2-12 | 4-7-8 | 4-7-8 | 6-2-12 | 6-9-4 | 2-0-0

Scale = 1:65.8



0-3-8 | 6-9-4 | 13-0-0 | 22-3-0 | 28-5-12 | 35-3-0
0-3-8 | 6-5-12 | 6-2-12 | 9-3-0 | 6-2-12 | 6-9-4

Plate Offsets (X,Y): [2:0-1-11,Edge], [8:0-1-11,Edge], [12:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.19	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.30	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.13	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 197 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-1-7 oc bracing.

REACTIONS (lb/size) 2=1584/0-3-8, 8=1584/0-3-8
Max Horz 2=129(load case 5)
Max Uplift2=-571(load case 5), 8=-571(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2726/1087, 3-4=-2126/945, 4-5=-1844/914, 5-6=-1844/914, 6-7=-2126/945, 7-8=-2726/1087, 8-9=0/47
BOT CHORD 2-14=-788/2353, 13-14=-788/2353, 12-13=-538/1951, 11-12=-538/1951, 10-11=-788/2353, 8-10=-788/2353
WEBS 3-14=0/214, 3-13=-600/328, 4-13=-177/609, 5-13=-328/148, 5-11=-328/148, 6-11=-177/609, 7-11=-600/328, 7-10=0/214, 5-12=0/111

JOINT STRESS INDEX
2 = 0.69, 3 = 0.41, 4 = 0.52, 5 = 0.58, 6 = 0.52, 7 = 0.41, 8 = 0.69, 10 = 0.34, 11 = 0.58, 12 = 0.47, 13 = 0.58 and 14 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 571 lb uplift at joint 2 and 571 lb uplift at joint 8.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061228

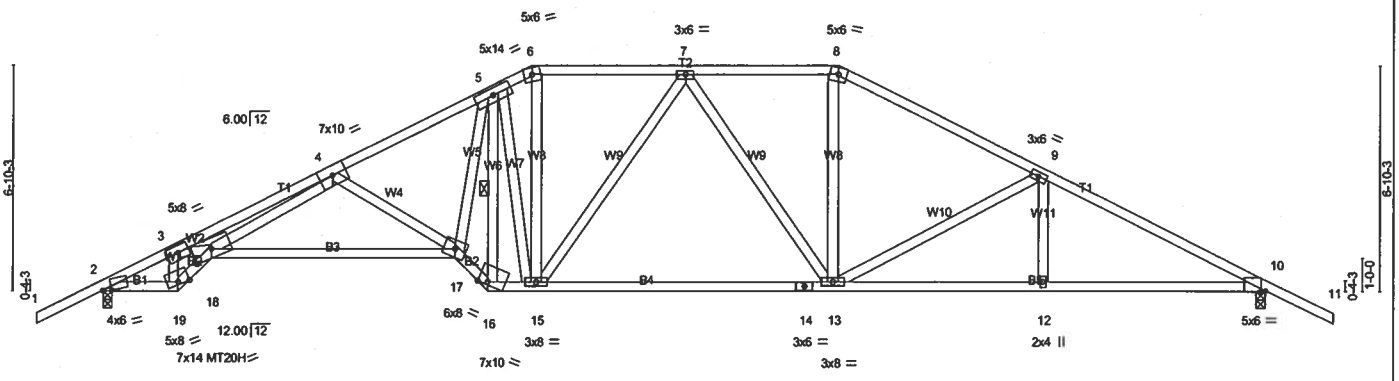
Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T04T	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:35 2006 Page 1

-2-0-0 | 2-3-8 | 6-11-12 | 11-8-0 | 13-0-0 | 17-7-8 | 22-3-0 | 28-5-12 | 35-3-0 | 37-3-0

2-0-0 | 2-3-8 | 4-8-4 | 4-8-4 | 1-4-0 | 4-7-8 | 4-7-8 | 6-2-12 | 6-9-4 | 2-0-0

Scale = 1:66.6



13-0-0

2-3-8 | 3-3-8 | 10-8-0 | 11-8-0 | 22-3-0 | 28-5-12 | 35-3-0

2-3-8 | 1-0-0 | 7-4-8 | 1-0-0 | 9-3-0 | 6-2-12 | 6-9-4

1-4-0

Plate Offsets (X,Y): [2:0-3-5,0-0-11], [10:0-1-11,Edge], [16:0-3-11,Edge], [18:0-7-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.95	Vert(LL)	-0.51	17-18	>817	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.83	17-18	>505	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.93	Horz(TL)	0.34	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 215 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-16

REACTIONS (lb/size) 2=1584/0-3-8, 10=1584/0-3-8
Max Horz 2=129(load case 5)
Max Uplift2=-571(load case 5), 10=-571(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2684/1013, 3-4=-6181/2132, 4-5=-2813/1162, 5-6=-2038/978, 6-7=-1850/899, 7-8=-1857/911, 8-9=-2141/942, 9-10=-2718/1089, 10-11=0/47
BOT CHORD 2-19=-748/2264, 18-19=-879/2767, 17-18=-1077/3136, 16-17=-722/2669, 15-16=-564/1945, 14-15=-546/1928, 13-14=-546/1928, 12-13=-789/2347, 10-12=-789/2347
WEBS 3-19=-1832/637, 3-18=-1057/3510, 4-18=-775/2886, 4-17=-819/444, 5-17=-706/2501, 5-16=-1762/382, 5-15=-470/348, 6-15=-306/691, 7-15=-252/162, 7-13=-264/163, 8-13=-181/620, 9-13=-577/333, 9-12=0/192

JOINT STRESS INDEX
2 = 0.88, 3 = 0.82, 4 = 0.68, 5 = 0.82, 6 = 0.43, 7 = 0.42, 8 = 0.51, 9 = 0.41, 10 = 0.69, 12 = 0.34, 13 = 0.58, 14 = 0.77, 15 = 0.60, 16 = 0.78, 17 = 0.76, 18 = 0.69 and 19 = 0.88

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 571 lb uplift at joint 2 and 571 lb uplift at joint 10.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

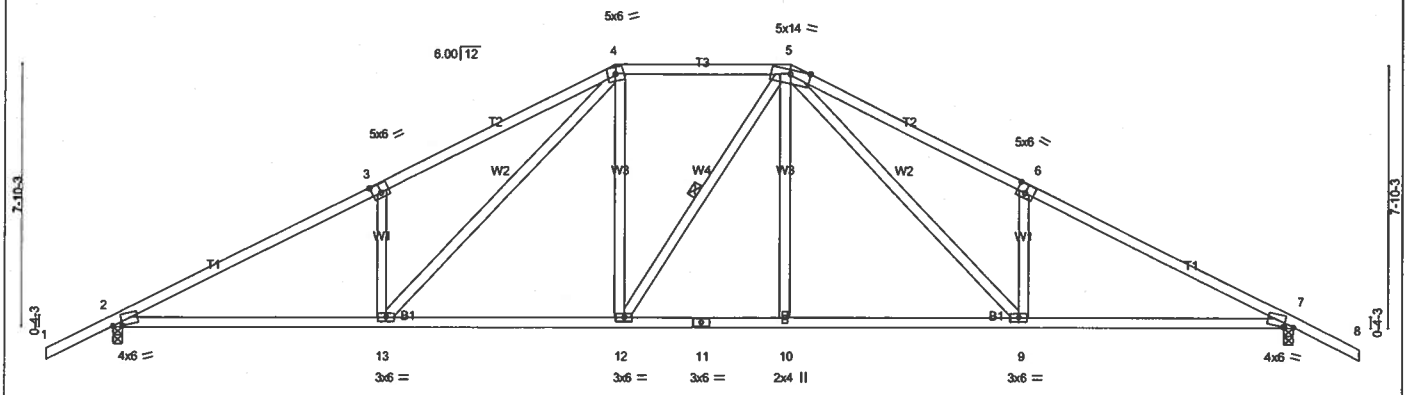
Dwg.#0531061229

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T05	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:37 2006 Page 1

-2-0-0 | 8-0-0 | 15-0-0 | 20-3-0 | 27-3-0 | 35-3-0 | 37-3-0
2-0-0 | 8-0-0 | 7-0-0 | 5-3-0 | 7-0-0 | 8-0-0 | 2-0-0

Scale = 1:65.8



0-3-8 | 8-0-0 | 15-0-0 | 20-3-0 | 27-3-0 | 35-3-0
0-3-8 | 7-8-8 | 7-0-0 | 5-3-0 | 7-0-0 | 8-0-0

Plate Offsets (X,Y): [2:0-3-5,0-0-11], [3:0-3-0,0-3-4], [6:0-3-0,0-3-4], [7:0-3-5,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.21	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.34	7-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.94	Horz(TL)	0.11	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 193 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-1-7 oc bracing.
WEBS 1 Row at midpt 5-12

REACTIONS (lb/size) 2=1584/0-3-8, 7=1584/0-3-8
Max Horz 2=-143(load case 6)
Max Uplift2=-586(load case 5), 7=-586(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2670/1087, 3-4=-2652/1292, 4-5=-1680/886, 5-6=-2652/1292, 6-7=-2670/1087, 7-8=0/47
BOT CHORD 2-13=-774/2297, 12-13=-424/1677, 11-12=-424/1680, 10-11=-424/1680, 9-10=-425/1676, 7-9=-774/2297
WEBS 3-13=-401/413, 4-13=-501/900, 4-12=-35/316, 5-12=-173/175, 5-10=0/172, 5-9=-500/901, 6-9=-401/412

JOINT STRESS INDEX
2 = 0.87, 3 = 0.77, 4 = 0.55, 5 = 0.55, 6 = 0.77, 7 = 0.87, 9 = 0.59, 10 = 0.34, 11 = 0.57, 12 = 0.43 and 13 = 0.59

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 586 lb uplift at joint 2 and 586 lb uplift at joint 7.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061230

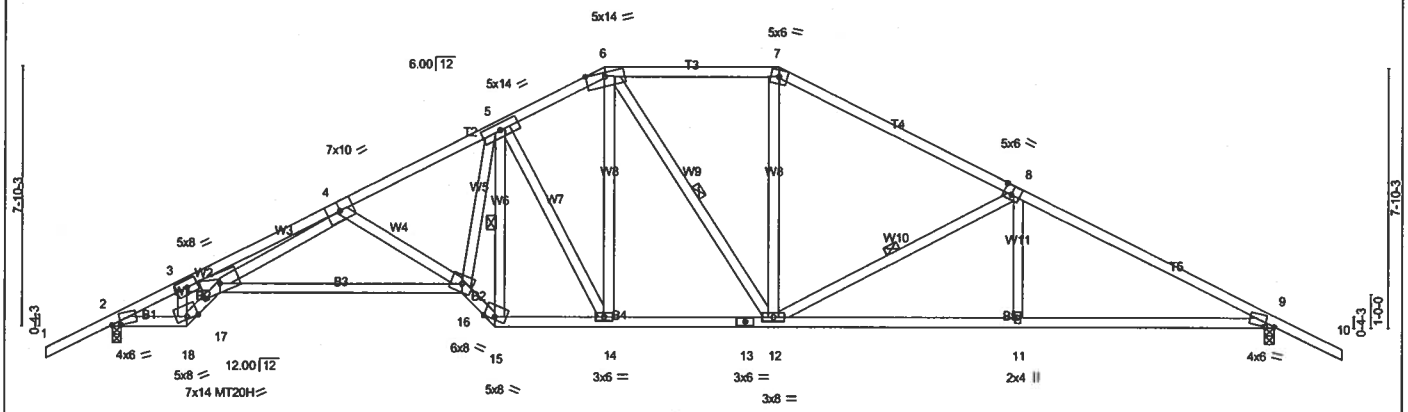
Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T05T	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:39 2006 Page 1

-2-0-0 | 2-3-8 | 7-1-12 | 11-8-0 | 15-0-0 | 20-3-0 | 27-5-12 | 35-3-0 | 37-3-0

2-0-0 | 2-3-8 | 4-10-4 | 4-6-4 | 3-4-0 | 5-3-0 | 7-2-12 | 7-9-4 | 2-0-0

Scale = 1:66.6



2-3-8 | 3-3-8 | 10-8-0 | 11-8-0 | 15-0-0 | 20-3-0 | 27-5-12 | 35-3-0

2-3-8 | 1-0-0 | 7-4-8 | 1-0-0 | 3-4-0 | 5-3-0 | 7-2-12 | 7-9-4

Plate Offsets (X,Y): [2:0-3-5,0-0-11], [8:0-3-0,0-3-4], [9:0-3-5,0-0-11], [17:0-7-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	-0.51	16-17	>824	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.82	16-17	>509	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(TL)	0.34	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 212 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-15, 6-12, 8-12

REACTIONS (lb/size) 2=1584/0-3-8, 9=1584/0-3-8
Max Horz 2=-143(load case 6)
Max Uplift2=-586(load case 5), 9=-586(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2684/1027, 3-4=-6183/2167, 4-5=-2815/1182, 5-6=-1877/934, 6-7=-1686/889, 7-8=-1968/907, 8-9=-2683/1091, 9-10=0/47
BOT CHORD 2-18=-759/2265, 17-18=-893/2768, 16-17=-1101/3146, 15-16=-745/2646, 14-15=-572/1936, 13-14=-409/1657, 12-13=-409/1657, 11-12=-780/2310, 9-11=-780/2310
WEBS 3-18=-1833/645, 3-17=-1079/3513, 4-17=-786/2879, 4-16=-822/447, 5-16=-737/2505, 5-15=-1717/483, 5-14=-592/345, 6-14=-256/571, 6-12=-127/201, 7-12=-127/493, 8-12=-723/403, 8-11=0/255

JOINT STRESS INDEX
2 = 0.88, 3 = 0.82, 4 = 0.57, 5 = 0.83, 6 = 0.50, 7 = 0.64, 8 = 0.75, 9 = 0.88, 11 = 0.34, 12 = 0.59, 13 = 0.62, 14 = 0.46, 15 = 0.83, 16 = 0.76, 17 = 0.69 and 18 = 0.89

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 586 lb uplift at joint 2 and 586 lb uplift at joint 9.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061231

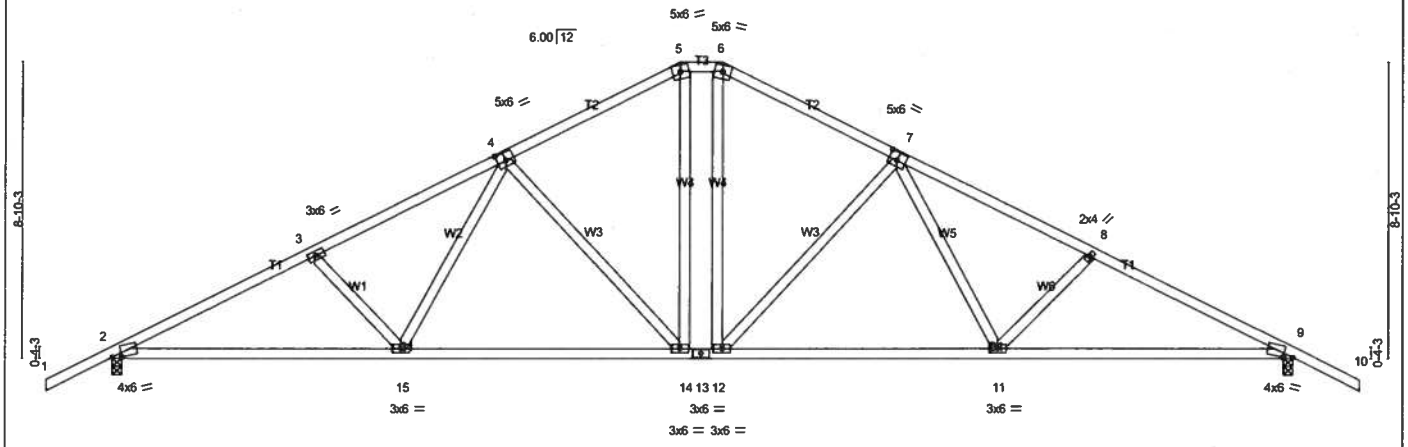
Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T06	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:40 2006 Page 1

-2-0-0 | 6-0-5 | 11-9-3 | 17-0-0 | 18-3-0 | 23-5-13 | 29-2-11 | 35-3-0 | 37-3-0

2-0-0 | 6-0-5 | 5-8-14 | 5-2-13 | 1-3-0 | 5-2-13 | 5-8-14 | 6-0-5 | 2-0-0

Scale = 1:65.8



0-3-8 | 8-7-12 | 17-0-0 | 18-3-0 | 26-5-12 | 35-3-0

0-3-8 | 8-4-4 | 8-4-4 | 1-3-0 | 8-2-12 | 8-9-4

Plate Offsets (X,Y): [2:0-3-5,0-0-11], [4:0-3-0,0-3-0], [7:0-3-0,0-3-0], [9:0-3-5,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.24	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.39	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.12	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 196 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-10-7 oc bracing.

