DATE 06/11	1/2008	Columbia County Bu	ilding Permit	struction	PERMIT 000027076
APPLICANT	LINDA RO		PHONE	752-2281	00002.07.5
ADDRESS	387	SW KEMP CT	LAKE CITY		FL 32024
OWNER	ADAM PA		PHONE	623-2383	
ADDRESS	404	SW AINSLEY GLEN	LAKE CITY	8.	FL 32024
CONTRACTO	R ADA	AM PAPKA	PHONE	623-2383	
LOCATION O	F PROPERT	TY 47S, TL ON WALTER AVE, TL O	N LITTLE RD, TR ON	MEADOWS,TR	ON
		AINSLEY GLEN, TO THE END C	F CUL-DE-SAC, ON L	EFT	
TYPE DEVELO	OPMENT	SFD,UTILITY EST	IMATED COST OF CO	NSTRUCTION	221050.00
HEATED FLO	OR AREA	3058.00 TOTAL ARE.	A 4421.00	HEIGHT _	STORIES 1
FOUNDATION	CONC	WALLS FRAMED RO	OOF PITCH 8/12	FI	LOOR SLAB
LAND USE &	ZONING	A-3	MAX	HEIGHT	
Minimum Set I	Back Require	ments: STREET-FRONT 30.00	REAR	25.00	SIDE 25.00
NO. EX.D.U.	0	FLOOD ZONE X	DEVELOPMENT PERM	MIT NO.	
PARCEL ID	01-5S-16-0	03405-210 SUBDIVISION	SOUTHWOOD MI	EADOWS	16
LOT 10	BLOCK	PHASE UNIT	TOTA	AL ACRES	
000001608	1//S-1-5-50	CBC1253409	Sh	ih hall	M
Culvert Permit?	No.	Culvert Waiver Contractor's License Num	ber	Applicant/Owner	Contractor
CULVERT		<u>08-065</u> BK		H	<u>Y</u>
Driveway Conn	nection	Septic Tank Number LU & Zonin	g checked by App	proved for Issuan	ce New Resident
COMMENTS:	ONE FOO	T ABOVE THE ROAD, NOC ON FILE			
SEC. 2.3.1 LEC	GAL LOT O	F RECORD		OSSERIE NO METER POSS	
				Check # or C	Cash 1502
		FOR BUILDING & ZONIN	G DEPARTMENT	ONLY	(footer/Slab)
Temporary Pow	ver	Foundation		Monolithic	
		date/app. by	date/app. by	-	date/app. by
Under slab roug	gh-in plumbi		2,597 (4), 262	Sheathing	/Nailing
Framing		date/app. by Rough-in plumbing abo	date/app. by	l floor	date/app. by
	date/app		ove slab and below wood		date/app. by
Electrical rough	h-in	Heat & Air Duct		Peri. beam (Linte	el)
		date/app. by	date/app. by	r our ocum (Emi	date/app. by
Permanent power		te/app. by C.O. Final	ate/app. by	Culvert	date/app. by
M/H tie downs,	blocking, el	ectricity and plumbing	(A. 5) (A. 5)	Pool	
Reconnection		date/app. Pump pole	Utility Po	le	date/app. by
	d	late/app. by date/a	app. by	date/app. b	
M/H Pole dat	e/app. by	Travel Trailer da	te/app. by	Re-roof	date/app. by
BUILDING PEI	RMIT FEE	\$ 1110.00 CERTIFICATION FEE	\$ 22.11	SURCHARGE	E FEE \$ 22.11
MISC. FEES \$		ZONING CERT. FEE \$ 50.00			TE FEE \$
FLOOD DEVEL		11-1		//-	AL FEE 1254.22
	OFFICE	150/01	CLERKS OFFICE	(//	1/

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application CK # 1502
For Office Use Only Application # 0801 - 60 Date Received 1-14-8 By Ut Permit # 608 27076
Zoning Official BLK Date 18.01.08 Flood Zone X FEMA Map # WA Zoning A-3
Land Use A-3 Elevation M/A MFE MFE River N/A Plans Examiner OK5 H Date 1-200
Comments 23.1 /a / LL & Read
NOC DEH Deed or PA Dite Plan Deed Info Parent Parcel #
□ Dev Permit # well le le R □ In Floodway □ Letter of Authorization from Contractor
□ Unincorporated area □ Incorporated area □ Town of Fort White □ Town of Fort White Compliance letter
Fax 752-2282
Name Authorized Person Signing Permit Linda or Melanic Roder Phone 752-2281
Address 387 Sw Kemp 4 Lake City FL 32024
Owners Name Adam Papka & Leah Papka Phone 623-2383
911 Address 404 SW Ainsley Gln Lake City FL 32024
911 Address 404 SW Ainsley Gln Lake City FL 32024 Contractors Name Adam Papka of Adams Framing Phone 623-2383
Address POB 1921 Lake City FL 32056
Fee Simple Owner Name & Address
Bonding Co. Name & Address/}
Architect/Engineer Name & Address Mark Disosway
Mortgage Lenders Name & Address_CCB
Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progress Energy
Property ID Number 01-55-16-03405-210 Estimated Cost of Construction 225 K
Subdivision Name South wood Meadows Lot 10 Block A Unit Phase
Driving Directions 475, 92 on Sw Little Road Ron Meadows
Terrace, Ron Ainslay GLN to end of cul-de-Sac. Cot
is left lot of culdesac Number of Existing Dwellings on Property
Construction of Single family dwelling Total Acreage 123 Lot Size 1-2390
Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height
Actual Distance of Structure from Property Lines - Front Side Side Side Rear
Number of Stories Heated Floor Area
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this installation.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

above written responsibilities in Columbia Count	
Affirmed under penalty of perjury to by the Owner and	desperated to a second of the
Personally known or Produced Identification	subscribed before me this $\underline{7}$ day of $\underline{99}$
State of Florida Notary Signature (For the Owner)	Linda R. Roder Commission #DD303275 Expires: Mar 24, 2008 Bonded Thru Atlantic Bonding Co., Inc.
CONTRACTORS AFFIDAVIT: By my signature I under written statement to the owner of all the above withis Building Permit.	erstand and agree that I have informed and provided this ritten responsibilities in Columbia County for obtaining
Contractor's Signature (Permitee)	Contractor's License Number CBC 1253 409 Columbia County Competency Card Number
Affirmed under penalty of perjury to by the Contractor at Personally known or Produced Identification	and subscribed before me this 4 day of Jan 20_0.8
State of Florida Notary Signature (For the Contractor)	Linda R. Roder Commission #DD303275 Expires: Mar 24, 2008 Bonded Thru

Adantic Bonding Co., Inc.

Notice of Authorization

1 Adam Papk	, do h	ereby auth	orize Linda Roder or	Melanie Roder,
to be my representative	and act on my	y behaf in a	ıll aspects of applying	for any
- building	_ permit to be le	ocated in _	Columbia	county.
Any homeowner and lega	al description	>	**	
Y	2			8 5 2
Contractor's signature Date			THE OF ELOS	Linda R. Roder Commission #DD303275 Expires: Mar 24, 2008 Bonded Thru Atlantic Bonding Co., Inc.
Sworn and subscribed	before me this	7	day of Jak	20048
Stufn	Vale		1 , .	``,
Notary Public				
My commision expires: _ Commision No Personally known _ Produced ID (Type):				**

Warranty Deed

THIS WARRANTY DEED made the ///day of) AAVAM

Ben Martin and his wife, Irene Martin

hereinafter called the grantor, to

Adam R. Papka and his wife, Leah Papka

whose post office address is: P.O. Box 1921, Lake City, FL 32056

hardhafter called the grantee:

Inst:200812000704 Date:1/14/2008 Time:12:04 PM Doc Stamp-Deed:0.70 DC,P.DeWitt Cason,Columbia County Page 1 of 1

(Wherever used herein the zerms "granter" and "granter" include all the parties to this instrument and the heirs, legal representatives and sesigns of individuals, and the successors and assigns of corporation)

Witnesseth: That the grantor, for and in consideration of the sum of \$10.00 and other valuable considerations, receipt whereof is hereby anknowledged, hereby grants, bargains, sells, allens, remises, releases, conveys, and confirms unto the grantee, all that certain land stude in Columbia County, Florida, viz. 07405-200

Lot 10, Block A, Southwood Meadows, Unit II, a subdivision according to the plat thereof recorded in Plat Book 6, Page 84, of the public records of Columbia County, Florida.

TOGETHER with all tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

TO HAVE AND TO HOLD, the same in fee simple forever.

AND the granter hereby covenants with said grantee that the granter is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to sold land and will defend the same against the lawful-claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2007.

IN WITNESS WHEREOF, the said grantor has signed and scaled these presents the day and year first above written.

Signed, seeled and delivered in our presence:

Matthew D. Rocco

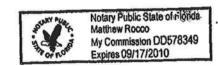
Jonathan Rocco

Witness:

STATE OF FLORIDA COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 1 43 day of Jewsey Ben Martin and his wife, Irane Martin, personally known to me or, if not personally known to me, who produced Driver's License for identification and who did not take an outh

(Notary Seal)

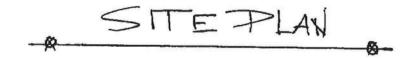


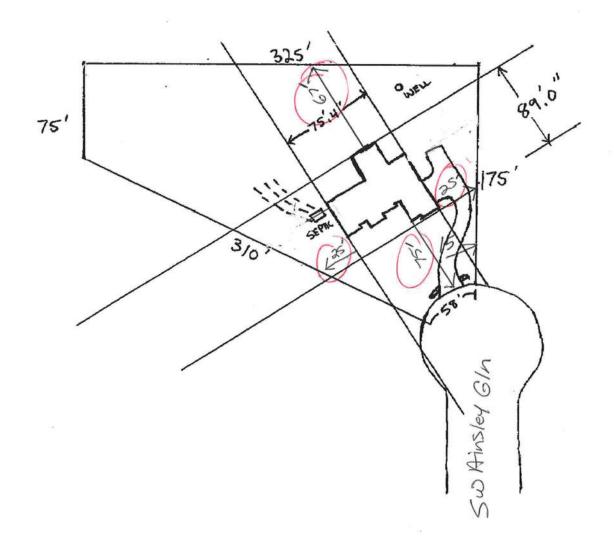
10# 01-55-16-03405-210

LOT 10 BLOCK A

Southwood Meadows SD

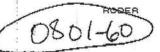






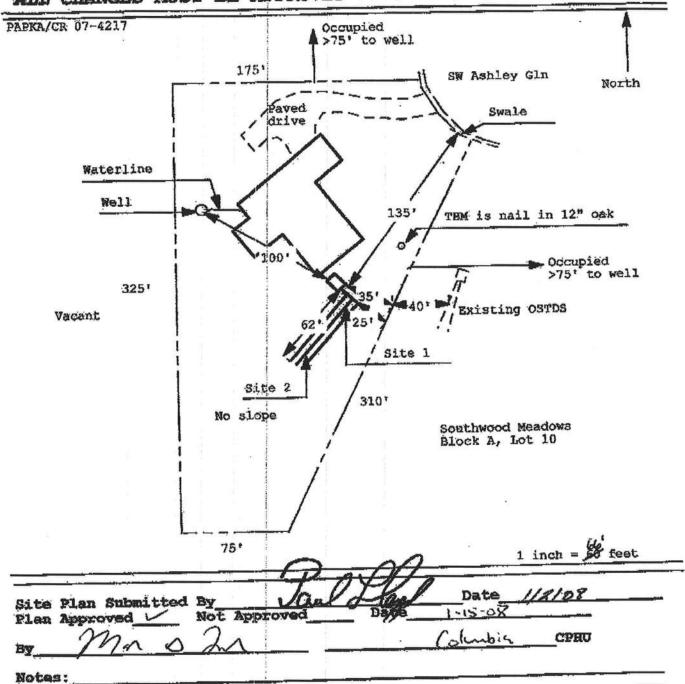


0801-60



Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number:

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



#1346 P.003 /006

-		_
	026	1-60)
(OSC	1-60)
-		

Inst:200812010944 Date:6/9/2008 Time:3:49 PM DC.P.DeWitt Cason,Columbia County Page 1 of 1 B:1152 P:68 NOTICE OF COMMENCEMENT 01-55+16-03405-County Clerk's Office Stamp or Seal Tax Parcel Identification Number THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Scannes the following information is provided in this NOTICE OF COMMENCEMENT. > outhwood Meadows F 10 1. Description of property (legal description): a) Street (Job) Address: 404 ale CHYFL 2. General description of improvements: 3. Owner Information a) Name and address: b) Name and address of fee simple titleholder
c) Interest in property Nome Site (if other than aware) P.D. B. 4. Contractor Information a) Name and address: b) Telephone No.: Fax No. (Opt.) 5. Surety Information a) Name and address: b) Amount of Bond: c) Telephone No.: Fax No. (Opr.) 6 Lender a) Name and address. b) Phone No. 7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served: a) Name and address: NA-Fax No. (Opt.) b) Telephone No.: 8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b). Florida Statutes: a) Name and address: b) Telephone No : ____ 9. Expiration date of Notice of Commencement (the eng is specifically WARRING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROVER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF CUMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTURNEY SEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT. NOTARY PUBLIC-STATE OF FLORID STATE OF FLORIDA Linda R. Roder COUNTY OF COLUMBIA Commission # DD755600 Signature of Owner or Owner's Authorized Office/Director/Partner/Man
Expires: MAR. 24, 2012 LUNDED THRU ATLANTIC BONDING CO., INC. The foregoing instrument was acknowledged before me . a Florida Notary, this (type of authority, Ag. officer, trastic, attorney (game of party on behalf of **OR Produced Identification** Type Personally Known

#1346 P.004 /006

CANON

ZRZZZS/985 85:01 800Z/TT/90

(0801-60)

06/09/2008 22:23 3867557022

HALLS

PAGE 01

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS



PHONE (386) 752-1854 FAX (386) 755-7022 904 NW MAIN BLVD. LAKE CITY, FLORIDA 32055

DONALD AND MARY HALL. OWNERS

June 10, 2008

Notice To All Contractors:

Re: Adam Papka - 01-5S-16-03405-210

Please be advised that due to the new building codes we will Use a large capacity diaphragm tank on all new well. This will in sure a minimum of one (1) minute draw down or One (1) minute refill. If a smaller diaphragm tank is used then We will install a cycle stop valve which will produce the same Results. All wells will have a pump & tank combination that Will be sufficient enough for each situation.

If you have any questions please feel free to call our office.

Thank You,

Donald Hall

Project Name:

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

611075Adam'sFramingConstruction

Builder: PAPKA

City, State: , Owner: Papka, Adam Residence Climate Zone: North	Permitting Office: (010M61A) Permit Number: 27076 Jurisdiction Number: 221000
1. New construction or existing 2. Single family or multi-family 3. Number of units, if multi-family 4. Number of Bedrooms 5. Is this a worst case? 6. Conditioned floor area (ft²) 7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default) a. U-factor:	12. Cooling systems a. Central Unit b. Central Unit c. N/A 13. Heating systems a. Electric Heat Pump b. Electric Heat Pump c. N/A 14. Hot water systems a. Electric Resistance b. N/A 15. N/A 16. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) 17. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)
Glass/Floor Area: 0.19 Total as-built	points: 38569 points: 40329 PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code. PREPARED BY: I hereby certify that this building, as designed, is in compliance with the Florida Energy Code. OWNER/AGENT; DATE:

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _

DATE:

SUMMER CALCULATIONS

ADDRESS:,,,	PERMIT #:

BASE					AS-	BUI	LT					
GLASS TYPES .18 X Condition Floor A	oned X	BSPM =	Points	Type/SC	Ove Ornt	erhang Len		Area X	SPI	ΛX	SOF	= Points
.18 3058	3.0	20.04	11030.8	Double, Clear	s	1.5	6.5	54.0	35.8	7	0.88	1698.2
1 1865				Double, Clear	W	99.0	6.0	20.0	38.5	2	0.37	288.6
				Double, Clear	S	16.0	6.0	40.0	35.8	7	0.43	622.4
2				Double, Clear	E	99.0	5.5	20.0	42.0	6	0.36	300.2
				Double, Clear	S	21.0	5.5	10.0	35.8	7	0.43	154.9
				Double, Clear	E	10.0	6.5	60.0	42.0	6	0.43	1080.9
				Double, Clear	S	1.5	0.0	72.0	35.8	7	0.43	1115.4
				Double, Clear	S	1.5	0.0	24.0	35.8	7	0.43	371.8
				Double, Clear	W	1.5	6.5	72.0	38.5	2	0.93	2571.6
				Double, Clear	S	1.5	6.5	18.0	35.8	7	0.88	566.1
				Double, Clear	W	1.5	6.5	36.0	38.5	2	0.93	1285.8
				Double, Clear	w	1.5	5.5	8.0	38.5	2	0.90	276.4
				Double, Clear	N	1.5	6.0	8.7	19.2	0	0.94	156.8
				Double, Clear	N	1.5	0.0	42.0	19.2	0	0.59	478.3
				Double, Clear	N	1.5	3.0	4.0	19.2	0	0.83	63.8
				Double, Clear	N	1.5	0.0	26.0	19.2	0	0.59	296.1
				Double, Clear	Е	1.5	4.0	6.0	42.0	6	0.82	205.8
				Double, Clear	Е	1.5	6.0	25.0	42.0	6	0.91	959.8
				Double, Clear	E	1.5	6.5	36.0	42.0		0.93	1403.0
				Double, Clear	E	1.5	3.0	8.0	42.0		0.73	244.1
				Double, Clear	N	1.5	0.0	6.0	19.2		0.59	68.3
				As-Built Total:	25.	202	57.5.	595.7				14208.5
WALL TYPES	Area	X BSPM	1 = Points	Туре		R-V	√alue	Area	х	SPN	1 =	Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1824.3		1.50	1	2736.5
Exterior	2020.3	1.70	3434.5	Frame, Wood, Exterior			13.0	196.0		1.50		294.0
Base Total:	2020.3	l e	3434.5	As-Built Total:				2020.3				3030.5
DOOR TYPES	Area	X BSPM	1 = Points	Туре				Area	X	SPN	1 =	Points
Adjacent	20.0	1.60	32.0	Exterior Insulated				120.0		4.10		492.0
Exterior	180.0	4.10	738.0	Exterior Insulated				60.0		4.10		246.0
	.50.0	4.10	700.0	Adjacent Insulated				20.0		1.60		32.0
Base Total:	200.0		770.0	As-Built Total:				200.0				770.0

SUMMER CALCULATIONS

ADDRESS:,,,	PERMIT #:

	BASE			AS-BUILT	
CEILING TYPE	S Area X I	BSPM =	Points	Type R-Value Area X SPM X	SCM = Points
Under Attic	3058.0	1.73	5290.3	Under Attic 30.0 3180.0 1.73 X 1.	00 5501.4
Base Total:	3058.0		5290.3	As-Built Total: 3180.0	5501.4
FLOOR TYPES	Area X i	BSPM =	Points	Type R-Value Area X SF	PM = Points
Slab Raised	254.0(p) 0.0	-37.0 0.00	-9398.0 0.0	Slab-On-Grade Edge Insulation 0.0 254.0(p -41.	20 -10464.8
Base Total:			-9398.0	As-Built Total: 254.0	-10464.8
INFILTRATION	Area X I	BSPM =	Points	Area X SF	PM = Points
	3058.0	10.21	31222.2	3058.0 10	.21 31222.2
Summer Bas	se Points:	42349	9.8	Summer As-Built Points:	44267.7
Total Summer Points	X System Multiplier		ooling Points	Total X Cap X Duct X System X Cre Component Ratio Multiplier Multiplier Multi (System - Points) (DM x DSM x AHU)	dit = Cooling iplier Points
42349.8	0.4266	1	8066.4	(sys 1: Central Unit 34000 btuh ,SEER/EFF (13.0) Ducts:Unc(S),Unc(R),Int(AH 44268 0.50 (1.09 x 1.147 x 0.91) 0.263 1.00 (sys 2: Central Unit 34000 btuh ,SEER/EFF (13.0) Ducts: None 44268 0.50 (1.00 x 1.147 x 1.00 0.263 1.00 44267.7 1.00 1.138 0.263 1.00	00 6611.2

WINTER CALCULATIONS

ADDRESS:,,,	PERMIT #:
ADDRESS.,,,	

	-	BAS	E					AS-	BUI	LT				
	Condition Floor Are		BWP	PM =	Points	Type/SC	Ove Omt	erhang Len	Hgt	Area X	WF	PM :	x wo	F = Point
.18	3058.0		12.7	74	7012.6	Double, Clear	S	1.5	6.5	54.0	13.	30	1.09	785.6
	000010					Double, Clear	W	99.0	6.0	20.0	20.	73	1.24	513.1
						Double, Clear	S	16.0	6.0	40.0	13.	30	3.65	1938.6
						Double, Clear	E	99.0	5.5	20.0	18.	79	1.51	566.3
						Double, Clear	S	21.0	5.5	10.0	13.	30	3.66	486.7
						Double, Clear	E	10.0	6.5	60.0	18.	79	1.40	1574.6
						Double, Clear	s	1.5	0.0	72.0	13.	30	3.66	3504.2
						Double, Clear	s	1.5	0.0	24.0	13.	30	3.66	1168.1
						Double, Clear	W	1.5	6.5	72.0	20.	73	1.02	1521.8
						Double, Clear	s	1.5	6.5	18.0	13.	.30	1.09	261.9
						Double, Clear	W	1.5	6.5	36.0	20.	.73	1.02	760.9
						Double, Clear	W	1.5	5.5	8.0	20.	.73	1.03	170.5
						Double, Clear	N	1.5	6.0	8.7	24	.58	1.00	214.3
						Double, Clear	N	1.5	0.0	42.0	24	.58	1.03	1060.5
						Double, Clear	- N	1.5	3.0	4.0	24	.58	1.01	99.2
						Double, Clear	N	1.5	0.0	26.0	24	.58	1.03	656.5
						Double, Clear	E	1.5	4.0	6.0	18	.79	1.07	121.1
						Double, Clear	E	1.5	6.0	25.0	18	.79	1.04	486.5
						Double, Clear	E	1.5	6.5	36.0	18	.79	1.03	697.3
						Double, Clear	E	1.5	3.0	8.0	18	.79	1.12	168.4
						Double, Clear	N	1.5	0.0	6.0	24	.58	1.03	151.5
						As-Built Total:				595.7				16907.5
WALL	TYPES	Area	X B\	WPM	= Points	Туре		R-	Value	Area	Χ	WP	PM =	Points
Adjacent		0.0		0.00	0.0	Frame, Wood, Exterior			13.0	1824.3		3.4	10	6202.6
Exterior	! !	2020.3		3.70	7475.1	Frame, Wood, Exterior			13.0	196.0		3.4		666.4
Base To	otal:	2020	.3		7475.1	As-Built Total:				2020.3				6869.0
Amount to a second	TYPES	Area	X BI	WPM	= Points	Туре				Area	х	WF	PM =	Points
		20.0	200	8.00	160.0	Exterior Insulated				120.0	_	8.4		1008.0
Adjacent						Exterior Insulated				60.0		8.4		504.0
Exterior		180.0		8.40	1512.0	Adjacent Insulated				20.0		8.0		160.0
Base To	ntal:	200	0		1672.0	As-Built Total:				200.0				1672.0

WINTER CALCULATIONS

ADDRESS:,,,	PERMIT #:
ADDITEOU.,,,	Assertation and Colored Colore

	BASE			AS-BUILT	
CEILING TYPE	S Area X	BWPM	= Points	Type R-Value Area X WPM X WCM =	Points
Under Attic	3058.0	2.05	6268.9	Under Attic 30.0 3180.0 2.05 X 1.00	6519.0
Base Total:	3058.0		6268.9	As-Built Total: 3180.0	6519.0
FLOOR TYPES	Area X	BWPM	= Points	Type R-Value Area X WPM =	Points
Slab Raised	254.0(p) 0.0	8.9 0.00	2260.6 0.0	Slab-On-Grade Edge Insulation 0.0 254.0(p 18.80	4775.2
Base Total:			2260.6	As-Built Total: 254.0	4775.2
INFILTRATION	I Area X	BWPM	= Points	Area X WPM =	Points
il ii	3058.0	-0.59	-1804.2	3058.0 -0.59	-1804.2
Winter Base	Points:	1 10	22885.0	Winter As-Built Points: 34	1938.5
Total Winter X Points	System Multipl		eating Points	Total X Cap X Duct X System X Credit = Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)	Heating Points
22885.0	0.62	74	14358.0	(sys 2: Electric Heat Pump 34000 btuh ,EFF(7.9) Ducts: None 34938.5 0.500(1.00 x 1.169 x 1.00) 0.432 1.000	R6.0 8763.5 8763.5 7527.0

FORM 600A-2004 EnergyGauge® 4.1

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS:,,,	PERMIT #:
7.00.1200.111	

BASE	AS-BUILT									
WATER HEATING Number of X Multiplier Bedrooms	= Total	Tank Volume	EF	Number of Bedrooms	х	Tank X Ratio	Multiplier X	Credit Multiplie		
3 2635.00	7905.0	80.0 As-Built To	0.93	3		1.00	2606.67	1.00	7820.0 7820.0	

	CODE COMPLIANCE STATUS													
	BASE						AS-BUILT							
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	
18066		14358		7905		40329	13222		17527		7820		38569	

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS:,,,	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: 3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 83.8

The higher the score, the more efficient the home.

Papka, Adam Residence, , , ,

		20 0 000 000			POSSESSES TATALISMS		
1.	New construction or existing	New	_	12.	Cooling systems		
2.	Single family or multi-family	Single family	_		Central Unit	Cap: 34.0 kBtu/hr	
3.	Number of units, if multi-family	1				SEER: 13.00	
4.	Number of Bedrooms	3		b.	Central Unit	Cap: 34.0 kBtu/hr	_
5.	Is this a worst case?	Yes	100000			SEER: 13.00	_
6.	Conditioned floor area (ft²)	3058 ft ²		c.	N/A		
7.	Glass type 1 and area: (Label reqd. by	13-104.4.5 if not default)					-
a.	. U-factor:	Description Area		13.	Heating systems		-
	(or Single or Double DEFAULT) 7a	(Dble Default) 595.7 ft ²	_		Electric Heat Pump	Cap: 34.0 kBtu/hr	
b	. SHGC:				•	HSPF: 7.90	
	(or Clear or Tint DEFAULT) 71	b. (Clear) 595.7 ft ²		b.	Electric Heat Pump	Cap: 34.0 kBtu/hr	
8.	Floor types				•	HSPF: 7.90	
	Slab-On-Grade Edge Insulation	R=0.0, 254.0(p) ft	-	C.	N/A		
	. N/A						
C.	N/A			14.	Hot water systems		
9.	Wall types				Electric Resistance	Cap: 80.0 gallons	
	Frame, Wood, Exterior	R=13.0, 1824.3 ft ²	_			EF: 0.93	
b.	Frame, Wood, Exterior	R=13.0, 196.0 ft ²	_	Ь.	N/A	221,0170	
	N/A		_				_
	N/A		_	c.	Conservation credits		_
	N/A		_		(HR-Heat recovery, Solar		_
	Ceiling types				DHP-Dedicated heat pump)		
	Under Attic	R=30.0, 3180.0 ft ²	_	15.	HVAC credits		
	N/A				(CF-Ceiling fan, CV-Cross ventilation,		_
	N/A				HF-Whole house fan,		
	Ducts				PT-Programmable Thermostat,		
	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 220.0 ft			MZ-C-Multizone cooling,		
b.	N/A		200		MZ-H-Multizone heating)		
I cei	rtify that this home has complied	with the Florida Energ	v Effic	iency	Code For Building		
Con	struction through the above ener	gy saving features which	h will	be in	stalled (or exceeded)	OF THE STATE	2
in th	nis home before final inspection.	Otherwise, a new FPI	Displa	v Car	d will be completed	37 00	B.
base	d on installed Code compliant fe	eatures	Lipid	, cai	a min oc completed	2 31	Bo
	der Signature:		Data				甜
			Date:			10	Z]
Add	ress of New Home:		City/F	L Zip);	GOD WE TRUST	,
						CO VIII CO	

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is <u>not</u> a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStat^M designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

Columbia County Building Department Culvert Permit

Culvert Permit No.

000001608

DATE 06/11/2008	PARCEL ID # 01-5S-16-03405-210	
APPLICANT LINDA RODER	PHONE	752-2281
ADDRESS 387 SW KEMP CT	LAKE CITY	FL 32024
OWNER ADAM PAPKA	PHONE	623-2383
ADDRESS 404 SW AINSLEY GLEN	LAKE CITY	FL 32024
CONTRACTOR ADAM PAPKA	PHONE	623-2383
LOCATION OF PROPERTY 47S, TL	ON WALTER AVE, TL ON LITTLE RD, TR C	ON MEADOWS,TR ON
AINSLEY GLEN, TO THE END OF CUL-DE-	SAC, ON LEFT	
SUBDIVISION/LOT/BLOCK/PHASE/	UNIT SOUTHWOOD MEADOWS	10
SIGNATURE (MIL	
INSTALLATION R	<u>EQUIREMENTS</u>	
X Culvert size will be 18 driving surface. Both e thick reinforced concre	inches in diameter with a total lenght of 3 nds will be mitered 4 foot with a 4 : 1 slo ete slab.	32 feet, leaving 24 feet of ope and poured with a 4 inch
a) a majority of the o b) the driveway to be Turnouts shall be o concrete or paved	E: Turnouts will be required as follows: urrent and existing driveway turnouts are served will be paved or formed with corconcrete or paved a minimum of 12 feet driveway, whichever is greater. The widt g paved or concreted turnouts.	ncrete. wide or the width of the
Culvert installation sh	all conform to the approved site plan sta	andards.
Department of Transp	ortation Permit installation approved sta	andards.
Other		
8		
ALL DOODED CAFETY DECUIDEMENTS	CHOULD BE FOLLOWED	ACTION AND ADDRESS OF THE PARTY

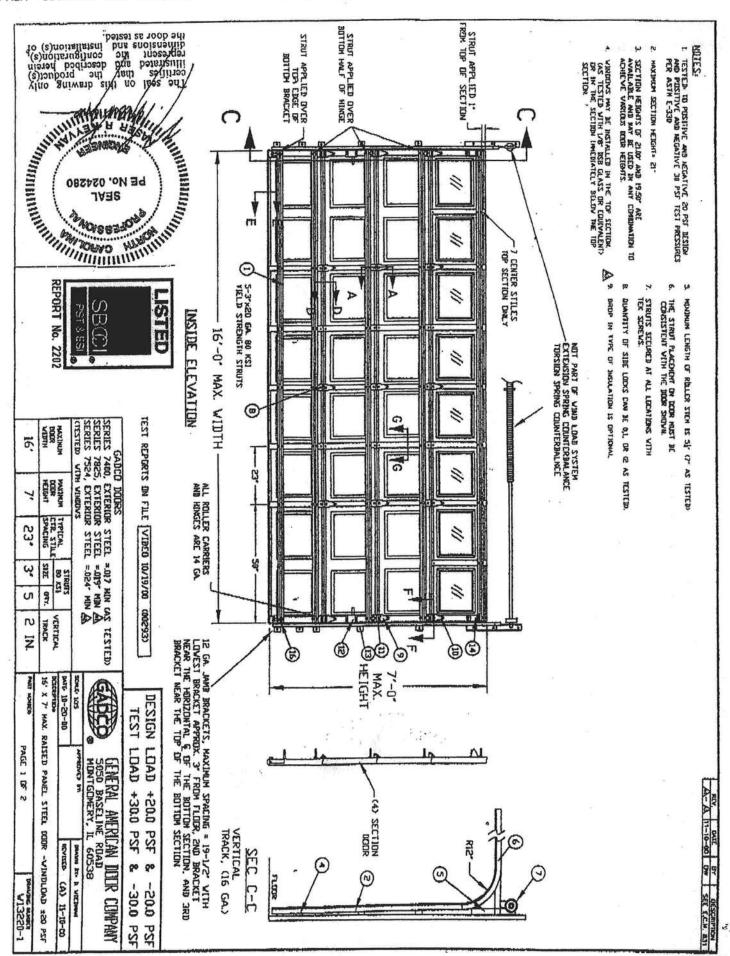
ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

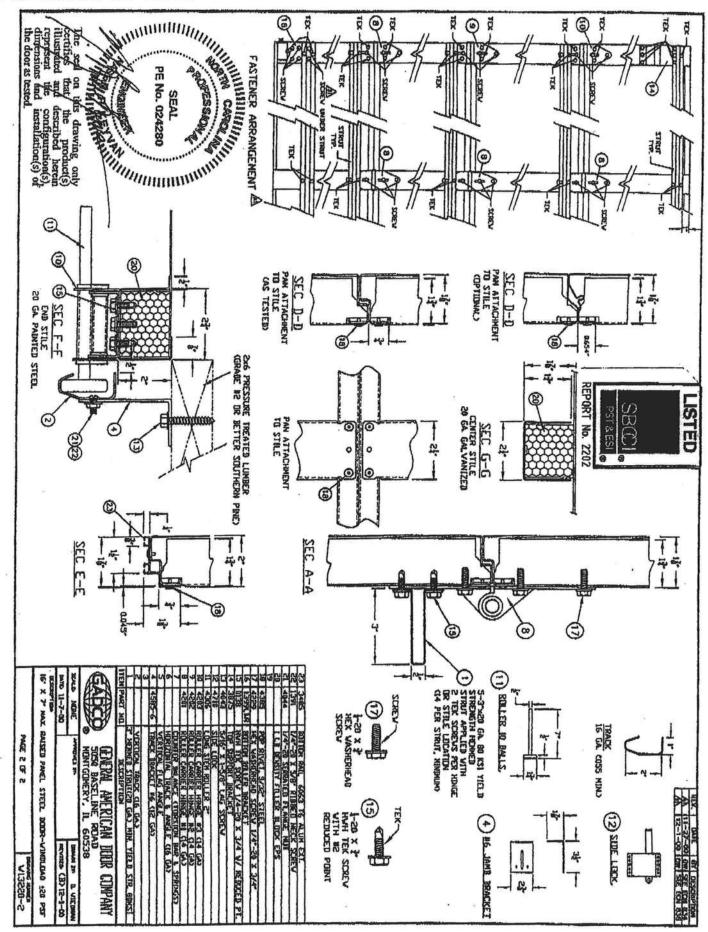
135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

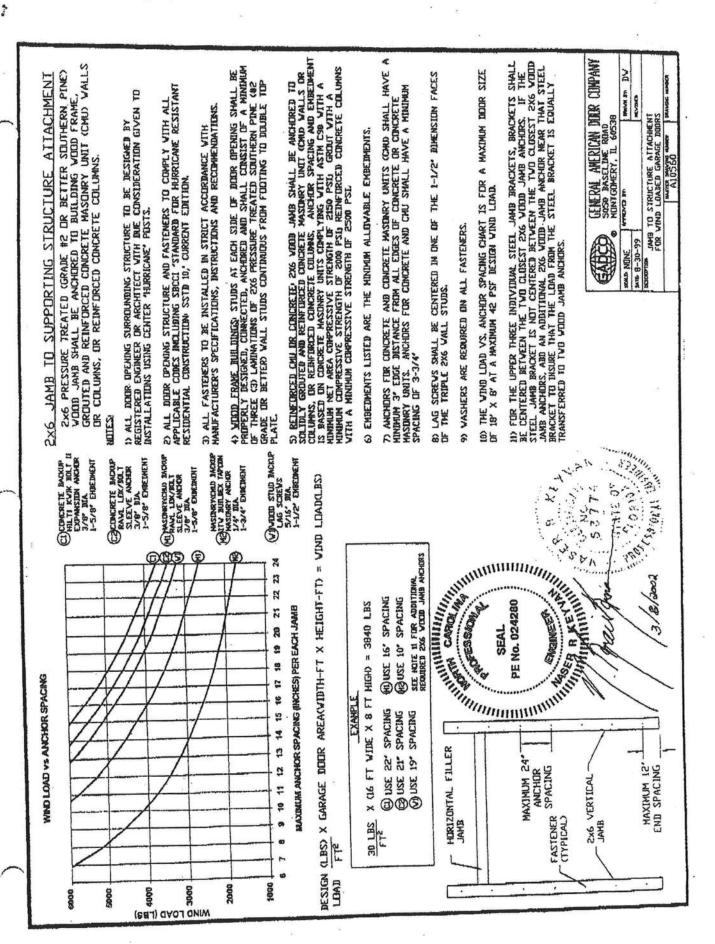






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Community Affairs

рса ноже

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Product Approval
USER: Public User

Product Approval Menu > Product or Application Search > Application List > Application Detail

COMMUNITY PLANNING OFFICE OF THE EMERGENCY DEVELOPMENT HOUSING & COMMUNITY MANAGEMENT FL#

Application Type Archived Comments Application Status Code Version

> New 2004 Approved FL5108

Product Manufacturer Address/Phone/Email

surich@miwd.com 650 W Market St Gratz, PA 17030 MI Windows and Doors (717) 365-3300 ext 2101

Authorized Signature

W) indow

Steven Urich surich@miwd.com

Technical Representative Address/Phone/Email

Address/Phone/Email Quality Assurance Representative

1 of 9



$\Delta \cdot L \cdot I$ (Validator / Operations Administrator)

AAMA CERTIFICATION PROGRAM

AUTHORIZATION FOR PRODUCT CERTIFICATION

MI Windows & Doors, Inc. P.O. Box 370 Gratz, PA 17030-0370

Attn: Bit Emley

The product described below is hereby approved for listing in the next issue of the AAMA Certified Products Directory. The approval is based on successful completion of tests, and the reporting to the Administrator of the results of tests, accompanied by related drawings, by an AAMA Accredited Laboratory.

The listing below will be added to the next published AAMA Certified Products Directory.

SPECIFICATION									
AAMA/NWWOA 101/LS, 2-97 H-R55*-36x62		RECORD OF PRODUCT TESTED							
COMPANY AND PLANT LOCATION	CODE NO.	SERIES MODEL & PRODUCT DESCRIPTION	MAXIMUM	SIZE TESTED					
MI Windows & Doors, Inc. (Oldsmar, FL) MI Windows & Doors, Inc. (Smyrna, TN)	MTL-8 MTL-9	185/3185 SH (Fin) (AL)(O/O)(OG) (ASTM)	FRAME 3'0' x 5'2"	<u>SASH</u> 210 x 27	By Request				

- This Certification will expire May 14, 2008 and requires validation until then by continued listing in the current AAMA Certified Products Directory.
- Product Tested and Reported by: Architectural Testing, Inc.

