DATE 10/20/2005

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000023740

APPLICANT KATIE REED PHONE 752-4072

ADDRESS 2230 SE BAYA DRIVE LAKE CITY FL 32025

OWNER WILLIAM & JANICE DAUGHERTY PHONE 752-4072

ADDRESS 210 NW KISSIMMEE WAY LAKE CITY FL 32055

CONTRACTOR DON REED PHONE 752-4072

LOCATION OF PROPERTY 41N, TL ON SUWANNEE VALLEY RD, TL ON KISSIMMEE WAY, 1ST DRIVE ON RIGHT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 94500.00

HEATED FLOOR AREA 1890.00 TOTAL AREA 2772.00 HEIGHT .00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT 20

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 25-2S-15-00093-009 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES 7.40

000000851 CGC036224 Katie Reed

Culvert Permit No. | Culvert Waiver | Contractor's License Number | Applicant/Owner/Contractor

WAIVER 05-1020-N BK JH Y

Driveway Connection | Septic Tank Number | LU & Zoning checked by | Approved for Issuance | New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE

Check # or Cash 4411

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 475.00 CERTIFICATION FEE $ 13.86 SURCHARGE FEE $ 13.86

MISC. FEES $ .00 ZONING CERT. FEE $ 50.00 FIRE FEE $ .00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 577.72

INSPECTORS OFFICE _____ CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0510-51 Date Received 10-17-05 By LH Permit # 851/23740

Application Approved by - Zoning Official BLK Date 10.19.05 Plans Examiner OK JTH Date 10-20-05

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments Zone X Per BritT Surveying PLAT

Applicants Name Katie Reed Phone 755-7272 386-752-4072

Address 2230 SE Baya Drive Suite 101 Lake City, FL 32025

Owners Name William and Janice Daugherty Phone 386-752-4072

911 Address 210 NW Kissimmee Way Lake city, FL 32055

Contractors Name Don Reed Construction, Inc. Phone 386-752-4072

Address 2230 SE Baya Drive Suite 101 Lake City, FL 32025

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Mark Disosway P.E. PO Box 868 Lake City, FL 32056

Mortgage Lenders Name & Address N/A

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 25-2S-15-00093-009 Estimated Cost of Construction $155,200.00

Subdivision Name _____ Lot ____ Block ____ Unit ____ Phase ____

Driving Directions 41N toward White Springs; go approx. 3 miles past I-10 and TL on Suwannee Valley Road; go approx. 3 1/2 miles and TL on Kissimmee Way; 1st drive on right

Type of Construction single family dwelling Number of Existing Dwellings on Property 2

Total Acreage 7.480 Lot Size _____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 233' Side 505' Side 288' Rear 84'

Total Building Height 20' Number of Stories 1 Heated Floor Area 1,890 Roof Pitch 6/12

Porch 322 Garage 560 TOTAL 2,772

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Don Reed

Notary Public State of Florida
Ingrid Geiger
My Commission DD385312
Expires 01/26/2009

Owner Builder or Agent (Including Contractor)

Contractor Signature

Contractors License Number CGC036224

Competency Card Number

NOTARY STAMP/SEAL

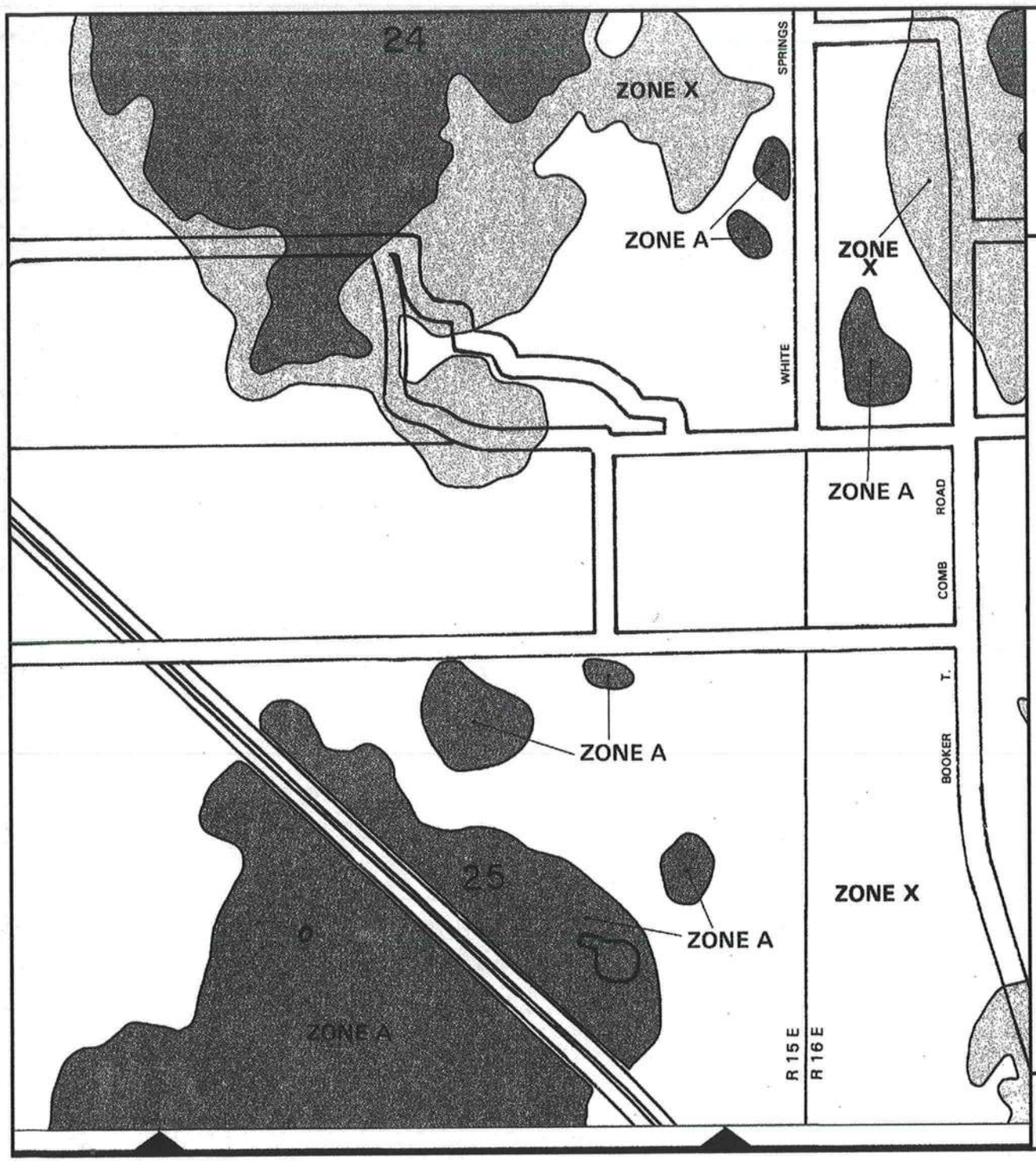
STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 12th day of Oct. 2005.

Personally known ✓ or Produced Identification ____

Ingrid Geiger

Notary Signature



N

APPROXIMATE SCALE IN FEET

1000 0 1000

NATIONAL FLOOD INSURANCE PROGRAM

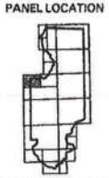
FIRM

FLOOD INSURANCE RATE MAP

COLUMBIA COUNTY, FLORIDA
(UNINCORPORATED AREAS)

PANEL 105 OF 290

PANEL LOCATION



COMMUNITY-PANEL NUMBER
120070 0105 B

EFFECTIVE DATE:
JANUARY 6, 1988

Federal Emergency Management Agency

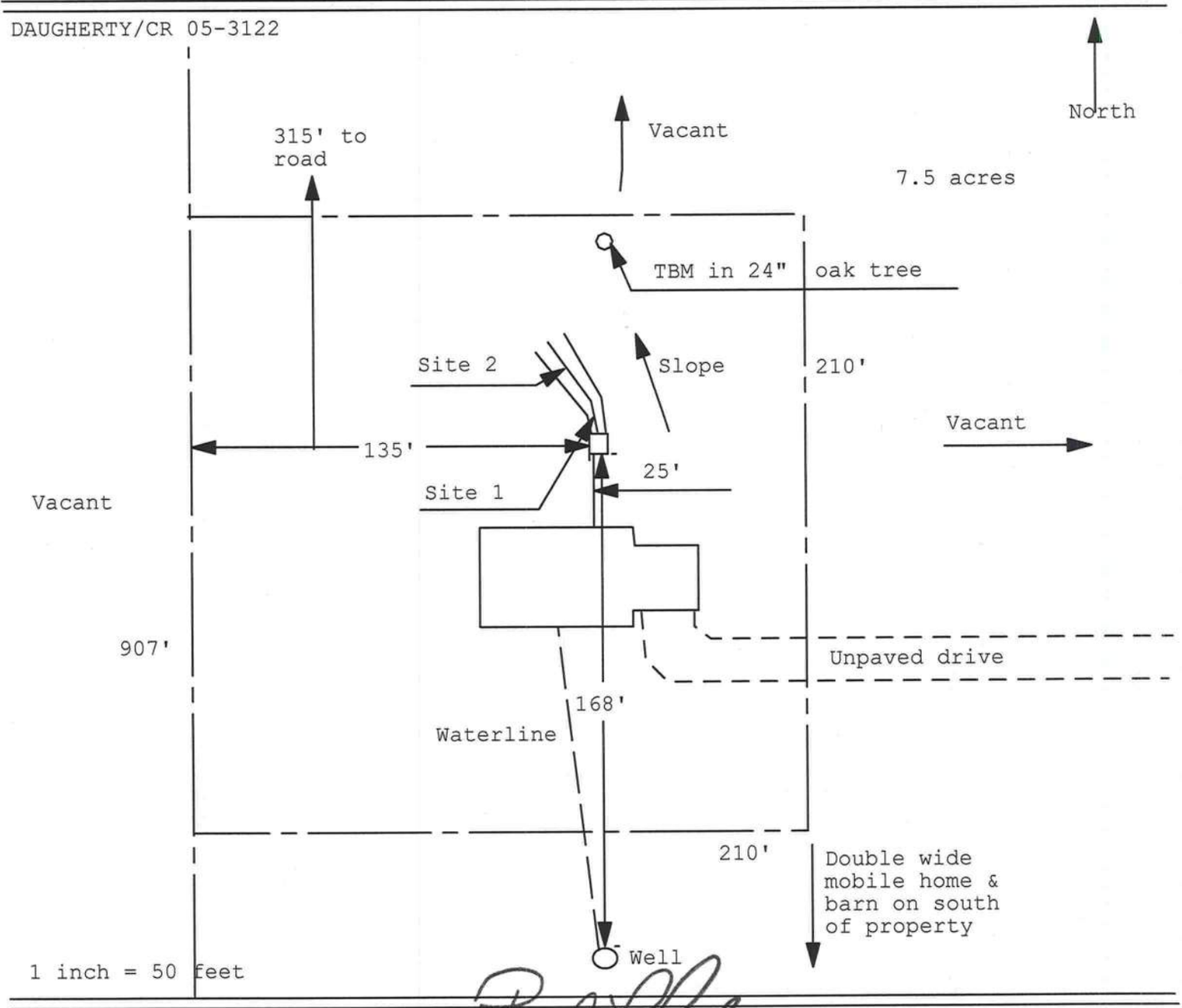
This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Version 1.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Futher information about National Flood Insurance Program flood hazard maps is available at www.fema.gov/mit/tsd.

Print Date: 10/19/2005 (printed at scale and type A)

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 25-1022N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



Site Plan Submitted By Paul Lloyd **Date** 9/30/05

Plan Approved ✓ **Not Approved** _____ **Date** 10-10-05

By _____ Columbia **CPHU**

Notes: _____

Permit No. _____ Tax Parcel No. _____

COLUMBIA COUNTY NOTICE OF COMMENCEMENT

Inst:2005023110 Date:09/20/2005 Time:13:52
DC,P.DeWitt Cason,Columbia County B:1058 P:2553

STATE OF FLORIDA

COUNTY OF COLUMBIA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: (legal description of the property, and street address if available.)
210 NW Kissimmee Way Lake City, FL 32055

Section 25, Township 2 South, Range 15 East: East 1/2 of the Northwest 1/4 of the Northeast 1/4, except for road right-of-way along North side and except West 306.60 feet and except the South 390 feet.

2. General description of improvement: single family dwelling

3. Owner information:

A. Name and address:
William and Janice Daugherty
PO Box 478 White Springs, FL 32096-0478

B. Interest in property:
100%

C. Name and address of fee simple titleholder (if other than owner):
N/A

4. Contractor: (name and address)
Don Reed Construction, Inc.
2230 SE Baya Drive Suite 101
Lake City, FL 32025

5. Surety

A. Name and address: N/A

B. Amount of bond:

6. Lender: (name and address) N/A

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 718.13 (1) (a) 7., Florida Statutes: (name and address)

8. In addition to himself, owner designates _____ of _____ to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) (a) 7., Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified) _____.

_____
(Signature of Owner)

SWORN TO and subscribed before me this 19th day of September, 2005.

_____
Notary Public

(NOTARIAL SEAL)

Notary Public State of Florida
Ingrid Geiger
My Commission DD385312
Expires 01/26/2009

My Commission Expires: 01/26/2009

Inst:2005023110 Date:09/20/2005 Time:13:52
DC,P.DeWitt Cason,Columbia County B:1058 P:2554

BK 0917 PG 0272

00-22358

FILED AND RECORDED IN PUBLIC RECORDS OF COLUMBIA COUNTY, FL.

'00 DEC 27 PM 3:36

This instrument prepared by
TERESA BYRD MORGAN
TERESA BYRD MORGAN, P.A.
Attorneys At Law
302 East Duval Street
Lake City, Florida 32055

OFFICIAL RECORDS

Documentary Stamp .70
Intangible Tax
P. DeWitt Cason
Clerk of Court
By MCK D.C.

MCK

PERSONAL REPRESENTATIVE'S DISTRIBUTIVE DEED

THIS PERSONAL REPRESENTATIVE'S DISTRIBUTIVE DEED made this 26 day of December, 2000, by and between WILLIAM F. DAUGHERTY, JR., whose post office address is Post Office Box 478, White Springs, Florida 32096, the duly qualified and acting personal representative of the Estate of WILLIAM F. DAUGHERTY, SR., deceased, as Grantor, and WILLIAM F. DAUGHERTY, JR., whose social security number is 079-38-4732 and whose post office address is Post Office Box 478, White Springs, Florida 32096, as Grantee.

WHEREAS, WILLIAM F. DAUGHERTY, SR. (the "Decedent"), died on October 25, 1999, and his estate is being probated in the Circuit Court, Third Judicial Circuit, Columbia County, Florida, Probate Division, Case Number 00-10 CP; and

WHEREAS, WILLIAM F. DAUGHERTY, JR., is the duly appointed and acting Personal Representative of the Estate of Decedent (the "Grantor"); and

WHEREAS, Decedent died intestate with no surviving spouse and all of Decedent's property passed by intestate succession to his sole lineal descendent, WILLIAM F. DAUGHERTY, JR.; and

WHEREAS, the Grantee named herein is the son and sole lineal descendant of Decedent; and

WHEREAS, the purpose of this deed is to transfer title to the herein described property from Decedent to the Grantee.

W I T N E S S E T H

NOW, THEREFORE, in consideration of the sum of TEN DOLLARS ($10.00) and other good and valuable considerations, in hand paid, at and before the sealing and delivery of these presents, the receipt of which is hereby acknowledged, Grantor has granted, bargained, sold and conveyed, and by these presents does grant, bargain, sell and convey unto the said Grantee, his heirs and assigns, all that tract or parcel of land lying and being in Columbia County, Florida, described as follows:

> Section 25, Township 2 South, Range 15 East: East 1/2 of the Northwest 1/4 of the Northeast 1/4, except for road right-of-way along North side and except West 306.60 feet and except the South 390 feet.

TO HAVE AND TO HOLD the said tract or parcel of land, with all and singular the rights, members and appurtenances thereof, to the same being, belonging, or in anywise appertaining, to the only proper use, benefit and behoof of the said Grantee, his heirs, and assigns, forever, in Fee Simple.

AND THE SAID Grantor, for his heirs, executors and administrators, will warrant and forever defend the right and title to the above described property, unto the said Grantee, his heirs and assigns, against the claims of all persons whomsoever.

IN WITNESS WHEREOF, the said Grantor has hereunto set his hand and seal, the day and year above written.

Signed, sealed and delivered
in the presence of:

Joanne Gugliuzza (SEAL)
WILLIAM F. DAUGHERTY, JR., as
Personal Representative of the
Estate of WILLIAM F. DAUGHERTY

Witness

Joanne Gugliuzza
Print or type name

Eileen House
Witness

Eileen House
Print or type name

BK 0917 PG 0273
OFFICIAL RECORDS

STATE OF NEW JERSEY

COUNTY OF OCEAN

The foregoing instrument was acknowledged before me on the 26th day of December, 2000, by WILLIAM F. DAUGHERTY, JR., as personal representative of the estate of WILLIAM F. DAUGHERTY, SR., deceased, who is personally known to me or produced FLORIDA LICENCE as identification.
CUSTOMER AT SUMMIT BANK

Joanne Gugliuzza
Notary Public State of New Jersey
JOANNE GUGLIUZZA
Print or type name

(NOTARIAL SEAL)

JOANNE GUGLIUZZA
NOTARY PUBLIC OF NEW JERSEY
My Commission Expires Feb.28, 2002

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Daugherty Residence**	Builder:	**Don Reed**
Address:	**Suwannee Valley Rd.**	Permitting Office:	Columbia
City, State:	**Lake City, FL**	Permit Number:	23740
Owner:	**William & Janice Daugherty**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 30.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 10.00
4. Number of Bedrooms	4	b. N/A	
5. Is this a worst case?	Yes		
6. Conditioned floor area (ft²)	1890 ft²	c. N/A	
7. Glass area & type			
a. Clear - single pane	0.0 ft²	13. Heating systems	
b. Clear - double pane	230.0 ft²	a. Electric Heat Pump	Cap: 30.0 kBtu/hr
c. Tint/other SHGC - single pane	0.0 ft²		HSPF: 6.80
d. Tint/other SHGC - double pane	0.0 ft²	b. N/A	
8. Floor types			
a. Slab-On-Grade Edge Insulation	R=0.0, 228.0(p) ft	c. N/A	
b. N/A			
c. N/A		14. Hot water systems	
9. Wall types		a. Electric Resistance	Cap: 40.0 gallons
a. Frame, Wood, Exterior	R=13.0, 1825.0 ft²		EF: 0.88
b. N/A		b. N/A	
c. N/A			
d. N/A		c. Conservation credits	
e. N/A		(HR-Heat recovery, Solar DHP-Dedicated heat pump)	
10. Ceiling types		15. HVAC credits	
a. Under Attic	R=30.0, 1890.0 ft²	(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 120.0 ft		
b. N/A			

Glass/Floor Area: 0.12

Total as-built points: 29788
Total base points: 32122

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 8-02-05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA
IN GOD WE TRUST

BUILDING OFFICIAL: _____

DATE: 8-02-05

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Suwannee Valley Rd., Lake City, FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Suwannee Valley Rd., Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
4		2746.00	10984.0	40.0	0.88	4		1.00	2746.00	1.00	10984.0
				As-Built Total:							**10984.0**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
10470	**10668**	**10984**	**32122**	**8888**	**9916**	**10984**	**29788**

PASS

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Suwannee Valley Rd., Lake City, FL,**	PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**17003.1**	**Winter As-Built Points:**					**17014.2**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
17003.1	**0.6274**	**10667.8**	17014.2 **17014.2**	1.000 **1.00**	(1.069 x 1.169 x 0.93) **1.162**	0.501 **0.501**	1.000 **1.000**	9915.9 **9915.9**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Suwannee Valley Rd., Lake City, FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1890.0	12.74	4334.1	Double, Clear	E	1.5	6.5	72.0	9.09	1.03	674.6	
				Double, Clear	E	12.0	7.5	42.0	9.09	1.41	537.1	
				Double, Clear	E	1.5	4.0	6.0	9.09	1.07	58.6	
				Double, Clear	E	3.0	5.5	12.5	9.09	1.14	129.2	
				Double, Clear	W	1.5	6.0	30.0	10.77	1.02	330.5	
				Double, Clear	NW	3.0	6.0	12.5	14.03	1.01	177.7	
				Double, Clear	W	1.5	6.0	18.3	10.77	1.02	202.0	
				Double, Clear	N	1.5	6.0	36.7	14.30	1.00	525.7	
				As-Built Total:				**230.0**			**2635.5**	

WALL TYPES	Area X	BWPM =	Points	Type	R-Value	Area X	WPM =	Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	13.0	1825.0	3.40	6205.0
Exterior	1825.0	3.70	6752.5					
Base Total:	**1825.0**		**6752.5**	**As-Built Total:**		**1825.0**		**6205.0**

DOOR TYPES	Area X	BWPM =	Points	Type	Area X	WPM =	Points
Adjacent	20.0	11.50	230.0	Exterior Wood	20.0	12.30	246.0
Exterior	73.0	12.30	897.9	Adjacent Wood	20.0	11.50	230.0
				Exterior Wood	53.0	12.30	651.9
Base Total:	**93.0**		**1127.9**	**As-Built Total:**	**93.0**		**1127.9**

CEILING TYPES	Area X	BWPM =	Points	Type	R-Value	Area X	WPM X WCM =	Points
Under Attic	1890.0	2.05	3874.5	Under Attic	30.0	1890.0	2.05 X 1.00	3874.5
Base Total:	**1890.0**		**3874.5**	**As-Built Total:**		**1890.0**		**3874.5**

FLOOR TYPES	Area X	BWPM =	Points	Type	R-Value	Area X	WPM =	Points
Slab	228.0(p)	8.9	2029.2	Slab-On-Grade Edge Insulation	0.0	228.0(p	18.80	4286.4
Raised	0.0	0.00	0.0					
Base Total:			**2029.2**	**As-Built Total:**		**228.0**		**4286.4**

INFILTRATION	Area X	BWPM =	Points		Area X	WPM =	Points
	1890.0	-0.59	-1115.1		1890.0	-0.59	-1115.1

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Suwannee Valley Rd., Lake City, FL,**	PERMIT #:

BASE				AS-BUILT						
Summer Base Points:			**24544.0**	**Summer As-Built Points:**						**22888.9**
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
24544.0	**0.4266**		**10470.5**	22888.9 **22888.9**	1.000 **1.00**	(1.090 x 1.147 x 0.91) **1.138**	0.341 **0.341**	1.000 **1.000**		8887.8 **8887.8**

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Suwannee Valley Rd., Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	**1890.0**	**20.04**	**6817.6**	Double, Clear	E	1.5	6.5	72.0	40.22	0.93	2683.3
				Double, Clear	E	12.0	7.5	42.0	40.22	0.42	711.0
				Double, Clear	E	1.5	4.0	6.0	40.22	0.82	196.8
				Double, Clear	E	3.0	5.5	12.5	40.22	0.70	350.2
				Double, Clear	W	1.5	6.0	30.0	36.99	0.91	1013.5
				Double, Clear	NW	3.0	6.0	12.5	25.46	0.78	249.5
				Double, Clear	W	1.5	6.0	18.3	36.99	0.91	619.3
				Double, Clear	N	1.5	6.0	36.7	19.22	0.94	661.5
				As-Built Total:				**230.0**			**6485.1**

WALL TYPES	Area X	BSPM =	Points	Type	R-Value	Area X	SPM =	Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	13.0	1825.0	1.50	2737.5
Exterior	1825.0	1.70	3102.5					
Base Total:	**1825.0**		**3102.5**	**As-Built Total:**		**1825.0**		**2737.5**

DOOR TYPES	Area X	BSPM =	Points	Type	Area X	SPM =	Points
Adjacent	20.0	2.40	48.0	Exterior Wood	20.0	6.10	122.0
Exterior	73.0	6.10	445.3	Adjacent Wood	20.0	2.40	48.0
				Exterior Wood	53.0	6.10	323.3
Base Total:	**93.0**		**493.3**	**As-Built Total:**	**93.0**		**493.3**

CEILING TYPES	Area X	BSPM =	Points	Type	R-Value	Area X	SPM X SCM =	Points
Under Attic	1890.0	1.73	3269.7	Under Attic	30.0	1890.0	1.73 X 1.00	3269.7
Base Total:	**1890.0**		**3269.7**	**As-Built Total:**		**1890.0**		**3269.7**

FLOOR TYPES	Area X	BSPM =	Points	Type	R-Value	Area X	SPM =	Points
Slab	228.0(p)	-37.0	-8436.0	Slab-On-Grade Edge Insulation	0.0	228.0(p	-41.20	-9393.6
Raised	0.0	0.00	0.0					
Base Total:			**-8436.0**	**As-Built Total:**		**228.0**		**-9393.6**

INFILTRATION	Area X	BSPM =	Points		Area X	SPM =	Points
	1890.0	10.21	19296.9		1890.0	10.21	19296.9

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 83.6

The higher the score, the more efficient the home.

William & Janice Daugherty, Suwannee Valley Rd., Lake City, FL,

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family	a.	Central Unit	Cap: 30.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 10.00
4.	Number of Bedrooms	4	b.	N/A	
5.	Is this a worst case?	Yes			
6.	Conditioned floor area (ft²)	1890 ft²	c.	N/A	
7.	Glass area & type				
a.	Clear - single pane	0.0 ft²	13.	Heating systems	
b.	Clear - double pane	230.0 ft²	a.	Electric Heat Pump	Cap: 30.0 kBtu/hr
c.	Tint/other SHGC - single pane	0.0 ft²			HSPF: 6.80
d.	Tint/other SHGC - double pane	0.0 ft²	b.	N/A	
8.	Floor types				
a.	Slab-On-Grade Edge Insulation	R=0.0, 228.0(p) ft	c.	N/A	
b.	N/A				
c.	N/A		14.	Hot water systems	
9.	Wall types		a.	Electric Resistance	Cap: 40.0 gallons
a.	Frame, Wood, Exterior	R=13.0, 1825.0 ft²			EF: 0.88
b.	N/A		b.	N/A	
c.	N/A				
d.	N/A		c.	Conservation credits	
e.	N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
a.	Under Attic	R=30.0, 1890.0 ft²	15.	HVAC credits	
b.	N/A			(CF-Ceiling fan, CV-Cross ventilation,	
c.	N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 120.0 ft		RB-Attic radiant barrier,	
b.	N/A			MZ-C-Multizone cooling,	
				MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

EnergyGauge® (Version: FLRCPB v3.2)

Mark Disosway, P.E.
POB 868, Lake City, FL 32056, Ph (386) 754-5419, Fax (386) 269-4871

October 19, 2005

Building Department

Owner Builder, William Daugherty, 210 NW Kissimmee Way, Lake City, FL, 32055 25-2S-15-00093-009

Dear Building Official:

Please accept this letter as addendum to the plans for the above referenced house to change all references to FBC 2001 to FBC 2004.

- The plan was drawn prior to the effective date for FBC 2004, 01 October 2005.
- Since the wind load requirements of FBC 2004 remain basically unchanged from FBC 2001 there are no structural changes required to this plan.

Mark Disosway, PE
Florida Registered Professional Engineer

Owner Builder

Don Reed Const. Fax 755-7272

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001
ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS LISTED ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INCLUDE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 SECTION 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA. OR ALTERNATE METHODOLOGIES APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND TWO-FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: U.S. HIGHWAY 41 FROM COLUMBIA COUNTYS NORTHERN BOUNDARY TO THE INTERSECTION OF MYRTIS ROAD, FOLLOW MYRTIS EAST TO THE INTERSECTION OF C.R. 245, FOLLOW C.R. 245 SOUTH TO THE SOUTHERN BOUNDARY OF COLUMBIA COUNTY.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ---------- 100 MPH
   ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------- 110 MPH
2. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

GENERAL REQUIREMENTS: Two (2) complete set of plans containing the following:

Applicant	Plans Examiner	
✓	✓	All drawings must be clear, concise and drawn to scale("Optional"details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans
✓	✓	Designer's name and signature on document(FBC 104.2.1) If licensed architect or engineer, official seal shall be affixed
✓	✓	**Site Plan including:**
		a) Dimensions of lot
		b) Dimensions of building setbacks
		c) Location of all other buildings on lot , well and septic tank if applicable, and all utility easements.
		d) Provide a full legal description of property
✓	✓	**Wind-load Engineering Summary, calculations and any details required**
		a) Plans or specifications must state compliance with FBC Section 1606
		b) The following information must be shown as per secition 1606 1.7 FBC
		a. Basic wind speed (MPH)
		b. Wind importance factor (I) and building category
		c. Wind exposure - if more than one wind expore is used . the wind exposure and applicable wind dircetion shall be indicated
		d. The applicable internal pressure coefficient
		e. Components and Cladding. The design wind pressure in terms of psf (kN/m^2), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional
		Elevations including:
✓	-	a) All Sides
✓	-	b) Roof pitch
✓	-	c) Overhang dimensions and detail with attic ventilation

d) Location, size and height above roof of chimneys
e) Location and size of skylights
d) Building height
e) Number of stories

Floor Plan including:

a) Rooms labeled and dimensioned
b) Shear walls
c) Windows and Doors(including garage doors) showing size, mfg, approval listing and attachmenspecs.(FBC1707)and safety glazing where needed (egress windows in bedrooms to be shown)
d) Fireplaces (gas appliance(vented or non-vented) or wood burning with hearth
e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
f) Must show and identify accessability requirements (accessible bathroom)

Foundation Plan including:

a) Location of all load bearing walls with required footings indicated as standard or monolithic and their dimensions and reinforcing
b) All posts and/or column footing including size and reinforcing
c) Any special support required by soil analysis such as piling
d) Location of any vertical steel

Roof System

a) Truss package including:
    1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
    2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

b) Conventional Framing Layout including
    1. Rafter size, species and spacing
    2. Attachment to wall and uplift
    3. Ridge Beam sized and valley framing and support details
    4. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

a) Masonry wall
    1. All materials making up wall
    2. Block size and motar type with size and spacing of reinforcement
    3. Lintel, tie-beam sizes and reinenforcement
    4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
    5. All required connectors with uplift rating and required number and size of fasteners for continous tie from roof to foundation
    6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
    7. Fire resistant construction (if required)
    8. Fireproofing requirements
    9. Show type of termite treatment (termiticide or alternative method)
    10. Slab on grade
        a. Vapor retarder (6 mil. polyethylene with joints lapped 6 inches and sealed)
        b. Must show control joints, synthetic fiber reinforcement or

welded wire fabric reinfrocement and supports

11. Indicate where pressuretreated wood will be placed
12. Provide insulation R value for the following:
    a. Attic spuce
    b. Exterior wall cavity
    c. Crawl space (if applicable)

b) Wood Frame wall
1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required connectors with uplift rating and required number and size of fasteners for continous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fasting requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if required)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6 mil polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinenforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

c) Metal Frame wall and roof (Designed, signed and sealed by Fl. Reg. Prof. Engineer or Architect)

Floor Framing System

a) Floor truss package including layout and details signed and sealed by Fl. Reg. P.E.
b) Floor joist size and spacing
c) Girder size and spacing
d) Attachment of joist to girder
e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
b) Ceiling fans
c) Smoke detectors
d) Service panel and sub-panel size and location(s)
e) Meter location with type of service entrance (overhead or underground)
f) Appliances and HVAC equipment

HVAC information

a) Manual J sizing equipment or equivalent computation
b) Exhaust fans in bathrooms

☐ ☐
☐ ☐

Energy Calculations (dimensions shall match plans)
Gas System Type (LP or Natural) Location and BTU demand of equipment
Disclosure Statement for Owner Builders — only if ~~owner~~ owner is building owner
Notice of Commencement
Private Potable Water
a) Size of pump motor
b) Size of pressure tank
c) Cycle Stop Valve if used

Project Information for:	L130844		
Builder:	DON REED	Date:	9/27/2005
Lot:	N/A	Start Number:	114
Subdivision:	210 N.W. KISSIMEE WAY		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	50		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		**Wind**		**Building Code:**	FBC2001
Roof (psf):	42	Wind Standard:	ASCE 7-98		
Floor (psf):	55	Wind Speed (mph):	120		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

REED, LARRY DON CGC 036224
Address: 2230 E BAYA AVE. STE 101
GLEN ST MARY FL 32040 Designer: 87

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:
1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI 1-1995 Section 2.2
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	092705114	9/27/2005	41	T24	092705154	9/27/2005
2	CJ1A	092705115	9/27/2005	42	V01	092705155	9/27/2005
3	CJ3	092705116	9/27/2005	43	V02	092705156	9/27/2005
4	CJ3A	092705117	9/27/2005	44	V03	092705157	9/27/2005
5	CJ5	092705118	9/27/2005	45	V04	092705158	9/27/2005
6	CJ5A	092705119	9/27/2005	46	V05	092705159	9/27/2005
7	EJ2	092705120	9/27/2005	47	V06	092705160	9/27/2005
8	EJ5	092705121	9/27/2005	48	V07	092705161	9/27/2005
9	EJ7	092705122	9/27/2005	49	V08	092705162	9/27/2005
10	EJ7A	092705123	9/27/2005	50	V09	092705163	9/27/2005
11	EJ7B	092705124	9/27/2005				
12	EJ7C	092705125	9/27/2005				
13	HGBL01	092705126	9/27/2005				
14	HJ2	092705127	9/27/2005				
15	HJ5	092705128	9/27/2005				
16	HJ7	092705129	9/27/2005				
17	HJ7A	092705130	9/27/2005				
18	T01	092705131	9/27/2005				
19	T02	092705132	9/27/2005				
20	T03	092705133	9/27/2005				
21	T04	092705134	9/27/2005				
22	T05	092705135	9/27/2005				
23	T06	092705136	9/27/2005				
24	T07	092705137	9/27/2005				
25	T08	092705138	9/27/2005				
26	T09	092705139	9/27/2005				
27	T10	092705140	9/27/2005				
28	T11	092705141	9/27/2005				
29	T12	092705142	9/27/2005				
30	T13	092705143	9/27/2005				
31	T14	092705144	9/27/2005				
32	T15	092705145	9/27/2005				
33	T16	092705146	9/27/2005				
34	T17	092705147	9/27/2005				
35	T18	092705148	9/27/2005				
36	T19	092705149	9/27/2005				
37	T20	092705150	9/27/2005				
38	T21	092705151	9/27/2005				
39	T22	092705152	9/27/2005				
40	T23	092705153	9/27/2005				

SEP 27 2005

Dwg.#092705113

Licensee Details

Licensee Information

Name:	**REED, LARRY DON (Primary Name)** **DON REED CONSTRUCTION INC (DBA Name)**
Main Address:	**2230 E BAYA AVE STE 101** **LAKE CITY Florida 32025**
County:	**COLUMBIA**
License Mailing:	
LicenseLocation:	**2230 E BAYA AVE STE 101** **LAKE CITY FL 32025**
County:	**COLUMBIA**

License Information

License Type:	**Certified General Contractor**
Rank:	**Cert General**
License Number:	**CGC036224**
Status:	**Current,Active**
Licensure Date:	**03/08/1986**
Expires:	**08/31/2006**
Special Qualifications	**Qualification Effective**
Bldg Code Core Course Credit	
Qualified Business License Required	**08/13/2004**

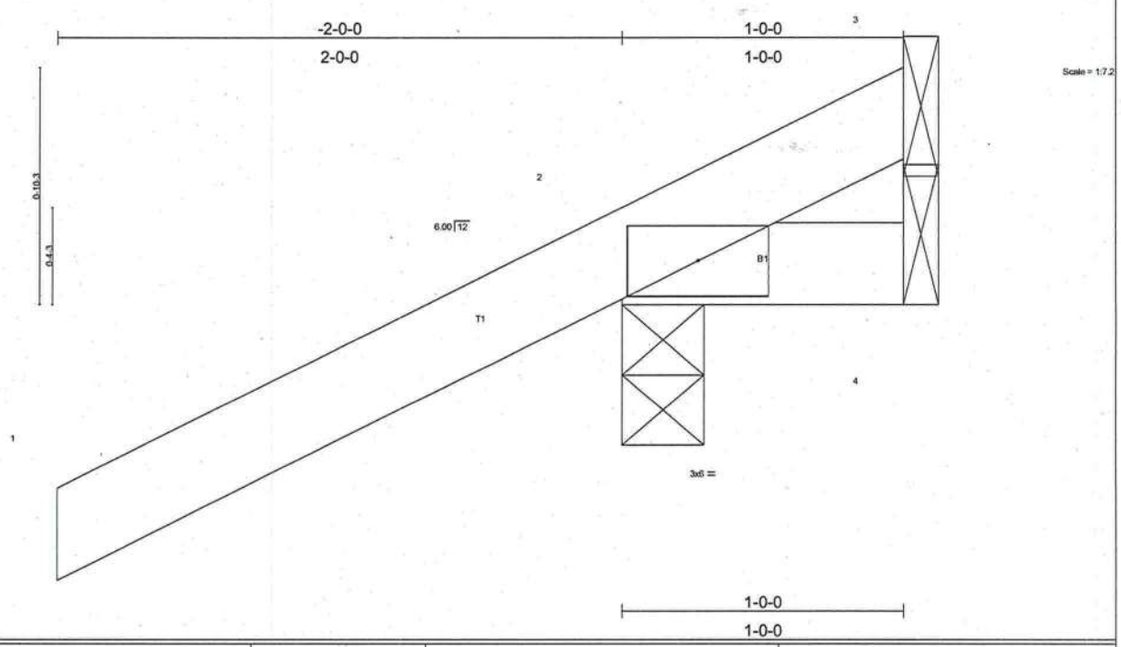
View Related License Information

View License Complaint

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9195
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705114
L130844	CJ1	MONO TRUSS	18	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:09 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=-90/Mechanical
Max Horz 2=103(load case 5)
Max Uplift2=-349(load case 5), 4=-11(load case 3), 3=-90(load case 1)
Max Grav 2=266(load case 1), 4=14(load case 1), 3=154(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/86
BOT CHORD 2-4=0/0

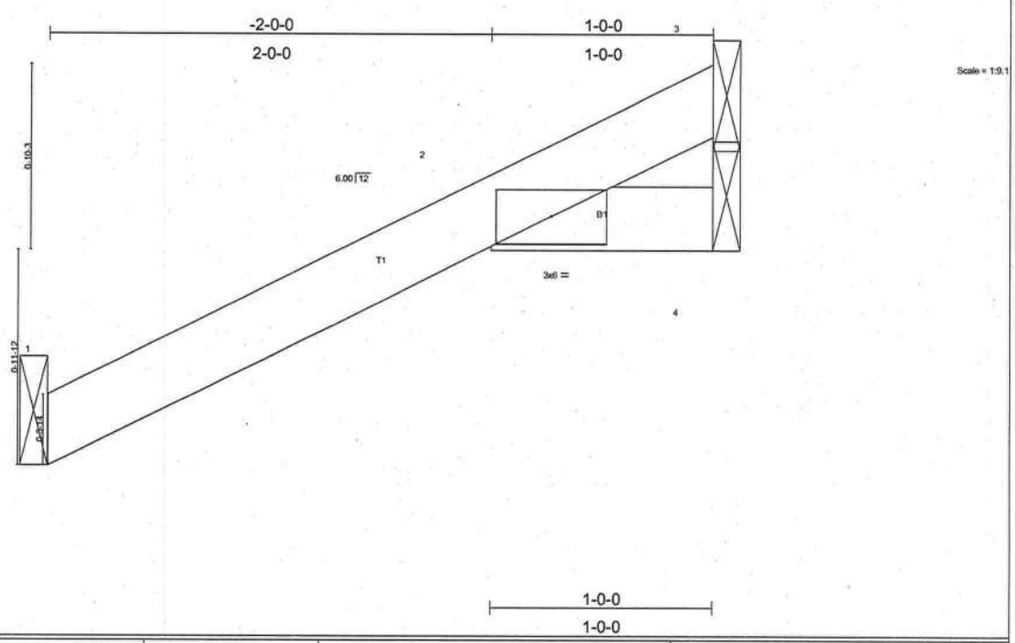
NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 349 lb uplift at joint 2, 11 lb uplift at joint 4 and 90 lb uplift at joint 3.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705115
L130844	CJ1A	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:10 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	0.03	1-2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	0.03	1-2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=98/Mechanical, 4=15/Mechanical, 3=96/Mechanical
Max Horz 1=80(load case 5)
Max Uplift1=-43(load case 5), 4=-12(load case 3), 3=-92(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-87/7, 2-3=-93/43
BOT CHORD 2-4=0/0