REACTIONS (lb/size) 2=1584/0-3-8, 9=1584/0-3-8
Max Horz 2=157(load case 5)
Max Uplift2=-598(load case 5), 9=-598(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2703/1143, 3-4=-2491/1097, 4-5=-1749/884, 5-6=-1513/849, 6-7=-1749/884, 7-8=-2480/1091, 8-9=-2701/1145, 9-10=0/47
BOT CHORD 2-15=-844/2343, 14-15=-598/1918, 13-14=-336/1513, 12-13=-336/1513, 11-12=-598/1920, 9-11=-846/2342
WEBS 3-15=-271/264, 4-15=-154/550, 4-14=-634/392, 5-14=-262/588, 6-12=-265/580, 7-12=-637/392, 7-11=-147/541, 8-11=-275/266

JOINT STRESS INDEX
2 = 0.88, 3 = 0.38, 4 = 0.54, 5 = 0.38, 6 = 0.37, 7 = 0.55, 8 = 0.34, 9 = 0.88, 11 = 0.46, 12 = 0.38, 13 = 0.80, 14 = 0.38 and 15 = 0.45

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 598 lb uplift at joint 2 and 598 lb uplift at joint 9.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061232

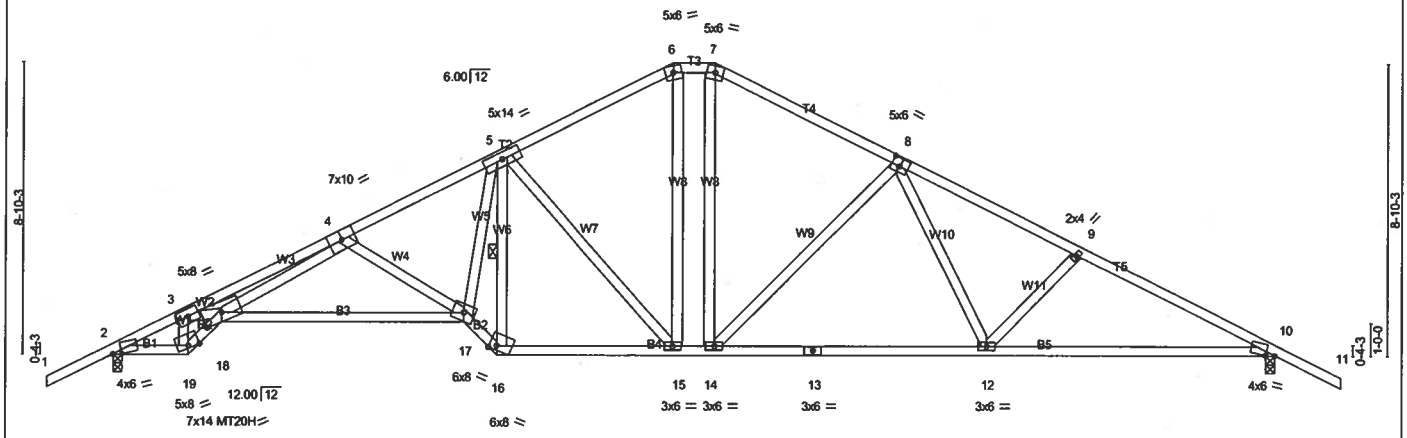
Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T06T	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:42 2006 Page 1

-2-0-0 | 2-3-8 | 6-11-12 | 11-8-0 | 17-0-0 | 18-3-0 | 23-9-0 | 29-2-11 | 35-3-0 | 37-3-0

2-0-0 | 2-3-8 | 4-8-4 | 4-8-4 | 5-4-0 | 1-3-0 | 5-6-0 | 5-5-11 | 6-0-5 | 2-0-0

Scale = 1:66.6



2-3-8 | 3-3-8 | 10-8-0 | 11-8-0 | 17-0-0 | 18-3-0 | 26-5-12 | 35-3-0

2-3-8 | 1-0-0 | 7-4-8 | 1-0-0 | 5-4-0 | 1-3-0 | 8-2-12 | 8-9-4

Plate Offsets (X,Y): [2:0-3-5,0-0-11], [8:0-3-0,0-3-0], [10:0-3-5,0-0-11], [16:0-2-11,Edge], [18:0-7-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.51	17-18	>827	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.82	17-18	>511	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.93	Horz(TL)	0.34	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 214 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-16

REACTIONS (lb/size) 2=1584/0-3-8, 10=1584/0-3-8
Max Horz 2=157(load case 5)
Max Uplift2=-598(load case 5), 10=-598(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2687/1045, 3-4=-6185/2202, 4-5=-2818/1205, 5-6=-1736/883, 6-7=-1508/850, 7-8=-1747/882, 8-9=-2482/1090, 9-10=-2700/1143, 10-11=0/47
BOT CHORD 2-19=-775/2268, 18-19=-912/2772, 17-18=-1116/3142, 16-17=-782/2656, 15-16=-591/1943, 14-15=-337/1508, 13-14=-606/1937, 12-13=-606/1937, 10-12=-844/2340
WEBS 3-19=-1837/658, 3-18=-1096/3510, 4-18=-804/2885, 4-17=-809/436, 5-17=-768/2504, 5-16=-1724/534, 5-15=-694/388, 6-15=-261/525, 7-14=-255/624, 8-14=-650/392, 8-12=-140/533, 9-12=-265/258

JOINT STRESS INDEX
2 = 0.88, 3 = 0.82, 4 = 0.55, 5 = 0.83, 6 = 0.46, 7 = 0.41, 8 = 0.54, 9 = 0.34, 10 = 0.88, 12 = 0.46, 13 = 0.86, 14 = 0.41, 15 = 0.39, 16 = 0.74, 17 = 0.76, 18 = 0.69 and 19 = 0.89

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 598 lb uplift at joint 2 and 598 lb uplift at joint 10.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061233

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T07	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:43 2006 Page 1

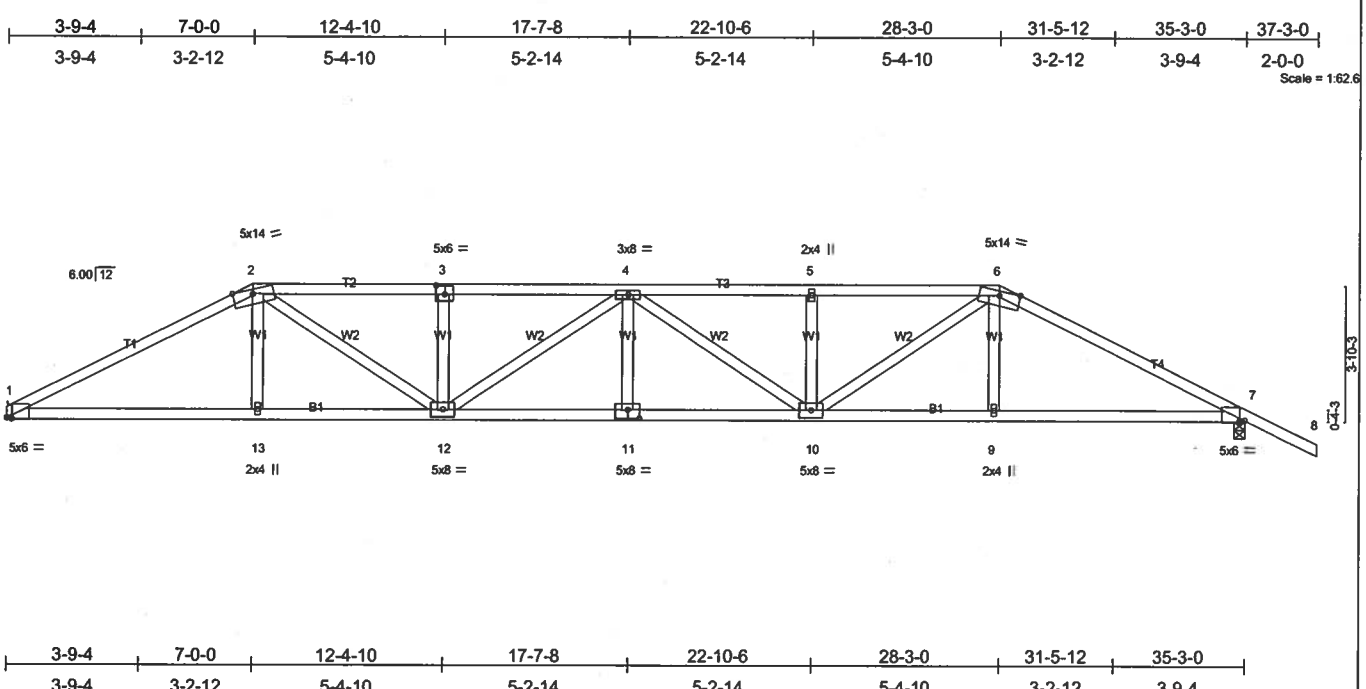


Plate Offsets (X,Y): [1:0-1-11,Edge], [3:0-3-0,0-3-0], [7:0-1-11,Edge], [11:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.40	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.64	11-12	>656	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.44	Horz(TL)	0.18	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 341 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-1 oc bracing.

REACTIONS (lb/size) 1=3058/Mechanical, 7=3183/0-3-8
Max Horz 1=-112(load case 5)
Max Uplift1=-1241(load case 3), 7=-1343(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6217/2650, 2-3=-7673/3393, 3-4=-7678/3398, 4-5=-7662/3393, 5-6=-7662/3393, 6-7=-6195/2639, 7-8=0/47
BOT CHORD 1-13=-2321/5478, 12-13=-2330/5513, 11-12=-3603/8373, 10-11=-3603/8373, 9-10=-2275/5469, 7-9=-2266/5435
WEBS 2-13=-242/848, 2-12=-1250/2695, 3-12=-646/539, 4-12=-873/386, 4-11=0/343, 4-10=-890/395, 5-10=-648/543, 6-10=-1271/2718, 6-9=-224/831

JOINT STRESS INDEX
1 = 0.76, 2 = 0.73, 3 = 0.52, 4 = 0.57, 5 = 0.34, 6 = 0.73, 7 = 0.76, 9 = 0.34, 10 = 0.63, 11 = 0.91, 12 = 0.63 and 13 = 0.34

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Refer to girder(s) for truss to truss connections.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1241 lb uplift at joint 1 and 1343 lb uplift at joint 7.
9) Girder carries hip end with 7-0-0 end setback.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 28-3-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-6=-118(F=-64), 6-8=-54, 1-13=-30, 9-13=-66(F=-36), 7-9=-30
Concentrated Loads (lb)
Vert: 13=-539(F) 9=-539(F)

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061234

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T07P	SPECIAL	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:44 2006 Page 1

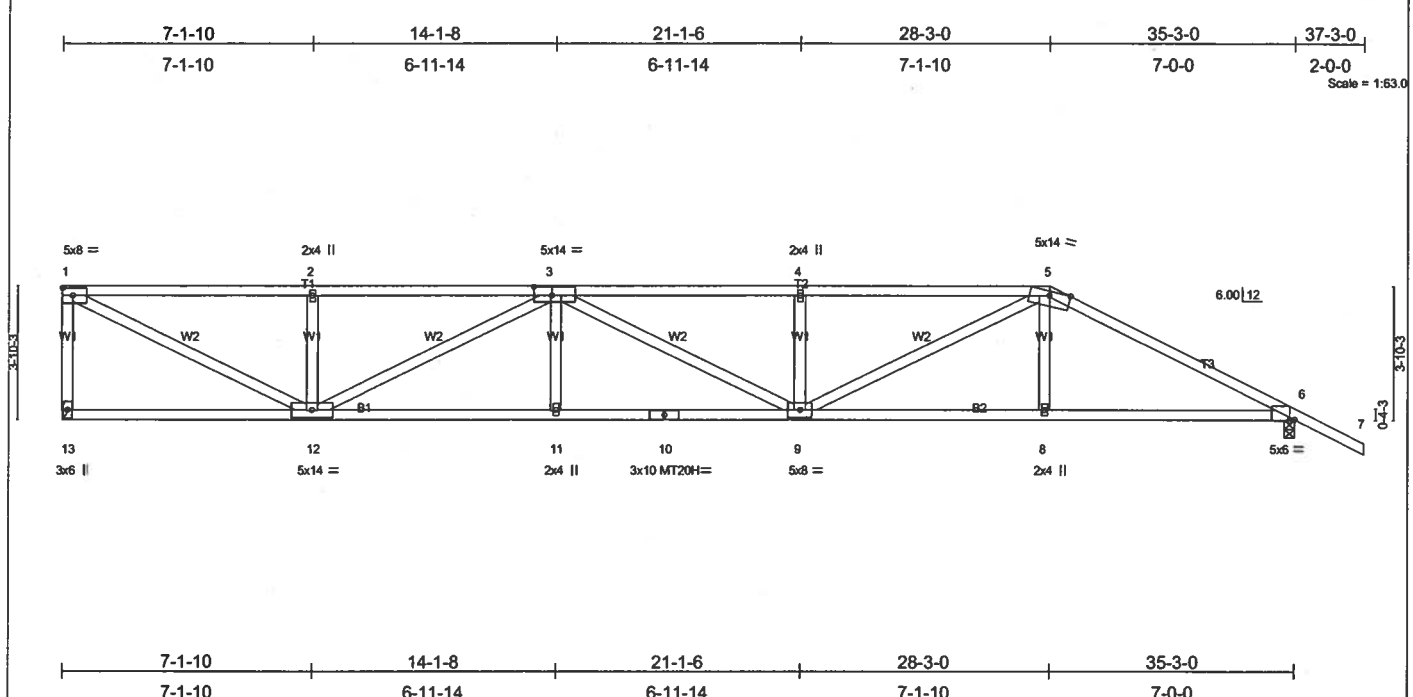


Plate Offsets (X,Y): [3:0-6-0,0-3-0], [6:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.75	Vert(LL)	-0.41	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.66	9-11	>635	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.88	Horz(TL)	0.15	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 356 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-9-7 oc bracing.