Report No.: 01-50360.02

Date of Report: June 14, 2004

NOTE: PLEASE REVIEW, AND ADVISE ALI IMMEDIATELY IF DATA, AS SHOWN, NEEDS CORRECTION.

Date:

August 1, 2005

CC: AAMA JGS/df

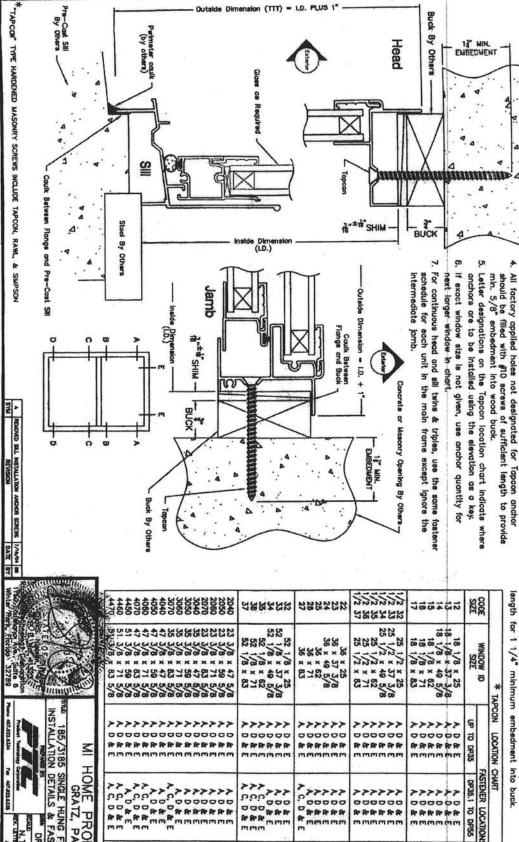
ACP-04 (Rev. 5/03)

Validated for Certification:

Associated Laboratories, Inc.

Authorized for Cartification:

American Architectural Manufacturers Association



TWO BY" (1 1/2") BUCKS

"TWO BY" bucks are engineered and fastened to masonry opening BY OTHERS.

Concrete header (shown) or steel lintel By Others

1. Before installation, caulk back of flange, or face of buck

"ONE BY" (3/4") BUCKS (SHOWN)

3/16" dia. masonry Tapcon must be of a length to have 1 1/4" embedment into masonry or concrete.

Shim as required with load bearing shims at each installation anchor as shown.

Follow the same instructions and fastener requirements for "one by" bucks except use \$10 screws of sufficient length for 1 1/4" minimum embedment into buck.



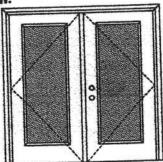
5

18 1/8 v 25 A D & F A I	UP TO DP35	WINDOW ID FASTENER	* TAPCON LOCATION CHART	r 1 1/4 minimum embeament into buck.
3404	DP35.1 TO DP55 DP55.1 TO DP69.3	ASTENER LOCATIONS		***

	25							
		2040 2050 2050 2060 2070 2070 2070 3050 3050 3050 4050 4070 4070 4070	ឧឧឧឧ	222222	2000元代の 2000円 200	755755	SIZE	COOE
A Same		22 3 3 6 × 47 5 6 8 22 3 3 6 × 17 5 6 8 22 3 3 6 × 17 5 6 8 22 3 3 6 × 17 5 6 8 22 3 3 6 × 17 5 6 8 2 2 2 3 6 × 17 5 6 8 2 2 2 3 6 × 17 5 6 8 2 2 2 3 6 × 17 5 6 8 2 2 2 3 6 × 17 5 6 8 2 2 3 6 × 17 5 6 8 2 2 3 6 × 17 5 6 8 2 2 3 6 × 17 5 6 8 2 3 5 6 8 2 3 6 × 17	52 1/8 × 25 52 1/8 × 37 3/8 52 1/8 × 49 5/8 52 1/8 × 62 52 1/8 × 71 52 1/8 × 83	36 × 25 36 × 37 3/8 36 × 49 5/8 36 × 62 36 × 71 36 × 83	25 1/2 × 25 25 1/2 × 37 3/8 25 1/2 × 49 5/8 25 1/2 × 62 25 1/2 × 62 25 1/2 × 71 25 1/2 × 83	18 1/8 × 37 3/8 18 1/8 × 37 3/8 18 1/8 × 49 5/8 18 1/8 × 62 18 1/8 × 71 18 1/8 × 83	1	WINDOW ID
MI HOME PRODUCTS GRATZ, PA 185/3185 SINGE HUNG FLANCE FRAME INSTALLATION DETAILS & FASTENER SCHEDULE DRL N.T.S. MILPO059 MILPO059		~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	**************************************	**************************************	**************************************	>>>>>> 000000 88888 mmmmmm	묶	
	NT P	22 22 22 22 22 22 22 22 22 22 22 22 22	>>> 000 000 000 000 000 000 000 000 000	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	~~~~~ 000000 84444 mmmmmm	>>>>> 000000 \$\$\$\$\$ mmmmm	DP35.1 TO DP55	8
	DICTA	>>	>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>>	>>>>>> 0,00 0,00 0,00 0,00 0,00 0,00 0,	>>>>>> 000000 000000 000000 0000000000	~~~~~ 000000 8888 88	DP55.1 TO DP69.3	

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:

Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door

Design Pressure +40.5/-40.5

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed -- see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0002-02.

APPROVED DOOR STYLES: 1/4 GLASS:











1/2 GLASS:













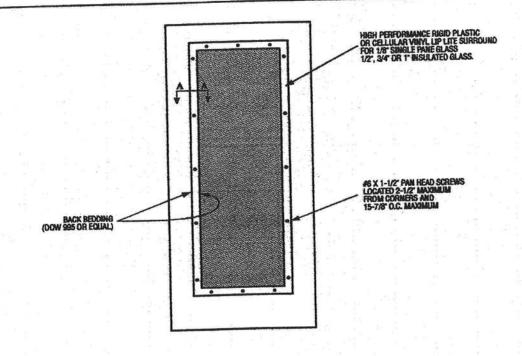


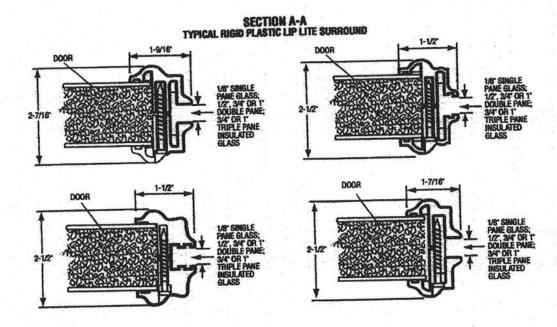
"This glass kit may also be used in the following door styles: 5-panel; 6-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.





GLASS INSERT IN DOOR OR SIDELITE PANEL









WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES: 3/4 GLASS:

















CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1864-5, 6, 7, 8; NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH MIAMI-DADE BCCO PA202

> COMPANY NAME CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer Kurt Balthazor, P.E. - License Number 56533

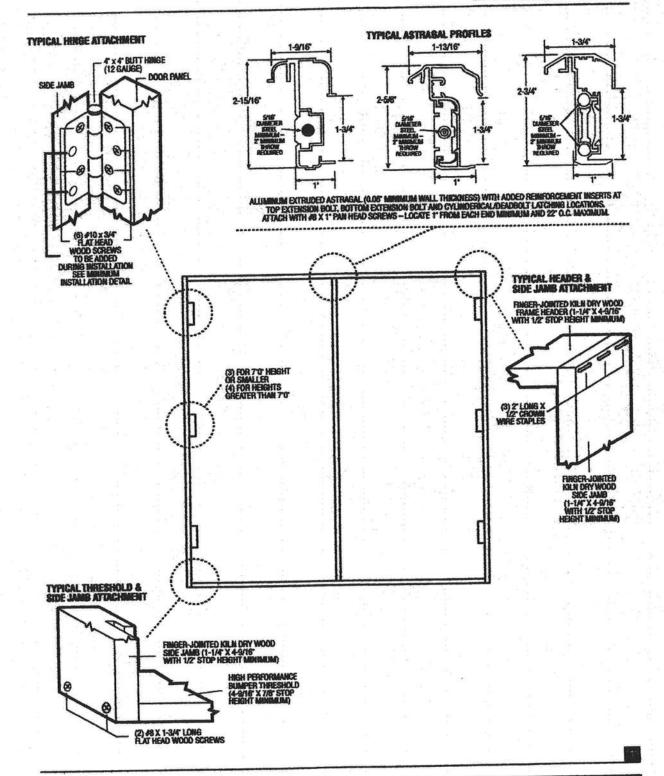
Johnson EntrySystems





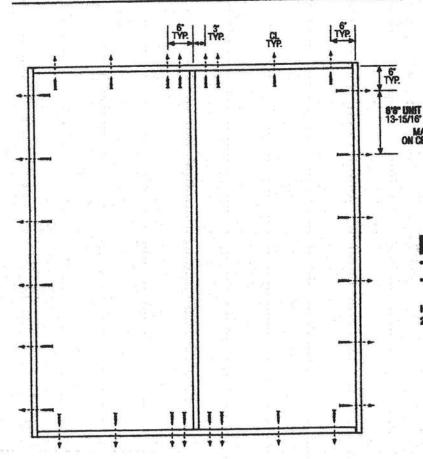
XX Unit

OUTSWING UNITS WITH DOUBLE DOOR





DOUBLE DOOR



Minimum Fasiener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per losation.

Latching Hardware:

Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylinderical and deadlock hardware be installed.

Notes:

- Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners
 analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
- The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
- 3. Wood bucks by others, must be anchored properly to transfer loads to the structure.





memunity Affairs





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HOUSING & COMMUNITY DEVELOPMENT **▶ COMMUNITY PLANNING** - EMERGENCY MANAGEMENT OFFICE OF THE

FL#

Comments Archived

FL1956-R1 Approved Revision 2004 Application Status Application Type Code Version

Product Manufacturer Address/Phone/Email

TAMKO Building Products, Inc. red_oconnor@tamko.com Joplin, MO 64802 (800) 641-4691 ext 2394 PO Box 1404

Authorized Signature

fred_oconnor@tamko.com Frederick O'Connor

Frederick J. O'Connor

PO Box 1404

Technical Representative Address/Phone/Email

(800) 641-4691 fred_oconnor@tamko.com Joplin, MO 64802

2/14/2007 11:22 A

Quality Assurance Representative Address/Phone/Email

Asphalt Shingles Roofing Subcategory Category

Certification Mark or Listing Compliance Method Underwriters Laboratories Inc. Certification Agency Year 2001 **ASTM D 3462** Standard Referenced Standard and Year (of Standard)

Equivalence of Product Standards Certified By Method 1 Option A Product Approval Method

06/09/2005 06/20/2005 06/25/2005 06/29/2005 Date Pending FBC Approval Date Submitted Date Approved Date Validated

Summary of Products

Description	
Model, Number or Name	
F. #	

2/14/2007 11:22 A

slopes of 2:12 or greater. Not approved for use in HVHZ.

Back

Next

DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436
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Product Approval Accepts:













Horthbrook Division

333 Pfroster Red Northorois, 1 60062-2096 USA www.i.com se: 1 847 272 5900

June 17, 2005

Tamko Roofing Products Ms. Kerri Eden P.O. Box 1404 220 W. 4th Street Joplin, MO 64802-1404

Our Reference: R2919

This is to confirm that "Elite Glass-Seal AR", "Heritage 30 AR", "Heritage 50 AR", "Glass-Seal AR" manufactured at Tuscaloosa, AL and "Elite Glass-Seal AR", "Heritage 30 AR", "Heritage XL AR", "Heritage 50 AR" manufactured at Frederick, MD and "Heritage 30 AR", "Heritage XL AR", and "Heritage 50 AR" manufactured in Dallas, TX are UL Listed asphalt glass mat shingles and have been evaluated in accordance with ANSI/UL 790, Class A (ASTM E108), ASTM D3462, ASTM D3161 or UL 997 modified to 110 mph when secured with four nails.

Let me know if you have any further questions.

Very truly yours,

Alpesh Patel (Ext. 42522)

Engineer Project

Fire Protection Division

Reviewed by,

Randall K. Laymon (Ext. 42687)

Engineer Sr Staff

Fire Protection Division



Application Instructions for

TAGE® VINTAGE™ AR – Phillipsburg, KS Laminated asphalt shingles

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO BUILD-ING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.

THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.

IN COLD WEATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.

IMPORTANT: It is not necessary to remove the plastic strip from the back of the shingles.

I. ROOF DECK

These shingles are for application to roof decks capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per foot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special instructions titled "Low Slope Application". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be installed at eaves and rakes.

PLYWOOD: All plywood shall be exterior grade as defined by the American Plywood Association. Plywood shall be a minimum of 3/8 in. thickness and applied in accordance with the recommendations of the American Plywood Association.

SHEATHING BOARDS: Boards shall be well-seasoned tongue-andgroove boards and not over 6 in. nominal width. Boards shall be a 1 in, nominal minimum thickness. Boards shall be properly spaced and nailed.

TAMKO does not recommend re-roofing over existing roof.

2. VENTILATION

Inadequate ventilation of attic spaces can cause accumulation of moisture in winter months and a build up of heat in the summer. These conditions can lead to:

- 1. Vapor Condensation
- 2. Buckling of shingles due to deck movement.
- 3. Rotting of wood members.
- Premature failure of roof.

To insure adequate ventilation and circulation of air, place louvers of sufficient size high in the gable ends and/or install continuous ridge and soffit vents. FHA minimum property standards require one square foot of net free ventilation area to each 150 square feet of space to be vented, or one square foot per 300 square feet if a vapor barrier is installed on the warm side of the ceiling or if at least one half of the ventilation is provided near the ridge. If the ventilation openings are screened, the total area should be doubled.

IT IS PARTICULARLY IMPORTANT TO PROVIDE ADEQUATE VEN-TILATION.

3. FASTENERS

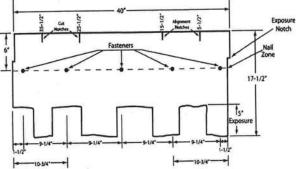
WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is recommended. Shingles must also be fastened according to the fastening instructions described below.

Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagram and described below, this will result in the termination of TAMKO's liabilities under the limited warranty. TAMKO will not be responsible for damage to shingles caused by winds in excess of the applicable miles per hour as stated in the limited warranty. See limited warranty for details.

FASTENING PATTERNS: Fasteners must be placed 6 in. from the top edge of the shingle located horizontally as follows:

1) Standard Fastening Pattern. (For use on decks with slopes 2 in. per foot to 21 in. per foot.) One fastener 1-1/2 in. back from each end, one 10-3/4 in. back from each end and one 20 in. from one end of the shingle for a total of 5 fasteners. (See standard fastening pattern illustrated below).

STANDARD FASTENING PATTERN



2) Mansard or Steep Slope Fastening Pattern. (For use on decks with slopes greater than 21 in. per foot.) Use standard nailing instructions with four additional nails placed 6 in. from the butt edge of the shingle making certain nails are covered by the next (successive) course of shingles.

(Continued)

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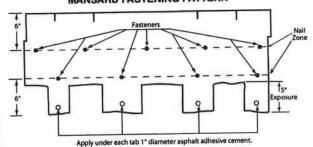


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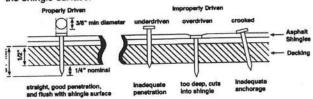
HERITAGE® VINTAGE™ AR – Phillipsburg, KS LAMINATED ASPHALT SHINGLES

Each shingle tab must be sealed underneath with quick setting asphalt adhesive cement immediately upon installation. Spots of cement must be equivalent in size to a \$.25 piece and applied to shingles with a 5 in. exposure, use 9 fasteners per shingle.

MANSARD FASTENING PATTERN



NAILS: TAMKO recommends the use of nails as the preferred method of application. Standard type roofing nails should be used. Nail shanks should be made of minimum 12 gauge wire, and a minimum head diameter of 3/8 in. Nails should be long enough to penetrate 3/4 in. into the roof deck. Where the deck is less than 3/4 in. thick, the nails should be long enough to penetrate completely through plywood decking and extend at least 1/8 in. through the roof deck. Drive nail head flush with the shingle surface.



4. UNDERLAYMENT

UNDERLAYMENT: An underlayment consisting of asphalt saturated felt must be applied over the entire deck before the installation of TAMKO shingles. Failure to add underlayment can cause premature failure of the shingles and leaks which are not covered by TAMKO's limited warranty. Apply the felt when the deck is dry. On roof decks 4 in. per foot and greater apply the felt parallel to the eaves lapping each course of the felt over the lower course at least 2 in. Where ends join, lap the felt 4 in. If left exposed, the underlayment felt may be adversely affected by moisture and weathering. Laying of the underlayment and the shingle application must be done together.

Products which are acceptable for use as underlayment are:

- TAMKO No. 15 Asphalt Saturated Organic Felt
- A <u>non-perforated</u> asphalt saturated organic felt which meets ASTM: D226, Type I or ASTM D4869, Type I
- Any TAMKO non-perforated asphalt saturated organic felt
- TAMKO TW Metal and Tile Underlayment,
 TW Underlayment and Moisture Guard Plus® (additional ventilation maybe required. Contact TAMKO's technical services department for more information)

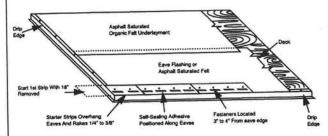
In areas where ice builds up along the eaves or a back-up of water from frozen or clogged gutters is a potential problem, TAMKO's Moisture Guard Plus® waterproofing underlayment (or any specialty eaves flashing product) may be applied to eaves, rakes, ridges, valleys, around chimneys, skylights or dormers to help prevent water damage. Contact TAMKO's Technical Services Department for more information.

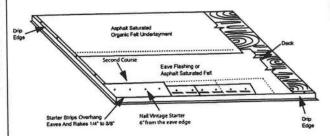
TAMKO does not recommend the use of any substitute products as shingle underlayment.

5. APPLICATION INSTRUCTIONS

STARTER COURSE: Two starter course layers must be applied prior to application of Heritage Vintage AR Shingles.

The first starter course may consist of TAMKO Shingle Starter, three tab self-sealing type shingles or a 9 inch wide strip of mineral surface roll roofing. If three tab self-sealing shingles are used, remove the exposed tab portion and install with the factory applied adhesive adjacent to the eaves. If using three tab self-sealing shingles or shingle starter, remove 18 in. from first shingle to offset the end joints of the Vintage Starter. Attach the first starter course with approved fasteners along a line parallel to and 3 in. to 4 in. above the eave edge. The starter course should overhang both the eave and rake edge 1/4 in. to 3/8 in. Over the first starter course, install Heritage Vintage Starter AR and begin at the left rake edge with a full size shingle and continue across the roof nailing the Heritage Vintage Starter AR along a line parallel to and 6 in. from the eave edge.





Note: Do not allow Vintage Starter AR joints to be visible between shingle tabs. Cutting of the starter may be required.

HERITAGE VINTAGE STARTER AR 12 1/2" x 36" 20 PIECES PER BUNDLE 60 LINEAL FT. PER BUNDLE

(Continued)

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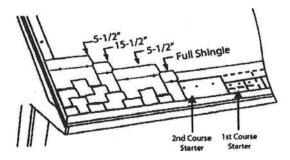
2



(CONTINUED from Pg. 2)

• HERITAGE® VINTAGE™ AR – Phillipsburg, KS LAMINATED ASPHALT SHINGLES

SHINGLE APPLICATION: Start the first course at the left rake edge with a full size shingle and overhang the rake edge 1/4 in. to 3/8 in.. To begin the second course, align the right side of the shingle with the 5-1/2 in. alignment notch on the first course shingle making sure to align the exposure notch. (See shingle illustration on next page) Cut the appropriate amount from the rake edge so the overhang is 1/4" to 3/8". For the third course, align the shingle with the 15-1/2 in. alignment notch at the top of the second course shingle, again being sure to align the exposure notch. Cut the appropriate amount from the rake edge. To begin the fourth course, align the shingle with the 5-1/2 in. alignment notch from the third course shingle while aligning the exposure notch. Cut the appropriate amount from the rake edge. Continue up the rake in as many rows as necessary using the same formula as outlined above. Cut pieces may be used to complete courses at the right side. As you work across the roof, install full size shingles taking care to align the exposure notches. Shingle joints should be no closer than 4 in.



6. LOW SLOPE APPLICATION

On pitches 2 in. per foot to 4 in. per foot cover the deck with two layers of underlayment. Begin by applying the underlayment in a 19 in. wide strip along the eaves and overhanging the drip edge by 1/4 to 3/4 in. Place a full 36 in. wide sheet over the 19 in. wide starter piece, completely overlapping it. All succeeding courses will be positioned to overlap the preceding course by 19 in. If winter temperatures average 25°F or less, thoroughly cement the laps of the entire underlayment to each other with plastic cement from eaves and rakes to a point of a least 24 in. inside the interior wall line of the building. As an alternative, TAMKO's Moisture Guard Plus self-adhering waterproofing underlayment may be used in lieu of the cemented felts.

7. VALLEY APPLICATION

TAMKO recommends an open valley construction with Heritage Vintage AR shingles.

To begin, center a sheet of TAMKO Moisture Guard Plus, TW Underlayment or TW Metal & Tile Underlayment in the valley.

After the underlayment has been secured, install the recommended corrosion resistant metal (26 gauge galvanized metal or an equivalent) in the valley. Secure the valley metal to the roof deck. Overlaps should be 12" and cemented.

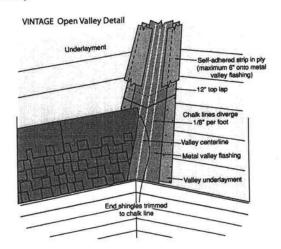
Following valley metal application; a 9" to 12" wide strip of TAMKO Moisture Guard Plus, TW Underlayment or TW Metal & Tile Underlayment should be applied along the edges of the metal valley flashing (max. 6" onto metal valley flashing) and on top of the valley underlayment. The valley will be completed with shingle application.

SHINGLE APPLICATION INSTRUCTIONS (OPEN VALLEY)

- Snap two chalk lines, one on each side of the valley centerline over the full length of the valley flashing. Locate the upper ends of the chalk lines 3" to either side of the valley centerline.
- The lower end should diverge from each other by 1/8" per foot.
 Thus, for an 8' long valley, the chalk lines should be 7" either side of the centerline at the eaves and for a 16' valley 8".

As shingles are applied toward the valley, trim the last shingle in each course to fit on the chalk line. Never use a shingle trimmed to less than 12" in length to finish a course running into a valley. If necessary, trim the adjacent shingle in the course to allow a longer portion to be used.

- Clip 1" from the upper corner of each shingle on a 45° angle to direct water into the valley and prevent it from penetrating between the courses.
- Form a tight seal by cementing the shingle to the valley lining with a 3" width of asphalt plastic cement (conforming to ASTM D 4586).



· CAUTION

Adhesive must be applied in smooth, thin, even layers.

Excessive use of adhesive will cause blistering to this product.

TAMKO assumes no responsibility for blistering.

(Continued)

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(CONTINUED from Pg. 3)

• HERITAGE® VINTAGE™ AR — Phillipsburg, KS LAMINATED ASPHALT SHINGLES

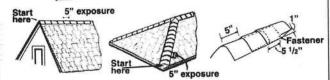
8. HIP AND RIDGE FASTENING DETAIL

Apply the shingles with a 5 in. exposure beginning at the bottom of the hip or from the end of the ridge opposite the direction of the prevailing winds. Secure each shingle with one fastener on each side, 5-1/2 in. back from the exposed end and 1 in. up from the edge. TAMKO recommends the use of TAMKO Heritage Vintage Hip & Ridge shingle products.

Fasteners should be 1/4 in. longer than the ones used for shingles.

IMPORTANT: PRIOR TO INSTALLATION, CARE NEEDS TO BE TAKEN TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLE IN COLD WEATHER.

Direction of prevailing wind



THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO BUILDING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.

TAMKO®, Moisture Guard Plus®, Nail Fast® and Heritage® are registered trademarks and Vintage™ is a trademark of TAMKO Building Products, Inc.

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4

Residential System Sizing Calculation

Summary Project Title:

Papka, Adam Residence

Project Title: 611075Adam'sFramingConstruction

Class 3 Rating Registration No. 0 Climate: North

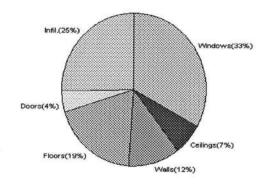
1/14/2008

				.,	
Location for weather data: Gaine	sville - De	faults: Lati	tude(29) Altitude(152 ft.) Temp Ran	ge(M)	
Humidity data: Interior RH (50%	6) Outdoo	r wet bulb (77F) Humidity difference(54gr.)		
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference 37 F		F	Summer temperature difference	17	F
Total heating load calculation	57606	Btuh	Total cooling load calculation	54208	Btuh
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	118.0	68000	Sensible (SHR = 0.75)	110.1	51000
Heat Pump + Auxiliary(0.0kW)	118.0	68000	Latent	215.0	17000
The state of the s	- 90.345.00		Total (Electric Heat Pump)	125.4	68000

WINTER CALCULATIONS

Winter Heating Load (for 3058 soft)

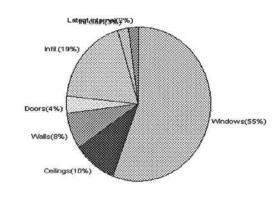
Load component			Load	
Window total	596	sqft	19176	Btuh
Wall total	2020	sqft	6635	Btuh
Door total	200	sqft	2590	Btuh
Ceiling total	3180	sqft	3747	Btuh
Floor total	254	sqft	11090	Btuh
Infiltration	355	cfm	14369	Btuh
Duct loss			0	Btuh
Subtotal			57606	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			57606	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 3058 sqft)

Load component			Load	
Window total	596	sqft	30067	Btuh
Wall total	2020	sqft	4214	Btuh
Door total	200	sqft	1960	Btuh
Ceiling total	3180	sqft	5266	Btuh
Floor total			0	Btuh
Infiltration	183	cfm	3415	Btuh
Internal gain			1380	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			46302	Btuh
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			6705	Btuh
Latent gain(ventilation)	0	Btuh		
Latent gain(internal/occu	1200	Btuh		
Total latent gain	7905	Btuh		
TOTAL HEAT GAIN		54208	Btuh	





For Florida residences only

EnergyGauge® System Sizing PREPARED BY:

DATE:

EnergyGauge® FLR2PB v4.1

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Papka, Adam Residence

Project Title: 611075Adam'sFramingConstruction

Class 3 Rating Registration No. 0 Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F This calculation is for Worst Case. The house has been rotated 315 degrees.

1/14/2008

Component Loads for Whole House

Olean Matel 0.07		Area(sqft) X	HTM=	Load
2, Clear, Metal, 0.87	NW	54.0	32.2	1738 Btuh
2, Clear, Metal, 0.87	NE	20.0	32.2	644 Btuh
2, Clear, Metal, 0.87	NW	40.0		1288 Btuh
2, Clear, Metal, 0.87	SW	20.0	32.2	644 Btuh
2, Clear, Metal, 0.87	NW	10.0		322 Btuh
2, Clear, Metal, 0.87	sw	60.0		1931 Btuh
2, Clear, Metal, 0.87	NW	72.0		2318 Btuh
2, Clear, Metal, 0.87	NW	24.0		773 Btuh
				2318 Btuh
				579 Btuh
61 14 6				1159 Btuh
그래요				258 Btuh
				280 Btuh
Before - 플레이트 (Belon 1985) - (1352 Btuh
A. C.				129 Btuh
				837 Btuh
그래에 그리고 있는데 아이들이 얼마나 아이들이 가면 그리고 있다.				193 Btuh
71. 10				805 Btuh
				1159 Btuh
				258 Btuh
[2] [1] [1] ' [1] [1] [2] [2] [2] [2] [2] [2] [2] [2] [2] [2				193 Btuh
				19176 Btuh
	R-Value		HTM=	Load
				5991 Btuh
				644 Btuh
			7.7.7	6635 Btuh
			HTM=	Load
				259 Btuh
(1) TOTO I NOTE (1) IN 10 10 10 10 10 10 10 10 10 10 10 10 10				777 Btuh
			2012 St. 2010 C. 2010	1554 Btuh
				2590Btuh
	R-Value		HTM=	Load
7.0	30.0	3180		3747 Btuh
			· · ·—	3747Btuh
	R-Value		HTM=	Load
Slab On Grade	0		43.7	11090 Btuh
loor Total			EMPAGENE	11090 Btuh
			e e	
	Z	one Envelope Su	btotal:	43237 Btuh
уре	ACH X	Zone Volume	CFM=	
latural	0.58	36696	354.7	14369 Btuh
	2, Clear, Metal, 0.87 3, Clear, Metal, 0.87 4, Clear, Metal, 0.87 5, Clear, Metal, 0.87 6, Clear, Metal, 0.87	2, Clear, Metal, 0.87 2, Clear, Metal, 0.87 3, Clear, Metal, 0.87 4, Clear, Metal, 0.87 5, Clear, Metal, 0.87 6, Clear, Metal, 0.87 7, Clear, Metal, 0.87	2, Clear, Metal, 0.87 2, Clear, Metal, 0.87 3, Clear, Metal, 0.87 4, Clear, Metal, 0.87 5, Clear, Metal, 0.87 5, Clear, Metal, 0.87 6, Clear, Metal, 0.87 7, Clear, Metal, 0.87	Clear, Metal, 0.87

Manual J Winter Calculations

Residential Load - Component Details (continued)

Papka, Adam Residence

Project Title: 611075Adam'sFramingConstruction

Class 3 Rating Registration No. 0 Climate: North

1/14/2008

Zone #1	Sensible Zone Subtota	sl 57606 Btuh
Ductload	Average sealed, R6.0, Supply(Attic), Return(Attic) (DL	M of 0.00) 0 Btuh

VHOLE HOUSE TOTA	NLS	
	Subtotal Sensible	57606 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	57606 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(Frame types - metal, wood or insulated metal)

(U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



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System Sizing Calculations - Winter

Residential Load - Room by Room Component Details Residence Project Title: Class 3

Papka, Adam Residence

Project Title: 611075Adam'sFramingConstruction Class 3 Rating Registration No. 0 Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F This calculation is for Worst Case. The house has been rotated 315 degrees.

1/14/2008

Component Loads for Zone #1: Main

1 2	In Olean Matel 0.07		Area(sqft) X	HTM=	Load
2	2, Clear, Metal, 0.87	NW	54.0	32.2	1738 Btuh
_	2, Clear, Metal, 0.87	NE	20.0	32.2	644 Btuh
3	2, Clear, Metal, 0.87	NW	40.0	32.2	1288 Btul
4	2, Clear, Metal, 0.87	SW	20.0	32.2	644 Btul
5 6 7	2, Clear, Metal, 0.87	NW	10.0	32.2	322 Btul
6	2, Clear, Metal, 0.87	sw	60.0	32.2	1931 Btul
7	2, Clear, Metal, 0.87	NW	72.0	32.2	2318 Btuh
8	2, Clear, Metal, 0.87	NW	24.0	32.2	773 Btul
9	2, Clear, Metal, 0.87	NE	72.0	32.2	2318 Btuł
10	2, Clear, Metal, 0.87	NW	18.0	32.2	579 Btul
11	2, Clear, Metal, 0.87	NE	36.0	32.2	1159 Btul
12	2, Clear, Metal, 0.87	NE	8.0	32.2	258 Btuh
13	2, Clear, Metal, 0.87	SE	8.7	32.2	280 Btuh
14	2, Clear, Metal, 0.87	SE	42.0	32.2	1352 Btuh
15	2, Clear, Metal, 0.87	SE	4.0	32.2	129 Btuh
16	2, Clear, Metal, 0.87	SE	26.0	32.2	837 Btuh
17	2, Clear, Metal, 0.87	sw	6.0	32.2	193 Btuh
18	2, Clear, Metal, 0.87	SW	25.0	32.2	805 Btul
19	2, Clear, Metal, 0.87	SW	36.0	32.2	1159 Btul
20	2, Clear, Metal, 0.87	SW	8.0	32.2	258 Btul
21	2, Clear, Metal, 0.87	SE	6.0	32.2	193 Btul
21	Window Total	SE	596(sqft)	32.2	193 Blui 19176 Btui
Walls	Type	R-Value	Area X	HTM=	Load
	Frame - Wood - Ext(0.09)	13.0	1824	3.3	5991 Btuh
1 2		13.0	196		
2	Frame - Wood - Ext(0.09)	13.0		3.3	644 Btuh
Daara	Wall Total		2020	LITAA	6635 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2 3	Insulated - Exterior		60	12.9	777 Btuh
3	Insulated - Exterior		120	12.9	1554 Btuh
0-111	Door Total	D 1/-1	200	11714	2590Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	3180	1.2	3747 Btuh
FI	Ceiling Total	D 17.1	3180	1.175.4	3747Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	254.0 ft(p)	43.7	11090 Btuh
	Floor Total		254		11090 Btuh
		Z	Zone Envelope Su	btotal:	43237 Btuh
Infiltration	Type Natural	ACH X 0.58	Zone Volume 36696	CFM= 354.7	14369 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Papka, Adam Residence

Project Title: 611075Adam'sFramingConstruction Class 3 Rating Registration No. 0 Climate: North

1/14/2008

Ductload	Average sealed, R6.0, Supply(Attic), Return(Attic)	(DLM of 0.00)	0 Btuh
Zone #1	Sensible Zone S	ubtotal	57606 Btuh

WHOLE HOUSE TOTAL	S	
	Subtotal Sensible Ventilation Sensible Total Btuh Loss	57606 Btuh 0 Btuh 57606 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(Frame types - metal, wood or insulated metal)

(U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Papka, Adam Residence

Project Title: 611075Adam'sFramingConstruction

Class 3 Rating Registration No. 0 Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

1/14/2008

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

	Type*		Over	hang	Window Area(sqft)			Н	ITM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	6.5ft.	54.0	0.0	54.0	29	60	3242	Btul
2	2, Clear, 0.87, None,N,N	NE	99ft.	6ft.	20.0	0.0	20.0	29	60	1201	Btul
3	2, Clear, 0.87, None,N,N	NW	16ft.	6ft.	40.0	0.0	40.0	29	60	2401	Btul
4	2, Clear, 0.87, None,N,N	SW	99ft.	5.5ft.	20.0	20.0	0.0	29	63	579	700.000
5	2, Clear, 0.87, None,N,N	NW	21ft.	5.5ft.	10.0	0.0	10.0	29	60	600	Btu
6	2, Clear, 0.87, None,N,N	SW	10ft.	6.5ft.	60.0	60.0	0.0	29	63	1738	Btu
7	2, Clear, 0.87, None,N,N	NW	1.5ft.	Oft.	72.0	0.0	72.0	29	60	4323	Btu
8	2, Clear, 0.87, None,N,N	NW	1.5ft.	Oft.	24.0	0.0	24.0	29	60	1441	Btu
9	2, Clear, 0.87, None,N,N	NE	1.5ft.	6.5ft.	72.0	0.0	72.0	29	60	4323	Btu
10	2, Clear, 0.87, None,N,N	NW	1.5ft.	6.5ft.	18.0	0.0	18.0	29	60	1081	Btu
11	2, Clear, 0.87, None,N,N	NE	1.5ft.	6.5ft.	36.0	0.0	36.0	29	60	2161	Btu
12	2, Clear, 0.87, None,N,N	NE	1.5ft.	5.5ft.	8.0	0.0	8.0	29	60	480	Btu
13 14	2, Clear, 0.87, None,N,N	SE	1.5ft. 1.5ft.	6ft.	8.7	1.7	7.0	29	63	486	Btu
15	2, Clear, 0.87, None,N,N 2, Clear, 0.87, None,N,N	SE	1.5ft.	Oft. 3ft.	42.0 4.0	42.0 3.0	0.0	29	63	1216	
16	2, Clear, 0.87, None,N,N	SE	1.5ft.	Oft.	26.0	26.0	1.0 0.0	29 29	63 63	148 753	Btul
17	2, Clear, 0.87, None,N,N	SW	1.5ft.	4ft.	6.0	3.0	3.0	29	63	273	Btu
18	2, Clear, 0.87, None,N,N	SW	1.5ft.	6ft.	25.0	7.6	17.4	29	63	1308	
19	2, Clear, 0.87, None,N,N	sw	1.5ft.	6.5ft.	36.0	12.1	23.9	29	63	1844	Btu
20	2, Clear, 0.87, None,N,N	SW	1.5ft.	3ft.	8.0	6.1	1.9	29	63	296	Btu
21	2, Clear, 0.87, None,N,N	SE	1.5ft.	Oft.	6.0	6.0	0.0	29	63	U	Btu
	Window Total	02	1.01.	J.L.	596 (0.0	20	00	30067	
Walls	Туре		R-Va	alue/U	-Value		(sqft)		нтм	Load	Dia
1	Frame - Wood - Ext			13.0/0		1824.3			2.1	3805	Btul
2	Frame - Wood - Ext			13.0/0			196.0		2.1	409	Btul
_	Wall Total			10.070	7.00	2020 (sqft)			2.1	4214	
Doors	Туре								нтм	Load	Diu
1	Insulated - Adjacent										Divi
2	Insulated - Adjacent					20.0			9.8 9.8	196 588	Btu
3	Insulated - Exterior					60.0 120.0			9.8	1176	Btul
3	Door Total								9.0		
Ceilings	Type/Color/Surface		R-Va	duo			0 (sqft)		НТМ	1960	Blui
	Vented Attic/DarkShingle		17-V			Area 318				Load	
1	Ceiling Total			30.0			0.0 0 (sqft)		1.7	5266 5266	
Floors			R-Va	duo		Si			нтм	5266	Dlui
	Type Slab On Grade		rv-v∂							Load	D4 :
1	Albana or its own about			0.0			54 (ft(p))		0.0	0	Btul
	Floor Total					254.	0 (sqft)			0	Btul
						Z	one Enve	elope Su	ıbtotal:	41508	Btul

Manual J Summer Calculations

Residential Load - Component Details (continued) Sidence Project Title: Class 611075Adam'sFramingConstruction Reg