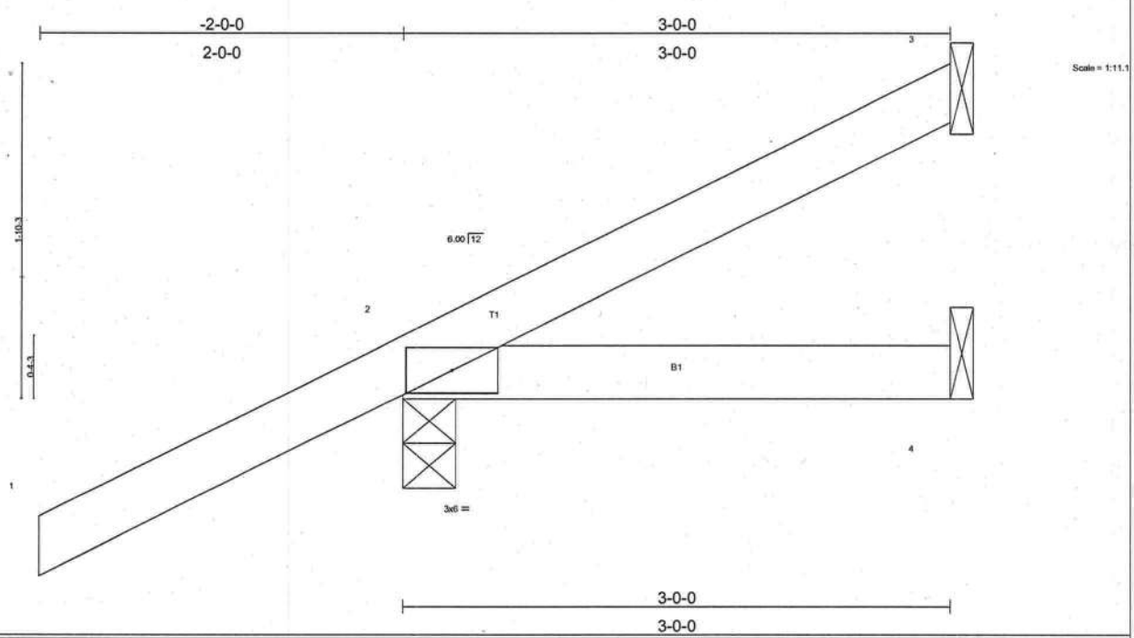
NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 43 lb uplift at joint 1, 12 lb uplift at joint 4 and 92 lb uplift at joint 3.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705116
L130844	CJ3	MONO TRUSS	19	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:10 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=157(load case 5)
Max Uplift3=-30(load case 4), 2=-292(load case 5), 4=-33(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

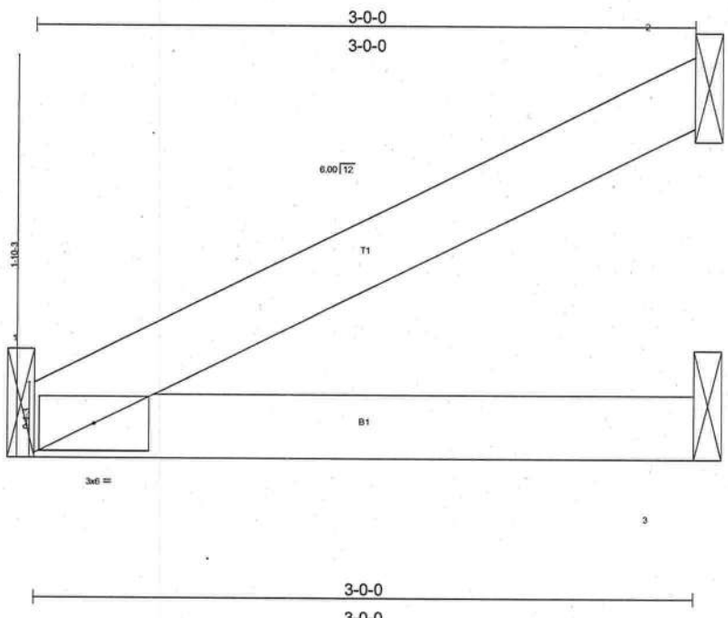
NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 3, 292 lb uplift at joint 2 and 33 lb uplift at joint 4.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705117
L130844	CJ3A	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:11 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.18	Vert(LL)	-0.00	1-3	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	1-3	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 10 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=121/Mechanical, 2=78/Mechanical, 3=43/Mechanical
Max Horz 1=83(load case 5)
Max Uplift1=-37(load case 5), 2=-88(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-90/29
BOT CHORD 1-3=0/0

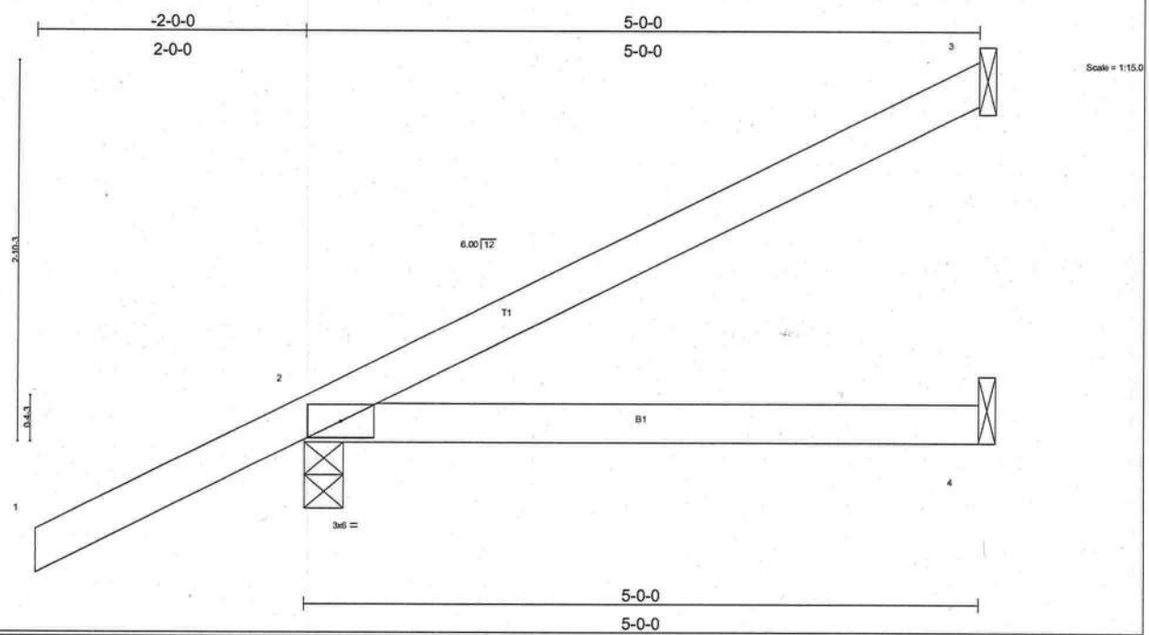
NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 1 and 88 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705118
L130844	CJ5	MONO TRUSS	14	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:11 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.10	2-4	>557	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	0.09	2-4	>633	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=211(load case 5)
Max Uplift3=-107(load case 5), 2=-320(load case 5), 4=-57(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-127/36
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 107 lb uplift at joint 3, 320 lb uplift at joint 2 and 57 lb uplift at joint 4.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705119
L130844	CJ5A	MONO TRUSS	2	1	Job Reference (optional)

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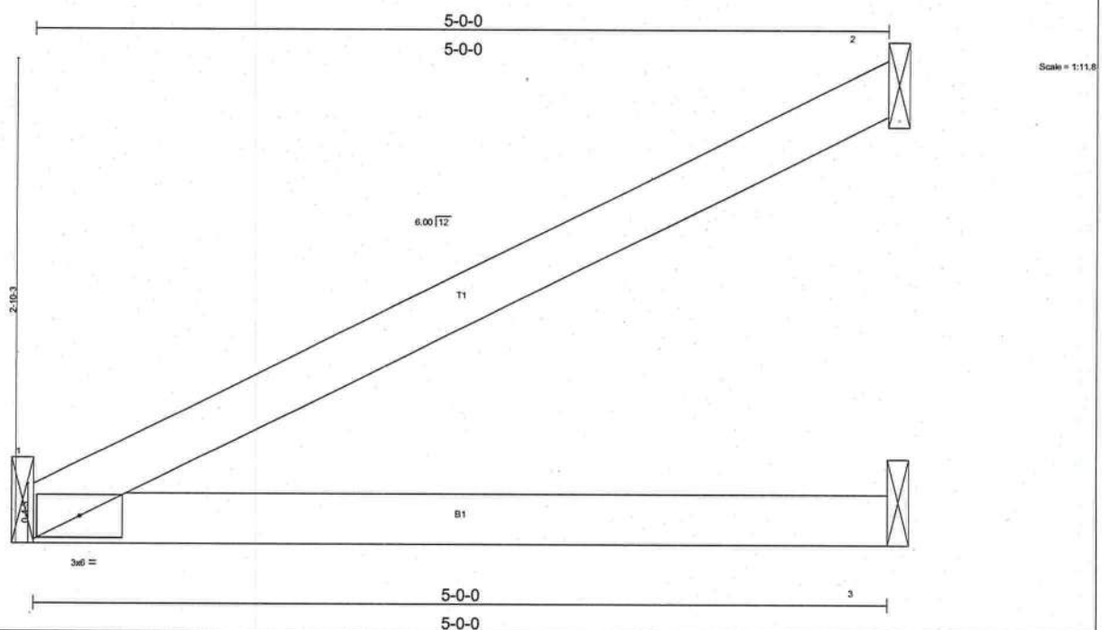

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.03	1-3	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.05	1-3	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=205/Mechanical, 2=132/Mechanical, 3=73/Mechanical
Max Horz 1=138(load case 5)
Max Uplift1=-63(load case 5), 2=-149(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-149/48
BOT CHORD 1-3=0/0

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 1 and 149 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705120
L130844	EJ2	MONO TRUSS	6	1	Job Reference (optional)

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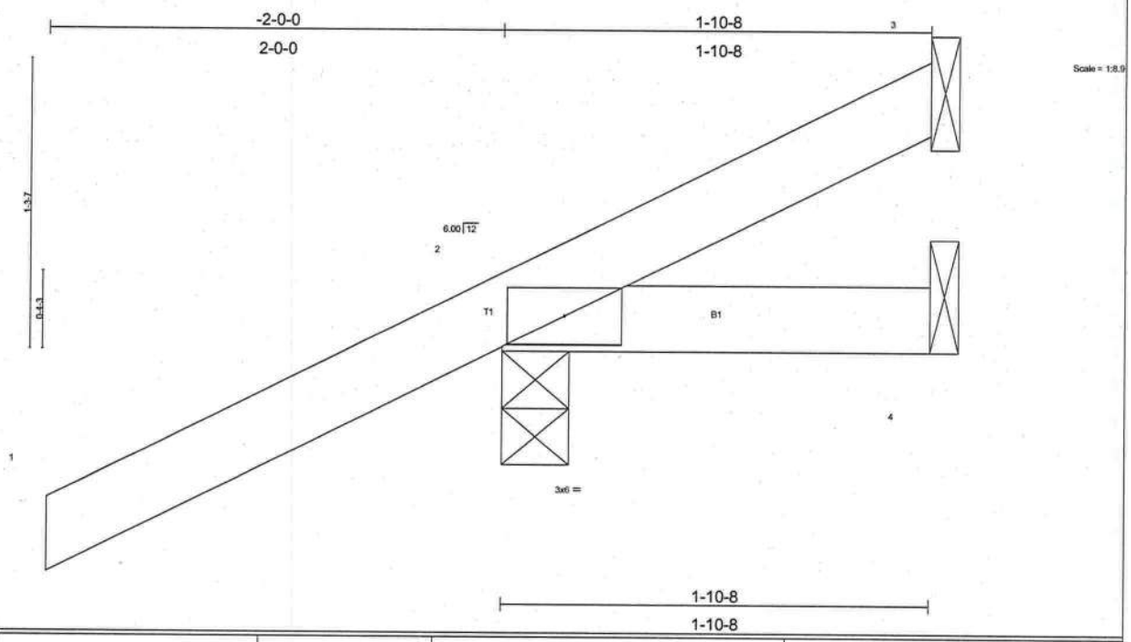

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.02	Vert(TL)	-0.00	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 9 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-10-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=247/0-3-8, 4=27/Mechanical, 3=-11/Mechanical
Max Horz 2=127(load case 5)
Max Uplift2=-258(load case 5), 3=-11(load case 1)
Max Grav 2=247(load case 1), 4=27(load case 1), 3=40(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-51/18
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 258 lb uplift at joint 2 and 11 lb uplift at joint 3.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705121
L130844	EJ5	MONO TRUSS	3	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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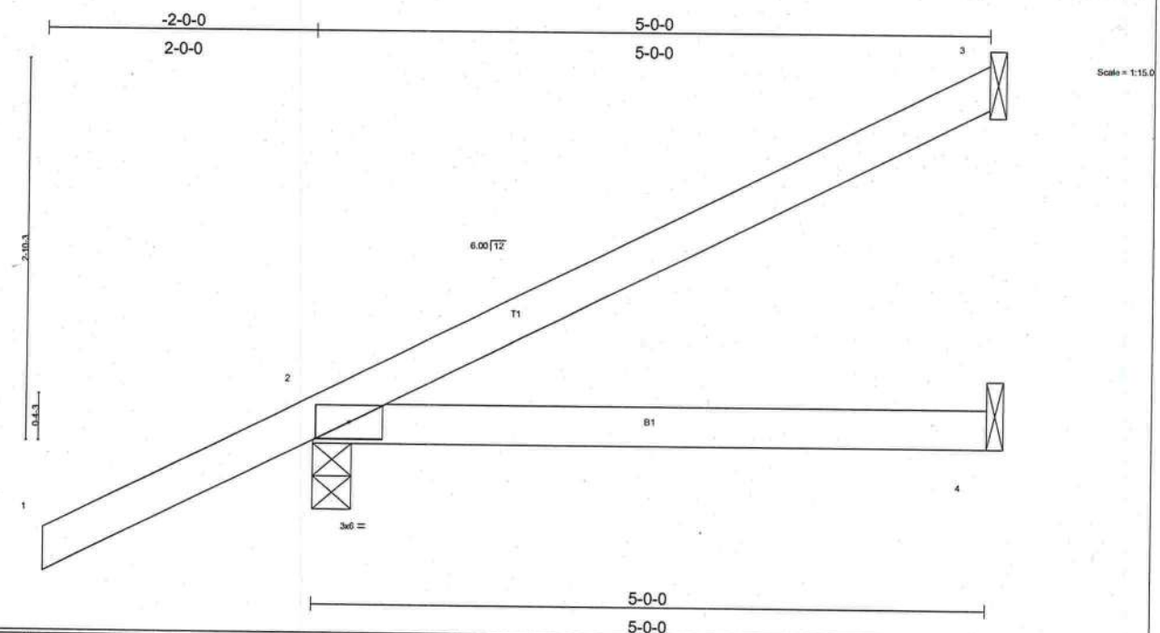

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.10	2-4	>557	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	0.09	2-4	>633	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=211(load case 5)
Max Uplift3=-107(load case 5), 2=-320(load case 5), 4=-57(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-127/36
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 107 lb uplift at joint 3, 320 lb uplift at joint 2 and 57 lb uplift at joint 4.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705122
L130844	EJ7	MONO TRUSS	21	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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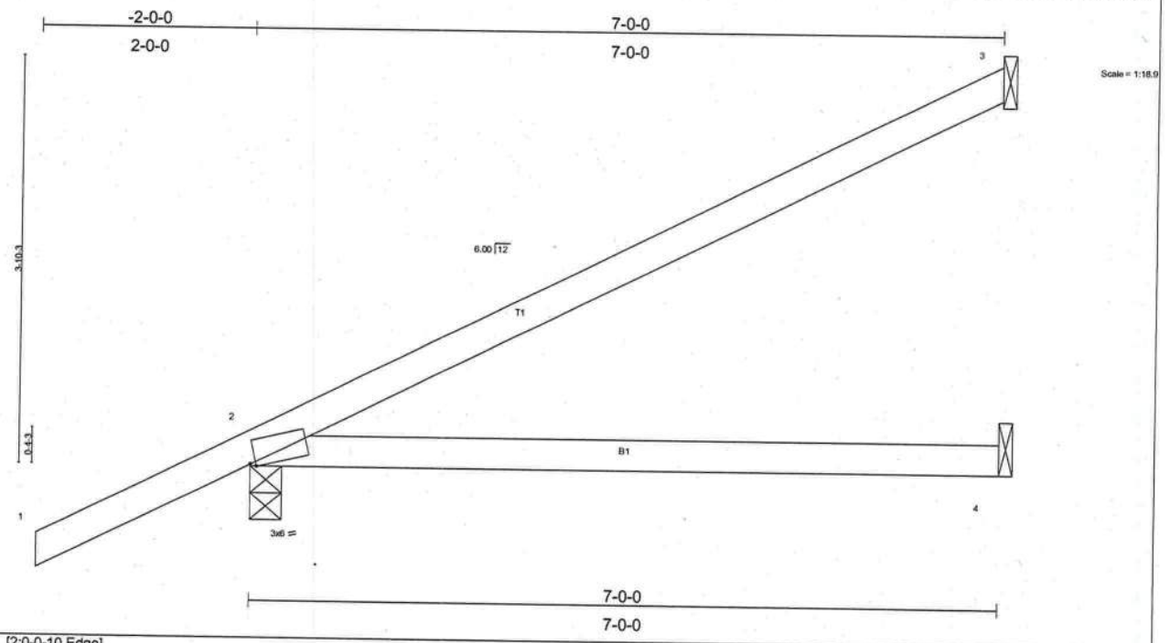

Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.32	2-4	>256	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	0.27	2-4	>303	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=419/0-3-8, 4=104/Mechanical
Max Horz 2=266(load case 5)
Max Uplift3=-176(load case 5), 2=-364(load case 5), 4=-84(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-107/58
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 176 lb uplift at joint 3, 364 lb uplift at joint 2 and 84 lb uplift at joint 4.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705123
L130844	EJ7A	MONO HIP	1	1	Job Reference (optional)

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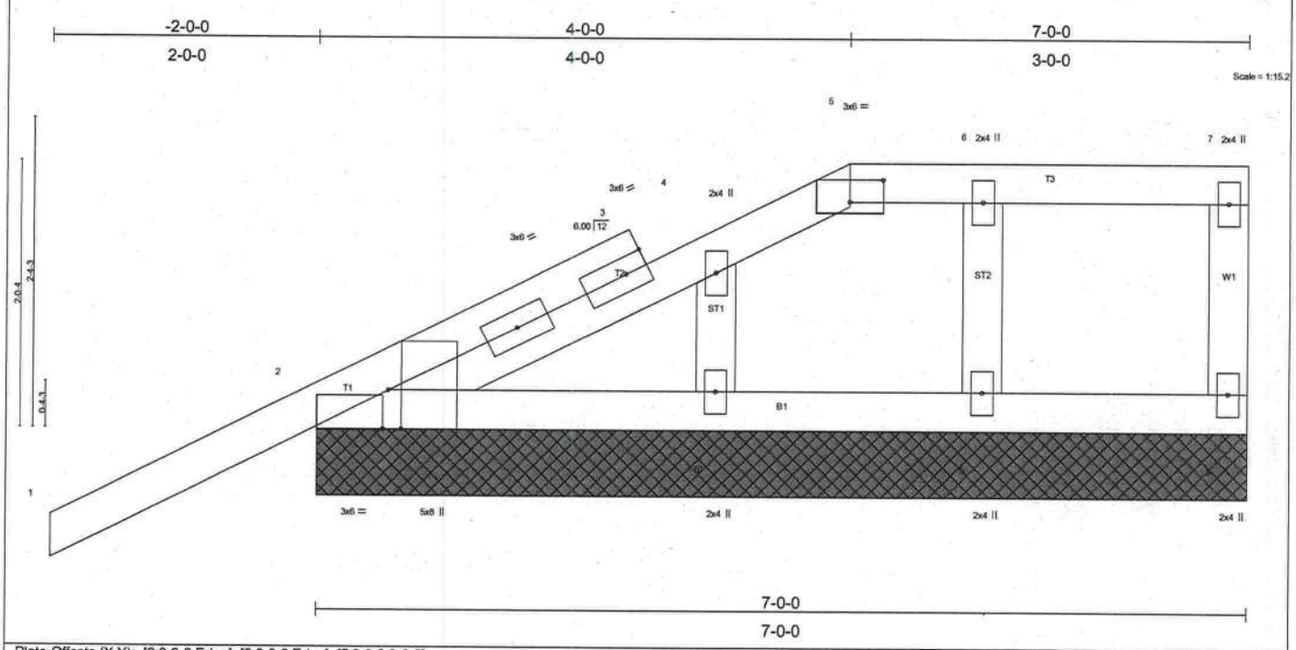

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [5:0-3-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.01	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.03	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.06	Horz(TL)	-0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 34 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=400/7-0-0, 8=101/7-0-0, 9=229/7-0-0, 10=245/7-0-0
Max Horz 2=169(load case 5)
Max Uplift2=-299(load case 5), 8=-49(load case 4), 9=-115(load case 3), 10=-78(load case 5)
Max Grav 2=400(load case 1), 8=103(load case 8), 9=229(load case 1), 10=245(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-20/75, 2-3=-75/3, 3-4=-93/27, 4-5=-52/17, 7-8=-75/84, 5-6=-13/13, 6-7=-13/13
BOT CHORD 2-10=-13/13, 9-10=-13/13, 8-9=-13/13
WEBS 6-9=-174/220, 4-10=-176/243

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 299 lb uplift at joint 2, 49 lb uplift at joint 8, 115 lb uplift at joint 9 and 78 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-87(F=-33), 2-8=-30, 5-7=-87(F=-33)

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705124
L130844	EJ7B	MONO HIP	1	1	Job Reference (optional)

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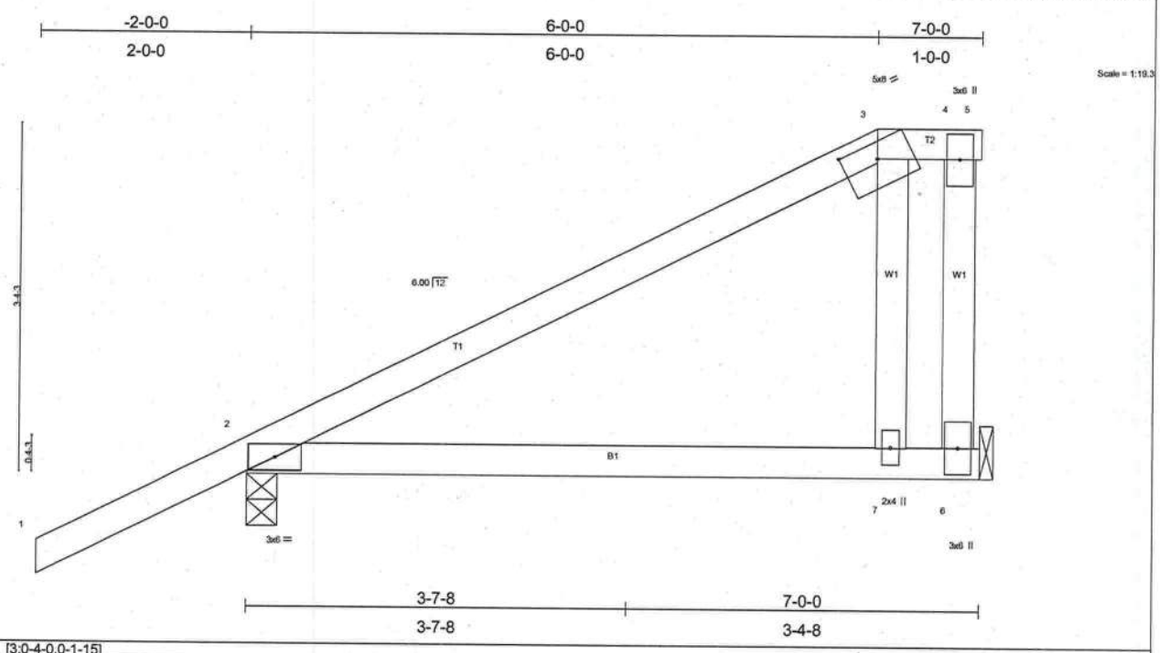


Plate Offsets (X,Y): [3:0-4-0,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	0.06	2-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.09	2-7	>843	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.10	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 34 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=263/Mechanical, 2=414/0-3-8
Max Horz 2=241(load case 5)
Max Uplift6=-129(load case 5), 2=-271(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-187/0, 3-4=-97/101, 4-5=0/0, 4-6=-86/0
BOT CHORD 2-7=-119/102, 6-7=-100/97
WEBS 3-7=-91/391

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 lb uplift at joint 6 and 271 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705125
L130844	EJ7C	MONO TRUSS	31	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:15 2005 Page 1

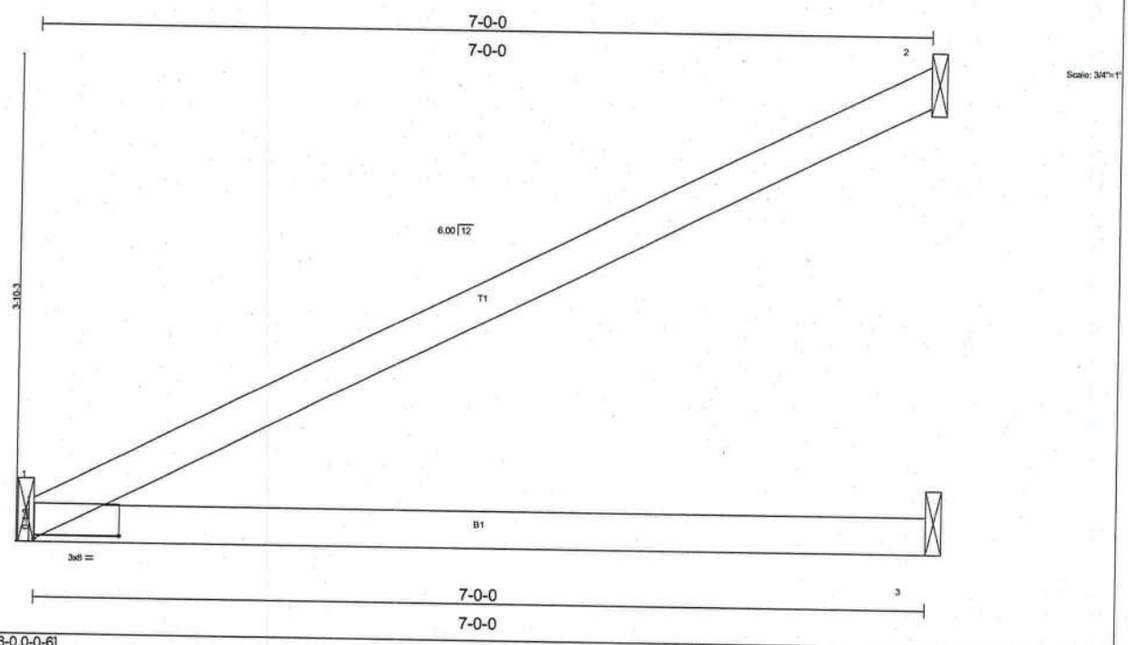


Plate Offsets (X,Y): [1:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.16	1-3	>520	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.26	1-3	>316	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=289/Mechanical, 2=173/Mechanical, 3=116/Mechanical
Max Horz 1=192(load case 5)
Max Uplift1=-90(load case 5), 2=-179(load case 5), 3=-9(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-107/62
BOT CHORD 1-3=0/0

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 90 lb uplift at joint 1, 179 lb uplift at joint 2 and 9 lb uplift at joint 3.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705126
L130844	HGBL01	ROOF TRUSS	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:16 2005 Page 1

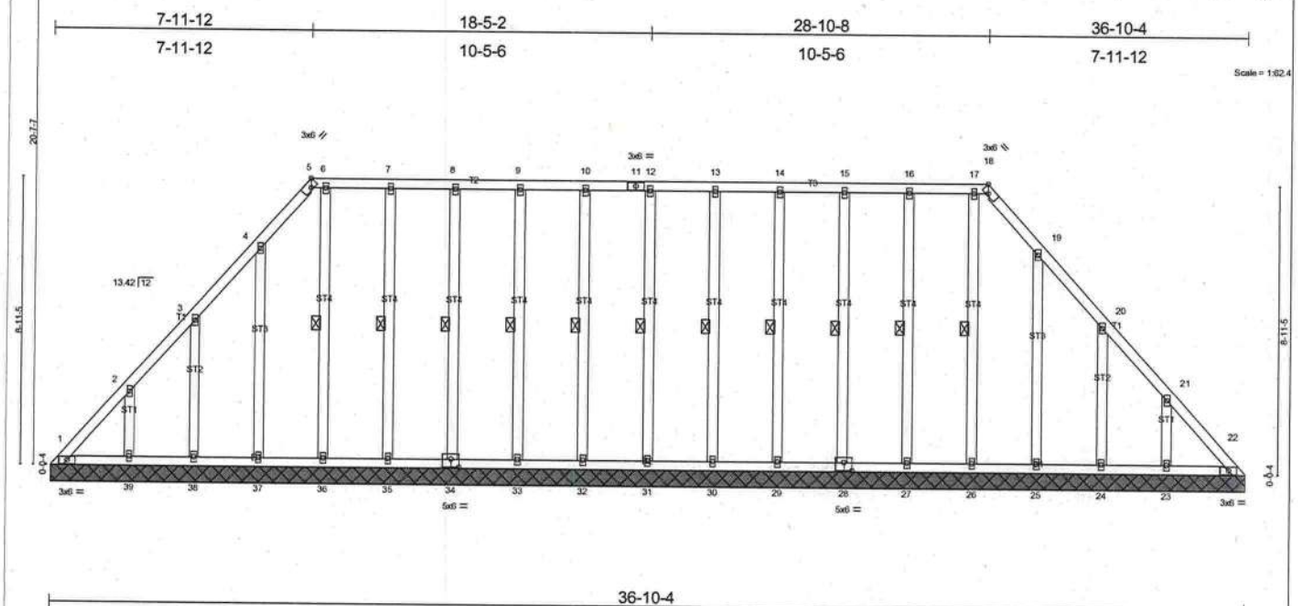


Plate Offsets (X,Y): [5:0-2-10,Edge], [18:0-2-10,Edge], [28:0-3-0,0-3-0], [34:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.01	22	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 296 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 12-31, 10-32, 9-33, 8-34, 7-35, 6-36, 13-30, 14-29, 15-28, 16-27, 17-26

REACTIONS (lb/size) 1=90/36-10-4, 31=168/36-10-4, 32=168/36-10-4, 33=169/36-10-4, 34=168/36-10-4, 35=168/36-10-4, 36=150/36-10-4, 37=164/36-10-4, 38=163/36-10-4, 39=193/36-10-4, 30=168/36-10-4, 29=169/36-10-4, 28=168/36-10-4, 27=168/36-10-4, 26=150/36-10-4, 25=164/36-10-4, 24=163/36-10-4, 23=193/36-10-4, 22=90/36-10-4
Max Horz 1=-369(load case 3)
Max Uplift1=-167(load case 3), 31=-100(load case 4), 32=-100(load case 3), 33=-100(load case 4), 34=-100(load case 4), 35=-117(load case 3), 36=-92(load case 4), 37=-183(load case 5), 38=-220(load case 5), 39=-261(load case 5), 30=-100(load case 4), 29=-100(load case 3), 28=-100(load case 3), 27=-110(load case 4), 26=-29(load case 4), 25=-171(load case 6), 24=-224(load case 6), 23=-259(load case 6), 22=-65(load case 4)
Max Grav 1=280(load case 4), 31=168(load case 1), 32=168(load case 8), 33=169(load case 8), 34=168(load case 1), 35=172(load case 8), 36=155(load case 7), 37=165(load case 7), 38=163(load case 1), 39=193(load case 7), 30=168(load case 7), 29=169(load case 7), 28=168(load case 1), 27=172(load case 7), 26=155(load case 8), 25=165(load case 8), 24=163(load case 1), 23=193(load case 8), 22=219(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-400/249, 2-3=-253/221, 3-4=-131/206, 4-5=-66/249, 18-19=-66/226, 19-20=-69/108, 20-21=-125/88, 21-22=-324/113, 5-6=-19/204, 6-7=-19/204, 7-8=-19/204, 8-9=-20/204, 9-10=-20/204, 10-11=-20/204, 11-12=-20/204, 12-13=-20/204, 13-14=-20/204, 14-15=-20/204, 15-16=-19/204, 16-17=-19/204, 17-18=-19/204
BOT CHORD 1-39=-76/271, 38-39=-76/271, 37-38=-76/271, 36-37=-76/271, 35-36=-76/271, 34-35=-76/271, 33-34=-77/270, 32-33=-77/270, 31-32=-77/270, 30-31=-77/270, 29-30=-77/270, 28-29=-77/270, 27-28=-76/271, 26-27=-76/271, 25-26=-76/271, 24-25=-76/271, 23-24=-76/271, 22-23=-76/271
WEBS 12-31=-108/112, 10-32=-108/112, 9-33=-108/112, 8-34=-108/112, 7-35=-113/128, 6-36=-130/104, 4-37=-104/194, 3-38=-107/238, 2-39=-117/250, 13-30=-108/112, 14-29=-108/112, 15-28=-108/112, 16-27=-113/122, 17-26=-95/41, 19-25=-104/181, 20-24=-107/242, 21-23=-117/248

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 167 lb uplift at joint 1, 100 lb uplift at joint 31, 100 lb uplift at joint 32, 100 lb uplift at joint 33, 100 lb uplift at joint 34, 117 lb uplift at joint 35, 92 lb uplift at joint 36, 183 lb uplift at joint 37, 220 lb uplift at joint 38, 261 lb uplift at joint 39, 100 lb uplift at joint 30, 100 lb uplift at joint 29, 100 lb uplift at joint 28, 110 lb uplift at joint 27, 29 lb uplift at joint 26, 171 lb uplift at joint 25, 224 lb uplift at joint 24, 259 lb uplift at joint 23 and 65 lb uplift at joint 22.
9) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

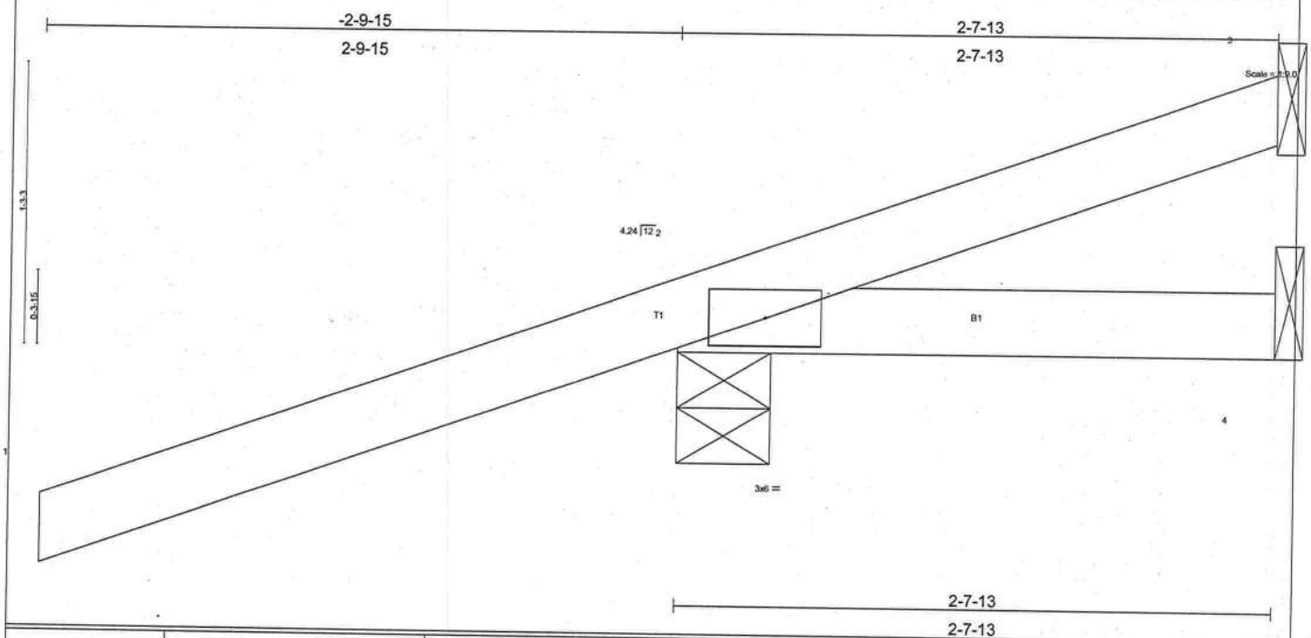
LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705127
L130844	HJ2	MONO TRUSS	2	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:17 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.74	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.02	Vert(TL)	-0.00	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=-70/Mechanical, 2=296/0-4-15, 4=18/Mechanical
Max Horz 2=64(load case 3)
Max Uplift3=-70(load case 1), 2=-321(load case 3)
Max Grav 3=150(load case 3), 2=296(load case 1), 4=18(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-45/55
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 3 and 321 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

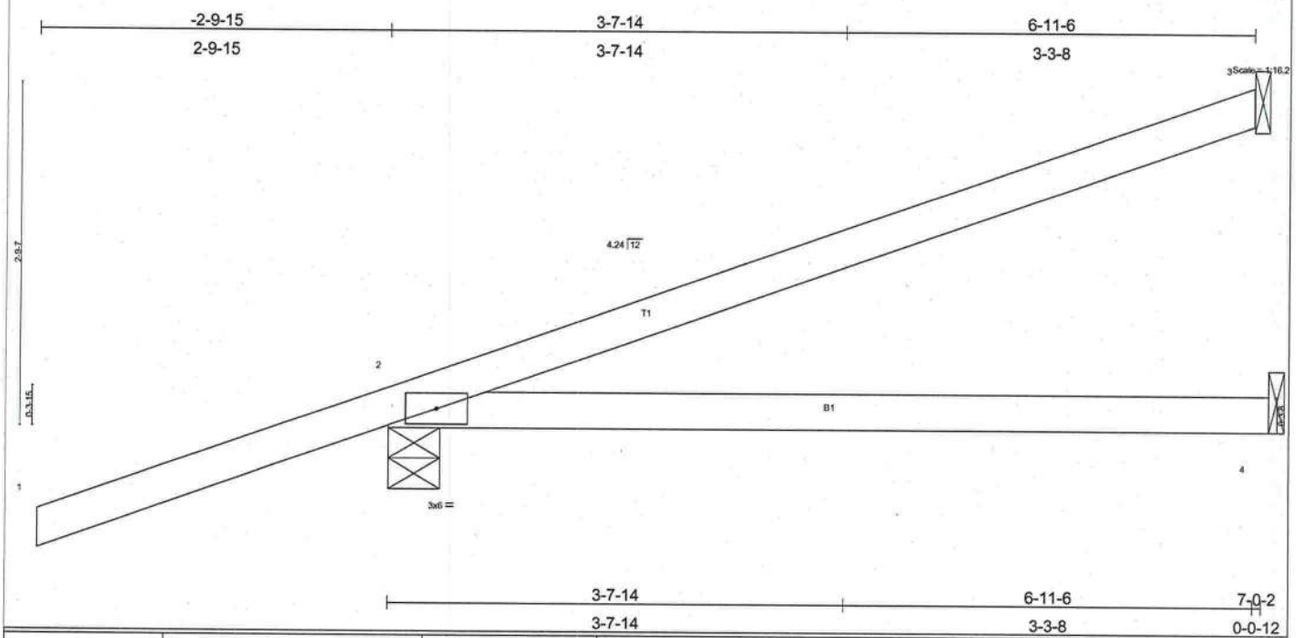
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-3=-41(F=6, B=6), 2=-0(F=15, B=15)-to-4=-23(F=4, B=4)

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705128
L130844	HJ5	MONO TRUSS	2	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:18 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.07	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.13	2-4	>626	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-11-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=188/Mechanical, 2=374/0-4-15, 4=111/Mechanical
Max Horz 2=196(load case 2)
Max Uplift3=-174(load case 2), 2=-306(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-78/44
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 174 lb uplift at joint 3 and 306 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-3=-95(F=-21, B=-21), 2=-0(F=15, B=15)-to-4=-53(F=-12, B=-12)

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
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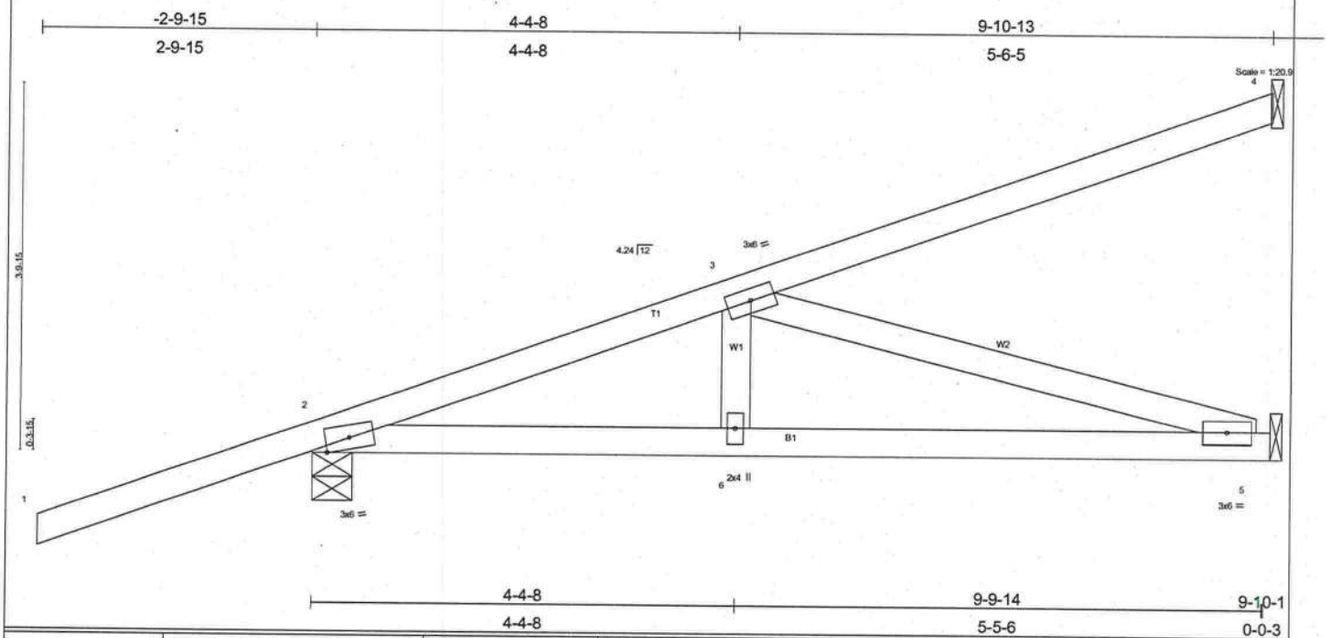
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705129
L130844	HJ7	MONO TRUSS	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:18 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.08	5-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.13	5-6	>888	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.48	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-1-3 oc bracing.