REACTIONS (lb/size) 13=3101/Mechanical, 6=3043/0-3-8
Max Horz 13=-226(load case 5)
Max Uplift13=-1321(load case 2), 6=-1203(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-13=-2874/1346, 1-2=-5077/2160, 2-3=-5077/2160, 3-4=-7665/3257, 4-5=-7665/3256, 5-6=-5929/2403, 6-7=0/47
BOT CHORD 12-13=0/184, 11-12=-3083/7528, 10-11=-3081/7543, 9-10=-3081/7543, 8-9=-2070/5241, 6-8=-2063/5205
WEBS 1-12=-2334/5490, 2-12=-792/628, 3-12=-2760/1177, 3-11=0/435, 3-9=-97/135, 4-9=-819/660, 5-9=-1245/2706, 5-8=-190/875

JOINT STRESS INDEX
1 = 0.56, 2 = 0.34, 3 = 0.66, 4 = 0.34, 5 = 0.82, 6 = 0.73, 8 = 0.34, 9 = 0.62, 10 = 0.90, 11 = 0.34, 12 = 0.65 and 13 = 0.46

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) Refer to girder(s) for truss to truss connections.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1321 lb uplift at joint 13 and 1203 lb uplift at joint 6.
9) Girder carries hip end with 7-0-0 right side setback, 0-0-0 left side setback, and 7-0-0 end setback.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 255 lb up at 28-3-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

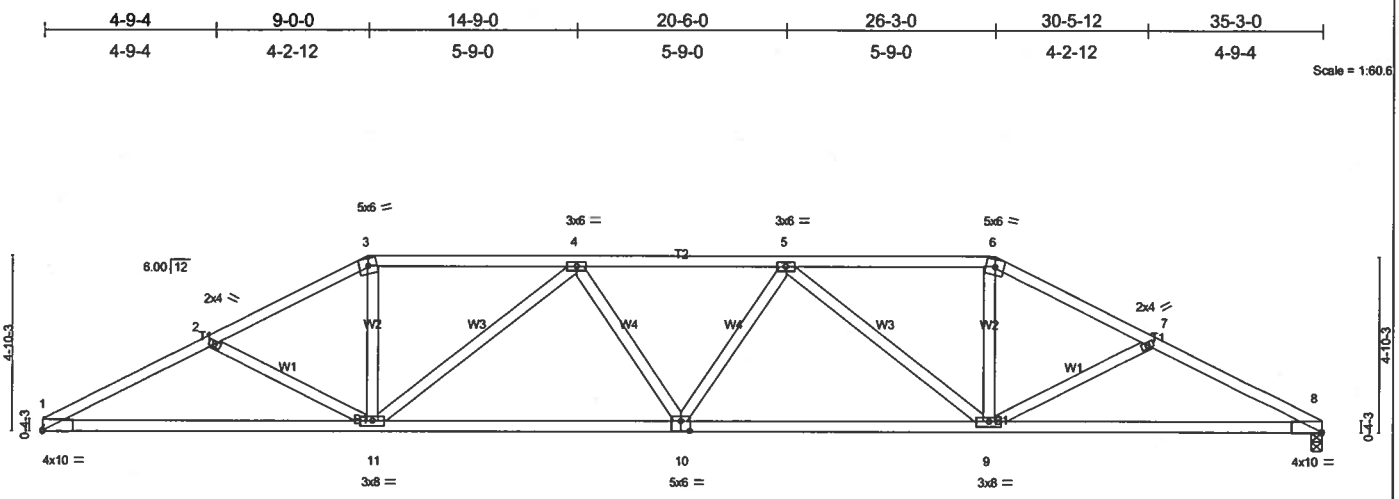
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-113(F=-58), 5-7=-54, 8-13=-62(F=-33), 6-8=-30
Concentrated Loads (lb)
Vert: 8=-539(F)

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061235

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T08	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:45 2006 Page 1



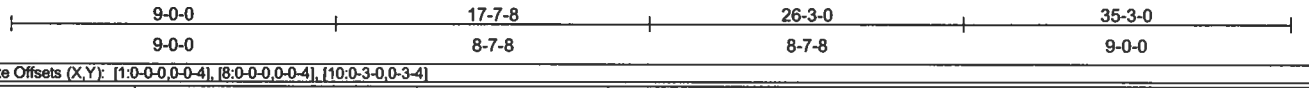


Plate Offsets (X,Y): [1:0-0-0,0-0-4], [8:0-0-0,0-0-4], [10:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.28	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.46	9-10	>920	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.15	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 171 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-3-1 oc bracing.

REACTIONS (lb/size) 1=1472/Mechanical, 8=1472/0-3-8
Max Horz 1=-66(load case 3)
Max Uplift1=-415(load case 5), 8=-414(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2790/1196, 2-3=-2534/1063, 3-4=-2247/1010, 4-5=-2845/1232, 5-6=-2233/1004, 6-7=-2519/1057, 7-8=-2757/1182
BOT CHORD 1-11=-997/2456, 10-11=-1018/2772, 9-10=-1016/2767, 8-9=-980/2417
WEBS 2-11=-268/256, 3-11=-264/835, 4-11=-760/346, 4-10=0/190, 5-10=0/197, 5-9=-770/348, 6-9=-261/827, 7-9=-240/243

JOINT STRESS INDEX
1 = 0.68, 2 = 0.34, 3 = 0.49, 4 = 0.43, 5 = 0.43, 6 = 0.49, 7 = 0.34, 8 = 0.68, 9 = 0.57, 10 = 0.79 and 11 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 415 lb uplift at joint 1 and 414 lb uplift at joint 8.

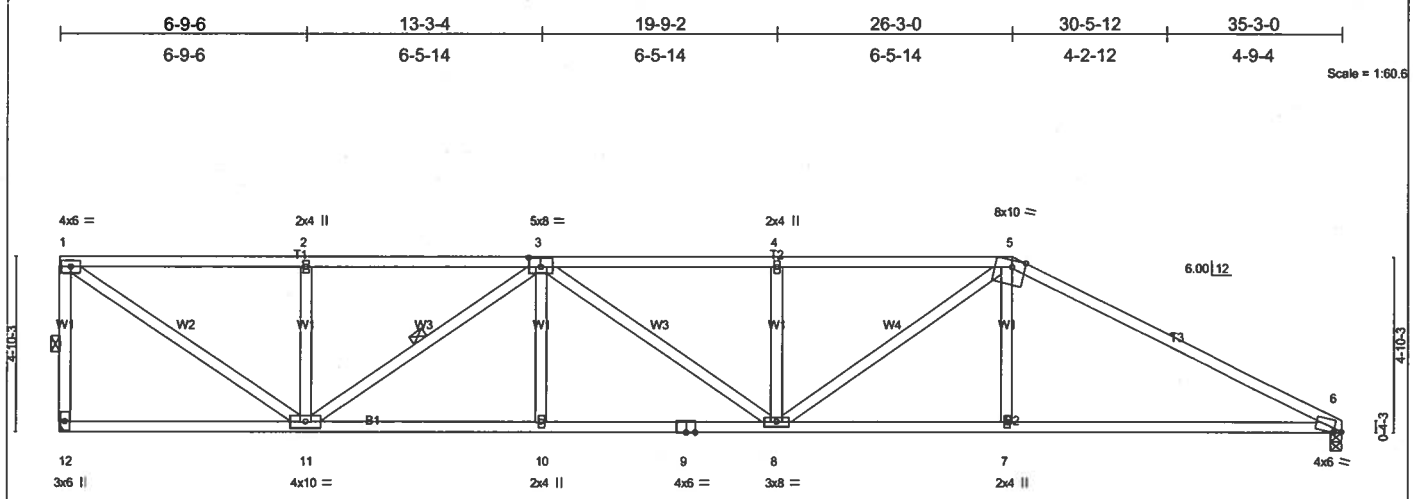
LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061236

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T08P	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:47 2006 Page 1



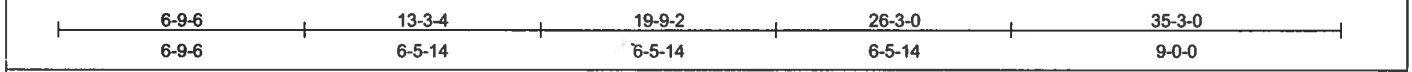


Plate Offsets (X,Y): [3:0-4-0,0-3-0], [5:0-4-3,Edge], [6:0-2-9,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	-0.35	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.57	6-7	>734	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.95	Horz(TL)	0.11	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 183 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-4-11 oc bracing.
WEBS 1 Row at midpt 1-12, 3-11

REACTIONS (lb/size) 12=1468/Mechanical, 6=1468/0-3-8
Max Horz 12=-210(load case 6)
Max Uplift12=-543(load case 3), 6=-408(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-12=-1366/605, 1-2=-1796/729, 2-3=-1796/729, 3-4=-2815/1177, 4-5=-2815/1177, 5-6=-2639/1029
BOT CHORD 11-12=0/209, 10-11=-883/2704, 9-10=-882/2705, 8-9=-882/2705, 7-8=-788/2272, 6-7=-788/2263
WEBS 1-11=-856/2110, 2-11=-362/264, 3-11=-1111/472, 3-10=0/204, 3-8=-76/134, 4-8=-302/255, 5-8=-369/663, 5-7=0/297

JOINT STRESS INDEX
1 = 0.87, 2 = 0.34, 3 = 0.40, 4 = 0.34, 5 = 0.99, 6 = 0.87, 7 = 0.34, 8 = 0.64, 9 = 0.90, 10 = 0.34, 11 = 1.00 and 12 = 0.38

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 543 lb uplift at joint 12 and 408 lb uplift at joint 6.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

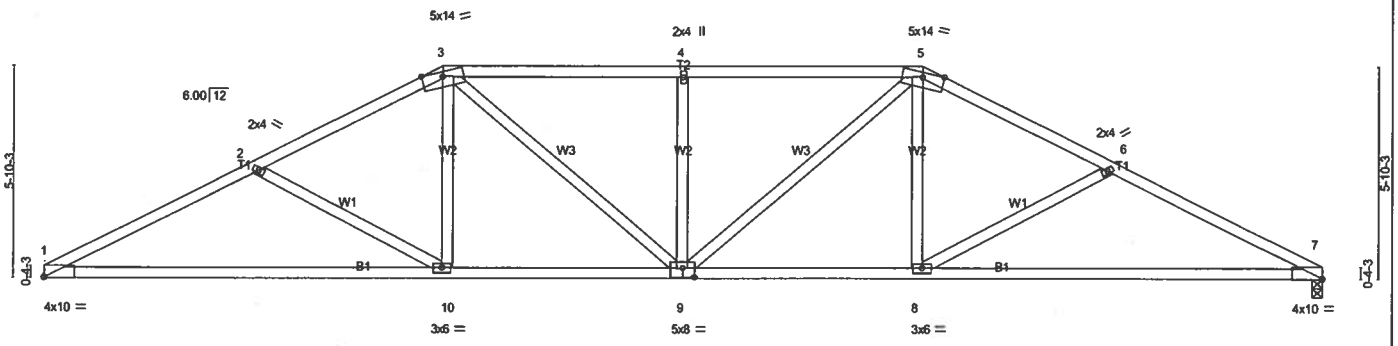
Dwg.#0531061237

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T09	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:48 2006 Page 1

5-9-4	11-0-0	17-7-8	24-3-0	29-5-12	35-3-0
5-9-4	5-2-12	6-7-8	6-7-8	5-2-12	5-9-4

Scale = 1:60.6



11-0-0	17-7-8	24-3-0	35-3-0
11-0-0	6-7-8	6-7-8	11-0-0

Plate Offsets (X,Y): [1:Edge,0-0-2], [7:Edge,0-0-2], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.44	1-10	>946	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.75	1-10	>560	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.30	Horz(TL)	0.13	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 174 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-8 oc bracing.

REACTIONS (lb/size) 1=1472/Mechanical, 7=1472/0-3-8
Max Horz 1=-81(load case 3)
Max Uplift1=-433(load case 5), 7=-433(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2709/1190, 2-3=-2376/1025, 3-4=-2360/1110, 4-5=-2360/1110, 5-6=-2366/1022, 6-7=-2685/1180
BOT CHORD 1-10=-978/2384, 9-10=-695/2077, 8-9=-691/2068, 7-8=-965/2356
WEBS 2-10=-367/325, 3-10=-105/494, 3-9=-237/490, 4-9=-382/268, 5-9=-239/500, 5-8=-100/481, 6-8=-345/314

JOINT STRESS INDEX
1 = 0.72, 2 = 0.34, 3 = 0.69, 4 = 0.34, 5 = 0.69, 6 = 0.34, 7 = 0.72, 8 = 0.35, 9 = 0.45 and 10 = 0.35

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 433 lb uplift at joint 1 and 433 lb uplift at joint 7.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

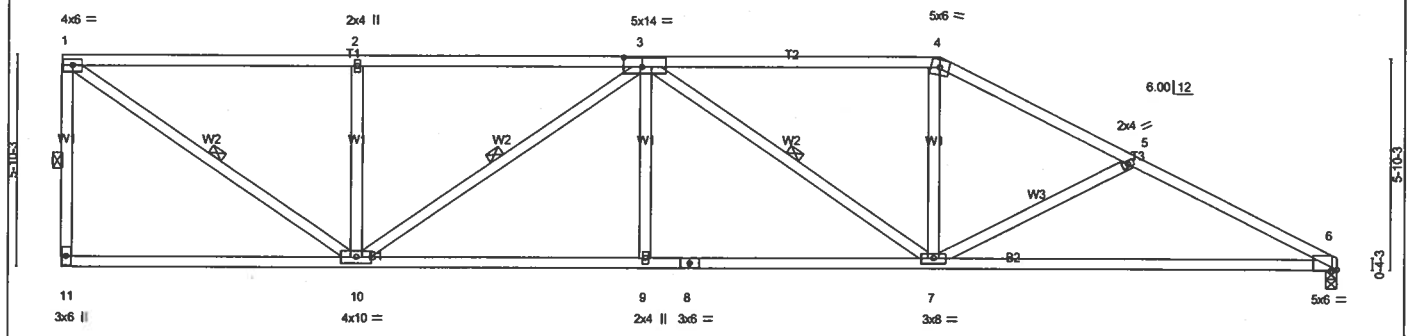
Dwg.#0531061238

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T09P	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:49 2006 Page 1

8-1-9 | 16-1-7 | 24-3-0 | 29-5-12 | 35-3-0
8-1-9 | 7-11-13 | 8-1-9 | 5-2-12 | 5-9-4

Scale = 1:60.7



8-1-9 | 16-1-7 | 24-3-0 | 35-3-0
8-1-9 | 7-11-13 | 8-1-9 | 11-0-0

Plate Offsets (X,Y): [3:0-6-0,0-3-0], [6:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.78	Vert(LL)	-0.43	6-7	>987	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.72	6-7	>585	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 189 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 1-11, 1-10, 3-10, 3-7