Papka, Adam Residence

Class 3 Rating Registration No. 0 Climate: North

1/14/2008

Infiltration	Type SensibleNatural	ACH 0.30	` '			CFM= 183.5	Load 3415	Btuh
Internal gain		Occupants 6	Bt:	uh/occup 230	ant +	Appliance 0	Load 1380	Btuh
	Average sealed, R6.0, Supply(Attic), Return(Attic)				DGM = 0.00			Btuh
				Se	nsib	le Zone Load	46302	Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Papka, Adam Residence

Project Title: 611075Adam'sFramingConstruction Class 3 Rating Registration No. 0 Climate: North

1/14/2008

WHOLE HOUSE TOTALS

=	Sensible Envelope Load All Zones	46302	Btuh
	Sensible Duct Load	0	Btuh
	Total Sensible Zone Loads	46302	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	46302	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	6705	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	0	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
	Latent total gain	7905	Btuh
	TOTAL GAIN	54208	Btuh

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details Residence Project Title: Class 3

Papka, Adam Residence

611075Adam'sFramingConstruction

Class 3 Rating Registration No. 0 Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference This calculation is for Worst Case. The house has been rotated 315 degrees. Summer Temperature Difference: 17.0 F

1/14/2008

Component Loads for Zone #1: Main

	Type*		Ove	rhang	Wind	dow Area	a(sqft)	Н	ITM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded	= ==	
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	6.5ft.	54.0	0.0	54.0	29	60	3242	Btuh
2	2, Clear, 0.87, None,N,N	NE	99ft.	6ft.	20.0	0.0	20.0	29	60	1201	Btuh
3	2, Clear, 0.87, None,N,N	NW	16ft.	6ft.	40.0	0.0	40.0	29	60	2401	Btul
4	2, Clear, 0.87, None,N,N	SW	99ft.	5.5ft.	20.0	20.0	0.0	29	63	579	Btul
5	2, Clear, 0.87, None,N,N	NW	21ft.	5.5ft.	10.0	0.0	10.0	29	60	600	Btul
6	2, Clear, 0.87, None,N,N	SW	10ft.	6.5ft.	60.0	60.0	0.0	29	63	1738	-
7	2, Clear, 0.87, None,N,N	NW	1.5ft.	Oft.	72.0	0.0	72.0	29	60	4323	Btul
8 9	2, Clear, 0.87, None,N,N	NW	1.5ft.	Oft.	24.0	0.0	24.0	29	60	1441	Btul
10	2, Clear, 0.87, None,N,N	NE	1.5ft. 1.5ft.	6.5ft. 6.5ft.	72.0 18.0	0.0	72.0 18.0	29	60	4323	Btul
11	2, Clear, 0.87, None,N,N 2, Clear, 0.87, None,N,N	NW	1.5ft.	6.5ft.	36.0	0.0	36.0	29 29	60 60	1081 2161	Btul
12	2, Clear, 0.87, None,N,N	NE	1.5ft.	5.5ft.	8.0	0.0	8.0	29	60	480	Btul
13	2, Clear, 0.87, None,N,N	SE	1.5ft.	6ft.	8.7	1.7	7.0	29	63	486	Btul
14	2, Clear, 0.87, None,N,N	SE	1.5ft.	Oft.	42.0	42.0	0.0	29	63	1216	100000000000000000000000000000000000000
15	2, Clear, 0.87, None,N,N	SE	1.5ft.	3ft.	4.0	3.0	1.0	29	63	148	Btul
16	2, Clear, 0.87, None, N, N	SE	1.5ft.	Oft.	26.0	26.0	0.0	29	63	753	Btu
17	2, Clear, 0.87, None, N, N	sw	1.5ft.	4ft.	6.0	3.0	3.0	29	63	273	Btul
18	2, Clear, 0.87, None, N, N	SW	1.5ft.	6ft.	25.0	7.6	17.4	29	63	1308	Btul
19	2, Clear, 0.87, None, N, N	SW	1.5ft.	6.5ft.	36.0	12.1	23.9	29	63	1844	Btul
20	2, Clear, 0.87, None, N, N	SW	1.5ft.	3ft.	8.0	6.1	1.9	29	63	296	Btul
21	2, Clear, 0.87, None, N, N	SE	1.5ft.	Oft.	6.0	6.0	0.0	29	63	174	Btul
	Window Total	7.2 - 7.2 - 7.2 - 7.2			596 (sqft)				30067	Btul
Walls	Туре		R-Va	alue/U	-Value	Area	(sqft)		MTH	Load	
1	Frame - Wood - Ext			13.0/0	0.09		24.3		2.1	3805	Btul
2	Frame - Wood - Ext			13.0/0	0.09	19	6.0		2.1	409	
	Wall Total					202	0 (sqft)		100000	4214	Btul
Doors	Туре				E.	Area	(sqft)		HTM	Load	
1	Insulated - Adjacent					20	0.0		9.8	196	Btul
2	Insulated - Exterior					60	0.0		9.8	588	Btul
3	Insulated - Exterior					12	0.0		9.8	1176	Btul
	Door Total					20	0 (sqft)			1960	Btul
Ceilings	Type/Color/Surface		R-Va	alue		Area	(sqft)		HTM	Load	
1	Vented Attic/DarkShingle			30.0		318	0.0		1.7	5266	Btul
	Ceiling Total					318	0 (sqft)			5266	Btul
Floors	Туре		R-Va	alue		Si	ze		НТМ	Load	
1	Slab On Grade		0.0		25	54 (ft(p))		0.0	0	Btul	
	Floor Total						0 (sqft)			0	Btul
						Z	one Enve	elope Su	ıbtotal:	41508	Btul

Manual J Summer Calculations

Residential Load - Component Details (continued)
sidence Project Title: Clas
611075Adam'sFramingConstruction Reg

Papka, Adam Residence

Class 3 Rating Registration No. 0 Climate: North

1/14/2008

Infiltration	Type SensibleNatural		O.30	Vo	olume(ci 36696	uft)	CFM= 183.5	Load 3415	Btuh
Internal gain		Occupa	ants 6	Btı X	nh/occup 230	ant +	Appliance 0	Load 1380	Btuh
	Average sealed,	R6.0, Supply(Attic),	Return	(Attic)			DGM = 0.00	0.0	Btuh
					Se	nsible	Zone Load	46302	Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Papka, Adam Residence

Project Title: 611075Adam'sFramingConstruction

Class 3 Rating Registration No. 0 Climate: North

1/14/2008

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	46302	Btuh
	Sensible Duct Load	0	Btuh
	Total Sensible Zone Loads	46302	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	46302	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	6705	Btuh
	Latent ventilation gain	0	Btuh
-2007	Latent duct gain	0	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
*	Latent other gain	0	Btuh
	Latent total gain	7905	Btuh
	TOTAL GAIN	54208	Btuh

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R)) (ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



For Florida residences only

Residential Window Diversity

MidSummer

Papka, Adam Residence

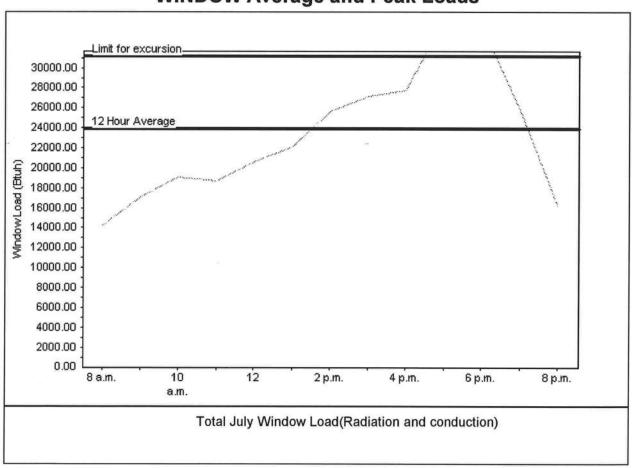
Project Title: 611075Adam'sFramingConstruction

Class 3 Rating Registration No. 0 Climate: North

1/14/2008

Weather data for: Gainesville - Def	aults		
Summer design temperature	92 F	Average window load for July	23941 Btu
Summer setpoint	75 F	Peak window load for July	35192 Btu
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	31123 Btu
Latitude	29 North	Window excursion (July)	4070 Btuh

WINDOW Average and Peak Loads



Warning: This application has glass areas that produce relatively large heat gains for part of the day. Variable air volume devices may be required to overcome spikes in solar gain for one or more rooms. A zoned system may be required or some rooms may require zone control.

EnergyGauge® System Sizing for Florida residences only/

PREPARED BY:

DATE:

EnergyGauge® FLR2PB v4.1



Cantilever Reinforcement

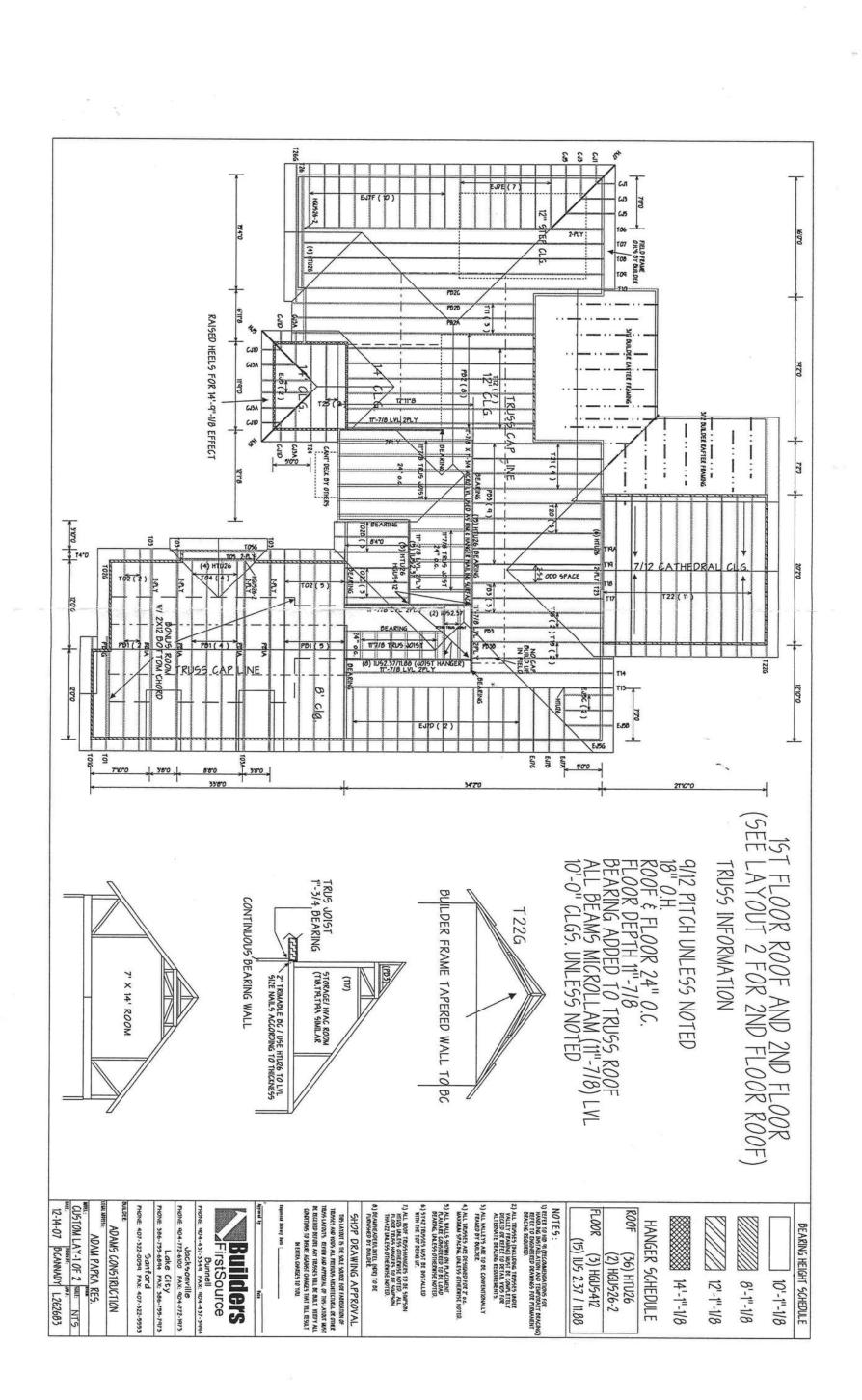
				Section	on A: Ca	antileve	ers less	than 5"	(Brick	Ledge)				Seci	tion B: (Cantilev	ers 5" t	to 24"		
D	THE	Roof				Ro	of Total L	oad					7	100	Roc	of Total L	.oad			
Depth	TJI®	Truss Span		35 PSF			45 PSF			55 PSF	41.		35 PSF			45 PSF		1 3	55 PSF	
		Opun				On-Cen	ter Joist	Spacing							On-Cen	ter Joist	Spacing	,		
F-1 119		4 3 5 5	16"	19.2"	24"	16"	19.2"	24"	16"	19.2"	24"	16"	19.2"	24"	16"	19.2"	24"	16"	19.2"	24"
		20'			E5		E5	E5		E5	E5						X			X
	MALE BUT	22'			E5		E5	E5	E5	E5	E5			100	1		X		X	X
91/2"	100	24'		E5	E5	E5	E5	E5	E5	E5	E5						X		X	X
111/8"	110	26'		E5	E5	E5	E5	E5	E5	E5	E6			X	TO UT	E2	X	E2	X	X
14"		28'		E5	X	E5	E5	X	E5	E5	Χ		E2	X	E2	X	X	X	X	Χ
	Lille	30'	E5	E5	X	E5	E5	X	E5	E5	X		E3	X	E3	X	X	X	X	X
		32'	E5	X	X	E5	X	X	E5	X	Χ	E2	X	X	Х	X	X	X	X	X
	AL THE	20'			E5		1/2 15	E5	HILE	E5	E5	ALC: N	1	-			100	T. H.	Service	X
12000		22'			E5		E5	E5		E5	E5						E2			Χ
91/2"		24'			E5	15 7	E5	E5	E5	E5	E5		Carlo de		1		E2	- LEG		X
14"	210	26'		E5	E5		E5	E5	E5	E5	E5						X		E2	χ
16"		28'	Die la	E5	E5	E5	E5	E5	E5	E5	E6	17-15-		E2		E2	X	E2	X	X
		30'		E5	E5	E5	E5	E5	E5	E5	E6			E3	E2	E3	Х	E3	X	X
		32'	E5	E5	X	E5	E5	X	E5	E5	Χ	Elemi	E2	X	E3 -	X	X	X	X	X
1		24'			E5		E5	E5	E5	E5	E5						E2			χ
91/2"		26'	(c)-1	E5	E5		E5	E5	E5	E5	E5						E2	777	E2	X
111/8"	230	28'		E5	E5	E5	E5	E5	E5	E5	E5					E2	E3	E2	E3	X
14"	230	30'	-1-	E5	E5	E5	E5	E5	E5	E5	E5	- PALL	at and	E2		E2	X	E2	X	X
16"		32'	E5	E5	Х	E5	E5	Х	E5	E5	X		E2	E3	E2	E3	X	E3	X	X
		34'	E5	E5	X	E5	E5	χ	E5	E5	X	14 - 13 -	E3	X	E3	X	Х	X	X	X
	-111	28'			E5		E5	E5	E5	E5	E5									E2
9,000		30'		E5	E5	13.10	E5	E5	E5	E5	E5	To de				7. 70	E1W	- E		E2
117/8"		32'		E5	E5	E5	E5	E5	E5	E5	E5						E2			E2
14"	360	34'		E5	E5	E5	E5	E5	E5	E5	E6						E2		E1W	E3
16"		36'		E5	E5	E5	E5	E5	E5	E5	E6			E1W			E2		E2	E3
		38'	E5	E5	E5	E5	E5	E5	E5	E5	E6	10 17/1		E1W			E2		E2	E3
11-31	10.00	40'	E5	E5	E5	E5	E5	E5	E5	E5	E6			E1W		E1W	E2		E2	E3
E-1		30'	4		E5	1	E5	E5		E5	E5		-Stire	7 -		J7		724-	Con	
	THE PARTY NAMED IN	32'			E5		E5	E5	E5	E5	E5									
111/8"	560	34'		3 4 5	E5	MEN	E5	E5	E5	E5	E5	ELIST		5		O.E.	M.		Selfer.	E2
14" 16"	300	36'		E5	E5		E5	E5	E5	E5	E6									E2
	The Real	38'	i i	E5	E5	E5	E5	E5	E5	E5	E6	No.		- 0					96 7	E2
-		40'		E5	E5	E5	E5	E5	E5	E5	E6						E1W			E2

How to Use This Table

- 1. Identify TJI® joist and depth.
- 2. Locate the Roof Truss Span (horizontal) that meets or exceeds your condition.
- Identify the cantilever condition (less than 5" or 5" to 24") and locate the Roof Total Load and On-Center Joist Spacing for your application.
- Scan down to find the appropriate cantilever detail and refer to drawing on page 12:
 - Blank cells indicate that no reinforcement is required.
 - E4 may be used in place of E2 or E3 except when using TJI® 560 joists.
 - X indicates that cantilever will not work. Use TJ-Beam® or TJ-Xpert® software, or reduce spacing of joists and recheck table.

General Notes

- Table is based on:
 - 15 psf roof dead load on a horizontal projection.
 - 80 plf exterior wall load with 3'-0" maximum width window or door openings.
 For larger openings, or multiple 3'-0" width openings spaced less than 6'-0" on-center, additional joists beneath the opening's trimmers may be required.
 - More restrictive of simple or continuous span.
 - Roof truss with 24" soffits.
- ¾" reinforcement refers to ¾" Exposure 1 plywood or other ¾" Exposure 1, 48/24-rated sheathing that is cut to match the full depth of the TJI® joist. Install with face grain horizontal. Reinforcing member must bear fully on the wall plate.
- Designed for 2x4 and 2x6 plate widths.
- For conditions beyond the scope of this table, including cantilevers longer than 24", use our TJ-Beam® or TJ-Xpert® software.



Cantilever Reinforcement

				Section	on A: C	antileve	ers less	than 5'	(Brick	Ledge)				Seci	ion B: (Cantile	ers 5" t	to 24"		13
Donth	TJI®	Roof				Ro	of Total I	Load	100	100			100	THE	Ro	of Total I	oad		T.F.	
Depth	1110	Truss Span		35 PSF		2000	45 PSF		No. of Lot	55 PSF	TANTA	THE PARTY	35 PSF			45 PSF		1	55 PSF	
	- 56	opun	122			On-Cen	ter Joist	Spacing	Į.					18-51	On-Cen	ter Joist	Spacing			
	1-5-0		16"	19.2"	24"	16"	19.2"	24"	16"	19.2"	24"	16"	19.2"	24"	16"	19.2"	24"	16"	19.2"	24"
		20'			E5		E5	E5		E5	E5						X			X
		22'			E5		E5	E5	E5	E5	E5		Van	- 3 E vo	mET.	100 100	X		X	X
91/2"		24'		E5	E5	E5	E5	E5	E5	E5	E5						X		X	X
117/8"	110	26'		E5	E5	E5	E5	E5	E5	E5	E6		E 4 14	X		E2	X	E2	X	X
14"		28'		E5	X	E5	E5	X	E5	E5	X		E2	X	E2	X	X	X	X	X
	Marian.	30'	E5	E5	X	E5	E5	X	E5	E5	Χ		E3	X	E3	X	X	X	X	X
		32'	E5	X	X	E5	X	Х	E5	Х	Χ	E2	X	X	X	Χ	X	X	Х	X
		20'		1 - 3	E5	قيير-	خنينتا	E5	1	E5	E5						P		Maria Control	X
		22'			E5		E5	E5		E5	E5						E2			X
91/2"		24'	h-		E5	WHA	E5	E5	E5	E5	E5						E2		18	X
111/8"	210	26'		E5	E5		E5	E5	E5	E5	E5						Х		E2	X
16"		28'	E. C.	E5	E5	E5	E5	E5	E5	E5	E6	- 17		E2		E2	X	E2	X	X
1,015		30'		E5	E5	E5	E5	E5	E5	E5	E6			E3	E2	E3	X	E3	X	X
N.		32'	E5	E5	X	E5	E5	X	E5	E5	X	-	E2	Χ	E3 -	X	X	X	X	X
		24'			E5		E5	E5	E5	E5	E5						E2			X
91/2"		26'	0. V	E5	E5	Call C	E5	E5	E5	E5	E5		1-1			- 1	E2		E2	X
117/8"	000	28'		E5	E5	E5	E5	E5	E5	E5	E5					E2	E3	E2	E3	X
14"	230	30'		E5	E5	E5	E5	E5	E5	E5	E5		EVID	E2		E2	Χ	E2	X	X
16"		32'	E5	E5	Х	E5	E5	X	E5	E5	X		E2	E3	E2	E3	Х	E3	X	X
		34'	E5	E5	X	E5	E5	X	E5	E5	Χ		E3	X	E3	X	Х	X	X	X
Carlotte Co.	SA PL	28'			E5		E5	E5	E5	E5	E5									E2
		30'	CIE C	E5	E5	700	E5	E5	E5	E5	E5	-7-1-	Land	7		200	E1W	1120		E2
111/8"		32'		E5	E5	E5	E5	E5	E5	E5	E5						E2			E2
14"	360	34'	150	E5	E5	E5	E5	E5	E5	E5	E6		10 11	Log To		Name of the last	E2		E1W	E3
16"	- 10 - 5	36'		E5	E5	E5	E5	E5	E5	E5	E6			E1W			E2		E2	E3
		38'	E5	E5	E5	E5	E5	E5	E5	E5	E6	11		E1W	-		E2		E2	E3
PILE	11111	40'	E5	E5	E5	E5	E5	E5	E5	E5	E6			E1W		E1W	E2		E2	E3
- divine	-	30'	1071	T WELL	E5	The last	E5	E5		E5	E5		-							
SHOWIN	11/1-1	32'			E5		E5	E5	E5	E5	E5									
111/8"		34'	1/3 19	XII	E5		E5	E5	E5	E5	E5		- NER			- 11		1100	TO THE REAL PROPERTY.	E2
14" 16"	560	36'		E5	E5		E5	E5	E5	E5	E6									E2
10	The order	38'	WHI.	E5	E5	E5	E5	E5	E5	E5	E6	Lun	370.	11. 11.19	E. L.	100		141		E2
		40'		E5	E5	E5	E5	E5	E5	E5	E6						E1W			E2

How to Use This Table

- 1. Identify TJI® joist and depth.
- 2. Locate the Roof Truss Span (horizontal) that meets or exceeds your condition.
- Identify the cantilever condition (less than 5" or 5" to 24") and locate the Roof Total Load and On-Center Joist Spacing for your application.
- Scan down to find the appropriate cantilever detail and refer to drawing on page 12:
 - Blank cells indicate that no reinforcement is required.
 - E4 may be used in place of E2 or E3 except when using TJI® 560 joists.
 - X indicates that cantilever will not work. Use TJ-Beam® or TJ-Xpert® software, or reduce spacing of joists and recheck table.

General Notes

- Table is based on:
 - 15 psf roof dead load on a horizontal projection.
 - 80 plf exterior wall load with 3'-0" maximum width window or door openings.
 For larger openings, or multiple 3'-0" width openings spaced less than 6'-0" on-center, additional joists beneath the opening's trimmers may be required.
 - More restrictive of simple or continuous span.
 - Roof truss with 24" soffits.
- ¾" reinforcement refers to ¾" Exposure 1 plywood or other ¾" Exposure 1, 48/24-rated sheathing that is cut to match the full depth of the TJI® joist. Install with face grain horizontal. Reinforcing member must bear fully on the wall plate.
- Designed for 2x4 and 2x6 plate widths.
- For conditions beyond the scope of this table, including cantilevers longer than 24", use our TJ-Beam® or TJ-Xpert® software.



Project Information for:

L262683

Builder:

Adams Framing & Construction, LLC

Lot:

Subdivision: County:

Aimsley Glen Columbia

Truss Count:

67

Design Program: MiTek 20/20 6.3 Building Code:

FBC2004/TPI2002

Truss Design Load Information: Gravity: Wind:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Drwg. #

J1921303

J1921304

J1921305

J1921306

J1921307

J1921308

J1921309

J1921310

J1921311

J1921312

J1921313

Truss ID

T23

T24

T25

T26

T27

T28

T29

T30

T31

T27G

T26G

Date

1/2/08

1/2/08

1/2/08

1/2/08

1/2/08

1/2/08

1/2/08

1/2/08

1/2/08

1/2/08

1/2/08

Floor (psf): 55.0

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions. Contractor of Record, responsible for structural engineering:

Adam R. Papka Florida License No. CBC1253409

Address: P.O. Box 1921 Lake City, Florida 32056

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

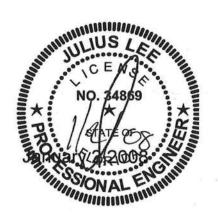
Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

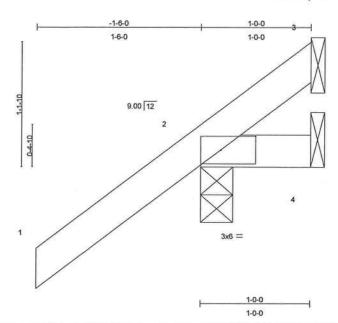
No.	Drwg. #	Truss ID	Date	No.	Drwg. #	Truss ID	Date	No.
1	J1921247	CJ1	1/2/08	29	J1921275	T02	1/2/08	57
2	J1921248	CJ1D	1/2/08	30	J1921276	T02B	1/2/08	58
3	J1921249	CJ3	1/2/08	31	J1921277	T02C	1/2/08	59
4	J1921250	CJ3A	1/2/08	32	J1921278	T02G	1/2/08	60
5	J1921251	CJ5	1/2/08	33	J1921279	T03	1/2/08	61
6	J1921252	EJ5	1/2/08	34	J1921280	T03A	1/2/08	62
7	J1921253	EJ5B	1/2/08	35	J1921281	T04	1/2/08	63
8	J1921254	EJ5C	1/2/08	36	J1921282	T05	1/2/08	64
9	J1921255	EJ5G	1/2/08	37	J1921283	T05G	1/2/08	65
10	J1921256	EJ7A	1/2/08	38	J1921284	T06	1/2/08	66
11	J1921257	EJ7B	1/2/08	39	J1921285	T07	1/2/08	67
12	J1921258	EJ7C	1/2/08	40	J1921286	T08	1/2/08	
13	J1921259	EJ7D	1/2/08	41	J1921287	T09	1/2/08	
14	J1921260	EJ7E	1/2/08	42	J1921288	T10	1/2/08	
15	J1921261	EJ7F	1/2/08	43	J1921289	T11	1/2/08	
16	J1921262	HJ5	1/2/08	44	J1921290	T12	1/2/08	
17	J1921263	HJ7	1/2/08	45	J1921291	T13	1/2/08	
18	J1921264	PB1	1/2/08	46	J1921292	T14	1/2/08	
19	J1921265	PB1A	1/2/08	47	J1921293	T15	1/2/08	
20	J1921266	PB1G	1/2/08	48	J1921294	T16	1/2/08	
21	J1921267	PB2	1/2/08	49	J1921295	T17	1/2/08	
22	J1921268	PB2A	1/2/08	50	J1921296	T18	1/2/08	
23	J1921269	PB2B	1/2/08	51	J1921297	T19	1/2/08	
24	J1921270	PB2C	1/2/08	52	J1921298	T19A	1/2/08	
25	J1921271	PB3	1/2/08	53	J1921299	T20	1/2/08	
26	J1921272	PB3B	1/2/08	54	J1921300	T21	1/2/08	
27	J1921273	T01	1/2/08	55	J1921301	T22	1/2/08	
28	J1921274	T01G	1/2/08	56	J1921302	T22G	1/2/08	



Job Truss Truss Type Qty Ply PAPKA RESIDENCE J1921247 L262683 CJ1 **JACK** 2 1 Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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Scale = 1:9.9

Plate Of	fsets (X,Y	(): [2:0-3-13,0-1-8]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI	į.	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240	Wo All Street	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)	31000000 3 0000 x					Weight: 6 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=179/0-3-8, 4=5/Mechanical, 3=-40/Mechanical

Max Horz 2=106(load case 6)

Max Uplift 2=-193(load case 6), 3=-40(load case 1)

Max Grav 2=179(load case 1), 4=14(load case 2), 3=74(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-59/55

2-4=0/0 **BOT CHORD**

JOINT STRESS INDEX

2 = 0.14

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 2,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	11001017
L262683	CJ1	JACK	2	1		J1921247
					Job Reference (optional)	

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NOTES

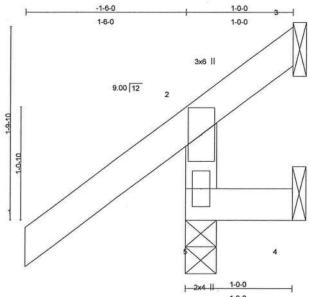
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 2 and 40 lb uplift at joint 3.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	914-1020 (FILE 10 FILE)
L262683	CJ1D	JACK	4	1		J1921248
			1 193		Job Reference (optional)	

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	9.00 12 2	1-0-0 3x6 II		Scale = 1:10
1-0-10		2x4 1-0-0	4	
		1-0-0		

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	0.00	5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	0.00	5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mati	rix)						Weight: 7 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or

1-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=203/0-3-8, 4=-15/Mechanical, 3=-45/Mechanical

Max Horz 5=109(load case 6)

Max Uplift 5=-146(load case 6), 4=-31(load case 6), 3=-45(load case 1)

Max Grav 5=203(load case 1), 4=7(load case 2), 3=38(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-5=-179/188, 1-2=0/53, 2-3=-54/27

BOT CHORD 4-5=0/0

JOINT STRESS INDEX

2 = 0.29 and 5 = 0.26

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 Ib uplift at joint 5, 31 lb uplift at joint 4 and 45 lb uplift at joint 3. Continued on page 2

January 2,2008

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	CJ1D	JACK	4	1		J1921248
	100.00		12	1 12	Job Reference (optional)	

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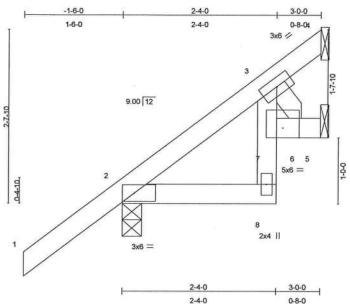
LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Flonda PE No. 34866 1 109 Coastal Bay Blvd Boynton Beach, FL 23435



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	СЈЗ	SPECIAL	2	1		J1921249
					Job Reference (optional)	

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		पुरम्म् व	//	2	3x6 =		8 2x4	6 5x6 =	5 001		
		<i>ν</i>		ŀ		2-4-0 2-4-0		3-0-0 0-8-0			
Plate Offse	ets (X,Y): [2:0-3-13,0-1-8]									
LOADING TCLL TCDL BCLL	(psf) 20.0 7.0 10.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr	2-0-0 1.25 1.25 YES	CSI TC BC WB	0.19 0.06 0.01	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.00 -0.00 0.00	(loc) 8 2-8 5	I/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20

(Matrix)

L	U	M	В	E	R	
-	-	-	-		. ~	

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

5.0

3-8 2 X 4 SYP No.3

Code FBC2004/TPI2002

WEBS

BCDL

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

Weight: 16 lb

3-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=34/Mechanical, 2=204/0-3-8, 5=29/Mechanical

Max Horz 2=173(load case 6)

Max Uplift 4=-43(load case 6), 2=-135(load case 6), 5=-25(load case 7) Max Grav 4=34(load case 1), 2=204(load case 1), 5=41(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/48, 2-3=-76/0, 3-4=-33/18

BOT CHORD 2-8=-12/17, 7-8=0/31, 3-7=-3/54, 6-7=-29/42, 5-6=0/0

3-6=-70/48 WEBS

JOINT STRESS INDEX

2 = 0.14, 3 = 0.05, 6 = 0.00, 7 = 0.02 and 8 = 0.02

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Colinade page 2

January 2,2008

Scale = 1:16.5

GRIP 244/190

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	AUTODORAN STORANSON
L262683	CJ3	SPECIAL	2	1		J1921249
		0, 20, 12	-		Job Reference (optional)	

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NOTES

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

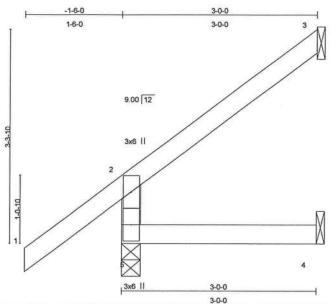
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 43 lb uplift at joint 4, 135 lb uplift at joint 2 and 25 lb uplift at joint 5.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	CJ3A	JACK	4	1		J1921250
					Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.01	4-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.00	4-5	>999	240	100000000000000000000000000000000000000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.02	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	A					Weight: 14 lb	

	_		_	_		
LUMBE	•	_	0	м	Ιħ	

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 **WEBS**

BRACING

TOP CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 5=204/0-3-8, 3=53/Mechanical, 4=10/Mechanical

Max Horz 5=196(load case 6)

Max Uplift 5=-125(load case 6), 3=-80(load case 6), 4=-46(load case 6) Max Grav 5=204(load case 1), 3=53(load case 1), 4=38(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-5=-187/133, 1-2=0/53, 2-3=-70/23

BOT CHORD 4-5=0/0

JOINT STRESS INDEX

2 = 0.45 and 5 = 0.18

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 125 lb uplift at joint 5, 80 lb uplift at joint 3 and 46 lb uplift at joint 4. Continued on page 2

January 2,2008

Scale = 1:16.8

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	CJ3A	JACK	4	1		J1921250
					Job Reference (optional)	

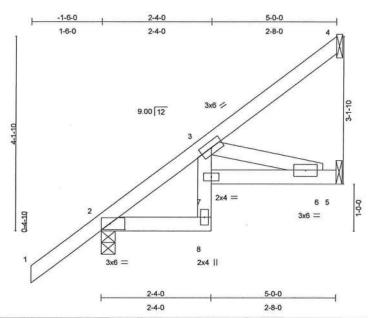
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LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	CJ5	SPECIAL	2	1		J1921251
TOWNS TOWNS	1.50-3000	Property reports and property of	1000	- "	Job Reference (optional)	

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Scale = 1:23.1

Plate Of	fsets (X,Y	'): [2:0-3-13,0-1-8]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.01	8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.01	8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 26 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

3-8 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins.

BOT CHORD Rigin

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=73/Mechanical, 2=257/0-3-8, 5=65/Mechanical

Max Horz 2=242(load case 6)

Max Uplift 4=-85(load case 6), 2=-124(load case 6), 5=-35(load case 6) Max Grav 4=73(load case 1), 2=257(load case 1), 5=78(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/48, 2-3=-165/0, 3-4=-70/35

BOT CHORD

2-8=-78/82, 7-8=0/31, 3-7=0/75, 6-7=-190/200, 5-6=0/0

WEBS

3-6=-208/197

JOINT STRESS INDEX

2 = 0.16, 3 = 0.11, 6 = 0.06, 7 = 0.11 and 8 = 0.07

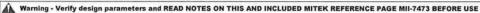
NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Julius Lee Truse Design Engineer Flanda PE No. 34866 1 109 Coasial Bay Blvd Boynton Besch, FL 33436

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Colive leads a page 2

January 2,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	CJ5	SPECIAL	2	1		J1921251
	000	0. Lov.L			Job Reference (optional)	

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NOTES

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 85 lb uplift at joint 4, 124 lb uplift at joint 2 and 35 lb uplift at joint 5.

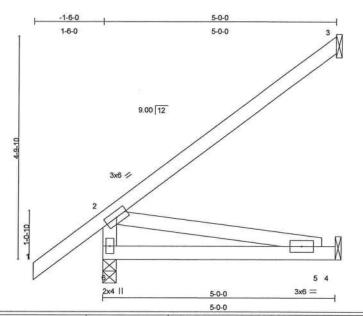
LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 24866 1930 Chestel Bay Blod Govinton Besch, FL 90435



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ5	JACK	2	1		J1921252
Alemente Tell Control	1000000			1.5	Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	0.09	5-6	>637	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.05	5-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.07	Horz(TL)	-0.01	3	n/a	n/a		
BCDL	BCDL 5.0 Code FBC2004/TPI2002		(Matrix)							Weight: 27 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or

5-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 6=257/0-3-8, 3=114/Mechanical, 4=24/Mechanical

Max Horz 6=265(load case 6)

Max Uplift 6=-148(load case 6), 3=-126(load case 6), 4=-90(load case 6) Max Grav 6=257(load case 1), 3=114(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 2-6=-234/117, 1-2=0/53, 2-3=-108/53

BOT CHORD

5-6=-294/1, 4-5=0/0

WEBS

2-5=-1/299

JOINT STRESS INDEX

2 = 0.11, 5 = 0.08 and 6 = 0.08

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

January 2,2008

Scale = 1:23.4



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ5	JACK	2	1		J1921252
Technical Services					Job Reference (optional)	

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NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 148 lb uplift at joint 6, 126 lb uplift at joint 3 and 90 lb uplift at joint 4.

LOAD CASE(S) Standard

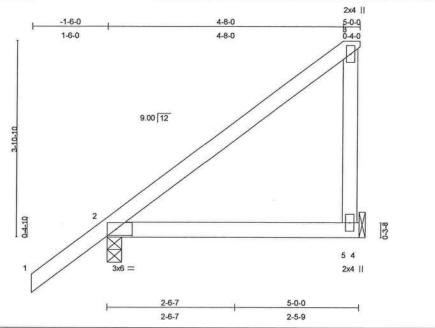
Julius Lee Truse Elesian Engineer Plonda PE No. 34889 1 100 Coasial Bay Blvd Boynton Beach, FL 33435



Job PAPKA RESIDENCE Truss Truss Type Qty Ply J1921253 L262683 EJ5B MONO HIP Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.20	Vert(LL)	-0.02	2-5	>999	360	MT20	244/190
TCDL	7.0	1	Lumber Increase	1.25	BC	0.15	Vert(TL)	-0.04	2-5	>999	240	330 10000	
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0		Code FBC2004/TF	212002	(Mati	rix)	Content of the Conten					Weight: 25 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

.

WEBS

2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 5-0-0

oc purlins, except end verticals. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=136/Mechanical, 2=254/0-3-8

Max Horz 2=238(load case 6)

Max Uplift 5=-115(load case 6), 2=-124(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/48, 2-3=-114/53, 3-5=-110/145

BOT CHORD 2-5=0/0, 4-5=0/0

JOINT STRESS INDEX

2 = 0.17, 3 = 0.07 and 5 = 0.08

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 115 lb upliftation joint 5 and 124 lb uplift at joint 2.

