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ALPINE
AN ITW COMPANY

This item has been digitally signed by William H. Krick on the date adjacent to the seal.

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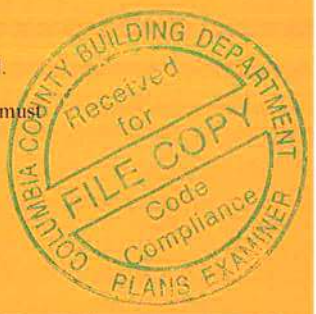
COA #0 278

Florida Certificate of Product Approval #FL 1999

04/30/2024



Site Information:	Page 1:
Customer: Seminole Trusses, Inc.	*Job Number:* B59010a
Job Description: Jones Residence	
Address: SW Leo Feagle Glen, Lake City, FL	

Job Engineering Criteria:	
Design Code: FBC 8th Ed. 2023 Res.	*IntelliVIEW Version:* 23.02.01A *JRef #:* 1XZG8570004
Wind Standard: ASCE 7-22 *Wind Speed (mph):* 130 *Building Type:* Closed	*Design Loading (psf):* 37.00

This package contains general notes pages, 30 truss drawing(s) and 4 detail(s).

Item	Drawing Number	Truss
1	121.24.1009.05641	CJ1
3	121.24.1009.05663	CJ3
5	121.24.1009.06150	GE1
7	121.24.1009.06765	HGT2
9	121.24.1009.05757	HT3
11	121.24.1009.07048	HT5
13	121.24.1009.07142	HT7
15	121.24.1009.06563	HT9
17	121.24.1009.06703	HT11
19	121.24.1009.06734	HT13
21	121.24.1009.06565	HT15
23	121.24.1009.05993	HT18
25	121.24.1009.06403	M2
27	121.24.1009.07532	SGT1
29	121.24.1009.07001	T2
31	CNNAILSP1014	
33	PB180160118	

Item	Drawing Number	Truss
2	121.24.1009.06781	CJ2
4	121.24.1009.06387	EJ5
6	121.24.1009.05616	HGT1
8	121.24.1009.06266	HJT4
10	121.24.1009.05977	HT4
12	121.24.1009.07000	HT6
14	121.24.1009.06234	HT8
16	121.24.1009.05914	HT10
18	121.24.1009.06235	HT12
20	121.24.1009.06923	HT14
22	121.24.1009.06119	HT16
24	121.24.1009.06467	M1
26	121.24.1009.07002	PB1
28	121.24.1009.05987	SGT2
30	121.24.1009.06516	T3
32	PB160160118	
34	REPCHRD1014	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
C = Coated lumber.
C-AT = AtTEK coated lumber.
C-FX = FX Lumber Guard coated lumber.
C -TW = TechWood 4400 coated lumber.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
FRT-PR = ProWood Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

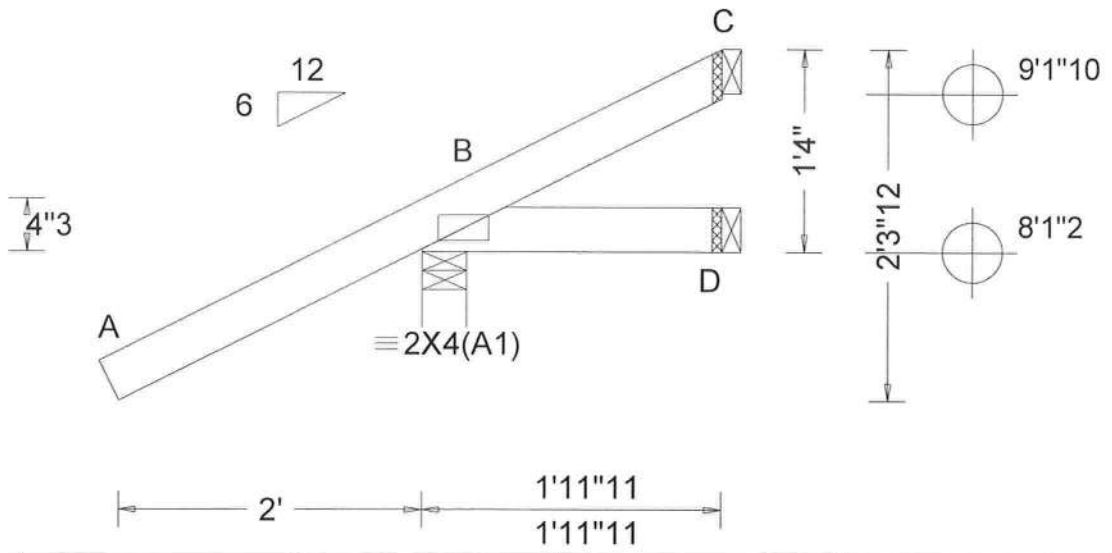
General Notes (continued)

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com

SEQN: 199419 / JACK FROM: RNB	Ply: 1 Qty: 8	**Job Number:** B59010a Jones Residence **Truss Label:** CJ1	Cust: R 857 JRef: 1XZG8570004 T22 / DrwNo: 121.24.1009.05641 SSB / WHK 04/30/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): NA			
VERT(CL): NA			
HORZ(LL): -0.001	B	-	-
HORZ(TL): 0.002	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.259
Max BC CSI: 0.062
Max Web CSI: 0.000

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	285	/-	/-	/178	/39	/43
D	24	/-6	/-	/20	/7	/-
C	-	/-5	/-	/23	/21	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Support)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

COA #0778
04/30/2024
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****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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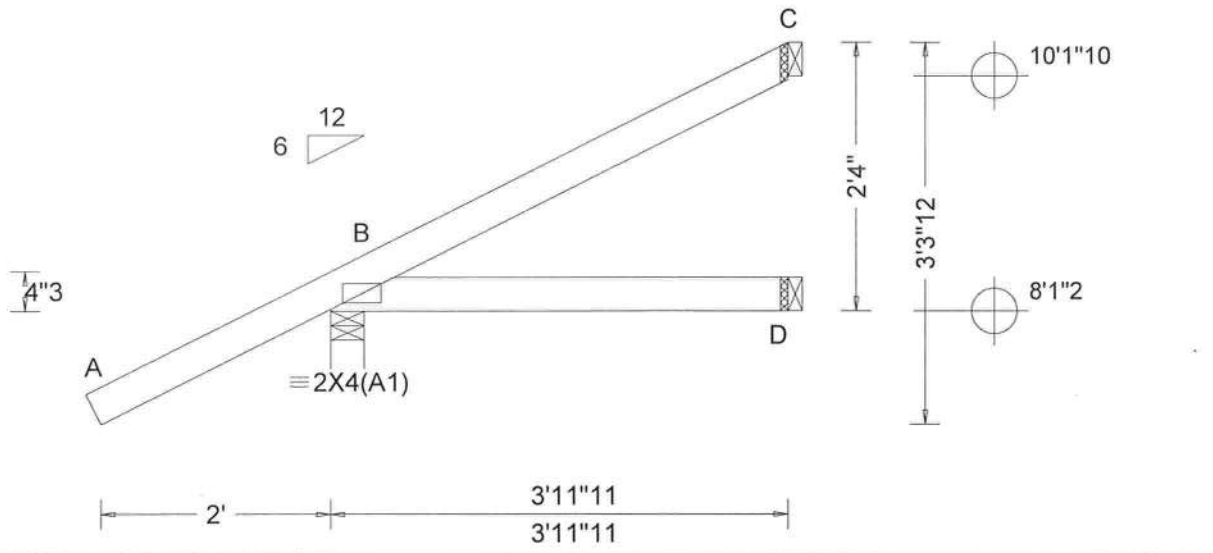
Glenview, IL 60025

SEQN: 199421 / JACK FROM: RNB	Ply: 1 Qty: 8	**Job Number:** B59010a Jones Residence **Truss Label:** CJ2	Cust: R 857 JRef: 1XZG8570004 T21 / DrwNo: 121.24.1009.06781 SSB / WHK 04/30/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/0(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.002 B - - Creep Factor: 2.0 Max TC CSI: 0.219 Max BC CSI: 0.089 Max Web CSI: 0.000 VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	318	/-	/-	/191	/22	/67
D	66	/-	/-	/35	/-	/-
C	76	/-	/-	/34	/27	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Support)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