REACTIONS (lb/size) 11=1468/Mechanical, 6=1468/0-3-8
Max Horz 11=-256(load case 6)
Max Uplift11=-537(load case 3), 6=-418(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-11=-1351/618, 1-2=-1710/708, 2-3=-1710/708, 3-4=-2082/925, 4-5=-2369/958, 5-6=-2678/1116
BOT CHORD 10-11=0/258, 9-10=-727/2362, 8-9=-726/2366, 7-8=-726/2366, 6-7=-909/2350
WEBS 1-10=-832/2009, 2-10=-438/321, 3-10=-798/370, 3-9=0/212, 3-7=-345/247, 4-7=-142/660, 5-7=-320/304

JOINT STRESS INDEX
1 = 0.83, 2 = 0.34, 3 = 0.40, 4 = 0.75, 5 = 0.34, 6 = 0.69, 7 = 0.57, 8 = 0.81, 9 = 0.34, 10 = 0.95 and 11 = 0.46

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 537 lb uplift at joint 11 and 418 lb uplift at joint 6.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

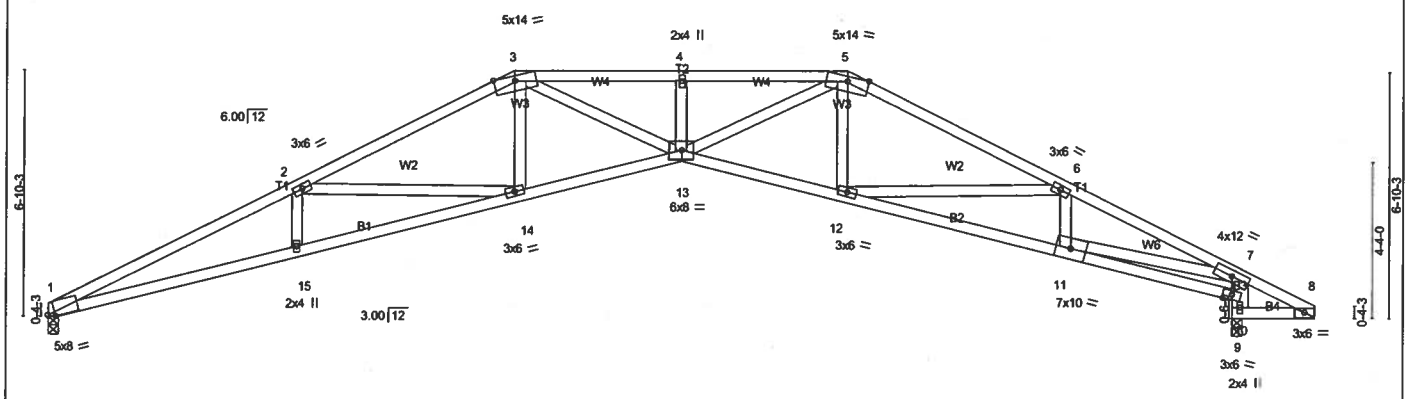
Dwg.#0531061239

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:50 2006 Page 1

6-10-15	13-0-0	17-7-8	22-3-0	28-4-1	32-11-8	35-3-0
6-10-15	6-1-1	4-7-8	4-7-8	6-1-1	4-7-7	2-3-8

Scale = 1:61.2



6-10-15	13-0-0	17-7-8	22-3-0	28-4-1	32-11-8	33-1-4
6-10-15	6-1-1	4-7-8	4-7-8	6-1-1	4-7-7	0-1-12

35-3-0

2-1-12

Plate Offsets (X,Y): [1:0-2-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.63	13-14	>627	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-1.01	13-14	>392	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.70	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 169 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B3 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-0-9 oc bracing.

REACTIONS (lb/size) 1=1382/0-3-8, 9=1566/0-3-8
Max Horz 1=96(load case 4)
Max Uplift1=-427(load case 5), 9=-532(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4771/1921, 2-3=-3740/1457, 3-4=-5074/1914, 4-5=-5074/1914, 5-6=-3494/1345, 6-7=-3509/1368, 7-8=-124/156
BOT CHORD 1-15=-1664/4318, 14-15=-1662/4314, 13-14=-1073/3404, 12-13=-970/3173, 11-12=-1139/3196, 10-11=-71/357, 9-10=-1521/706, 7-10=-1493/695, 8-9=-109/126
WEBS 2-15=0/207, 2-14=-903/575, 3-14=-147/460, 3-13=-600/1968, 4-13=-185/165, 5-13=-710/2215, 5-12=-44/237, 6-12=-127/226, 6-11=-245/207, 7-11=-1113/2752

JOINT STRESS INDEX
1 = 0.91, 2 = 0.41, 3 = 0.88, 4 = 0.34, 5 = 0.88, 6 = 0.41, 7 = 0.61, 8 = 0.23, 9 = 0.56, 10 = 0.79, 11 = 0.50, 12 = 0.38, 13 = 0.83, 14 = 0.38 and 15 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 427 lb uplift at joint 1 and 532 lb uplift at joint 9.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061240

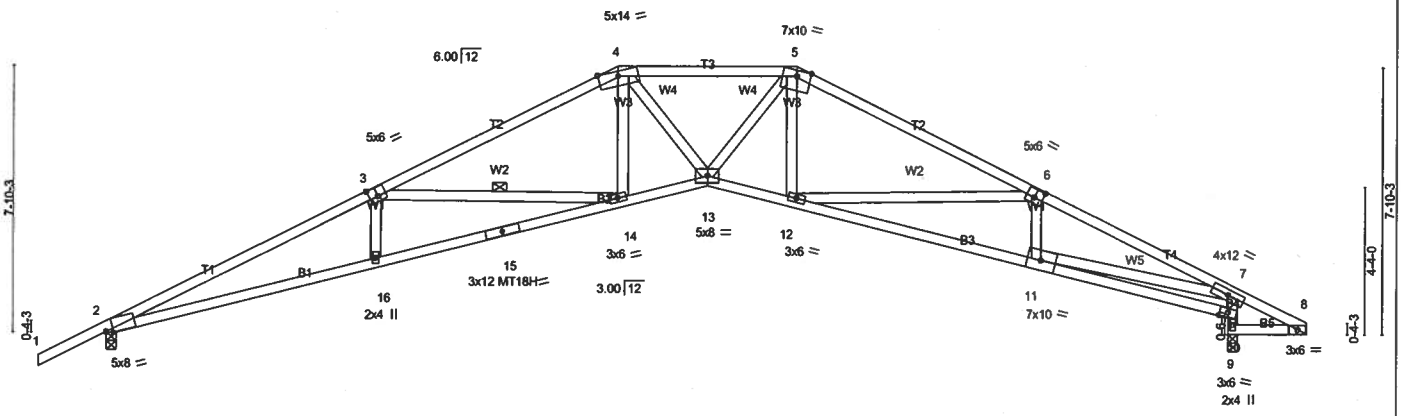
Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T11	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:52 2006 Page 1

-2-0-0 | 7-10-15 | 15-0-0 | 20-3-0 | 27-4-1 | 32-11-8 | 35-3-0

2-0-0 | 7-10-15 | 7-1-1 | 5-3-0 | 7-1-1 | 5-7-7 | 2-3-8

Scale = 1:64.7



35-3-0

7-10-15 | 15-0-0 | 17-7-8 | 20-3-0 | 27-4-1 | 32-11-8 | 33-11-4

7-10-15 | 7-1-1 | 2-7-8 | 2-7-8 | 7-1-1 | 5-7-7 | 0-1-12

2-1-12

Plate Offsets (X,Y): [2:0-2-7,Edge], [3:0-3-0,0-3-4], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.73	Vert(LL)	-0.53	14-16	>744	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.86	14-16	>462	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.59	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 173 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except* T1 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except* B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 3-14

REACTIONS (lb/size) 9=1567/0-3-8, 2=1498/0-3-8
Max Horz 2=167(load case 5)
Max Uplift9=-549(load case 6), 2=-565(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4636/1802, 3-4=-3373/1287, 4-5=-3463/1370, 5-6=-3261/1239, 6-7=-3600/1408, 7-8=-127/155
BOT CHORD 2-16=-1536/4192, 15-16=-1538/4161, 14-15=-1532/4188, 13-14=-872/3036, 12-13=-830/2930, 11-12=-1163/3277, 10-11=-79/380, 9-10=-1526/726, 7-10=-1499/724, 8-9=-101/121
WEBS 3-16=0/240, 3-14=-1113/648, 4-14=-162/533, 4-13=-223/803, 5-13=-288/962, 5-12=-72/347, 6-12=-384/349, 6-11=-160/192, 7-11=-1131/2822

JOINT STRESS INDEX
2 = 0.90, 3 = 0.78, 4 = 0.87, 5 = 0.66, 6 = 0.68, 7 = 0.63, 8 = 0.23, 9 = 0.56, 10 = 0.84, 11 = 0.51, 12 = 0.38, 13 = 0.95, 14 = 0.38, 15 = 0.88 and 16 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 549 lb uplift at joint 9 and 565 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061241

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:53 2006 Page 1

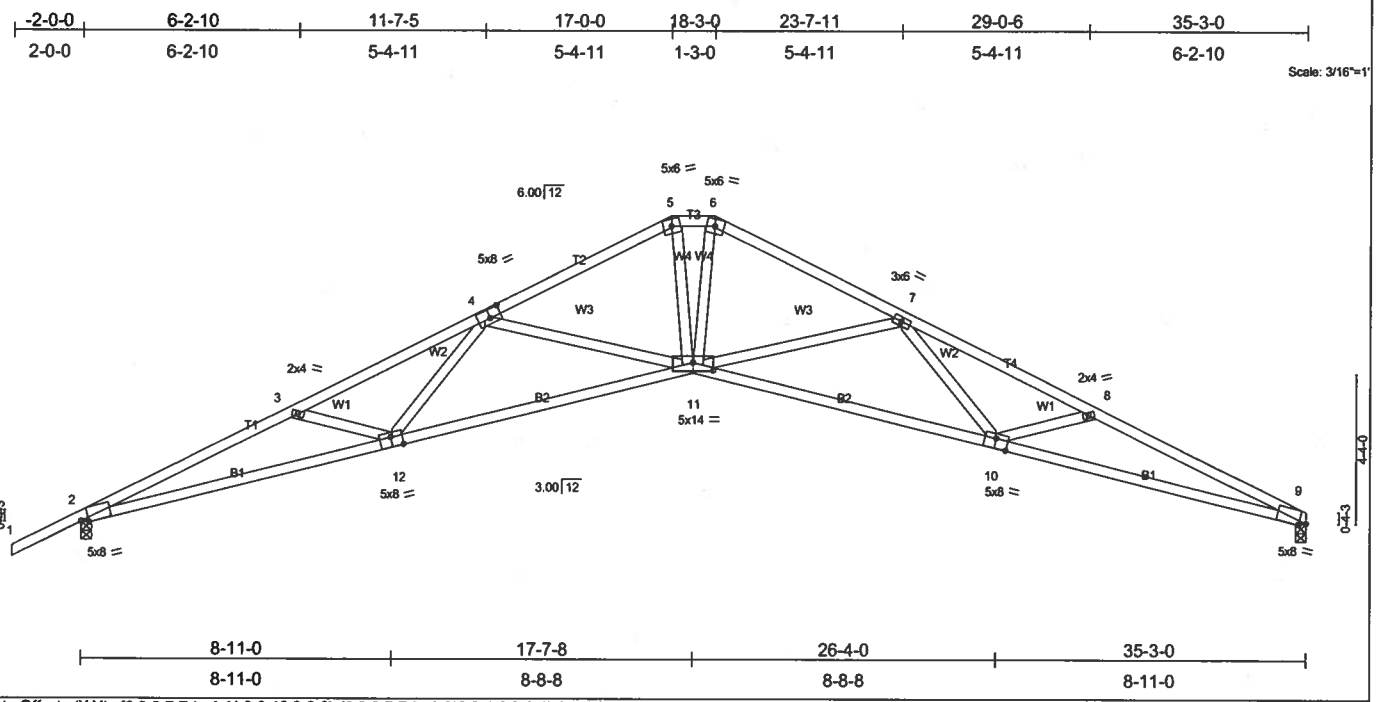


Plate Offsets (X,Y): [2:0-2-7,Edge], [4:0-3-12,0-3-0], [9:0-2-7,Edge], [10:0-4-0,0-3-4], [11:0-7-0,0-2-12], [12:0-4-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.78	Vert(LL)	-0.70	11-12	>596	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-1.13	11-12	>370	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.76	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-1-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-9-5 oc bracing.

REACTIONS (lb/size) 9=1465/0-3-8, 2=1588/0-3-8
Max Horz 2=180(load case 5)
Max Uplift9=-472(load case 6), 2=-599(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4999/2069, 3-4=-4661/1858, 4-5=-3320/1313, 5-6=-3099/1297, 6-7=-3320/1313, 7-8=-4701/1923, 8-9=-5040/2161
BOT CHORD 2-12=-1794/4533, 11-12=-1381/3893, 10-11=-1406/3909, 9-10=-1891/4592
WEBS 3-12=-289/322, 4-12=-161/640, 4-11=-873/556, 5-11=-460/1300, 6-11=-452/1295, 7-11=-889/581, 7-10=-213/672, 8-10=-312/359

JOINT STRESS INDEX
2 = 0.97, 3 = 0.34, 4 = 0.57, 5 = 0.80, 6 = 0.80, 7 = 0.45, 8 = 0.34, 9 = 0.97, 10 = 0.95, 11 = 0.95 and 12 = 0.95

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 9, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 472 lb uplift at joint 9 and 599 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

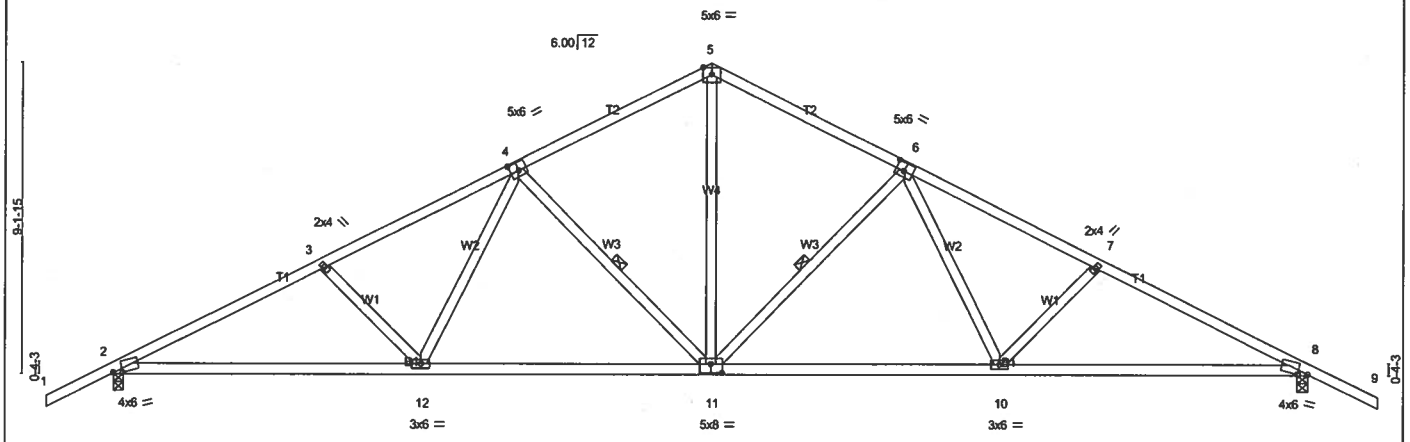
Dwg.#0531061242

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T13	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:38:55 2006 Page 1