LOAD CASE(S) Standard

January 2,2008

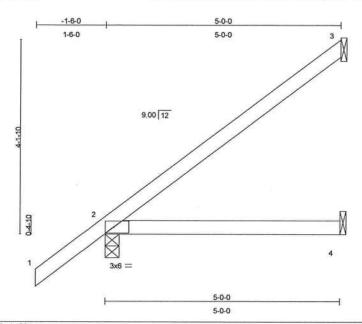
Scale = 1:21.5

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TP1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ5C	JACK	2	1		J1921254
		5.55.5			Job Reference (optional)	

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sets (X,Y	(): [2:0-3-13,0-1-8]										
(psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	240		
10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, , ,					Weight: 20 lb	
	(psf) 20.0 7.0 10.0	S (psf) SPACING 20.0 Plates Increase 7.0 Lumber Increase 10.0 * Rep Stress Incr	6 (psf) SPACING 2-0-0 20.0 Plates Increase 1.25 7.0 Lumber Increase 1.25 10.0 * Rep Stress Incr YES	6 (psf)	(psf) SPACING 2-0-0 CSI 20.0 Plates Increase 1.25 TC 0.22 7.0 Lumber Increase 1.25 BC 0.16 10.0 * Rep Stress Incr YES WB 0.00	G (psf) SPACING 2-0-0 CSI DEFL 20.0 Plates Increase 1.25 TC 0.22 Vert(LL) 7.0 Lumber Increase 1.25 BC 0.16 Vert(TL) 10.0 * Rep Stress Incr YES WB 0.00 Horz(TL)	(psf) SPACING 2-0-0 CSI DEFL in 20.0 Plates Increase 1.25 TC 0.22 Vert(LL) -0.03 T.0 Lumber Increase 1.25 BC 0.16 Vert(TL) -0.05 T.0 * Rep Stress Incr YES WB 0.00 Horz(TL) -0.00	(s) (psf) SPACING 2-0-0 CSI DEFL in (loc) 20.0 Plates Increase 1.25 TC 0.22 Vert(LL) -0.03 2-4 T.0 Lumber Increase 1.25 BC 0.16 Vert(TL) -0.05 2-4 T.0.0 * Rep Stress Incr YES WB 0.00 Horz(TL) -0.00 3	(psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl 20.0 Plates Increase 1.25 TC 0.22 Vert(LL) -0.03 2-4 >999 T.0 Lumber Increase 1.25 BC 0.16 Vert(TL) -0.05 2-4 >999 T.0.0 * Rep Stress Incr YES WB 0.00 Horz(TL) -0.00 3 n/a	(psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d 20.0 Plates Increase 1.25 TC 0.22 Vert(LL) -0.03 2-4 >999 360 T.0 Lumber Increase 1.25 BC 0.16 Vert(TL) -0.05 2-4 >999 240 10.0 * Rep Stress Incr YES WB 0.00 Horz(TL) -0.00 3 n/a n/a	SPACING 2-0-0 CSI DEFL in (loc) l/defl L/d PLATES

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=257/0-3-8, 4=24/Mechanical

Max Horz 2=242(load case 6)

Max Uplift 3=-135(load case 6), 2=-124(load case 6)

Max Grav 3=114(load case 1), 2=257(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/48, 2-3=-117/54

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.17

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Lee Truse Design Engineer Florida PE No. 34868 1 109 Caastal Bay Blyd Boynton Besch, FL 33436

January 2,2008

Scale = 1:23.1

Marming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ5C	JACK	2	1		J1921254
	1 1000.50.50	71.572.5		- 1	Job Reference (optional)	

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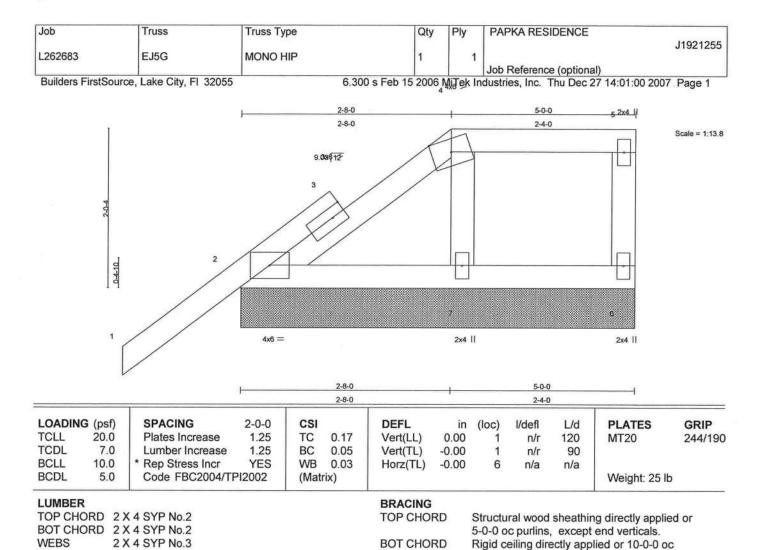
NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 135 lb uplift at joint 3 and 124 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Law Truse Design Endineer Flonda FE No. 34869 1109 Ceastal Bay Blyd Boynton Gesch, Ft. 33435





bracing.

REACTIONS (lb/size) 2=193/5-0-0, 6=65/5-0-0, 7=134/5-0-0

Max Horz 2=148(load case 6)

Max Uplift 2=-143(load case 6), 6=-46(load case 4), 7=-49(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-55/7, 3-4=-21/16, 4-5=-1/0, 5-6=-59/71

BOT CHORD 2-7=-9/9, 6-7=0/0

4-7=-103/107 **WEBS**

JOINT STRESS INDEX

2 = 0.78, 3 = 0.00, 3 = 0.15, 4 = 0.04, 5 = 0.04, 6 = 0.04 and 7 = 0.06

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other

Gable requires continuous bottom chord bearing.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 2,2008

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ5G	MONO HIP	1	1		J1921255
	2000	morto riii			Job Reference (optional)	

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NOTES

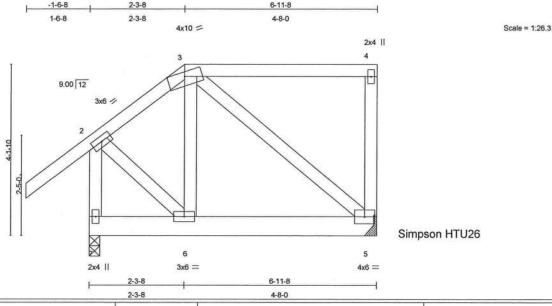
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 143 lb uplift at joint 2, 46 lb uplift at joint 6 and 49 lb uplift at joint 7.
- 7) Truss designed for wind loads in plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ7A	MONO HIP	1	1		J1921256
					Job Reference (optional)	

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.00	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	5-6	>999	240	a content entreve	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.10	Horz(TL)	0.00	5	n/a	n/a		
BCDL 5.0 Code FBC2004/TPI2002		PI2002	(Mat	rix)						Weight: 55 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.1D 2 X 4 SYP No.3 WFBS

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 7=460/0-3-0, 5=346/Mechanical

Max Horz 7=166(load case 5)

Max Uplift 7=-129(load case 5), 5=-134(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

2-7=-430/135, 1-2=0/55, 2-3=-230/86, 3-4=-30/14, 4-5=-121/66

TOP CHORD **BOT CHORD**

6-7=-106/11, 5-6=-84/165

WEBS

2-6=-75/255, 3-6=-73/96, 3-5=-175/100

JOINT STRESS INDEX

2 = 0.63, 3 = 0.28, 4 = 0.64, 5 = 0.16, 6 = 0.15 and 7 = 0.31

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 7.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 Ib uplift at joint 7 and 134 lb uplift at joint 5. Continued on page 2

January 2,2008

航 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ7A	MONO HIP	1	1		J1921256
2					Job Reference (optional)	

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NOTES

7) Girder carries tie-in span(s): 5-0-0 from 0-0-0 to 6-11-8

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

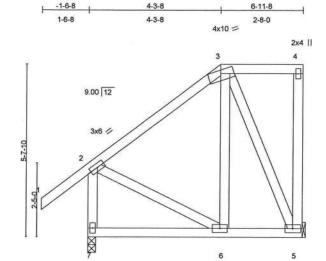
1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-2=-54, 2-3=-54, 3-4=-54, 5-7=-53(F=-43)



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ7B	MONO HIP	1	1		J1921257
					Job Reference (optional)	

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2x4 ||

					4-3-8		2-8-0					
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.01	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	6-7	>999	240	A TOTAL CONTRACT	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.10	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)	1 100 100 100 100 100 100 100 100 100 1					Weight: 58 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

3x6 =

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

3x6 =

REACTIONS (lb/size) 7=316/0-3-0, 5=202/Mechanical

Max Horz 7=216(load case 6)

Max Uplift 7=-53(load case 6), 5=-106(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-7=-293/137, 1-2=0/55, 2-3=-160/51, 3-4=-3/2, 4-5=-46/49

BOT CHORD 6-7=-276/19, 5-6=-106/71

WEBS 2-6=-15/193, 3-6=-68/99, 3-5=-161/243

JOINT STRESS INDEX

2 = 0.54, 3 = 0.38, 4 = 0.09, 5 = 0.16, 6 = 0.08 and 7 = 0.33

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 7. Continued on page 2

Trues Design Engineer Flonds PE No. 34888 1 100 Cassiel Bay Blyd Boynton Beach, FL 33435

January 2,2008

Scale = 1:35.5

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ7B	MONO HIP	1	1		J1921257
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:01 2007 Page 2

NOTES

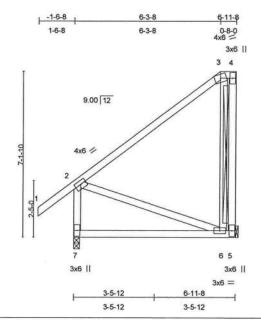
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 7 and 106 lb uplift at joint 5.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ7C	MONO HIP	1	1		J1921258
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:01 2007 Page 1



Scale = 1:46.7

Plate Of	fsets (X,)	(): [2:0-3-0,0-1-12]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.04	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	-0.07	6-7	>999	240	A STATE OF THE STA	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.20	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 58 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3 *Except*

4-5 2 X 4 SYP No.2

BRACING

BOT CHORD

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 9-7-10 oc

bracing.

WEBS T-Brace: 2 X 4 SYP No.3 - 4-5

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 7=316/0-3-0, 5=202/Mechanical

Max Horz 7=264(load case 6)

Max Uplift 7=-15(load case 6), 5=-168(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-7=-271/51, 1-2=0/55, 2-3=-139/61, 3-4=-25/29, 4-5=-120/73

BOT CHORD 6-7=-408/70, 5-6=-24/22

WEBS 2-6=-48/399, 3-6=-238/328

JOINT STRESS INDEX

2 = 0.71, 3 = 0.65, 4 = 0.25, 5 = 0.26, 6 = 0.17 and 7 = 0.27

January 2,2008

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ7C	MONO HIP	1	1		J1921258
				18	Job Reference (optional)	

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NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 7.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 7 and 168 lb uplift at joint 5.

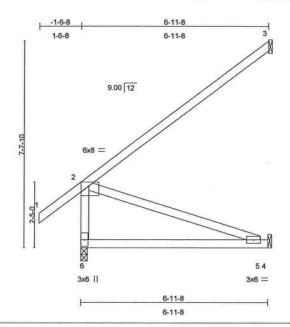
LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 34869 1109 Coessel Bay Blvd Bovoton Descel Bay Blvd



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ7D	JACK	12	1		J1921259
LEGEGGG	2075	o non	1.2		Job Reference (optional)	

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Scale = 1:40.2

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.06	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.11	5-6	>747	240	36-161-161-03-07	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.13	Horz(TL)	-0.01	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 39 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

000 1 0000

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 9-1-12 oc

bracing.

REACTIONS (lb/size) 6=319/0-3-0, 3=144/Mechanical, 4=61/Mechanical

Max Horz 6=278(load case 6)

1000051

Max Uplift 6=-3(load case 6), 3=-111(load case 6), 4=-77(load case 6) Max Grav 6=319(load case 1), 3=144(load case 1), 4=110(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

2-6=-274/41, 1-2=0/55, 2-3=-143/64

BOT CHORD

5-6=-476/114, 4-5=0/0

WEBS

2-5=-120/503

JOINT STRESS INDEX

2 = 0.50, 5 = 0.13 and 6 = 0.37

NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

E) All hearings age assumed to be SYP No.2 crushing capacity of 565.00 psi

Truse Design Engineer Florida PE No. 24969 1109 Chastel Bay Blvd. Boynton Besch. FL 93431

January 2,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ7D	JACK	12	1		J1921259
			777		Job Reference (optional)	

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NOTES

4) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3 lb uplift at joint 6, 111 lb uplift at joint 3 and 77 lb uplift at joint 4.

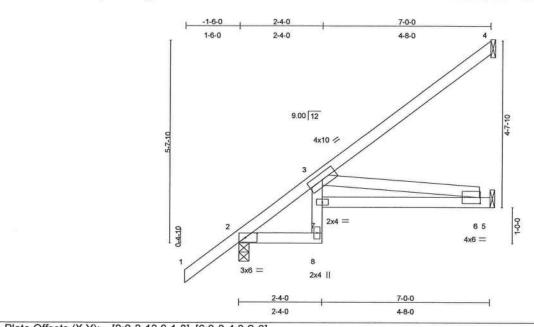
LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PB No. 24869 1 109 Coestel Bay Blvd.



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ7E	SPECIAL	7	1		J1921260
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:02 2007 Page 1



Scale = 1:30.2

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	0.04	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.05	6-7	>999	240	200000000000000000000000000000000000000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.21	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 35 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

3-8 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing, Except: 8-9-8 oc bracing: 6-7.

REACTIONS (lb/size) 4=104/Mechanical, 2=317/0-3-8, 5=102/Mechanical

Max Horz 2=223(load case 6)

Max Uplift 4=-79(load case 6), 2=-77(load case 6), 5=-33(load case 6) Max Grav 4=104(load case 1), 2=317(load case 1), 5=117(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/48, 2-3=-272/0, 3-4=-103/47

BOT CHORD

2-8=-158/164, 7-8=-2/53, 3-7=0/137, 6-7=-507/524, 5-6=0/0

WEBS 3-6=-531/513

JOINT STRESS INDEX

2 = 0.45, 3 = 0.79, 6 = 0.17, 7 = 0.66 and 8 = 0.51

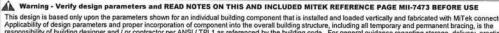
NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

& All hearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

January 2,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ7E	SPECIAL	7	1		J1921260
					Job Reference (optional)	

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NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 4, 77 lb uplift at joint 2 and 33 lb uplift at joint 5.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	EJ7F	JACK	10	1		J1921261
			.0	1	Job Reference (optional)	

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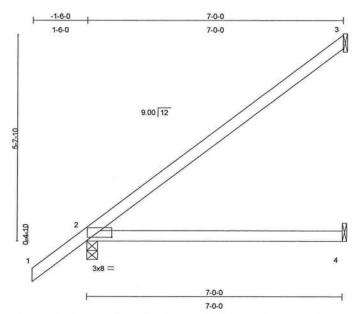


Plate Offsets (X,Y): [2:0-8-3,0-1-2] 2-0-0 LOADING (psf) SPACING CSI DEFL (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 1.25 TC 0.42 Plates Increase Vert(LL) 2-4 360 0.11 >773 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.29 Vert(TL) -0.17>469 240 2-4 BCLL 10.0 Rep Stress Incr WB 0.00 YES Horz(TL) -0.00 3 n/a n/a **BCDL** Code FBC2004/TPI2002 5.0 (Matrix) Weight: 27 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD Rigid ce

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=157/Mechanical, 2=317/0-3-8, 4=50/Mechanical

Max Horz 2=223(load case 6)

Max Uplift 3=-118(load case 6), 2=-77(load case 6)

Max Grav 3=157(load case 1), 2=317(load case 1), 4=96(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/48, 2-3=-155/74

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.71

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 118 Colhi អាច្រុំ អ្នក ខ្លាំង ខ្

Truse Cesign Engineer Florida PE No. 24868 1100 Coastal Bay Blvd Boynton Besch, FL 3343

January 2,2008

Scale = 1:29.7

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	0.00.00.00.00.00.00.00.00.00.00.00.00.0
L262683	EJ7F	JACK	10	1		J1921261
					Job Reference (optional)	

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LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	14024262
L262683	HJ5	MONO TRUSS	2	1		J1921262
					Job Reference (optional)	

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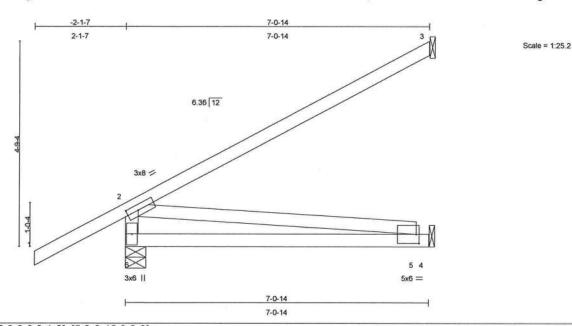


Plate Of	fsets (X,Y	(): [2:0-3-3,0-1-8], [5:	0-0-12,0-2	2-8]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	0.09	5-6	>946	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.11	5-6	>770	240	II VANCAGAMA	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.17	Horz(TL)	-0.01	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 37 lb	

LUMBEK	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 7-0-14 oc purlins, except end verticals. Rigid ceiling directly applied or 9-9-13 oc bracing.

REACTIONS (lb/size) 6=277/0-5-11, 3=185/Mechanical, 4=56/Mechanical

Max Horz 6=243(load case 5)

Max Uplift 6=-209(load case 5), 3=-186(load case 5), 4=-92(load case 5)
Max Grav 6=277(load case 1), 3=185(load case 1), 4=113(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

2-6=-250/121, 1-2=0/57, 2-3=-112/57

BOT CHORD

5-6=-381/207, 4-5=0/0

WEBS

2-5=-208/384

JOINT STRESS INDEX

2 = 0.76, 5 = 0.12 and 6 = 0.41

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Truse Design Engineer Florida PE No. 34868 1 100 Gastal Bay Blvd Boynton Besch. Ft. 93435

January 2,2008

🔬 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	HJ5	MONO TRUSS	2	1		J1921262
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:03 2007 Page 2

NOTES

- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 209 lb uplift at joint 6, 186 lb uplift at joint 3 and 92 lb uplift at joint 4.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54 Trapezoidal Loads (plf)

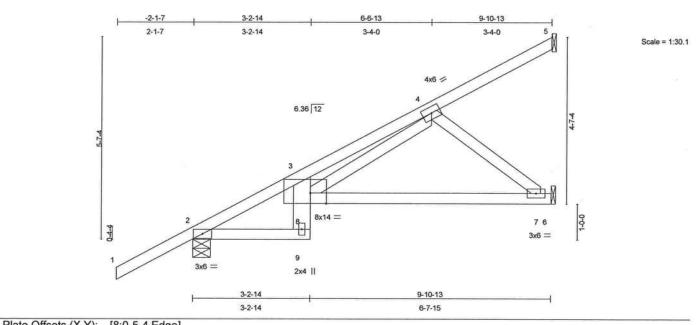
Vert: 2=-2(F=26, B=26)-to-3=-95(F=-21, B=-21), 6=0(F=5, B=5)-to-4=-18(F=-4, B=-4)

Julius Lee Truse Cesign Engineer Florida PE No. 34869 1109 Coastel Bay Blvd Boydon Bastel F. 23444



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	HJ7	SPECIAL	1	1		J1921263
					Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.08	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.18	7-8	>625	240	All March Sussesser A	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.23	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TR	212002	(Mat	rix)						Weight: 50 lb	

L			

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

3-9 2 X 6 SYP No.1D

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Ri

Rigid ceiling directly applied or 10-0-0 oc

bracing, Except: 6-0-0 oc bracing: 8-9.

REACTIONS (lb/size) 5=161/Mechanical, 2=279/0-5-11, 6=314/Mechanical

Max Horz 2=312(load case 5)

Max Uplift 5=-182(load case 5), 2=-73(load case 5), 6=-194(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1

1-2=0/53, 2-3=-263/0, 3-4=-850/107, 4-5=-112/55

BOT CHORD

2-9=-84/211, 8-9=-30/61, 3-8=-245/125, 7-8=-294/337, 6-7=0/0

WEBS 4-7

4-7=-432/377, 4-8=-70/716

JOINT STRESS INDEX

2 = 0.60, 3 = 0.00, 4 = 0.35, 7 = 0.14, 8 = 0.25 and 9 = 0.42

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 182 Colhi HDI at joint 2 and 194 lb uplift at joint 6.

Truse Cesian Engineer Florida ME No. 34866 1109 Ceastal Bay Blvd Boynton Beach, FL 33436

January 2,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	347 4 (7 A C A M) (7 C + 4 M A C A M)
L262683	НЈ7	SPECIAL	1	1		J1921263
					Job Reference (optional)	

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NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=51(F=25, B=25)-to-3=13(F=7, B=7), 3=-41(F=7, B=7)-to-5=-134(F=-40, B=-40), 2=-0(F=5, B=5)-to-9=-7(F=1,

B=1), 8=-7(F=1, B=1)-to-6=-25(F=-7, B=-7)

Julius Les Truss Cesign Engineer Florida PE No. 24869 1109 Coastal Bay Blvd



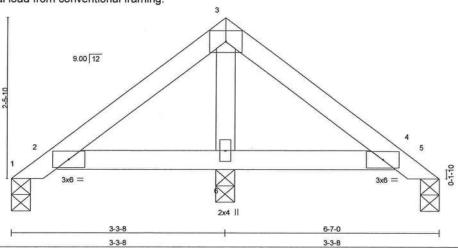
Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	PB1	PIGGYBACK	11	1		J1921264
					Job Reference (optional)	

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4x6 =

Warning: This truss has not been designed to support any additional load from conventional framing.



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.08	Vert(LL)	-0.00	4-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.00	4-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 22 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WFRS 2 X 4 SYP No.3 BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=40/0-3-8, 5=40/0-3-8, 6=325/0-3-8

Max Horz 1=-66(load case 4)

Max Uplift 1=-8(load case 7), 5=-23(load case 4), 6=-74(load case 6) Max Grav 1=59(load case 10), 5=59(load case 11), 6=325(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-58/62, 2-3=-49/131, 3-4=-49/131, 4-5=-30/17

BOT CHORD 2-6=-64/98, 4-6=-64/98

WEBS

3-6=-262/174

JOINT STRESS INDEX

2 = 0.21, 3 = 0.17, 4 = 0.21 and 6 = 0.10

NOTES

1) Unbalanced roof live loads have been considered for this design.

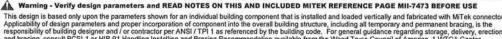
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Scale = 1:16.7



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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	210012
L262683	PB1	PIGGYBACK	11	1		J1921264
					Job Reference (optional)	

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NOTES

- 5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 1, 23 lb uplift at joint 5 and 74 lb uplift at joint 6.
- 7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard



Job Truss Truss Type Qty Ply PAPKA RESIDENCE J1921265 L262683 PB1A **PIGGYBACK** Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 02 10:55:20 2008 Page 1 3-3-8 3-3-8 Scale = 1:16.7 4x6 = Note: A single ply Piggyback must be attached to a single ply of a multi ply supporting truss. Engineer of Record to specify any additional required connections. 9.00 12 3x6 = 3x6 = 6-7-0

LOADIN TCLL TCDL BCLL	IG (psf) 20.0 7.0 10.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr	2-10-0 1.25 1.25 NO	TC BC WB	0.06 0.07 0.15	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.00 -0.00 0.00	(loc) 2 2 5	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCDL	5.0	Code FBC2004/T	PI2002	(Mati	rix)						Weight: 29 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D BOT CHORD 2 X 4 SYP No.2 **WEBS**

2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD JOINTS

2-0-0 oc purlins (6-0-0 max.)

3-3-8

(Switched from sheeted: Spacing > 2-0-0). Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Brace at Jt(s): 3, 4

REACTIONS (lb/size) 1=111/0-3-8, 5=111/0-3-8, 7=787/0-3-8

Max Horz 1=93(load case 5)

Max Uplift 1=-29(load case 7), 5=-40(load case 7), 7=-190(load case 6) Max Grav 1=129(load case 10), 5=130(load case 11), 7=787(load case 1)

3-3-8

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-82/95, 2-3=-40/88, 3-4=-41/87, 4-5=-61/42

BOT CHORD 2-6=-19/77, 4-6=-17/77

WEBS 6-7=-787/424, 3-6=-706/412

JOINT STRESS INDEX

2 = 0.20, 3 = 0.34, 4 = 0.20 and 6 = 0.10

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 1, 5, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

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Continued on page 2

航 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	14004005
L262683	PB1A	PIGGYBACK	4	1		J1921265
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 02 10:55:20 2008 Page 2

NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 1, 40 lb uplift at joint 5 and 190 lb uplift at joint 7.
- 7) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-2=-93, 2-3=-77, 3-4=-76, 4-5=-93, 2-4=-14

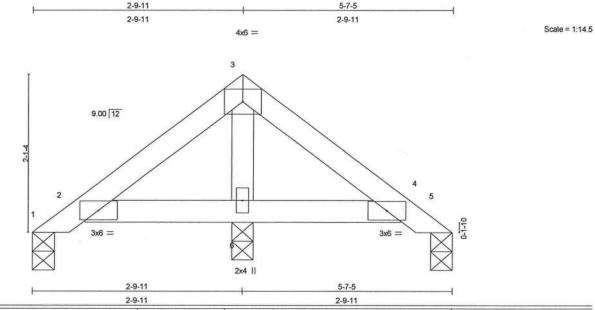
Concentrated Loads (lb)

Vert: 3=-435(F)



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	14004000
L262683	PB1G	PIGGYBACK	1	1		J1921266
					Job Reference (optional)	

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LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.16	Vert(LL)	-0.00	4	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.01	4-6	>999	240	70000000	
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.08	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0		Code FBC2004/TI	PI2002	(Mati	rix)						Weight: 19 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-7-5

oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=92/0-3-8, 5=92/0-3-8, 6=620/0-3-8

Max Horz 1=-69(load case 4)

Max Uplift 1=-44(load case 7), 5=-52(load case 7), 6=-289(load case 6) Max Grav 1=107(load case 10), 5=107(load case 11), 6=620(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-56/68, 2-3=-104/245, 3-4=-104/245, 4-5=-53/28

BOT CHORD 2-6=-110/113, 4-6=-110/113

WEBS 3-6=-516/310

JOINT STRESS INDEX

2 = 0.38, 3 = 0.30, 4 = 0.38 and 6 = 0.19

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

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Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	11001000
L262683	PB1G	PIGGYBACK	1	1		J1921266
					Job Reference (optional)	

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NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 1, 52 lb uplift at joint 5 and 289 lb uplift at joint 6.
- 7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) Truss designed for wind loads in plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

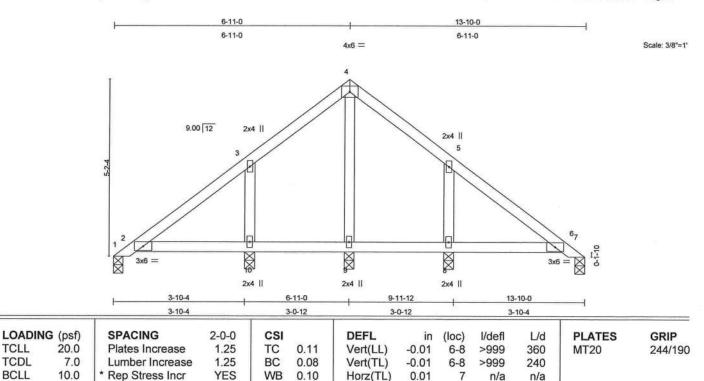
Uniform Loads (plf)

Vert: 1-2=-153(F=-87), 2-3=-141(F=-87), 3-4=-141(F=-87), 4-5=-153(F=-87), 2-4=-10



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	PB2	PIGGYBACK	8	1		J1921267
					Job Reference (optional)	

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LUMBER

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 WFRS

5.0

BRACING

TOP CHORD

Structural wood sheathing directly applied or

Weight: 58 lb

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

REACTIONS (lb/size) 1=55/0-3-8, 7=55/0-3-8, 9=254/0-3-8, 10=252/0-3-8, 8=252/0-3-8

Max Horz 1=141(load case 5)

Code FBC2004/TPI2002

Max Uplift 1=-39(load case 4), 9=-1(load case 5), 10=-137(load case 6), 8=-131(load

(Matrix)

Max Grav 1=68(load case 10), 7=68(load case 11), 9=254(load case 1), 10=259(load case 10), 8=259(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-149/144, 2-3=-120/153, 3-4=-30/131, 4-5=-2/131, 5-6=-96/153, 6-7=-35/5

BOT CHORD 2-10=-72/141, 9-10=-72/141, 8-9=-72/141, 6-8=-72/141

WEBS 4-9=-244/34, 3-10=-199/209, 5-8=-199/209

JOINT STRESS INDEX

2 = 0.26, 3 = 0.10, 4 = 0.13, 5 = 0.10, 6 = 0.26, 8 = 0.12, 9 = 0.09 and 10 = 0.12

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. Continued on page 2

January 2,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	PB2	PIGGYBACK	8	1		J1921267
					Job Reference (optional)	

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NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 5) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 1, 1 lb uplift at joint 9, 137 lb uplift at joint 10 and 131 lb uplift at joint 8.
- 7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34899 1 109 Coestal Bay Blyd Boyoton Basson Et 22445



Job Truss Truss Type Qty Ply PAPKA RESIDENCE J1921268 L262683 PB2A HIP CAP 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:07 2007 Page 1 6-0-0 7-10-0 13-10-0 6-0-0 1-10-0 Scale = 1:28.4 4x6 = 4x6 > 5 2x4 || 2x4 || 9.00 12 X 12 10 2x4 || 2x4 || 2x4 || 2x4 || 3-10-4 6-1-12 6-9-4 7-8-4 9-11-12 13-10-0 0-11-0 3-10-4 0-7-8 2-3-8 2-3-8 3-10-4 LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl I/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.10 Vert(LL) -0.01 7-9 >999 360 MT20 244/190 7.0 TCDL Lumber Increase 1.25 BC 0.11 Vert(TL) -0.01 7-9 >999 240 BCLL 10.0 Rep Stress Incr WB 0.05 YES Horz(TL) 0.01 8 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 62 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 6-0-0 oc purlins.

BOT CHORD

bracing.

WEBS

REACTIONS (lb/size) 1=60/0-3-8, 8=60/0-3-8, 13=259/0-3-8, 9=259/0-3-8, 11=230/0-3-8

Max Horz 1=-122(load case 4)

Max Uplift 1=-31(load case 4), 8=-7(load case 4), 13=-129(load case 6), 9=-121(load

case 7), 11=-32(load case 5)

Max Grav 1=76(load case 10), 8=76(load case 11), 13=262(load case 10), 9=262(load case 11), 11=230(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-125/124, 2-3=-93/140, 3-4=-11/96, 5-6=-1/96, 6-7=-92/140, 7-8=-39/8,

4-5=0/67

BOT CHORD 2-13=-62/139, 12-13=-62/139, 11-12=-67/140, 10-11=-67/140, 9-10=-62/139,

7-9=-62/139

2 X 4 SYP No.3

WEBS 4-12=-127/40, 5-10=-127/38, 3-13=-181/193, 6-9=-181/193

JOINT STRESS INDEX

2 = 0.30, 3 = 0.10, 4 = 0.05, 5 = 0.05, 6 = 0.10, 7 = 0.30, 9 = 0.11, 10 = 0.04, 12 = 0.04 and 13 = 0.11

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. Continued on page 2

Rigid ceiling directly applied or 6-0-0 oc

January 2,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	*******
L262683	PB2A	HIP CAP	1	1		J1921268
					Job Reference (optional)	

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NOTES

3) Provide adequate drainage to prevent water ponding.

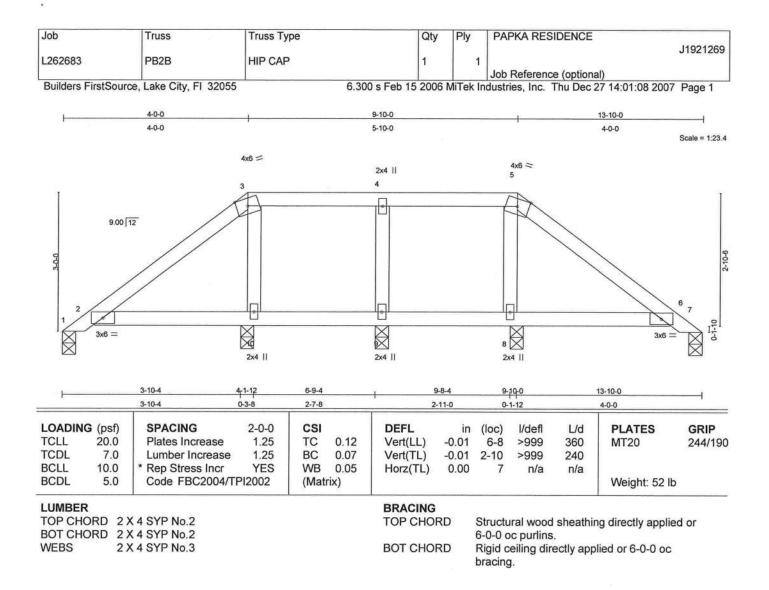
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 1, 7 lb uplift at joint 8, 129 lb uplift at joint 13, 121 lb uplift at joint 9 and 32 lb uplift at joint 11.
- 8) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard





REACTIONS (lb/size) 1=42/0-3-8, 7=42/0-3-8, 10=318/0-3-8, 8=318/0-3-8, 9=148/0-3-8

Max Horz 1=81(load case 5)

Max Uplift 1=-11(load case 4), 7=-25(load case 4), 10=-98(load case 5), 8=-77(load

case 7), 9=-76(load case 4)

Max Grav 1=66(load case 10), 7=66(load case 11), 10=318(load case 1), 8=318(load case 1), 9=168(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-74/79, 2-3=-72/180, 5-6=-72/180, 6-7=-34/18, 3-4=-1/109, 4-5=-1/109

BOT CHORD 2-10=-94/122, 9-10=-109/133, 8-9=-109/133, 6-8=-94/122

WEBS 3-10=-261/184, 5-8=-261/184, 4-9=-155/110

JOINT STRESS INDEX

2 = 0.26, 3 = 0.32, 4 = 0.06, 5 = 0.32, 6 = 0.26, 8 = 0.10, 9 = 0.06 and 10 = 0.10

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	PB2B	HIP CAP	1	1		J1921269
		J. III			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:08 2007 Page 2

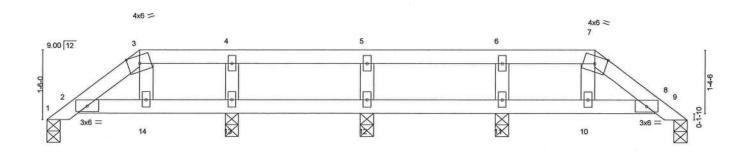
NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 11 lb uplift at joint 1, 25 lb uplift at joint 7, 98 lb uplift at joint 10, 77 lb uplift at joint 8 and 76 lb uplift at joint 9.
- 8) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	J1921270
L262683	PB2C	HIP CAP	1	1	Job Reference (optional)	31321270
Builders FirstS	ource, Lake City, FI	32055 6.3	300 s Feb 15 2006	MiTek In	ndustries, Inc. Thu Dec 27 14:01:09 2	007 Page 1
2-0-	-0		11-10-0		13-1	10-0
2-0-	-0		9-10-0		2-0	0-0 Scale = 1:23.4



1	2-1-12	3-10-4	1	6-9-4		1	9-11-12			11-8-4	13-10-0	
	2-1-12	1-8-8		2-11-0			3-2-8		-	1-8-8	2-1-12	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	-0.01	10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	10	>999	240	Charles Allegans	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 46 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD	2 X 4 SYP No.2		6-0-0 oc purlins.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=131/0-3-8, 9=131/0-3-8, 13=225/0-3-8, 12=157/0-3-8, 11=225/0-3-8

Max Horz 1=39(load case 5)

Max Uplift 1=-26(load case 6), 9=-29(load case 7), 13=-89(load case 5), 12=-70(load

case 4), 11=-79(load case 4)

Max Grav 1=131(load case 1), 9=131(load case 1), 13=229(load case 10),

12=165(load case 10), 11=229(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-69/43, 2-3=-129/73, 7-8=-129/73, 8-9=-69/43, 3-4=-93/81, 4-5=-93/82,

5-6=-93/82, 6-7=-93/81

BOT CHORD 2-14=-25/93, 13-14=-22/93, 12-13=-22/93, 11-12=-22/93, 10-11=-22/93,

8-10=-23/93

WEBS 3-14=-7/32, 7-10=-7/32, 4-13=-166/131, 5-12=-151/133, 6-11=-166/131

JOINT STRESS INDEX

2 = 0.24, 3 = 0.04, 4 = 0.07, 5 = 0.07, 6 = 0.07, 7 = 0.04, 8 = 0.24, 10 = 0.02, 11 = 0.07, 12 = 0.07, 13 = 0.07 and 14 = 0.02

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. Continued on page 2

dulius Lee Trues Clesion Engineer Florida PE No. 34869 1199 Chastel Bay Blyd Boynton Beach, FL 33435

January 2,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	PB2C	HIP CAP	1	1		J1921270
				1	Job Reference (optional)	

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NOTES

- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 1, 29 lb uplift at joint 9, 89 lb uplift at joint 13, 70 lb uplift at joint 12 and 79 lb uplift at joint 11.
- 9) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

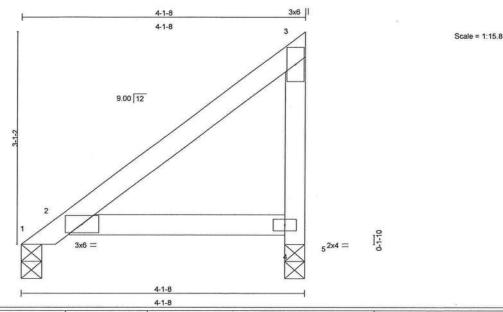
LOAD CASE(S) Standard

Julius Les Truss Design Engineer Flonda PE No. 34899 1199 Coastel Bay Blvd



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	PB3	MONO PIGGYBACK	13	1	T .	J1921271
1202000	1 60	MONOTIGGIBAGK	13		Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-7-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.02	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.02	2-4	>999	240	TANGE OF THE PARTY	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)	5-13-13-13-13-13-13-13-13-13-13-13-13-13-					Weight: 16 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD 2-0-0 oc purlins, except end verticals

(Switched from sheeted: Spacing > 2-0-0).