COA #6178

04/30/2024

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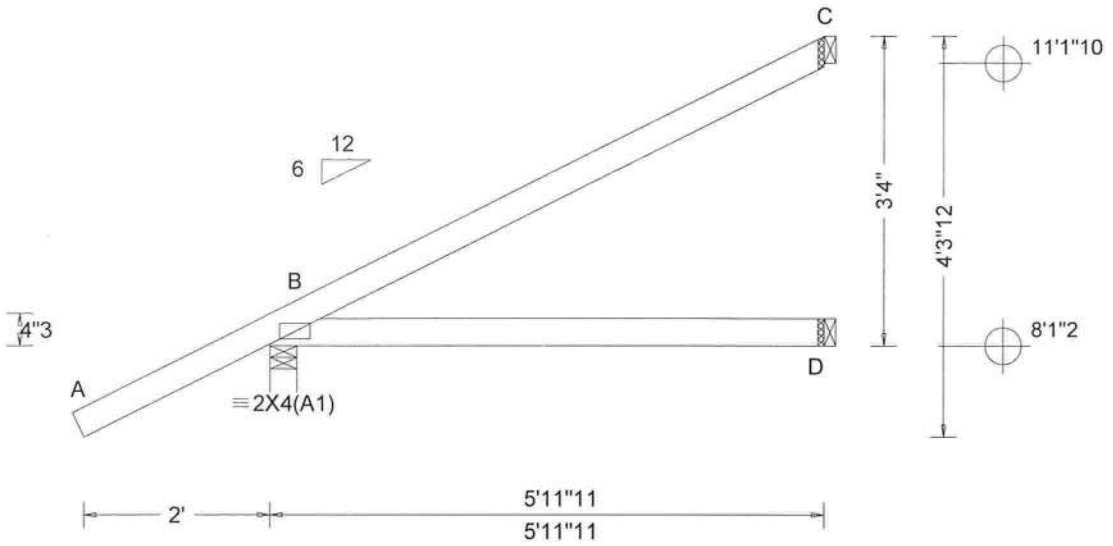
Glenview, IL 60025

SEQN: 199423 / JACK FROM: RNB	Ply: 1 Qty: 8	**Job Number:** B59010a Jones Residence **Truss Label:** CJ3	Cust: R 857 JRef: 1XZG8570004 T20 / DrwNo: 121.24.1009.05663 SSB / WHK 04/30/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 6.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/0(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.005 B - - HORZ(TL): 0.010 B - - Creep Factor: 2.0 Max TC CSI: 0.295 Max BC CSI: 0.244 Max Web CSI: 0.000 VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	381	/-	/-	/224	/15	/91
D	105	/-	/-	/56	/-	/-
C	135	/-	/-	/65	/45	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Support)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

COA #0278

04/30/2024

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155 Harlem Ave
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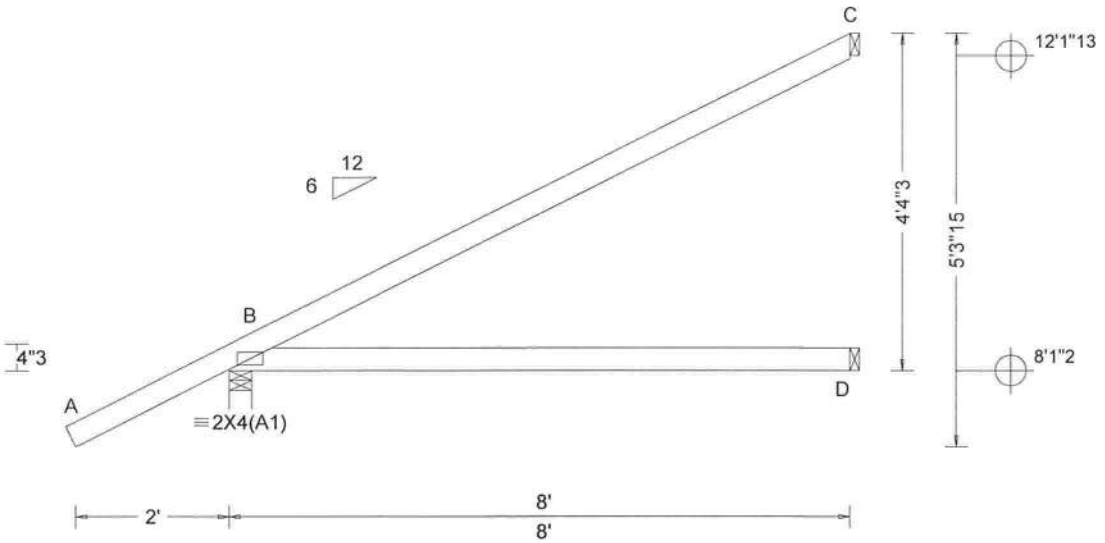
Glenview, IL 60025

SEQN: 199427 / EJAC FROM: RNB	Ply: 1 Qty: 15	**Job Number:** B59010a Jones Residence **Truss Label:** EJ5	Cust: R 857 JRef: 1XZG8570004 T23 / DrwNo: 121.24.1009.06387 SSB / WHK 04/30/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	NA		
VERT(CL):	NA		
HORZ(LL):	0.017 B	-	-
HORZ(TL):	0.031 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.617
Max BC CSI: 0.470
Max Web CSI: 0.000

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	451	/-	/-	/263	/10	/115
D	144	/-	/-	/77	/-	/-
C	190	/-	/-	/94	/63	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Support)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

COA #0278
04/30/2024
Florida Certificate of Product Approval #FL 1999

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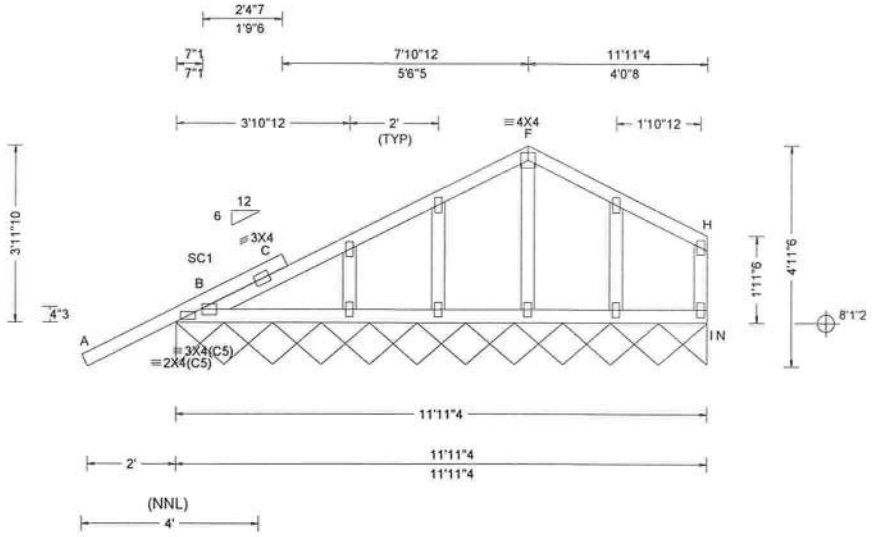
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199357 / GABL FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** GE1	Cust: R 857 JRef: 1XZG8570004 T16 / DrwNo: 121.24.1009.06150 SSB / WHK 04/30/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.004	C	999	360
VERT(CL): 0.007	C	999	240
HORZ(LL): -0.002	H	-	-
HORZ(TL): 0.004	H	-	-

Creep Factor: 2.0
Max TC CSI: 0.320
Max BC CSI: 0.046
Max Web CSI: 0.322

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
N*	117	/-	/-	/54	/19	/14

Wind reactions based on MWFRS
N Brg Wid = 143 Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #1;

Plating Notes

All plates are 2X4 except as noted.

Plates sized for a minimum of 3.50 sq.in./piece.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 6.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/890.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

COA #0778
04/30/2024
Florida Certificate of Product Approval #FL 1999

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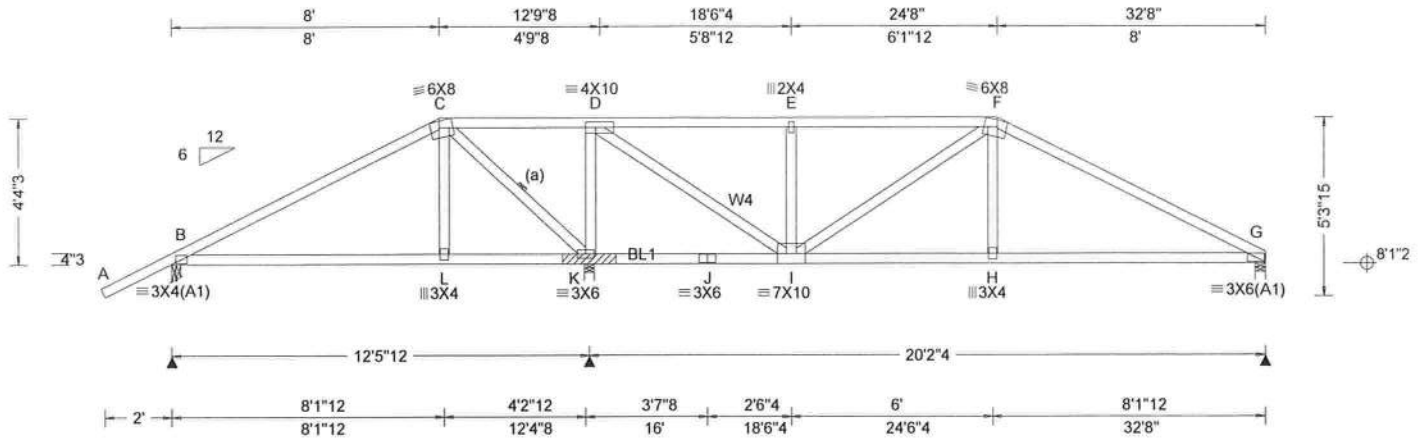
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199435 / HIPS FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** HGT1	Cust: R 857 JRef: 1XZG8570004 T30 / DrwNo: 121.24.1009.05616 SSB / WHK 04/30/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.27 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.083	H	999	360
VERT(CL): 0.153	H	999	240
HORZ(LL): 0.019	G	-	-
HORZ(TL): 0.035	G	-	-