REACTIONS (lb/size) 4=264/Mechanical, 2=530/0-4-15, 5=382/Mechanical
Max Horz 2=320(load case 2)
Max Uplift4=-279(load case 2), 2=-346(load case 2), 5=-94(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-856/159, 3-4=-120/64
BOT CHORD 2-6=-381/792, 5-6=-381/792
WEBS 3-6=0/163, 3-5=-826/397

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 279 lb uplift at joint 4, 346 lb uplift at joint 2 and 94 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705130
L130844	HJ7A	MONO TRUSS	1	1	

Builders FirstSource, Lake City, Fl 32055

Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:19 2005 Page 1

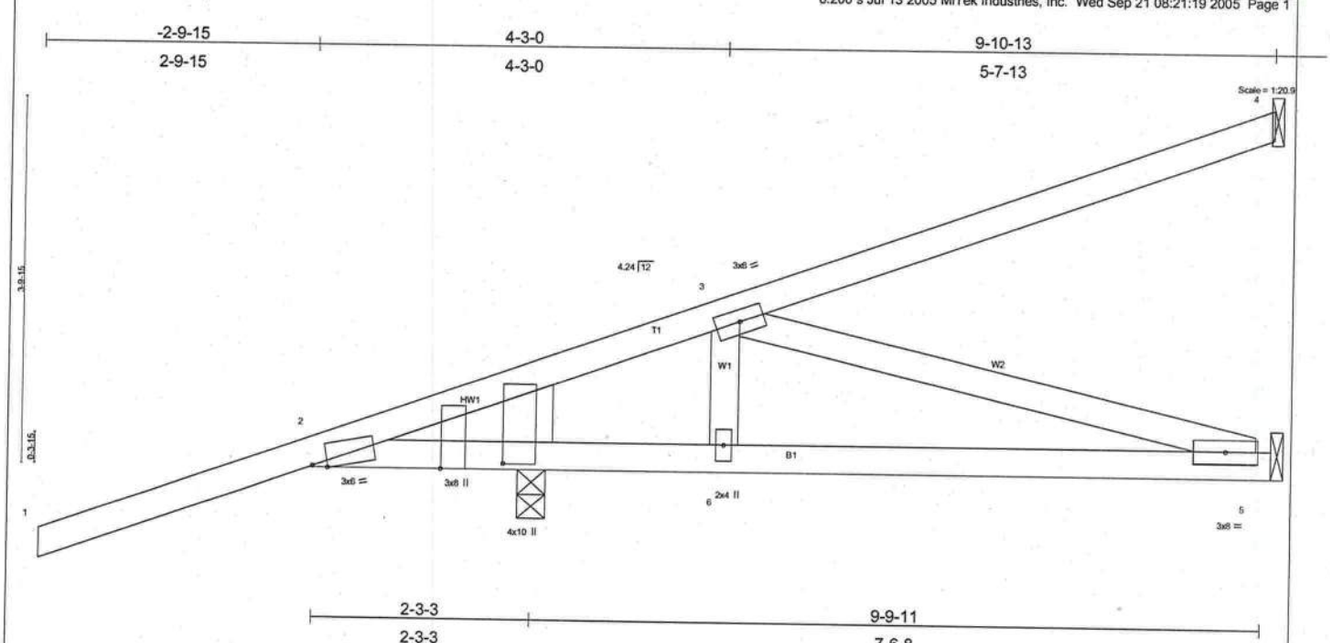


Plate Offsets (X,Y): [2:0-1-13,Edge], [2:0-0-4,Edge], [2:0-0-8,1-11-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.08	5-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.14	5-6	>831	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.51	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 50 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 8 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-11-9 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 5=381/Mechanical, 2=525/0-3-8
Max Horz 2=319(load case 2)
Max Uplift4=-283(load case 2), 5=-92(load case 2), 2=-340(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-884/172, 3-4=-122/65
BOT CHORD 2-6=-396/822, 5-6=-396/822
WEBS 3-5=-854/411, 3-6=0/165

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 283 lb uplift at joint 4, 92 lb uplift at joint 5 and 340 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-2(F=26, B=26)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
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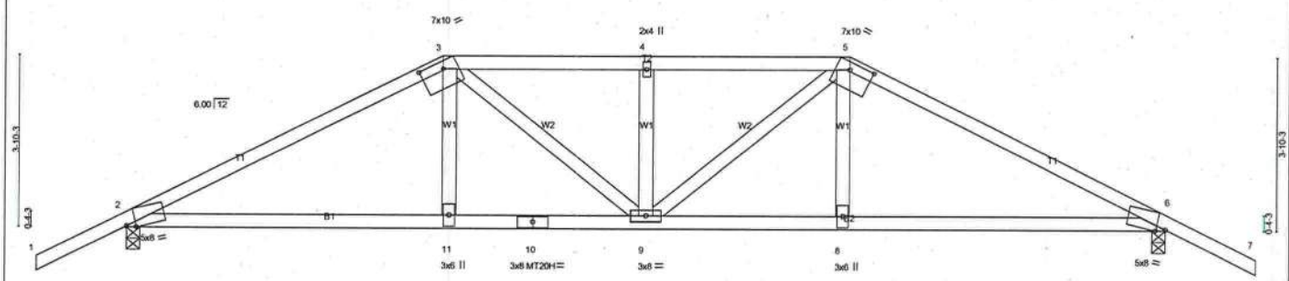
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705131
L130844	T01	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:20 2005 Page 1

-2-0-0	3-9-4	7-0-0	11-6-0	16-0-0	19-2-12	23-0-0	25-0-0
2-0-0	3-9-4	3-2-12	4-6-0	4-6-0	3-2-12	3-9-4	2-0-0

Scale = 1:44.8



3-9-4	7-0-0	11-6-0	16-0-0	19-2-12	23-0-0
3-9-4	3-2-12	4-6-0	4-6-0	3-2-12	3-9-4

Plate Offsets (X,Y): [2:0-2-7,Edge], [3:0-6-4,0-2-0], [5:0-6-4,0-2-0], [6:0-2-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.20	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.32	9-11	>855	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.12	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 108 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-11-0 oc bracing.

REACTIONS (lb/size) 2=2048/0-3-8, 6=2048/0-3-8
Max Horz 2=-136(load case 5)
Max Uplift2=-1111(load case 4), 6=-1111(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3719/1902, 3-4=-3743/2038, 4-5=-3743/2038, 5-6=-3719/1903, 6-7=0/47
BOT CHORD 2-11=-1646/3234, 10-11=-1658/3267, 9-10=-1658/3267, 8-9=-1609/3267, 6-8=-1597/3234
WEBS 3-11=-300/812, 3-9=-473/719, 4-9=-523/565, 5-9=-473/719, 5-8=-300/812

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1111 lb uplift at joint 2 and 1111 lb uplift at joint 6.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 348 lb up at 16-0-0, and 539 lb down and 348 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-118(F=-64), 5-7=-54, 2-11=-30, 8-11=-65(F=-35), 6-8=-30
Concentrated Loads (lb)
Vert: 11=-539(F) 8=-539(F)

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705132
L130844	T02	HIP	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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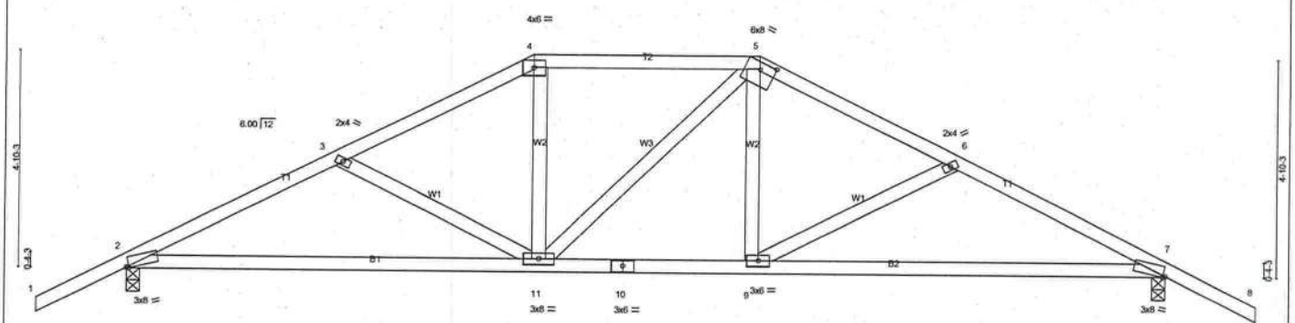
-2-0-0 | 4-9-4 | 9-0-0 | 14-0-0 | 18-2-12 | 23-0-0 | 25-0-0

2-0-0 | 4-9-4 | 4-2-12 | 5-0-0 | 4-2-12 | 4-9-4 | 2-0-0

Scale = 1:44.8



9-0-0 | 14-0-0 | 23-0-0

9-0-0 | 5-0-0 | 9-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [5:0-4-0,0-1-15], [7:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.18	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.31	7-9	>889	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.12	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 115 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-7-7 oc bracing.

REACTIONS (lb/size) 2=1070/0-3-8, 7=1070/0-3-8
Max Horz 2=-159(load case 6)
Max Uplift2=-502(load case 5), 7=-502(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1592/977, 3-4=-1340/807, 4-5=-1161/795, 5-6=-1340/807, 6-7=-1592/977, 7-8=0/47
BOT CHORD 2-11=-670/1381, 10-11=-387/1160, 9-10=-387/1160, 7-9=-670/1382
WEBS 3-11=-261/322, 4-11=-101/323, 5-11=-104/106, 5-9=-101/323, 6-9=-262/321

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 502 lb uplift at joint 2 and 502 lb uplift at joint 7.

LOAD CASE(S) Standard

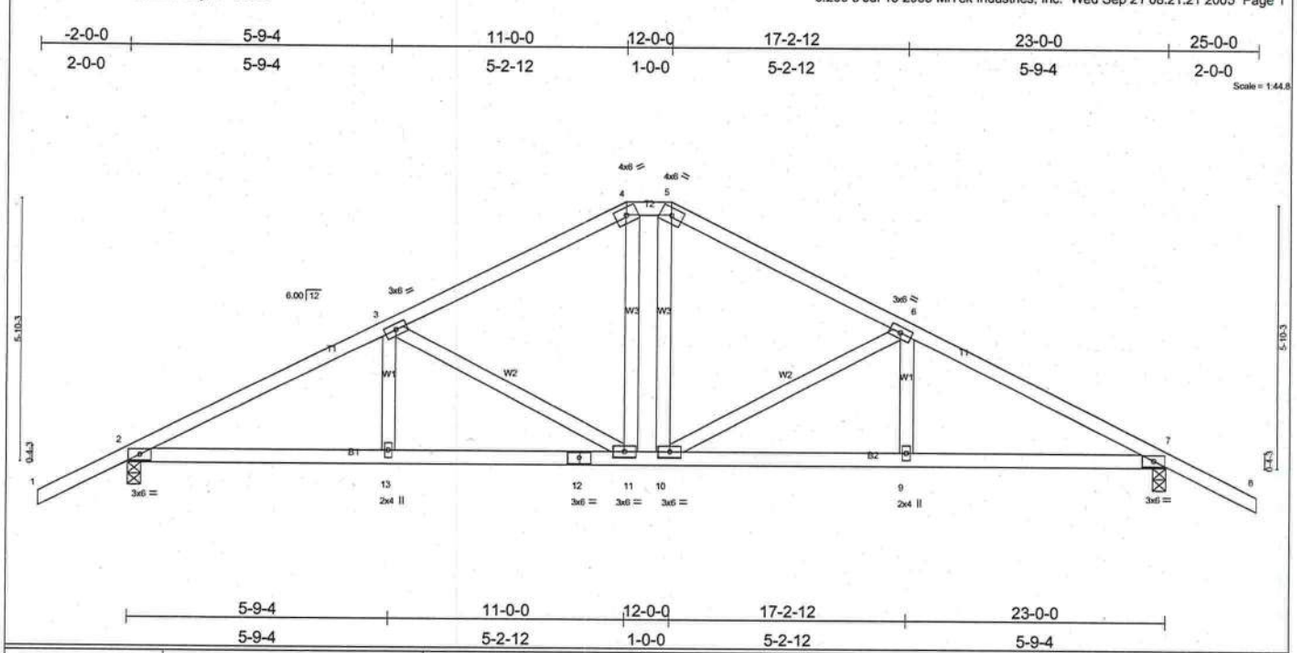
SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705133
L130844	T03	HIP	1	1	

Builders FirstSource, Lake City, Fl 32055

Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:21 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	0.08	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.13	11-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.34	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 120 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-8-15 oc bracing.

REACTIONS (lb/size) 2=1070/0-3-8, 7=1070/0-3-8
Max Horz 2=-183(load case 6)
Max Uplift2=-524(load case 5), 7=-524(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1654/982, 3-4=-1153/798, 4-5=-980/797, 5-6=-1153/798, 6-7=-1654/982, 7-8=0/47
BOT CHORD 2-13=-663/1408, 12-13=-663/1408, 11-12=-663/1408, 10-11=-319/980, 9-10=-663/1408, 7-9=-663/1408
WEBS 3-13=0/188, 3-11=-523/397, 4-11=-177/334, 5-10=-177/334, 6-10=-523/397, 6-9=0/188

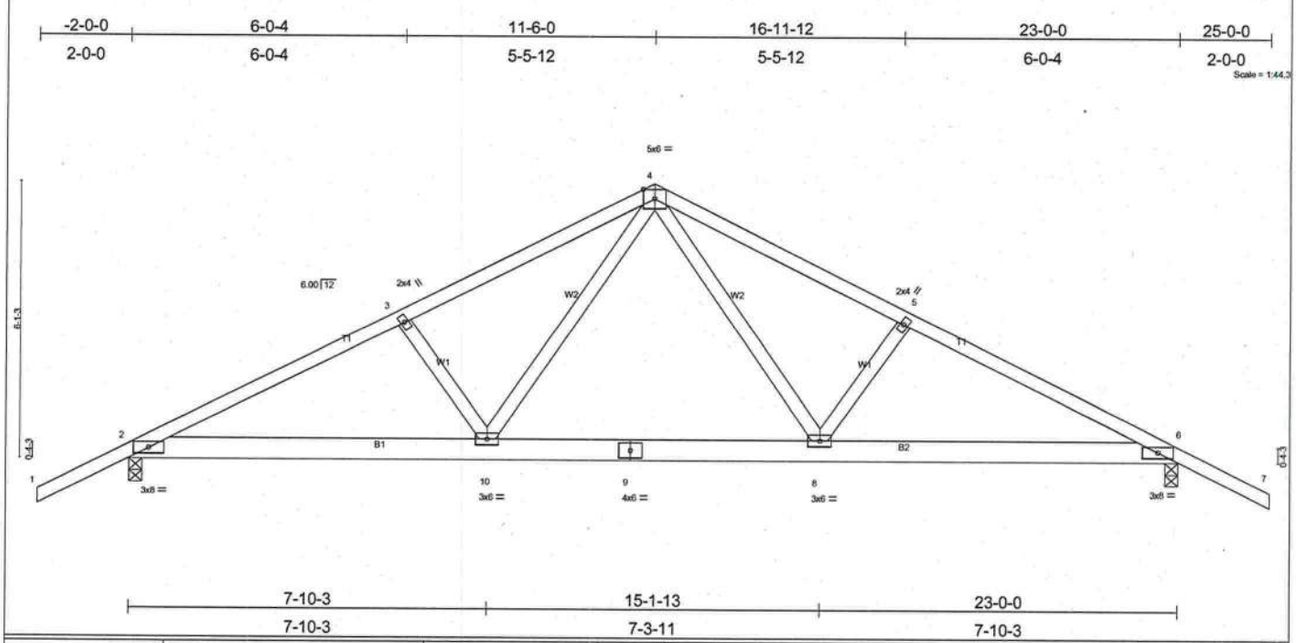
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 524 lb uplift at joint 2 and 524 lb uplift at joint 7.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705134
L130844	T04	COMMON	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:22 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	0.14	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.20	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.38	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 128 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-7-1 oc bracing.

REACTIONS (lb/size) 2=1252/0-3-8, 6=1252/0-3-8
Max Horz 2=191(load case 5)
Max Uplift2=-617(load case 5), 6=-617(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-2062/1305, 3-4=-1891/1283, 4-5=-1891/1283, 5-6=-2062/1305, 6-7=0/51
BOT CHORD 2-10=-949/1777, 9-10=-486/1209, 8-9=-486/1209, 6-8=-949/1777
WEBS 3-10=-279/382, 4-10=-470/799, 4-8=-470/799, 5-8=-279/382

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 617 lb uplift at joint 2 and 617 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705135
L130844	T05	HIP	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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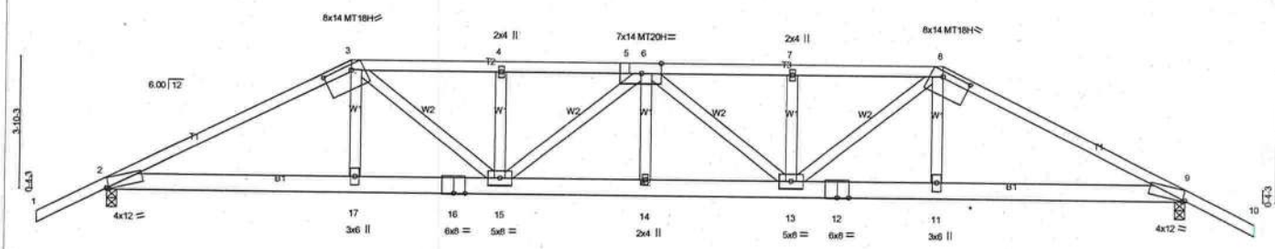
-2-0-0	3-9-4	7-0-0	11-3-14	15-6-0	19-8-2	24-0-0	27-2-12	31-0-0	33-0-0
2-0-0	3-9-4	3-2-12	4-3-14	4-2-2	4-2-2	4-3-14	3-2-12	3-9-4	2-0-0

Scale = 1:58.0



3-9-4	7-0-0	11-3-14	15-6-0	19-8-2	24-0-0	27-2-12	31-0-0
3-9-4	3-2-12	4-3-14	4-2-2	4-2-2	4-3-14	3-2-12	3-9-4

Plate Offsets (X,Y): [2:0-0-13,Edge], [3:0-9-12,0-1-12], [5:0-6-12,Edge], [8:0-9-12,0-1-12], [9:0-0-13,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.80	Vert(LL)	-0.42	14	>876	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.67	14	>547	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.60	Horz(TL)	0.15	9	n/a	n/a	MT18H	244/190
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 181 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-5-6 oc bracing.

REACTIONS (lb/size) 2=2780/0-3-8, 9=2780/0-3-8
Max Horz 2=-138(load case 5)
Max Uplift2=-1440(load case 4), 9=-1440(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-5382/2863, 3-4=-6141/3408, 4-5=-6141/3409, 5-6=-6141/3409, 6-7=-6141/3409, 7-8=-6141/3408, 8-9=-5382/2863, 9-10=0/51
BOT CHORD 2-17=-2508/4731, 16-17=-2521/4765, 15-16=-2521/4765, 14-15=-3567/6637, 13-14=-3567/6637, 12-13=-2474/4765, 11-12=-2474/4765, 9-11=-2461/4731
WEBS 3-17=-304/816, 3-15=-1111/1861, 4-15=-500/516, 6-15=-674/380, 6-14=0/317, 6-13=-674/380, 7-13=-500/516, 8-13=-1112/1861, 8-11=-304/816

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1440 lb uplift at joint 2 and 1440 lb uplift at joint 9.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 348 lb up at 24-0-0, and 539 lb down and 348 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-8=-118(F=-64), 8-10=-54, 2-17=-30, 11-17=-65(F=-35), 9-11=-30
Concentrated Loads (lb)
Vert: 17=-539(F) 11=-539(F)

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705136
L130844	T06	HIP	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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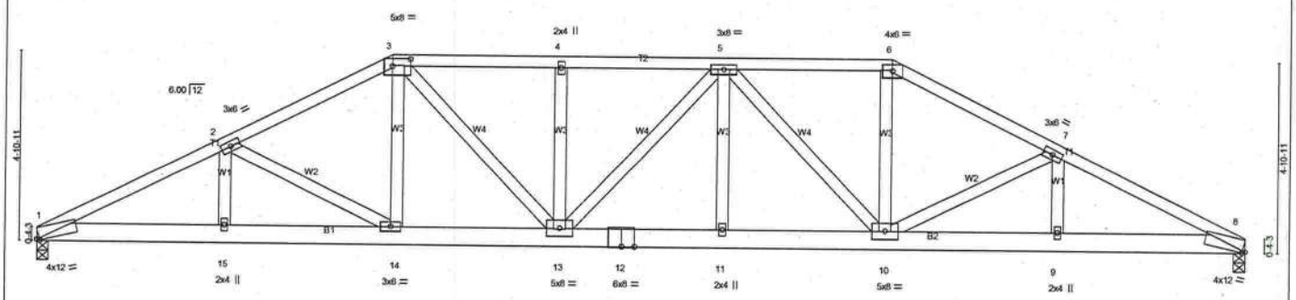
4-9-12	9-1-0	13-4-15	17-7-1	21-11-0	26-2-4	31-0-0
4-9-12	4-3-4	4-3-15	4-2-3	4-3-15	4-3-4	4-9-12

Scale = 1:51.9



4-9-12	9-1-0	13-4-15	17-7-1	21-11-0	26-2-4	31-0-0
4-9-12	4-3-4	4-3-15	4-2-3	4-3-15	4-3-4	4-9-12

Plate Offsets (X,Y): [1:0-0-13,Edge], [3:0-5-8,0-2-4], [8:0-0-13,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.31	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-0.49	11-13	>752	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.69	Horz(TL)	0.13	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 192 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-10-10 oc bracing.

REACTIONS (lb/size) 1=2808/0-3-8, 8=2808/0-3-8
Max Horz 1=107(load case 4)
Max Uplift1=-1162(load case 4), 8=-1162(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5359/2187, 2-3=-4514/1929, 3-4=-4719/2079, 4-5=-4719/2079, 5-6=-4048/1777, 6-7=-4513/1927, 7-8=-5359/2189
BOT CHORD 1-15=-1981/4731, 14-15=-1981/4731, 13-14=-1694/4026, 12-13=-1984/4723, 11-12=-1984/4723, 10-11=-1984/4723, 9-10=-1905/4731, 8-9=-1905/4731
WEBS 2-15=-188/557, 2-14=-841/501, 3-14=-442/1020, 3-13=-533/1085, 4-13=-220/196, 5-13=-75/62, 5-11=-171/520, 5-10=-1078/534, 6-10=-701/1744, 7-10=-842/505, 7-9=-191/559

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1162 lb uplift at joint 1 and 1162 lb uplift at joint 8.
5) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 31-0-0
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 6-8=-54, 1-8=-129(F=-99)

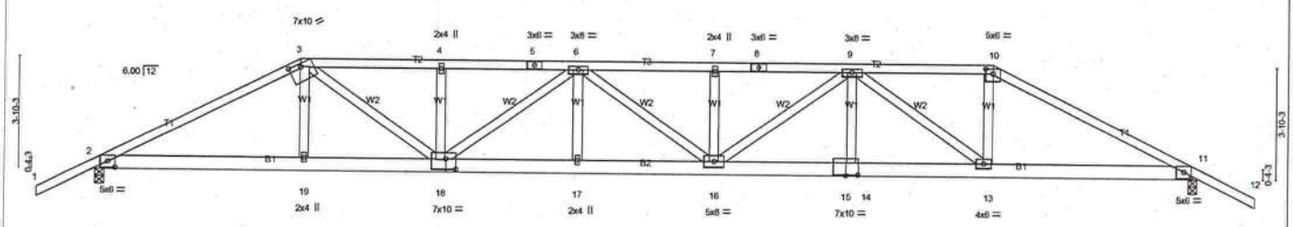
SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705137
L130844	T07	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:24 2005 Page 1

-2-0-0	3-9-4	7-0-0	11-9-14	16-5-15	21-2-1	25-10-2	30-8-0	33-10-12	37-8-0	39-8-0
2-0-0	3-9-4	3-2-12	4-9-14	4-8-2	4-8-2	4-8-2	4-9-14	3-2-12	3-9-4	2-0-0

Scale = 1:69.1



3-9-4	7-0-0	11-9-14	16-5-15	21-2-1	25-10-2	30-8-0	33-10-12	37-8-0
3-9-4	3-2-12	4-9-14	4-8-2	4-8-2	4-8-2	4-9-14	3-2-12	3-9-4

Plate Offsets (X,Y): [2:0-3-0,0-2-9], [3:0-5-0,0-1-7], [10:0-3-0,0-2-0], [11:0-3-0,0-2-9], [15:0-5-0,0-0-4], [18:0-4-4,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.39	16-17	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.63	16-17	>717	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.47	Horz(TL)	0.13	11	n/a	n/a		
BCDL	5.0	Code	FBC2004/TPI2002	(Matrix)							Weight: 442 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except* T1 2 X 4 SYP No.2, T1 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-0-2 oc bracing.

REACTIONS (lb/size) 2=3389/0-3-8, 11=3389/0-3-8
Max Horz 2=-138(load case 5)
Max Uplift2=-1737(load case 3), 11=-1737(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-6732/3639, 3-4=-8300/4647, 4-5=-8299/4647, 5-6=-8299/4647, 6-7=-9439/5270, 7-8=-9439/5270, 8-9=-9439/5270, 9-10=-6028/3342, 10-11=-6716/3629, 11-12=0/51
BOT CHORD 2-19=-3202/5936, 18-19=-3215/5971, 17-18=-5204/9485, 16-17=-5204/9485, 15-16=-4528/8335, 14-15=-4528/8335, 13-14=-4528/8335, 11-13=-3145/5922
WEBS 3-19=-300/827, 3-18=-1734/2958, 4-18=-561/577, 6-18=-1501/854, 6-17=0/350, 6-16=-97/66, 7-16=-535/534, 9-16=-784/1401, 9-14=0/345, 9-13=-2963/1745, 10-13=-1287/2543

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1737 lb uplift at joint 2 and 1737 lb uplift at joint 11.
7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 348 lb up at 30-8-0, and 539 lb down and 348 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-10=-118(F=-64), 10-12=-54, 2-19=-30, 13-19=-65(F=-35), 11-13=-30
Concentrated Loads (lb)
Vert: 19=-539(F) 13=-539(F)

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

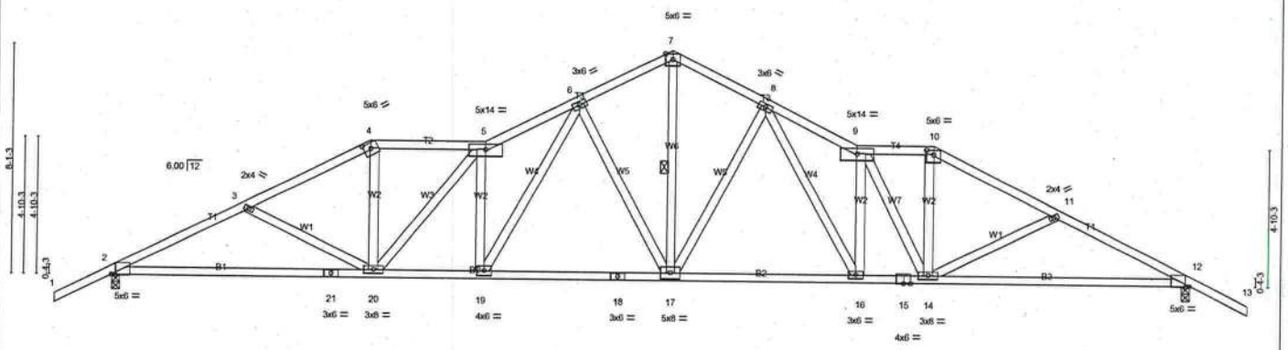
Job	Truss	Truss Type	Qty	Ply	Dwg.#092705138
L130844	T08	SPECIAL	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:25 2005 Page 1

-2-0-0	4-9-4	9-0-0	13-0-0	19-6-0	26-0-0	28-8-0	32-10-12	37-8-0	39-8-0
2-0-0	4-9-4	4-2-12	4-0-0	6-6-0	6-6-0	2-8-0	4-2-12	4-9-4	2-0-0

Scale = 1:70.6



9-0-0	13-0-0	19-6-0	26-0-0	28-8-0	37-8-0
9-0-0	4-0-0	6-6-0	6-6-0	2-8-0	9-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [4:0-3-0,0-2-7], [10:0-3-0,0-2-0], [12:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.32	17-19	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.51	17-19	>880	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.15	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 231 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-1-8 oc bracing.
WEBS 1 Row at midpt 7-17

REACTIONS (lb/size) 2=1686/0-3-8, 12=1686/0-3-8
Max Horz 2=236(load case 5)
Max Uplift2=-785(load case 5), 12=-769(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2911/1807, 3-4=-2684/1652, 4-5=-2383/1564, 5-6=-3294/2209, 6-7=-2151/1526, 7-8=-2152/1527, 8-9=-3097/2097, 9-10=-2382/1574, 10-11=-2684/1663, 11-12=-2913/1819, 12-13=0/47
BOT CHORD 2-21=-1403/2546, 20-21=-1403/2546, 19-20=-1490/2892, 18-19=-1116/2308, 17-18=-1116/2308, 16-17=-1084/2250, 15-16=-1397/2725, 14-15=-1397/2725, 12-14=-1414/2549
WEBS 3-20=-230/295, 4-20=-475/927, 5-20=-788/517, 5-19=-1018/745, 6-17=-886/708, 7-17=-1152/1694, 8-17=-762/639, 9-16=-851/615, 9-14=-708/478, 10-14=-496/950, 11-14=-237/298, 6-19=-825/1278, 8-16=-690/1041

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 785 lb uplift at joint 2 and 769 lb uplift at joint 12.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

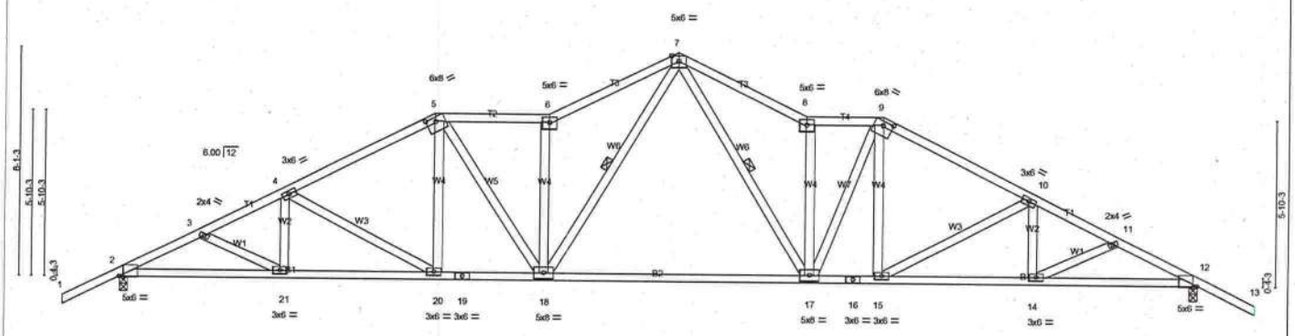
Job	Truss	Truss Type	Qty	Ply	Dwg.#092705139
L130844	T09	SPECIAL	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:26 2005 Page 1

-2-0-0	5-9-4	11-0-0	15-0-0	19-6-0	24-0-0	26-8-0	31-10-12	37-8-0	39-8-0
2-0-0	5-9-4	5-2-12	4-0-0	4-6-0	4-6-0	2-8-0	5-2-12	5-9-4	2-0-0

Scale = 1:70.6



5-9-4	11-0-0	15-0-0	24-0-0	26-8-0	31-10-12	37-8-0
5-9-4	5-2-12	4-0-0	9-0-0	2-8-0	5-2-12	5-9-4

Plate Offsets (X,Y): [2:0-1-11,Edge], [5:0-4-0,0-1-15], [9:0-4-0,0-1-15], [12:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.40	17-18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.66	17-18	>684	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.80	Horz(TL)	0.14	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 234 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-4-8 oc bracing.
WEBS 1 Row at midpt 7-18, 7-17

REACTIONS (lb/size) 2=1686/0-3-8, 12=1686/0-3-8
Max Horz 2=236(load case 5)
Max Uplift2=-785(load case 5), 12=-769(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2937/1745, 3-4=-2885/1752, 4-5=-2496/1594, 5-6=-2531/1692, 6-7=-2897/2000, 7-8=-2787/1938, 8-9=-2437/1640, 9-10=-2496/1604, 10-11=-2885/1761, 11-12=-2936/1754, 12-13=0/47
BOT CHORD 2-21=-1350/2536, 20-21=-1343/2571, 19-20=-1034/2187, 18-19=-1034/2187, 17-18=-794/1866, 16-17=-1043/2184, 15-16=-1043/2184, 14-15=-1351/2572, 12-14=-1357/2535
WEBS 4-21=0/162, 4-20=-457/357, 5-20=-167/312, 5-18=-316/603, 6-18=-1444/1062, 7-18=-874/1371, 7-17=-769/1182, 8-17=-1333/981, 9-17=-289/612, 9-15=-180/267, 10-15=-462/356, 10-14=0/163, 3-21=-24/51, 11-14=-27/54

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 785 lb uplift at joint 2 and 769 lb uplift at joint 12.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705140
L130844	T10	SPECIAL	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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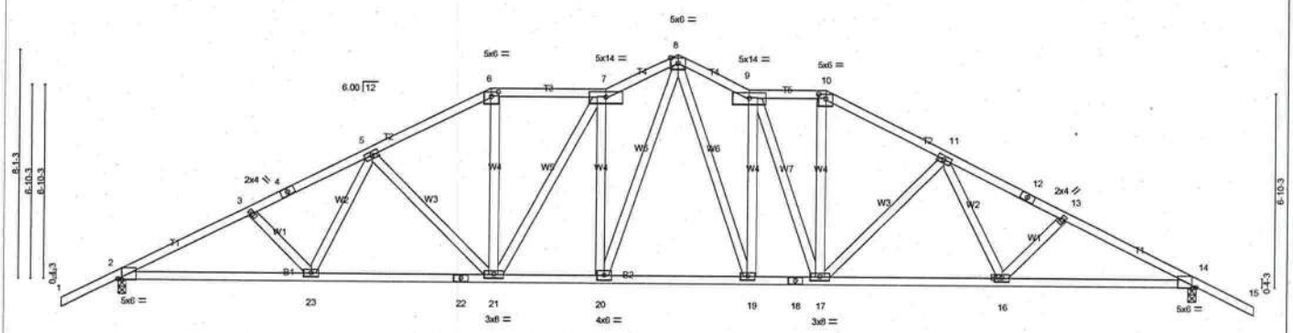
-2-0-0	4-8-5	8-10-3	13-0-0	17-0-0	19-6-0	22-0-0	24-8-0	28-9-13	32-11-11	37-8-0	39-8-0
2-0-0	4-8-5	4-1-13	4-1-13	4-0-0	2-6-0	2-6-0	2-8-0	4-1-13	4-1-13	4-8-5	2-0-0

Scale = 1:70.6



6-9-4	13-0-0	17-0-0	22-0-0	24-8-0	30-10-12	37-8-0
6-9-4	6-2-12	4-0-0	5-0-0	2-8-0	6-2-12	6-9-4

Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-3-0,0-2-0], [10:0-3-0,0-2-0], [14:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.25	19-20	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.40	19-20	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.14	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 246 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-3-13 oc bracing.

REACTIONS (lb/size) 2=1686/0-3-8, 14=1686/0-3-8
Max Horz 2=-236(load case 6)
Max Uplift2=-785(load case 5), 14=-769(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2972/1794, 3-4=-2816/1731, 4-5=-2765/1742, 5-6=-2306/1531, 6-7=-2037/1442, 7-8=-2485/1767, 8-9=-2424/1729, 9-10=-2035/1446, 10-11=-2305/1536, 11-12=-2766/1747, 12-13=-2817/1736, 13-14=-2972/1799, 14-15=0/47
BOT CHORD 2-23=-1395/2583, 22-23=-1169/2324, 21-22=-1169/2324, 20-21=-991/2205, 19-20=-770/1873, 18-19=-965/2156, 17-18=-965/2156, 16-17=-1174/2325, 14-16=-1399/2583
WEBS 3-23=-169/248, 5-23=-134/362, 5-21=-444/409, 6-21=-435/753, 7-21=-366/197, 7-20=-909/728, 8-20=-737/1097, 8-19=-649/934, 9-19=-776/637, 9-17=-377/199, 10-17=-449/766, 11-17=-450/412, 11-16=-134/363, 13-16=-169/247

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 785 lb uplift at joint 2 and 769 lb uplift at joint 14.