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

JOINTS 1 Brace at Jt(s): 3

REACTIONS (lb/size) 1=160/0-3-8, 5=159/0-3-8

Max Horz 1=119(load case 6)

Max Uplift 5=-88(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-148/0, 2-3=-99/27, 4-5=-159/161, 3-4=-100/131

BOT CHORD 2-4=-44/43

JOINT STRESS INDEX

2 = 0.52, 3 = 0.25 and 4 = 0.54

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

January 2,2008 Continued on page 2





Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	80/1920/2011
L262683	PB3	MONO PIGGYBACK	13	1		J1921271
					Job Reference (optional)	

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NOTES

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 88 lb uplift at joint 5.
- 6) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

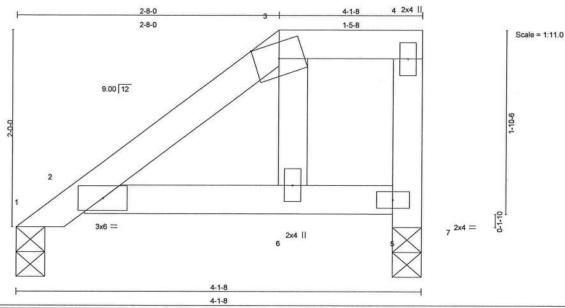
Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	PB3B	MONO HIP PIGGYBACK	1	1		J1921272
					Job Reference (optional)	

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.01	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.01	Horz(TL)	-0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 16 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

4-1-8 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=124/0-3-8, 7=123/0-3-8

Max Horz 1=62(load case 6)

Max Uplift 1=-15(load case 6), 7=-37(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-65/0, 2-3=-91/20, 3-4=-53/51, 5-7=-123/119, 4-5=-65/68

BOT CHORD 2-6=-57/55, 5-6=-52/53

WEBS 3-6=-26/54

JOINT STRESS INDEX

2 = 0.26, 3 = 0.03, 4 = 0.39, 5 = 0.53 and 6 = 0.03

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp. B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. Continued on page 2

January 2,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	PB3B	MONO HIP PIGGYBACK	1	1		J1921272
					Job Reference (optional)	

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NOTES

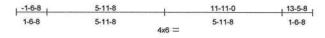
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 1 and 37 lb uplift at joint 7.
- 7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T01	COMMON	1	4		J1921273
L202000	101	COMMON	,		Job Reference (optional)	

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Scale = 1:54.9

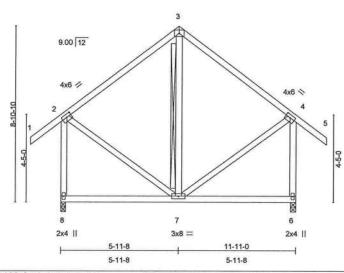


Plate Offsets	(X,Y):	[2:0-3-0	0,0-1-12]	, [4:0-3-0	,0-1-12]
	- 1				

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.02	7-8	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	-0.03	7-8	>999	240	1.00,144,99594	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		14 Careta Martines					Weight: 91 lb	

 ш	M	D	_	R

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

BRACING

WEBS

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc

BOT CHORD

bracing.

T-Brace:

2 X 4 SYP No.3 - 3-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=463/0-3-0, 6=463/0-3-0

Max Horz 8=288(load case 5)

Max Uplift 8=-119(load case 6), 6=-119(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/55, 2-3=-265/268, 3-4=-265/268, 4-5=0/55, 2-8=-434/309, 4-6=-434/309

BOT CHORD 7-8=-251/272, 6-7=-16/87

3-7=-118/71, 2-7=-104/186, 4-7=-104/187 **WEBS**

JOINT STRESS INDEX

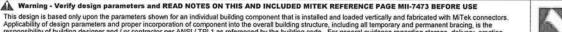
2 = 0.65, 3 = 0.54, 4 = 0.65, 6 = 0.49, 7 = 0.14 and 8 = 0.49

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

January 2,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occe. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 593 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T01	COMMON	1	1		J1921273
					Job Reference (optional)	

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NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 lb uplift at joint 8 and 119 lb uplift at joint 6.

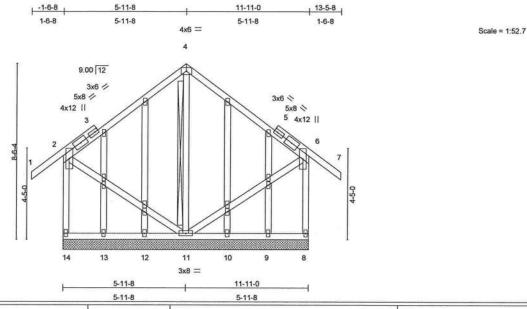
LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida PE No. 3-1869 1109 Coastal Bay Blivit



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	A-1014
L262683	T01G	GABLE	1	1		J1921274
					Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.02	6-7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	0.03	6-7	n/r	90	Carried Carried	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.15	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	Code FBC2004/TPI2002		rix)	N. C.					Weight: 128 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS**

2 X 4 SYP No.3

OTHERS 2 X 4 SYP No.3 BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 -4-11

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 14=206/11-11-0, 8=206/11-11-0, 11=465/11-11-0, 12=25/11-11-0,

13=-1/11-11-0, 10=25/11-11-0, 9=-1/11-11-0

Max Horz 14=-354(load case 4)

Max Uplift 14=-75(load case 6), 8=-76(load case 7), 11=-334(load case 6), 13=-8(load

case 10), 9=-8(load case 11)

Max Grav 14=239(load case 10), 8=239(load case 11), 11=465(load case 1), 12=61(load case 2), 13=57(load case 2), 10=61(load case 2), 9=57(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/48, 2-3=-64/60, 3-4=-19/139, 4-5=-18/139, 5-6=-64/60, 6-7=0/48,

2-14=-208/117, 6-8=-208/117

BOT CHORD 13-14=-322/329, 12-13=-322/329, 11-12=-322/329, 10-11=-23/68, 9-10=-23/68,

WEBS 4-11=-370/126, 2-11=-123/260, 6-11=-124/261

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T01G	GABLE	1	1		J1921274
		100000000000000000000000000000000000000		1 8	Job Reference (optional)	

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JOINT STRESS INDEX

2 = 0.72, 3 = 0.19, 3 = 0.00, 3 = 0.00, 3 = 0.26, 4 = 0.62, 5 = 0.19, 5 = 0.00, 5 = 0.26, 5 = 0.00, 6 = 0.72, 8 = 0.35, 9 = 0.00, 10 = 0.00, 11 = 0.19, 12 = 0.00, 13 = 0.00, 14 = 0.35, 15 = 0.00, 15 = 0.00, 16 = 0.00, 17 = 0.00, 17 = 0.00, 18 = 0.00, 19 = 0.00, 19 = 0.00, 19 = 0.00, 19 = 0.00, 19 = 0.00, 19 = 0.00, 19 = 0.00, 19 = 0.00, 19 = 0.00, 10

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 75 lb uplift at joint 14, 76 lb uplift at joint 8, 334 lb uplift at joint 11, 8 lb uplift at joint 13 and 8 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Truss Cesign Engineer Flonda PE No. 24869 1109 Coastal Bay Blvd Boydon Basen Florans



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T02	ATTIC	7	1		J1921275
L202003	102	ATTIC			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:14 2007 Page 1

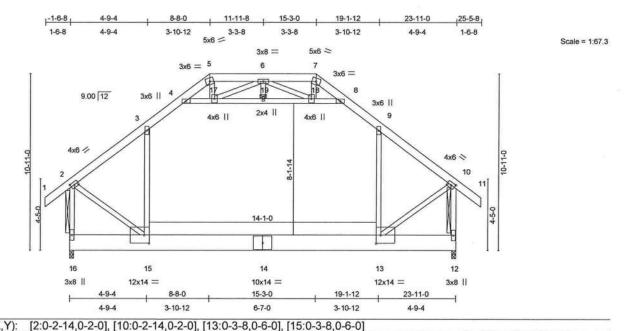


Plate Offsets (X,Y): LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl L/d **PLATES GRIP** in (loc) TCLL 20.0 Plates Increase 1.25 TC 0.44 Vert(LL) -0.17 13-15 >999 360 MT20 244/190 TCDL 7.0 1.25 BC 0.47 Lumber Increase Vert(TL) -0.27 13-15 >999 240 BCLL 10.0 Rep Stress Incr YES WB 0.42 Horz(TL) 0.01 12 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 264 lb

LUMBER	
TOP CHORD	2 X 6 SYP No.1D
BOT CHORD	2 X 12 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and

2-0-0 oc purlins (6-0-0 max.): 5-7.

BOT CHORD

Rigid ceiling directly applied or 9-11-1 oc

bracing.

WEBS

1 Row at midpt

T-Brace:

2 X 4 SYP No.3 -

2-16, 10-12

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 16=1710/0-3-0, 12=1710/0-3-0 Max Horz 16=328(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/55, 2-3=-1463/189, 3-4=-1122/315, 4-5=-229/243, 5-6=-60/275, 6-7=-60/275,

7-8=-229/242, 8-9=-1122/315, 9-10=-1463/189, 10-11=0/55, 2-16=-1954/220,

10-12=-1954/220

BOT CHORD 15-16=-284/312, 14-15=-27/1058, 13-14=-27/1058, 12-13=-16/77

3-15=-12/446, 9-13=-12/446, 4-17=-1083/121, 17-19=-864/8, 18-19=-864/8, **WEBS**

8-18=-1083/121, 5-17=-37/226, 7-18=-37/226, 6-19=0/31, 6-17=-403/128,

6-18=-403/127, 2-15=-14/1322, 10-13=-14/1322

Continued on page 2

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🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T02	ATTIC	- 7	1		J1921275
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:14 2007 Page 2

JOINT STRESS INDEX

2 = 0.74, 3 = 0.18, 4 = 0.36, 5 = 0.41, 6 = 0.56, 7 = 0.41, 8 = 0.36, 9 = 0.18, 10 = 0.74, 12 = 0.31, 13 = 0.21, 14 = 0.46, 15 = 0.21, 16 = 0.31, 17 = 0.33, 18 = 0.33 and 19 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Ceiling dead load (5.0 psf) on member(s). 3-4, 8-9, 4-17, 17-19, 18-19, 8-18; Wall dead load (5.0 psf) on member(s). 3-15, 9-13
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 13-15
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

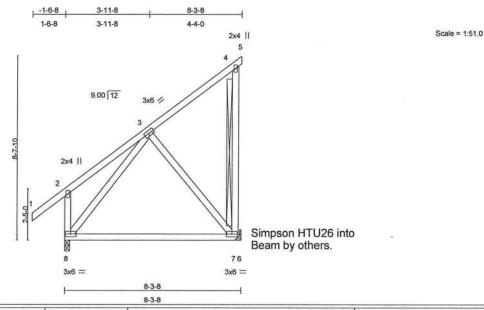
LOAD CASE(S) Standard

Julius Les Truse Design Engineer Florida PE, No. 24865 1109 Coestel Bay Blvd Occasion Bassell Rey



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T02B	MONO TRUSS	3	1		J1921276
2202000	1025	Mono mode	0		Job Reference (optional)	

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.08	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.14	7-8	>661	240	0.0000000000000000000000000000000000000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.20	Horz(TL)	-0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	ode FBC2004/TPI2002 (N		rix)						Weight: 64 lb	

L				
	u		_	

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS T-Brace: 2 X 4 SYP No.3 - 4-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=352/0-3-0, 7=249/Mechanical

Max Horz 8=310(load case 6) Max Uplift 7=-215(load case 6) Recommended hanger connection based on manufacturer tested capacities and nail calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Hanger connection to be reviewed and approved by the Architect/Engineer of Record.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-8=-208/297, 1-2=0/55, 2-3=-118/205, 3-4=-86/49, 4-5=-2/0, 4-7=-97/113

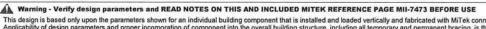
BOT CHORD 7-8=-196/97, 6-7=0/0 **WEBS** 3-8=-294/56, 3-7=-142/308

JOINT STRESS INDEX

2 = 0.67, 3 = 0.18, 4 = 0.38, 7 = 0.38 and 8 = 0.68

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Continued on page 2



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T02B	MONO TRUSS	3	1		J1921276
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:14 2007 Page 2

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 215 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida FE No. 34868 1169 Coastal Bay Blvd Govoton Bason, Ft. 33435

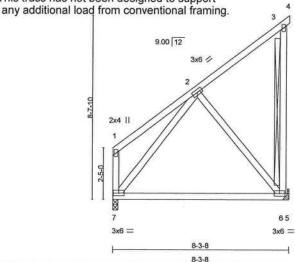


Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T02C	MONO TRUSS	3	1		J1921277
EEOEOOO	1020	More moss			Job Reference (optional)	

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Scale = 1:51.0



Simpson HTU26 into Beam by others.

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.08	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.14	6-7	>667	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.16	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0 Code FBC2004/TPI2002		(Mat	rix)						Weight: 61 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS T-Brace: 2 X 4 SYP No.3 - 3-6

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 7=251/0-3-0, 6=258/Mechanical

Max Horz 7=246(load case 6) Max Uplift 6=-210(load case 6)

Recommended hanger connection based on manufacturer tested capacities and nail calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Hanger connection to be reviewed and approved by the Architect/Engineer of Record.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-7=-127/146, 1-2=-124/146, 2-3=-85/48, 3-4=-2/0, 3-6=-95/110

BOT CHORD 6-7=-195/107, 5-6=0/0 2-7=-231/62, 2-6=-158/307 **WEBS**

JOINT STRESS INDEX

1 = 0.46, 2 = 0.17, 3 = 0.36, 6 = 0.38 and 7 = 0.68

January 2,2008

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T02C	MONO TRUSS	3	1		J1921277
					Job Reference (optional)	

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NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 210 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34869 1100 Ceastel Bay Blvd Boydon Beach Et 23435



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T02G	GABLE	1	1	₹.	J1921278
					Job Reference (optional)	

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Scale = 1:65.6

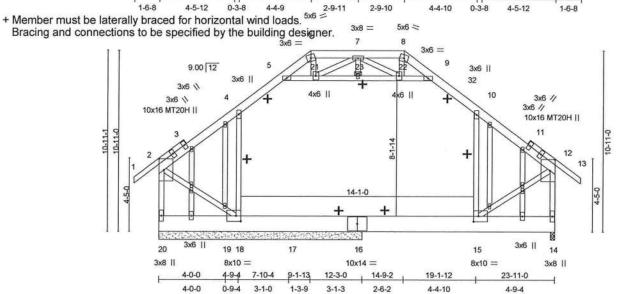


Plate Of	fsets (X,Y):	[2:0-8-0,0-3-8], [12:	0-8-0,0-3-8], [15:0-3	3-8,0-4-0]	, [18:0-3-8,0-4	-0]					
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.03	15-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.05	15-16	>999	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.37	Horz(TL)	0.01	14	n/a	n/a	100000000000000000000000000000000000000	
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 289 lb	į.

LUMBER	
TOP CHORD	2 X 6 SYP No 1D *Evcent*

1-3 2 X 4 SYP No.2, 11-13 2 X 4 SYP No.2

BOT CHORD 2 X 12 SYP No.2

2 X 4 SYP No.3 WEBS

2 X 4 SYP No.3 **OTHERS**

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals, and 2-0-0 oc

purlins (6-0-0 max.): 6-8.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. **WEBS** 1 Row at midpt

REACTIONS (lb/size) 20=1385/0-3-0, 16=1038/0-3-8, 14=1393/0-3-0, 17=131/0-3-8, 19=115/0-3-8

Max Horz 20=417(load case 5)

Max Uplift 20=-626(load case 4), 16=-9(load case 7), 14=-349(load case 7), 17=-176(load

case 5), 19=-278(load case 5)

Max Grav 20=1385(load case 1), 16=1081(load case 12), 14=1393(load case 1), 17=221(load case 11), 19=420(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-6/66, 2-3=-974/437, 3-4=-907/432, 4-5=-993/456, 5-6=-665/455, 6-7=-534/459,

7-8=-626/407, 8-9=-771/452, 9-32=-907/471, 10-32=-1014/468, 10-11=-900/312,

11-12=-944/286, 12-13=0/45, 2-20=-1373/591, 12-14=-1271/387

BOT CHORD 19-20=-359/385, 18-19=-359/385, 17-18=-275/725, 16-17=-275/725, 15-16=-275/725,

14-15=-20/76

4-18=-499/321, 10-15=-427/201, 5-21=-312/254, 21-23=-350/149, 22-23=-350/149,

9-22=-201/134, 6-21=-123/228, 8-22=-61/141, 7-23=0/25, 7-21=-397/275, 7-22=-232/163,

2-18=-481/932, 12-15=-319/927

JOINT STRESS INDEX

WEBS

2 = 0.44, 3 = 0.00, 3 = 0.34, 3 = 0.56, 4 = 0.56, 4 = 0.16, 5 = 0.15, 6 = 0.29, 7 = 0.57, 8 = 0.26, 9 = 0.15, 10 = 0.16, 11 = 0.00, 11 = 0.3411 = 0.43, 11 = 0.43, 12 = 0.38, 14 = 0.21, 15 = 0.15, 16 = 0.28, 18 = 0.15, 20 = 0.30, 21 = 0.33, 22 = 0.33, 23 = 0.34, 24 = 0.34 (26) + 0.24 (26)Continued $\frac{26}{50}$ = 0.34, $\frac{26}{50}$ = 0.34, $\frac{27}{50}$ = 0.16, $\frac{28}{50}$ = 0.34, $\frac{29}{50}$ = 0.34, $\frac{30}{50}$ = 0.16, $\frac{31}{50}$ = 0.34 and $\frac{31}{50}$ = 0.34

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is 1 responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, er and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T02G	GABLE	1	1		J1921278
					Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Provide adequate drainage to prevent water ponding.
- 5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) Ceiling dead load (5.0 psf) on member(s). 4-5, 9-10, 5-21, 21-23, 22-23, 9-22; Wall dead load (5.0 psf) on member(s).4-18, 10-15
- 10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 17-18, 16-17, 15-16
- 11) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 626 lb uplift at joint 20, 9 lb uplift at joint 16, 349 lb uplift at joint 14, 176 lb uplift at joint 17 and 278 lb uplift at joint 19.
- 13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 14) Gable truss supports 1' 4" max. rake gable overhang.

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-2=-79(F=-25), 2-4=-79(F=-25), 4-5=-89(F=-25), 5-6=-79(F=-25), 6-7=-79(F=-25), 7-8=-106, 8-9=-106, 9-32=-116, 10-32=-64, 10-12=-54, 12-13=-54, 18-20=-10, 15-18=-110, 14-15=-10, 5-9=-10

Drag: 4-18=-10, 10-15=-10

Julius Lee Truse Design Engineer Florida PE No. 24869 1100 Coestal Bay Blvd Bovoton Basser (1244)



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	14004070
L262683	Т03	ATTIC	3	_		J1921279
				2	Job Reference (optional)	

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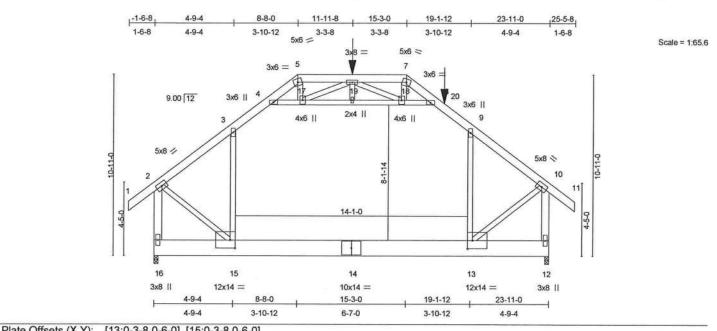


Plate Of	isets (A, Y):	[13:0-3-8,0-6-0], [1	5:0-3-8,0-6	·0]								
LOADIN	G (psf)	SPACING	2-10-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.12	13-15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.20	13-15	>999	240		
BCLL	10.0	Rep Stress Incr	NO	WB	0.57	Horz(TL)	0.01	12	n/a	n/a		
BCDL	5.0	Code FBC2004/T	PI2002	(Mat	rix)	27. 75.					Weight: 540 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D **BOT CHORD** 2 X 12 SYP No.2

WEBS

2 X 4 SYP No.3 *Except*

2-16 2 X 6 SYP No.1D, 10-12 2 X 6 SYP No.1D

BRACING

JOINTS

TOP CHORD 2-0-0 oc purlins (6-0-0 max.), except end verticals

(Switched from sheeted: Spacing > 2-0-0). **BOT CHORD**

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Brace at Jt(s): 5, 7, 2, 10

REACTIONS (lb/size) 16=2826/0-3-0, 12=3866/0-3-0

Max Horz 16=466(load case 4)

Max Uplift 16=-86(load case 5), 12=-192(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/81, 2-3=-2566/98, 3-4=-2056/208, 4-5=-668/483, 5-6=-341/503, 6-7=-541/559,

7-8=-785/515, 8-20=-1909/198, 9-20=-2213/229, 9-10=-2744/93, 10-11=0/81,

2-16=-3429/134, 10-12=-3612/171

BOT CHORD

15-16=-403/476, 14-15=-150/1890, 13-14=-150/1890, 12-13=-10/208

WEBS

3-15=0/716, 9-13=-89/563, 4-17=-1704/64, 17-19=-1182/0, 18-19=-1182/0, 8-18=-1594/19,

5-17=-129/554, 7-18=-101/454, 6-19=0/39, 6-17=-1093/359, 6-18=-873/297,

2-15=-210/2383, 10-13=-183/2208

JOINT STRESS INDEX

2 = 0.66, 3 = 0.16, 4 = 0.29, 5 = 0.31, 6 = 0.57, 7 = 0.31, 8 = 0.29, 9 = 0.16, 10 = 0.66, 12 = 0.41, 13 = 0.20, 14 = 0.33, 15 = 0.20, 16 = 0.20, 0.41, 17 = 0.34, 18 = 0.34 and 19 = 0.34

us Les 22 Design Engineer 1da PE No. 34888 8 Gastal Bay Elvi 1000 Besch. FL 33435

January 2,2008

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T03	ATTIC	3			J1921279
LEGEGGG	100	ATTIO	J	2	Job Reference (optional)	

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NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.

Bottom chords connected as follows: 2 X 12 - 2 rows at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 5) Provide adequate drainage to prevent water ponding.
- 6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Ceiling dead load (5.0 psf) on member(s). 3-4, 8-9, 4-17, 17-19, 18-19, 8-18; Wall dead load (5.0 psf) on member(s).3-15, 9-13
- 8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 13-15
- 9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 16 and 192 lb uplift at ioint 12.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

```
LOAD CASE(S) Standard Except:
```

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-77, 2-3=-77, 3-4=-91, 4-5=-77, 5-7=-76, 7-8=-77, 8-20=-91, 10-11=-77, 15-16=-14, 13-15=-156, 12-13=-156(F=-142),

Drag: 3-15=-14, 9-13=-14

Concentrated Loads (lb)

Vert: 6=-435(F) 20=-220(F)

Trapezoidal Loads (plf)

Vert: 20=-152(F=-61)-to-9=-166(F=-76), 9=-152(F=-76)-to-10=-192(F=-115)

9) Attic Floor: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 1-2=-20, 2-3=-20, 3-4=-34, 4-5=-20, 5-7=-20, 7-8=-20, 8-20=-34, 10-11=-20, 15-16=-14,

13-15=-156, 12-13=-156(F=-142), 4-8=-14

Drag: 3-15=-14, 9-13=-14

Concentrated Loads (lb)

Vert: 6=-163(F) 20=-83(F)

Trapezoidal Loads (plf)

Vert: 20=-57(F=-23)-to-9=-72(F=-38), 9=-57(F=-38)-to-10=-97(F=-77)

10) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-77, 2-3=-77, 3-4=-91, 4-5=-77, 5-7=-76, 7-8=-20, 8-20=-34, 10-11=-20, 15-16=-14,

13-15=-156, 12-13=-156(F=-142), 4-8=-14

Drag: 3-15=-14, 9-13=-14

Concentrated Loads (lb)

Vert: 6=-435(F) 20=-220(F)

Trapezoidal Loads (plf)

Vert: 20=-95(F=-61)-to-9=-110(F=-76), 9=-96(F=-76)-to-10=-135(F=-115)

11) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-20, 2-3=-20, 3-4=-34, 4-5=-20, 5-7=-76, 7-8=-77, 8-20=-91, 10-11=-77, 15-16=-14,

13-15=-156, 12-13=-156(F=-142), 4-8=-14

Drag: 3-15=-14, 9-13=-14

Concentrated Loads (lb)

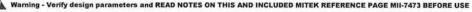
Vert: 6=-435(F) 20=-220(F)

Trapezoidal Loads (plf)

Vert: 20=-152(F=-61)-to-9=-166(F=-76), 9=-152(F=-76)-to-10=-192(F=-115)



January 2,2008



Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	11001000
L262683	T03A	ATTIC	1	_		J1921280
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 02 11:00:00 2008 Page 1

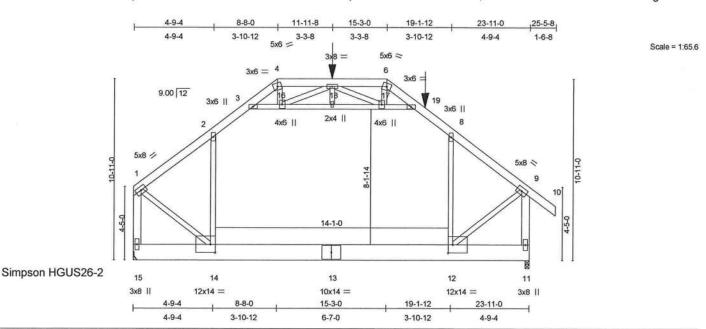


Plate Of	fsets (X,Y	:	[12:0-3-8,0-6-0], [14	4:0-3-8,0-6-	-0]							- W	
LOADIN	G (psf)		SPACING	2-10-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.41	Vert(LL)	-0.12	12-14	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.20	12-14	>999	240		
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.58	Horz(TL)	0.01	11	n/a	n/a		
BCDL	5.0		Code FBC2004/T	PI2002	(Mat	rix)						Weight: 531 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D

BOT CHORD 2 X 12 SYP No.2

WEBS

2 X 4 SYP No.3 *Except*

1-15 2 X 6 SYP No.1D, 9-11 2 X 6 SYP No.1D

BRACING

TOP CHORD

BOT CHORD JOINTS

(Switched from sheeted: Spacing > 2-0-0).

2-0-0 oc purlins (6-0-0 max.), except end verticals

Rigid ceiling directly applied or 10-0-0 oc bracing. 1 Brace at Jt(s): 1, 4, 6, 9

REACTIONS (lb/size) 15=2685/Mechanical, 11=3871/0-3-0

Max Horz 15=-415(load case 3) Max Uplift 11=-179(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-2570/99, 2-3=-2062/194, 3-4=-661/488, 4-5=-333/509, 5-6=-539/554, 6-7=-783/512.

7-19=-1915/177, 8-19=-2219/206, 8-9=-2753/67, 9-10=0/81, 1-15=-3272/100,

9-11=-3625/137

BOT CHORD

14-15=-319/462, 13-14=-129/1898, 12-13=-129/1898, 11-12=-11/209

WEBS

2-14=0/715, 8-12=-87/565, 3-16=-1708/40, 16-18=-1185/0, 17-18=-1185/0, 7-17=-1598/0, 4-16=-130/554, 6-17=-96/455, 5-18=0/39, 5-16=-1093/359, 5-17=-875/287, 1-14=-198/2355,

9-12=-155/2218

JOINT STRESS INDEX

1 = 0.66, 2 = 0.16, 3 = 0.29, 4 = 0.31, 5 = 0.57, 6 = 0.31, 7 = 0.29, 8 = 0.16, 9 = 0.66, 11 = 0.41, 12 = 0.20, 13 = 0.33, 14 = 0.20, 15 = 0.41, 12 = 0.0.41, 16 = 0.34, 17 = 0.34 and 18 = 0.34

llus Lee 19e Design Engineer 19da PE No. 34eeb 88 Ceestel Bay Blvd 19mm Besch, FL 99435

January 2,2008

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T03A	ATTIC	1			J1921280
				2	Job Reference (optional)	

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NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.

Bottom chords connected as follows: 2 X 12 - 2 rows at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 5) Provide adequate drainage to prevent water ponding.
- 6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Ceiling dead load (5.0 psf) on member(s). 2-3, 7-8, 3-16, 16-18, 17-18, 7-17; Wall dead load (5.0 psf) on member(s).2-14, 8-12
- 8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
- 9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 179 lb uplift at joint 11.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

```
LOAD CASE(S) Standard Except:
```

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-77, 2-3=-91, 3-4=-77, 4-6=-76, 6-7=-77, 7-19=-91, 9-10=-77, 14-15=-14, 12-14=-156, 11-12=-156(F=-142), 3-7=-14

Drag: 2-14=-14, 8-12=-14

Concentrated Loads (lb)

Vert: 5=-435(F) 19=-220(F)

Trapezoidal Loads (plf)

Vert: 19=-152(F=-61)-to-8=-166(F=-76), 8=-152(F=-76)-to-9=-192(F=-115)

9) Attic Floor: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 1-2=-20, 2-3=-34, 3-4=-20, 4-6=-20, 6-7=-20, 7-19=-34, 9-10=-20, 14-15=-14, 12-14=-156, 11-12=-156(F=-142), 3-7=-14

Drag: 2-14=-14, 8-12=-14

Concentrated Loads (lb)

Vert: 5=-163(F) 19=-83(F)

Trapezoidal Loads (plf)

Vert: 19=-57(F=-23)-to-8=-72(F=-38), 8=-57(F=-38)-to-9=-97(F=-77)

10) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-77, 2-3=-91, 3-4=-77, 4-6=-76, 6-7=-20, 7-19=-34, 9-10=-20, 14-15=-14, 12-14=-156,

11-12=-156(F=-142), 3-7=-14

Drag: 2-14=-14, 8-12=-14

Concentrated Loads (lb)

Vert: 5=-435(F) 19=-220(F)

Trapezoidal Loads (plf)

Vert: 19=-95(F=-61)-to-8=-110(F=-76), 8=-96(F=-76)-to-9=-137(F=-117)

11) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-20, 2-3=-34, 3-4=-20, 4-6=-76, 6-7=-77, 7-19=-91, 9-10=-77, 14-15=-14, 12-14=-156,

11-12=-156(F=-142), 3-7=-14

Drag: 2-14=-14, 8-12=-14

Concentrated Loads (lb)

Vert: 5=-435(F) 19=-220(F)

Trapezoidal Loads (plf)

Vert: 19=-152(F=-61)-to-8=-167(F=-76), 8=-153(F=-76)-to-9=-194(F=-117)

Julius Lee Truse Cesian Engineer Florida PE No. 34888 1189 Geestel Bay Blyd Gwyson Besch El 18448



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T04	ATTIC	4	1		J1921281
					Job Reference (optional)	

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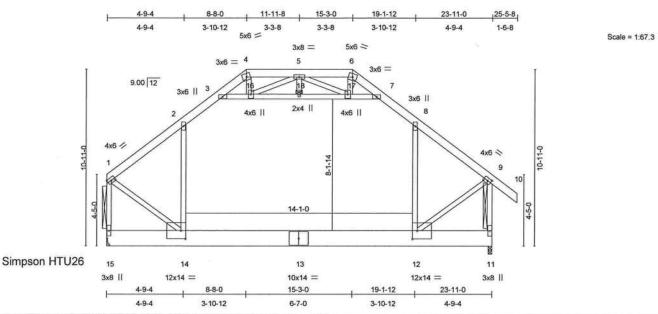


Plate Offsets (X,Y): [9:0-2-14,0-2-0], [12:0-3-8,0-6-0], [14:0-3-8,0-6-0]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.17	12-14	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.28	12-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.01	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 260 lb	

11	IIV	R	F	R

TOP CHORD 2 X 6 SYP No.1D BOT CHORD 2 X 12 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and

2-0-0 oc purlins (6-0-0 max.): 4-6.

BOT CHORD

Rigid ceiling directly applied or 9-8-12 oc

bracing.

WEBS

1 Row at midpt

3-7

T-Brace:

2 X 4 SYP No.3 -

1-15, 9-11

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 15=1616/Mechanical, 11=1713/0-3-0

Max Horz 15=-292(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1461/129, 2-3=-1127/295, 3-4=-223/245, 4-5=-52/277, 5-6=-59/271,

6-7=-228/245, 7-8=-1126/289, 8-9=-1469/155, 9-10=0/55, 1-15=-1851/152.

9-11=-1963/177

BOT CHORD 14-15=-221/305, 13-14=-13/1063, 12-13=-13/1063, 11-12=-16/76

WEBS 2-14=-14/444, 8-12=-3/448, 3-16=-1086/95, 16-18=-867/0, 17-18=-867/0,

7-17=-1086/73, 4-16=-40/226, 6-17=-32/227, 5-18=0/31, 5-16=-404/128,

5-17=-404/120, 1-14=-16/1318, 9-12=0/1329

Julius Les Trues Design Engineer Florida FE No. 3-1869 1 100 Coastal Ray Blvd Boynton Beach, FL 33-435

Continued on page 2

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Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T04	ATTIC	4	1		J1921281
					Job Reference (optional)	

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JOINT STRESS INDEX

1 = 0.80, 2 = 0.18, 3 = 0.36, 4 = 0.41, 5 = 0.56, 6 = 0.41, 7 = 0.36, 8 = 0.18, 9 = 0.80, 11 = 0.31, 12 = 0.21, 13 = 0.46, 14 = 0.21, 15 = 0.31, 16 = 0.33, 17 = 0.33 and 18 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Ceiling dead load (5.0 psf) on member(s). 2-3, 7-8, 3-16, 16-18, 17-18, 7-17; Wall dead load (5.0 psf) on member(s). 2-14, 8-12
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

LOAD CASE(S) Standard

Julius Lee Truse Cesign Engineer Florida PE No. 34866 1109 Ceasiel Bay Blyd Boynton Beach, Et 33435



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T05	HIP	1			J1921282
		,		2	Job Reference (optional)	

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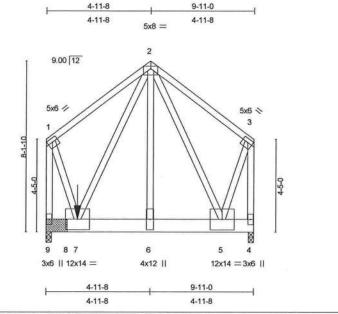


Plate Of	fsets (X,Y)	[1:0-2-12,0-2-0], [3:	0-2-12,0-2	-0]								
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.00	TC	0.93	Vert(LL)	-0.03	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.00	BC	0.26	Vert(TL)	-0.06	6-7	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.47	Horz(TL)	0.00	4	n/a	n/a		
BCDL	BCDL 5.0 Code FBC2004/TPI2002		(Matrix)			, , ,				Weight: 224 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 8 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.2 *Except*

1-9 2 X 4 SYP No.3, 3-4 2 X 4 SYP No.3

BRACING

BOT CHORD

TOP CHORD Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 9=5469/0-3-4 (0-3-0 + bearing block), 4=4474/0-3-0

Max Horz 9=-263(load case 3)

Max Uplift 9=-1548(load case 6), 4=-1290(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1792/600, 2-3=-1409/496, 1-9=-5015/1447, 3-4=-3901/1145

BOT CHORD 8-9=-241/220, 7-8=-241/220, 6-7=-542/1592, 5-6=-542/1592, 4-5=-42/73

WEBS 2-6=-935/3243, 1-7=-1135/4067, 3-5=-978/3135, 2-7=-544/309, 2-5=-1286/460

JOINT STRESS INDEX

1 = 0.92, 2 = 0.51, 3 = 0.92, 4 = 0.56, 5 = 0.41, 6 = 0.40, 7 = 0.41, 8 = 0.00, 8 = 0.00, 8 = 0.00, 9 = 0.56, 9 = 0.00 and 9 = 0.00

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 8 - 2 rows at 0-3-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) 2 X 8 SYP 2400F 2.0E bearing block 12" long at jt. 9 attached to each face with 4 rows of 10d (0.131"x3") Contains spaced 3" ozc. 16 Total fasteners per block. Bearing is assumed to be SYP.

January 2,2008

Scale = 1:51.7

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	14004000
L262683	T05	HIP	1			J1921282
The second secon				2	Job Reference (optional)	

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NOTES

4) Unbalanced roof live loads have been considered for this design.

- 5) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1548 lb uplift at joint 9 and 1290 lb uplift at joint 4.

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 1-2=-54, 2-3=-54, 7-9=-10, 4-7=-813(F=-803)

Concentrated Loads (lb)

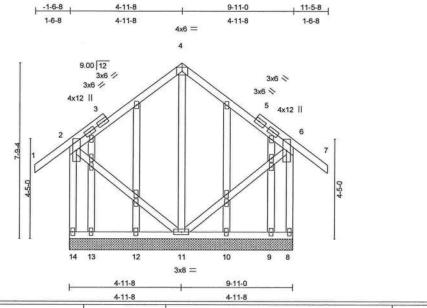
Vert: 7=-2685(F)

Julius Lee Trues Design Engineer Florida PE No. 24869 1109 Coastal Bay filvid Boynton Beach, Et. 23435



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T05G	GABLE	1	1	J192	1283
L202000	1000	GABLE	1	_ '	Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	0.01	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	6-7	n/r	90		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.29	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		, ,				1110	Weight: 113 lb	

LUMBER	
TOP CHORD	2 X 4 SYF

P No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 **OTHERS**

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals. **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc

bracing.