-2-0-0	6-2-13	11-11-3	17-7-8	23-3-13	29-0-3	35-3-0	37-3-0
2-0-0	6-2-13	5-8-5	5-8-5	5-8-5	5-8-5	6-2-13	2-0-0

Scale = 1:64.9



9-1-0	17-7-8	26-2-0	35-3-0
9-1-0	8-6-8	8-6-8	9-1-0

Plate Offsets (X,Y): [2:0-3-5,0-0-11], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [8:0-3-5,0-0-11], [11:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.23	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.38	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 185 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-10-7 oc bracing.
WEBS 1 Row at midpt 4-11, 6-11

REACTIONS (lb/size) 2=1584/0-3-8, 8=1584/0-3-8
Max Horz 2=162(load case 5)
Max Uplift2=-601(load case 5), 8=-601(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2673/1147, 3-4=-2457/1090, 4-5=-1713/876, 5-6=-1713/876, 6-7=-2457/1090, 7-8=-2673/1147, 8-9=0/47
BOT CHORD 2-12=-846/2330, 11-12=-591/1903, 10-11=-591/1903, 8-10=-846/2330
WEBS 3-12=-289/275, 4-12=-153/540, 4-11=-641/397, 5-11=-531/1154, 6-11=-641/397, 6-10=-153/540, 7-10=-289/275

JOINT STRESS INDEX
2 = 0.87, 3 = 0.34, 4 = 0.54, 5 = 0.46, 6 = 0.54, 7 = 0.34, 8 = 0.87, 10 = 0.46, 11 = 0.58 and 12 = 0.46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 601 lb uplift at joint 2 and 601 lb uplift at joint 8.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061243

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T14	SCISSOR	5	1	Job Reference (optional)

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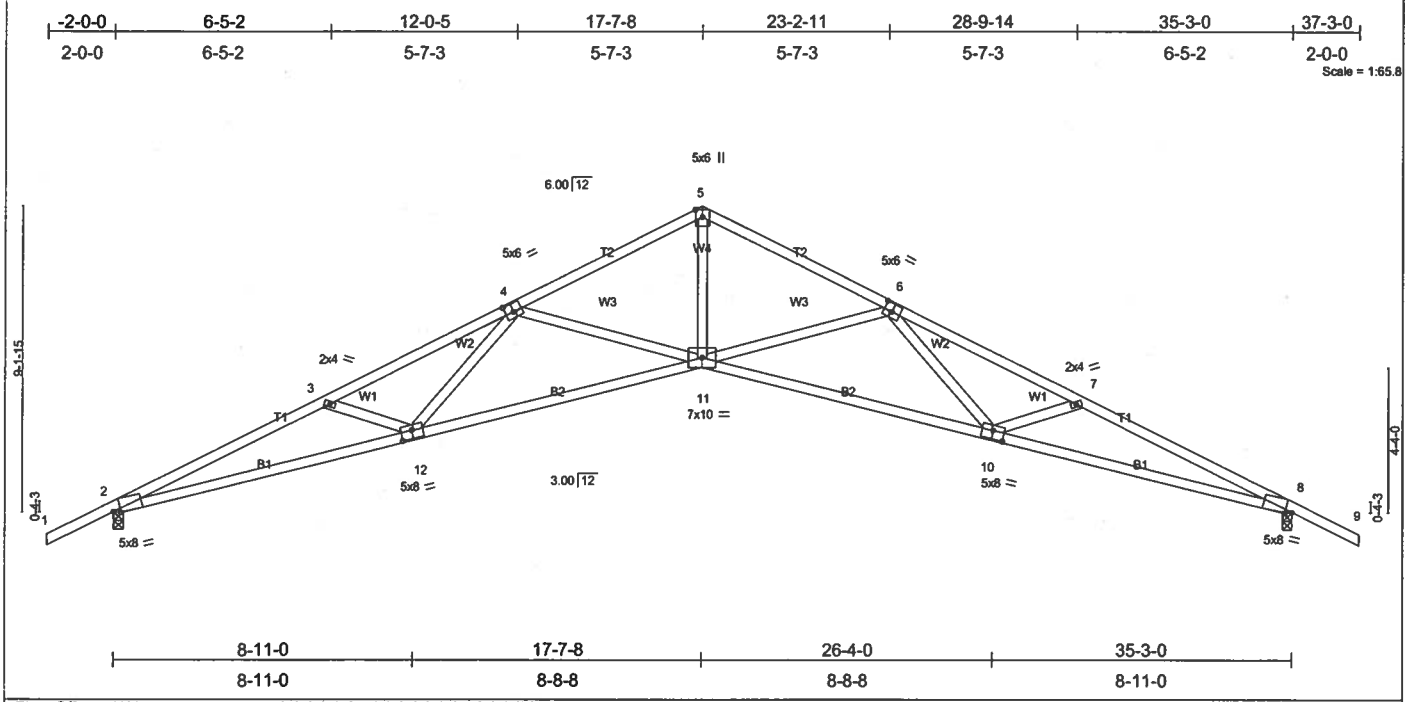

Plate Offsets (X,Y): [2:0-2-7,Edge], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [8:0-2-7,Edge], [10:0-4-0,0-3-0], [12:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.69	11-12	>608	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-1.11	11-12	>377	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.74	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 165 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-2-11 oc bracing.

REACTIONS (lb/size) 2=1584/0-3-8, 8=1584/0-3-8
Max Horz 2=161(load case 5)
Max Uplift2=-601(load case 5), 8=-601(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4985/1974, 3-4=-4646/1772, 4-5=-3296/1217, 5-6=-3296/1217, 6-7=-4646/1772, 7-8=-4985/1974, 8-9=0/46
BOT CHORD 2-12=-1629/4520, 11-12=-1181/3813, 10-11=-1181/3813, 8-10=-1629/4520
WEBS 3-12=-293/330, 4-12=-204/704, 4-11=-845/539, 5-11=-843/2579, 6-11=-845/539, 6-10=-204/704, 7-10=-293/330

JOINT STRESS INDEX
2 = 0.96, 3 = 0.34, 4 = 0.60, 5 = 0.56, 6 = 0.60, 7 = 0.34, 8 = 0.96, 10 = 0.99, 11 = 0.72 and 12 = 0.99

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 601 lb uplift at joint 2 and 601 lb uplift at joint 8.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061244

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T15	HIP	1	1	Job Reference (optional)

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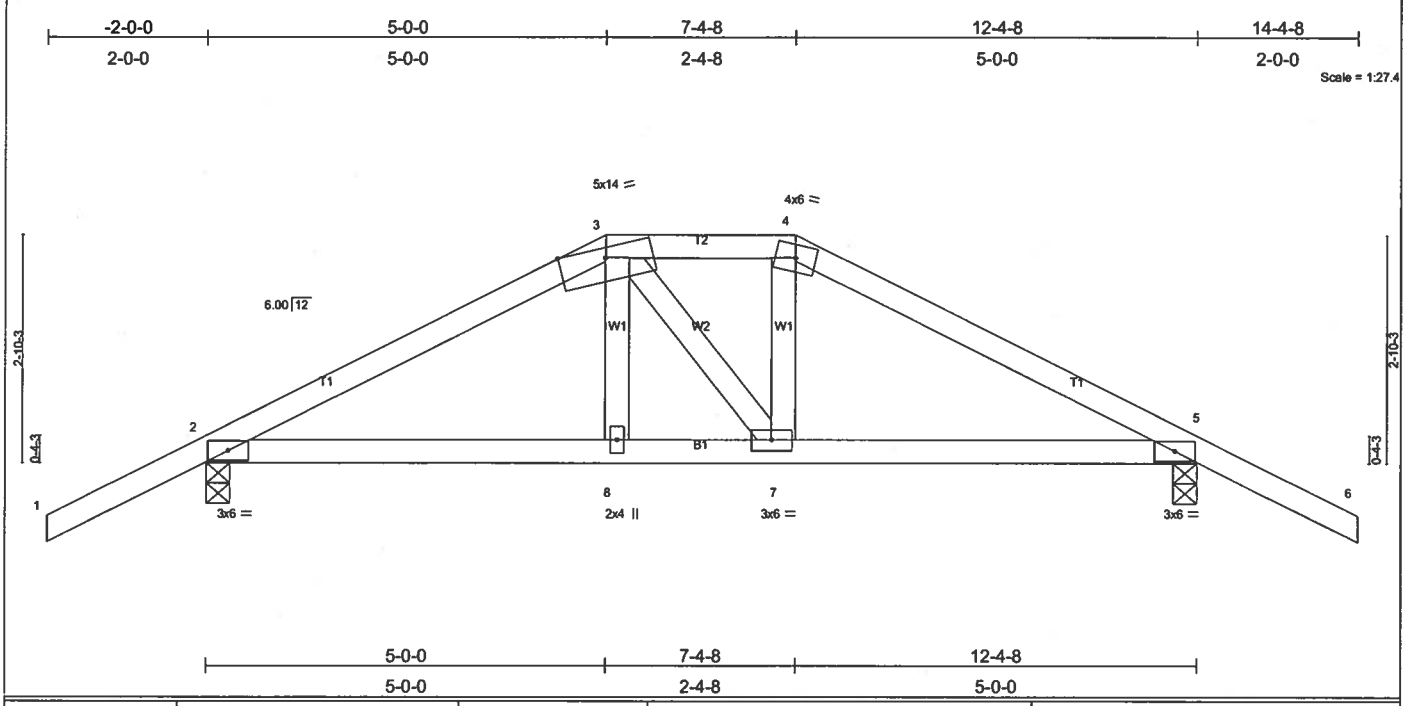

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.04	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.06	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.14	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 57 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-3-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=934/0-3-8, 5=934/0-3-8
Max Horz 2=-73(load case 5)
Max Uplift2=-449(load case 4), 5=-449(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1371/508, 3-4=-1183/493, 4-5=-1374/508, 5-6=0/47
BOT CHORD 2-8=-382/1159, 7-8=-386/1181, 5-7=-374/1162
WEBS 3-8=-109/373, 3-7=-68/76, 4-7=-128/428

JOINT STRESS INDEX
2 = 0.62, 3 = 0.39, 4 = 0.40, 5 = 0.62, 7 = 0.28 and 8 = 0.27

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 449 lb uplift at joint 2 and 449 lb uplift at joint 5.
6) Girder carries hip end with 5-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 126 lb up at 7-4-8, and 245 lb down and 126 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-91(F=-37), 4-6=-54, 2-8=-30, 7-8=-51(F=-21), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-245(F) 7=-245(F)

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061245

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T15P	HIP	1	1	Job Reference (optional)

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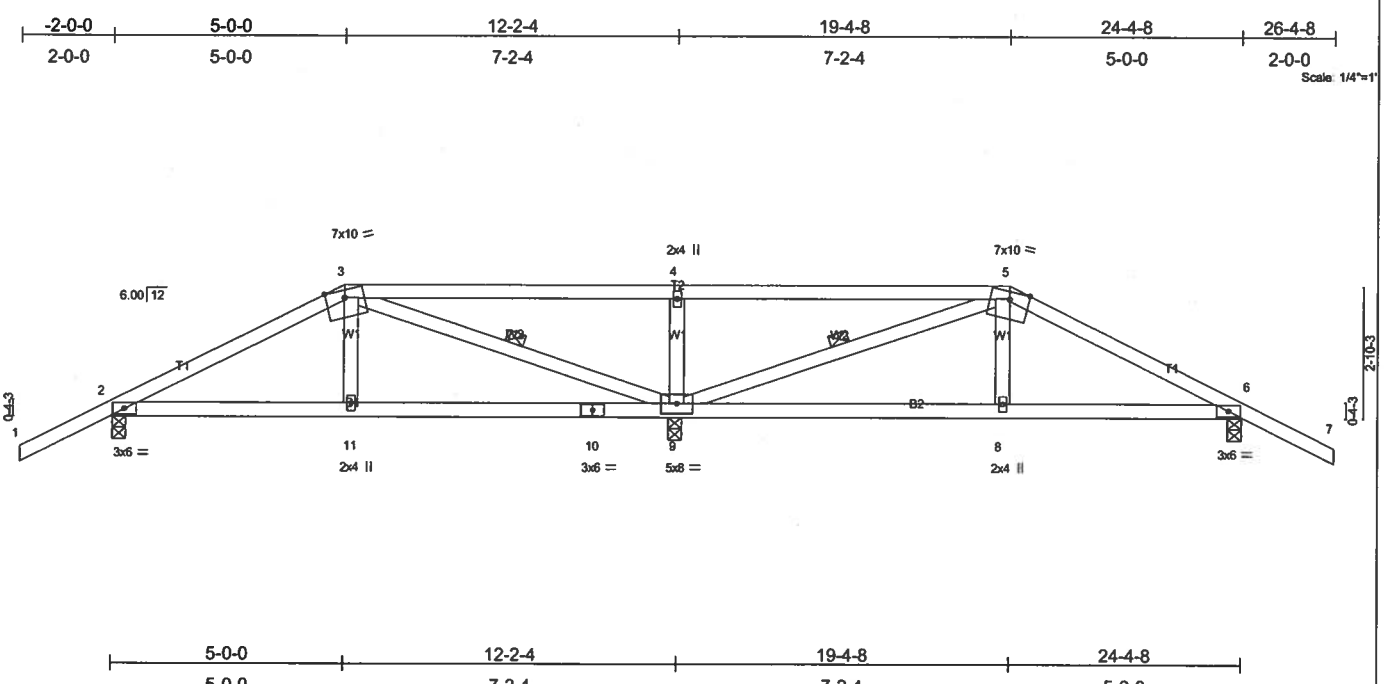

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	0.09	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.13	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.35	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 113 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-12 oc bracing.
WEBS 1 Row at midpt 3-9, 5-9

REACTIONS (lb/size) 2=792/0-3-8, 9=1860/0-3-8, 6=792/0-3-8
Max Horz 2=-73(load case 5)
Max Uplift2=-534(load case 4), 9=-923(load case 3), 6=-388(load case 5)
Max Grav 2=793(load case 8), 9=1860(load case 1), 6=793(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1147/637, 3-4=-31/130, 4-5=-31/130, 5-6=-1147/403, 6-7=0/47
BOT CHORD 2-11=-540/975, 10-11=-553/1002, 9-10=-553/1002, 8-9=-308/1002, 6-8=-305/975
WEBS 3-11=-241/474, 3-9=-1199/690, 4-9=-686/532, 5-9=-1199/448, 5-8=-51/474