LOAD CASE(S) Standard

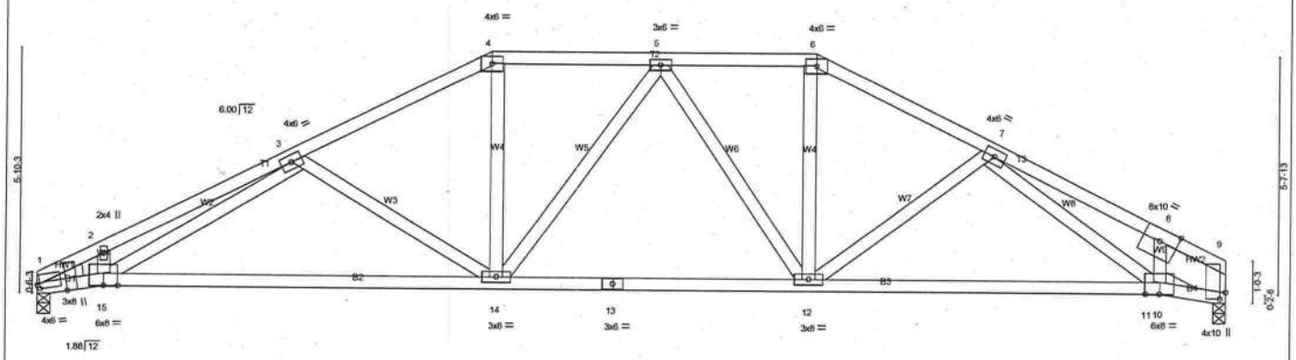
SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705141
L130844	T11	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:27 2005 Page 1

1-6-11 | 6-1-6 | 10-8-0 | 18-4-0 | 22-4-10 | 26-5-5 | 28-0-0
1-6-11 | 4-6-10 | 4-6-10 | 7-8-0 | 4-0-10 | 4-0-10 | 1-6-11

Scale: 1/4"=1'



1-6-11 | 10-8-0 | 18-4-0 | 26-5-5 | 28-0-0
1-6-11 | 9-1-5 | 7-8-0 | 8-1-5 | 1-6-11

Plate Offsets (X,Y): [1:0-2-12,Edge], [1:0-0-10,0-1-2], [9:0-1-14,0-1-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.24	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.40	14-15	>838	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.10	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 161 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3
SLIDER Right 2 X 8 SYP No.1D 1-9-5

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-10-4 oc bracing.

REACTIONS (lb/size) 9=1160/0-3-8, 1=1168/0-3-8
Max Horz 1=-133(load case 6)
Max Uplift9=-410(load case 6), 1=-422(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2640/1391, 2-3=-2384/1408, 3-4=-1704/1079, 4-5=-1483/1041, 5-6=-1426/1003, 6-7=-1630/1043, 7-8=-1576/1080, 8-9=-1866/1151
BOT CHORD 1-15=-1118/2155, 14-15=-1016/1782, 13-14=-682/1516, 12-13=-682/1516, 11-12=-856/1542, 9-10=-792/1334
WEBS 2-15=-11/377, 3-15=-136/416, 3-14=-371/434, 4-14=-226/496, 5-12=-258/178, 6-12=-233/483, 7-12=-176/309, 7-11=-229/106, 10-11=-782/1318, 8-11=-148/457, 5-14=-172/149

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 9, 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 410 lb uplift at joint 9 and 422 lb uplift at joint 1.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

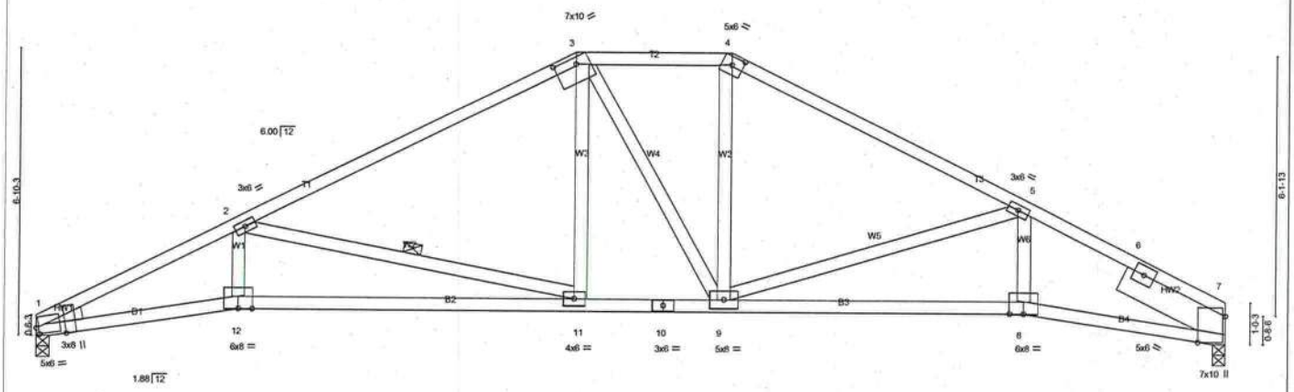
Job	Truss	Truss Type	Qty	Ply	Dwg.#092705142
L130844	T12	SPECIAL	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:28 2005 Page 1

4-9-2	12-8-0	16-4-0	23-2-14	28-0-0
4-9-2	7-10-14	3-8-0	6-10-14	4-9-2

Scale: 1/4"=1'



4-9-2	12-8-0	16-4-0	23-2-14	28-0-0
4-9-2	7-10-14	3-8-0	6-10-14	4-9-2

Plate Offsets (X,Y): [1:0-2-12,Edge], [1:0-0-10,0-1-14], [3:0-6-4,0-2-0], [4:0-3-0,0-2-7], [7:0-7-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.80	Vert(LL)	-0.27	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.44	11-12	>762	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.17	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 152 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except* T3 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3
SLIDER Right 2 X 8 SYP No.1D 2-9-5

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-0-7 oc bracing.
WEBS 1 Row at midpt 2-11

REACTIONS (lb/size) 7=1168/0-3-8, 1=1168/0-3-8
Max Horz 1=-157(load case 6)
Max Uplift7=-433(load case 6), 1=-445(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2940/1828, 2-3=-1689/1086, 3-4=-1407/1073, 4-5=-1616/1070, 5-6=-2271/1446, 6-7=-2342/1424
BOT CHORD 1-12=-1540/2580, 11-12=-1521/2535, 10-11=-641/1437, 9-10=-641/1437, 8-9=-1101/1920, 7-8=-1110/1948
WEBS 2-12=-184/599, 2-11=-1140/908, 3-11=-174/444, 3-9=-221/160, 4-9=-191/428, 5-9=-561/516, 5-8=-86/424

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 7, 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 433 lb uplift at joint 7 and 445 lb uplift at joint 1.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705143
L130844	T13	SPECIAL	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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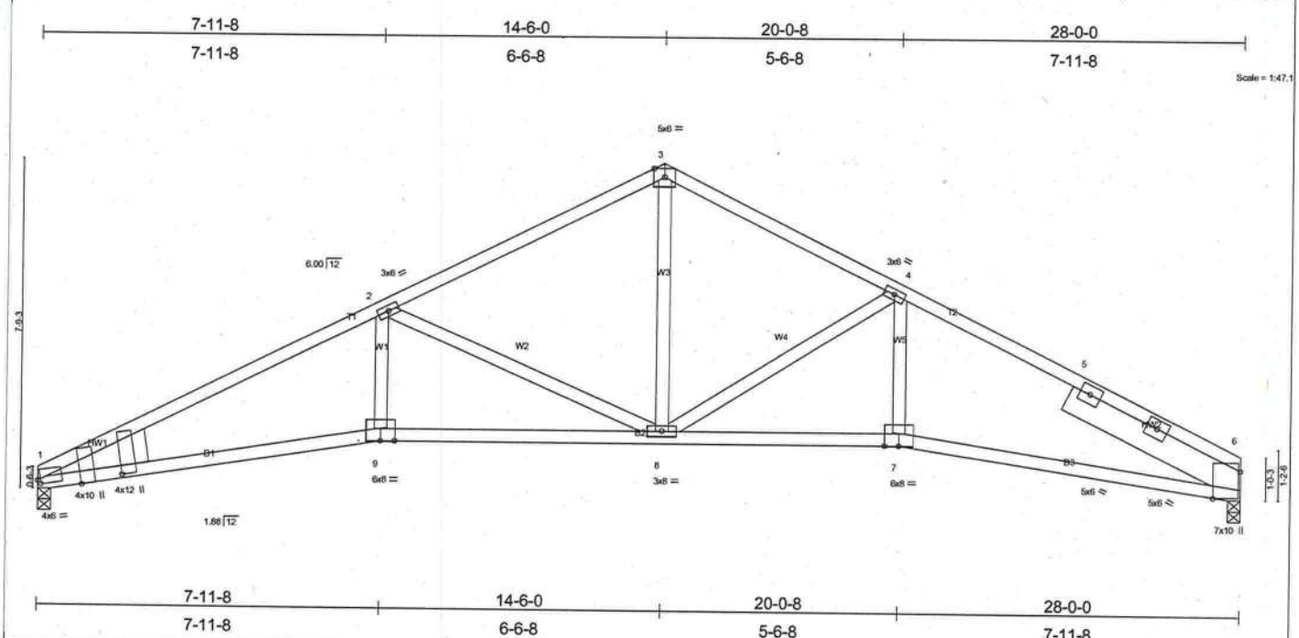

Plate Offsets (X,Y): [1:0-1-12,1-11-8], [1:0-2-12,Edge], [1:0-0-10,0-1-2], [6:0-7-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	0.26	1-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-0.42	1-9	>804	180		
BCLL	10.0	Rep Stress Incr	YES	WB	1.00	Horz(TL)	0.19	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 150 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 10 SYP No.2
SLIDER Right 2 X 8 SYP No.1D 4-6-12

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-9-11 oc bracing.

REACTIONS (lb/size) 6=1168/0-3-8, 1=1168/0-3-8
Max Horz 1=-178(load case 6)
Max Uplift6=-451(load case 6), 1=-462(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2748/1725, 2-3=-1580/1113, 3-4=-1535/1117, 4-5=-2222/1466, 5-6=-2337/1441
BOT CHORD 1-9=-1391/2393, 8-9=-1367/2341, 7-8=-1072/1921, 6-7=-1090/1960
WEBS 2-9=-209/626, 2-8=-1104/879, 3-8=-638/992, 4-8=-706/597, 4-7=-136/483

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 6, 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 451 lb uplift at joint 6 and 462 lb uplift at joint 1.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
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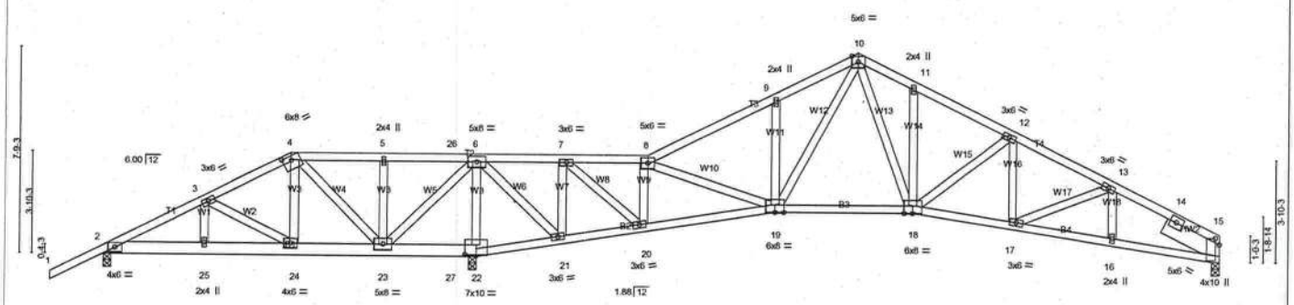
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705144
L130844	T14	SPECIAL	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:30 2005 Page 1

-2-0-0	3-9-4	7-0-0	10-5-12	13-11-8	17-1-12	20-4-0	25-1-2	28-2-0	30-2-14	33-11-7	37-7-15	41-8-0
2-0-0	3-9-4	3-2-12	3-5-12	3-5-12	3-2-4	3-2-4	4-9-2	3-0-14	2-0-14	3-8-9	3-8-9	4-0-1

Scale = 1:75.4



13-11-8

3-9-4	7-0-0	10-5-12	13-0-0	17-1-12	20-4-0	25-1-2	30-2-14	33-11-7	37-7-15	41-8-0
3-9-4	3-2-12	3-5-12	2-6-4	0-11-8 3-2-4	3-2-4	4-9-2	5-1-13	3-8-9	3-8-9	4-0-1

Plate Offsets (X,Y): [4:0-4-0,0-1-15], [15:0-2-10,0-1-10], [22:0-5-0,0-0-13]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.05	18-19	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.09	18-19	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.45	Horz(TL)	0.03	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 525 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 8 SYP No.1D 2-4-4

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 15=908/0-3-8, 2=783/0-3-8, 22=5243/0-3-8
Max Horz 2=251(load case 4)
Max Uplift15=-368(load case 5), 2=-781(load case 4), 22=-3076(load case 4)
Max Grav 15=908(load case 1), 2=858(load case 6), 22=5243(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-1272/1045, 3-4=-989/960, 4-5=-343/737, 5-26=-342/737, 6-26=-342/737, 6-7=-487/980, 7-8=-185/78, 8-9=-1040/307, 9-10=-1024/417, 10-11=-1203/543, 11-12=-1240/460, 12-13=-1535/597, 13-14=-1589/638, 14-15=-1655/628
BOT CHORD 2-25=-1017/1090, 24-25=-1017/1090, 23-24=-873/885, 23-27=-1843/799, 22-27=-1870/815, 21-22=-1897/825, 20-21=-991/629, 19-20=-56/289, 18-19=-101/807, 17-18=-339/1388, 16-17=-444/1348, 15-16=-441/1337
WEBS 3-25=-93/128, 3-24=-274/228, 4-24=-436/662, 4-23=-867/484, 5-23=-398/417, 6-23=-1805/2785, 6-22=-3077/1802, 6-21=-410/1205, 7-21=-1049/454, 7-20=-553/1438, 8-20=-946/424, 8-19=-244/757, 9-19=-225/257, 10-19=-140/231, 10-18=-396/739, 11-18=-130/167, 12-18=-396/298, 12-17=-43/125, 13-17=-4/111, 13-16=0/75

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-4-0 oc, 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Bearing at joint(s) 15 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 368 lb uplift at joint 15, 781 lb uplift at joint 2 and 3076 lb uplift at joint 22.
8) Girder carries hip end with 28-8-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2207 lb down and 1424 lb up at 13-0-0, and 539 lb down and 348 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-26=-118(F=-64), 8-26=-54, 8-10=-54, 10-15=-54, 2-24=-30, 24-27=-65(F=-35), 22-27=-30, 19-22=-30, 18-19=-30, 15-18=-30
Concentrated Loads (lb)
Vert: 24=-539(F) 27=-2207(F)

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705145
L130844	T15	SPECIAL	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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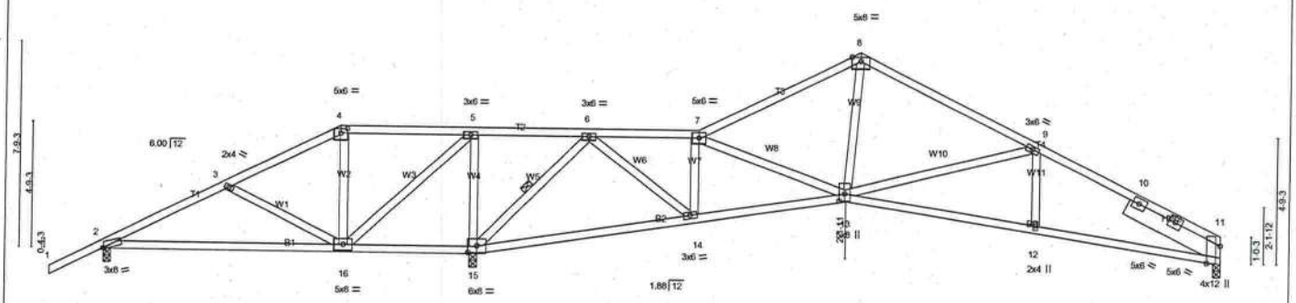
-2-0-0	4-8-4	8-10-0	13-11-8	18-0-12	22-2-0	28-2-0	34-9-4	41-8-0
2-0-0	4-8-4	4-1-12	5-1-8	4-1-4	4-1-4	6-0-0	6-7-4	6-10-12

Scale = 1:75.4



8-10-0	13-11-8	22-2-0	27-7-8	27-8-8	34-9-4	41-8-0
8-10-0	5-1-8	8-2-8	5-5-8	0-1-0	7-0-12	6-10-12

Plate Offsets (X,Y): [2:0-0-10,Edge], [4:0-3-0,0-2-0], [11:0-7-13,Edge], [13:0-3-2,0-2-8], [15:0-4-0,0-2-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	0.27	2-16	>617	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	0.21	2-16	>782	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.07	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 226 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
SLIDER	Right 2 X 8 SYP No.1D 3-11-10

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-0-10 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 4-11-9 oc bracing.
WEBS	1 Row at midpt 6-15

REACTIONS (lb/size) 11=929/0-3-8, 2=212/0-3-8, 15=2460/0-3-8
Max Horz 2=250(load case 5)
Max Uplift11=-400(load case 6), 2=-432(load case 5), 15=-1242(load case 5)
Max Grav 11=929(load case 1), 2=296(load case 7), 15=2460(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/47, 2-3=0/387, 3-4=0/620, 4-5=0/527, 5-6=-692/1428, 6-7=-710/545, 7-8=-1173/790, 8-9=-1114/756, 9-10=-1689/1112, 10-11=-1790/1087
BOT CHORD	2-16=-312/27, 15-16=-1428/1005, 14-15=-413/389, 13-14=-269/770, 12-13=-799/1490, 11-12=-799/1488
WEBS	3-16=-316/425, 4-16=-487/212, 5-16=-1095/1228, 5-15=-1109/980, 6-15=-1508/906, 6-14=-789/1415, 7-14=-860/647, 7-13=-32/339, 8-13=-267/577, 9-13=-595/542, 9-12=0/203

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 400 lb uplift at joint 11, 432 lb uplift at joint 2 and 1242 lb uplift at joint 15.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705146
L130844	T16	SPECIAL	1	1	

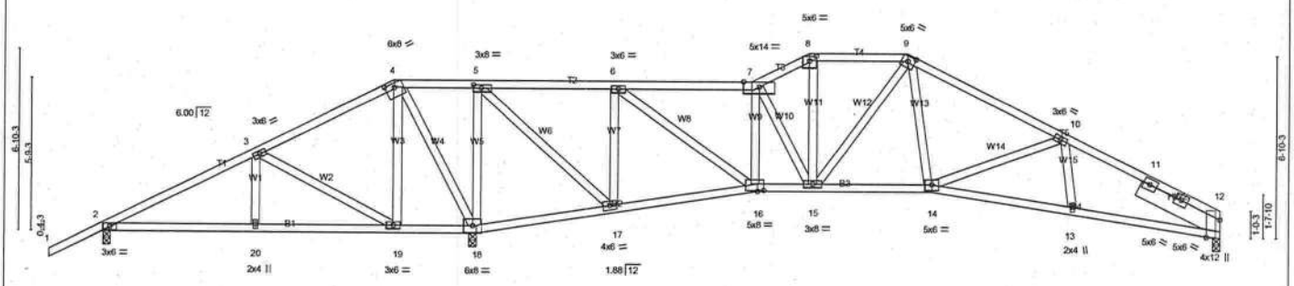
Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:31 2005 Page 1

-2-0-0 | 5-8-4 | 10-10-0 | 13-11-8 | 19-0-12 | 24-2-0 | 26-4-0 | 30-0-0 | 35-8-4 | 41-8-0

2-0-0 | 5-8-4 | 5-1-12 | 3-1-8 | 5-1-4 | 5-1-4 | 2-2-0 | 3-8-0 | 5-8-4 | 5-11-12

Scale = 1:75.4



5-8-4 | 10-10-0 | 13-11-8 | 19-0-12 | 24-5-2 | 26-4-0 | 30-10-14 | 36-1-11 | 41-8-0

5-8-4 | 5-1-12 | 3-1-8 | 5-1-4 | 5-4-6 | 1-10-14 | 4-6-14 | 5-2-13 | 5-6-5

Plate Offsets (X,Y): [4:0-4-0,0-1-15], [5:0-3-8,0-1-8], [8:0-3-0,0-2-0], [9:0-3-0,0-2-7], [12:0-7-13,Edge], [16:0-2-12,0-0-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.12	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.19	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.06	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 250 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 8 SYP No.1D 3-5-6

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-5-5 oc bracing.

REACTIONS (lb/size) 12=897/0-3-8, 2=175/0-3-8, 18=2527/0-3-8
Max Horz 2=228(load case 5)
Max Uplift12=-368(load case 6), 2=-420(load case 5), 18=-1253(load case 5)
Max Grav 12=897(load case 1), 2=253(load case 7), 18=2527(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=0/444, 3-4=-238/921, 4-5=-604/1307, 5-6=-45/212, 6-7=-824/527, 7-8=-933/616, 8-9=-810/578, 9-10=-1271/767, 10-11=-1629/993, 11-12=-1718/969
BOT CHORD 2-20=-375/123, 19-20=-375/123, 18-19=-764/489, 17-18=-1335/1002, 16-17=-228/417, 15-16=-162/780, 14-15=-277/1004, 13-14=-715/1409, 12-13=-708/1419
WEBS 3-20=-246/187, 3-19=-549/746, 4-19=-539/361, 4-18=-1094/1001, 5-18=-1239/704, 5-17=-781/1504, 6-17=-1026/659, 6-16=-682/1256, 7-16=-646/485, 7-15=-35/155, 8-15=-196/311, 9-15=-387/267, 9-14=-243/505, 10-14=-326/413, 10-13=0/137

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 368 lb uplift at joint 12, 420 lb uplift at joint 2 and 1253 lb uplift at joint 18.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
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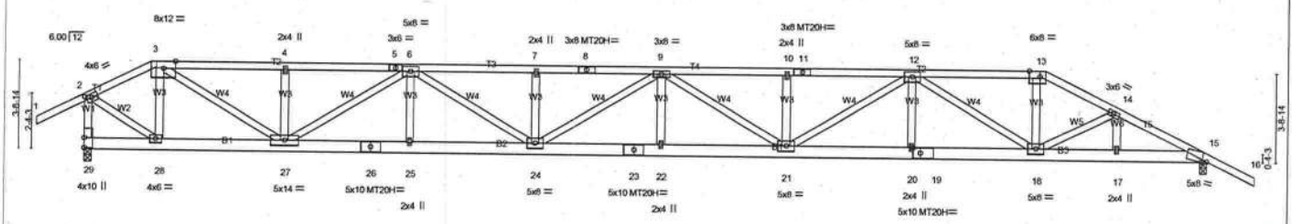
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705147
L130844	T17	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:32 2005 Page 1

-2-0-0	3-0-0	8-4-11	13-7-10	18-10-9	24-1-7	29-4-6	34-7-5	40-0-0	43-2-12	47-0-0	49-0-0
2-0-0	3-0-0	5-4-11	5-2-15	5-2-15	5-2-15	5-2-15	5-2-15	5-4-11	3-2-12	3-9-4	2-0-0

Scale = 1:84.6



3-0-0	8-4-11	13-7-10	18-10-9	24-1-7	29-4-6	34-7-5	40-0-0	43-2-12	47-0-0
3-0-0	5-4-11	5-2-15	5-2-15	5-2-15	5-2-15	5-2-15	5-4-11	3-2-12	3-9-4

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [13:0-5-6,Edge], [15:0-2-7,Edge], [19:0-3-11,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.82	Vert(LL)	0.96	22-24	>584	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-1.50	22-24	>374	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.99	Horz(TL)	0.22	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 598 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-2-13 oc bracing.

REACTIONS (lb/size) 15=4213/0-3-8, 29=4426/0-3-8
Max Horz 29=-141(load case 5)
Max Uplift15=-2221(load case 2), 29=-2373(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/52, 2-3=-3993/2245, 3-4=-8694/4944, 4-5=-8691/4940, 5-6=-8691/4940, 6-7=-14371/8144, 7-8=-14371/8144, 8-9=-14371/8144, 9-10=-14115/7997, 10-11=-14115/7997, 11-12=-14115/7997, 12-13=-7861/4426, 13-14=-8573/4782, 14-15=-8365/4541, 15-16=0/51, 2-29=-4286/2322
BOT CHORD 28-29=-10/167, 27-28=-1915/3498, 26-27=-6926/12372, 25-26=-6926/12372, 24-25=-6926/12372, 23-24=-8426/15094, 22-23=-8426/15094, 21-22=-8426/15094, 20-21=-6568/11833, 19-20=-6568/11833, 18-19=-6568/11833, 17-18=-3973/7400, 15-17=-3973/7400
WEBS 3-28=-1908/1244, 3-27=-3508/6148, 4-27=-704/682, 6-27=-4375/2496, 6-25=0/446, 6-24=-1348/2378, 7-24=-589/599, 9-24=-861/495, 9-22=0/446, 9-21=-1195/701, 10-21=-586/592, 12-21=-1569/2743, 12-20=0/370, 12-18=-4760/2740, 13-18=-1718/3323, 14-18=-376/472, 14-17=-204/177, 2-28=-2386/4289

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2221 lb uplift at joint 15 and 2373 lb uplift at joint 29.
8) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 3-0-0
9) Girder carries hip end with 7-0-0 right side setback, 3-0-0 left side setback, and 7-0-0 end setback.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 348 lb up at 40-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 3-13=-118(F=-64), 13-16=-54, 28-29=-129(F=-99), 18-28=-65(F=-35), 15-18=-30
Concentrated Loads (lb)
Vert: 18=-539(F)

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

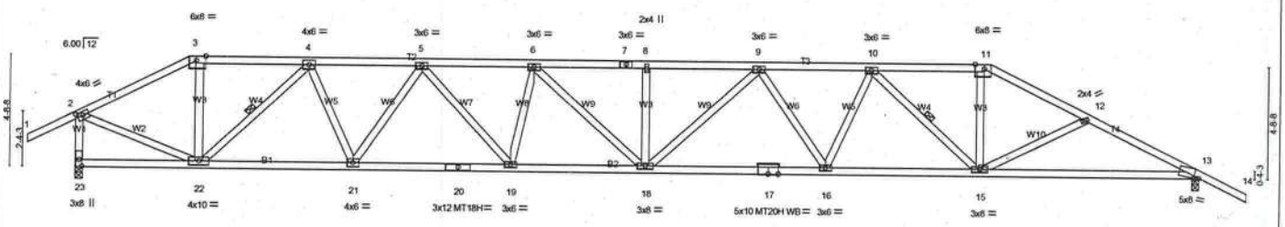
Job	Truss	Truss Type	Qty	Ply	Dwg.#092705148
L130844	T18	HIP	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:33 2005 Page 1

-2-0-0	5-0-0	9-8-9	14-5-2	19-1-11	23-10-5	28-6-14	33-3-7	38-0-0	42-2-12	47-0-0	49-0-0
2-0-0	5-0-0	4-8-9	4-8-9	4-8-9	4-8-9	4-8-9	4-8-9	4-8-9	4-2-12	4-9-4	2-0-0

Scale = 1:84.6



5-0-0	11-7-3	18-2-6	24-9-10	31-4-13	38-0-0	47-0-0
5-0-0	6-7-3	6-7-3	6-7-3	6-7-3	6-7-3	9-0-0

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [3:0-4-10,Edge], [11:0-4-10,Edge], [13:0-2-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.68	16-18	>829	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	-1.09	16-18	>514	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.27	13	n/a	n/a	MT18H	244/190
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 264 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* B3 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 4-22, 10-15

REACTIONS (lb/size) 23=2078/0-3-8, 13=2078/0-3-8
Max Horz 23=-160(load case 6)
Max Uplift23=-847(load case 4), 13=-798(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/52, 2-3=-2219/1210, 3-4=-1965/1161, 4-5=-3823/2072, 5-6=-4906/2628, 6-7=-5200/2782, 7-8=-5200/2782, 8-9=-5200/2782, 9-10=-4576/2467, 10-11=-3214/1801, 11-12=-3571/1903, 12-13=-3749/2038, 13-14=0/47, 2-23=-2005/1246
BOT CHORD 22-23=-56/182, 21-22=-1485/3364, 20-21=-1994/4381, 19-20=-1994/4381, 18-19=-2294/5000, 17-18=-2278/4913, 16-17=-2278/4913, 15-16=-1943/4264, 13-15=-1608/3286
WEBS 3-22=-293/772, 4-22=-1972/996, 4-21=-500/1175, 5-21=-1040/596, 5-19=-361/820, 6-19=-461/315, 6-18=-120/286, 8-18=-252/222, 9-18=-194/441, 9-16=-661/418, 10-16=-341/823, 10-15=-1516/797, 11-15=-585/1321, 12-15=-133/257, 2-22=-940/2067

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 847 lb uplift at joint 23 and 798 lb uplift at joint 13.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

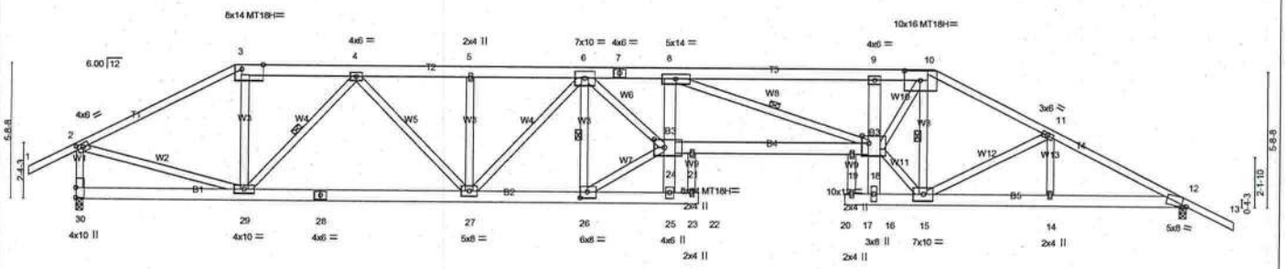
Job	Truss	Truss Type	Qty	Ply	Dwg.#092705149
L130844	T19	SPECIAL	1	1	

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:34 2005 Page 1

-2-0-0	7-0-0	11-10-2	16-8-4	21-6-6	26-4-8	32-6-8	36-0-0	41-2-12	47-0-0	49-0-0
2-0-0	7-0-0	4-10-2	4-10-2	4-10-2	4-10-2	6-2-0	3-5-8	5-2-12	5-9-4	2-0-0

Scale = 1:65.5



7-0-0	16-8-4	26-4-8	32-6-8	36-0-0	41-2-12	47-0-0
7-0-0	9-8-4	9-8-4	6-2-0	3-5-8	5-2-12	5-9-4

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [3:0-10-10,Edge], [10:0-8-0,0-4-15], [12:0-2-7,Edge], [18:0-3-8,0-3-12], [24:0-5-4,0-4-0], [26:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.70	19-21	>798	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.97	Vert(TL)	-1.13	19-21	>497	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.90	Horz(TL)	0.40	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 363 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T2 2 X 6 SYP No.1D, T3 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W7 2 X 4 SYP No.2, W6 2 X 4 SYP No.2, W11 2 X 4 SYP No.2
W10 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-7-2 oc bracing.
WEBS 1 Row at midpt 4-29, 6-26, 8-18, 10-15

REACTIONS (lb/size) 30=2084/0-3-8, 12=2089/0-3-8
Max Horz 30=-245(load case 6)
Max Uplift30=-792(load case 4), 12=-760(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/52, 2-3=-2537/1365, 3-4=-2232/1333, 4-5=-4027/2220, 5-6=-4027/2220, 6-7=-6985/3550, 7-8=-6985/3550, 8-9=-6069/3120, 9-10=-5603/2892, 10-11=-3494/1927, 11-12=-3887/2052, 12-13=0/51, 2-30=-1984/1268
BOT CHORD 29-30=-101/201, 28-29=-1394/3242, 27-28=-1394/3242, 26-27=-1940/4359, 25-26=-267/494, 23-25=0/0, 22-23=0/0, 24-25=-181/225, 8-24=-7/276, 21-24=-3265/7336, 19-21=-3265/7336, 18-19=-3265/7336, 16-18=-173/184, 9-18=-424/343, 17-20=0/0, 16-17=0/0, 15-16=-238/431, 14-15=-1619/3409, 12-14=-1619/3409
WEBS 3-29=-316/872, 4-29=-1584/818, 4-27=-530/1169, 5-27=-253/242, 6-26=-2233/1013, 24-26=-1943/4490, 6-24=-1509/3465, 8-18=-1468/755, 15-18=-1543/3725, 10-18=-1945/4514, 10-15=-2357/954, 11-15=-356/314, 11-14=0/119, 2-29=-984/2179, 6-27=-518/276, 21-23=-219/323, 17-19=-174/250

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 792 lb uplift at joint 30 and 760 lb uplift at joint 12.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

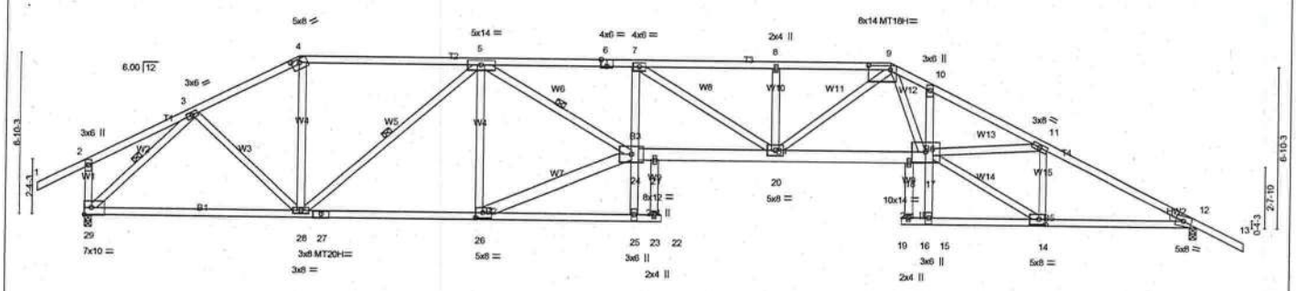
Job	Truss	Truss Type	Qty	Ply	Dwg.#092705150
L130844	T20	SPECIAL	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:35 2005 Page 1

-2-0-0	4-7-12	9-0-0	16-8-4	24-4-8	29-2-4	34-0-0	34-6-8	40-6-0	47-0-0	49-0-0
2-0-0	4-7-12	4-4-4	7-8-4	7-8-4	4-9-12	4-9-12	0-6-8	5-11-8	6-6-0	2-0-0

Scale = 1:85.5



9-0-0	16-8-4	24-4-8	29-2-4	34-6-8	40-6-0	47-0-0
9-0-0	7-8-4	7-8-4	4-9-12	5-4-4	5-11-8	6-6-0

Plate Offsets (X,Y): [4:0-4-0,0-1-15], [6:0-3-0,Edge], [9:0-11-0,0-2-0], [14:0-3-8,0-2-8], [26:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.74	20-21	>755	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-1.19	20-21	>471	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.99	Horz(TL)	0.53	12	n/a	n/a	MT18H	244/190
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 317 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.1D, B4 2 X 6 SYP No.1D, B6 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W7 2 X 6 SYP No.1D, W14 2 X 4 SYP No.2
WEDGE
Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-1-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-11-12 oc bracing.
WEBS 1 Row at midpt 5-28, 5-24, 3-29

REACTIONS (lb/size) 12=2089/0-3-8, 29=2084/0-3-8
Max Horz 29=-270(load case 6)
Max Uplift12=-791(load case 6), 29=-734(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/52, 2-3=-202/170, 3-4=-2491/1461, 4-5=-2222/1394, 5-6=-5920/3093, 6-7=-5920/3093, 7-8=-5682/2978, 8-9=-5682/2978, 9-10=-6140/3251, 10-11=-6221/3178, 11-12=-3825/2078, 12-13=0/47, 2-29=-332/416
BOT CHORD 28-29=-737/1764, 27-28=-1375/3252, 26-27=-1375/3252, 25-26=-65/150, 23-25=0/0, 22-23=0/0, 24-25=-25/140, 7-24=-174/205, 21-24=-2565/5962, 20-21=-2565/5962, 18-20=-1992/4729, 17-18=-1992/4729, 15-17=-59/111, 10-17=-78/188, 16-19=0/0, 15-16=0/0, 14-15=-64/120, 12-14=-1624/3331
WEBS 3-28=-279/671, 4-28=-309/737, 5-28=-1420/699, 5-26=-1080/583, 24-26=-1419/3359, 5-24=-1357/3093, 7-20=-467/287, 8-20=-277/251, 9-20=-642/1317, 9-17=-1005/1910, 14-17=-1791/3688, 11-17=-799/2180, 11-14=-1630/921, 3-29=-2306/1287, 21-23=-113/152, 16-18=-96/158

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 791 lb uplift at joint 12 and 734 lb uplift at joint 29.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705151
L130844	T21	SPECIAL	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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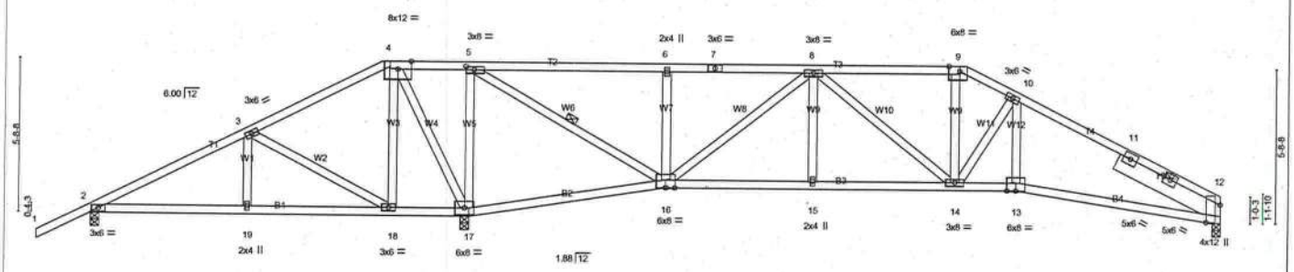
-2-0-0	5-9-4	11-0-0	13-11-8	21-2-11	26-7-6	32-0-0	34-1-5	41-8-0
2-0-0	5-9-4	5-2-12	2-11-8	7-3-3	5-4-10	5-4-10	2-1-5	7-6-11

Scale = 1:74.5



5-9-4	11-0-0	13-11-8	21-2-11	26-7-6	32-0-0	34-1-5	41-8-0
5-9-4	5-2-12	2-11-8	7-3-3	5-4-10	5-4-10	2-1-5	7-6-11

Plate Offsets (X,Y): [5:0-3-8,0-1-8], [9:0-4-10,Edge], [12:0-7-13,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	-0.11	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.17	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.06	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 240 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 8 SYP No.1D 4-4-1

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-7-13 oc bracing.
WEBS 1 Row at midpt 5-16

REACTIONS (lb/size) 12=925/0-3-8, 2=230/0-3-8, 17=2445/0-3-8
Max Horz 2=203(load case 5)
Max Uplift12=-371(load case 6), 2=-501(load case 5), 17=-1191(load case 4)
Max Grav 12=941(load case 8), 2=255(load case 7), 17=2445(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-25/511, 3-4=-231/825, 4-5=-411/1150, 5-6=-471/355, 6-7=-471/355, 7-8=-471/355, 8-9=-1225/806, 9-10=-1340/876, 10-11=-1654/968, 11-12=-1752/946
BOT CHORD 2-19=-434/132, 18-19=-434/132, 17-18=-720/357, 16-17=-1180/792, 15-16=-389/1129, 14-15=-389/1129, 13-14=-648/1427, 12-13=-661/1459
WEBS 3-19=-249/193, 3-18=-575/761, 4-18=-553/344, 4-17=-977/887, 5-17=-1231/705, 5-16=-875/1792, 6-16=-358/316, 8-16=-866/477, 8-15=0/156, 8-14=-98/148, 9-14=-249/333, 10-14=-388/415, 10-13=-98/374

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 371 lb uplift at joint 12, 501 lb uplift at joint 2 and 1191 lb uplift at joint 17.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705152
L130844	T22	HIP	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:37 2005 Page 1

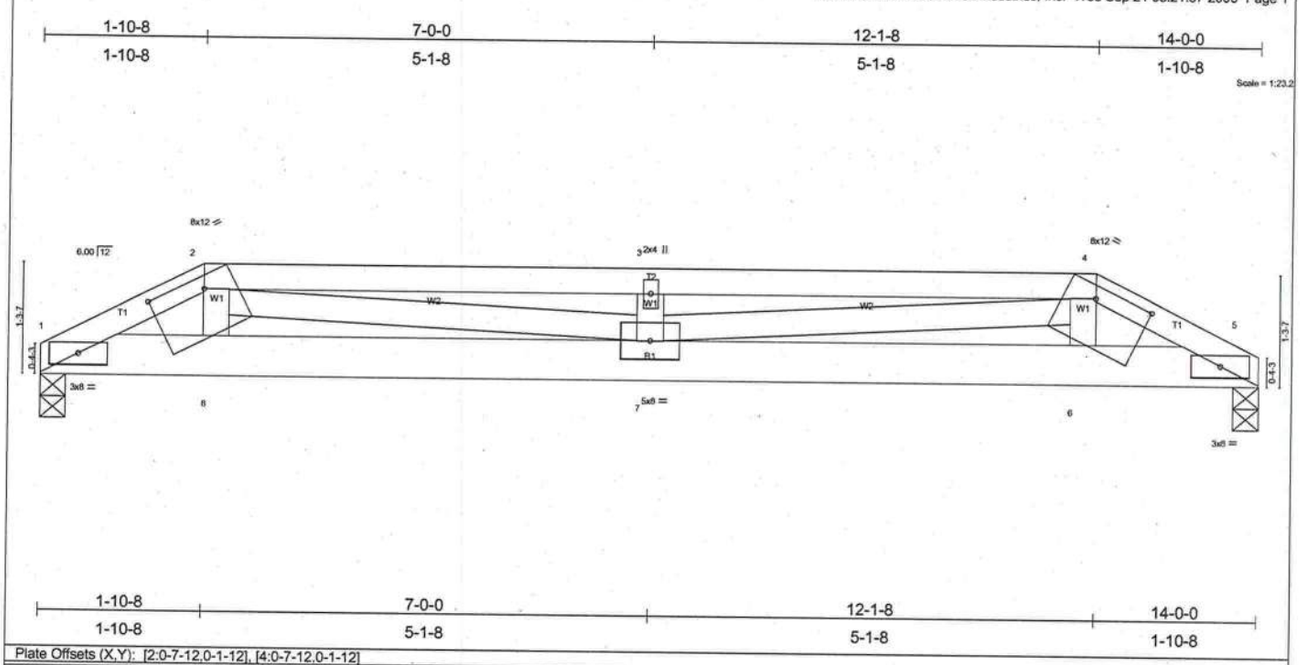


Plate Offsets (X,Y): [2:0-7-12,0-1-12], [4:0-7-12,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.24	7	>675	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.39	7	>422	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.51	Horz(TL)	0.03	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 71 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-5-0 oc bracing.