Creep Factor: 2.0
Max TC CSI: 0.856
Max BC CSI: 0.822
Max Web CSI: 0.922

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	531	/-	/-	/-	/43	/-
K	4402	/-	/-	/-	/336	/-
G	1445	/-	/-	/-	/93	/-

Wind reactions based on MWFRS
B Brg Wid = 3.0 Min Req = 1.5 (Support)
K Brg Wid = 3.5 Min Req = -
G Brg Wid = 3.5 Min Req = 1.8 (Support)
Bearings B, K, & G Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
C - D	1257	- 103	E - F	125	- 1665
D - E	125	- 1664	F - G	209	- 2656

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
K - J	75	- 1069	I - H	2325	- 161
J - I	75	- 1069	H - G	2296	- 165

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - L	756	0	E - I	276	- 855
C - K	118	- 1883	I - F	44	- 799
K - D	380	- 2656	H - F	811	0
D - I	3306	- 243			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W4 2x4 SP #1;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	56 plf at	-2.13 to	56 plf at	8.00
TC: From	28 plf at	8.00 to	28 plf at	24.67
TC: From	56 plf at	24.67 to	56 plf at	32.67
BC: From	4 plf at	-2.13 to	4 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	8.03
BC: From	10 plf at	8.03 to	10 plf at	24.64
BC: From	20 plf at	24.64 to	20 plf at	32.67

TC: 295 lb Conc. Load at 8.03,24.64
TC: 190 lb Conc. Load at 10.06,12.06,14.06,16.06 16.60,18.60,20.60,22.60
BC: 560 lb Conc. Load at 8.03,24.64
BC: 144 lb Conc. Load at 10.06,12.06,14.06,16.06 16.60,18.60,20.60,22.60

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Bearing Block(s)

Brg blocks:0.128"x3", min. nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
2	12.333'	1	19"	13	SPF Standard

Brg block to be same size and species as chord.
Refer to drawing CNNAILSP1014 for more information.

COA #0278
04/30/2024
Florida Certificate of Product Approval #FL 1999

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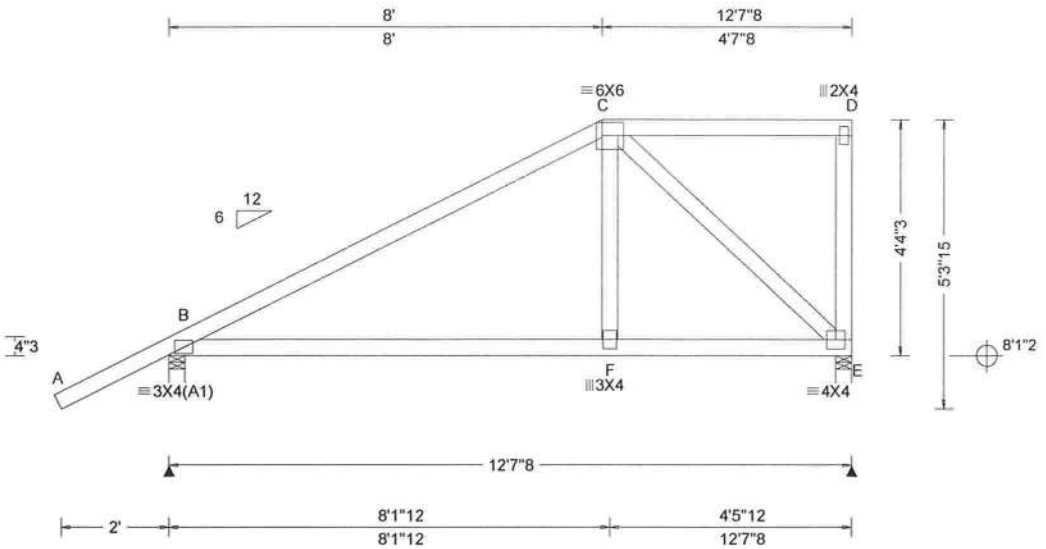
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199447 / HIPM FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** HGT2	Cust: R 857 JRef: 1XZG8570004 T3 / DrwNo: 121.24.1009.06765 SSB / WHK 04/30/2024



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.022	F	999	360
VERT(CL): 0.041	F	999	240
HORZ(LL): 0.010	B	-	-
HORZ(TL): 0.019	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.786
Max BC CSI: 0.565
Max Web CSI: 0.958

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	980	/-	/-	/-	/94	/82
E	1448	/-	/-	/-	/93	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Support)
E Brg Wid = 3.5 Min Req = 1.8 (Support)
Bearings B & E Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - C	123	- 1293

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - F	1060	- 4	F - E	1090	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - F	826	0	C - E	52	- 1448

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Special Loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	56 plf at	-2.13 to	56 plf at	8.00
TC: From	28 plf at	8.00 to	28 plf at	12.63
BC: From	4 plf at	-2.13 to	4 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	8.06
BC: From	10 plf at	8.06 to	10 plf at	12.63

TC: 295 lb Conc. Load at 8.06
TC: 190 lb Conc. Load at 10.12,12.12
BC: 560 lb Conc. Load at 8.06
BC: 144 lb Conc. Load at 10.12,12.12

Plating Notes
Plates sized for a minimum of 3.50 sq.in./piece.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

COA #0278
04/30/2024
Florida Certificate of Product Approval #FL 1999

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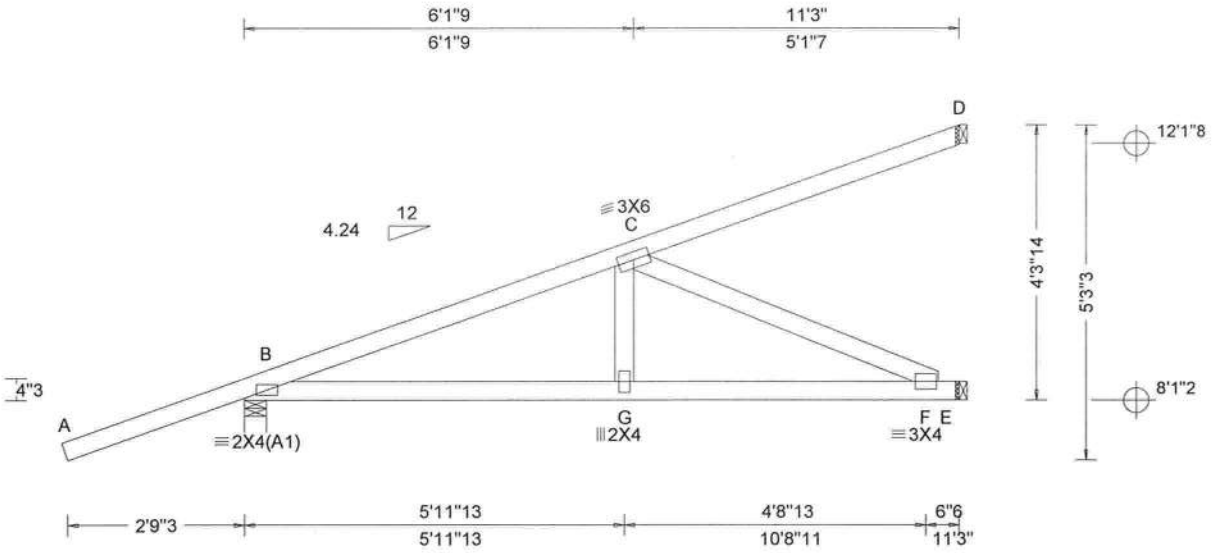
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199425 / HIP_ FROM: RNB	Ply: 1 Qty: 4	**Job Number:** B59010a Jones Residence **Truss Label:** HJT4	Cust: R 857 JRef: 1XZG8570004 T26 / DrwNo: 121.24.1009.06266 SSB / WHK 04/30/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.032	G	999	360
VERT(CL): 0.061	G	999	240
HORZ(LL): 0.007	F	-	-
HORZ(TL): 0.013	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.592
Max BC CSI: 0.689
Max Web CSI: 0.508

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	416	/-	/-	/-	/73	/-
E	416	/-	/-	/-	/16	/-
D	106	/-	/-	/-	/26	/-

Wind reactions based on MWFRS
B Brg Wid = 4.2 Min Req = 1.5 (Support)
E Brg Wid = 1.5 Min Req = -
D Brg Wid = 1.5 Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - C	98	-856

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - G	805	-87	G - F	791	-90

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
C - F	98	-869

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Special Loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 1 plf at -2.93 to 55 plf at -0.10
TC: From 2 plf at -0.10 to 2 plf at 11.25
BC: From 0 plf at -2.93 to 4 plf at -0.10
BC: From 2 plf at 0.00 to 2 plf at 11.25
TC: -2 lb Conc. Load at 2.79
TC: 151 lb Conc. Load at 5.62
TC: 269 lb Conc. Load at 8.45
BC: 48 lb Conc. Load at 2.79
BC: 132 lb Conc. Load at 5.62
BC: 210 lb Conc. Load at 8.45

Plating Notes
Plates sized for a minimum of 3.50 sq.in./piece.