JOINT STRESS INDEX
2 = 0.64, 3 = 0.69, 4 = 0.34, 5 = 0.69, 6 = 0.64, 8 = 0.35, 9 = 0.30, 10 = 0.34 and 11 = 0.35

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 534 lb uplift at joint 2, 923 lb uplift at joint 9 and 388 lb uplift at joint 6.
6) Girder carries hip end with 5-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 126 lb up at 19-4-8, and 245 lb down and 126 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-86(F=-32), 5-7=-54, 2-11=-30, 8-11=-47(F=-17), 6-8=-30
Concentrated Loads (lb)
Vert: 11=-245(F) 8=-245(F)

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061246

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T16	COMMON	3	1	Job Reference (optional)

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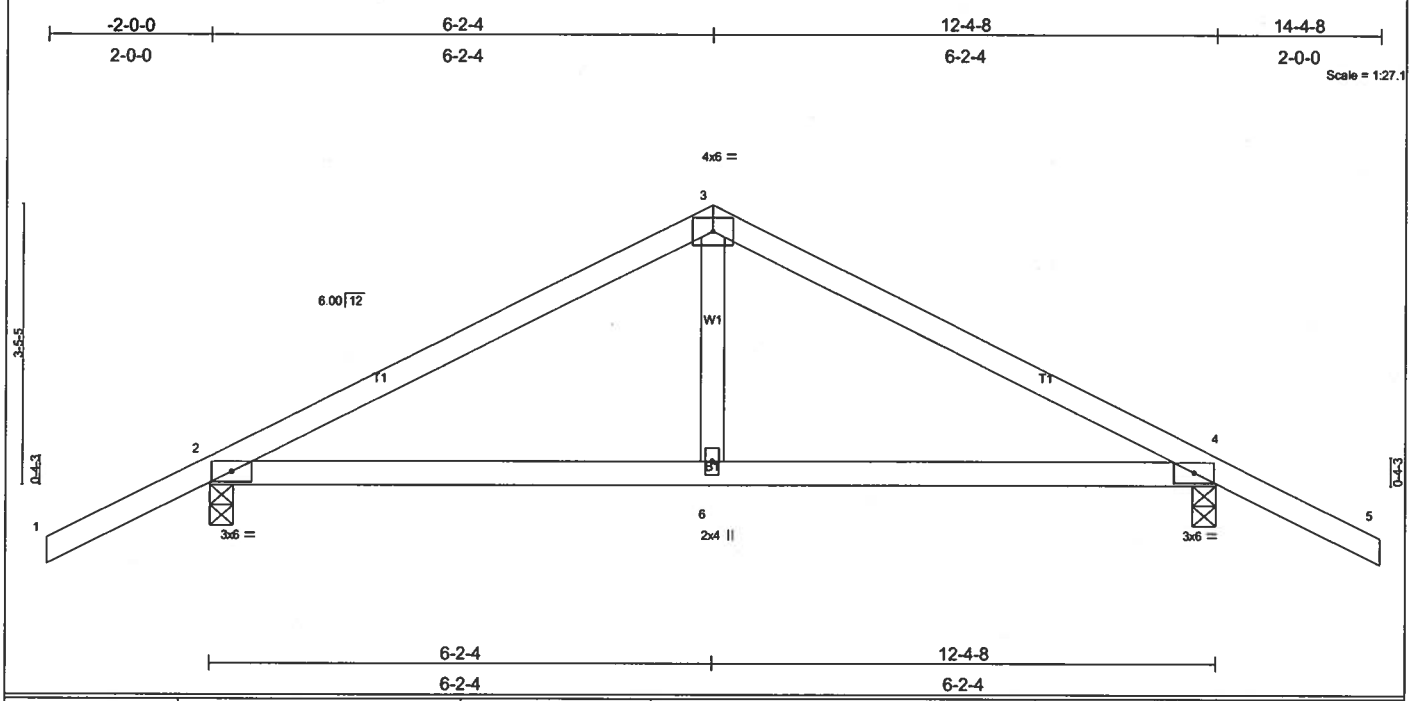


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.04	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	-0.07	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 50 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=623/0-3-8, 4=623/0-3-8
Max Horz 2=81(load case 5)
Max Uplift2=-294(load case 5), 4=-294(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-684/270, 3-4=-684/270, 4-5=0/47
BOT CHORD 2-6=-80/547, 4-6=-80/547
WEBS 3-6=0/212

JOINT STRESS INDEX
2 = 0.44, 3 = 0.72, 4 = 0.44 and 6 = 0.15

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 294 lb uplift at joint 2 and 294 lb uplift at joint 4.

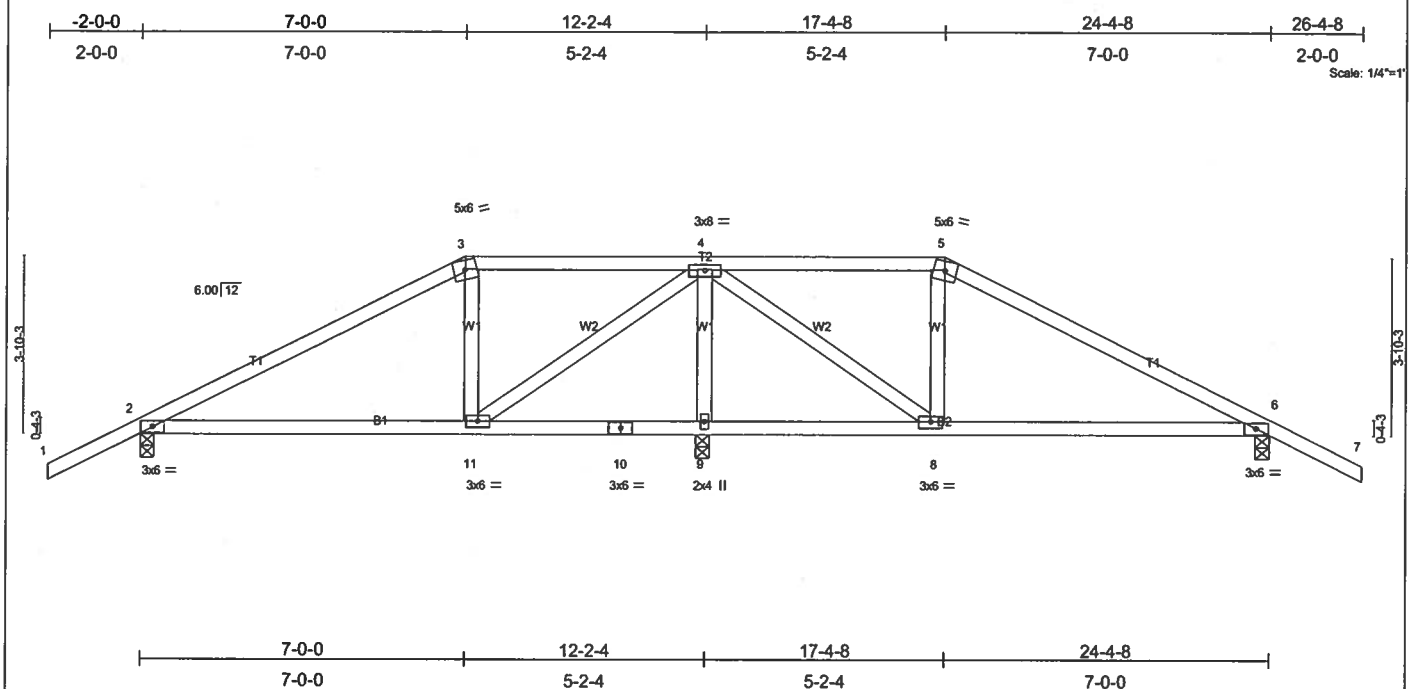
LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061247

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T16P	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:39:02 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.16	2-11	>922	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.13	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 114 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=570/0-3-8, 9=1115/0-3-8, 6=570/0-3-8
Max Horz 2=-87(load case 6)
Max Uplift2=-441(load case 5), 9=-459(load case 4), 6=-305(load case 6)
Max Grav 2=576(load case 9), 9=1115(load case 1), 6=576(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-514/538, 3-4=-380/559, 4-5=-380/236, 5-6=-514/190, 6-7=0/47
BOT CHORD 2-11=-302/386, 10-11=-174/212, 9-10=-174/212, 8-9=-174/212, 6-8=-30/386
WEBS 3-11=-168/82, 4-11=-626/663, 4-9=-997/642, 4-8=-231/663, 5-8=-168/181

JOINT STRESS INDEX
2 = 0.50, 3 = 0.65, 4 = 0.64, 5 = 0.65, 6 = 0.50, 8 = 0.39, 9 = 0.37, 10 = 0.16 and 11 = 0.39

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 441 lb uplift at joint 2, 459 lb uplift at joint 9 and 305 lb uplift at joint 6.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061248

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T17P	HIP	1	1	Job Reference (optional)

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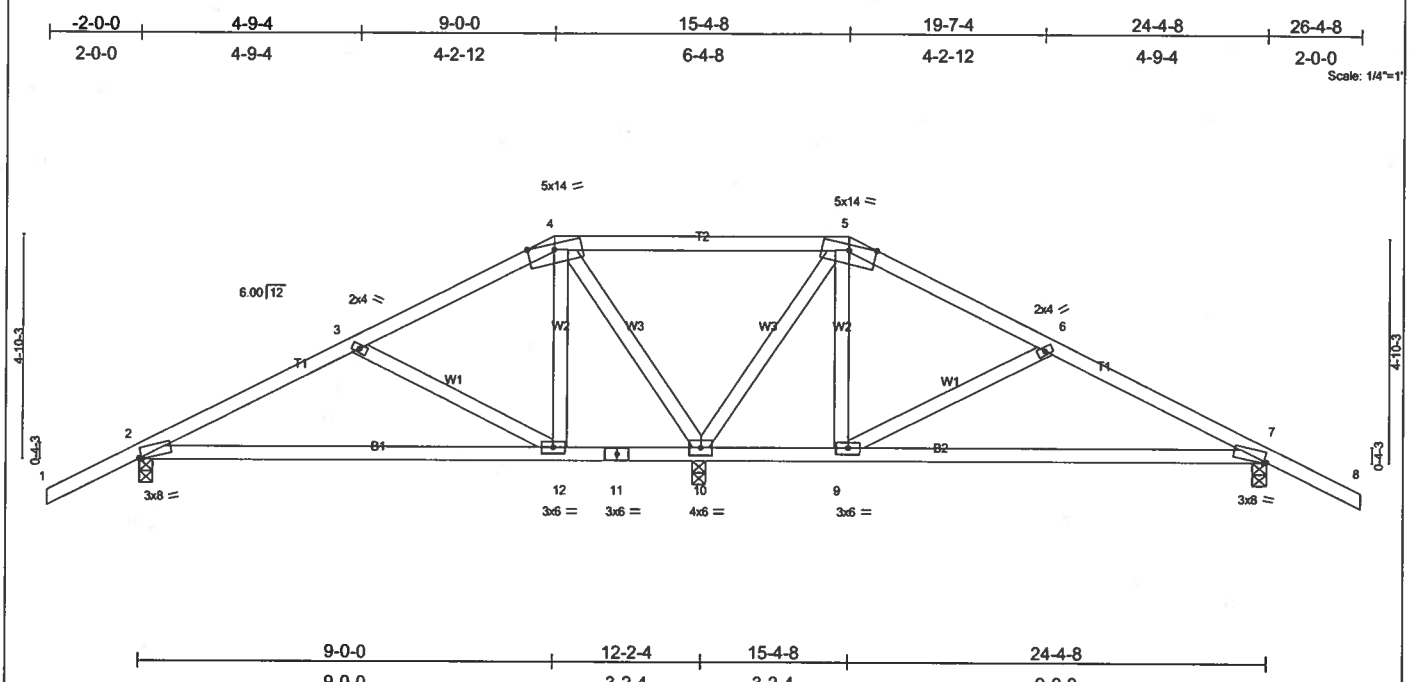

Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.30	2-12	>475	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	0.25	2-12	>584	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 125 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=497/0-3-8, 10=1260/0-3-8, 7=497/0-3-8
Max Horz 2=-101(load case 6)
Max Uplift2=-401(load case 5), 10=-518(load case 5), 7=-269(load case 6)
Max Grav 2=531(load case 9), 10=1260(load case 1), 7=531(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-439/424, 3-4=-183/263, 4-5=-121/391, 5-6=-183/56, 6-7=-439/155, 7-8=0/47
BOT CHORD 2-12=-245/360, 11-12=-79/134, 10-11=-79/134, 9-10=0/238, 7-9=-17/360
WEBS 3-12=-273/305, 4-12=-502/378, 4-10=-798/732, 5-10=-798/336, 5-9=-45/378, 6-9=-273/216

JOINT STRESS INDEX
2 = 0.85, 3 = 0.34, 4 = 0.76, 5 = 0.76, 6 = 0.34, 7 = 0.85, 9 = 0.35, 10 = 0.30, 11 = 0.29 and 12 = 0.35

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 401 lb uplift at joint 2, 518 lb uplift at joint 10 and 269 lb uplift at joint 7.

LOAD CASE(S) Standard

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061249

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T18H	HIP	1	1	Job Reference (optional)

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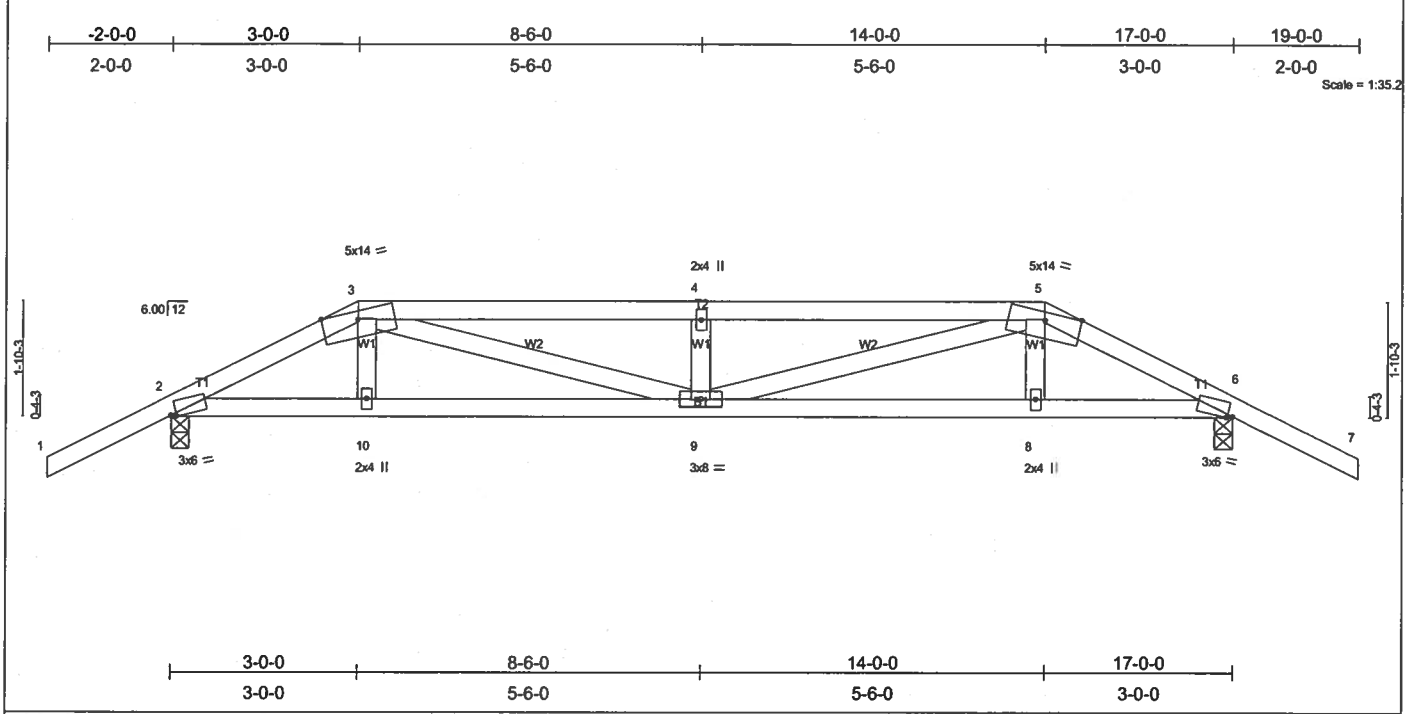


Plate Offsets (X,Y): [2:0-1-0,0-0-7], [6:0-1-0,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.14	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.21	9-10	>970	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.37	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 80 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-11-5 oc bracing.