REACTIONS (lb/size) 1=1072/0-3-8, 5=1072/0-3-8
Max Horz 1=22(load case 4)
Max Uplift1=-478(load case 3), 5=-478(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2683/1241, 2-3=-4040/1909, 3-4=-4040/1909, 4-5=-2683/1242
BOT CHORD 1-8=-1123/2389, 7-8=-1162/2500, 6-7=-1148/2500, 5-6=-1108/2389
WEBS 2-8=-252/698, 2-7=-766/1588, 3-7=-208/206, 4-7=-768/1588, 4-6=-252/698

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 478 lb uplift at joint 1 and 478 lb uplift at joint 5.
5) Girder carries tie-in span(s): 7-0-0 from 1-10-8 to 12-1-8
6) Girder carries hip end with 1-10-8 right side setback, 1-10-8 left side setback, and 2-2-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 18 lb down and 11 lb up at 12-1-8, and 18 lb down and 11 lb up at 1-10-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-4=-52(F=2), 4-5=-54, 1-8=-30, 6-8=-128(F=-98), 5-6=-30
Concentrated Loads (lb)
Vert: 8=-18(F) 6=-18(F)

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705153
L130844	T23	HIP	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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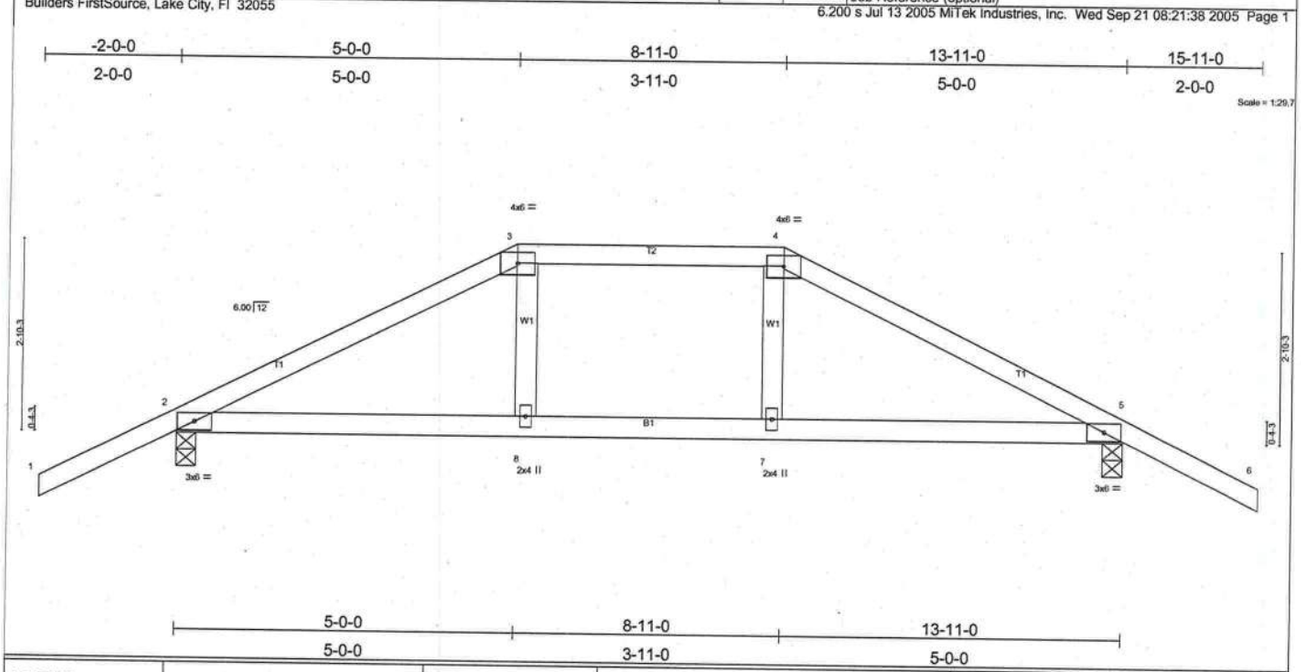

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	0.08	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.10	5-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.14	Horz(TL)	0.03	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 57 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-8-8 oc bracing.

REACTIONS (lb/size) 2=1042/0-3-8, 5=1042/0-3-8
Max Horz 2=-112(load case 5)
Max Uplift2=-795(load case 4), 5=-795(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1618/1028, 3-4=-1405/946, 4-5=-1618/1028, 5-6=0/47
BOT CHORD 2-8=-853/1381, 7-8=-866/1405, 5-7=-851/1381
WEBS 3-8=-274/444, 4-7=-274/444

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 795 lb uplift at joint 2 and 795 lb uplift at joint 5.
5) Girder carries hip end with 5-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 158 lb up at 8-11-0, and 245 lb down and 158 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-91(F=-37), 4-6=-54, 2-8=-30, 7-8=-50(F=-20), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-245(F) 7=-245(F)

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705154
L130844	T24	COMMON	2	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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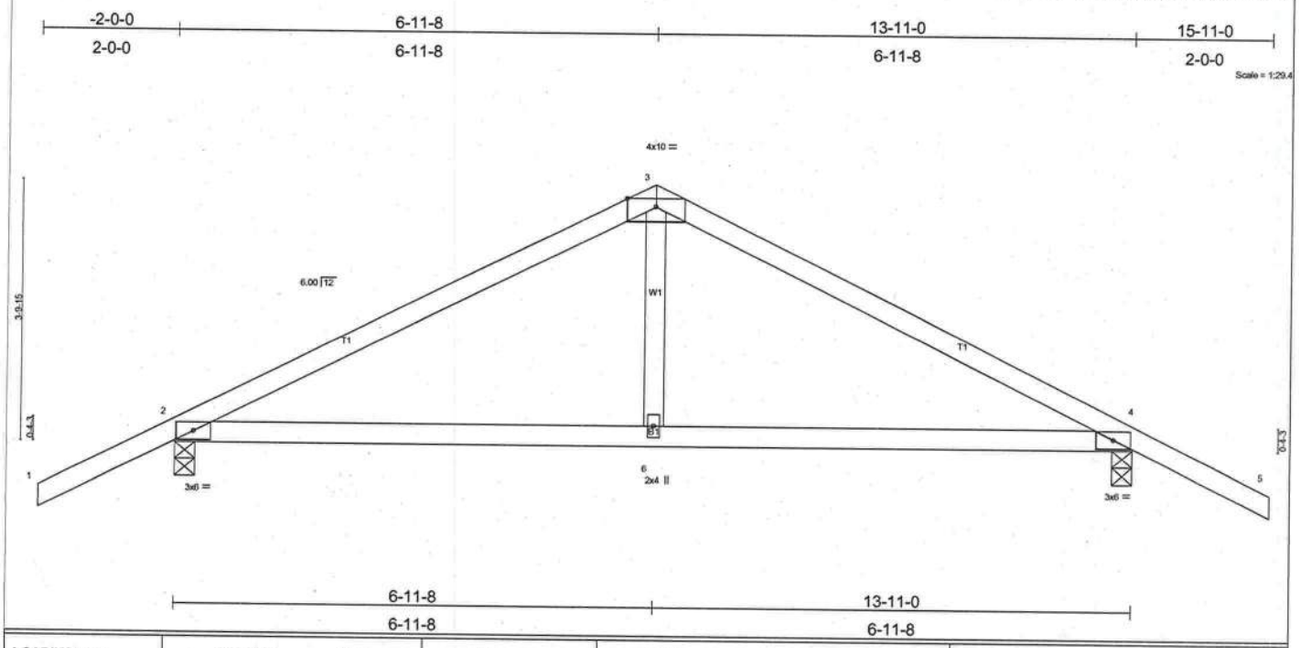

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	0.17	4-6	>946	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	0.15	4-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.12	Horz(TL)	-0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 56 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-8-8 oc bracing.

REACTIONS (lb/size) 2=688/0-3-8, 4=688/0-3-8
Max Horz 2=135(load case 5)
Max Uplift2=-586(load case 5), 4=-586(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-790/1132, 3-4=-790/1132, 4-5=0/47
BOT CHORD 2-6=-783/635, 4-6=-783/635
WEBS 3-6=-517/248

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 586 lb uplift at joint 2 and 586 lb uplift at joint 4.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	Dwg.#092705155
L130844	V01	VALLEY	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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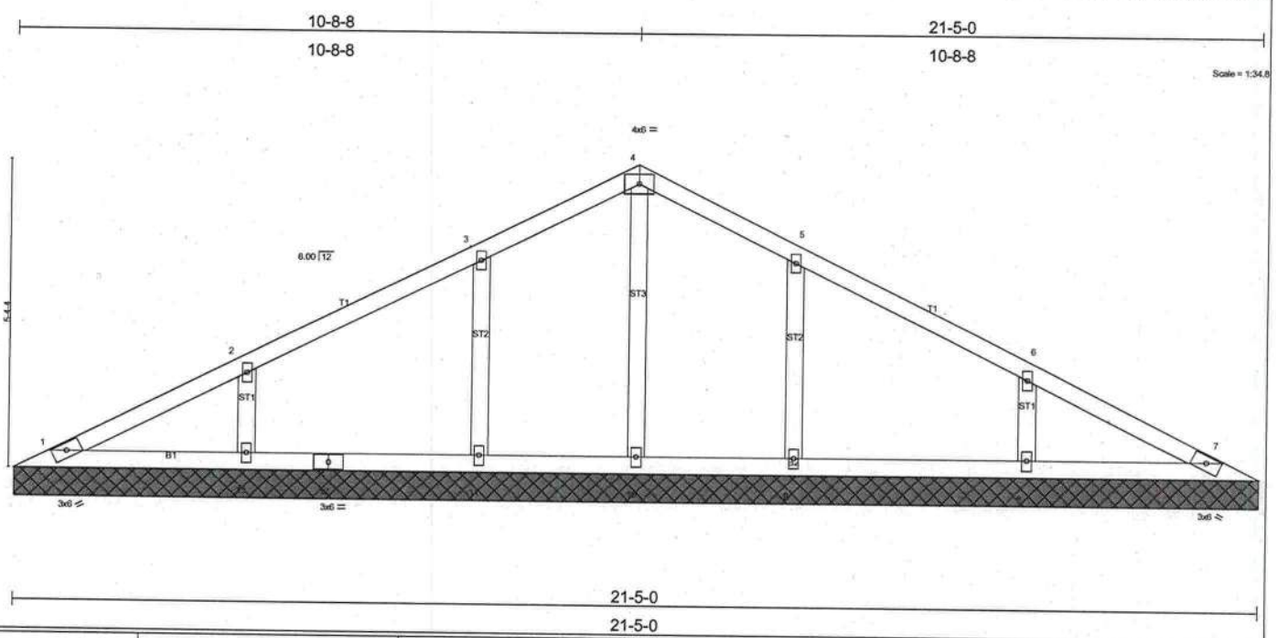

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 87 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=112/21-5-0, 7=112/21-5-0, 10=175/21-5-0, 8=355/21-5-0, 9=293/21-5-0, 13=355/21-5-0, 11=293/21-5-0
Max Horz 1=120(load case 5)
Max Uplift1=-38(load case 6), 7=-15(load case 6), 8=-229(load case 6), 9=-189(load case 6), 13=-229(load case 5), 11=-190(load case 5)
Max Grav 1=112(load case 1), 7=112(load case 1), 10=175(load case 1), 8=355(load case 1), 9=297(load case 8), 13=355(load case 1), 11=297(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-130/47, 2-3=-65/118, 3-4=-53/229, 4-5=-53/229, 5-6=-65/97, 6-7=-80/40
BOT CHORD 1-13=0/109, 12-13=0/109, 11-12=0/109, 10-11=0/109, 9-10=0/109, 8-9=0/109, 7-8=0/109
WEBS 4-10=-107/0, 6-8=-223/316, 5-9=-193/274, 2-13=-223/316, 3-11=-193/274

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are 2x4 MT20 unless otherwise indicated.
4) Gable requires continuous bottom chord bearing.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 1, 15 lb uplift at joint 7, 229 lb uplift at joint 8, 189 lb uplift at joint 9, 229 lb uplift at joint 13 and 190 lb uplift at joint 11.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705156
L130844	V02	VALLEY	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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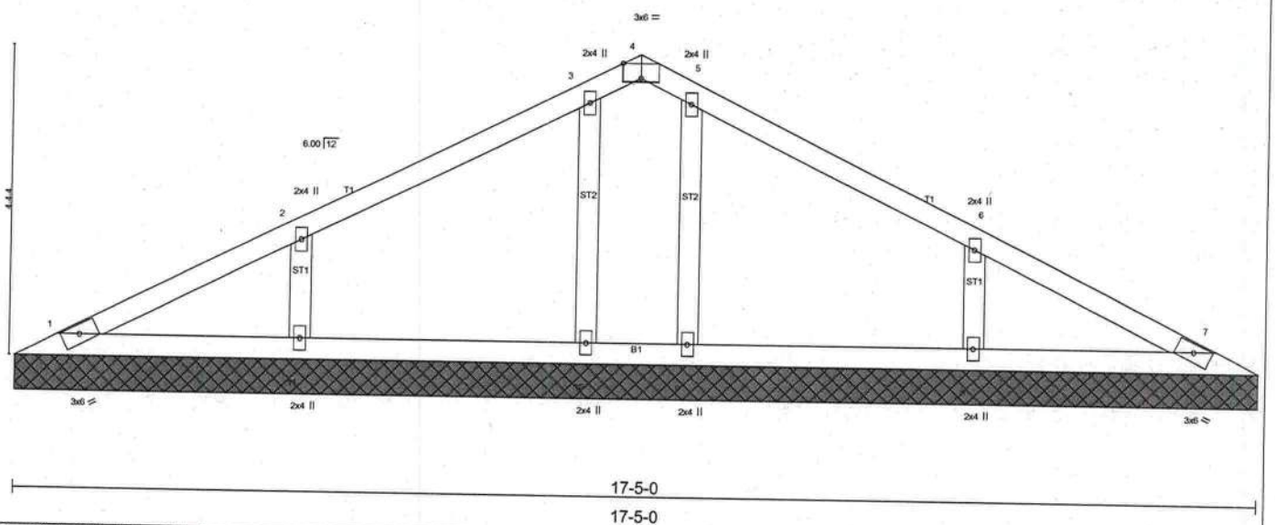
8-8-8 / 8-8-8 — 17-5-0 / 8-8-8

Scale = 1:28.3



17-5-0 / 17-5-0

Plate Offsets (X,Y): [4:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 68 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=96/17-5-0, 7=96/17-5-0, 8=360/17-5-0, 9=224/17-5-0, 11=360/17-5-0, 10=224/17-5-0
Max Horz 1=96(load case 5)
Max Uplift1=-18(load case 6), 7=-7(load case 6), 8=-234(load case 6), 9=-101(load case 6), 11=-234(load case 5), 10=-118(load case 5)
Max Grav 1=100(load case 7), 7=100(load case 8), 8=360(load case 1), 9=241(load case 8), 11=360(load case 1), 10=241(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-133/70, 2-3=-39/77, 3-4=-17/122, 4-5=-17/122, 5-6=-39/65, 6-7=-112/70
BOT CHORD 1-11=-18/145, 10-11=-18/145, 9-10=-18/145, 8-9=-18/145, 7-8=-18/145
WEBS 6-8=-226/324, 5-9=-167/132, 2-11=-226/324, 3-10=-167/137

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 1, 7 lb uplift at joint 7, 234 lb uplift at joint 8 , 101 lb uplift at joint 9, 234 lb uplift at joint 11 and 118 lb uplift at joint 10.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705157
L130844	V03	VALLEY	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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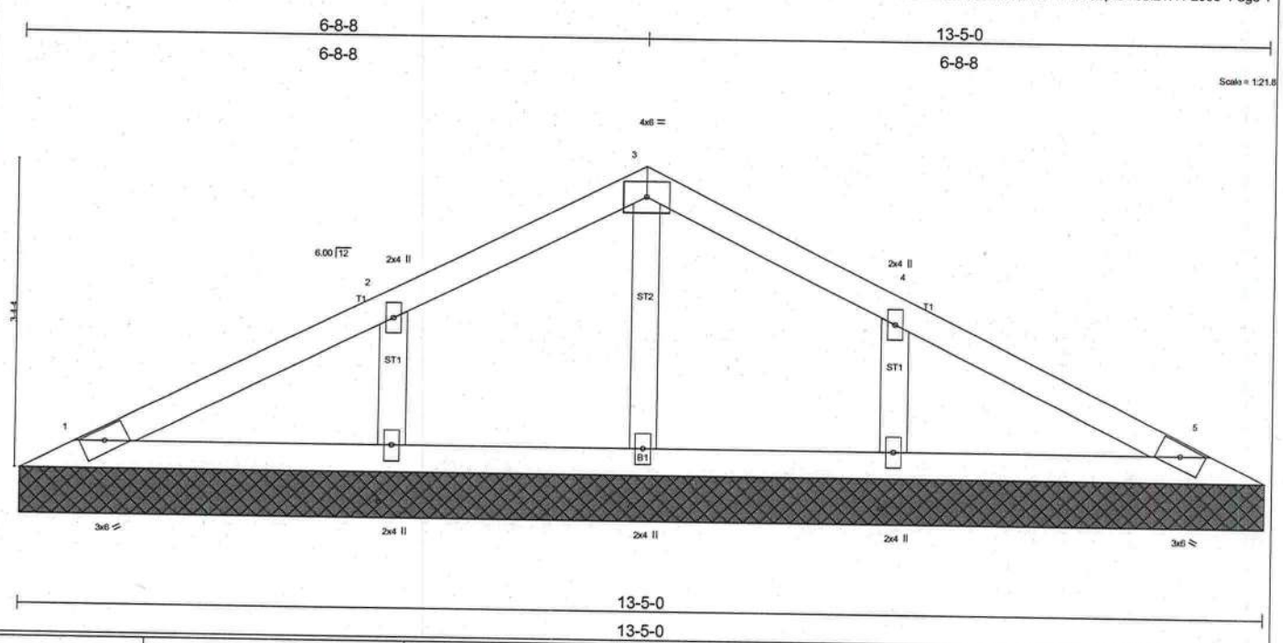

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=121/13-5-0, 5=121/13-5-0, 7=167/13-5-0, 6=307/13-5-0, 8=307/13-5-0
Max Horz 1=72(load case 5)
Max Uplift1=-40(load case 6), 5=-44(load case 6), 7=-5(load case 5), 6=-200(load case 6), 8=-200(load case 5)
Max Grav 1=121(load case 1), 5=121(load case 1), 7=167(load case 1), 6=311(load case 8), 8=311(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-54/50, 2-3=-53/142, 3-4=-53/142, 4-5=-50/37
BOT CHORD 1-8=0/63, 7-8=0/63, 6-7=0/63, 5-6=0/63
WEBS 3-7=-108/38, 4-6=-194/287, 2-8=-194/287

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 1, 44 lb uplift at joint 5, 5 lb uplift at joint 7, 200 lb uplift at joint 6 and 200 lb uplift at joint 8.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705158
L130844	V04	VALLEY	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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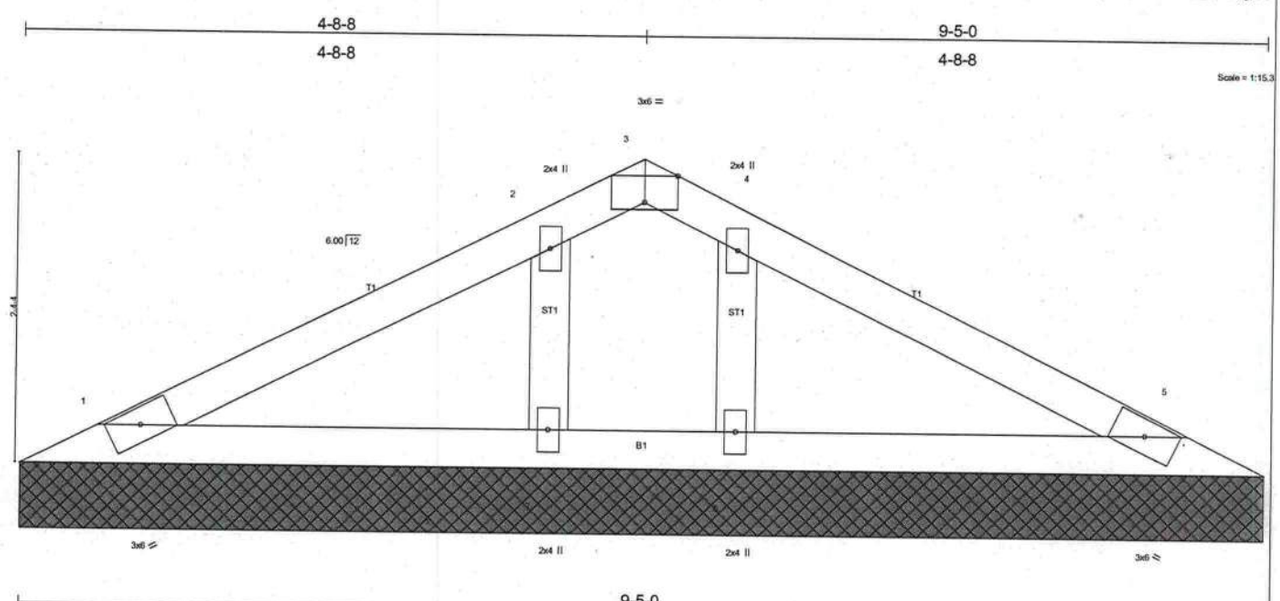

Plate Offsets (X,Y): [3:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.10	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 9-5-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=98/9-5-0, 5=98/9-5-0, 6=246/9-5-0, 7=246/9-5-0
Max Horz 1=48(load case 5)
Max Uplift1=-25(load case 5), 5=-33(load case 6), 6=-151(load case 6), 7=-159(load case 5)
Max Grav 1=103(load case 7), 5=103(load case 8), 6=265(load case 8), 7=265(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-88/90, 2-3=-1/34, 3-4=-1/34, 4-5=-88/90
BOT CHORD 1-7=-40/145, 6-7=-40/145, 5-6=-40/145
WEBS 4-6=-172/224, 2-7=-172/224

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 1, 33 lb uplift at joint 5, 151 lb uplift at joint 6 and 159 lb uplift at joint 7.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
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Job	Truss	Truss Type	Qty	Ply	Dwg.#092705159
L130844	V05	VALLEY	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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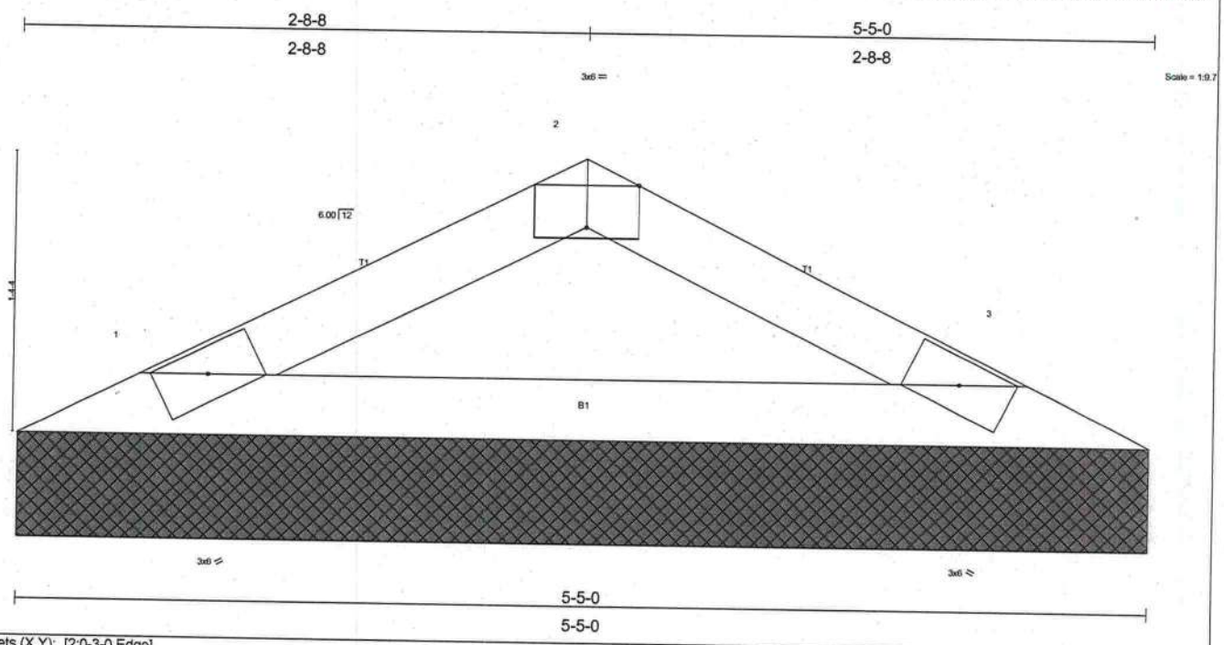

Plate Offsets (X,Y): [2:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 15 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=176/5-5-0, 3=176/5-5-0
Max Horz 1=25(load case 5)
Max Uplift1=-69(load case 5), 3=-69(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-146/215, 2-3=-146/215
BOT CHORD 1-3=-138/113

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 1 and 69 lb uplift at joint 3.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
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Job	Truss	Truss Type	Qty	Ply	Dwg.#092705160
L130844	V06	VALLEY	1	1	

Builders FirstSource, Lake City, Fl 32055

Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:43 2005 Page 1

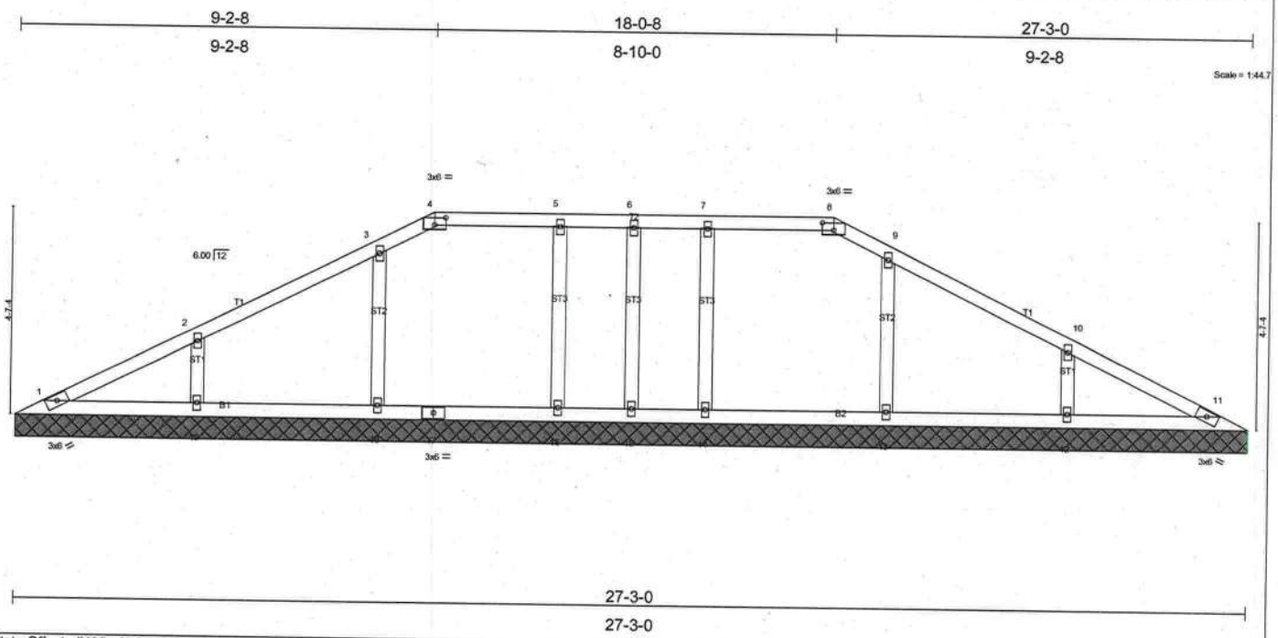


Plate Offsets (X,Y): [4:0-3-0,0-2-0], [8:0-3-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.01	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 115 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=139/27-3-0, 11=139/27-3-0, 15=37/27-3-0, 12=347/27-3-0, 13=319/27-3-0, 14=268/27-3-0, 19=347/27-3-0, 18=319/27-3-0, 16=268/27-3-0
Max Horz 1=102(load case 5)
Max Uplift1=-29(load case 3), 11=-19(load case 4), 15=-41(load case 3), 12=-235(load case 6), 13=-143(load case 6), 14=-113(load case 4), 19=-234(load case 5), 18=-154(load case 5), 16=-111(load case 4)
Max Grav 1=142(load case 7), 11=142(load case 8), 15=37(load case 1), 12=347(load case 8), 13=319(load case 1), 14=271(load case 7), 19=347(load case 7), 18=319(load case 1), 16=271(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-135/68, 2-3=-126/104, 3-4=-118/183, 4-5=-68/169, 5-6=-68/169, 6-7=-68/169, 7-8=-68/169, 8-9=-118/183, 9-10=-126/81, 10-11=-109/46
BOT CHORD 1-19=-22/130, 18-19=-22/130, 17-18=-22/130, 16-17=-22/130, 15-16=-22/130, 14-15=-22/130, 13-14=-22/130, 12-13=-22/130, 11-12=-22/130
WEBS 6-15=-35/63, 10-12=-220/320, 9-13=-198/229, 7-14=-167/134, 2-19=-220/320, 3-18=-198/229, 5-16=-167/131

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 1, 19 lb uplift at joint 11, 41 lb uplift at joint 15, 235 lb uplift at joint 12, 143 lb uplift at joint 13, 113 lb uplift at joint 14, 234 lb uplift at joint 19, 154 lb uplift at joint 18 and 111 lb uplift at joint 16.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705161
L130844	V07	VALLEY	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:44 2005 Page 1

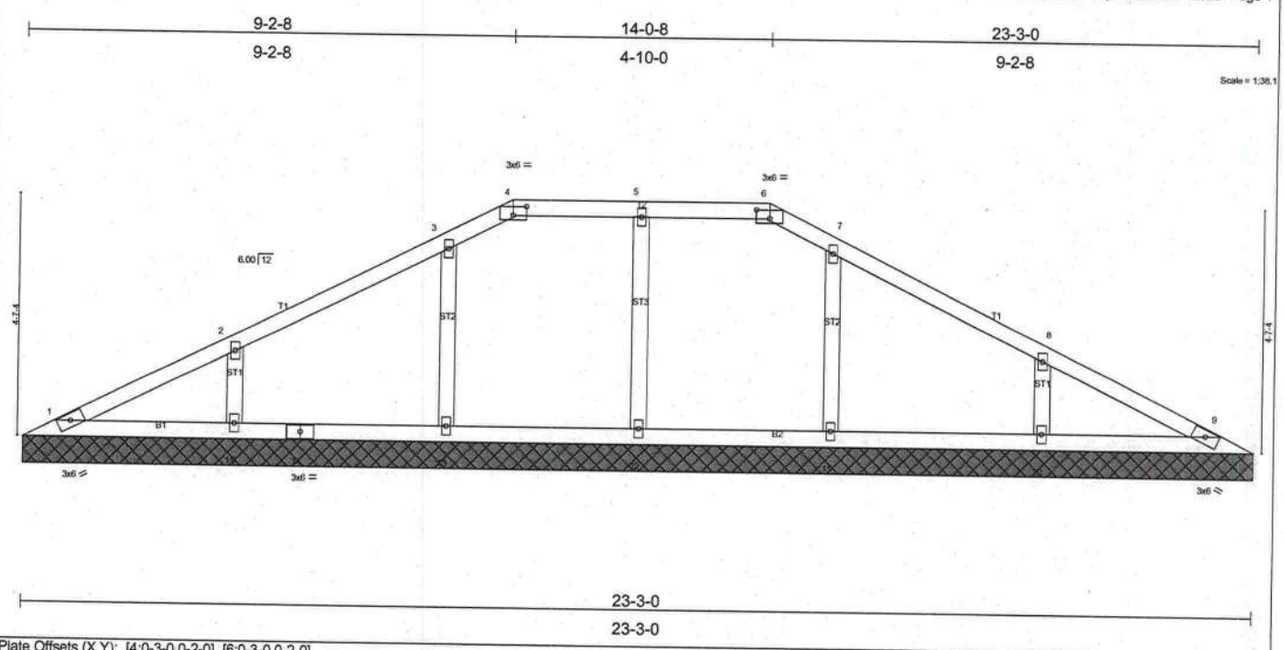


Plate Offsets (X,Y): [4:0-3-0,0-2-0], [6:0-3-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 91 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=127/23-3-0, 9=127/23-3-0, 12=280/23-3-0, 10=351/23-3-0, 11=306/23-3-0, 15=351/23-3-0, 13=306/23-3-0
Max Horz 1=102(load case 5)
Max Uplift1=-21(load case 3), 9=-11(load case 3), 12=-116(load case 4), 10=-235(load case 6), 11=-144(load case 6), 15=-234(load case 5), 13=-154(load case 5)
Max Grav 1=129(load case 7), 9=129(load case 8), 12=282(load case 8), 10=352(load case 8), 11=306(load case 1), 15=352(load case 7), 13=306(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-138/52, 2-3=-101/88, 3-4=-91/174, 4-5=-46/161, 5-6=-46/161, 6-7=-91/174, 7-8=-101/66, 8-9=-105/28
BOT CHORD 1-15=-7/138, 14-15=-7/138, 13-14=-7/138, 12-13=-7/138, 11-12=-7/138, 10-11=-7/138, 9-10=-7/138
WEBS 5-12=-176/137, 8-10=-222/321, 7-11=-193/227, 2-15=-222/321, 3-13=-193/227

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 1, 11 lb uplift at joint 9, 116 lb uplift at joint 12, 235 lb uplift at joint 10, 144 lb uplift at joint 11, 234 lb uplift at joint 15 and 154 lb uplift at joint 13.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705162
L130844	V08	VALLEY	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:45 2005 Page 1

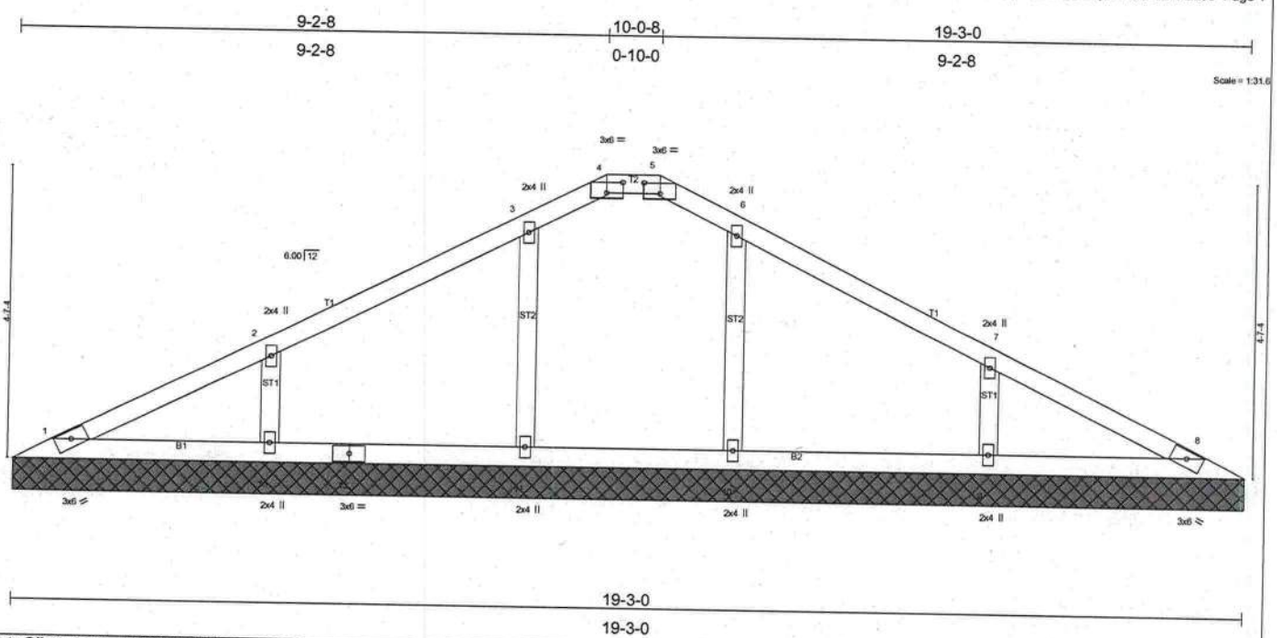


Plate Offsets (X,Y): [4:0-3-0,0-2-0], [5:0-3-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 73 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=111/19-3-0, 8=111/19-3-0, 9=355/19-3-0, 10=291/19-3-0, 13=355/19-3-0, 11=291/19-3-0
Max Horz 1=-102(load case 6)
Max Uplift9=-236(load case 6), 10=-140(load case 6), 13=-235(load case 5), 11=-151(load case 5)
Max Grav 1=114(load case 7), 8=114(load case 8), 9=355(load case 1), 10=291(load case 1), 13=355(load case 1), 11=291(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-184/38, 2-3=-69/41, 3-4=-56/90, 4-5=-16/91, 5-6=-56/90, 6-7=-69/35, 7-8=-184/38
BOT CHORD 1-13=0/208, 12-13=0/208, 11-12=0/208, 10-11=0/208, 9-10=0/208, 8-9=0/208
WEBS 7-9=-224/325, 6-10=-186/213, 2-13=-224/325, 3-11=-186/213

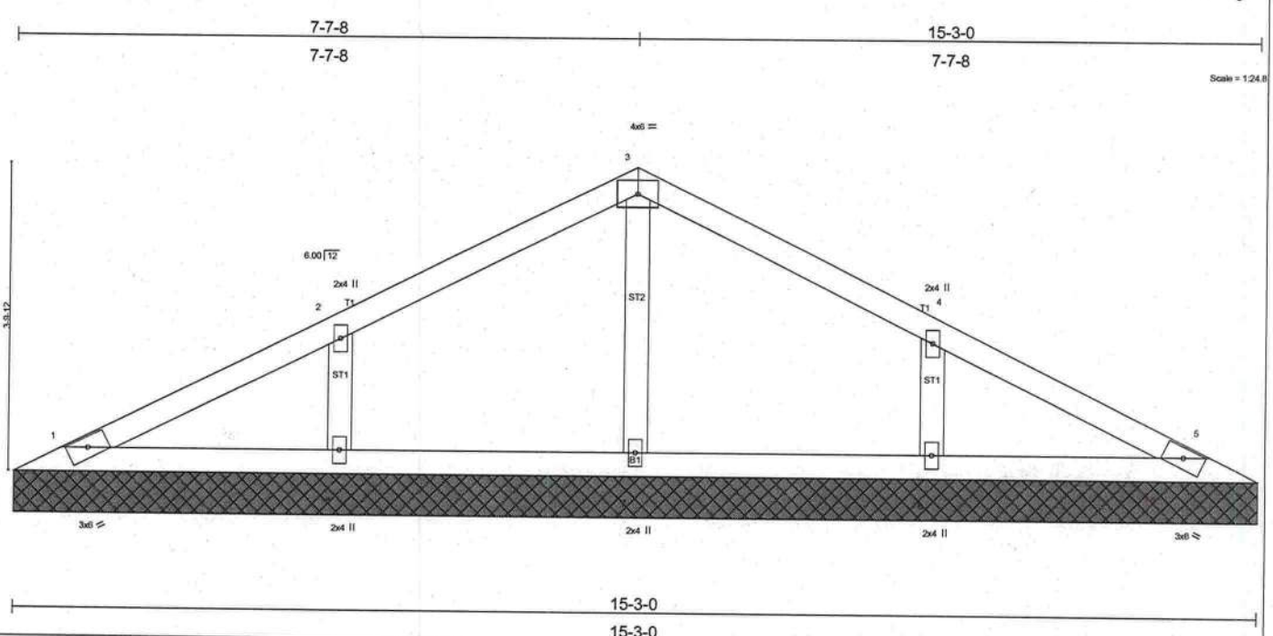
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Gable requires continuous bottom chord bearing.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 236 lb uplift at joint 9, 140 lb uplift at joint 10, 235 lb uplift at joint 13 and 151 lb uplift at joint 11.