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

COA #0278
04/30/2024
Florida Certificate of Product Approval #FL 1999

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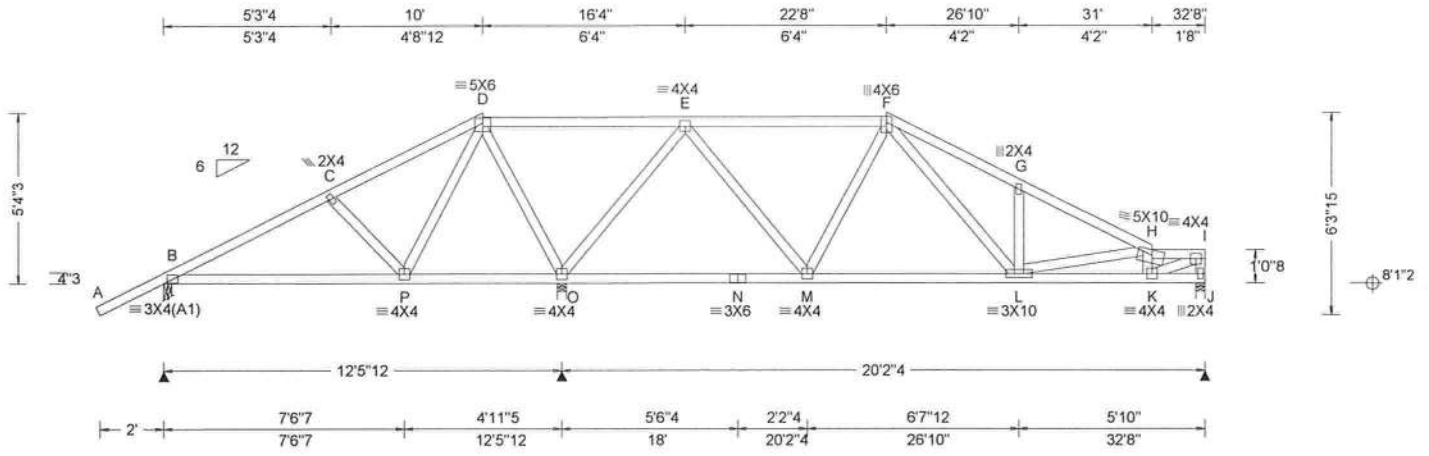
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199417 / SPEC FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** HT3	Cust: R 857 JRef: 1XZG8570004 T2 / DrwNo: 121.24.1009.05757 SSB / WHK 04/30/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.27 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.032	G	999	360
VERT(CL): 0.062	G	999	240
HORZ(LL): 0.005	E	-	-
HORZ(TL): 0.010	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.451
Max BC CSI: 0.330
Max Web CSI: 0.822

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	460	/-	/-	/240	/24	/116
O	1616	/-	/-	/772	/71	/-
J	638	/-	/-	/341	/18	/-

Wind reactions based on MWFRS
B Brg Wid = 3.0 Min Req = 1.5 (Support)
O Brg Wid = 3.5 Min Req = 2.0 (Support)
J Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings B, O, & J Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
D - E	633	- 91	G - H	168	- 1023
E - F	148	- 417	H - I	179	- 1155
F - G	229	- 1010			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
M - L	539	- 71	L - K	1152	- 177

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
P - D	382	- 12	F - L	512	- 61
D - O	181	- 731	H - K	118	- 499
O - E	342	- 1060	K - I	1253	- 184
E - M	536	- 8	I - J	103	- 598

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

COA #0778
04/30/2024
Florida Certificate of Product Approval #FL 1999

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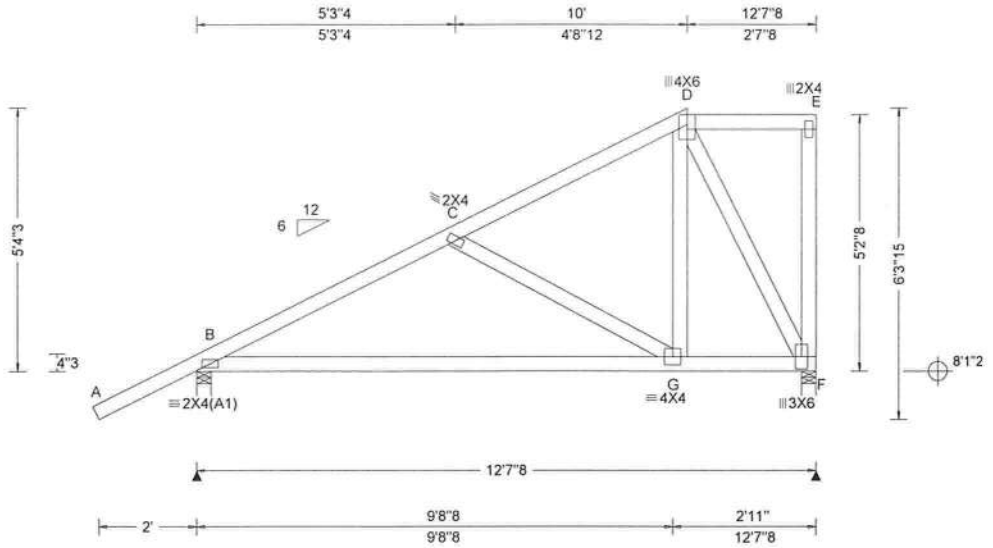
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199445 / HIPM FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** HT4	Cust: R 857 JRef: 1XZG8570004 T4 / DrwNo: 121.24.1009.05977 SSB / WHK 04/30/2024



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.011	C	999	360
VERT(CL): 0.027	B	999	240
HORZ(LL): 0.005	B	-	-
HORZ(TL): 0.015	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.205
Max BC CSI: 0.480
Max Web CSI: 0.372

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	619	/-	/-	/352	/34	/166
F	460	/-	/-	/261	/21	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Support)
F Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings B & F Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - C	148	-629

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - G	519	-245

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
G - D	418	0	D - F	127	-452

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes
Plates sized for a minimum of 3.50 sq.in./piece.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

COA #0278
04/30/2024
Florida Certificate of Product Approval #FL 1999

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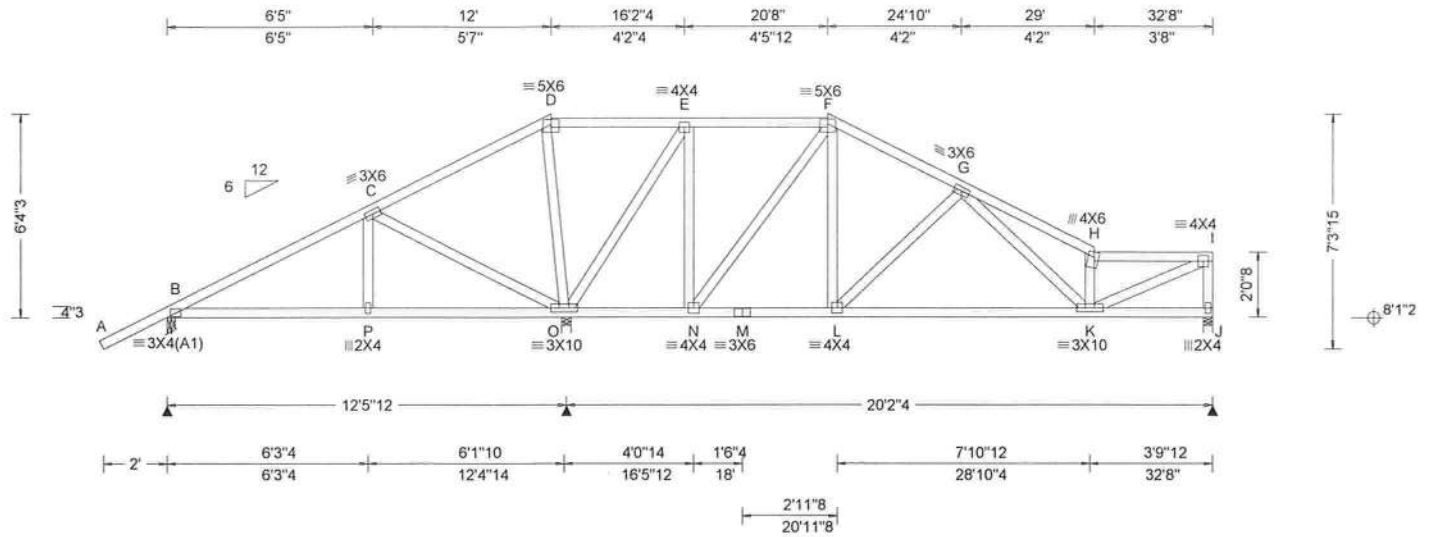
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199413 / SPEC FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** HT5	Cust: R 857 JRef: 1XZG8570004 T32 / DrwNo: 121.24.1009.07048 SSB / WHK 04/30/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.27 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.034	H	999	360
VERT(CL): 0.066	H	999	240
HORZ(LL): 0.007	F	-	-
HORZ(TL): 0.013	I	-	-