REACTIONS (lb/size) 14=198/9-11-0, 8=198/9-11-0, 11=367/9-11-0, 12=23/9-11-0, 13=-5/9-11-0,

10=23/9-11-0, 9=-5/9-11-0

Max Horz 14=-328(load case 4)

Max Uplift 14=-104(load case 4), 8=-100(load case 5), 11=-252(load case 6),

13=-32(load case 8), 9=-32(load case 9)

Max Grav 14=227(load case 10), 8=227(load case 11), 11=367(load case 1),

12=63(load case 2), 13=45(load case 2), 10=63(load case 2), 9=45(load

case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/48, 2-3=-59/88, 3-4=-7/114, 4-5=-6/114, 5-6=-59/88, 6-7=0/48, 2-14=-197/127

6-8=-197/127

BOT CHORD 13-14=-283/295, 12-13=-283/295, 11-12=-283/295, 10-11=-32/83, 9-10=-32/83,

8-9=-32/83

WEBS 4-11=-292/65, 2-11=-115/222, 6-11=-116/222

JOINT STRESS INDEX

2 = 0.80, 3 = 0.00, 3 = 0.23, 3 = 0.23, 4 = 0.41, 5 = 0.00, 5 = 0.23, 5 = 0.23, 6 = 0.80, 8 = 0.47, 9 = 0.00, 10 = 0.00, 11 = 0.16, 12 = 0.1612 = 0.00, 13 = 0.00, 14 = 0.47, 15 = 0.00, 15 = 0.00, 16 = 0.00, 17 = 0.00, 17 = 0.00, 18 = 0.00, 18 = 0.00, 19 = 0.00, 20 = 0.00, 18 = 0.000.00, 20 = 0.00, 21 = 0.00, 21 = 0.00, 22 = 0.00 and 22 = 0.00

Continued on page 2

January 2,2008

Scale: 1/4"=1"





Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T05G	GABLE	1	1		J1921283
		0,10212	·		Job Reference (optional)	

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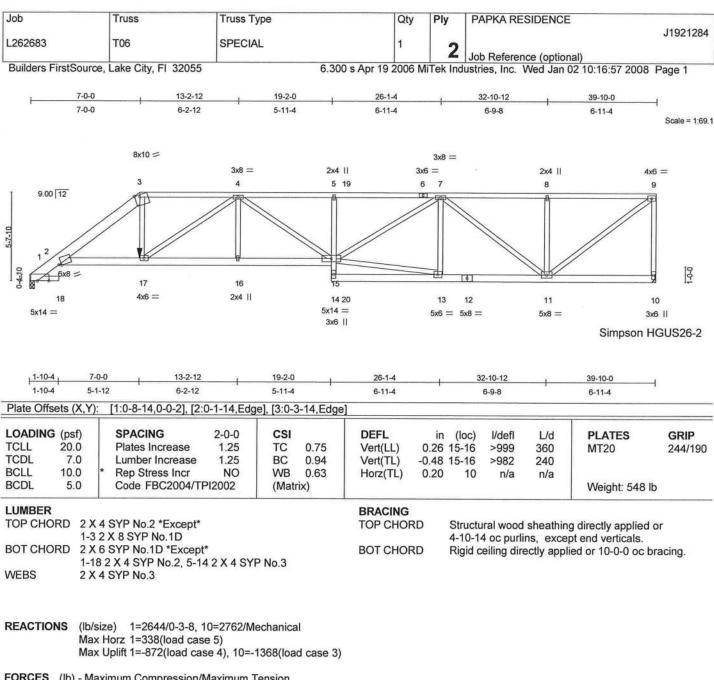
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 104 lb uplift at joint 14, 100 lb uplift at joint 8, 252 lb uplift at joint 11, 32 lb uplift at joint 13 and 32 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 24999 1109 Geesial Bay Blyd Goveron Bassen Et 22445





FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1747/453, 2-3=-4804/1794, 3-4=-3981/1511, 4-5=-6400/2776, 5-19=-6383/2852,

6-19=-6383/2852, 6-7=-6383/2852, 7-8=-3043/1480, 8-9=-3043/1480, 9-10=-2662/1381

BOT CHORD 1-18=0/0, 2-17=-1649/3898, 16-17=-2527/5781, 15-16=-2527/5781, 14-15=-12/173, 5-15=-614/386, 14-20=-397/905, 13-20=-397/905, 12-13=-2246/4769, 11-12=-2246/4769,

10-11=-57/88

WEBS 3-17=-870/2192, 4-17=-2205/1036, 4-16=-117/467, 4-15=-507/755, 13-15=-1870/3906,

7-15=-715/1905, 7-13=-491/398, 7-11=-2182/969, 8-11=-826/630, 9-11=-1800/3738

JOINT STRESS INDEX

1 = 0.10, 2 = 0.85, 3 = 0.68, 4 = 0.57, 5 = 0.53, 6 = 0.52, 7 = 0.91, 8 = 0.34, 9 = 0.78, 10 = 0.33, 11 = 0.87, 12 = 0.52, 13 = 0.62, 14 = 0.52, 13 = 0.62, 14 = 0.52, 13 = 0.62, 14 = 0.52, 15 = 0.0.78, 15 = 0.80, 16 = 0.34 and 17 = 0.50

January 2,2008

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek conne Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erec and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T06	SPECIAL	1	_		J1921284
				2	Job Reference (optional)	

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NOTES

2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc, 2 X 4 - 1 row at 0-9-0 oc.

 Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

4) Provide adequate drainage to prevent water ponding.

5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 872 lb uplift at joint 1 and 1368 lb uplift at joint 10.

LOAD CASE(S) Standard

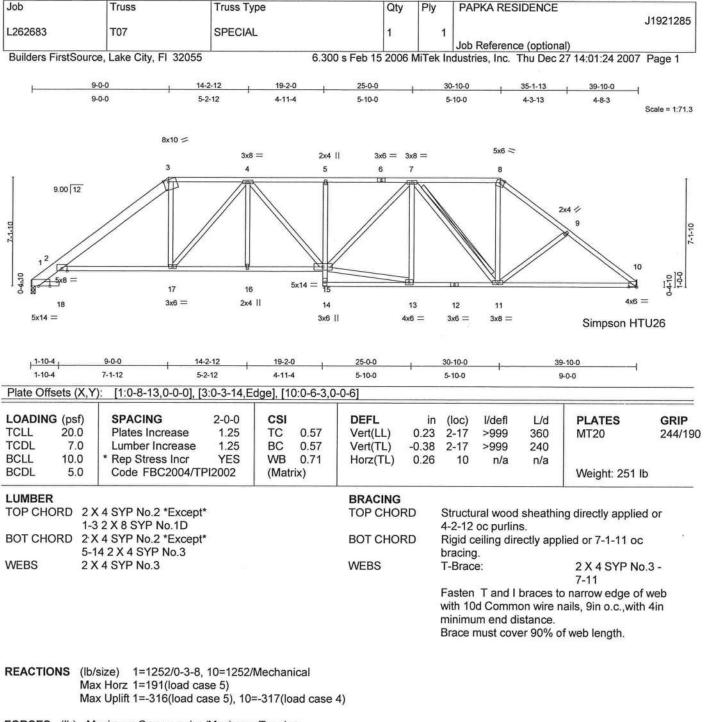
 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-19=-79(F=-25), 9-19=-118(F=-64), 1-18=-10, 2-17=-10, 15-17=-64(F=-53), 14-20=-64(F=-53), 10-20=-22(F=-12) Concentrated Loads (lb)

Vert: 17=-411(F)

Julius Les Truse Design Engineer Florida PE No. 24899 1199 Ceastel Bay Blvd Boynton Beach, FL 33435





FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=-784/399, 2-3=-1904/910, 3-4=-1518/880, 4-5=-2110/1184, 5-6=-2100/1181, 6-7=-2100/1181, 7-8=-1305/794, 8-9=-1683/903, 9-10=-1867/938

BOT CHORD 1-18=0/0, 2-17=-577/1499, 16-17=-784/1914, 15-16=-784/1914, 14-15=0/95,

5-15=-298/214, 13-14=-51/144, 12-13=-706/1705, 11-12=-706/1705,

10-11=-660/1439

3-17=-223/618, 4-17=-690/318, 4-16=0/139, 4-15=-156/339, 13-15=-665/1585,

7-15=-253/585, 7-13=-242/153, 7-11=-691/328, 8-11=-342/689, 9-11=-189/214

Julius Les Truse Design Engineer Flonda PE No. 24869 1 100 Ceastal Bay Blvd Devoton Desert El 23435

Continued on page 2

TOP CHORD

WEBS



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T07	SPECIAL	1	1		J1921285
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:24 2007 Page 2

JOINT STRESS INDEX

1 = 0.09, 2 = 0.51, 3 = 0.42, 4 = 0.56, 5 = 0.33, 6 = 0.45, 7 = 0.56, 8 = 0.48, 9 = 0.33, 10 = 0.67, 11 = 0.56, 12 = 0.55, 13 = 0.67, 14 = 0.37, 15 = 0.60, 16 = 0.33 and 17 = 0.39

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 316 lb uplift at joint 1 and 317 lb uplift at joint 10.
- 8) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34869 1199 Cressel Bay Blyd Boynton Beach, FL 23435



Job Truss Truss Type Qty Ply PAPKA RESIDENCE J1921286 L262683 T08 SPECIAL 1 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:25 2007 Page 1 11-0-0 5-6-0 15-2-12 19-2-0 24-0-0 28-10-0 34-3-10 39-10-0 5-6-0 4-2-12 3-11-4 4-10-0 4-10-0 5-5-10 5-6-6 Scale = 1:71.6 8x10 = 5x6 = 2x4 || 3x6 = 3x8 = 4 5 6 8 9.00 12 4x8 // 3x6 < 5x14 = 18 17 3x8 = 2x4 || 3x8 = 15 14 13 12 11 5x14 = 2x4 || 4x6 = 3x6 = 2x4 || Simpson HTU26 3x8 = 1-10-4 5-6-0 11-0-0 19-2-0 24-0-0 28-10-0 39-10-0 1-10-4 3-7-12 5-6-0 8-2-0 4-10-0 4-10-0 5-5-10 5-6-6 Plate Offsets (X,Y): [1:0-8-9,0-0-0], [4:0-3-14,Edge], [10:0-8-3,0-0-14] LOADING (psf) SPACING 2-0-0 DEFL CSI L/d **PLATES** GRIP (loc) I/defl in TCLL 20.0 Plates Increase 1.25 TC 0.57 Vert(LL) 0.16 16-17 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.61 Vert(TL) -0.31 16-17 >999 240 BCLL 10.0 Rep Stress Incr YES WB 0.45 Horz(TL) 0.20 10 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 280 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 *Except* TOP CHORD Structural wood sheathing directly applied or 1-4 2 X 8 SYP No.1D 4-4-12 oc purlins. **BOT CHORD** 2 X 4 SYP No.2 *Except* **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc 6-15 2 X 4 SYP No.3 bracing. Except: 2 X 4 SYP No.3 **WEBS** T-Brace: 2 X 4 SYP No.3 -6-16 **WEBS** T-Brace: 2 X 4 SYP No.3 -5-17, 7-14, 7-12 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 1=1252/0-3-8, 10=1252/Mechanical

Max Horz 1=232(load case 5)

Max Uplift 1=-271(load case 5), 10=-274(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-784/394, 2-3=-2207/1064, 3-4=-1788/927, 4-5=-1354/818, 5-6=-1679/985,

6-7=-1674/985, 7-8=-1211/779, 8-9=-1592/875, 9-10=-1900/912

BOT CHORD 1-19=0/0, 2-18=-815/1866, 17-18=-815/1867, 16-17=-574/1562, 15-16=0/73,

6-16=-238/174, 14-15=-53/58, 13-14=-524/1426, 12-13=-524/1426,

11-12=-627/1442, 10-11=-627/1442

3-18=0/160, 3-17=-600/413, 4-17=-359/737, 5-17=-512/264, 5-16=-132/302,

14-16=-482/1398, 7-16=-204/487, 7-14=-255/132, 7-12=-527/258, 8-12=-319/617,

Continued on page 12=-305/270, 9-11=0/182

WEBS

Truse Design Engineer Florida PE No. 24869 1 100 Caastal Bay Blvd Boynton Beach, Ft. 33435







Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T08	SPECIAL	1	1		J1921286
		10.000			Job Reference (optional)	

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JOINT STRESS INDEX

1 = 0.09, 2 = 0.53, 3 = 0.32, 4 = 0.48, 5 = 0.44, 6 = 0.33, 7 = 0.59, 8 = 0.52, 9 = 0.42, 10 = 0.78, 11 = 0.33, 12 = 0.59, 13 = 0.45, 14 = 0.61, 15 = 0.73, 16 = 0.55, 17 = 0.59 and 18 = 0.33

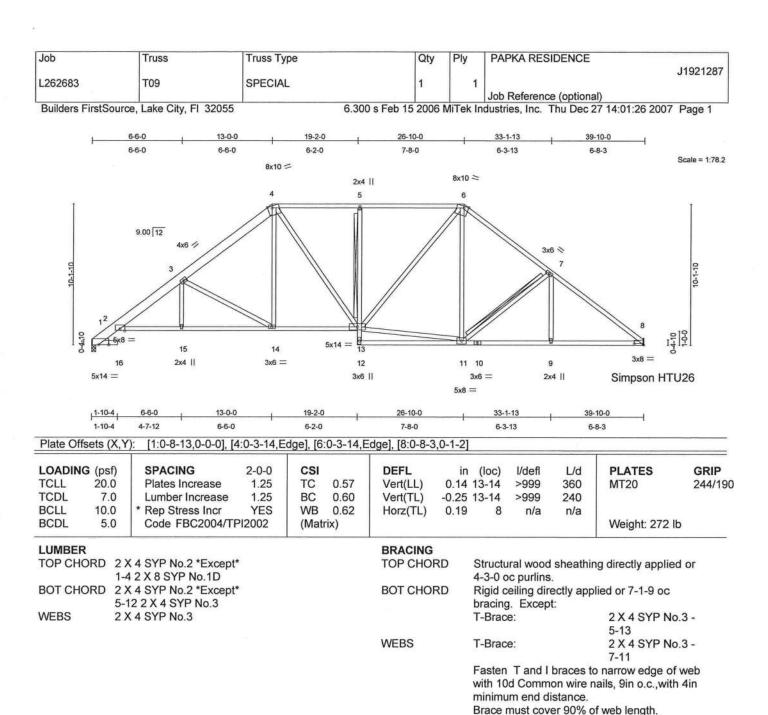
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 271 lb uplift at joint 1 and 274 lb uplift at joint 10.
- 8) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard

Julius Lee Truse Cesion Engineer Planda PE No. 34898 1189 Cestal Bay Blvd Bovnton Beach, FL 33435





REACTIONS (lb/size) 1=1252/0-3-8, 8=1252/Mechanical

Max Horz 1=274(load case 5)

Max Uplift 1=-224(load case 6), 8=-228(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-784/389, 2-3=-2129/1009, 3-4=-1674/883, 4-5=-1409/877, 5-6=-1406/881,

6-7=-1508/845, 7-8=-1874/887

BOT CHORD 1-16=0/0, 2-15=-745/1771, 14-15=-744/1772, 13-14=-377/1237, 12-13=0/113,

5-13=-374/248, 11-12=-13/115, 10-11=-587/1409, 9-10=-587/1409, 8-9=-587/1409

WEBS 3-15=0/207, 3-14=-619/437, 4-14=-172/412, 4-13=-223/392, 11-13=-327/1040,

6-13=-269/517, 6-11=-103/269, 7-11=-362/320, 7-9=0/211

Truss Design Engineer Florida PE No. 3-1865 1 109 Coastal Bay Blvd Boynton Besch, FL 33-136

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	Т09	SPECIAL	1	1		J1921287
				14 22	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:26 2007 Page 2

JOINT STRESS INDEX

1 = 0.09, 2 = 0.52, 3 = 0.32, 4 = 0.43, 5 = 0.33, 6 = 0.65, 7 = 0.42, 8 = 0.74, 9 = 0.33, 10 = 0.45, 11 = 0.46, 12 = 0.48, 13 = 0.38, 14 = 0.34 and 15 = 0.33

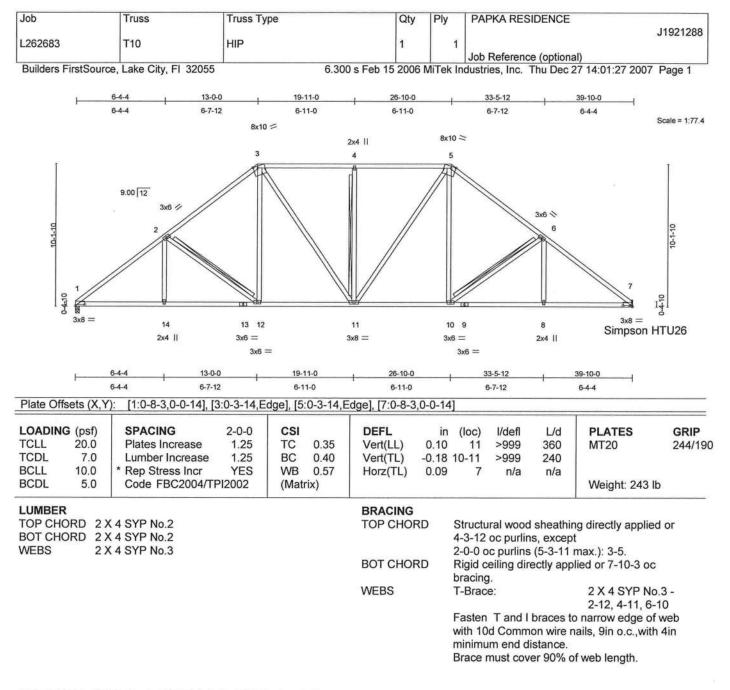
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 1 and 228 lb uplift at joint 8.
- 8) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Flonda PE No. 34869 1 109 Geestel Bay Blvd





REACTIONS (lb/size) 1=1265/0-3-8, 7=1265/Mechanical

Max Horz 1=273(load case 5)

Max Uplift 1=-229(load case 6), 7=-229(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1894/897, 2-3=-1532/848, 3-4=-1297/846, 4-5=-1297/846, 5-6=-1532/848,

6-7=-1894/897

BOT CHORD 1-14=-600/1424, 13-14=-600/1424, 12-13=-600/1424, 11-12=-345/1142,

10-11=-345/1142, 9-10=-600/1424, 8-9=-600/1424, 7-8=-600/1424

WEBS 2-14=0/211, 2-12=-354/316, 3-12=-137/343, 3-11=-228/386, 4-11=-387/246,

5-11=-228/386, 5-10=-137/343, 6-10=-354/316, 6-8=0/211

Truss Design Engineer Florida PE No. 34889 1100 Caastal Bay Blvd Boynton Besch, FL 33435

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T10	HIP	1	1		J1921288
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:27 2007 Page 2

JOINT STRESS INDEX

1 = 0.78, 2 = 0.42, 3 = 0.62, 4 = 0.33, 5 = 0.62, 6 = 0.42, 7 = 0.78, 8 = 0.33, 9 = 0.46, 10 = 0.34, 11 = 0.57, 12 = 0.34, 13 = 0.46 and 14 = 0.33

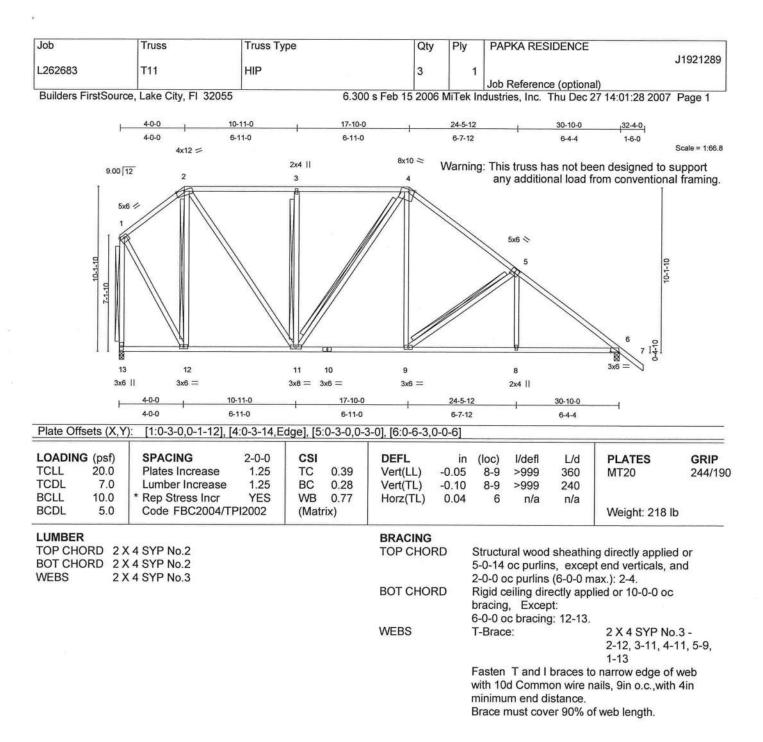
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 229 lb uplift at joint 1 and 229 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34869 1109 Coestel Bay Flyd Doyston Basel Francis





REACTIONS (lb/size) 13=975/0-3-8, 6=1069/0-3-8

Max Horz 13=-398(load case 4)

Max Uplift 13=-240(load case 4), 6=-266(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-494/408, 2-3=-717/555, 3-4=-717/555, 4-5=-1053/605, 5-6=-1409/637,

6-7=0/48, 1-13=-958/519

BOT CHORD 12-13=-151/343, 11-12=-178/354, 10-11=-115/761, 9-10=-115/761, 8-9=-318/1036,

6-8=-318/1037

2-12=-562/284, 2-11=-309/648, 3-11=-402/267, 4-11=-166/76, 4-9=-134/344,

5-9=-345/297, 5-8=0/210, 1-12=-269/714

Truss Design Engineer Florida PE No. 34869 1 100 Ceastal Bay Blvd Boynton Beach, FL 33436

Continued on page 2

WEBS



Job	Truss Type		Qty	Ply	PAPKA RESIDENCE	
L262683	T11	HIP	3	1		J1921289
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:28 2007 Page 2

JOINT STRESS INDEX

1 = 0.79, 2 = 0.92, 3 = 0.33, 4 = 0.60, 5 = 0.68, 6 = 0.76, 8 = 0.33, 9 = 0.34, 10 = 0.26, 11 = 0.65, 12 = 0.57 and 13 = 0.40

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 240 lb uplift at joint 13 and 266 lb uplift at joint 6.

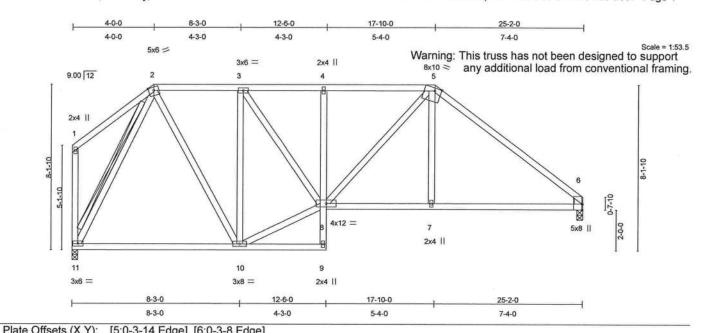
LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Flonda PE No. 24869 1109 Crastal Bay Blyd Boynton Beach, Ft. 30436



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T12	SPECIAL	7	1		J1921290
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:29 2007 Page 1



T late Of	13013 (7, 1). [5.0-5-14, Lage], [6	J.U-J-U,LU	gej								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	0.11	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.16	10-11	>999	240	NI NI STANSONI	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0			(Mat	rix)	, ,		858	(400)(441)	2-20-20-0	Weight: 171 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except*
	4-9 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3
WEDGE	
Right: 2 X 4 S	YP No.3

BRACING TOP CHORD

BOT CHORD

WEBS

P CHORD Structural wood sheathing directly applied or 5-7-12 oc purlins, except end verticals, and

2-0-0 oc purlins (6-0-0 max.): 2-5.

Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 9-10.

T-Brace:

2 X 4 SYP No.3 -

2-11

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 11=796/0-3-8, 6=796/0-3-8

Max Horz 11=-158(load case 4)

Max Uplift 11=-189(load case 5), 6=-178(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-82/87, 2-3=-570/402, 3-4=-853/515, 4-5=-857/512, 5-6=-1080/490,

1-11=-110/107

10-11=-162/337, 9-10=-18/21, 8-9=0/25, 4-8=-266/167, 7-8=-244/756, 6-7=-243/753

WEBS 2-10=-232/513, 3-10=-637/318, 8-10=-210/610, 3-8=-196/494, 5-8=-194/263,

5-7=-2/229, 2-11=-718/361

Julius Lee Truse Design Engineer Florida PE No. 24869 1199 Createl Bay Blvd Boyton Beach, FL 33435

JOINT STRESS INDEX

BOT CHORD

Contihued 45, $2a_{10} = 0.24$, 3 = 0.41, 4 = 0.33, 5 = 0.72, 6 = 0.50, 6 = 0.00, 7 = 0.33, 8 = 0.88, 9 = 0.33, 10 = 0.59 and 11 = 0.53

January 2,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T12	SPECIAL	7	1		J1921290
				16	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:29 2007 Page 2

NOTES

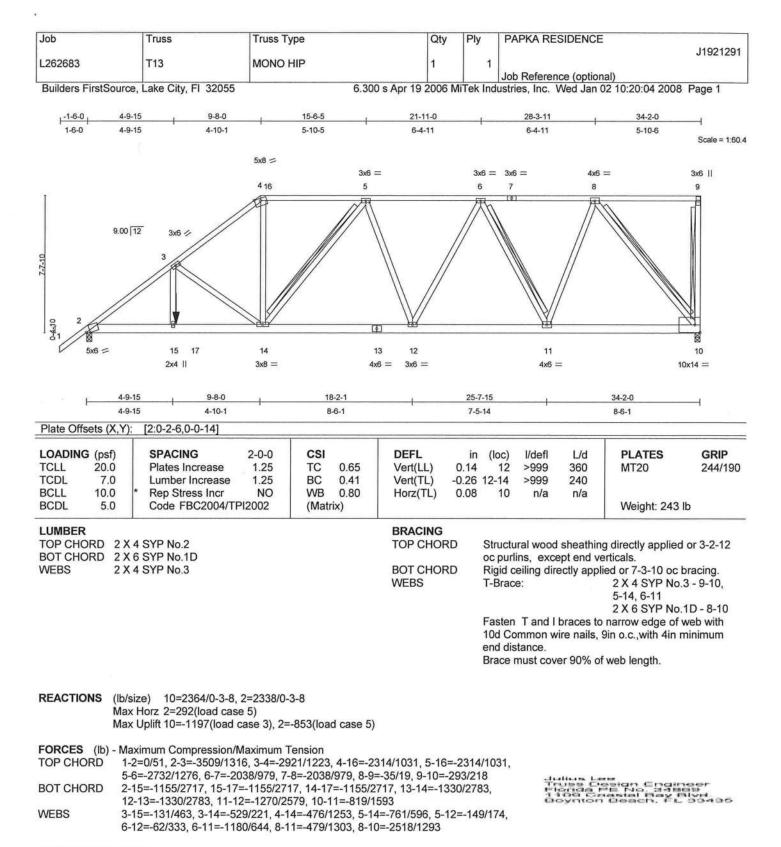
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 189 lb uplift at joint 11 and 178 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34869 1 109 Coastal Bay Blvd





JOINT STRESS INDEX

2 = 0.91, 3 = 0.44, 4 = 0.93, 5 = 0.49, 6 = 0.48, 7 = 0.44, 8 = 0.83, 9 = 0.45, 10 = 0.39, 11 = 0.83, 12 = 0.49, 13 = 0.73, 14 = 0.63 and 15 = 0.34

January 2,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T13	MONO HIP	1	1		J1921291
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 02 10:20:04 2008 Page 2

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1197 lb uplift at joint 10 and 853 lb uplift at joint 2.
- 6) Girder carries tie-in span(s): 7-0-0 from 6-0-0 to 10-0-0
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

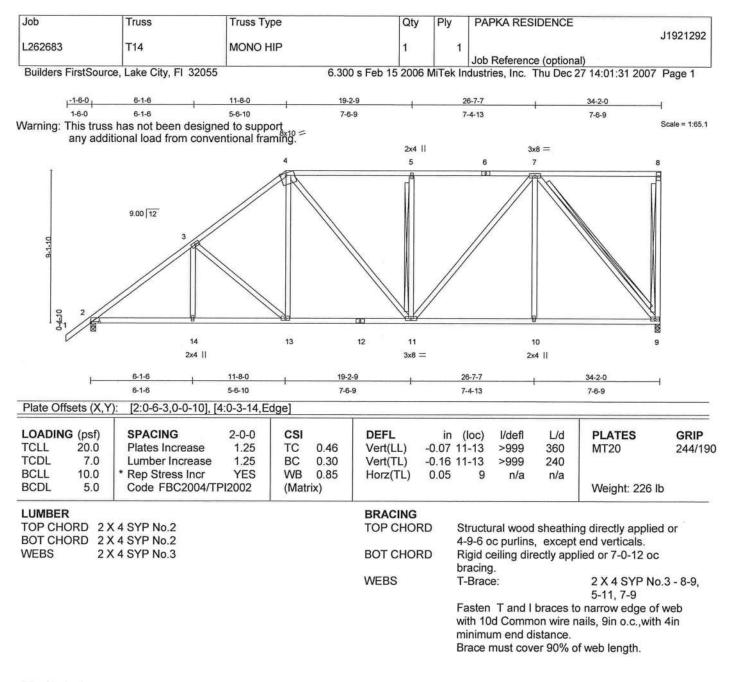
Uniform Loads (plf)

Vert: 1-4=-54, 4-16=-54, 9-16=-118(F=-64), 2-17=-10, 14-17=-85(F=-75), 10-14=-22(F=-12)

Concentrated Loads (lb) Vert: 15=-346(F)

> Julius Lee Truse Cesian Endineer Flonda PE No. 24889 1100 Ceastal Bay Rivi Boviton Beach, FL 22425





REACTIONS (lb/size) 9=1082/0-3-8, 2=1175/0-3-8

Max Horz 2=338(load case 6)

Max Uplift 9=-317(load case 4), 2=-248(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/48, 2-3=-1581/655, 3-4=-1275/641, 4-5=-1052/607, 5-6=-1052/607,

6-7=-1052/607, 7-8=-20/10, 8-9=-179/127

BOT CHORD 2-14=-788/1172, 13-14=-788/1172, 12-13=-595/956, 11-12=-595/956,

10-11=-415/743, 9-10=-415/743

WEBS 3-14=0/180, 3-13=-284/252, 4-13=-118/325, 4-11=-167/148, 5-11=-405/286,

7-11=-299/482, 7-10=0/234, 7-9=-1126/631

JOINT STRESS INDEX

NT STRESS INDEX
2 = 0.71, 3 = 0.42, 4 = 0.60, 5 = 0.33, 6 = 0.32, 7 = 0.56, 8 = 0.34, 9 = 0.40, 10 = 0.33, 11 = 0.56, 12 = 0.37, 13 = 0.34 and 14

January 2,2008 Continuedon page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.

Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, et and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T14	MONO HIP	1	1		J1921292
		morto (m			Job Reference (optional)	

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NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 3x6 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 317 lb uplift at joint 9 and 248 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 34869 1199 Ceastel Bay Blyd Boynton Beach, Ft. 93436



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T15	MONO HIP	2	1		J1921293
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:31 2007 Page 1

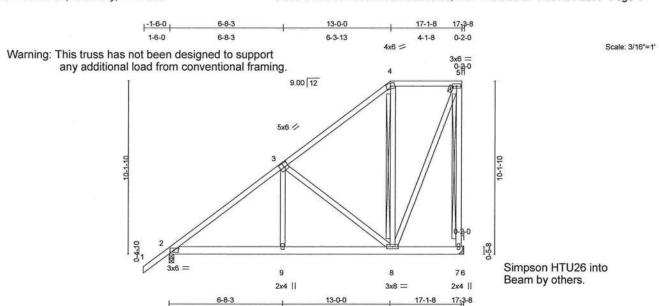


Plate Offsets (X,Y): [2:0-3-4,0-1-8], [3:0-3-0,0-3-0] LOADING (psf) SPACING 2-0-0 CSI DEFL GRIP (loc) I/defl L/d **PLATES** 20.0 TCLL 1.25 TC 0.27 Vert(LL) >999 360 Plates Increase -0.022-9 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.12 Vert(TL) -0.032-9 >999 240 **BCLL** 10.0 Rep Stress Incr 0.68 YES WB Horz(TL) 0.01 n/a n/a Code FBC2004/TPI2002 BCDL 5.0 (Matrix) Weight: 135 lb

11	M	D	_	D

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D 2 X 4 SYP No.3 WEBS

BRACING

6-3-13

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and

2-0-0 oc purlins (6-0-0 max.): 4-5.

0-2-0

4-1-8

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 - 5-7.

4-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

JOINTS

1 Brace at Jt(s): 5

REACTIONS (lb/size) 7=537/Mechanical, 2=632/0-3-8

Max Horz 2=371(load case 6)

Recommended hanger connection based on manufacturer tested capacities and nail calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication Max Uplift 7=-191(load case 6), 2=-127(load case 6) for additional information. Hanger connection to be reviewed and approved by the Architect/Engineer of Record.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/51, 2-3=-700/96, 3-4=-329/62, 4-5=-175/132, 5-7=-510/375

BOT CHORD 2-9=-382/474, 8-9=-382/474, 7-8=-3/3, 6-7=0/0

WEBS 3-9=0/226, 3-8=-376/311, 4-8=-168/190, 5-8=-358/467

JOINT STRESS INDEX

2 = 0.72, 3 = 0.63, 4 = 0.62, 5 = 0.48, 7 = 0.33, 8 = 0.63 and 9 = 0.33

Continued on page 2

January 2,2008

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	274227420
L262683	T15	MONO HIP	2	1		J1921293
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:31 2007 Page 2

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 191 lb uplift at joint 7 and 127 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida FE No. 24866 1109 Crestel Bay Blvd Boynton Beach, FL 22425



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	71.7600000000000000000000000000000000000
L262683	T16	MONO HIP	2	1		J1921294
LLULUUU	1,10	mene in			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:32 2007 Page 1

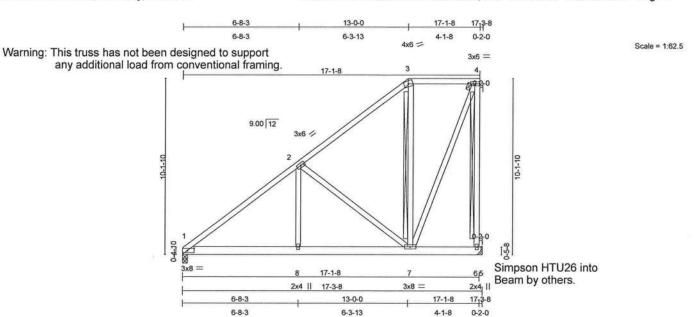


Plate Offsets (X,Y): [1:0-4-8,0-1-8] LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl L/d **PLATES** GRIP (loc) 20.0 TCLL 1.25 TC 0.27 Plates Increase Vert(LL) 0.03 1-8 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.14 Vert(TL) -0.041-8 >999 240 BCLL 10.0 Rep Stress Incr 0.69 YES WB Horz(TL) 0.01 6 n/a n/a Code FBC2004/TPI2002 BCDL 5.0 (Matrix) Weight: 132 lb

LUME	3ER
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TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals, and

2-0-0 oc purlins (6-0-0 max.): 3-4.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 - 4-6,

3-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance. Brace must cover 90% of web length.

JOINTS

1 Brace at Jt(s): 4

REACTIONS (lb/size) 1=539/0-3-8, 6=542/Mechanical

Max Horz 1=313(load case 6)

Max Uplift 1=-53(load case 6), 6=-196(load case 6)

Recommended hanger connection based on manufacturer tested capacities and nail calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Hanger connection to be reviewed and approved by the Architect/Engineer of Record.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-715/121, 2-3=-331/64, 3-4=-176/134, 4-6=-513/380

BOT CHORD 1-8=-403/487, 7-8=-403/487, 6-7=-3/3, 5-6=0/0

WEBS

2-8=-1/230, 2-7=-391/336, 3-7=-167/187, 4-7=-363/470

JOINT STRESS INDEX

1 = 0.74, 2 = 0.42, 3 = 0.61, 4 = 0.48, 6 = 0.33, 7 = 0.63 and 8 = 0.33

Continued on page 2

January 2,2008

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T16	MONO HIP	2	1		J1921294
			_	1	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:32 2007 Page 2

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 1 and 196 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34869 1109 Ceastel Bay Blvd Boynton Beach, FL 33435



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	10/20/20/20/20
L262683	T17	ROOF TRUSS	1	1		J1921295
7.5000000000000000000000000000000000000					Job Reference (optional)	

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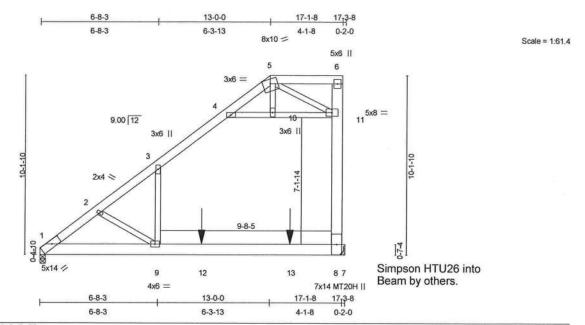


Plate Of	fsets (X,Y):	: [1:0-6-0,0-2-8]		_								
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.34	8-9	>582	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.69	8-9	>289	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.01	8	n/a	n/a		
BCDL	BCDL 5.0 Code FBC2004/		212002	(Mat	rix)	1 2012/13013 (2012)					Weight: 156	b

LUMBER

TOP CHORD 2 X 6 SYP No.1D BOT CHORD 2 X 8 SYP 2400F 2.0E **WEBS**

2 X 4 SYP No.3 *Except*

6-8 2 X 8 SYP No.1D

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-9-4

oc purlins, except end verticals, and 2-0-0 oc

purlins (10-0-0 max.): 5-6.

capacities and nail calculations. Conditions may exist that require

BOT CHORD Rigid ceiling directly applied or 7-1-7 oc bracing. Recommended hanger connection based on manufacturer tested

REACTIONS (lb/size) 1=964/0-3-8, 8=1472/Mechanical Max Horz 1=308(load case 6)

different connections than indicated. Refer to manufacturer publication for additional information. Hanger connection to be reviewed and approved by the Architect/Engineer of Record.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1383/0, 2-3=-1032/0, 3-4=-580/0, 4-5=-7/401, 5-6=-329/1067, 8-11=-469/273,

6-11=-93/62

BOT CHORD 1-9=-390/1182, 9-12=-255/590, 12-13=-255/590, 8-13=-255/590, 7-8=0/0

WFBS 3-9=0/590, 5-10=0/57, 4-10=-958/0, 10-11=-953/0, 5-11=-791/516, 2-9=-700/160

1 = 0.28, 2 = 0.34, 3 = 0.23, 4 = 0.33, 5 = 0.24, 6 = 0.35, 8 = 0.68, 9 = 0.34, 10 = 0.16 and 11 = 0.32

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) 200.0lb AC unit load placed on the bottom chord, 11-8-1 from left end, supported at two points, 5-0-0 apart.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

E) All plates are MT20 plates unless otherwise indicated.