LOAD CASE(S) Standard

SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#092705163
L130844	V09	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Sep 21 08:21:45 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 55 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=121/15-3-0, 5=121/15-3-0, 7=270/15-3-0, 6=333/15-3-0, 8=333/15-3-0
Max Horz 1=83(load case 5)
Max Uplift1=-43(load case 6), 5=-39(load case 6), 7=-39(load case 5), 6=-220(load case 6), 8=-220(load case 5)
Max Grav 1=121(load case 1), 5=121(load case 1), 7=270(load case 1), 6=341(load case 8), 8=341(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-65/57, 2-3=-69/151, 3-4=-69/151, 4-5=-58/33
BOT CHORD 1-8=0/63, 7-8=0/63, 6-7=0/63, 5-6=0/63
WEBS 3-7=-169/103, 4-6=-217/304, 2-8=-217/304

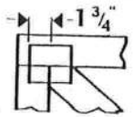
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 43 lb uplift at joint 1, 39 lb uplift at joint 5, 39 lb uplift at joint 7 , 220 lb uplift at joint 6 and 220 lb uplift at joint 8.

LOAD CASE(S) Standard

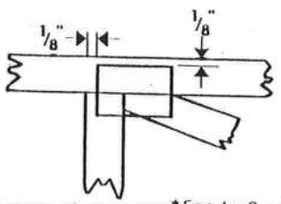
SEPTEMBER 27, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

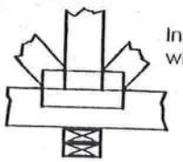
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

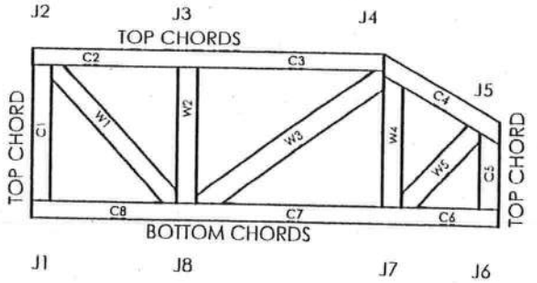
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC

SERIES/MODEL: 420/430/440
PRODUCT TYPE: Aluminum Sliding Glass Door

	Summary of Results		
Title	**Test Specimen #1**	**Test Specimen #2**	**Test Specimen #3**
Rating	SGD-R25 182 x 96	SGD-R35 182 x 80	SGD-R40 144 x 96
Operating Force	17 lbf max.	17 lbf max.	N/A
Air Infiltration	0.23 cfm/ft^2	0.27 cfm/ft^2	N/A
Water Resistance Test Pressure	3.75/6.0/9.0 psf	6.0 psf	N/A
Uniform Load Deflection Test Pressure	±35.0 psf	±35.0 psf	+40.0 psf/-40.1 psf
Uniform Load Structural Test Pressure	±37.5 psf	±52.5 psf	+60.0 psf/-60.2 psf
Forced Entry Resistance	Grade 10	Grade 10	N/A

Reference should be made to ATI Report No. 52112.01-122-47 for complete test specimen description and data.

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No.:	52112.01-122-47
Revision 1:	09/13/04
Test Dates:	06/30/04
Through:	08/12/04
Report Date:	08/30/04
Expiration Date:	07/02/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on three Series/Model 420/430/440, aluminum sliding glass doors at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: SGD-R25 182 x 96; Test Specimen #2: SGD-R35 182 x 80; Test Specimen #3: SGD-R40 144 x 96. Test specimen description and results are reported herein.

Test Specification: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 420/430/440

Product Type: Aluminum Sliding Glass Door

Test Specimen #1: SGD-R25 182 x 96 (XXO)

Overall Size: 15' 1-3/4" wide by 8' 0" high

Active Door Panel Size (2): 5' 0-1/2" wide by 7' 11" high

Fixed Door Panel Size: 5' 1" wide by 7' 11" high

Screen Size: 5' 0-3/8" wide by 7' 11" high

Overall Area: 121.2 ft^2

Reinforcement: The active and fixed interlocking stile utilized a steel U-shaped reinforcement (Drawing #9917525). The fixed intermediate jamb utilized a steel reinforcement (Drawing #9917520).

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Test Specimen Description: (Continued)

Test Specimen #2: SGD-R35 182 x 80 (OXX)

Overall Size: 15' 1-3/4" wide by 6' 8" high

Active Door Panel Size (2): 5' 0-1/2" wide by 6' 7" high

Fixed Door Panel Size: 4' 8-7/8" wide by 6' 2-5/8" high

Screen Size: 5' 0-3/8" wide by 6' 7" high

Overall Area: 101 ft^2

Reinforcement: No reinforcement was utilized.

Test Specimen #3: SGD-R40 144 x 96 (XOX)

Overall Size: 12' 0" wide by 8' 0" high

Active Door Panel Size: 3' 8-1/4" wide by 7' 10-1/2" high

Fixed Door Panel Size (2): 3' 8-3/4" wide by 7' 6-1/2" high

Screen Size: 3' 11-1/2" wide by 7' 11-3/8" high

Overall Area: 96 ft^2

Reinforcement: The active and fixed interlocking stile utilized a steel U-shaped reinforcement (Drawing #9917525). The fixed intermediate jamb utilized a steel reinforcement (Drawing #9917520). The interlock utilized an aluminum reinforcement (Drawing #SECT4237).

The following descriptions apply to all specimens.

Finish: All aluminum was white.

Glazing Details: All glazing consisted of a single sheet of 3/16" thick clear tempered glass that was channel glazed with a wrap around rubber gasket.

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.187" backed by 0.270" high polypile with center fin	2 Rows	Stiles

Frame Construction: The frame was constructed of extruded aluminum. Corners were coped, butted, sealed, and fastened with two #8 by 5/8" screws.

Door Panel Construction: The door panels were constructed of extruded aluminum members. Corners were coped, butted, and fastened with one 1/4" by 3/4" screw at the bottom and two #8 by 3/4" screws at the top.

Screen Construction: The screen was constructed of extruded aluminum members. Corners were coped, butted, and fastened with one 1/4" by 3/4" and one #8 by 1" screw at the bottom and one #8 by 1" screw at the top.

Hardware:

Description	Quantity	Location
Locking handle	1	44" from active panel bottom
Roller assembly	2	3" from bottom rail ends
Screen locking handle	1	46" from screen bottom rail

Drainage:

Description	Quantity	Location
Sloped sill	1	Sill

Installation: The units were installed into a #2 Spruce-Pine-Fir wood test buck. The units were fastened to the test buck with two rows of #8 by 1-1/4" screws, 8" from each end and 23" on center. The exterior perimeter was sealed with silicone.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO)			
2.2.1.6.1	Operating Force	17 lbf	20 lbf max.
	Breakaway force	24 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283		
	1.57 psf (25 mph)	0.23 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: *The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*

2.1.3	Water Resistance per ASTM E 547		
	(with and without screen)		
	2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330		
	(Deflections reported were taken on the meeting rail)		
	(Loads were held for 52 seconds)		
	15.0 psf (positive)	0.56"	See Note #2
	15.0 psf (negative)	0.57"	See Note #2

Note #2: *The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

2.1.4.2	Uniform Load Structural per ASTM E 330		
	(Permanent sets reported were taken on the meeting stile)		
	(Loads were held for 10 seconds)		
	22.5 psf (positive)	0.02"	0.30" max.
	22.5 psf (negative)	0.03"	0.30" max.
2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction - 70 lbs		
	Locking stile	0.12"/24%	0.50"/100%
	Interlock stile	0.12"/24%	0.50"/100%

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO) (Continued)			
2.2.1.6.2	Deglazing Test per ASTM E 987 In remaining direction - 50 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 842		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 through A6	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) 3.75 psf	No leakage	No leakage
4.3	Water Resistance per ASTM E 547 (with and without screen) (with sill riser) 6.0 psf	No leakage	No leakage
4.3	Water Resistance per ASTM E 547 (with and without screen) (with 2-5/8" Dade County sill extension) 9.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	35.0 psf (positive)	2.98"	See Note #2
	35.0 psf (negative)	2.52"	See Note #2

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO) (Continued)			
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	37.5 psf (positive)	0.20"	0.36" max.
	37.5 psf (negative)	0.19"	0.36" max.
Test Specimen #2: SGD-R35 182 x 80 (OXX)			
2.2.1.6.1	Operating Force	17 lbf	20 lbf max.
	Breakaway force	21 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283		
	1.57 psf (25 mph)	0.27 cfm/ft^2	0.3 cfm/ft^2 max.
	Note #1: *The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*		
2.1.3	Water Resistance per ASTM E 547 (with and without screen)		
	2.86 psf	No leakage	No leakage
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Locking stile	0.12"/24%	0.50"/100%
	Interlock stile	0.12"/24%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 842		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 through A6	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #2: SGD-R35 182 x 80 (OXX) (Continued)			
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) (with sill riser) 6.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	35.0 psf (positive)	1.28"	See Note #2
	35.0 psf (negative)	1.33"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	52.5 psf (positive)	0.13"	0.30" max.
	52.5 psf (negative)	0.15"	0.30" max.
Test Specimen #3: SGD-R40 144 x 96 (XOX)			
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	40.0 psf (positive)	1.42"	See Note #2
	40.1 psf (negative)	1.28"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	60.0 psf (positive)	0.27"	0.37" max.
	60.2 psf (negative)	0.30"	0.37" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess vlm

Digitally Signed for: Mark A. Hess by Vicki L. McElwain

Mark Hess
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P.E.
Senior Project Engineer

MH:vlm

MI WINDOWS AND DOORS, INC.

420 / 430 / 440 SERIES ALUMINUM SLIDING GLASS DOOR

COMAPARATIVE ANALYSIS CHART IN DESIGN PRESSURE

STEEL AND ALUMINUM REINFORCING

09/08/2004

SGD ALUM & STL REINF

PANEL WIDTH >>	24	30	36	48
PANEL HEIGHT 80	85	71	62	51
96	69	57	49	40

TEST REPORT NO: ATI-52112.01-122-47
DESIGN PRESSURE ACHIEVED IN TEST: POS. & NEG. 40.0 PSF
WATER TEST PRESSURE:
- 1-3/8 IN. SILL RISER: 3.75 PSF
- 1-7/8 IN. SILL RISER: 6.0 PSF
- 2-5/8 IN. SILL RISER: 9.0 PSF

OVERALL TEST SIZE: 12'-0" X 8'-0" NOMINAL

OVERALL PANEL SIZE: 48 IN. X 96 IN. NOMINAL
GLAZING: SINGLE PC. OF 3/16 IN. THK. TEMPERED GLASS
REINFORCING: STEEL IN INTERLOCKING STILES AND INTERMEDIATE JAMB. ADDITIONAL ALUM. REINFORCING ON EXTERIOR OF OPERATING INTERLOCK STILE.
CONFIGURATION: XOX

LIMITATIONS:
THE ABOVE ARE POSITIVE AND NEGATIVE STRUCTURAL DESIGN LOADS FROM COMPARATIVE ANALYSIS & HAVE NOT BEEN CAPPED BY RESULTS OF WATER PERFORMANCE TESTING.
WHERE LOCAL CODE REQUIRES WATER RESISTANCE TESTING TO PASS A MIN. 15% OF DESIGN PRESSURE, ALLOWABLE POSITIVE DESIGN PRESSURE WOULD BE CAPPED AS FOLLOWS:
- WHERE 1-3/8 IN. SILL RISER IS EMPLOYED POSITIVE DESIGN PRESSURE IS CAPPED AT 25.0 PSF.
- WHERE 1-7/8 IN. SILL RISER IS EMPLOYED POSITIVE DESIGN PRESSURE IS CAPPED AT 40.0 PSF.
- WHERE 2-5/8 IN. SILL RISER IS EMPLOYED POSITIVE DESIGN PRESSURE IS CAPPED AT 60.0 PSF.

PANEL WIDTHS AND HEIGHTS ARE NOMINAL, IN INCHES.

PREPARED BY:
PRODUCT TECHNOLOGY CORPORATION
1150 LOUISIANA AVENUE, SUITE 6
WINTER PARK, FLORIDA 32789
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www.ptc-corp.com

MI WINDOWS AND DOORS, INC.

420 / 430 / 440 SERIES ALUMNUM SLIDING GLASS DOOR

09/08/2004

COMAPARATIVE ANALYSIS CHART IN DESIGN PRESSURE

SGD non-Reinf

PANEL WIDTH >>	24	30	36	48	60
PANEL HEIGHT 80	64	54	47	39	35

TEST REPORT NO: ATI-52112.01-122-47
DESIGN PRESSURE ACHIEVED IN TEST: POS. & NEG. 35.0 PSF
WATER TEST PRESSURE:
- 1-3/8 IN. SILL RISER: 3.75 PSF
- 1-7/8 IN. SILL RISER: 6.0 PSF
- 2-5/8 IN. SILL RISER: 9.0 PSF

OVERALL SIZE TESTED: 15'-0" X 6'-8" NOMINAL
OVERALL PANEL SIZE TESTED: 5'-0" X 6'-8" NOMINAL
GLAZING: SINGLE PC. OF 3/16 IN THICK TEMP. GLASS
REINFORCING: NONE
CONFIGURATION TESTED: XXO

LIMITATIONS:
THE ABOVE ARE POSITIVE AND NEGATIVE STRUCTURAL DESIGN LOADS FROM COMPARATIVE ANALYSIS & HAVE NOT BEEN CAPPED BY RESULTS OF WATER PERFORMANCE TESTING.
WHERE LOCAL CODE REQUIRES WATER RESISTANCE TESTING TO PASS A MIN. 15% OF DESIGN PRESSURE, ALLOWABLE POSITIVE DESIGN PRESSURE WOULD BE CAPPED AS FOLLOWS:
- WHERE 1-3/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURE = 25.0 PSF
- WHERE 1-7/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURE = 40.0PSF
- WHERE 2-5/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURE = 60.0 PSF

PANEL WIDTHS AND HEIGHTS ARE NOMINAL, IN INCHES.

PREPARED BY:
PRODUCT TECHNOLOGY CORPORATION
1150 LOUISIANA AVENUE, SUITE 6
WINTER PARK, FLORIDA 32789
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www.ptc-corp.com

MI WINDOWS AND DOORS, INC.

420 / 430 / 440 SERIES ALUMINUM SLIDING GLASS DOOR STEEL REINFORCED 09/08/2004

COMAPARATIVE ANALYSIS CHART IN DESIGN PRESSURE SGD STL REINF

PANEL WIDTH >>	24	30	36	48	60
PANEL HEIGHT 80	61	51	44	37	33
96	49	41	35	29	25

TEST REPORT NO: ATI-52112.01-122-47
DESIGN PRESSURE ACHIEVED IN TEST: POS. & NEG. 25.0 PSF
WATER TEST PRESSURE:
- 1-3/8 IN. SILL RISER: 3.75 PSF
- 1-7/8 IN. SILL RISER: 6.0 PSF
- 2-5/8 IN. SILL RISER: 9.0 PSF

OVERALL SIZE TESTED: 15'-0" X 8'-0" NOMINAL
OVERALL PANEL SIZE TESTED: 60 IN. X 96 IN. NOMINAL
GLAZING: SINGLE PC. OF 3/16 IN. THK. TEMPERED GLASS
REINFORCING: STEEL IN INTERLOCKING STILES, AND FIXED INTERMEDIATE JAMB
CONFIGURATION TESTED: OXX

LIMITATIONS:
THE ABOVE ARE POSITIVE AND NEGATIVE STRUCTURAL DESIGN LOADS FROM COMPARATIVE ANALYSIS & HAVE NOT BEEN CAPPED BY RESULTS OF WATER PERFORMANCE TESTING.
WHERE LOCAL CODE REQUIRES WATER RESISTANCE TESTING TO PASS A MIN. 15% OF DESIGN PRESSURE, ALLOWABLE POSITIVE DESIGN PRESSURE WOULD BE CAPPED AS FOLLOWS:
- WHERE 1-3/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURES ARE CAPPED AT 25.0 PSF.
- WHERE 1-7/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURES ARE CAPPED AT 40.0 PSF.
- WHERE 2-5/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURES ARE CAPPED AT 60.0 PSF.

PANEL WIDTHS AND HEIGHTS ARE NOMINAL, IN INCHES.

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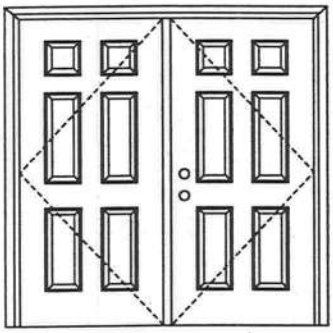
www.ptc-corp.com

XX
Opaque Outswing Unit

COP-WL-MA0122-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft™ Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

Masonite®

XX
Opaque Outswing Unit

COP-WL-MA0122-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003
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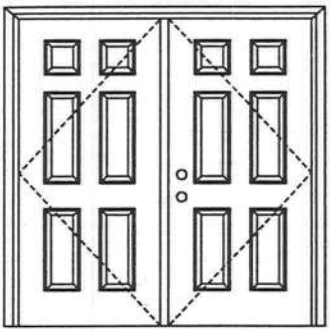
Masonite®

XX
Opaque Inswing Unit

COP-WL-MA0102-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure

+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0002-02.

MINIMUM INSTALLATION DETAIL:

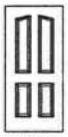
Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003
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Masonite®

XX
Opaque Inswing Unit

COP-WL-MA0102-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

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Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

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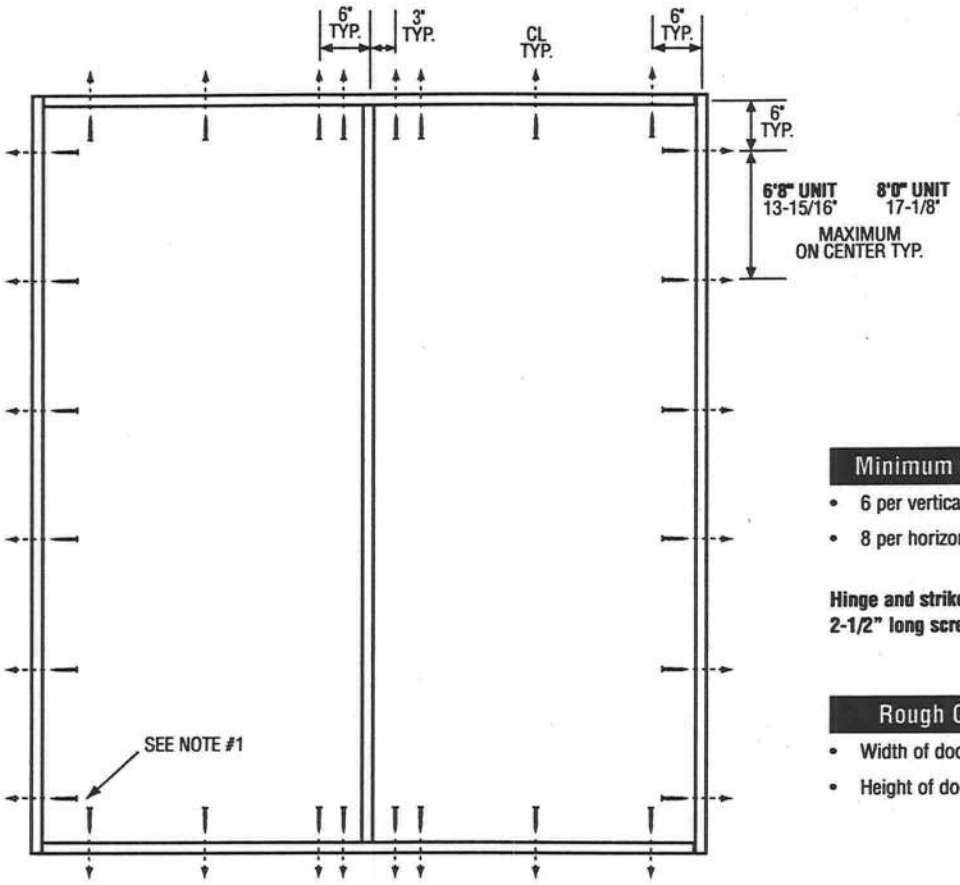
XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0247*, 0267*, 3242*, 3247, 3262* or 3267**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 10, 2003
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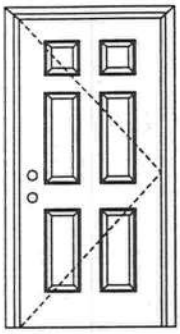
Masonite®

X
Opaque Inswing Unit

COP-WL-MA0101-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+76.0/-76.0
limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft™ Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003
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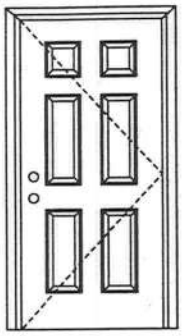
Masonite®

X
Opaque Outswing Unit

COP-WL-MA0121-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

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Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+76.0/-76.0
limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0011-02.

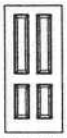
MINIMUM INSTALLATION DETAIL:

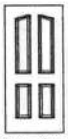
Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

Masonite®

X
Opaque Outswing Unit

COP-WL-MA0121-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

NCTL 210-1973-1, 2, 3

Certifying Engineer and License Number: Barry Portney, P.E. 16258

CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886
COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey WH®

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003
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Masonite®

X
Opaque Inswing Unit

COP-WL-MA0101-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

NCTL 210-1973-1, 2, 3

Certifying Engineer and License Number: Barry Portney, P.E. 16258

CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996.

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

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MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

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Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

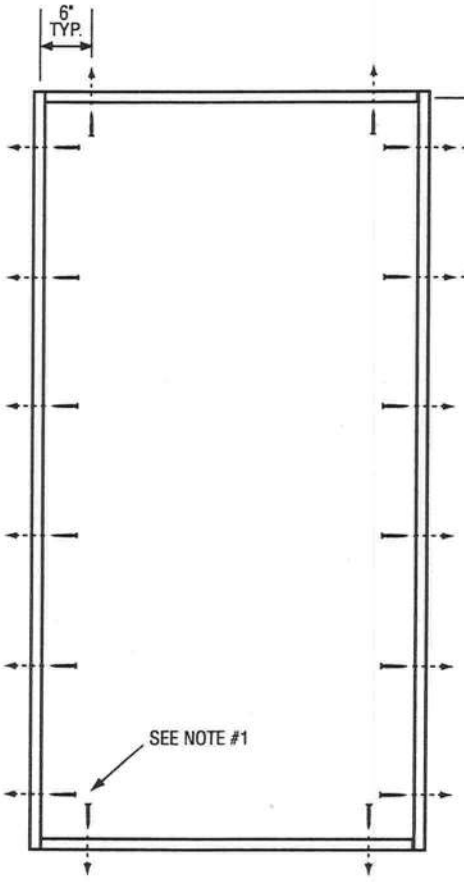
March 10, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

Masonite®

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0246*, 0266*, 3241*, 3246, 3261* or 3266**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 10, 2003

Masonite®

CAPITOL™ WINDOWS AND DOORS

- **Series 165 Single Hung and Fixed Windows**
- **Series 650 Single Hung and Fixed Windows**
- **Series 168 Horizontal Slider and Fixed Windows**
- **Series 680 Horizontal Slider and Fixed Windows**

NOTE: SEE INDIVIDUAL TEST REPORT(S) FOR DP RATINGS AND MAXIMUM ALLOWABLE SIZES.

INSTALLATION INSTRUCTIONS FOR "APPROVED FOR FLORIDA" ALUMINUM FIN WINDOWS

Capitol Windows & Doors appreciates your recent purchase of a maintenance free prime window, which will not rust, rot, mildew, or warp. This is a quality product that left our factory in good condition – proper handling and installation are just as important as good design and workmanship. Please follow these recommendations to allow this product to complete its function.

1 Handle units one at a time in the closed and locked position and take care not to scratch frame or glass or to bend the nailing fin. Place a continuous bead of caulk on the back side of nail fin (mounting flange).

2. Set unit plumb and square into opening and make sure that there is 3/16" ± 1/16" clearance around the frame. Fasten unit into opening in the closed and locked position, making sure that fasteners are screwed in straight in order to avoid twisting or bowing of the frame. Make sure that sill is straight and level. Check operation of unit frequently as fasteners are set.

3. Use # 8 sheet metal or wood screws with a minimum of 1" penetration into the framing (stud). Place first screws (two at each corner) 3" from end of fin. For positive and negative DPs (design pressures) up to **35**, do not exceed **24"** spacing of additional screws. For DPs from **35.1 to 50**, do not exceed **18"** spacing.

4. Caulk entire perimeter of fin to mounting surface joint and caulk over screw heads. Note: this step can be eliminated if 4" wide adhesive type flashing is used (sill 1st., jambs 2nd., head 3rd.).

5. Fill voids between frame and construction with loose batten type insulation or <u>non-expanding</u> aerosol foam specifically formulated for windows and doors to eliminate drafts. The use of <u>expanding</u> aerosol type insulating foam, which can bow the frame, waives all stated warranties.

6. Remove plaster, mortar, paint, and debris that has collected on the unit and make sure that sash/vent tracks and interlocks are also clean. Do not use abrasives, solvents, ammonia, vinegar, alkaline, or acid solutions for clean-up, especially with insulated glass units as their use could cause chemical breakdown of the glass seal. Take care not to scratch glass; scratches severely weaken glass and it could eventually break from thermal expansion and contraction. Clean units with water and mild detergent.

- CAUTION -

Capitol Windows & Doors or its representatives are unable to control and cannot assume responsibility for the selection and placement of their products in a building or structure in a manner required by laws, statutes, and/or building codes. The purchaser is solely responsible for knowledge of and adherence to the same. BetterBilt window products are not provided with safety glazing unless specifically ordered with such. Many laws and codes require safety glazing (tempered glass) near doors, bathtubs, and shower enclosures. Also be aware of other code requirements such as emergency egress and structural / energy performance.

Corporate Headquarters:
M.I. Home Products
650 West Market St.
Gratz, PA 17030-0370
(717) 365-3300

www.mihp.com

JULY 29, 2003

STEVEN M. URICH CERTIFICATE No. 57795 STATE OF FLORIDA PROFESSIONAL ENGINEER

Rev. 7-24-03

AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 450/650/850 Drop In Glazing
TYPE: Aluminum Single Hung Window

Title	Summary of Results
AAMA Rating	H-LC30 53 x 90
Operating Force	24 lb max.
Air Infiltration	0.11 cfm/ft²
Water Resistance Test Pressure	6.75 psf
Uniform Load Deflection Test Pressure	+32.8 psf -47.2 psf
Uniform Load Structural Test Pressure	+49.2 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-42487.01 for complete test specimen description and data.

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-42487.01
Test Date: 08/14/02
And: 08/15/02
Report Date: 10/02/02
Expiration Date: 08/15/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on a Series/Model 450/650/850 Drop In Glazing, aluminum single hung window at their facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-LC30 53 x 90 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 450/650/850 Drop In Glazing

Type: Aluminum Single Hung Window

Overall Size: 4' 5-1/8" wide by 7' 5-5/8" high

Interior Sash Size: 4' 2-3/4" wide by 3' 8-7/8" high

Fixed Daylight Opening Size: 4' 0" wide by 3' 5-3/8" high

Screen Size: 4' 0-3/4" wide by 3' 8-3/4" high

Finish: The unit was white.

Glazing Details: The specimen utilized 5/8" thick, sealed insulating glass constructed from two sheets of 3/32" thick, clear annealed glass and a metal reinforced butyl spacer system. The lites were interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.190" high by 0.187" polypile with center fin	1 Row	Fixed meeting rail interlock
0.190" high by 0.187" polypile with center fin	2 Rows	Interior sash stiles
1/4" vinyl foam-filled bulb seal	1 Row	Interior sash bottom rail
5/8" wide by 7/8" long polypile plug	4 Pieces	Interior sash, all corners

Frame Construction: The frame was constructed of extruded aluminum. Each corner was coped, butted, sealed, and fastened with two #8 x 1" screws per corner through the head and sill into jamb screw boss. End caps were utilized on the ends of the meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was then secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum. Each corner was coped, butted, and fastened with one #8 x 1-1/4" screw per corner.

Screen Construction: The screen was constructed of roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock	2	Interior sash, 6-1/2" from top rail ends
Spring-loaded coil balance	2	One per jamb
Plastic tilt latch	2	Interior sash top rail ends
Metal tilt latch pin	2	Interior sash bottom rail ends
Screen spring-loaded retainer pin	2	6-3/4" from rails on stiles

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The specimen was installed into a #2 2 x 8 Spruce-Pine-Fir wood buck. #8 x 1-5/8" drywall screws were placed 3" from corners and 15" on center around nailing fin. Polyurethane was used a s a sealant around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	24 lbs	35 lbs max.
2.1.2	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.11 cfm/ft²	0.3 cfm/ft² max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 3.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	@ 25.0 psf (positive)	0.64"*	0.29" max.
	@ 25.0 psf (negative)	0.54"*	0.29" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 37.5 psf (positive)	0.04"	0.20" max.
	@ 37.5 psf (negative)	0.03"	0.20" max.

**Exceeds L/175 for deflection, but meets all other test requirements.*

Test Results:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987-88)		
	In operating direction at 70 lbs		
	Interior sash meeting rail	0.12"/25%	0.50"/100%
	Interior sash bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Interior sash right stile	0.06"/12%	0.50"/100%
	Interior sash left stile	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 32.8 psf (positive)	0.85"*	0.29" max.
	@ 47.2 psf (negative)	0.87"*	0.29" max.
4.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 49.2 psf (positive)	0.09"	0.20" max.
	@ 70.8 psf (negative)	0.12"	0.20" max.

**Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:mlb
01-42487.01

Allen. N. Reeves. P.E.
Director - Engineering Services
11 OCTOBER 2002

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650
TYPE: Aluminum Picture Window

Title of Test	Results
Rating	F-R45 60 x 80
Overall Design Pressure	+45.0 psf -47.2 psf
Air Infiltration	0.04 cfm/ft²
Water Resistance	8.25 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41135.01 dated 03-26-02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess

Mark A. Hess, Technician

MAH:mlf

Allen N. Reams
1 APRIL 2002

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41135.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650, aluminum picture window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a F-R45 60 x 80 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650

Type: Aluminum Picture Window

Overall Size: 5' 0" wide by 6' 8" high

Daylight Opening Size: 4' 9-1/4" wide by 6' 5-1/4" high

Finish: All aluminum was white.

Glazing Details: The test specimen utilized 7/8" thick, sealed insulating glass constructed from two sheets of 3/16" thick, clear annealed glass and a metal reinforced butyl spacer system. The glass was interior glazed against double-sided adhesive foam tape and secured with aluminum snap-in glazing beads.

130 Derry Court
York, PA 17402-9409
phone: 717-764-7700
fax: 717-764-4129
www.architest.com

Allen N. Reum
1 APRIL 2002

Test Specimen Description: (Continued)

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss.

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck. #8 x 2-1/2" installation screws were utilized 18" on center around the interior perimeter. Polyurethane was utilized to seal the exterior.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.2	Air Infiltration (ASTM E 283-91)		
	@ 1.57 psf (25 mph)	0.04 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101 I.S. 2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-00)		
	WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97)		
	(Measurements reported were taken on the jamb)		
	(Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.01"	0.41" max.
	@ 34.7 psf (negative)	0.01"	0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97)		
	(Measurements reported were taken on the jamb)		
	(Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.0"	0.29" max.
	@ 52.1 psf (negative)	0.01"	0.29" max.

Allen N. Reeves
1 APRIL 2002

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance (ASTM F 588-97)		
	Type: D Grade: 10		
	Hand and Tool Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) WTP – 8.25 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.02"	0.41" max.
	@ 47.2 psf (negative)	0.02"	0.41" max.
4.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.01"	0.29" max.
	@ 70.8 psf (negative)	0.02"	0.29" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:nlb
01-41135.01

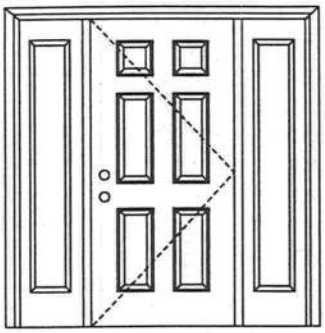
Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

OXO
Opaque Outswing Unit

COP-WL-MA0124-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Single Door with 2 Sidelites
Maximum unit size = 5'4" x 6'8"

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the iTS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Design Pressure

+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED on opaque panel, but is required on glazed panels.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0014-02 or MAD-WL-MA0017-02 and MAD-WL-MA0041-02.

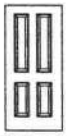
MINIMUM INSTALLATION DETAIL:

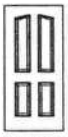
Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003
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OXO
Opaque Outswing Unit

COP-WL-MA0124-02

FIBERGLASS DOORS

APPROVED SIDELITE STYLES:


129 Series


200 Series


12R, 12L, 23R, 23L, 24R, 24L Series


450 Series


152 Series


149 Series


109 Series


120, 122 Series


300 Series

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panel glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

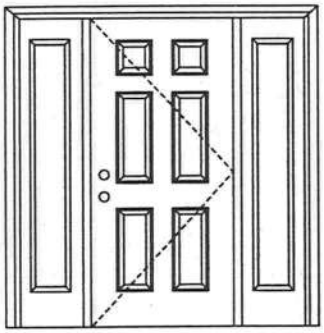
Masonite®

OXO
Opaque Inswing Unit

COP-WL-MA0104-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Single Door with 2 Sidelites
Maximum unit size = 5'4" x 6'8"

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Design Pressure

+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED on opaque panel, but is required on glazed panels.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0004-02 or MAD-WL-MA0007-02 and MAD-WL-MA0041-02.

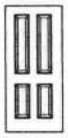
MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

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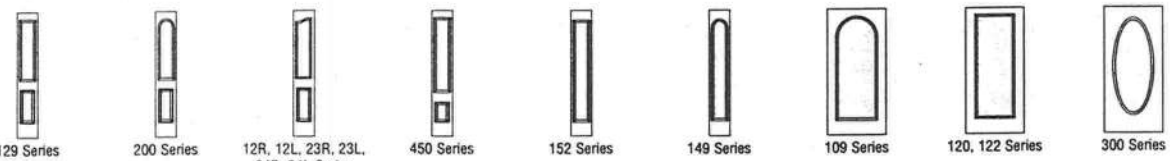
OXO
Opaque Inswing Unit

COP-WL-MA0104-02

FIBERGLASS DOORS

APPROVED SIDELITE STYLES:



129 Series | 200 Series | 12R, 12L, 23R, 23L, 24R, 24L Series | 450 Series | 152 Series | 149 Series | 109 Series | 120, 122 Series | 300 Series

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panel glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

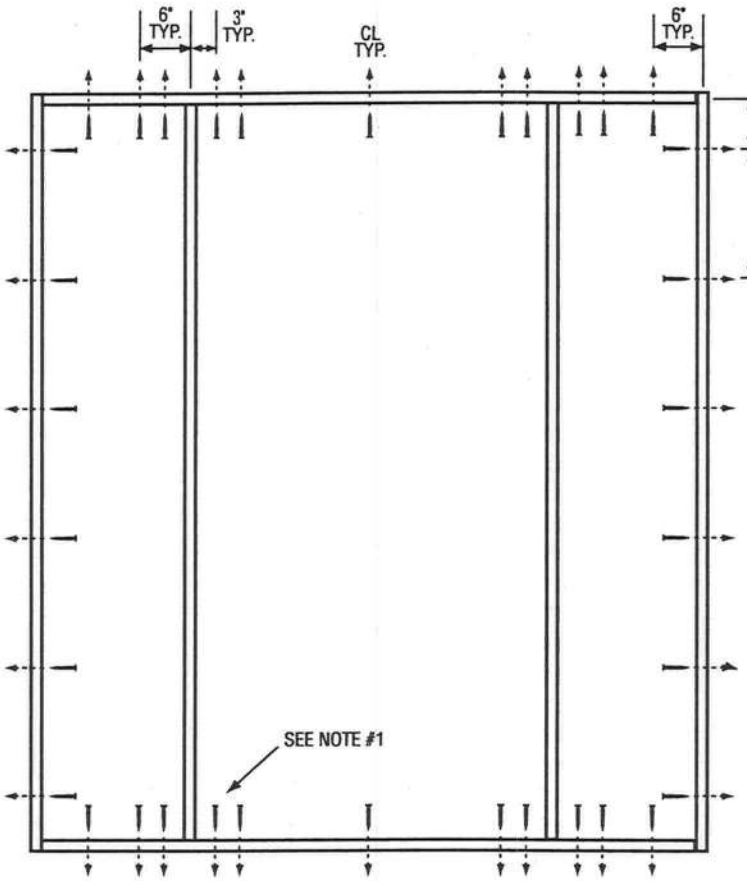
March 10, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

Masonite®

OXO
Unit

MID-WL-MA0004-02

SINGLE DOOR WITH 2 SIDELITES



Minimum Fastener Count

- 6 per vertical framing member
- 11 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0249*, 0269*, 3244*, 3249, 3264* or 3269**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Mrach 10, 2003

Masonite®

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC

SERIES/MODEL: 420/430/440
PRODUCT TYPE: Aluminum Sliding Glass Door

	Summary of Results		
Title	**Test Specimen #1**	**Test Specimen #2**	**Test Specimen #3**
Rating	SGD-R25 182 x 96	SGD-R35 182 x 80	SGD-R40 144 x 96
Operating Force	17 lbf max.	17 lbf max.	N/A
Air Infiltration	0.23 cfm/ft^2	0.27 cfm/ft^2	N/A
Water Resistance Test Pressure	3.75/6.0/9.0 psf	6.0 psf	N/A
Uniform Load Deflection Test Pressure	±35.0 psf	±35.0 psf	+40.0 psf/-40.1 psf
Uniform Load Structural Test Pressure	±37.5 psf	±52.5 psf	+60.0 psf/-60.2 psf
Forced Entry Resistance	Grade 10	Grade 10	N/A

Reference should be made to ATI Report No. 52112.01-122-47 for complete test specimen description and data.