Creep Factor: 2.0
Max TC CSI: 0.360
Max BC CSI: 0.347
Max Web CSI: 0.900

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	439	/-	/-	/221	/26	/146
O	1651	/-	/-	/811	/63	/-
J	630	/-	/-	/324	/22	/-

Wind reactions based on MWFRS
B Brg Wid = 3.0 Min Req = 1.5 (Support)
O Brg Wid = 3.5 Min Req = 2.1 (Support)
J Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings B, O, & J Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
C - D	627	- 87	G - H	226	- 1199
D - E	550	- 40	H - I	183	- 1085
F - G	154	- 495			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
N - M	376	- 48	L - K	668	- 120
M - L	376	- 48			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - O	89	- 570	L - G	116	- 386
D - O	144	- 471	G - K	503	- 59
O - E	238	- 950	K - H	195	- 721
E - N	530	- 22	K - I	1183	- 165
N - F	74	- 581	I - J	131	- 607
F - L	438	0			

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes
Plates sized for a minimum of 3.50 sq.in./piece.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

COA #0278
04/30/2024
Florida Certificate of Product Approval #FL 1999

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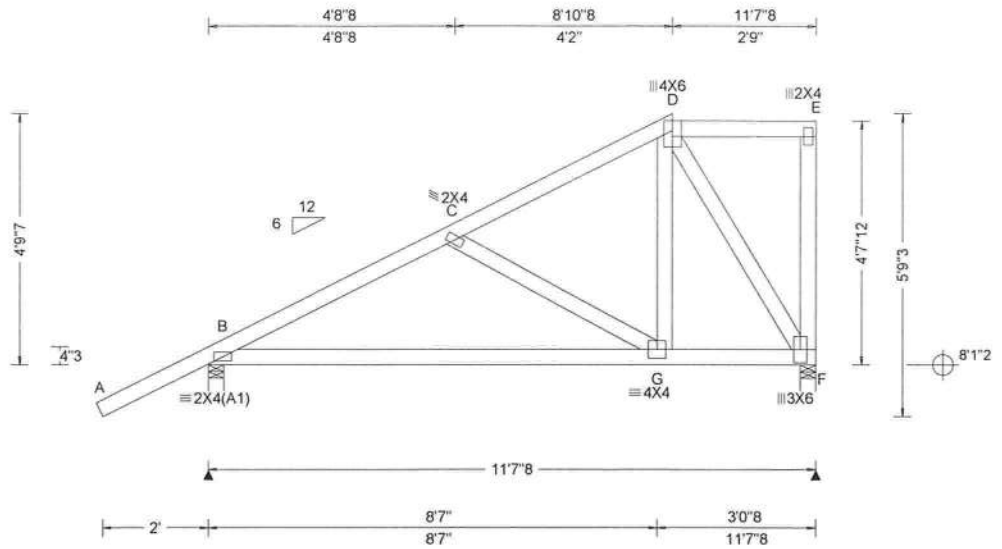
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199458 / HIPM FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** HT6	Cust: R 857 JRef:1XZG8570004 T17 / DrwNo: 121.24.1009.07000 SSB / WHK 04/30/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.009	C	999	360
VERT(CL): 0.019	B	999	240
HORZ(LL): 0.003	F	-	-
HORZ(TL): 0.010	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.205
Max BC CSI: 0.369
Max Web CSI: 0.293

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	582	/-	/-	/332	/34	/150
F	421	/-	/-	/235	/19	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Support)
F Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings B & F Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - C	140	-578

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - G	475	-233

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
D - F	136	-405

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes
Plates sized for a minimum of 3.50 sq.in./piece.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

COA #0778
04/30/2024
Florida Certificate of Product Approval #FL 1999

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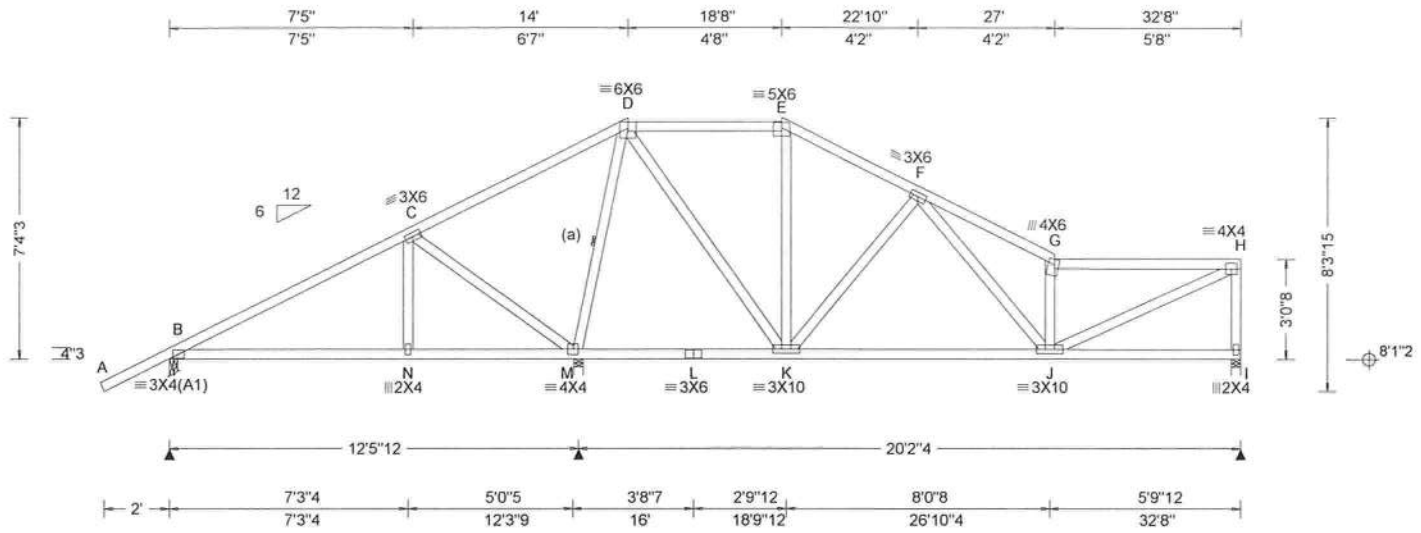
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199410 / SPEC FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** HT7	Cust: R 857 JRef: 1XZG8570004 T33 / DrwNo: 121.24.1009.07142 SSB / WHK 04/30/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.27 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.043	G	999	360
VERT(CL): 0.081	G	999	240
HORZ(LL): 0.010	D	-	-
HORZ(TL): 0.020	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.397
Max BC CSI: 0.352
Max Web CSI: 0.661

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	518	/-	/-	/273	/-	/176
M	1458	/-	/-	/738	/-	/-
I	685	/-	/-	/341	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.0 Min Req = 1.5 (Support)
M Brg Wid = 3.5 Min Req = 1.8 (Support)
I Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings B, M, & I Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	56	-385	F - G	272	-1178
C - D	415	-47	G - H	217	-1041
E - F	153	-433			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
K - J	626	-133

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - M	94	-598	F - J	594	-122
M - D	233	-1038	J - G	265	-796
D - K	672	-94	J - H	1135	-186
K - F	131	-447	H - I	173	-641

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Plating Notes
Plates sized for a minimum of 3.50 sq.in./piece.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