January 2,2008

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T17	ROOF TRUSS	1	1		J1921295
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 02 10:24:48 2008 Page 2

NOTES

- 6) Ceiling dead load (5.0 psf) on member(s). 3-4, 4-10, 10-11; Wall dead load (5.0 psf) on member(s).3-9
- 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 8-9
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

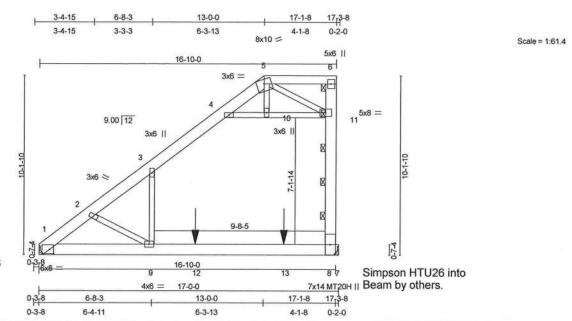
LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida PE No. 34869 1169 Coastal Bay Blvd Boynton Beach, Ft. 38436



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	Market Control
L262683	T18	ROOF TRUSS	1	1		J1921296
					Job Reference (optional)	

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Simpson HTU26

Plate Of	fsets (X,Y):	[1:0-2-12,0-3-0]										
LOADIN	IG (psf)		SPACING	2-7-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	1	Plates Increase	1.25	TC	0.85	Vert(LL)	-0.33	8-9	>592	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.64	8-9	>304	240	MT20H	187/143
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.54	Horz(TL)	0.01	8	n/a	n/a	100000000	
BCDL	5.0		Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 167 lb	1

LUMBER

REACTIONS

TOP CHORD 2 X 8 SYP 2400F 2.0E *Except*

5-6 2 X 6 SYP No.1D

BOT CHORD 2 X 8 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.3 *Except*

6-8 2 X 8 SYP 2400F 2.0E

BRACING

TOP CHORD 2-0-0

2-0-0 oc purlins (6-0-0 max.), except end verticals

(Switched from sheeted: Spacing > 2-0-0).

BOT CHORD Rigid ceiling directly applied or 5-8-6 oc bracing.

JOINTS 1 Brace at Jt(s): 6, 5

Recommended hanger connection based on manufacturer tested capacities and nail calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Hanger connection to be reviewed and approved by the Architect/Engineer of Record.

Max Horz 1=397(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-1775/0, 2-3=-1312/0, 3-4=-665/0, 4-5:

1-2=-1775/0, 2-3=-1312/0, 3-4=-665/0, 4-5=0/369, 5-6=-366/1281, 8-11=-647/373,

6-11=-108/86

BOT CHORD 1-9=-624/1482, 9-12=-318/686, 12-13=-318/686, 8-13=-318/686, 7-8=0/0

(lb/size) 1=1222/Mechanical, 8=1824/Mechanical

WEBS 3-9=0/905, 5-10=0/88, 4-10=-941/0, 10-11=-933/0, 5-11=-1156/656, 2-9=-924/348

JOINT STRESS INDEX

1 = 0.69, 2 = 0.24, 3 = 0.33, 4 = 0.33, 5 = 0.28, 6 = 0.42, 8 = 0.77, 9 = 0.49, 10 = 0.16 and 11 = 0.38

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) 200.0lb AC unit load placed on the bottom chord, 11-8-1 from left end, supported at two points, 5-0-0 apart.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live Confilmated on page 2

Truss Design Engineer Florida FE No. 34869 1109 Geastel Bay Blvd. Boynton Beach, FL 33435

January 2,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T18	ROOF TRUSS	1	1		J1921296
	10028				Job Reference (optional)	

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NOTES

5) All plates are MT20 plates unless otherwise indicated.

6) Ceiling dead load (5.0 psf) on member(s). 3-4, 4-10, 10-11; Wall dead load (5.0 psf) on member(s).3-9

7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 8-9

8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

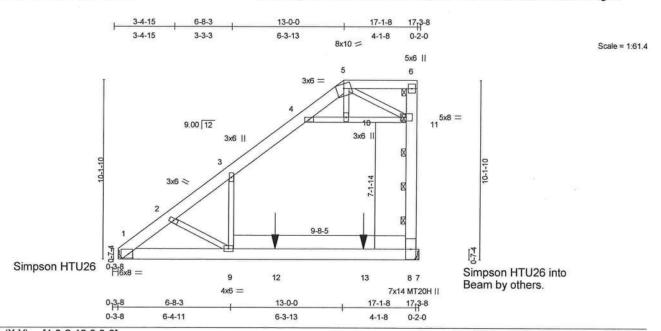
LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida FE No. 34868 1109 Coestal Bay Blvd Boynton Beach, FL 33435



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	2000-2000-2000
L262683	T19	ROOF TRUSS	1	1		J1921297
					Job Reference (optional)	

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LOADIN	G (psf)		SPACING	2-7-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.85	Vert(LL)	-0.33	8-9	>592	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.64	8-9	>304	240	MT20H	187/143
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.54	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	1	Code FBC2004/TF	PI2002	(Mati	rix)					0000000	Weight: 167 I	b

LUMBER

REACTIONS

TOP CHORD 2 X 8 SYP 2400F 2.0E *Except*

5-6 2 X 6 SYP No.1D

BOT CHORD 2 X 8 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.3 *Except*

6-8 2 X 8 SYP 2400F 2.0E

(lb/size) 1=1222/Mechanical, 8=1824/Mechanical

BRACING

TOP CHORD 2-0-0 oc purlins (6-0-0 max.), except end verticals

(Switched from sheeted: Spacing > 2-0-0).

BOT CHORD Rigid ceiling directly applied or 5-8-6 oc bracing.

JOINTS 1 Brace at Jt(s): 6, 5

Recommended hanger connection based on manufacturer tested capacities and nail calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Hanger connection to be reviewed and approved by the Architect/Engineer of Record.

FORCES (lb) - Maximum Compression/Maximum Tension

Max Horz 1=397(load case 6)

TOP CHORD 1-2=-1775/0, 2-3=-1312/0, 3-4=-665/0, 4-5=0/369, 5-6=-366/1281, 8-11=-647/373,

6-11=-108/86

BOT CHORD 1-9=-624/1482, 9-12=-318/686, 12-13=-318/686, 8-13=-318/686, 7-8=0/0

WEBS 3-9=0/905, 5-10=0/88, 4-10=-941/0, 10-11=-933/0, 5-11=-1156/656, 2-9=-924/348

JOINT STRESS INDEX

1 = 0.69, 2 = 0.24, 3 = 0.33, 4 = 0.33, 5 = 0.28, 6 = 0.42, 8 = 0.77, 9 = 0.49, 10 = 0.16 and 11 = 0.38

NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

20.0lb AC unit load placed on the bottom chord, 11-8-1 from left end, supported at two points, 5-0-0 apart.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live Confirmed on page 2

Truse Design Engineer Flonds PE No. 34868 1100 Coastal Bay Blvd Boynton Beach, Ft. 33435

January 2,2008

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	Contractive Contra
L262683	T19	ROOF TRUSS	1	1		J1921297
	1 0.000				Job Reference (optional)	

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NOTES

5) All plates are MT20 plates unless otherwise indicated.

6) Ceiling dead load (5.0 psf) on member(s). 3-4, 4-10, 10-11; Wall dead load (5.0 psf) on member(s).3-9

7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 8-9

8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

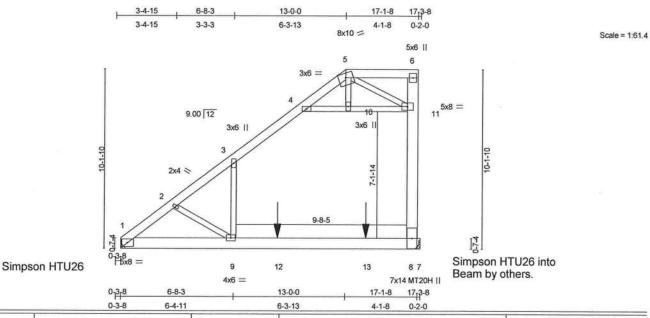
LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida FE No. 34988 1 109 Coastel Bay Blyri



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T19A	ROOF TRUSS	1	1		J1921298
					Job Reference (optional)	

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LOADIN	11		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.91	Vert(LL)	-0.34	8-9	>578	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.99	Vert(TL)	-0.69	8-9	>287	240	MT20H	187/143
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.61	Horz(TL)	0.01	8	n/a	n/a	ACCUSTOMENTS IN	
BCDL	5.0		Code FBC2004/TI	PI2002	(Mati	rix)						Weight: 156 lb	

LUMBER

REACTIONS

BOT CHORD

TOP CHORD 2 X 6 SYP No.1D BOT CHORD 2 X 8 SYP No.1D

WEBS 2 X 4 SYP No.3 *Except*

6-8 2 X 8 SYP No.1D

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-9-11

oc purlins, except end verticals, and 2-0-0 oc

purlins (10-0-0 max.): 5-6.

BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

Recommended hanger connection based on manufacturer tested capacities and nail calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Hanger connection to be reviewed and approved by the Architect/Engineer of Record.

Max Horz 1=308(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

(lb/size) 1=962/Mechanical, 8=1455/Mechanical

TOP CHORD 1-2=-1335/0, 2-3=-1022/0, 3-4=-574/0, 4-5=-12/397, 5-6=-341/1064, 8-11=-470/278,

6-11=-93/62

1-9=-416/1118, 9-12=-257/583, 12-13=-257/583, 8-13=-257/583, 7-8=0/0

WEBS 3-9=0/589, 5-10=0/57, 4-10=-946/0, 10-11=-941/0, 5-11=-793/525, 2-9=-632/188

JOINT STRESS INDEX

1 = 0.58, 2 = 0.34, 3 = 0.23, 4 = 0.33, 5 = 0.24, 6 = 0.35, 8 = 0.68, 9 = 0.34, 10 = 0.16 and 11 = 0.32

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 200.0lb AC unit load placed on the bottom chord, 11-8-1 from left end, supported at two points, 5-0-0
 apart.
- Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) Ceiling dead load (5.0 psf) on member(s). 3-4, 4-10, 10-11; Wall dead load (5.0psf) on member(s).3-9 Continued on page 2

Truse Design Engineer Florida PE No. 34868 1 109 Coastel Bay Blyd Boynton Beach, FL 33435

January 2,2008

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
L262683	T19A	ROOF TRUSS	1	1		J1921298
	E I	U.S. Problems grant - Trins 200 attraction and a			Job Reference (optional)	

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NOTES

7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 8-9

8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

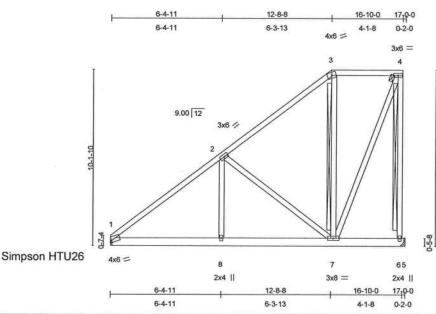
LOAD CASE(S) Standard

Julius Law Truse Design Engineer Florida PE No. 34868 1 100 Coastal Bay Slvd





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Simpson HTU26 into Beam by others.

Plate Offsets	(X.Y):	[1:0-1-1,	0-1-11

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	0.03	1-8	>999	360	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	-0.04	1-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	, ,					Weight: 131 lb	

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	ш	vı	н		ĸ

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D **WEBS** 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals, and

2-0-0 oc purlins (6-0-0 max.): 3-4.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 - 4-6,

Scale = 1:62.5

3-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

JOINTS

1 Brace at Jt(s): 4

REACTIONS (lb/size) 1=532/Mechanical, 6=535/Mechanical

Max Horz 1=313(load case 6)

Max Uplift 1=-50(load case 6), 6=-196(load case 6)

Recommended hanger connection based on manufacturer tested capacities and nail calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Hanger connection to be reviewed and approved by the Architect/Engineer of Record.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-700/117, 2-3=-327/64, 3-4=-174/134, 4-6=-507/381

BOT CHORD

1-8=-400/471, 7-8=-400/471, 6-7=-3/3, 5-6=0/0

WEBS

2-8=-1/225, 2-7=-373/332, 3-7=-170/190, 4-7=-364/464

JOINT STRESS INDEX

1 = 0.79, 2 = 0.42, 3 = 0.62, 4 = 0.48, 6 = 0.33, 7 = 0.63 and 8 = 0.33

Continued on page 2

January 2,2008



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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	2014 PA 0000 - CO - 100 - CO - CO
L262683	T20	MONO HIP	3	1		J1921299
LEGEGGG	120	IMO, TO THE			Job Reference (optional)	

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NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 1 and 196 lb uplift at joint 6.

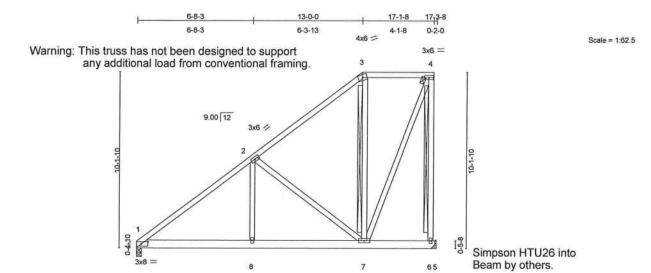
LOAD CASE(S) Standard

Julius Lee Truss Clesian Engineer Flonda FE No. 34868 1109 Coestal Bay Blvd Boynton Besch, FL 33435



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T21	MONO HIP	4	1		J1921300
				1	Job Reference (optional)	

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[1:0-4-8,0-1-8] Plate Offsets (X,Y): LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl GRIP L/d **PLATES** in (loc) TCLL 20.0 Plates Increase 1.25 TC 0.27 Vert(LL) >999 360 0.03 1-8 MT20 244/190 TCDL 7.0 1.25 BC Lumber Increase 0.14 Vert(TL) -0.041-8 >999 240 **BCLL** 10.0 Rep Stress Incr WB 0.69 YES Horz(TL) 0.01 6 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 132 lb

6-3-13

2x4 ||

6-8-3

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D

2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or

2x4 II

17,3-8

0-2-0

6-0-0 oc purlins, except end verticals, and

2-0-0 oc purlins (6-0-0 max.): 3-4.

BOT CHORD

3x8 =

17-1-8

4-1-8

Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS T-Brace: 2 X 4 SYP No.3 - 4-6,

3-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

JOINTS

1 Brace at Jt(s): 4

REACTIONS (lb/size) 1=539/0-3-8, 6=542/Mechanical

Max Horz 1=313(load case 6)

Max Uplift 1=-53(load case 6), 6=-196(load case 6)

Recommended hanger connection based on manufacturer tested capacities and nail calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Hanger connection to be reviewed and approved by the Architect/Engineer of Record.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-715/121, 2-3=-331/64, 3-4=-176/134, 4-6=-513/380

BOT CHORD 1-8=-403/487, 7-8=-403/487, 6-7=-3/3, 5-6=0/0

WEBS 2-8=-1/230, 2-7=-391/336, 3-7=-167/187, 4-7=-363/470

JOINT STRESS INDEX

1 = 0.74, 2 = 0.42, 3 = 0.61, 4 = 0.48, 6 = 0.33, 7 = 0.63 and 8 = 0.33

Continued on page 2

January 2,2008

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T21	MONO HIP	4	1		J1921300
	100000			25	Job Reference (optional)	

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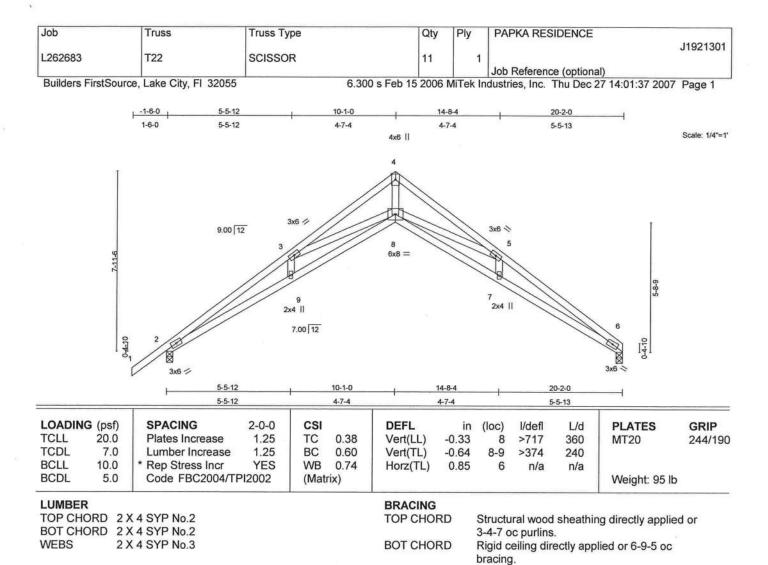
NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 1 and 196 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Florida PE No. 34898 I 100 Cessial Bay Blyd Boynton Beach, Ft, 33435





REACTIONS (lb/size) 2=729/0-3-8, 6=632/0-3-8

Max Horz 2=232(load case 5)

Max Uplift 2=-203(load case 6), 6=-128(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-2828/947, 3-4=-2217/471, 4-5=-2217/473, 5-6=-2879/1026

BOT CHORD 2-9=-741/2484, 8-9=-748/2499, 7-8=-825/2549, 6-7=-825/2539

WEBS 3-9=0/135, 3-8=-557/556, 4-8=-390/2303, 5-8=-603/627, 5-7=0/138

JOINT STRESS INDEX

2 = 0.82, 3 = 0.42, 4 = 0.55, 5 = 0.42, 6 = 0.82, 7 = 0.33, 8 = 0.92 and 9 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. Continued on page 2

January 2,2008

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T22	SCISSOR	11	1		J1921301
					Job Reference (optional)	

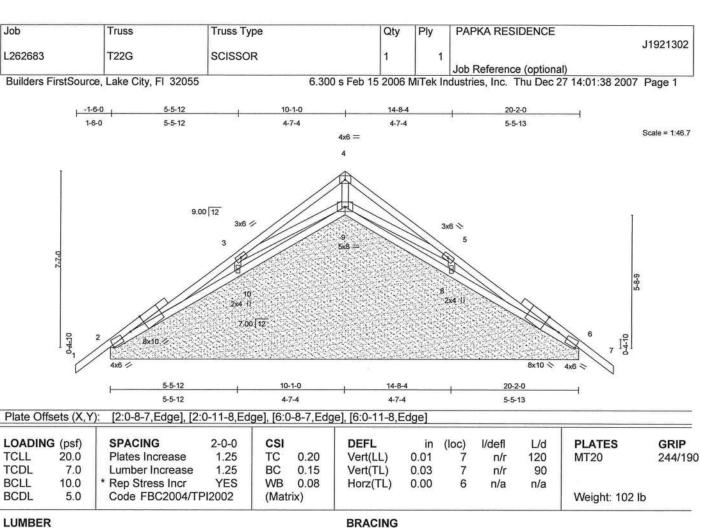
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:37 2007 Page 2

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 203 lb uplift at joint 2 and 128 lb uplift at joint 6.

LOAD CASE(S) Standard





TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 **WEBS**

TOP CHORD

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

Structural wood sheathing directly applied or

REACTIONS (lb/size) 2=237/20-2-0, 6=237/20-2-0, 10=364/20-2-0, 9=250/20-2-0, 8=364/20-2-0

Max Horz 2=262(load case 5)

Max Uplift 2=-210(load case 7), 6=-257(load case 7), 10=-280(load case 6),

9=-37(load case 5), 8=-267(load case 7)

Max Grav 2=237(load case 1), 6=237(load case 1), 10=371(load case 10), 9=282(load case 7), 8=371(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/42, 2-3=-157/201, 3-4=-95/139, 4-5=-95/126, 5-6=-88/87, 6-7=0/42

BOT CHORD 2-10=-86/154, 9-10=-83/148, 8-9=-37/119, 6-8=-34/115

WEBS 3-10=-303/300, 3-9=-1/142, 4-9=-208/32, 5-9=-35/161, 5-8=-303/288

JOINT STRESS INDEX

2 = 0.14, 2 = 0.13, 3 = 0.42, 4 = 0.31, 5 = 0.42, 6 = 0.14, 6 = 0.13, 8 = 0.33, 9 = 0.53 and 10 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions

Contraction page 2

Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	14004000
L262683	T22G	SCISSOR	1	1		J1921302
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:38 2007 Page 2

NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Gable requires continuous bottom chord bearing.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 210 lb uplift at joint 2, 257 lb uplift at joint 6, 280 lb uplift at joint 10, 37 lb uplift at joint 9 and 267 lb uplift at joint 8.
- 7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 6, 10, 9, 8.
- 8) Truss designed for wind loads in plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard

Julius Les Truse Design Engineer Florida PE, No. 34869 1189 Coastel Bay Blvd Boydon Deser Et 30436



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T23	COMMON	1			J1921303
	,	Joanna.		2	Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 02 10:33:23 2008 Page 1

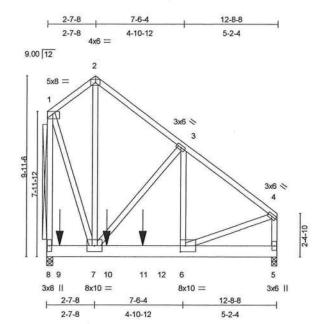


Plate Of	fsets (X,Y): [1:0-3-8,0-1-13], [6:	0-3-8,0-4-0]								
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.00	TC	0.65	Vert(LL)	-0.03	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.00	BC	0.21	Vert(TL)	-0.06	6-7	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.25	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/T	PI2002	(Mat	rix)	, ,					Weight: 255 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 8 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.2 *Except*

1-8 2 X 4 SYP No.3, 4-5 2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

T-Brace: 2 X 4 SYP No.3 - 1-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum

end distance. Brace must cover 90% of web length.

REACTIONS

(lb/size) 8=3408/0-4-0, 5=2252/0-3-8

Max Horz 8=-201(load case 6)

Max Uplift 8=-1007(load case 6), 5=-555(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-882/277, 2-3=-937/267, 3-4=-1897/466, 1-8=-2619/792, 4-5=-1753/423

BOT CHORD 8-9=-62/199, 7-9=-62/199, 7-10=-325/1452, 10-11=-325/1452, 11-12=-325/1452,

6-12=-325/1452, 5-6=-62/130

WEBS 2-7=-242/854, 3-7=-1193/458, 3-6=-352/1257, 1-7=-660/2167, 4-6=-304/1424

JOINT STRESS INDEX

1 = 0.41, 2 = 0.30, 3 = 0.61, 4 = 0.72, 5 = 0.33, 6 = 0.16, 7 = 0.42 and 8 = 0.21

NOTES

 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc. Truse Design Engineer Florida PE No. 34899 1100 Chastel Bay Blvd Boynton Beach, FL 35435

January 2,2008

Scale = 1:59.8

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the
responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection
and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center,
6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T23	COMMON	1			J1921303
				2	Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 02 10:33:23 2008 Page 2

NOTES

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1007 lb uplift at joint 8 and 555 lb uplift at joint 5.
- 8) Girder carries tie-in span(s): 16-10-0 from 6-3-8 to 12-8-8

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 1-2=-54, 2-4=-54, 8-12=-10, 5-12=-243(F=-233)

Concentrated Loads (lb)

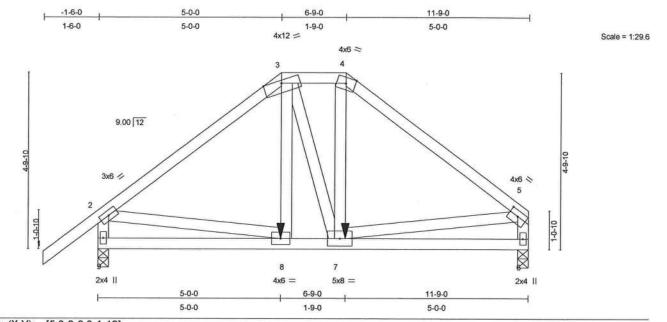
Vert: 9=-1222(F) 10=-1222(F) 11=-962(F)

Julius Lee Truse Cesign Engineer Florida FE No. 34869 1109 Ceastel Bay Blvd Boynton Beach, Ft. 33435



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T24	HIP	1	1		J1921304
					Job Reference (optional)	

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.02	8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	-0.02	8-9	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.12	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 77 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 9=686/0-3-8, 6=584/0-3-8

Max Horz 9=158(load case 4)

Max Uplift 9=-443(load case 5), 6=-369(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/53, 2-3=-684/420, 3-4=-488/381, 4-5=-688/417, 2-9=-654/391, 5-6=-553/317

BOT CHORD

8-9=-253/106, 7-8=-371/483, 6-7=-142/173

WEBS

3-8=-176/160, 3-7=-111/96, 4-7=-248/233, 2-8=-271/377, 5-7=-277/335

JOINT STRESS INDEX

2 = 0.52, 3 = 0.44, 4 = 0.29, 5 = 0.75, 6 = 0.68, 7 = 0.15, 8 = 0.16 and 9 = 0.68

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 443 lb uplift at Coinintied and 369eb uplift at joint 6.

January 2,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T24	HIP	1	1	*	J1921304
					Job Reference (optional)	

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NOTES

7) Girder carries hip end with 5-0-0 end setback.

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

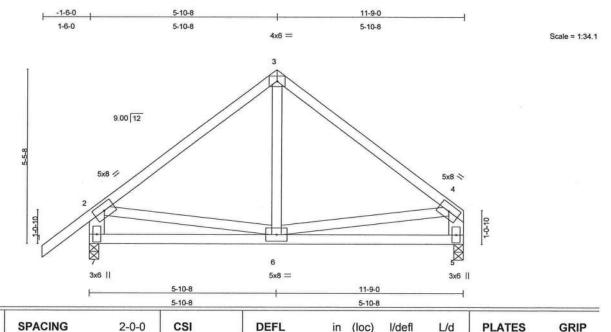
Vert: 1-2=-54, 2-3=-54, 3-4=-91(F=-37), 4-5=-54, 8-9=-10, 7-8=-17(F=-7), 6-7=-10

Concentrated Loads (lb)

Vert: 8=-187(F) 7=-187(F)



		Truss Type	Qty	Ply	PAPKA RESIDENCE	
					J192	21305
L262683	T25	COMMON	2	1		
					Job Reference (optional)	



LUMBER

TCLL

TCDL

BCLL

BCDL

LOADING (psf)

20.0

7.0

5.0

10.0

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 *Except*

2-7 2 X 6 SYP No.1D, 4-5 2 X 6 SYP No.1D

Plates Increase

* Rep Stress Incr

Lumber Increase

Code FBC2004/TPI2002

BRACING

BOT CHORD

Vert(LL)

Vert(TL)

Horz(TL)

TOP CHORD

0.04

-0.02

-0.00

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

360

240

n/a

MT20

Weight: 69 lb

244/190

Rigid ceiling directly applied or 10-0-0 oc

bracing.

5-6

5-6

5

>999

>999

n/a

REACTIONS (lb/size) 7=462/0-3-8, 5=354/0-3-8

Max Horz 7=178(load case 5)

Max Uplift 7=-289(load case 6), 5=-211(load case 7)

1.25

1.25

YES

TC

BC

WB

(Matrix)

0.24

0.16

0.15

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/56, 2-3=-371/465, 3-4=-370/464, 2-7=-433/479, 4-5=-325/362

BOT CHORD 6-7=-304/139, 5-6=-301/204

WEBS 3-6=-324/156, 2-6=-142/150, 4-6=-140/169

JOINT STRESS INDEX

2 = 0.72, 3 = 0.55, 4 = 0.72, 5 = 0.42, 6 = 0.07 and 7 = 0.42

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Truse Design Engineer Florida FE No. 34869 1109 Coastal Bay Blvd Boynton Beach, FL 33436

January 2,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T25	COMMON	2	1		J1921305
				1	Job Reference (optional)	

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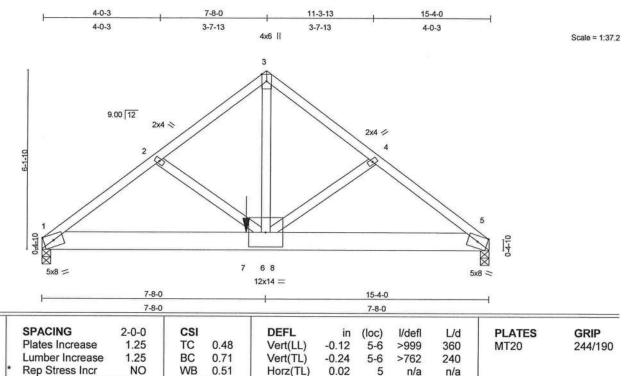
NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 289 lb uplift at joint 7 and 211 lb uplift at joint 5.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE
L262683	TOC	COMMON			J192130
L202003	T26	COMMON	1	2	Job Reference (optional)
Builders FirstSo	ource, Lake City, FI 3	2055 6.3	300 s Apr 19 2006 N		lustries, Inc. Wed Jan 02 10:34:55 2008 Page 1



LUMBER

TCLL

TCDL

BCLL

BCDL

LOADING (psf)

TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** 2 X 8 SYP No.1D

WEBS 2 X 4 SYP No.2

20.0

10.0

5.0

7.0

BRACING

(Matrix)

TOP CHORD

Structural wood sheathing directly applied or 5-3-13

Weight: 194 lb

n/a

oc purlins.

n/a

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=3016/0-3-8, 5=5026/0-3-8

Max Horz 1=-157(load case 3)

Code FBC2004/TPI2002

Max Uplift 1=-818(load case 5), 5=-1415(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4969/1372, 2-3=-4805/1375, 3-4=-4837/1380, 4-5=-5037/1392

BOT CHORD 1-7=-1104/3903, 6-7=-1104/3903, 6-8=-1092/4039, 5-8=-1092/4039

WEBS 2-6=-117/116, 3-6=-1561/5505, 4-6=-287/167

JOINT STRESS INDEX

1 = 0.50, 2 = 0.06, 3 = 0.68, 4 = 0.06, 5 = 0.50 and 6 = 0.37

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

All loads are considered equally applied to all plies, except if noted as front (F) or back (B) for LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or LOAD CASE(S) section. 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

January 2,2008

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T26	COMMON	1			J1921306
				2	Job Reference (optional)	- 1

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 02 10:34:55 2008 Page 2

NOTES

5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 818 lb uplift at joint 1 and 1415 lb uplift at joint 5.
- 8) Girder carries tie-in span(s): 39-10-0 from 8-0-0 to 15-4-0

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 1-8=-10, 5-8=-611(F=-601)

Concentrated Loads (lb) Vert: 7=-2762(F)

> Julius Les Truss Cesign Engineer Flonda PE No. 24869 1109 Coastel Bay Blyd Baydon Dassey



Job	Truss	Truss Type		Qty	Ply	PAP	KA RESI	DENCE		
_262683	T26G	GABLE		1		1				J1921307
D 11 - F. 10	Laborate St. Co.		2 2 2 2 2					(optional		
Builders FirstSource	, Lake City, FI 32055		6.300 s	Feb 15 2006	MiTek	Industrie	s, Inc. T	hu Dec 2	7 14:01:42 2007	Page 1
<u> </u>	6-0	7-8-0				15-4-0			16-10-0	
1-	6-0	7-8-0				7-8-0			1-6-0	Scale = 1:36
				4x6 = 6						Action Mark
04-10 04-10	3x6 × 3x6 × 3	00 12 5			7	B	8	3x6 \\ 3x6 \\ 9	10 0	
1/	4x6 =	16	1	14 5-4-0 5-4-0	13	12		4	x6 =	
COADING (psf) FCLL 20.0 FCDL 7.0 BCLL 10.0 BCDL 5.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/Ti	2-0-0 CS 1.25 TC 1.25 BC YES WE PI2002 (Ma	0.15 0.05	Vert(LL) Vert(TL) Horz(TL)	in -0.00 -0.00 0.00	(loc) 11 11 10	I/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20 Weight: 87 lb	GRIP 244/19
BOT CHORD 2 X	4 SYP No.2 4 SYP No.2 4 SYP No.3			BRACING TOP CHOP BOT CHOP) I ds	6-0-0 oc	purlins.		g directly applie	d or
Max	16=199/15-4-(Horz 2=-195(load c Uplift 2=-103(load c 16=-138(load Grav 2=195(load ca	ase 6), 10=-124(case 6), 13=-10 se 10), 10=195(ase 10), 16=199	0, 12=199/15 (load case 7) 1(load case 7 load case 11	-4-0 , 15=-105(lo '), 12=-143(lo), 14=153(lo	ad case load case ad case	e 6), se 7) e 1),				
San Bridge De Bridge Bridge	ximum Compression	7, 3-4=-130/122,	4-5=-63/106	i, 5-6=-24/14	43, 6-7=					
7-8 8OT CHORD 2-1 10	2=0/47, 2-3=-141/10. 3=-32/71, 8-9=-68/68 16=-23/182, 15-16=-; -12=-23/182 14=-129/0, 5-15=-96/	23/182, 14-15=-2	23/182, 13-14	N		0.00.00.00.00	-11- 11-11-11-11-11-11-11-11-11-11-11-11	dius La USS Co Crida F	rm esian Endi es No. 246 estal Bay i Geach, Fi	165

12 = 0.09, 13 = 0.06, 14 = 0.05, 15 = 0.06 and 16 = 0.09

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T26G	GABLE	1	1		J1921307
		J, 1522		·	Job Reference (optional)	

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NOTES

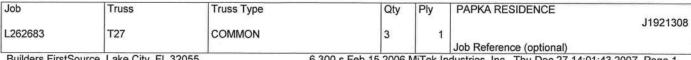
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 103 lb uplift at joint 2, 124 lb uplift at joint 10, 105 lb uplift at joint 15, 138 lb uplift at joint 16, 101 lb uplift at joint 13 and 143 lb uplift at joint 12.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 24869 1100 Ceastel Bay Blvd Dovoton Beach, Ft. 22425





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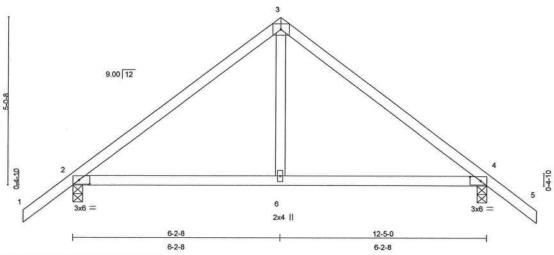


Plate Offsets	(X Y).	[2:0-3-13,0-1-8], [4:0-3-13,0-1-8]
i idio officio		[2.0 0 10,0 1 0], [1.0 0 10,0 1 0]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.03	4-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.05	4-6	>999	240	Control of the	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	4	n/a	n/a		
BCDL 5.0 Code FBC2004/TPI2002		(Mat	rix)						Weight: 54 lb			

1	ı	n	n	F	D
		n			

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=477/0-3-8, 4=477/0-3-8

Max Horz 2=129(load case 5)

Max Uplift 2=-154(load case 6), 4=-154(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/48, 2-3=-456/190, 3-4=-456/190, 4-5=0/48

BOT CHORD 2-6=-8/287, 4-6=-8/287

WEBS 3-6=0/212

JOINT STRESS INDEX

2 = 0.60, 3 = 0.66, 4 = 0.60 and 6 = 0.15

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 2,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T27	COMMON	2			J1921308
L202003	121	COMMON	3	1	Job Reference (optional)	

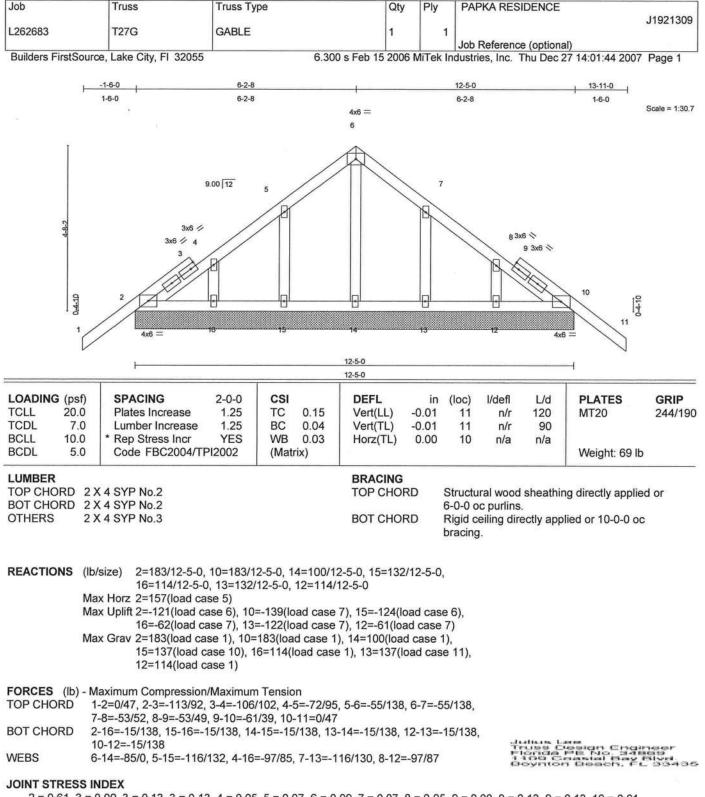
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:43 2007 Page 2

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 154 lb uplift at joint 2 and 154 lb uplift at joint 4.