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No.:	52112.01-122-47
Revision 1:	09/13/04
Test Dates:	06/30/04
Through:	08/12/04
Report Date:	08/30/04
Expiration Date:	07/02/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on three Series/Model 420/430/440, aluminum sliding glass doors at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: SGD-R25 182 x 96; Test Specimen #2: SGD-R35 182 x 80; Test Specimen #3: SGD-R40 144 x 96. Test specimen description and results are reported herein.

Test Specification: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 420/430/440

Product Type: Aluminum Sliding Glass Door

Test Specimen #1: SGD-R25 182 x 96 (XXO)

Overall Size: 15' 1-3/4" wide by 8' 0" high

Active Door Panel Size (2): 5' 0-1/2" wide by 7' 11" high

Fixed Door Panel Size: 5' 1" wide by 7' 11" high

Screen Size: 5' 0-3/8" wide by 7' 11" high

Overall Area: 121.2 ft^2

Reinforcement: The active and fixed interlocking stile utilized a steel U-shaped reinforcement (Drawing #9917525). The fixed intermediate jamb utilized a steel reinforcement (Drawing #9917520).

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Test Specimen Description: (Continued)

Test Specimen #2: SGD-R35 182 x 80 (OXX)

Overall Size: 15' 1-3/4" wide by 6' 8" high

Active Door Panel Size (2): 5' 0-1/2" wide by 6' 7" high

Fixed Door Panel Size: 4' 8-7/8" wide by 6' 2-5/8" high

Screen Size: 5' 0-3/8" wide by 6' 7" high

Overall Area: 101 ft^2

Reinforcement: No reinforcement was utilized.

Test Specimen #3: SGD-R40 144 x 96 (XOX)

Overall Size: 12' 0" wide by 8' 0" high

Active Door Panel Size: 3' 8-1/4" wide by 7' 10-1/2" high

Fixed Door Panel Size (2): 3' 8-3/4" wide by 7' 6-1/2" high

Screen Size: 3' 11-1/2" wide by 7' 11-3/8" high

Overall Area: 96 ft^2

Reinforcement: The active and fixed interlocking stile utilized a steel U-shaped reinforcement (Drawing #9917525). The fixed intermediate jamb utilized a steel reinforcement (Drawing #9917520). The interlock utilized an aluminum reinforcement (Drawing #SECT4237).

The following descriptions apply to all specimens.

Finish: All aluminum was white.

Glazing Details: All glazing consisted of a single sheet of 3/16" thick clear tempered glass that was channel glazed with a wrap around rubber gasket.

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.187" backed by 0.270" high polypile with center fin	2 Rows	Stiles

Frame Construction: The frame was constructed of extruded aluminum. Corners were coped, butted, sealed, and fastened with two #8 by 5/8" screws.

Door Panel Construction: The door panels were constructed of extruded aluminum members. Corners were coped, butted, and fastened with one 1/4" by 3/4" screw at the bottom and two #8 by 3/4" screws at the top.

Screen Construction: The screen was constructed of extruded aluminum members. Corners were coped, butted, and fastened with one 1/4" by 3/4" and one #8 by 1" screw at the bottom and one #8 by 1" screw at the top.

Hardware:

Description	Quantity	Location
Locking handle	1	44" from active panel bottom
Roller assembly	2	3" from bottom rail ends
Screen locking handle	1	46" from screen bottom rail

Drainage:

Description	Quantity	Location
Sloped sill	1	Sill

Installation: The units were installed into a #2 Spruce-Pine-Fir wood test buck. The units were fastened to the test buck with two rows of #8 by 1-1/4" screws, 8" from each end and 23" on center. The exterior perimeter was sealed with silicone.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO)			
2.2.1.6.1	Operating Force	17 lbf	20 lbf max.
	Breakaway force	24 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283		
	1.57 psf (25 mph)	0.23 cfm/ft^2	0.3 cfm/ft^2 max.
2.1.3	Water Resistance per ASTM E 547 (with and without screen)		
	2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330		
	(Deflections reported were taken on the meeting rail)		
	(Loads were held for 52 seconds)		
	15.0 psf (positive)	0.56"	See Note #2
	15.0 psf (negative)	0.57"	See Note #2
2.1.4.2	Uniform Load Structural per ASTM E 330		
	(Permanent sets reported were taken on the meeting stile)		
	(Loads were held for 10 seconds)		
	22.5 psf (positive)	0.02"	0.30" max.
	22.5 psf (negative)	0.03"	0.30" max.
2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction - 70 lbs		
	Locking stile	0.12"/24%	0.50"/100%
	Interlock stile	0.12"/24%	0.50"/100%

***Note #1**: The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*

***Note #2**: The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO) (Continued)			
2.2.1.6.2	Deglazing Test per ASTM E 987 In remaining direction - 50 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 842		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 through A6	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) 3.75 psf	No leakage	No leakage
4.3	Water Resistance per ASTM E 547 (with and without screen) (with sill riser) 6.0 psf	No leakage	No leakage
4.3	Water Resistance per ASTM E 547 (with and without screen) (with 2-5/8" Dade County sill extension) 9.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	35.0 psf (positive)	2.98"	See Note #2
	35.0 psf (negative)	2.52"	See Note #2

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO) (Continued)			
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	37.5 psf (positive)	0.20"	0.36" max.
	37.5 psf (negative)	0.19"	0.36" max.
Test Specimen #2: SGD-R35 182 x 80 (OXX)			
2.2.1.6.1	Operating Force	17 lbf	20 lbf max.
	Breakaway force	21 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.27 cfm/ft^2	0.3 cfm/ft^2 max.
Note #1: *The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*			
2.1.3	Water Resistance per ASTM E 547 (with and without screen) 2.86 psf	No leakage	No leakage
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Locking stile	0.12"/24%	0.50"/100%
	Interlock stile	0.12"/24%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 842		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 through A6	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #2: SGD-R35 182 x 80 (OXX) (Continued)			
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) (with sill riser) 6.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	35.0 psf (positive)	1.28"	See Note #2
	35.0 psf (negative)	1.33"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	52.5 psf (positive)	0.13"	0.30" max.
	52.5 psf (negative)	0.15"	0.30" max.
Test Specimen #3: SGD-R40 144 x 96 (XOX)			
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	40.0 psf (positive)	1.42"	See Note #2
	40.1 psf (negative)	1.28"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	60.0 psf (positive)	0.27"	0.37" max.
	60.2 psf (negative)	0.30"	0.37" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess vlm

Digitally Signed for: Mark A. Hess by Vicki L. McElwain

Mark Hess
Technician

MH:vlm

Digitally Signed by: Steven M. Urich

Steven M. Urich, P.E.
Senior Project Engineer

MI WINDOWS AND DOORS, INC.

420 / 430 / 440 SERIES ALUMINUM SLIDING GLASS DOOR

COMAPARATIVE ANALYSIS CHART IN DESIGN PRESSURE

STEEL AND ALUMINUM REINFORCING

09/08/2004

SGD ALUM & STL REINF

PANEL WIDTH >>	24	30	36	48
PANEL HEIGHT 80	85	71	62	51
96	69	57	49	40

TEST REPORT NO: ATI-52112.01-122-47
DESIGN PRESSURE ACHIEVED IN TEST: POS. & NEG. 40.0 PSF
WATER TEST PRESSURE:
- 1-3/8 IN. SILL RISER: 3.75 PSF
- 1-7/8 IN. SILL RISER: 6.0 PSF
- 2-5/8 IN. SILL RISER: 9.0 PSF

OVERALL TEST SIZE: 12'-0" X 8'-0" NOMINAL

OVERALL PANEL SIZE: 48 IN. X 96 IN. NOMINAL
GLAZING: SINGLE PC. OF 3/16 IN. THK. TEMPERED GLASS
REINFORCING: STEEL IN INTERLOCKING STILES AND INTERMEDIATE JAMB. ADDITIONAL ALUM. REINFORCING ON EXTERIOR OF OPERATING INTERLOCK STILE.
CONFIGURATION: XOX

LIMITATIONS:
THE ABOVE ARE POSITIVE AND NEGATIVE STRUCTURAL DESIGN LOADS FROM COMPARATIVE ANALYSIS & HAVE NOT BEEN CAPPED BY RESULTS OF WATER PERFORMANCE TESTING.
WHERE LOCAL CODE REQUIRES WATER RESISTANCE TESTING TO PASS A MIN. 15% OF DESIGN PRESSURE, ALLOWABLE POSITIVE DESIGN PRESSURE WOULD BE CAPPED AS FOLLOWS:
- WHERE 1-3/8 IN. SILL RISER IS EMPLOYED POSITIVE DESIGN PRESSURE IS CAPPED AT 25.0 PSF.
- WHERE 1-7/8 IN. SILL RISER IS EMPLOYED POSITIVE DESIGN PRESSURE IS CAPPED AT 40.0 PSF.
- WHERE 2-5/8 IN. SILL RISER IS EMPLOYED POSITIVE DESIGN PRESSURE IS CAPPED AT 60.0 PSF.

PANEL WIDTHS AND HEIGHTS ARE NOMINAL, IN INCHES.

PREPARED BY:
PRODUCT TECHNOLOGY CORPORATION
1150 LOUISIANA AVENUE, SUITE 6
WINTER PARK, FLORIDA 32789
PHONE 407 622-6334 FAX 407 622.6335
www.ptc-corp.com

MI WINDOWS AND DOORS, INC.

420 / 430 / 440 SERIES ALUMNUM SLIDING GLASS DOOR

09/08/2004

SGD non-Reinf

COMAPARATIVE ANALYSIS CHART IN DESIGN PRESSURE

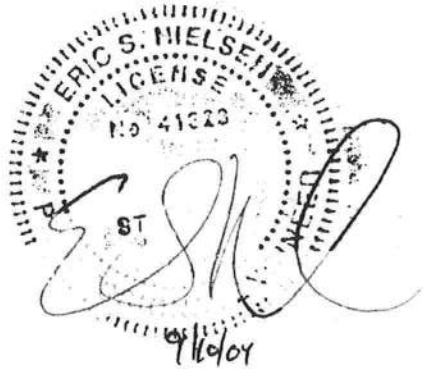
PANEL WIDTH >>	24	30	36	48	60
PANEL HEIGHT 80	64	54	47	39	35

TEST REPORT NO: ATI-52112.01-122-47
DESIGN PRESSURE ACHIEVED IN TEST: POS. & NEG. 35.0 PSF
WATER TEST PRESSURE:
1-3/8 IN. SILL RISER: 3.75 PSF
1-7/8 IN. SILL RISER: 6.0 PSF
2-5/8 IN. SILL RISER: 9.0 PSF

OVERALL SIZE TESTED: 15'-0" X 6'-8" NOMINAL
OVERALL PANEL SIZE TESTED: 5'-0" X 6'-8" NOMINAL
GLAZING: SINGLE PC. OF 3/16 IN THICK TEMP. GLASS
REINFORCING: NONE
CONFIGURATION TESTED: XXO

LIMITATIONS:
THE ABOVE ARE POSITIVE AND NEGATIVE STRUCTURAL DESIGN LOADS FROM COMPARATIVE ANALYSIS & HAVE NOT BEEN CAPPED BY RESULTS OF WATER PERFORMANCE TESTING.
WHERE LOCAL CODE REQUIRES WATER RESISTANCE TESTING TO PASS A MIN. 15% OF DESIGN PRESSURE, ALLOWABLE POSITIVE DESIGN PRESSURE WOULD BE CAPPED AS FOLLOWS:
WHERE 1-3/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURE = 25.0 PSF
WHERE 1-7/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURE = 40.0PSF
WHERE 2-5/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURE = 60.0 PSF
PANEL WIDTHS AND HEIGHTS ARE NOMINAL, IN INCHES.

PREPARED BY:
PRODUCT TECHNOLOGY CORPORATION
1150 LOUISIANA AVENUE, SUITE 6
WINTER PARK, FLORIDA 32789
PHONE 407 622-6334 FAX 407 622-6335
www.ptc-corp.com



MI WINDOWS AND DOORS, INC.

420 / 430 / 440 SERIES ALUMINUM SLIDING GLASS DOOR | STEEL REINFORCED | 09/08/2004

COMAPARATIVE ANALYSIS CHART IN DESIGN PRESSURE | SGD STL REINF

PANEL WIDTH >>	24	30	36	48	60
PANEL HEIGHT 80	61	51	44	37	33
96	49	41	35	29	25

TEST REPORT NO: ATI-52112.01-122-47
DESIGN PRESSURE ACHIEVED IN TEST: POS. & NEG. 25.0 PSF
WATER TEST PRESSURE:
- 1-3/8 IN. SILL RISER: 3.75 PSF
- 1-7/8 IN. SILL RISER: 6.0 PSF
- 2-5/8 IN. SILL RISER: 9.0 PSF

OVERALL SIZE TESTED: 15'-0" X 8'-0" NOMINAL
OVERALL PANEL SIZE TESTED: 60 IN. X 96 IN. NOMINAL
GLAZING: SINGLE PC. OF 3/16 IN. THK. TEMPERED GLASS
REINFORCING: STEEL IN INTERLOCKING STILES, AND FIXED INTERMEDIATE JAMB
CONFIGURATION TESTED: OXX

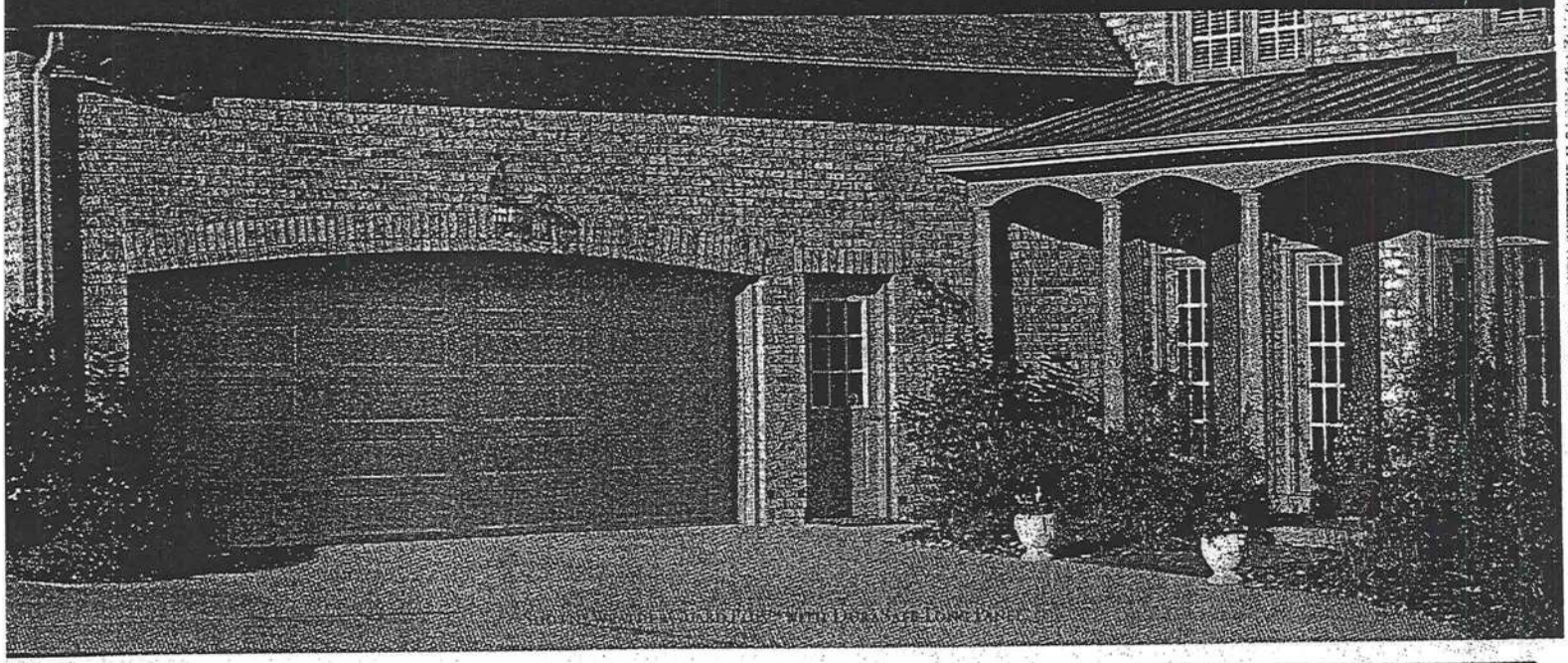
LIMITATIONS:
THE ABOVE ARE POSITIVE AND NEGATIVE STRUCTURAL DESIGN LOADS FROM COMPARATIVE ANALYSIS & HAVE NOT BEEN CAPPED BY RESULTS OF WATER PERFORMANCE TESTING.
WHERE LOCAL CODE REQUIRES WATER RESISTANCE TESTING TO PASS A MIN. 15% OF DESIGN PRESSURE, ALLOWABLE POSITIVE DESIGN PRESSURE WOULD BE CAPPED AS FOLLOWS:
- WHERE 1-3/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURES ARE CAPPED AT 25.0 PSF.
- WHERE 1-7/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURES ARE CAPPED AT 40.0 PSF.
- WHERE 2-5/8 IN. SILL RISER IS EMPLOYED, POSITIVE DESIGN PRESSURES ARE CAPPED AT 60.0 PSF.

PANEL WIDTHS AND HEIGHTS ARE NOMINAL, IN INCHES.

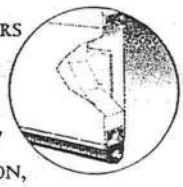
PREPARED BY:
PRODUCT TECHNOLOGY CORPORATION
1150 LOUISIANA AVENUE, SUITE 6
WINTER PARK, FLORIDA 32789
PHONE 407 622-6334 FAX 407 622.6335
www.ptc-corp.com

Amarr GARAGE DOORS

BEST | WEATHERGUARD™ SERIES
FEATURING OUR **DuraSafe System**



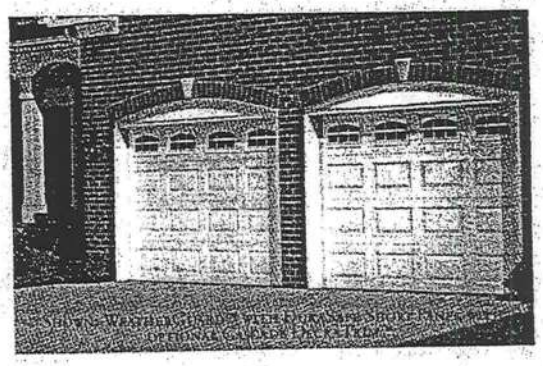
WEATHERGUARD PLUS™ WITH **DuraSafe**



THE WEATHERGUARD PLUS OFFERS DISCERNING HOMEOWNERS A MASTERFUL COMBINATION OF PREMIUM FEATURES. SUPERIOR TRIPLE-LAYER CONSTRUCTION, 2" (5.1 CM) POLYSTYRENE INSULATION, AN R-VALUE OF 8.34, AND UNMATCHED BEAUTY PUT THE WEATHERGUARD PLUS AT THE TOP OF ITS CLASS.

WEATHERGUARD™ WITH **DuraSafe**

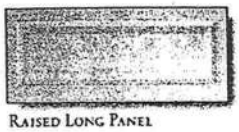
TOP-QUALITY TRIPLE-LAYER CONSTRUCTION AND 1 3/8" (3.5 CM) POLYSTYRENE INSULATION MAKE OUR WEATHERGUARD STEEL DOOR STRONG, QUIET, AND ENERGY EFFICIENT. FEATURING AN R-VALUE OF 5.73, THE WEATHERGUARD IS THE PERFECT ADDITION TO YOUR HOME FOR YEARS OF TROUBLE FREE SERVICE AND GREAT LOOKS.



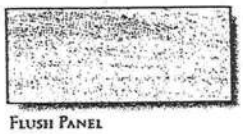
DESIGN ELEMENTS

THE WEATHERGUARD SERIES DOORS ARE AVAILABLE WITH A RAISED SHORT, RAISED LONG, OR FLUSH PANEL DESIGN IN YOUR CHOICE OF FOUR COLORS.*

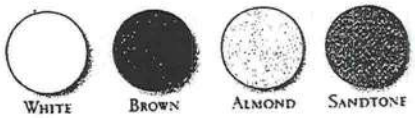
RAISED SHORT PANEL



RAISED LONG PANEL



FLUSH PANEL

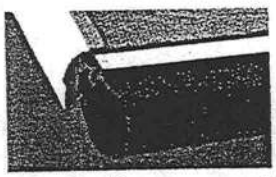


WHITE BROWN ALMOND SANDTONE

* ACTUAL PAINT COLORS MAY VARY FROM SAMPLES SHOWN.

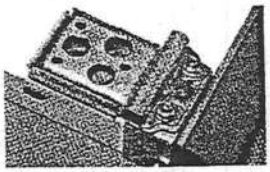
Bottom Seal

NEW ALUMINUM BOTTOM SEAL MEANS EASY AND FAST INSTALLATION AND MAINTENANCE... AS WELL AS A BETTER SEAL AGAINST THE ELEMENTS.



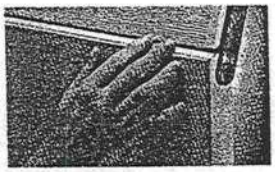
Bottom Bracket

NEW TAMPER RESISTANT BOTTOM BRACKET HELPS PREVENT ACCIDENTS, YET ALLOWS FOR ROLLER MAINTENANCE/CHANGE WITHOUT DISASSEMBLY. FULL LENGTH ROLLER TUBE PREVENTS SLIP-OUTS.



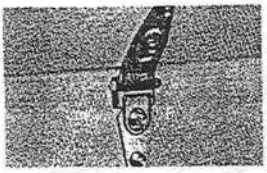
Door Sections

THE SECTION JOINT OF THE FUTURE: TODAY. NEW SECTION PROFILE ASSURES PINCH RESISTANCE BOTH INSIDE AND OUT, EXCEEDING INDUSTRY STANDARDS - NEITHER FINGERS NOR WEATHER GETS IN.



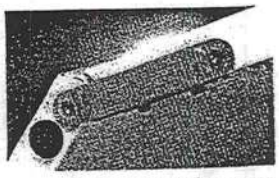
Center Hinge

FLUSH MOUNT INBOARD DESIGN CENTER HINGES PROVIDE PINCH RESISTANT PROTECTION AND A LOW PROFILE CLEAN LOOK ON THE INSIDE OF THE DOOR.



End Hinge

WITH MOST OF ITS ACTION HIDDEN INSIDE THE DOOR, OUR RE-ENGINEERED END HINGES LEAVE NO ROOM FOR EVEN THE SMALLEST FINGERS.

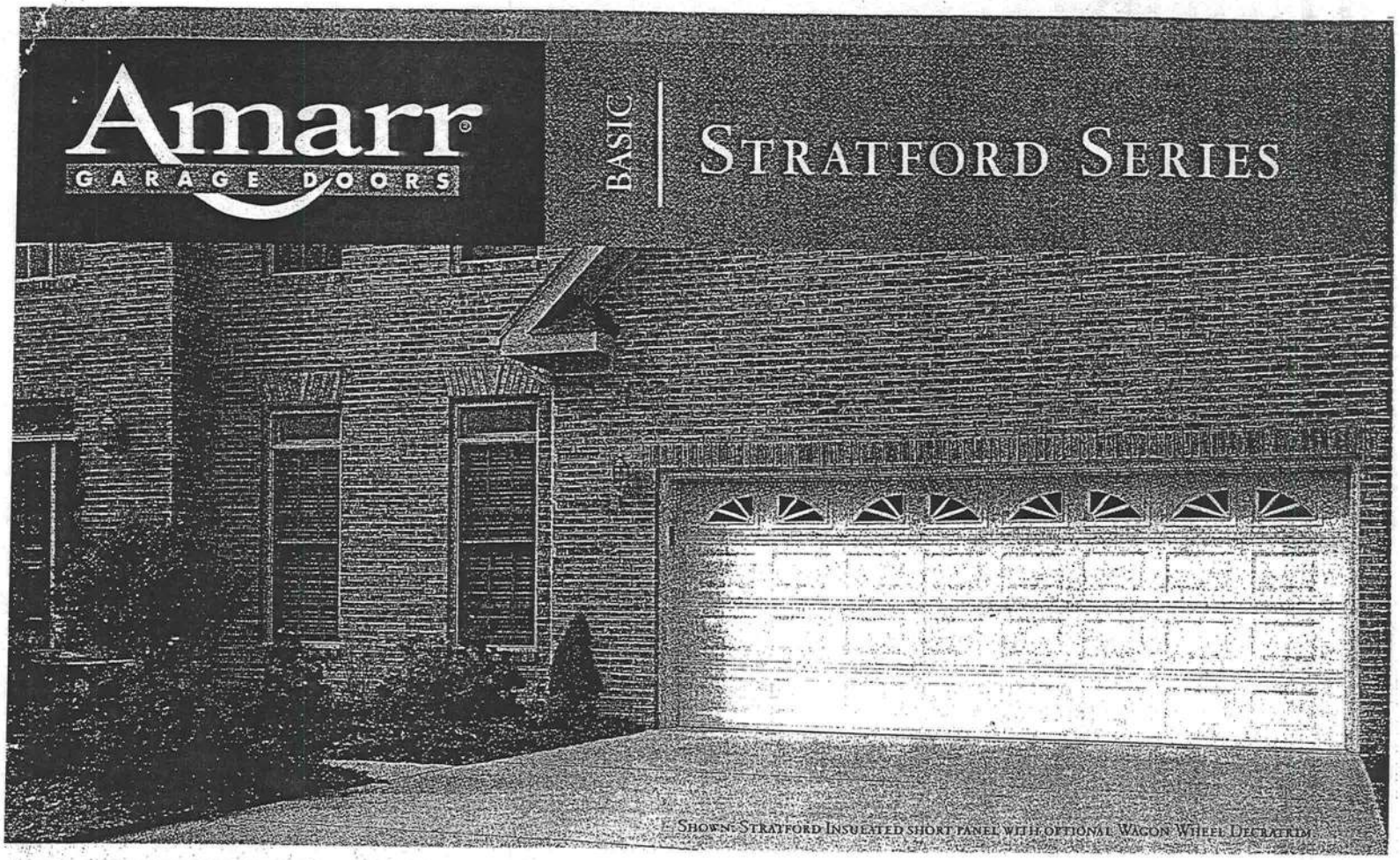


AMARR DURASAFE DOORS UNDER 8'9" WILL BE SUPPLIED WITH DURASAFE HARDWARE. DASMA STANDARDS FOR PINCH-RESISTANCE DO NOT APPLY TO DOORS OVER 8' HIGH SINCE THE POTENTIAL PINCH POINTS ARE ABOVE TYPICAL GRASPING HEIGHTS; AMARR DOORS OVER 8'9" ARE SUPPLIED WITH CONVENTIONAL HARDWARE. THE BOTTOM BRACKET, DOOR SECTIONS, CENTER HINGE AND END HINGE SHOWN ABOVE ARE PATENTED. DOORS SHOWN ARE ELECTRICALLY OPERATED. NON-ELECTRICALLY OPERATED DOORS SHOULD HAVE EXTERIOR AND INTERIOR LIFT HANDLES ATTACHED TO THE DOOR.

Amarr®
GARAGE DOORS

BASIC

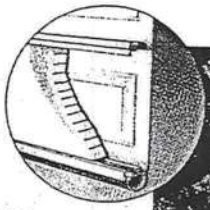
STRATFORD SERIES



SHOWN: STRATFORD INSULATED SHORT PANEL WITH OPTIONAL WAGON WHEEL DECRATRIM

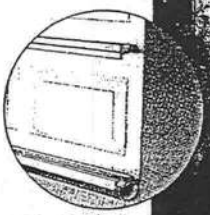
STRATFORD INSULATED

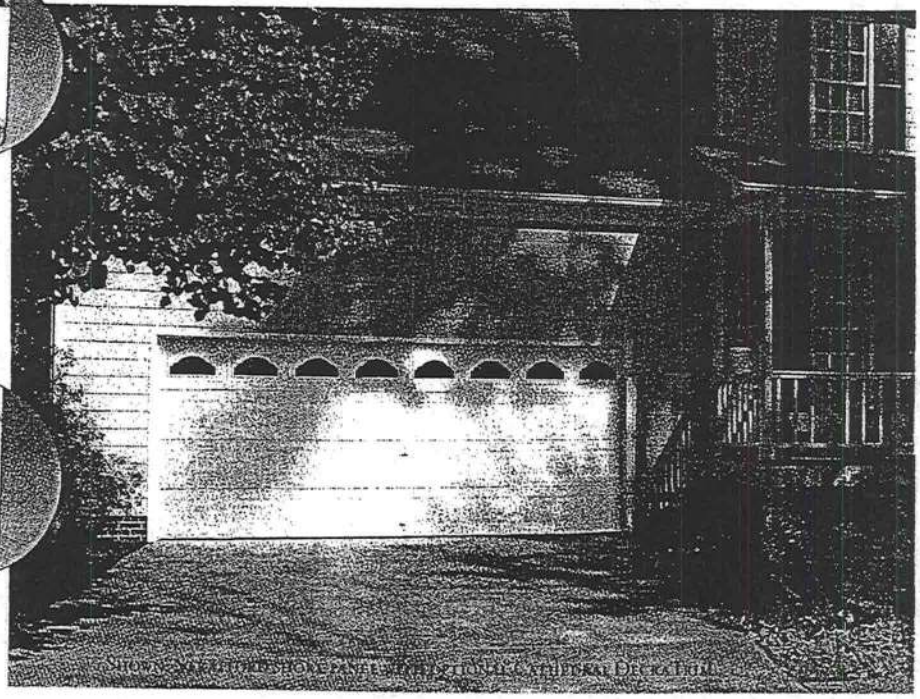
THE 2" (5.1 CM) THICK STRATFORD INSULATED PROVIDES HOMEOWNERS EXCELLENT THERMAL PROTECTION AND HANDSOME GOOD LOOKS. FEATURES INCLUDE DOUBLE-LAYER CONSTRUCTION OF STURDY 25-GAUGE STEEL, AND 1 7/16" (3.7 CM) POLYSTYRENE INSULATION WITH LAMINATED BACKING AND AN R-VALUE OF 5.65.



STRATFORD

A SUPERLATIVE ADDITION TO ANY HOME, THE STRATFORD'S DURABLE SINGLE-LAYER CONSTRUCTION, 25-GAUGE STEEL, AND ATTRACTIVE DESIGN PROVIDE HOMEOWNERS WITH EXCEPTIONAL VALUE.

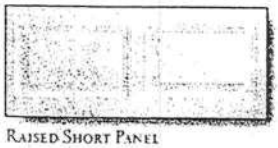




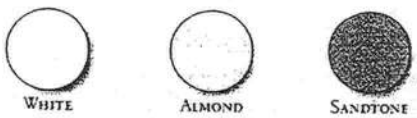
SHOWN: STRATFORD SHORT PANEL WITH OPTIONAL CATHEDRAL DECRATRIM

DESIGN ELEMENTS

THE STRATFORD SERIES DOORS ARE AVAILABLE WITH A RAISED SHORT PANEL DESIGN IN YOUR CHOICE OF THREE COLORS.*



RAISED SHORT PANEL



WHITE ALMOND SANDTONE

* ACTUAL PAINT COLORS MAY VARY FROM SAMPLES SHOWN.

Residential System Sizing Calculation

Summary

William & Janice Daugherty
Suwannee Valley Rd.
Lake City, FL

Project Title:
Daugherty Residence

Code Only
Professional Version
Climate: North

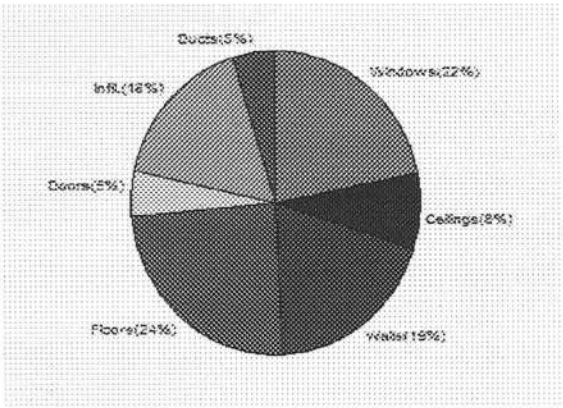
8/2/2005

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**29606**	**Btuh**	**Total cooling load calculation**	**29726**	**Btuh**
Submitted heating capacity	30000	Btuh	Submitted cooling capacity	30000	Btuh
Submitted as % of calculated	101.3	%	Submitted as % of calculated	100.9	%

WINTER CALCULATIONS

Winter Heating Load (for 1890 sqft)

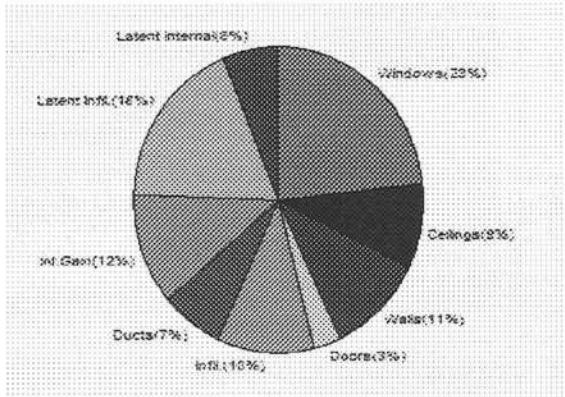
Load component			Load	
Window total	230	sqft	6509	Btuh
Wall total	1825	sqft	5658	Btuh
Door total	93	sqft	1494	Btuh
Ceiling total	1890	sqft	2457	Btuh
Floor total	228	ft	7205	Btuh
Infiltration	114	cfm	4875	Btuh
Subtotal			**28197**	**Btuh**
Duct loss			1410	Btuh
TOTAL HEAT LOSS			**29606**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1890 sqft)

Load component			Load	
Window total	230	sqft	6944	Btuh
Wall total	1825	sqft	3176	Btuh
Door total	93	sqft	928	Btuh
Ceiling total	1890	sqft	2684	Btuh
Floor total			0	Btuh
Infiltration	156	cfm	3093	Btuh
Internal gain			3600	Btuh
Subtotal(sensible)			**20425**	**Btuh**
Duct gain			2042	Btuh
Total sensible gain			**22467**	**Btuh**
Latent gain(infiltration)			5418	Btuh
Latent gain(internal)			1840	Btuh
Total latent gain			**7258**	**Btuh**
TOTAL HEAT GAIN			**29726**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY:
DATE: 8-02-05

System Sizing Calculations - Winter

Residential Load - Component Details

William & Janice Daugherty
Suwannee Valley Rd.
Lake City, FL

Project Title:
Daugherty Residence

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

8/2/2005

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	72.0	28.3	2038 Btuh
2	2, Clear, Metal, DEF	N	42.0	28.3	1189 Btuh
3	2, Clear, Metal, DEF	N	6.0	28.3	170 Btuh
4	2, Clear, Metal, DEF	N	12.5	28.3	354 Btuh
5	2, Clear, Metal, DEF	S	30.0	28.3	849 Btuh
6	2, Clear, Metal, DEF	SW	12.5	28.3	354 Btuh
7	2, Clear, Metal, DEF	S	18.3	28.3	519 Btuh
8	2, Clear, Metal, DEF	W	36.7	28.3	1038 Btuh
	Window Total		230		6509 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Exterior	13.0	1825	3.1	5658 Btuh
	Wall Total		1825		5658 Btuh
Doors	Type		Area X	HTM=	Load
1	Wood - Exter		20	17.9	359 Btuh
2	Wood - Adjac		20	9.2	184 Btuh
3	Wood - Exter		53	17.9	951 Btuh
	Door Total		93		1494Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1890	1.3	2457 Btuh
	Ceiling Total		1890		2457Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	228.0 ft(p)	31.6	7205 Btuh
	Floor Total		228		7205 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	17010(sqft)	114	4875 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			114	4875 Btuh

Totals for Heating	**Subtotal**	**28197 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1410 Btuh**
	Total Btuh Loss	**29606 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

Manual J Summer Calculations

Residential Load - Component Details (continued)

William & Janice Daugherty
Suwannee Valley Rd.
Lake City, FL

Project Title:
Daugherty Residence

Code Only
Professional Version
Climate: North

8/2/2005

Totals for Cooling	**Subtotal**	**20425**	**Btuh**
	Duct gain(using duct multiplier of 0.10)	**2042**	**Btuh**
	Total sensible gain	**22467**	**Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**5418**	**Btuh**
	Latent occupant gain (8 people @ 230 Btuh per person)	**1840**	**Btuh**
	Latent other gain	**0**	**Btuh**
	TOTAL GAIN	**29726**	**Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

System Sizing Calculations - Summer

Residential Load - Component Details

William & Janice Daugherty
Suwannee Valley Rd.
Lake City, FL

Project Title:
Daugherty Residence

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F 8/2/2005

Window	Type		Overhang Len	Overhang Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	HTM Unshaded	Load	
	Panes/SHGC/U/InSh/ExSh	Ornt									
1	2, Clear, DEF, N, N	N	1.5	6.5	72.0	0.0	72.0	22	22	1584	Btuh
2	2, Clear, DEF, N, N	N	12	7.5	42.0	0.0	42.0	22	22	924	Btuh
3	2, Clear, DEF, N, N	N	1.5	4	6.0	0.0	6.0	22	22	132	Btuh
4	2, Clear, DEF, N, N	N	3	5.5	12.5	0.0	12.5	22	22	275	Btuh
5	2, Clear, DEF, N, N	S	1.5	6	30.0	30.0	0.0	22	37	660	Btuh
6	2, Clear, DEF, N, N	SW	3	6	12.5	10.1	2.4	22	62	370	Btuh
7	2, Clear, DEF, N, N	S	1.5	6	18.3	18.3	0.0	22	37	403	Btuh
8	2, Clear, DEF, N, N	W	1.5	6	36.7	0.9	35.8	22	72	2595	Btuh
	Window Total				230					6944	Btuh

Walls	Type	R-Value	Area	HTM	Load	
1	Frame - Exterior	13.0	1825.0	1.7	3176	Btuh
	Wall Total		1825.0		3176	Btuh

Doors	Type	Area	HTM	Load	
1	Wood - Exter	20.0	10.0	200	Btuh
2	Wood - Adjac	20.0	10.0	200	Btuh
3	Wood - Exter	53.0	10.0	529	Btuh
	Door Total	93.0		928	Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load	
1	Under Attic/Dark	30.0	1890.0	1.4	2684	Btuh
	Ceiling Total		1890.0		2684	Btuh

Floors	Type	R-Value	Size	HTM	Load	
1	Slab-On-Grade Edge Insulation	0.0	228.0 ft(p)	0.0	0	Btuh
	Floor Total		228.0		0	Btuh

Infiltration	Type	ACH	Volume	CFM=	Load	
	Natural	0.55	17010	156.2	3093	Btuh
	Mechanical			0	0	Btuh
	Infiltration Total			156	3093	Btuh

Internal gain		Occupants		Btuh/occupant		Appliance	Load	
		8	X	300	+	1200	3600	Btuh

Webbie

Columbia County Building Department
Culvert Waiver

Culvert Waiver No. 000000851

DATE: 10/20/2005 BUILDING PERMIT NO. 23740

APPLICANT KATIE REED PHONE 752-4072

ADDRESS 2230 SE BAYA DRIVE LAKE CITY FL 32025

OWNER WILLIAM & JANICE DAUGHERTY PHONE 752-4072

ADDRESS 210 NW KISSIMMEE WAY LAKE CITY FL 32055

CONTRACTOR DON REED PHONE 752-4072

LOCATION OF PROPERTY 41N, TL ON SUWANNEE VALLEY RD, TL ON KISSIMMEE WAY, 1ST DRIVE ON LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT

PARCEL ID # 25-2S-15-00093-009

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE: Katie Reed

A SEPARATE CHECK IS REQUIRED
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

PUBLIC WORKS DEPARTMENT USE ONLY

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE CULVERT WAIVER IS:

✓ APPROVED _____ NOT APPROVED - NEEDS A CULVERT PERMIT

COMMENTS:

SIGNED: [signature] DATE: 10/27/05

RECEIVED
OCT 24 2005
BY:

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

COLUMBIA COUNTY · GATEWAY TO FLORIDA ·

Notice of Treatment 11761

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: 536 SE Baya Ave
City Lake City Phone 7521703

Site Location: Subdivision /
Lot # _____ Block# _____ Permit # ~~2372~~ 23748
Address 210 NW Kissimmee Way

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☑ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☑ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling	2772	793	4.5

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

1-9-06	0915	Gunny (F254)
Date	Time	Print Technician's Name

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink

10/05 ©

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 25-2S-15-00093-009 **Building permit No.** 000023740

Use Classification SFD,UTILITY **Fire:** 0.00

Permit Holder DON REED **Waste:**

Owner of Building WILLIAM & JANICE DAUGHERTY **Total:** 0.00

Location: 210 NW KISSIMMEE WAY, LAKE CITY, FL

Date: 03/29/2006

John Kence

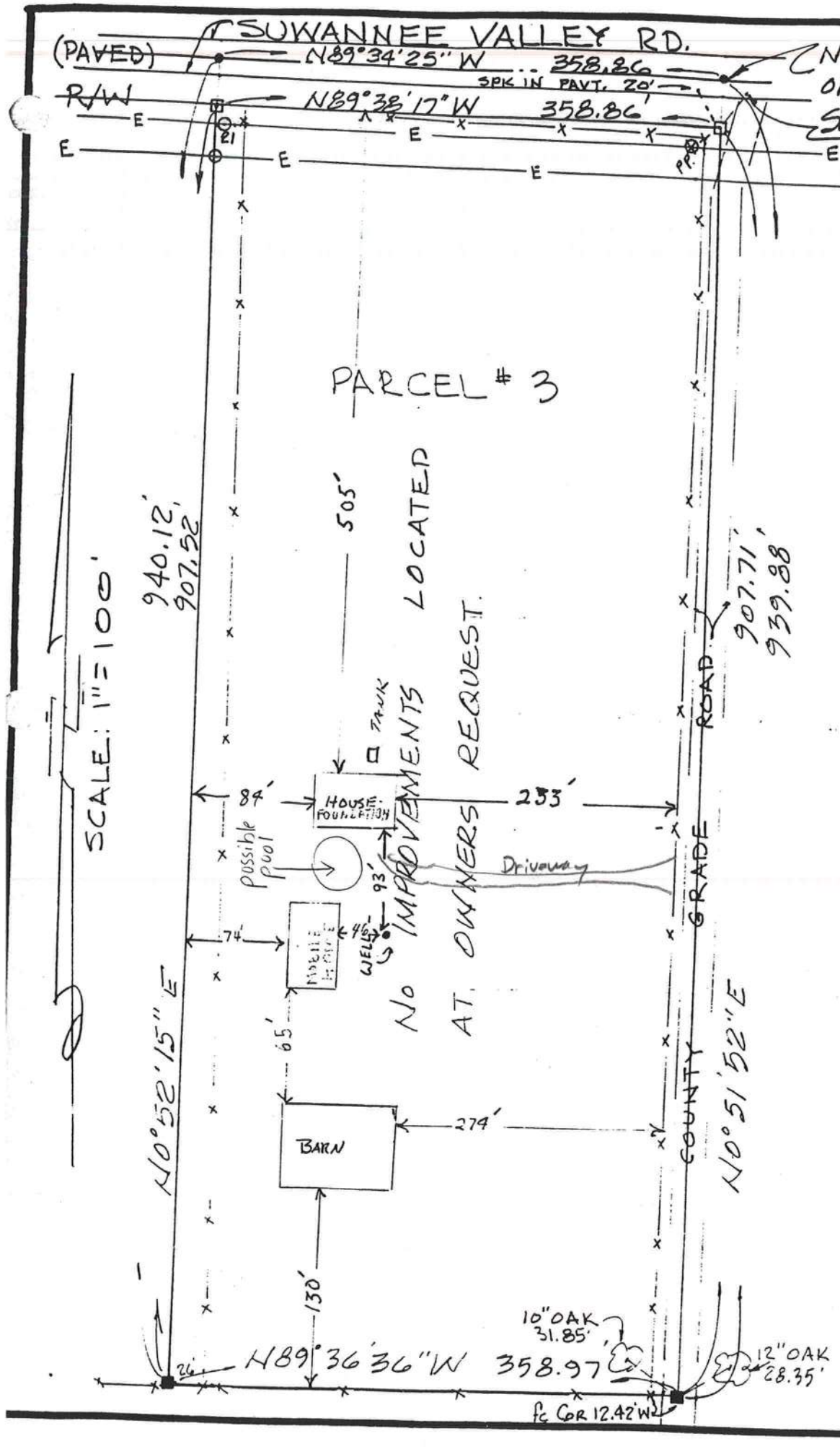
Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

003007

BOUNDARY SURVEY IN SECTION 25, TOWNSHIP 2 SOUTH RANGE 15 EAST, COLUMBIA COUNTY, FLORIDA.