REACTIONS (lb/size) 2=962/0-3-8, 6=962/0-3-8
Max Horz 2=-59(load case 5)
Max Uplift2=-595(load case 4), 6=-595(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1566/954, 3-4=-2340/1484, 4-5=-2340/1484, 5-6=-1566/954, 6-7=0/47
BOT CHORD 2-10=-813/1352, 9-10=-824/1370, 8-9=-812/1370, 6-8=-801/1352
WEBS 3-10=-115/208, 3-9=-684/1021, 4-9=-356/263, 5-9=-684/1021, 5-8=-115/208

JOINT STRESS INDEX
2 = 0.75, 3 = 0.50, 4 = 0.34, 5 = 0.50, 6 = 0.75, 8 = 0.34, 9 = 0.93 and 10 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 595 lb uplift at joint 2 and 595 lb uplift at joint 6.
6) Girder carries hip end with 3-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 63 lb down and 32 lb up at 14-0-0, and 63 lb down and 32 lb up at 3-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

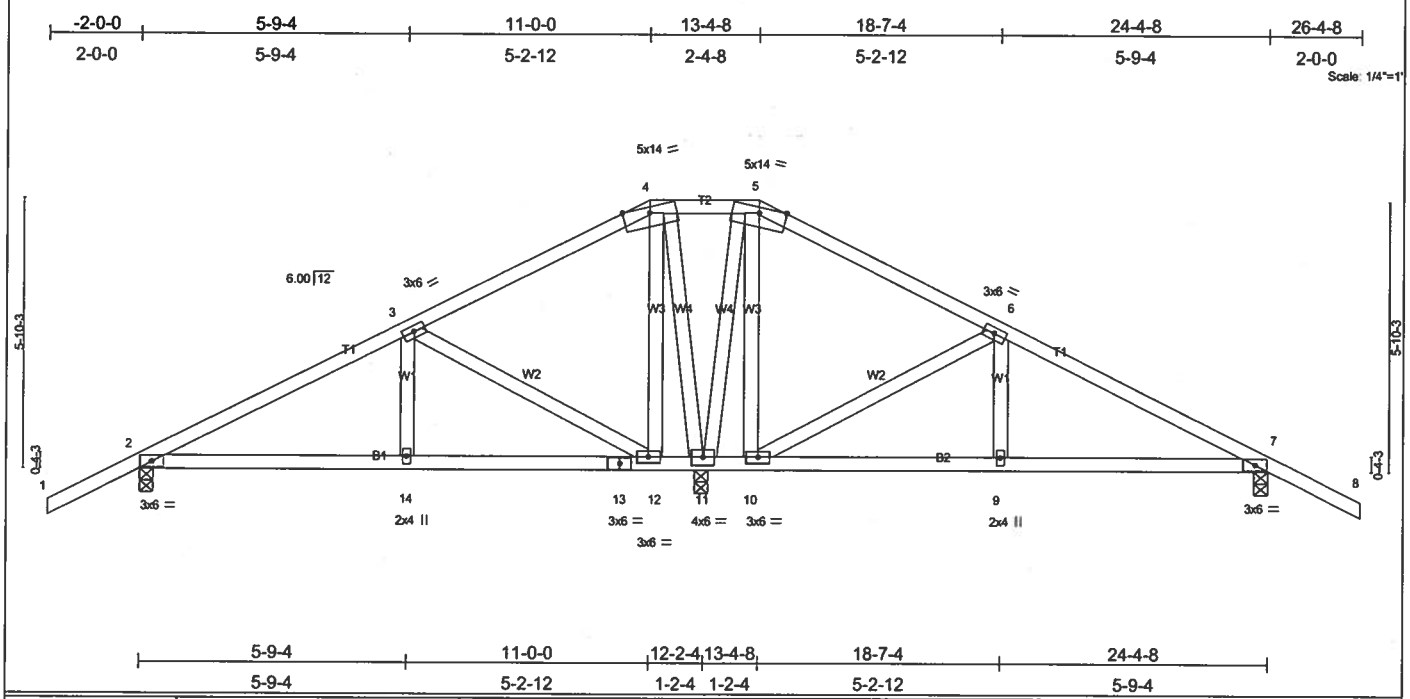
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-64(F=-10), 5-7=-54, 2-10=-30, 8-10=-35(F=-5), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-63(F) 8=-63(F)

MAY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061250

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T19P	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 14:39:06 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.05	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.05	2-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.35	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 140 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=457/0-3-8, 11=1341/0-3-8, 7=457/0-3-8
Max Horz 2=-115(load case 6)
Max Uplift2=-387(load case 5), 11=-572(load case 5), 7=-260(load case 6)
Max Grav 2=498(load case 9), 11=1341(load case 1), 7=498(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-426/453, 3-4=-40/321, 4-5=-66/353, 5-6=-101/321, 6-7=-426/100, 7-8=0/47
BOT CHORD 2-14=-242/320, 13-14=-242/320, 12-13=-242/320, 11-12=-228/304, 10-11=-228/378, 9-10=0/320, 7-9=0/320
WEBS 3-14=-268/189, 3-12=-543/634, 4-12=-424/294, 4-11=-607/587, 5-11=-607/230, 5-10=-45/294, 6-10=-543/283, 6-9=0/189

JOINT STRESS INDEX
2 = 0.55, 3 = 0.41, 4 = 0.47, 5 = 0.47, 6 = 0.41, 7 = 0.55, 9 = 0.34, 10 = 0.35, 11 = 0.26, 12 = 0.35, 13 = 0.15 and 14 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 387 lb uplift at joint 2, 572 lb uplift at joint 11 and 260 lb uplift at joint 7.

LOAD CASE(S) Standard

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061251

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T20	COMMON	1	2	Job Reference (optional)

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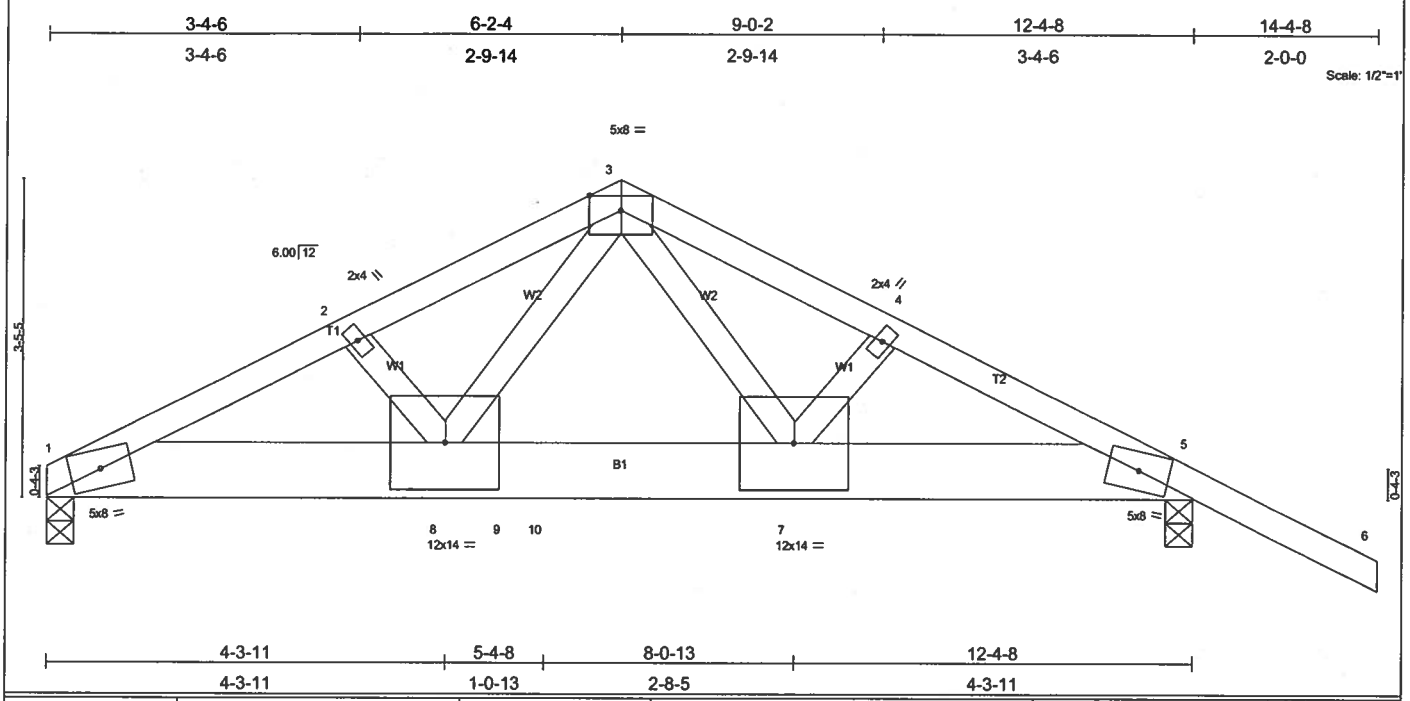

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.09	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.15	7-8	>993	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.86	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 152 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4917/0-3-8, 5=2632/0-3-8
Max Horz 1=-114(load case 5)
Max Uplift1=-1829(load case 4), 5=-1054(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-7885/2931, 2-3=-7800/2924, 3-4=-5309/1962, 4-5=-5397/1958, 5-6=0/54
BOT CHORD 1-8=-2605/7051, 8-9=-1450/4110, 9-10=-1450/4110, 7-10=-1450/4110, 5-7=-1693/4795
WEBS 2-8=-135/129, 3-8=-2023/5368, 3-7=-397/1196, 4-7=-90/106

JOINT STRESS INDEX
1 = 0.82, 2 = 0.03, 3 = 0.78, 4 = 0.03, 5 = 0.82, 7 = 0.46 and 8 = 0.46

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1829 lb uplift at joint 1 and 1054 lb uplift at joint 5.
7) Girder carries tie-in span(s): 35-3-0 from 0-0-0 to 5-0-0
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3058 lb down and 1155 lb up at 5-4-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 1-9=-722(F=-692), 5-9=-30
Concentrated Loads (lb)
Vert: 10=-3058(F)

MAY 31, 2006 TRUSS DESIGN ENGINEER:
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Dwg.#0531061252

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T20P	SPECIAL	1	2	Job Reference (optional)

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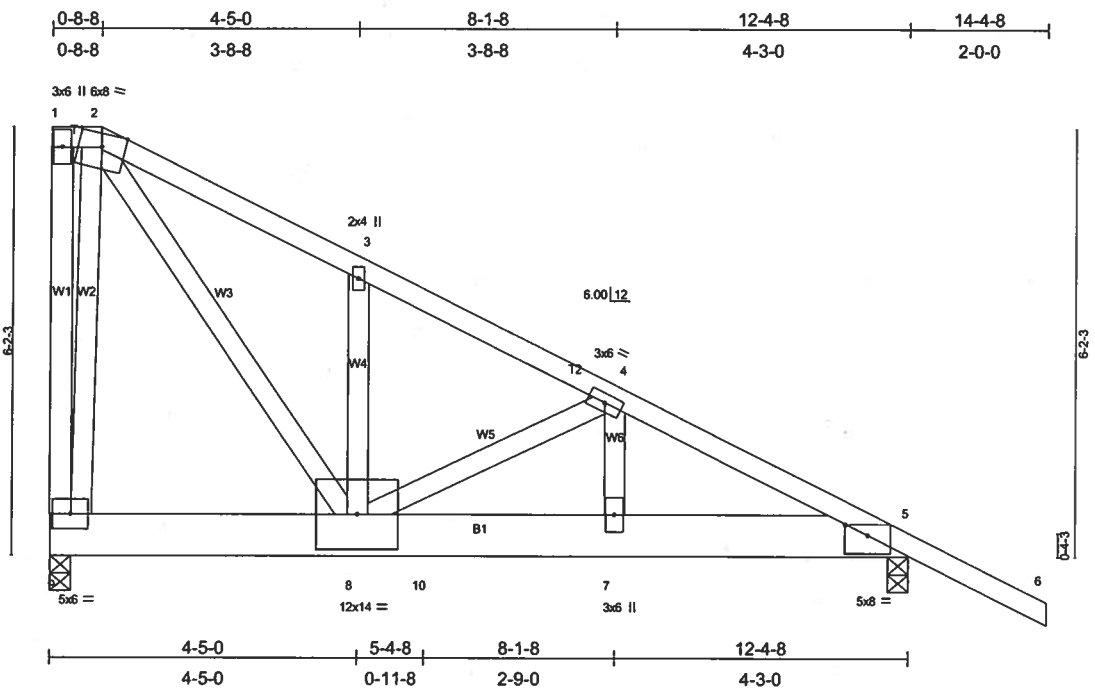

Plate Offsets (X,Y): [5:0-4-0,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.07	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.12	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.95	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 201 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 9=4690/0-3-8, 5=2498/0-3-8
Max Horz 9=-336(load case 5)
Max Uplift9=-1816(load case 5), 5=-967(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-9=-345/135, 1-2=-66/27, 2-3=-4050/1516, 3-4=-4058/1419, 4-5=-4953/1683, 5-6=0/53
BOT CHORD 8-9=-7/362, 8-10=-1395/4400, 7-10=-1395/4400, 5-7=-1395/4400
WEBS 2-9=-2977/1229, 2-8=-2286/5932, 3-8=-202/186, 4-8=-924/374, 4-7=-171/595

JOINT STRESS INDEX
1 = 0.28, 2 = 0.69, 3 = 0.05, 4 = 0.22, 5 = 0.56, 7 = 0.10, 8 = 0.40 and 9 = 0.41

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1816 lb uplift at joint 9 and 967 lb uplift at joint 5.
7) Girder carries tie-in span(s): 35-3-0 from 0-0-0 to 4-4-8
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3101 lb down and 1171 lb up at 5-4-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-6=-54, 8-9=-722(F=-692), 5-8=-30
Concentrated Loads (lb)
Vert: 10=-3101(F)

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Dwg.#0531061253

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T21	QUEENPOST	1	1	Job Reference (optional)