COA #6778
04/30/2024
Florida Certificate of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
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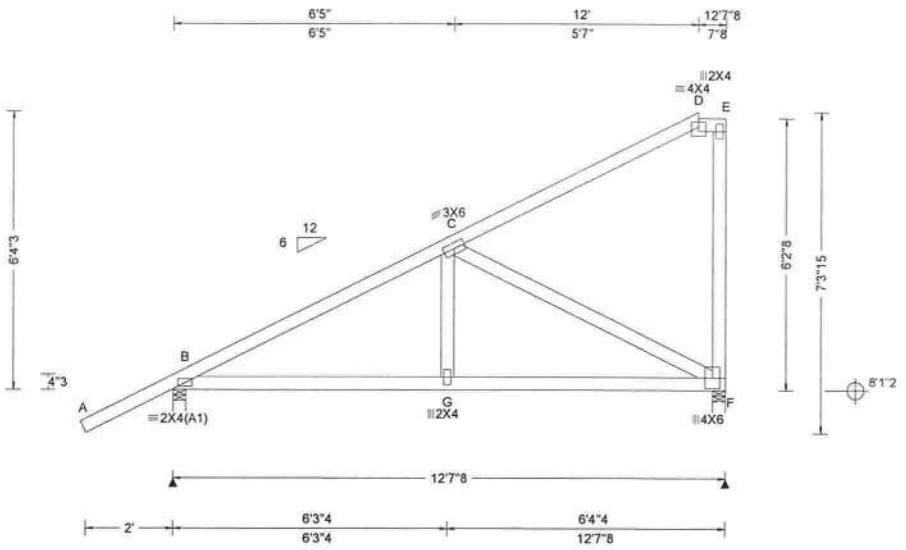
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199443 / HIPM FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** HT8	Cust: R 857 JRef: 1XZG8570004 T5 / DrwNo: 121.24.1009.06234 SSB / WHK 04/30/2024



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.032	D	999	360
VERT(CL): 0.058	D	999	240
HORZ(LL): 0.015	E	-	-
HORZ(TL): 0.028	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.428
Max BC CSI: 0.366
Max Web CSI: 0.547

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	619	/-	/-	/348	/32	/197
F	460	/-	/-	/285	/21	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Support)
F Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings B & F Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - C	84	-657

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - G	530	-197	G - F	527	-199

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
C - F	157	-592

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes
Plates sized for a minimum of 3.50 sq.in./piece.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

COA #0278
04/30/2024
Florida Certificate of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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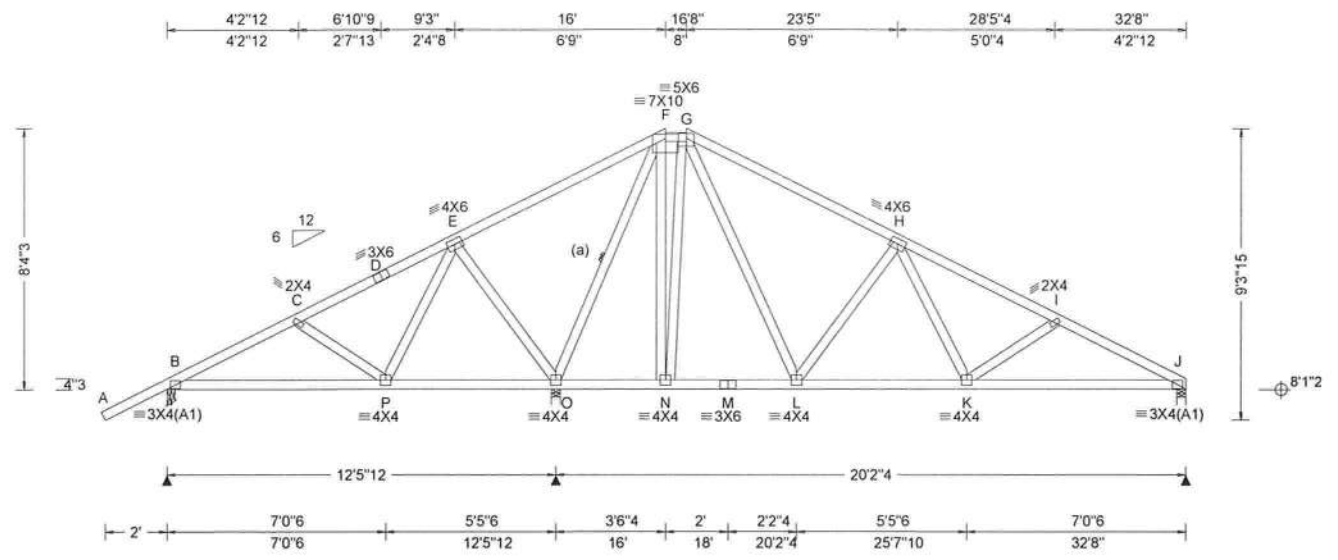
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199406 / HIPS FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** HT9		Cust: R 857 JRef: 1XZG8570004 T10 / DrwNo: 121.24.1009.06563 SSB / WHK 04/30/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.27 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.033	H	999	360
VERT(CL): 0.064	H	999	240
HORZ(LL): 0.009	J	-	-
HORZ(TL): 0.017	J	-	-

Creep Factor: 2.0
Max TC CSI: 0.440
Max BC CSI: 0.313
Max Web CSI: 0.919

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	488	/-	/-	/257	/-	/163
O	1484	/-	/-	/756	/-	/-
J	699	/-	/-	/396	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.0 Min Req = 1.5 (Support)
O Brg Wid = 3.5 Min Req = 1.9 (Support)
J Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings B, O, & J Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	11	-403	H - I	78	-987
E - F	460	0	I - J	98	-1182
G - H	115	-554			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
L - K	716	0	K - J	1020	-47

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
E - O	125	-539	N - G	188	-714
O - F	74	-1032	G - L	566	-42
F - N	773	-143	L - H	116	-519

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

COA #0278
04/30/2024
Florida Certificate of Product Approval #FL 1999

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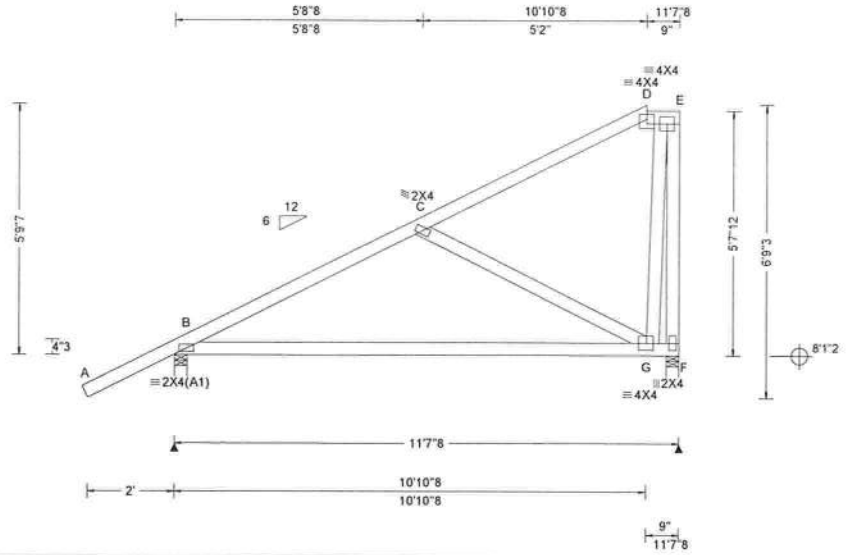
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199456 / HIPM FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** HT10	Cust: R 857 JRef: 1XZG8570004 T1 / DrwNo: 121.24.1009.05914 SSB / WHK 04/30/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 6.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/0(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.029 D 999 360 VERT(CL): 0.053 D 999 240 HORZ(LL): 0.015 E - - HORZ(TL): 0.027 E - - Creep Factor: 2.0 Max TC CSI: 0.381 Max BC CSI: 0.614 Max Web CSI: 0.448 VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	582	/-	/-	/329	/32	/180
F	421	/-	/-	/258	/19	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Support)
F Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings B & F Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - C	134	-535

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - G	436	-233

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - G	222	-409	E - F	13	-792
G - E	816	0			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