LEGEND:
1. ■ = CONCRETE MONUMENT FOUND.
2. □ = CONCRETE MONUMENT, P.L.S. NO. 1079, SET.
3. ⊗ = POWER POLE.



DESCRIPTION:

THE EAST ½ OF THE W¼ OF THE NE¼ OF SECTION 25, TOWNSHIP 2 SOUTH, RANGE 15 EAST, LESS ROAD RIGHT-OF-WAY ALONG THE NORTH SIDE THEREOF AND LESS THE WEST 306.8 FEET, AND LESS THE SOUTH 390.0 FEET THEREOF. COLUMBIA COUNTY, FLORIDA. CONTAINING 7.48 ACRES MORE OR LESS. SUBJECT TO A ROAD RIGHT-OF-WAY ALONG THE EAST SIDE THEREOF.

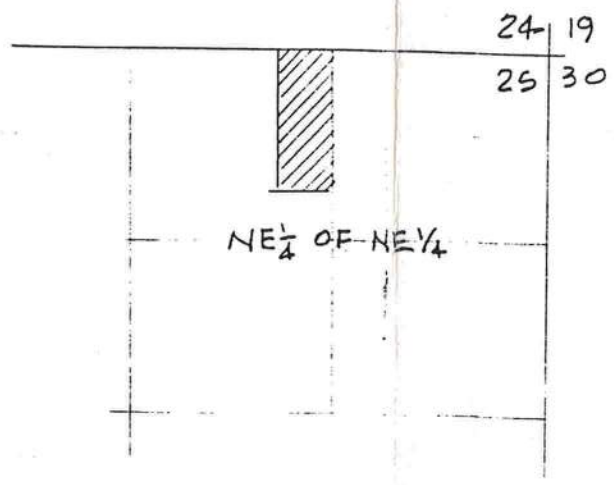
237' 517

SURVEYOR'S NOTES:

1. BOUNDARY BASED ON MONUMENTATION FOUND AND SECTION BREAKDOWN BY THIS OFFICE.
2. BEARINGS BASED ON PREVIOUS SURVEY INFORMATION, AND DEEDS OF RECORD.
3. THIS PARCEL IS IN ZONE "X" AND IS DETERMINED TO BE OUTSIDE THE 500 YEAR FLOOD PLAIN AS PER FLOOD INSURANCE RATE MAP, DATED 6 JANUARY 1988, COMMUNITY PANEL NO. 120070 0105B.
4. CERTIFIED TO:

   REGIONAL TITLE COMPANY

   WILLIAM DAUGHTREY



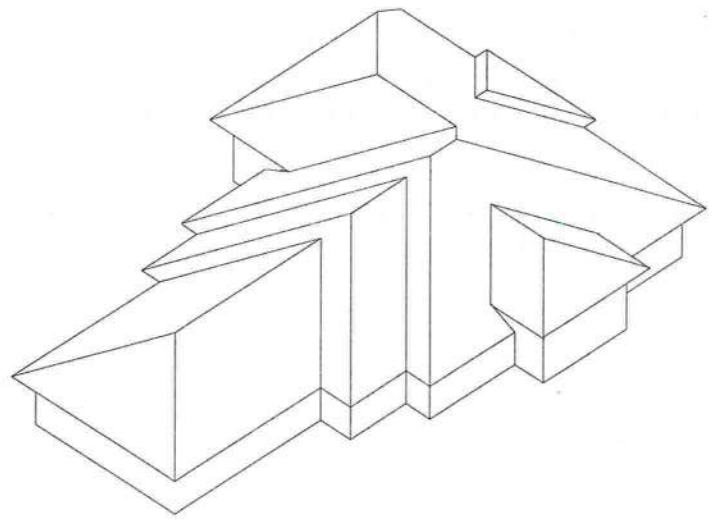
LOCATION SKETCH
SEC. 25, T2S, R15E
NO SCALE.

SURVEYOR'S CERTIFICATION:
I, THE UNDERSIGNED REGISTERED LAND SURVEYOR, HEREBY CERTIFY THAT A SURVEY OF THE ABOVE DESCRIBED PROPERTY WAS MADE UNDER MY DIRECTION AND THAT THIS IS A TRUE AND CORRECT REPRESENTATION THEREOF TO THE BEST OF MY KNOWLEDGE AND BELIEF THAT THERE ARE NO ENCROACHMENTS EXCEPT AS SHOWN AND THAT THE IMPROVEMENTS ARE AS INDICATED HEREON. THIS SURVEY MEETS THE MINIMUM TECHNICAL STANDARDS FOR LAND SURVEYING IN THE STATE OF FLORIDA (CHAPTER 21 HH-6 F.A.C.)

LAUREN E. BRITT. P.L.S.
FLA. CERT. NO. 1079

DATE: 6-14-89
FB: 96 PG: 73
FOR: LOWERY/DAUGHTREY

BRITT SURVEYING
1426 W. DUVAL STREET
LAKE CITY, FLORIDA
32055
(904) 752-7163



BEARING HEIGHT SCHEDULE

8'-0"

10'-0"

ROOF PITCH 6/12

OVERHANG 1'-6"

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved By: _____ Date: _____

Builders FirstSource

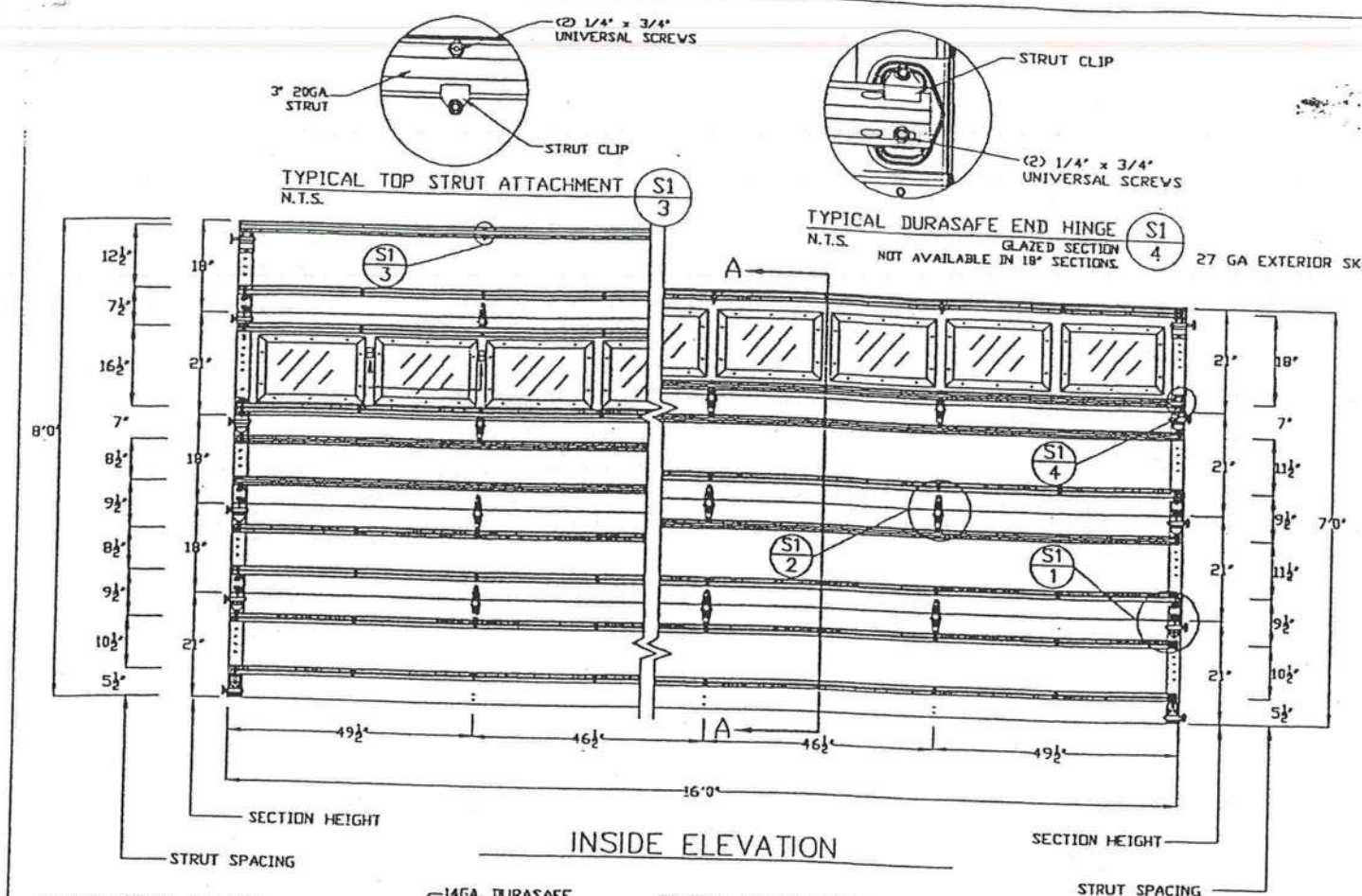
Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

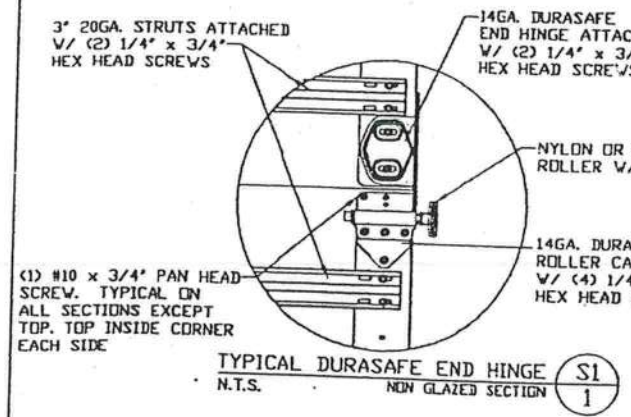
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

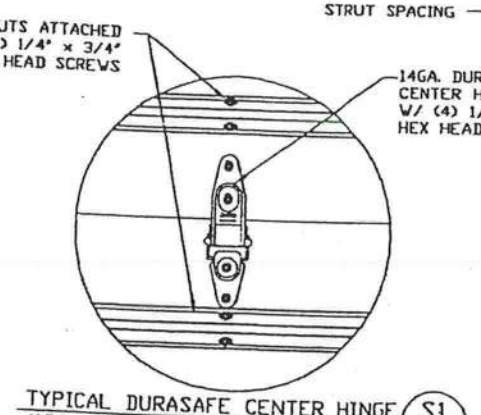
Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

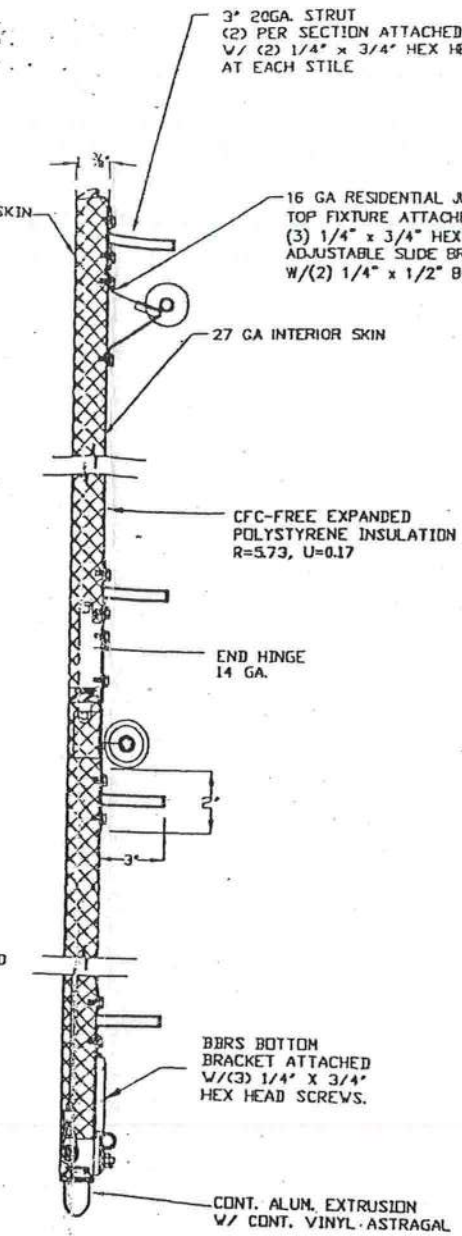
Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: DON REED CONST.			
LEGAL ADDRESS: COLUMBIA CO.			
MODEL: DAUGHERTY RES.		REVISION:	
		SCALE: NTS	
DATE: 09/20/05	DRAWN BY: ROBERT GULOTTI	JOB #: L130844	





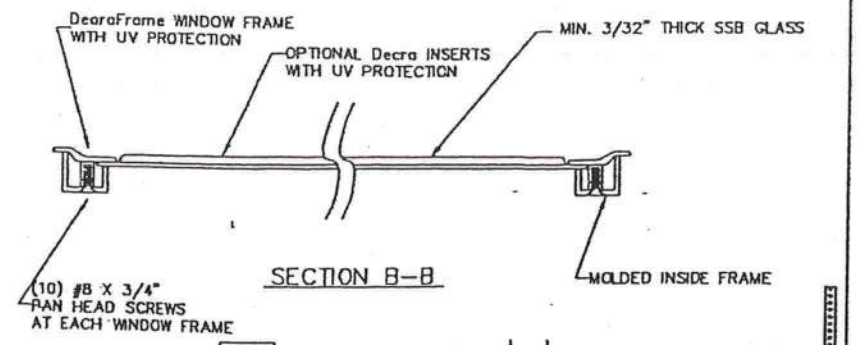


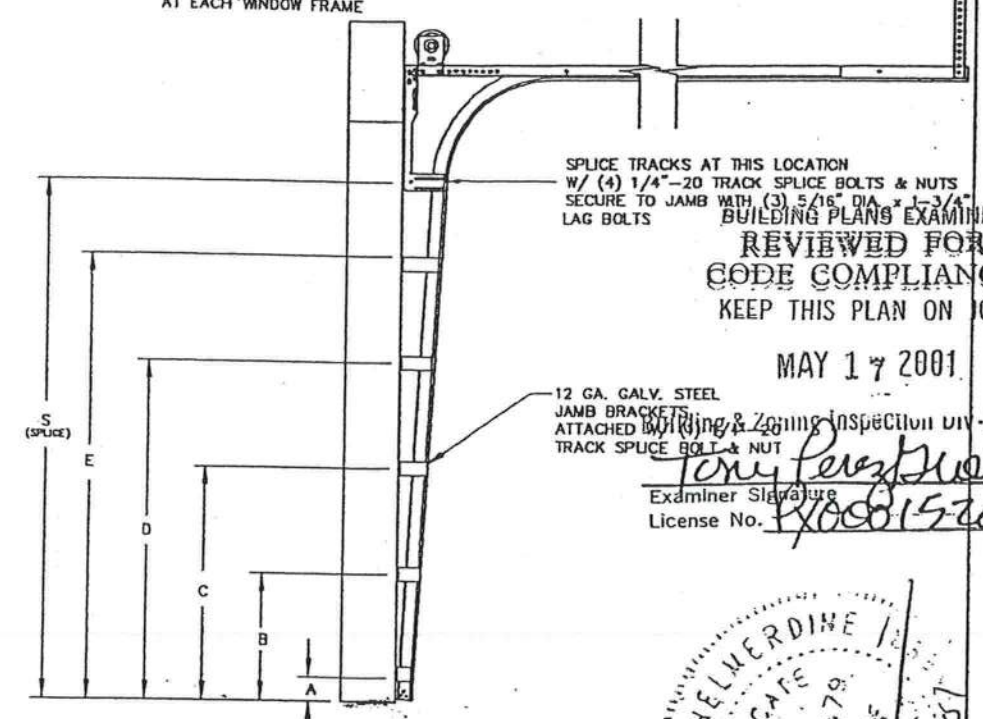


SECTION A–A (SIDE VIEW)

GLAZING OPTION CROSS SECTION

TEST No. SBC–580–020 ON MAY 24, 2000 INCLUDED GLASS WINDOWS IN THE DOOR BEING USED. THE TEST PRESSURES WERE +49.5 PSF AND –51.9 PSF. BY COMPARISION, EIGHT (8) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF THE 16' x 7' AND 16' X 8' MODEL 1500–D DOORS.



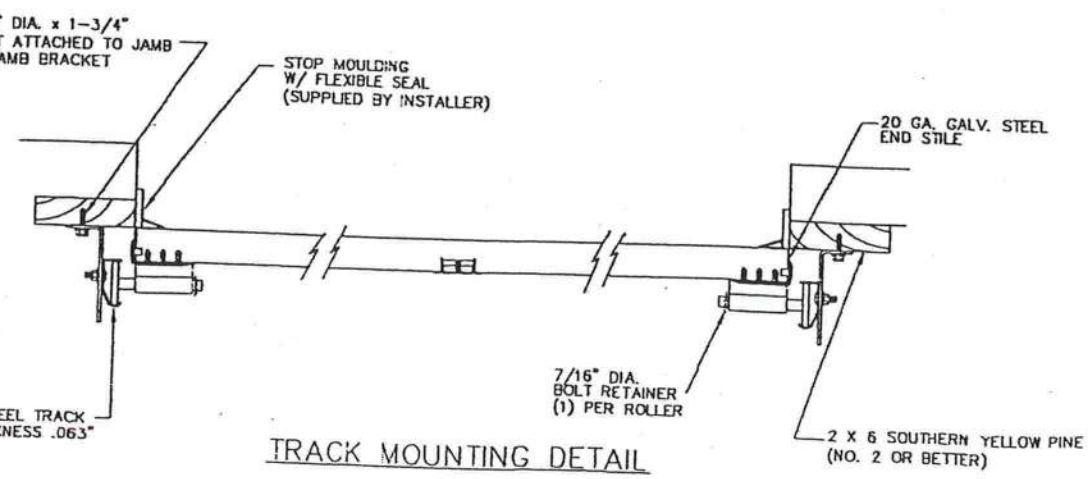


TRACK CONFIGURATION FOR 6'6" UP TO 8' TALL DOORS

JAMB BRACKET LOCATIONS

	A	B	C	D	E	S
6'–6"	4"	21–1/2"	39"	57"		70"
7'–0"	4"	21–1/2"	42"	63"		76"
7'–6"	4"	18–1/2"	36"	54"	72"	82"
8'–0"	4"	21–1/2"	39"	57"	75"	88"

BUILDING PLANS EXAMINER
REVIEWED FOR CODE COMPLIANCE
KEEP THIS PLAN ON JOB
MAY 17 2001
Building & Zoning Inspection Div - Jax., FL.
Examiner Signature
License No. RA0015720



TRACK MOUNTING DETAIL

WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 110 MPH FASTEST–MILE BASIC WIND SPEEDS

VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" X 3" LAG SCREWS STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO 2,200 PSI CONCRETE
HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C.
HILTI SLEEVE ANCHOR 3/8" X 2–3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET RED HEAD 3/8" X 3" STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO C–90 BLOCK
HILTI SLEEVE ANCHOR 3/8" X 2–3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET TAPCON 1/4" X 2–3/4" STARTING 6" FROM ENDS, USE PAIRS OF FASTENERS (3" APART) AT 16" O.C.

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
*PREPARATION OF WOOD JAMBS BY OTHERS

SPECIFICATIONS AND NOTES

1. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
2. DOOR SECTIONS SHALL BE 27 GA. MIN. (.018") INTERIOR AND EXTERIOR ROLLED FORMED LIGHT COMMERCIAL QUALITY, G–40 GALVANIZATION
3. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
4. DOORS UPTO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTRED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
6. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN ASTM E330–90, AND THE SOUTHERN BUILDING CODE SECTION 1606 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING THE FOLLOWING PARAMETERS:
   A. BASIC WIND SPEED OF 110 MPH
   B. DOOR CAN BE INSTALLED WITH 5 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
   C. 15' MEAN ROOF HEIGHT AT ANY SLOPE
   D. USE FACTOR OF 1.0
   E. EXPOSURE RATING OF C

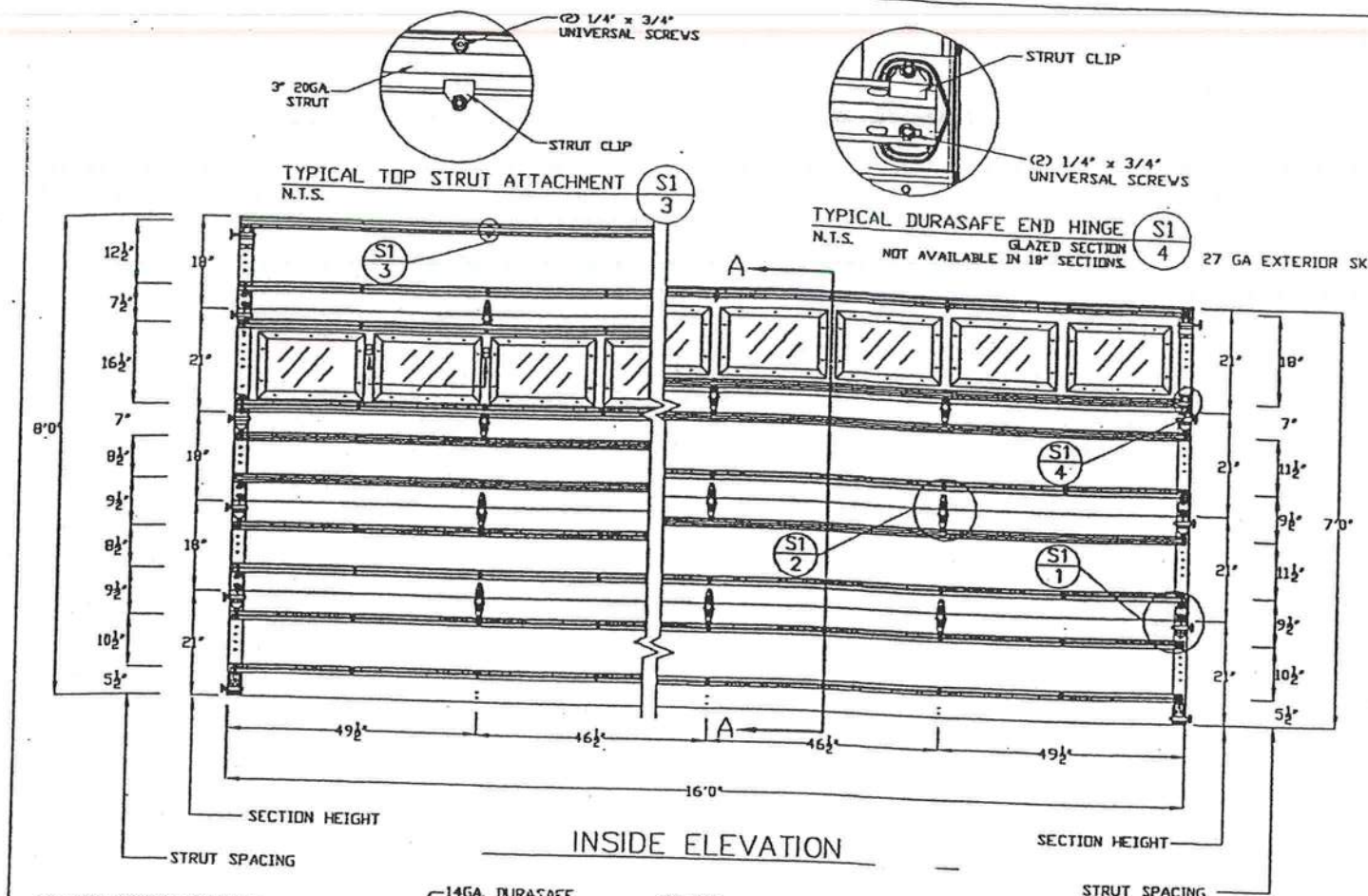
MAX SIZE 16' x 8'
DESIGN LOADS +29.5 PSF –29.5 PSF
TEST LOADS +45.3 PSF –45.3 PSF

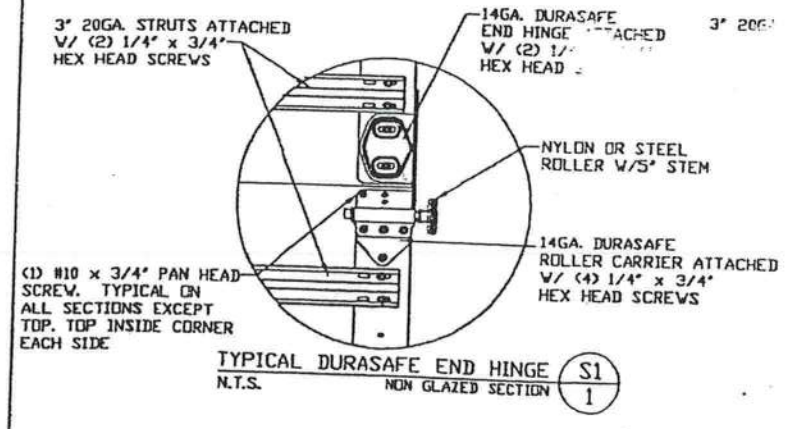
REV	DESCRIPTION OF REVISIONS	DATE	BY

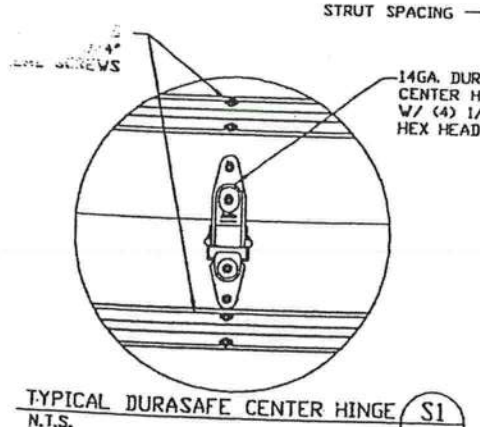
Amarr GARAGE DOORS
5921 QUARRY CREEK BLVD. WINSTON–SALEM, N.C. 27105

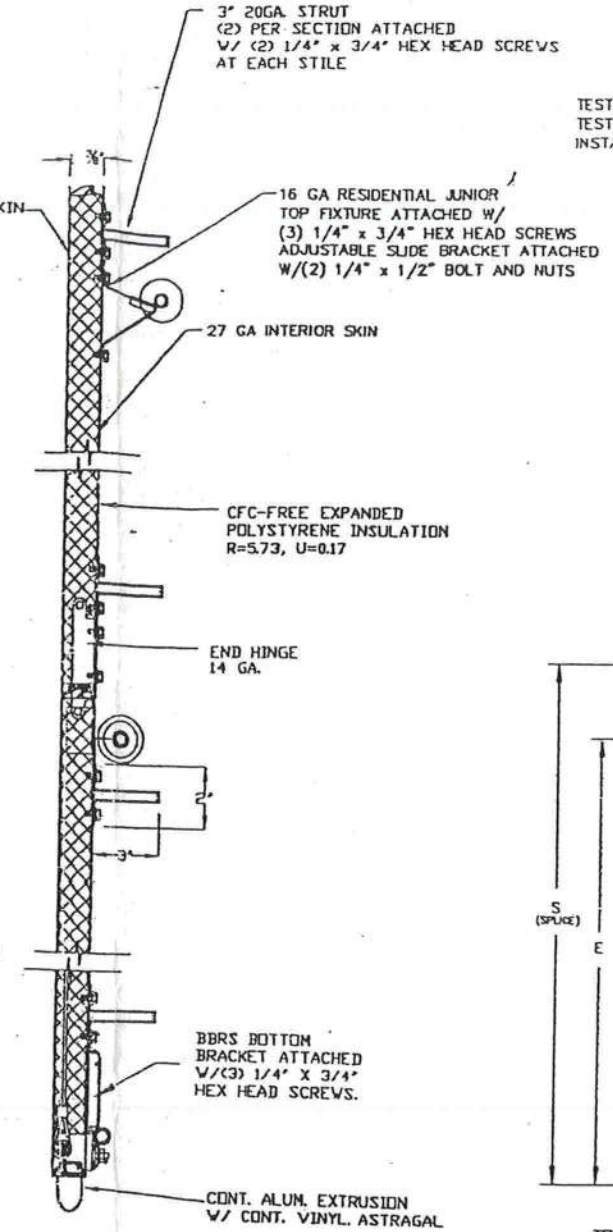
MODEL #1500 WeatherGuard

SIZE	DRAWN BY DLJ	DATE 04/18/01	DRAWING NUMBER
B	CHECKED BY	DATE	SBC–580–019–J
SCALE: NOT TO SCALE			SHEET 1 OF 1









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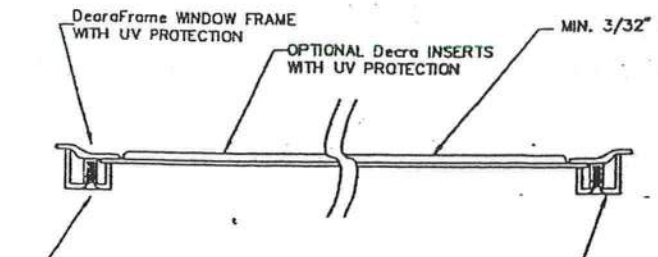

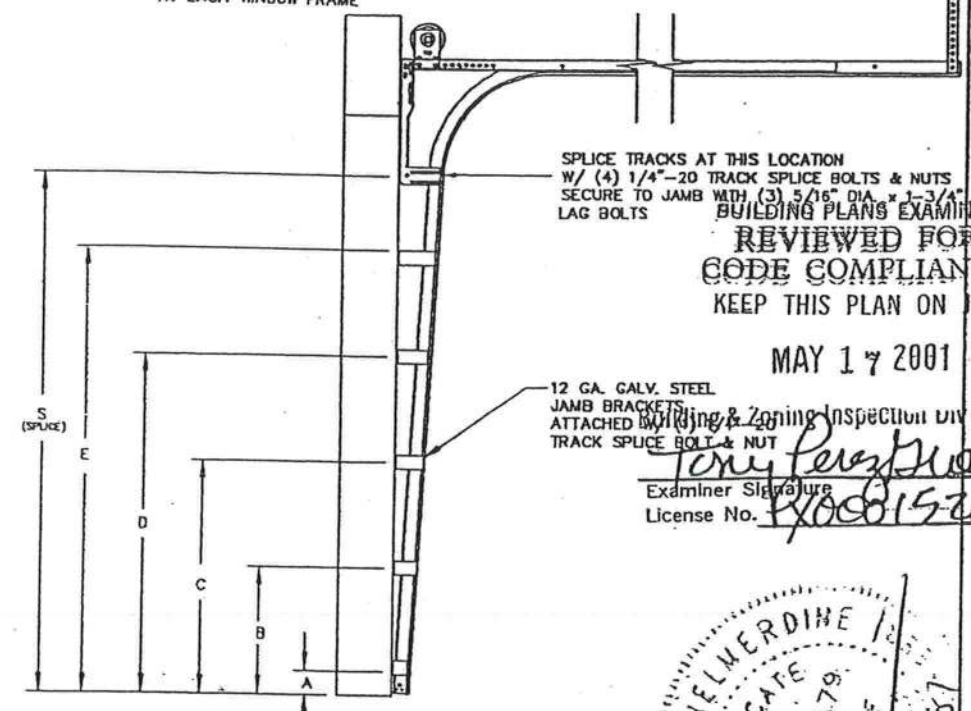

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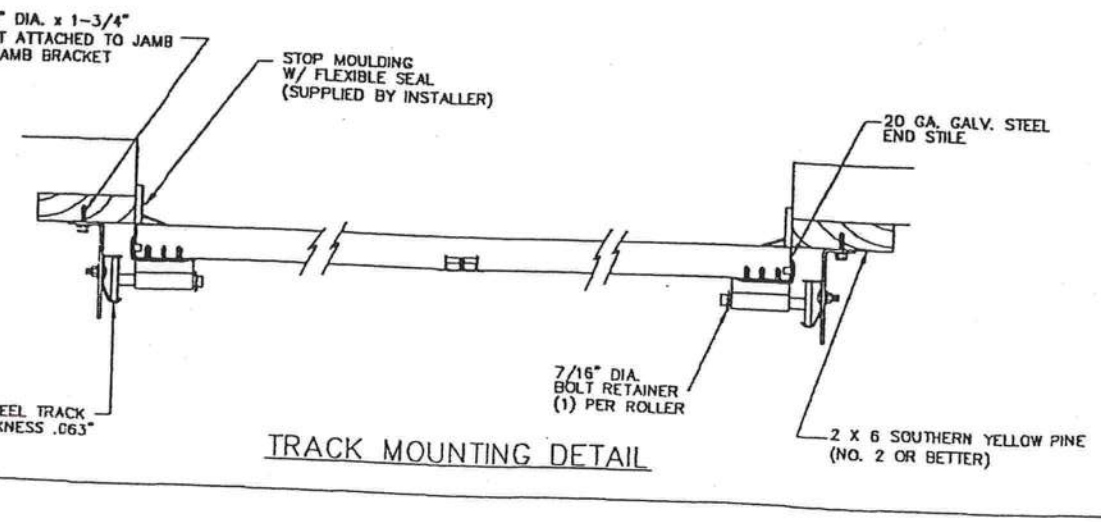
BUILDING PLANS EXAMINER
REVIEWED FOR CODE COMPLIANCE
KEEP THIS PLAN ON JOB
MAY 17 2001
Building & Zoning Inspection Div. - Jax., FL.
Examiner Signature
License No. RA0061520



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MAX SIZE 16' x 8'

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-29.5 PSF

TEST LOADS
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-49.3 PSF

REV	DESCRIPTION OF REVISIONS	DATE	BY

Amarr GARAGE DOORS
5921 QUARRY CREEK BLVD. WINSTON-SALEM, N.C. 27105

MODEL #1500 WeatherGuard

SIZE	DRAWN BY DJ	DATE 04/11/01	DRAWING NUMBER
B	CHECKED BY	DATE	SBC-580-019-J
SCALE: NOT TO SCALE			SHEET 1 OF 1

23740

Notice of Intent for Preventative Treatment for Termites

(As required by Florida Building Code 104.2.6)

Date: 12/12/05

210 NW Kissimmee Way — Lake City

(Address of Treatment or Lot/Block of Treatment) — City

Florida Pest Control & Chemical Co.

www.flapest.com

Product to be used: Bora-Care Termiticide (Wood Treatment)

Chemical to be used: 23% Disodium Octaborate Tetrahydrate

Application will be performed onto structural wood at dried-in stage of construction. Bora-Care Termiticide application shall be applied according to EPA registered label directions as stated in the Florida Building Code Section 1861.1.8

(Information to be provided to local building code offices prior to concrete foundation installation.)

6/04 ©