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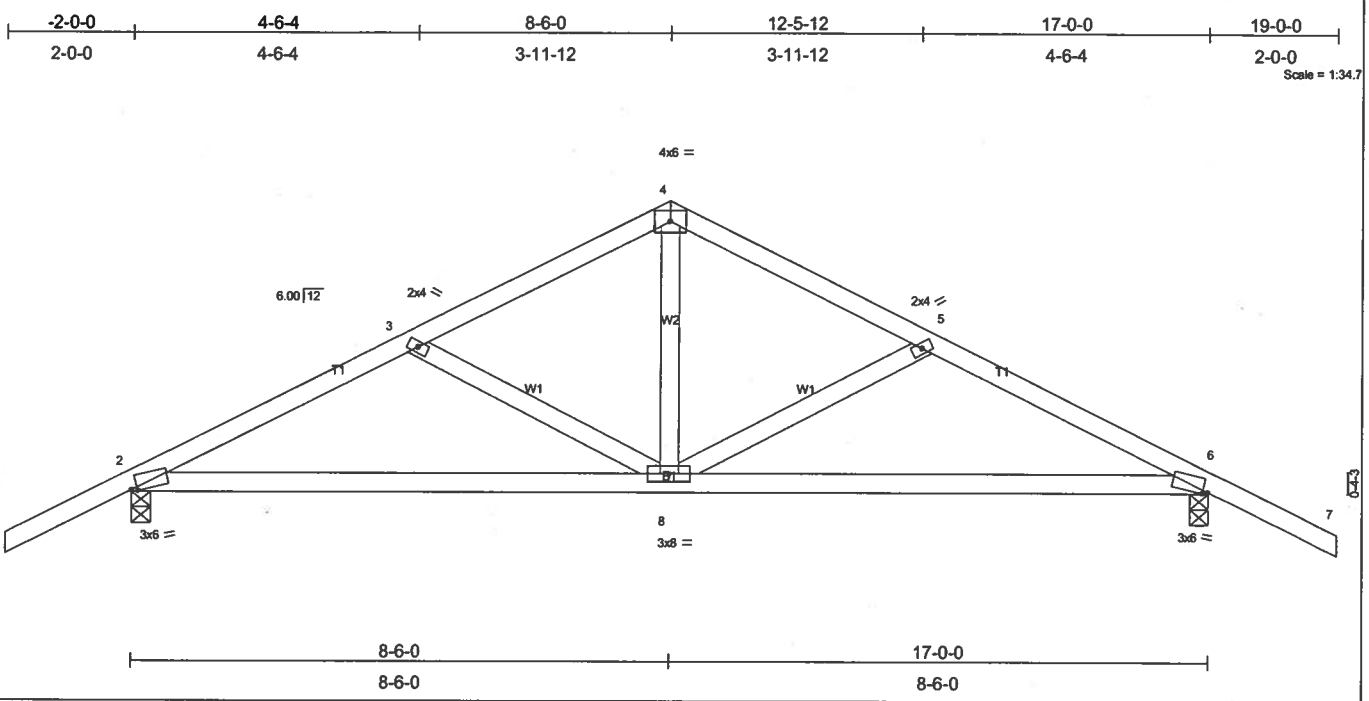

Plate Offsets (X,Y): [2:0-1-1,0-0-7], [6:0-1-1,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.09	6-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.16	6-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.16	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 80 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=818/0-3-8, 6=818/0-3-8
Max Horz 2=-98(load case 6)
Max Uplift2=-355(load case 5), 6=-355(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1085/452, 3-4=-852/355, 4-5=-852/355, 5-6=-1085/452, 6-7=0/47
BOT CHORD 2-8=-266/930, 6-8=-244/930
WEBS 3-8=-259/197, 4-8=-114/496, 5-8=-259/197

JOINT STRESS INDEX
2 = 0.79, 3 = 0.34, 4 = 0.40, 5 = 0.34, 6 = 0.79 and 8 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 355 lb uplift at joint 2 and 355 lb uplift at joint 6.

LOAD CASE(S) Standard

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0531061254

Job	Truss	Truss Type	Qty	Ply	VICTORIA- EMERALD COVE
MASTER	T21G	DROP TC GABLE	1	1	Job Reference (optional)

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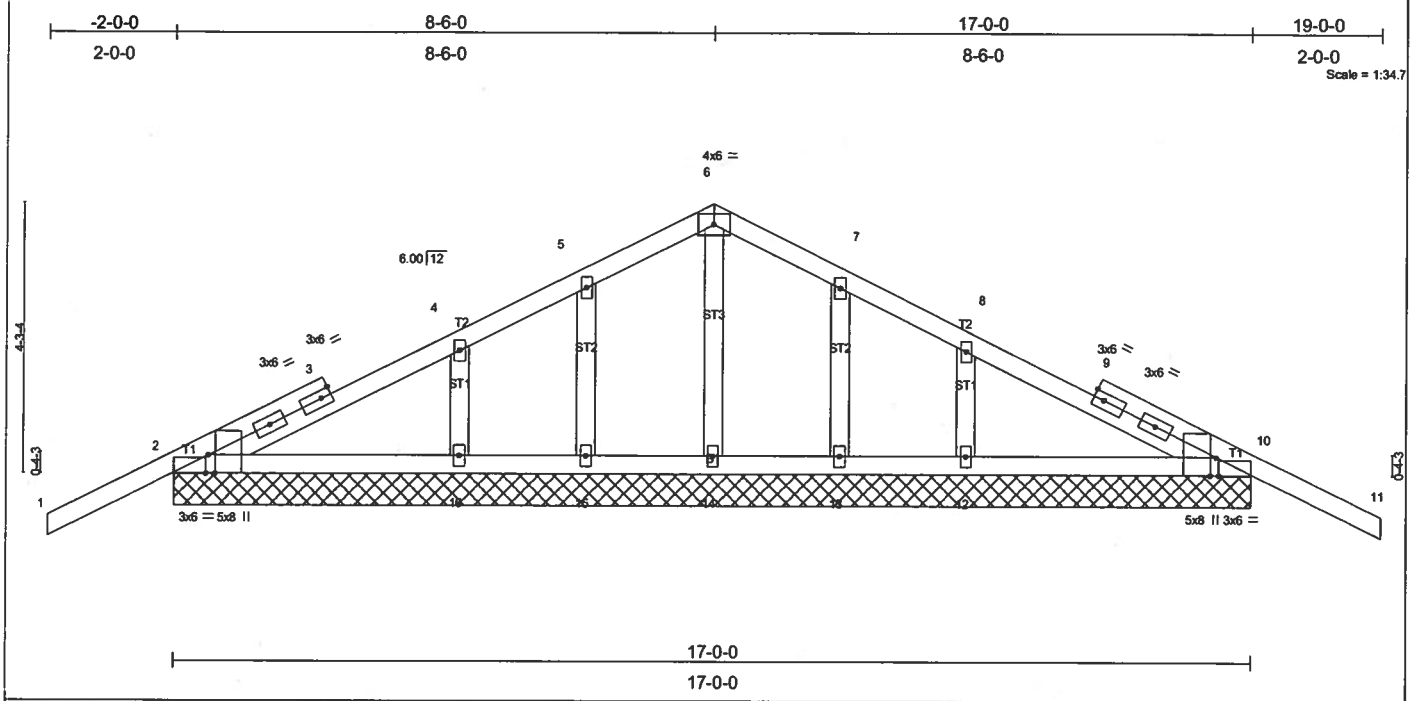

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [10:0-3-8,Edge], [10:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.03	11	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.04	11	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.10	Horz(TL)	0.00	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 85 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=537/17-0-0, 10=537/17-0-0, 14=393/17-0-0, 15=193/17-0-0, 16=525/17-0-0, 13=193/17-0-0, 12=525/17-0-0
Max Horz 2=93(load case 5)
Max Uplift2=-283(load case 5), 10=-299(load case 6), 14=-52(load case 5), 15=-110(load case 5), 16=-193(load case 5), 13=-107(load case 6), 12=-197(load case 6)
Max Grav 2=541(load case 9), 10=541(load case 10), 14=393(load case 1), 15=198(load case 9), 16=525(load case 9), 13=198(load case 10), 12=525(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-16/99, 2-3=-53/70, 3-4=-75/177, 4-5=-11/101, 5-6=0/132, 6-7=0/132, 7-8=-11/101, 8-9=-35/177, 9-10=-39/70, 10-11=-16/99
BOT CHORD 2-16=-65/141, 15-16=-65/141, 14-15=-65/141, 13-14=-65/141, 12-13=-65/141, 10-12=-65/141
WEBS 6-14=-316/66, 5-15=-171/129, 4-16=-398/248, 7-13=-171/129, 8-12=-398/248

JOINT STRESS INDEX
2 = 0.66, 2 = 0.18, 3 = 0.00, 3 = 0.42, 3 = 0.42, 4 = 0.34, 5 = 0.34, 6 = 0.25, 7 = 0.34, 8 = 0.34, 9 = 0.00, 9 = 0.42, 9 = 0.42, 10 = 0.66, 10 = 0.18, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.34 and 16 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 283 lb uplift at joint 2, 299 lb uplift at joint 10, 52 lb uplift at joint 14, 110 lb uplift at joint 15, 193 lb uplift at joint 16, 107 lb uplift at joint 13 and 197 lb uplift at joint 12.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-114(F=-60), 6-11=-114(F=-60), 2-10=-30

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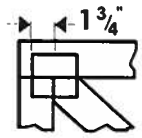
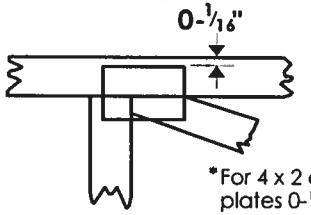
Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

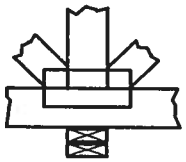
PLATE SIZE

4 x 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

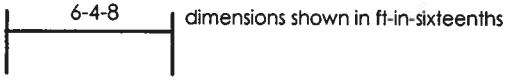


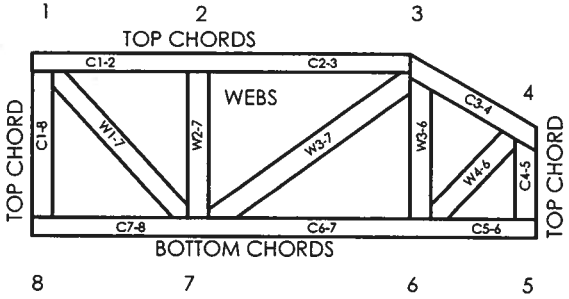
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System





JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

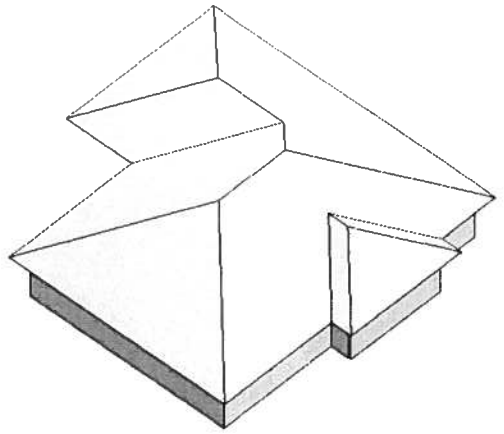
TEE-LOK®

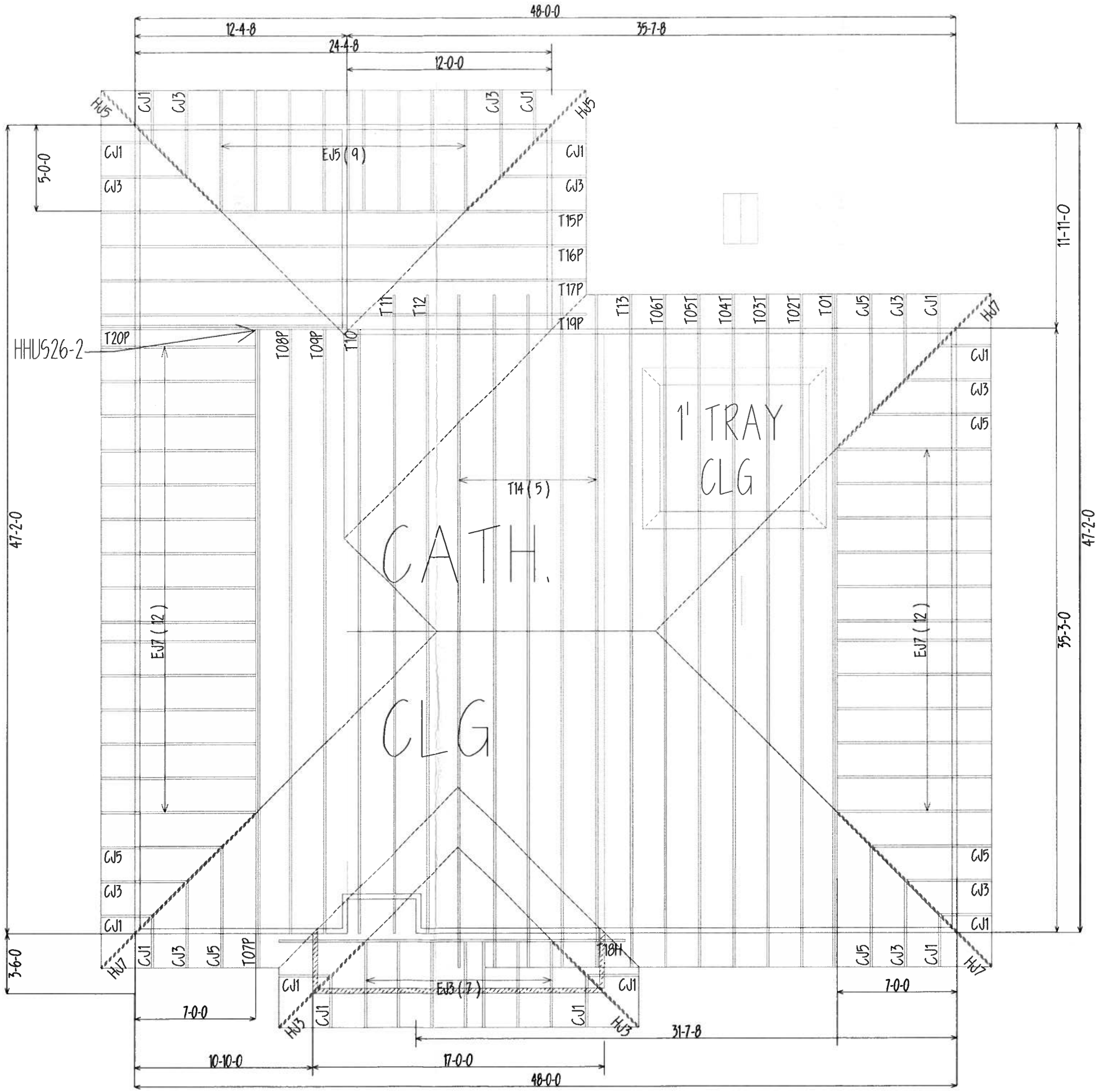
MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.





BEARING HEIGHT SCHEDULE

8'-0"

9'-0"

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL VIO5 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date : _____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: CORNERSTONE		
LEGAL ADDRESS: LOT 71 EMERALD COVE		
MODEL: VICTORIA EC		REVISION:
		SCALE: NTS
DATE: 5/24/06	DRAWN BY: JRD	JOB #: L166538