LOAD CASE(S) Standard





2 = 0.61, 3 = 0.00, 3 = 0.13, 3 = 0.13, 4 = 0.05, 5 = 0.07, 6 = 0.09, 7 = 0.07, 8 = 0.05, 9 = 0.00, 9 = 0.13, 9 = 0.13, 10 = 0.61, 12 = 0.05, 13 = 0.07, 14 = 0.03, 15 = 0.07 and 16 = 0.05

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	50-27 MG 50 CO C (00-20-20-20)
L262683	T27G	GABLE	1	1		J1921309
	on the second	2			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:44 2007 Page 2

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 121 lb uplift at joint 2, 139 lb uplift at joint 10, 124 lb uplift at joint 15, 62 lb uplift at joint 16, 122 lb uplift at joint 13 and 61 lb uplift at joint 12.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida PE No. 24869 1109 Coastal Bay Blyd Boynton Beach, FL 23435



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	11001010
L262683	T28	COMMON	1	1		J1921310
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:45 2007 Page 1

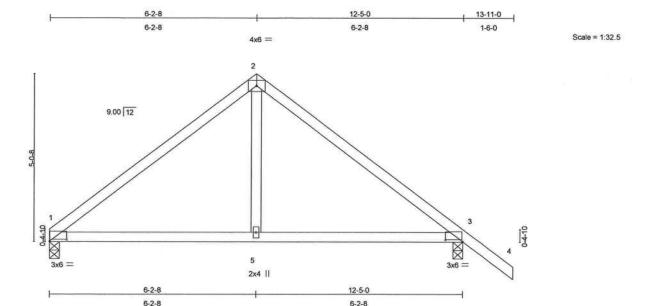


Plate Offsets (X,Y): [1:0-6-3,0-0-10], [3:0-6-3,0-0-10]

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.04	1-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.22	Vert(TL)	-0.07	1-5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	V 0.388383700 3 000070 5 2					Weight: 51 lb	

1	ı	Iħ	A	R	F	D

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 1=382/0-3-8, 3=483/0-3-8

Max Horz 1=-151(load case 4)

Max Uplift 1=-77(load case 6), 3=-155(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-467/207, 2-3=-470/213, 3-4=0/48

BOT CHORD WEBS 1-5=-22/299, 3-5=-22/299 2-5=0/215

JOINT STRESS INDEX

Continued on page 2

1 = 0.68, 2 = 0.70, 3 = 0.68 and 5 = 0.15

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Truse Design Engineer Florida PE No. 24889 1 100 Coastal Bay Blvd Boynton Beach, FL 33435

January 2,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T28	COMMON	1	1		J1921310
				ļ .	Job Reference (optional)	

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5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 77 lb uplift at joint 1 and 155 lb uplift at joint 3.

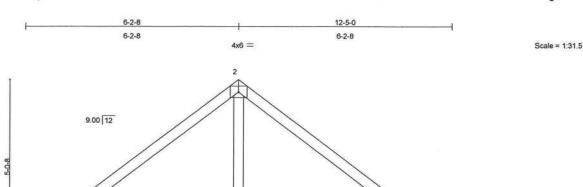
LOAD CASE(S) Standard



Job PAPKA RESIDENCE Truss Truss Type Qty Ply J1921311 L262683 T29 COMMON 2 1 Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:45 2007 Page 1



4 2x4 || 6-2-8 12-5-0 6-2-8 6-2-8

Plate Offsets	(X,Y):	[1:0-6-3,0-0-10],	[3:0-6-3,0-0-10]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.04	3-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.22	Vert(TL)	-0.07	3-4	>999	240	100000000000000000000000000000000000000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	SWINSTON MANAGEMENT					Weight: 49 lb	

LUMBE	3

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 1=388/0-3-8, 3=388/0-3-8

Max Horz 1=131(load case 5)

Max Uplift 1=-79(load case 6), 3=-79(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-481/230, 2-3=-481/230

BOT CHORD

1-4=-71/310, 3-4=-71/310

WEBS

2-4=-13/217

JOINT STRESS INDEX

1 = 0.68, 2 = 0.74, 3 = 0.68 and 4 = 0.15

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 2,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T29	COMMON	2	1		J1921311
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:45 2007 Page 2

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 1 and 79 lb uplift at joint 3.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	14004040
L262683	T30	COMMON	2	1		J1921312
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:46 2007 Page 1

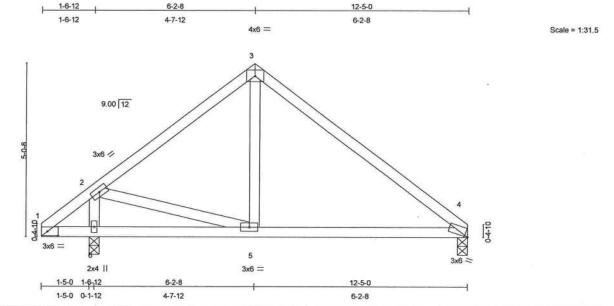


Plate Offsets (X,Y): [1:0-3-13,0-1-8], [4:0-1-1,0-0-2]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.05	4-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.07	4-5	>999	240) Dens (USWE)	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)	, ,					Weight: 57 lb	

	L	U	٨	/	B	E	R
--	---	---	---	---	---	---	---

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

REACTIONS (lb/size) 4=335/0-3-8, 6=450/0-3-8

Max Horz 6=131(load case 5)

Max Uplift 4=-70(load case 7), 6=-92(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-24/68, 2-3=-347/196, 3-4=-382/178

BOT CHORD WEBS 1-6=-35/26, 5-6=-150/157, 4-5=-25/229 2-6=-429/261, 2-5=-77/253, 3-5=0/144

JOINT STRESS INDEX

Continued on page 2

1 = 0.12, 2 = 0.16, 3 = 0.60, 4 = 0.83, 5 = 0.14 and 6 = 0.15

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee Truse Design Engineer Florida PE No. 24868 1100 Coastel Bay Blvd Boydon Beach E. 23434

January 2,2008

** Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T30	COMMON	2	1		J1921312
			-	1	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:46 2007 Page 2

NOTES

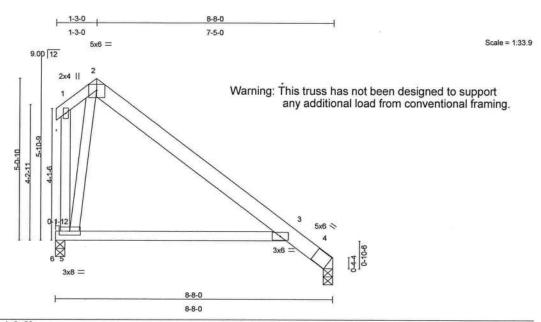
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 4 and 92 lb uplift at joint 6.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T31	SPECIAL	6	1		J1921313
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:47 2007 Page 1



	1135 W EO											
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	0.07	3-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.13	3-5	>771	240	10.4.79-0-403-403	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.04	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)					0.70.00	Weight: 48 lb	

BRACING

LL	INA	D	=	D
L	MAI	ь	_	$\boldsymbol{\Gamma}$

TOP CHORD 2 X 4 SYP No.2 *Except*

2-4 2 X 6 SYP No.1D

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

TOP CHORD

CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=266/0-3-8, 5=266/0-3-8

Max Horz 5=-163(load case 7)

Max Uplift 4=-37(load case 7), 5=-94(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-80/93, 2-3=-152/31, 3-4=-139/63, 1-5=-144/156

BOT CHORD 5-6=0/0, 3-5=-3/166

WEBS 2-5=-357/345

JOINT STRESS INDEX

1 = 0.46, 2 = 0.80, 2 = 0.00, 3 = 0.50 and 5 = 0.80

NOTES

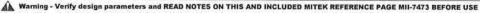
1) Unbalanced roof live loads have been considered for this design.

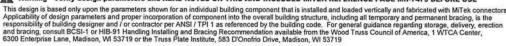
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Chambearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

Truse Cesian Endineer Florida PE No. 24869 1199 Ceastal Bay Blvd. Boymon Beach, FL 33425







Job	Truss	Truss Type	Qty	Ply	PAPKA RESIDENCE	
L262683	T31	SPECIAL	6	1		J1921313
				105	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Dec 27 14:01:47 2007 Page 2

NOTES

- 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 4 and 94 lb uplift at joint 5.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34868 I 109 Coestel Bey Blvd

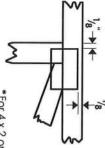


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless plates to both sides of truss and Dimensions are in inches. Apply dimensions indicate otherwise securely seat.



*For 4 x 2 orientation, locate of truss and vertical web. plates 1/8" from outside edge



*This symbol indicates the required direction of slots in connector plates

PLATE SIZE

4 × 4

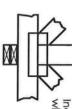
The first dimension is the width perpendicular to slots. Second to slots. dimension is the length parallel

LATERAL BRACING



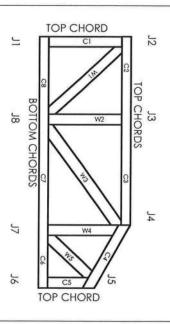
continuous lateral bracing. Indicates location of required

BEARING



which bearings (supports) occur. Indicates location of joints at

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

ICBO 3907, 4922

BOCA

96-31, 96-67

SBCCI 9667, 9432A

NER

561

WISC/DILHR

960022-W, 970036-N



MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 12 Cut members to bear tightly against each other.
- ω Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- 4 Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

5

- 6 Unless expressly noted, this design is not preservative treated lumber applicable for use with fire retardant or
- 7 Camber is a non-structural consideration and practice is to camber for dead load deflection is the responsibility of truss fabricator. General
- 00 Plate type, size and location dimensions shown indicate minimum plating requirements
- 9 Lumber shall be of the species and size, and grade specified. in all respects, equal to or better than the
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- 12. Anchorage and / or load transferring others unless shown. connections to trusses are the responsibility of
- Do not overload roof or floor trusses with stacks of construction materials
- 14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
- 15. Care should be exercised in handling erection and installation of trusses.

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ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

	DIAGONAL VERTICAL I DOUBLED I HAGE IS HAGE IS TOTAL LEW TOTAL LEW IN	MAX GA	BLE VER	TICAL LENG	TH
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	L BRACE OFTON: L LENGTH ANY BE PRIEND DIAGONAL S USED. CONNECT L BRACE FOR SAGE ENGIN 1S 14*. ENGIN 1S 14*. CONNECT DIAGONAL AT MIDPOINT OF VERTICAL CONNECT DIAGONAL AT	SP HH	SP HF	SPF SPF SPF	GABLE VERTICAL SPACING SPECIES
## WAR PEACIN PLATE OF AME THESE STRUC		\$1UD STANDARD \$1ANDARD \$1 #2 #2 \$1UD \$1UD	STUD STANDARD STANDARD #2 #2 #3 STUD STANDARD STANDARD STANDARD	AUS SY	CRADE
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***ARRONG** TRUSSES REDURE EXTREME CARE IN FABRICATING, HANDLING, SUPPING, INSTALLING AND BRADING. RETER ID BESS 1-42 (BUILDING CIN-PONN) SAFETY INFORMATION, PUBLISHED BY TPJ (TRUSS PARE EXTITUTE, 283 INFORMATIO RE, SURTE 200, MADISON, VI. 337199 AND VITCA (*VIDIO TRUSS CID-NELL OF REPORTING ENTERLY AND ENTERLY CONTRACTIONS CHARLES THE CONTRACT ATTACED IN THE CONTRACT ATTACED SANCE HAVE A PROPERLY ATTACED STRUCTURAL PARELS AND BUTTOM CHORD SANCE HAVE A PROPERLY ATTACED STRUCTURAL PARELS AND BUTTOM CHORD SANCE HAVE A PROPERLY ATTACED RIGID CELLING.	SEED TO SEED THE SEED	5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6		5 10 5 10 5 10 5 10 5 10 5 10 5 10 5 10	GROUP A
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ATTACEC RIG			8 11 1 B 8 8 8 11 11 11 11 11 11 11 11 11 11 11	5 5 6 6 7 7 7 5 6 6 7 1 1 5 6 6 6 7 1 1 5 6 6 6 7 1 1 5 6 6 6 7 1 1 5 6 6 6 7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	BRACE *
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J.J.	EXA #EN OR BETTER	hadred I I I I I		0 4 8 8 8 8 11 11 5 2 2 2 6 6	UP B
ULIUS CONS. ENGI	BEALEN SON HE STATES	7 7 7 7 7 7 7	1111111	5 0 0 4 10 0 0 0 10 0	2Xe 1
JEERS P.A. THE STATE OF FLORIDA THE STATE OF FLORIDA	l 11 // 18		14 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 0 0 0 1 8 8 8 0 1 8	₩ *
	VERTICAL LENGTH			12 11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	A dn
MAX. TOT. LD.			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	12 11 12 11 1 1 1 1 1 1 1 1 1 1 1 1 1 1	BRACE **
LD. 60 PSF	CABLE END SUPP DUTLLONGERS # PLYWOOD OVER ATTACH EACH "L" # # POR (1) "L" B # POR (2) "L" END Z CABLE V VERTICA LESS THAN GREATER TI LESS THAN GREATER TE PEAX, SPLI	CABLE TI LIVE LOAD DEPLE PROVIDE UPLIT : CONTINUOUS BI	SOUTHERN F	SPRUCE-PINI 11 / 12 STA 13 STANDARD STANDARD	potomic op

OUP A: HEM HEM #3 SOUTHER #3 STANI OUP B: DOUGLAS F DOUGLAS F AL DOUGLAS F	HI HILL HILL HILL HILL HILL HILL HILL H	1 11	STANDARD	STUD #3	IGLAS FIR-LARCH	STUD	#2 STANDARI	GR PRUCE-PINE-FIR	
	DOUGLAS	GROUP B:	COLOR	STUD	SOUTHERN PINE	#3 STANDARD	12	I	

TRUSS DETAIL NOTES:

LIT CONNECTIONS FOR 136 FLF OVER US BELARING (6 PEF WC DEAD LOAD).

SUPPORTS LOAD FROM 4: 0*
RES WITH E: 0* DYERHANG, DW 12*
OVERHANG. PLECTION CHATERIA IS L/240.

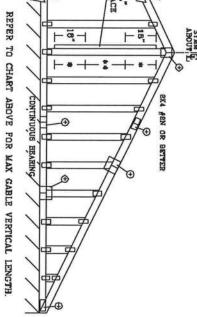
H "L" BRACE WITH 10d NAILS.
"L" BRACE: SPACE NAILS AT 2" O.C.
ND ZONES AND 4" O.C. BETWEEN ZONES.
"L" BRACES: SPACE NAILS AT 3" O.C.
ND ZONES AND 6" O.C. BETWEEN ZONES. CUST BE A MINIMUM OF 80% OF WEB

FOR	N DESIGN	TRUSS	MOMIN	i g	EFTAR TO
	2.5X4	L	11 B	MAH	EATER
	200	TUB	0 0	I NAH	EATER
233	1X4 OR		9	N 4	AL SS
Ø	NO SPL		HI5N8	KVT I	VERT
"	LE SIZE	PLAT	RTICAL	VER	CABLE

•					_	L	1		
TOT.									
LD.									
60					+ REI	GRE	GRE	E S	
60 PSF					72R 10	ATER T	EATER THAN 11	ESS THAN 4	VERTICAL
	-ENG	DRWG	DATE	REF	REFER TO COMMON TRUSS DESIG	GREATER THAN 11' 6"	GREATER THAN 4 D', BUT	4 0	1
		MATER	11,	ASC	TRUSS	Ī.	BUT		
		NITEK STD GABLE 15 E HT	11/26/03	ASCE7-02-GAB13015	REFER TO COMMON THUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.	2.5X4	2%4	1X4 OR 2X3	NO SPLICE
		1 I I		3015					

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I Ш 1.00, EXPOSURE a

					_												_								
DIAGONAL VERTICAL DOUBLED BRACE IS BRACH AT EACH TOTAL LE		M	ΑΣ	ζ	(j A	A E	3I	Æ	C	V	E	R	r	'I(C	ΔI	,	L	Æ,	'N	10	[';	H	
	1	2	"	0),(3.			1 (6'	,	0	.(Ξ.		;	S	1		C),(C		SPACING	CARI
BRACE OPTION: LENGTH MAY BE WHEN DIAGONAL USED, CONNECT BRACE FOR SOG! END. MAY WEB WGTH 15 14',	LHL	7 1	S	111	I	לעלי	בוב	t	JFI.	7	Ŋ	111	Į	OFF	STI	ţ	J T T	Ŋ T	2	111	Į,	ひてっ	בובים	SPECIES	CABLE VERTICAL
	STANDARD	#3	#2	STANDARD	STUD	ü	#1 / #2	cq	S	*3	47	STANDARD	STUD	#3		STANDARD	CILLS.	200	\$ 1	STANDARD	STUD	#3	#1 / #2	SPACING SPECIES GRADE	BRACE
SBUEL THEVE	4 4		4. 4.	7	3' 11"		4.0			30, 5	2 4	7	3' 7,	3' 7"		3	_	3 0	3 8	1	3' 1"		ယ လ	BRACES	5
	0 4	6, 6,	6 11	7-	6' 3		6 11.	4. 9.	5 6	5 2		7	5, 6,	5. 5.	6 4	3' 10"	A	0 6	5		4' 6"	4. 5	5. 6.	GROUP A	\vdash
	5 6 4	1	7' 6"		6.3.	6 3 _"	7. 5.	4' 9"	5 6	5. 7.			6' 5"	5. 5.	6. 6.	3' 10"	A			3. B.	4' 5"	4. 5.		GROUP B	(1) IX4 "L" BRACE
. "	7 8		ය ශූ ය		a, 3,	8' 3"	8.	6' 3"	7 3		7. 6.		7. 2.	7' 2"	7' 6"	ο _ι ;		6, 6,		6.0		6. 10.	6, 6,	GROUP	• (1) 2X4
TIE" LOSO	7 6		8 11		8' 3"		8.	6. 3	7 3	7' 4"	0 0	6. 23	7' 2"	7' 2"	7' 8"		70	7. 0		5.	5,	S,	6. 9.	A GROUP	(1) 2X4 "L" BRACE • (2) 2X4 "L" BRACE •• (1) 2X6
-1	9. 10.	9' 10"	9, 10		9' 10"	9, 10,	9. 10.	B, 5	8, 11	20,00			8' 11"				3, 10		7' 10		-7.	7.	7 10	B GROUP A	• (2) 2X4
ext ten	9, 9,	Н	10' 7"	6,	9	+	10. 1.	8. 2.	80,00	9,0	5 00	8' 3'	8	8' 11'	9.	90, 0	+	+	8,	6' 9"	7' 10"	7' 10	8.0.	A GROUP B	"L" BRACE
7/ 8	12, 11	12	12, 11	11.	12.	12'	12	9,	11 4	+	11	8		-]	= ,	00 0	1	10	10'	-1	9' 1"	8	10. 3.	B GROUP	** (1) 2X6
	13. 1.		13 11		12'	12	13.	٥	-	1 12	12 B	. 8	11.	11	12	00,0	4 6	1			9, 1,	-	10. 2	>	"L" BRACE .
	14 0	Н	14.0	14	14	+	1	7 2		+	+		14		+	10, 10,	200	12'	12' 3'	10	+	12'	12. 3.	CROUP B CROUP A	
	14.0	Н	14.0	1.4		_	+	+	1 1 1		\vdash	_	-		14	10' 10'	12	13'		10	12'	+	12' 7	A CROUP B	(2) ZXB "L" BRACE
Į.	L	Ц		Ш	•		1	*	1	1			•	-	1	1	1	_		1	•		4	₩ <u> </u>	1
GABLE END SUPPORTS LOAD FROM 4 O" DUTLIDDRERS WITH E O" DVERHANG, OR 12" PLYWOOD DVERHANG. ATTACH EACH 'L' BRACE WITH 104 NAILS. 4 FOR (1) 'L' BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWIEN ZONES 4 FOR (2) 'L' BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWIEN ZONES "L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH. GABLE VERTUCAL PLATE SIZES	CONTINUOUS BEARING (6 PSF TC DEAD LOAD).	PROVIDE THE INT CONNECTIONS THE 18 L/ 240	TO THE PROPERTY OF THE PARTY OF	CARLE TRUSS DETAIL NOTES				#1 #1		11	HEM-FIR	GROOF B:			STANDARD	П	DOGGLAS FIX-LAKEN SOUTHE	1	2	STANDARD STANDARD	OUP A:		BRACING GROUP SPECIES AND GRADES:		
O" O" IZ" AT E" O.C. STWIZEN ZONES SAT 3" O.C. STWIZEN ZONES AT 8" O.C. FW BOX OF WEB	EAD LOAD).	7240.		OTER			#2	LIN-MAINLE	DOG! GI						STANDARD	STUD	SOUTHERN PINE		STANDARD	HEM-PIR			GRADES:		



VERTICAL LENGTH SHOWN IN TABLE ABOVE.

ZX4 SP OR
DIT-L #2 OR
BETTER DIAGONAL
BRACE; SINGLE
OR DOUBLE
CUT (AS SHOWN)
AT UPPER END

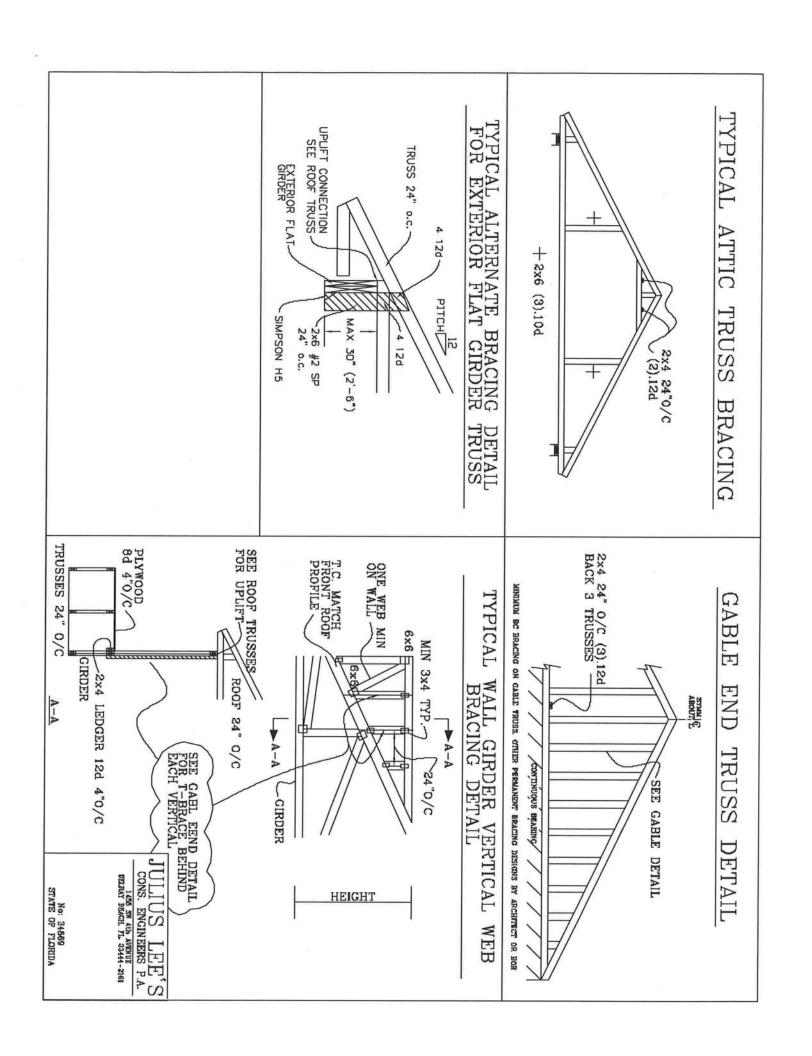
MIDPOINT OF VERTICAL WEB.

TOR.	PLATES.	HEEL I	PLICE, AND I	SAX, S
L	2.5X4	ľ.	THAN 11' 6	RATER
	2X4	TUB	THAN 4 D	LISS
ex:3	1X4 DR		AN 4' 0"	HI S
CE	NO SPL		BCAL LENCTH	VERT
0,	E SIZES	PLATE	VERTICAL	ABLE

PEAK, SPLICE, AND HEEL	GREATER THAN 11' 6"	CREATER THAN 4 D', BUT	LESS THAN 4: 0"
TRUSS DESIGN FOR	2.5%4	T 2X4	1X4 DR EX

No: 34869 STATE OF FLORIDA			DELRAY BEACH, PL 33444-2161	SINE	JULIUS LEE'S
WAX	MAX.				
MAX SPACING 24.0"	TOT. LD. 60 PSF				
CING	EĐ.				
2	60				
5	PSF				
3/2		-ENG	DWG x	DATE	REF
			DWG MITEK STD GABLE 30' E HT	11/26/03	ASCE7-02-GAB13030
					3

MAYARRIMGEN TRASSES REDURE EXTREME CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND BROCING. BETH TO BEST 1-43 SAULLING COMPENENT SAFETY (HEDRANIDA), PUBLISHED IN TPI CIRRASS PART INSTITUTE, 383 D'ONDERED IR. SUITE 200, MINISON, HE SAFETY PARTICES, AND LYCK AUGOD TRUSS CLOUGH. OF ANERICA, 6300 ENTERPRISE UK, MIGISON, UT SATINJO FOR SAFETY PARTICES PRIZE TO PERFORMING THESE TAKTIDES, UNICESS OF THE CARE TO PERFORMING THESE TAKTIDES, UNICESS OF THE CARE TO PERFORMING THESE TAKTIDES, UNICESS OF THE CARE TO PERFORMING THESE TAKTIDES. UNICESS OF THE CARE TO PERFORM THE CARE PROPERTY ATTACHED RIGHD CEILING.



BOT CHORD 2X4 2X4 300 222 BETTER BETTER BETTER

PIGGYBACK DETAIL

TYPE

SPANS

UP

32

8 5

52

2.5X4

2.6X4

6X6

5X8

5X6 335

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX. TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO BUCINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS: 110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL-5 PSF, WIND BC DL=5 PSF 110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-5 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN HCT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C. WIND TC DL=6 PSF, WIND HC DL=6 PSF

H H C H

> 584 .5X3 4X8 284 30,

5X5

.5X4 5X6

1.5X4

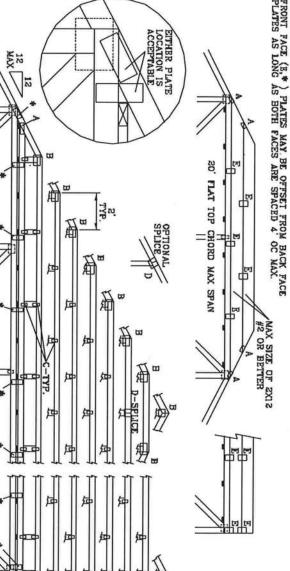
1.5X4 5X6

4XB OR 3X6 TRULOX AT 4'
HOTATED VERTICALLY

DC,

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.575" EQUAL PER FACE PER PLY. (4) NAILS IN EACH 1 BE CONNECTED. REFER TO DRAWING 160 TL FOH INFORMATION.

NAILS, OR MEMBER TO



errova i eam	
WEB LENGTH	REQUIRED BRACING
o' To 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECI MEMBER. OR BETTER. AND 80% LENG! MEMBER. ATTACH WITH 8d NAILS AT
10' TO 14'	2x4 "I" BRACE. SAME GRADE, SPECIES AS WI MEMBER. OR BETTER, AND 80% LENGTH OF WI MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF PABRICATION. ATTACH TO SUPPORTING TRUSS BYTHE (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4" OC OR LESS. 8 1/4" N

l	
	THIS
	DRAWING
	REPLACES
	DRAWINGS
	634,016
	834,017 &
	& B47.0
	ñ

*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

		WAVARRINGON TRUSSES REQUIRE EXTREME CAME IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BACING. REFER TID BEST I TOD GUILLING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE CRUSS PLATE INSTITUTE. SAS OFOUR TOD TA, SUITE 200, HANDSON, U.S. 32759 AND WITCH A VOIDS TRUSS COLUMN FOR FRONCES FUNCTIONS. UNLESS DIVERVIEW, INFORMATION, INFORMATION, INFORMATION, INFORMATION, INFORMATION FOR STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED BIGGO CELLING.	THE PROPERTY OF THE PROPERTY O
STATE OF FLORIDA		JULIUS LEE'S P.A. 1400 SW 4th AVENUE DEERAY BEACH, FL 3944-2161	THIS DRAW
SPACING 24.0"	47 PSF AT 1.15 DUR. FAC.	MAX LOADING 55 PSF AT 1.33 DUR. FAC. 50 PSF AT 1.25 DUR. FAC.	THIS DRAWING REPLACES DRAWINGS 634,016 634,017
		REF PIGGYBACK DATE 09/12/07 DRWGMITEK STD PIGGY -ENG JL	634,016 834,017 & 847,045

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.

BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.

WEBS 2X4 SP #3 OR BETTER.

- * ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE).
- ** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

 (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
 FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
 ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
 BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 8" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN OR BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

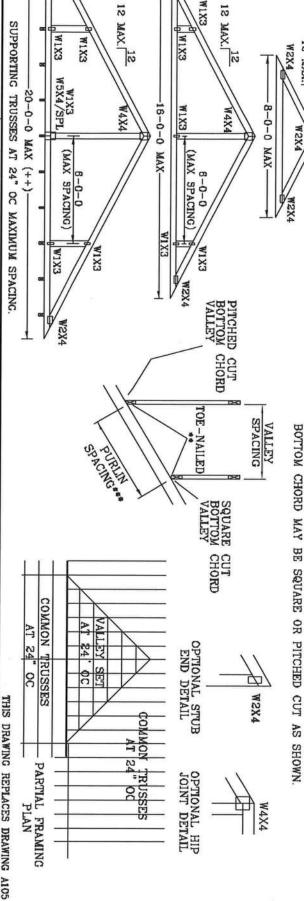
**+ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES

CUT FROM 2X6 OR LARGER AS REQ'D

4-0-0 MAX

12 NAX.

NOT EXCEED 12'0".



WAVARINGAY TRUSKES REQUIRE EXTREME EARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO BEST 1-03 GBUILING COPPINGT SAFETY INFORMATION, PUBLISHED BY THE CRASS FALT INSTITUTE, SED OPOGFRED 28, SUDITE 20, MADISHA, V., 53799 AND VICA VOUDD TRASS COLNICIL OF ARERICA, SOID OFFICERSE LA, MADISHA, V. 53799 AND VICA VOUDD TRASS COLNICIL OF REREAL AND OFFICERS IN THE REPORT OF THE

			W. SNIPPING, INSTALLING AND NN, PABLITHED BY THE CHAISS OF VICE AVOIDS TRAISS COLNEIL THEES PARION TO PERFEDENCY VE PROPERLY ATTACKED THEE PRINCE				
STATE OF FLORIDA	No: 34869			DELRAY BEACH, FL 35444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S	
SP/	DUR	TOT	BC	BC	TC	TC LL	ı
SPACING	UR.FAC. 1.25	TOT. LD.	F	DI	DL	F	I
	S	32 40	0	C,	~2	20	I
24"	1.25	40	0	Ç	15	20	١
		PSF	PSF	PSF	PSF	PSF	I
			PSF -ENG JL	PSF DRWG	PSF DATE	PSF REF	I
			JL	VALTRUSS1103	11/26/03	VALLEY DETAIL	

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

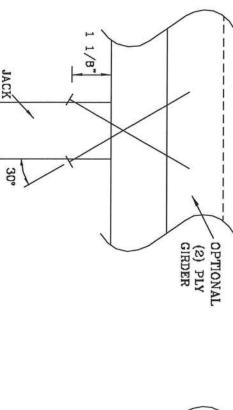
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

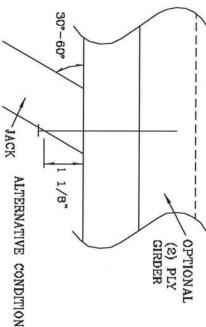
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

ALL VALLE	ט	.4	ယ	N	TOE-NAILS	NUMBER OF	
THE NAM SE	493#	394#	296#	187#	1 PLY		
MILLIAM	639#	511#	383#	256#	2 PLIES	SOUTHERN PINE	
DO AN OR	452#	361#	271#	181#	1 PLY	DOUGLAS	
DADDIATE	585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH	
NOTEN	390#	312#	234#	156#	1 PLY		
ALL VALUES MAY BE MILIMOLIED BY ADDRODOLOG DIAMON OF LOAD EXCENSE	507#	406#	304#	203#	2 PLIES	HEM-FIR	
人の中へ日	384#	307#	230#	154#	1 PLY	SPRUCE	
	496#	397#	#862	189#	2 PLIES	SPRUCE PINE FIR	

AUTO PO MODILITATED DI ATTIVOTNIATE DONALION 5 LUAD FACTOR.





THIS DRAWING REPLACES DRAWING 784040

	HAVARRUNG H. TRUSSES REDURE EXTREME CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BEST 1-33 CHULIDHG CORPINE SAFETY RETORALTIDAS, PURLISHED BY TRY CRUSS PLATE INSTITUE, 38 D'HOCKRID BY, SUITE 200, NADISCH, VI. 53719) AND VICE (HOLD TRUSS CONCEINED, SUITE 200, NADISCH, VI. 53719) AND VICE (HOLD TRUSS CONCEINED, LIN, MARISCH, VI. 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNICESS OTHERWISE INDICATED, THE SAFETY PROPERTY ATTACHED RIGHD CEILING. STRUCTURAL PANELS AND BOTTON CHORD SHALL HAVE A PROPERTY ATTACHED RIGHD CEILING.						
STATE OF FLORIDA	No: 34869 STATE OF FLORIDA			DELRAY BEACH, FL 33444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S	
SPACING	DUR. FA	TOT. LD.	BC LL	BC DL	TC DL	TC LL	
-	FAC. 1.00	. PSF	PSF	PSF	PSF	PSF	
			-ENG	DRWG	DATE	REF	
			JL	CNTONAIL1103	09/12/07	TOE-NAIL	

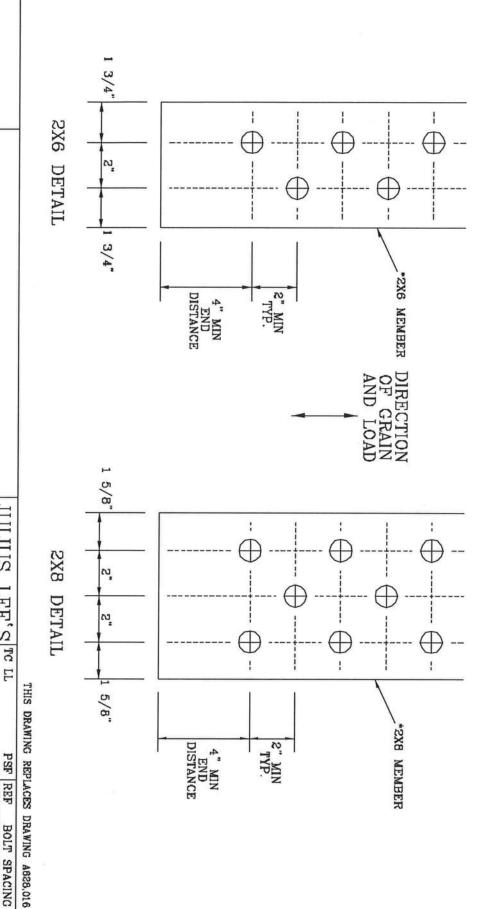
DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL T0GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



ULIUS LEE'S cons. ENGINEERS P.A. DELRAY BEACH, FL 33444-2161 TC DL BC LL TC BC DL DUR. FAC TOT. LD. T PSF PSF PSF PSF PSF REF DATE DRWG -ENG CNBOLTSP1103 11/26/03

No: 34869 STATE OF FLORIDA

SPACING

TRULOX CONNECTION DETAIL

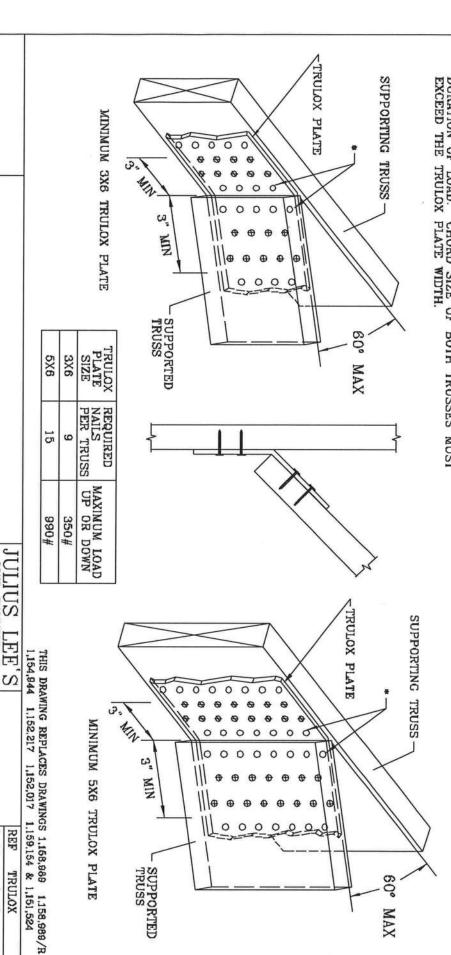
II GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (+).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



CONS. ENGINEERS P.A.

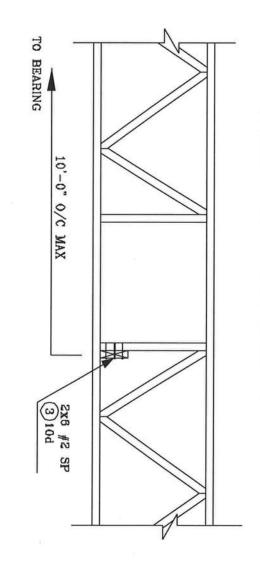
1455 SW 4th AVENUE
DELRAY BEACK, 7L 35444-2151

DATE

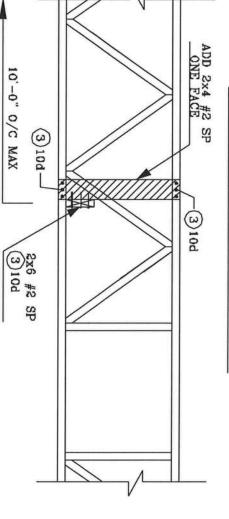
11/26/03 CNTRULOX1103

No: 34869 STATE OF FLORIDA

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S CONS. ENGINEERS P.A.

1455 SM 4th AVENUE
DELRAY BEACH, D. 33441-2161

No: 34869 STATE OF FLORIDA TO BEARING