COA #0278

04/30/2024

Florida Certificate of Product Approval #FL 1999

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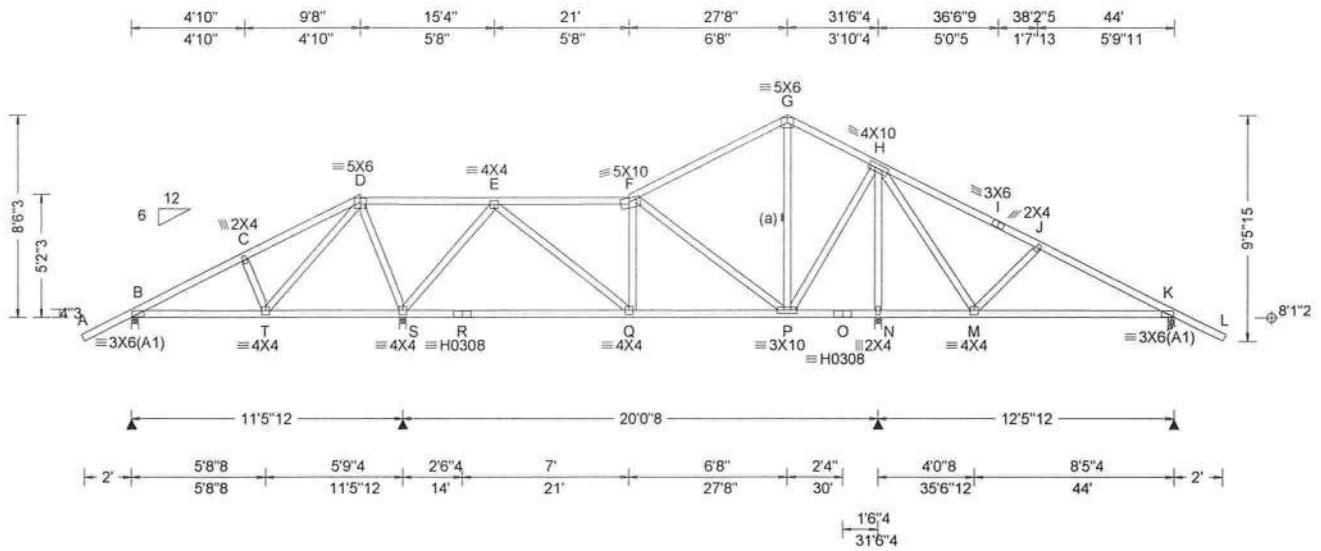
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199461 / SPEC FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** HT11	Cust: R 857 JRef: 1XZG8570004 T13 / DrwNo: 121.24.1009.06703 SSB / WHK 04/30/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.020 F 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.039 F 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.004 K - -
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.011 K - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.341
Load Duration: 1.25	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.426
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.962
	C&C Dist a: 4.40 ft	FT/RT:20(0)/0(0)	
	Loc. from endwall: not in 13.00 ft	Plate Type(s):	VIEW Ver: 23.02.01A.1204.18
	GCpi: 0.18	WAVE, HS	
	Wind Duration: 1.60		

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	437	/-	/-	/207	/-	/189
S	1455	/-	/-	/784	/-	/-
N	1298	/-	/-	/665	/-	/-
K	525	/-	/-	/320	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Support)
S Brg Wid = 3.5 Min Req = 1.8 (Support)
N Brg Wid = 3.5 Min Req = 1.6 (Support)
K Brg Wid = 3.0 Min Req = 1.5 (Support)
Bearings B, S, N, & K Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
D - E	494	-29	J - K	17	-437
E - F	141	-515			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
Q - P	513	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
T - D	438	-69	F - P	135	-468
D - S	147	-618	P - H	611	-50
S - E	257	-912	N - H	193	-1232
E - Q	554	-9	H - M	466	0

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

COA #0778

04/30/2024

Florida Certificate of Product Approval #FL 1999

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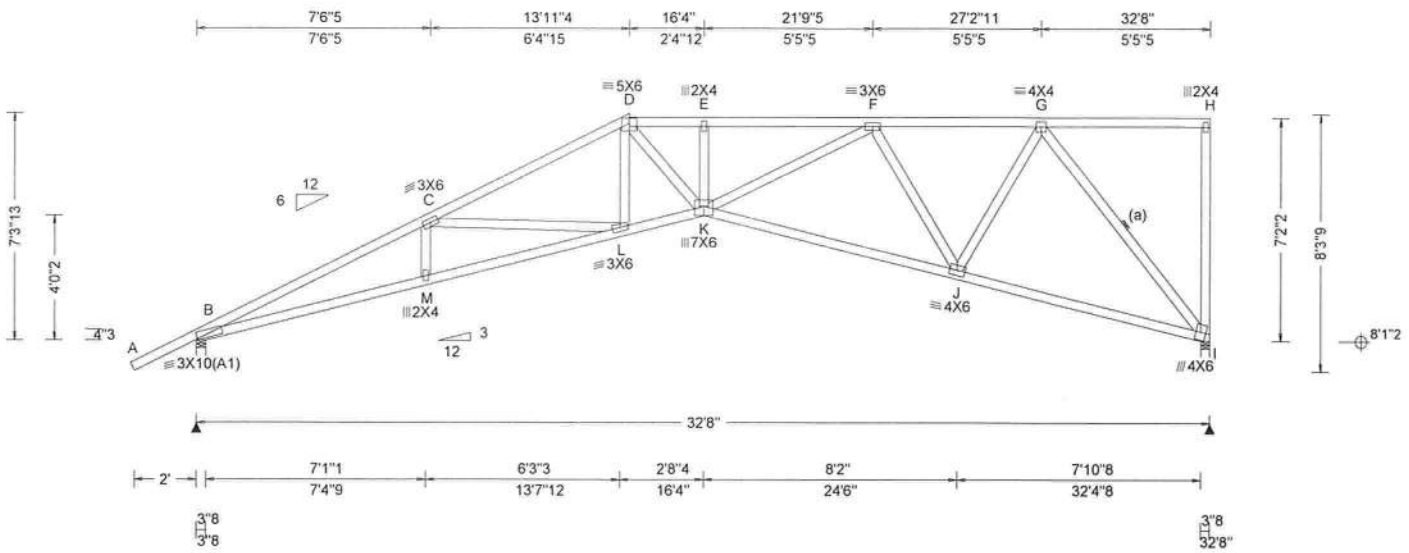
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199398 / COMN FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** HT12	Cust: R 857 JRef:1XZG8570004 T19 / DrwNo: 121.24.1009.06235 SSB / WHK 04/30/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.316 L 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.595 L 656 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.221 I - -
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.417 I - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.563
Load Duration: 1.25	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.831
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.745
	C&C Dist a: 3.27 ft	FT/RT:20(0)/0(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1380	/-	/-	/755	/-	/226
I	1236	/-	/-	/606	/54	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.7 (Support)
I Brg Wid = 3.5 Min Req = 1.6 (Support)
Bearings B & I Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	779	- 4066	E - F	980	- 3511
C - D	806	- 3250	F - G	384	- 1565
D - E	981	- 3512			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - M	3672	- 995	K - J	2219	- 759
M - L	3681	- 1004	J - I	1077	- 414
L - K	2912	- 934			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - L	62	- 706	F - J	454	- 1190
D - K	1088	- 339	J - G	1110	- 227
K - F	1553	- 448	G - I	525	- 1655

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 2x6 #3 or better "T" reinforcement. 80% length of web member. Attached with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

COA #6478
04/30/2024
Florida Certificate of Product Approval #FL 1999

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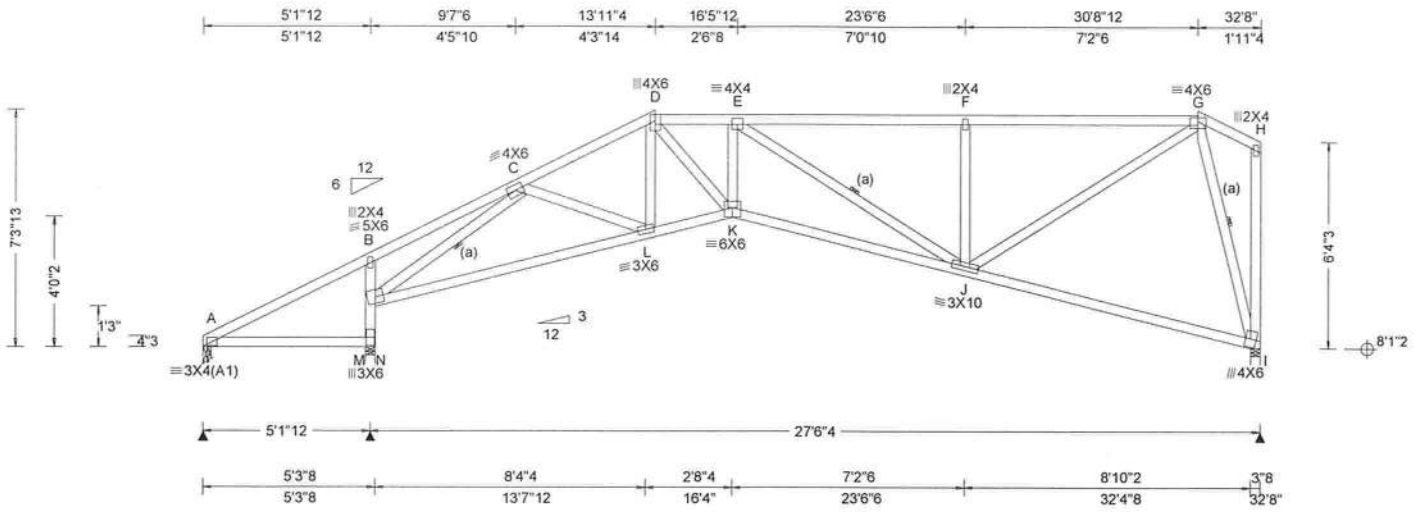
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199376 / COMN FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** HT13	Cust: R 857 JRef: 1XZG8570004 T11 / DrwNo: 121.24.1009.06734 SSB / WHK 04/30/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.133 K 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.254 K 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.088 I - -
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.168 I - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.512
Load Duration: 1.25	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.744
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.974
	C&C Dist a: 3.27 ft	FT/RT:20(0)/0(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	166	/-	/-	/147	/14	/203
N	1306	/-	/-	/752	/57	/-
I	1041	/-	/-	/482	/14	/-

Wind reactions based on MWFRS
A Brg Wid = 3.0 Min Req = 1.5 (Support)
N Brg Wid = 3.5 Min Req = 1.6 (Support)
I Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings A, N, & I Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
C - D	481	- 2024	E - F	425	- 1463
D - E	710	- 2410	F - G	429	- 1469

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
M - L	1574	- 538	K - J	2477	- 849
L - K	1822	- 602			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
M - N	348	- 1250	J - F	298	- 481
M - C	661	- 2024	J - G	1428	- 403
D - K	1002	- 375	G - I	371	- 1016
E - J	378	- 1115			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

Shim all supports to solid bearing.

COA #0278
04/30/2024
Florida Certificate of Product Approval #FL 1999

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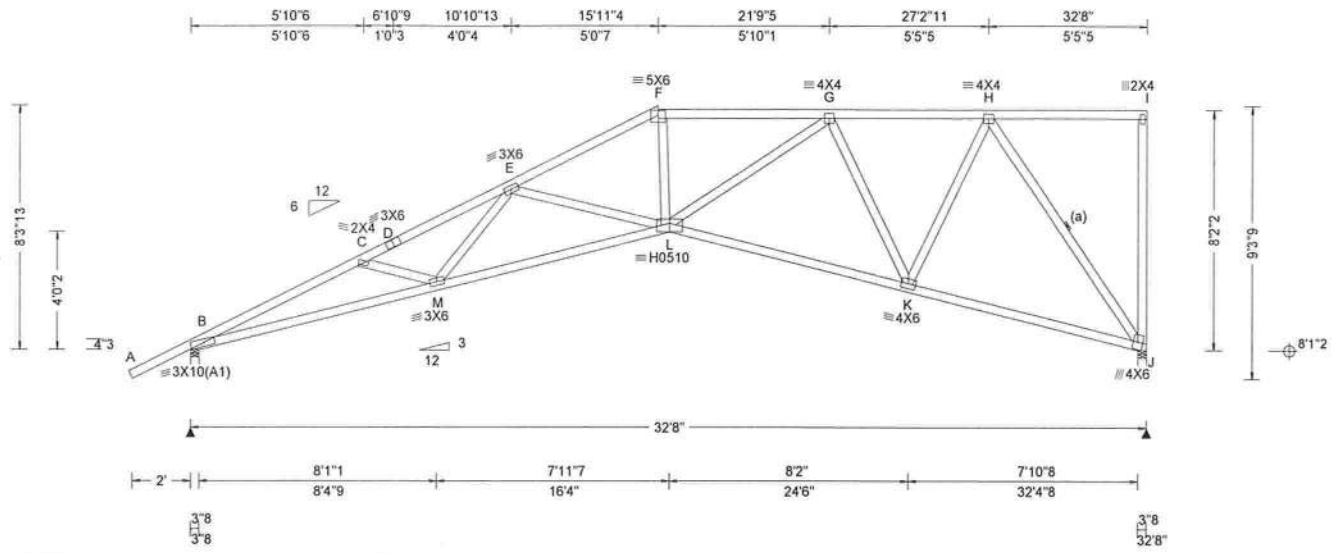
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199396 / COMN FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** HT14	Cust: R 857 JRef: 1XZG8570004 T35 / DrwNo: 121.24.1009.06923 SSB / WHK 04/30/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 6.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.27 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/0(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.293 E 999 360 VERT(CL): 0.552 E 707 240 HORZ(LL): 0.195 J - - HORZ(TL): 0.367 J - - Creep Factor: 2.0 Max TC CSI: 0.609 Max BC CSI: 0.769 Max Web CSI: 0.939 VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1380	/-	/-	/762	/-	/257
J	1236	/-	/-	/619	/55	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.7 (Support)
J Brg Wid = 3.5 Min Req = 1.6 (Support)
Bearings B & J Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	715	-4107	E - F	701	-2835
C - D	654	-3812	F - G	681	-2588
D - E	671	-3785	G - H	324	-1297

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - M	3718	-991	L - K	1802	-638
M - L	3255	-961	K - J	916	-373

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
M - E	505	0	G - K	401	-1045
E - L	99	-667	K - H	1022	-204
F - L	951	-153	H - J	484	-1530
L - G	1050	-286			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 2x6 #3 or better "T" reinforcement. 80% length of web member. Attached with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

COA #0278

04/30/2024

Florida Certificate of Product Approval #FL 1999

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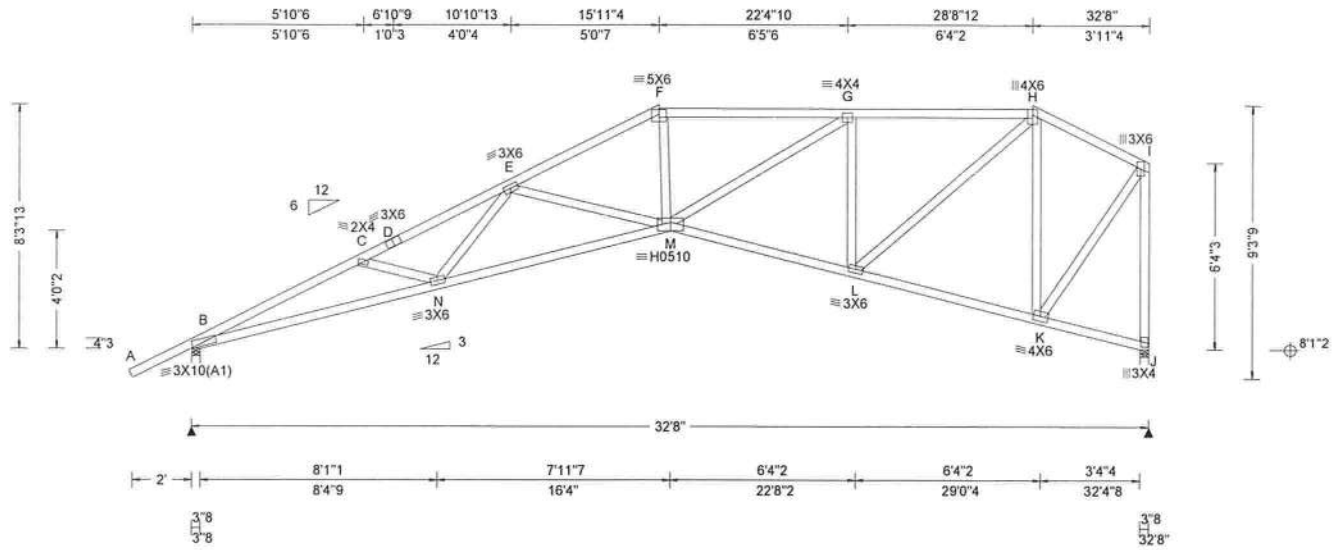
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199379 / COMN FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** HT15	Cust: R 857 JRef:1XZG8570004 T31 / DrwNo: 121.24.1009.06565 SSB / WHK 04/30/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.296 E 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.558 E 699 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.192 J - -
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.361 J - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.610
Load Duration: 1.25	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.768
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.897
	C&C Dist a: 3.27 ft	FT/RT:20(0)/0(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE, HS	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1380	/-	/-	/760	/-	/236
J	1236	/-	/-	/593	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.7 (Support)
J Brg Wid = 3.5 Min Req = 1.6 (Support)
Bearings B & J Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	699	- 4108	F - G	663	- 2586
C - D	636	- 3813	G - H	424	- 1613
D - E	653	- 3786	H - I	229	- 700
E - F	684	- 2833			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - N	3720	- 892	M - L	1727	- 546
N - M	3253	- 860	L - K	666	- 247

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
N - E	512	0	L - H	1322	- 365
E - M	97	- 666	H - K	333	- 884
F - M	938	- 138	K - I	1035	- 272
M - G	1098	- 262	I - J	294	- 1213
G - L	398	- 1017			

COA #6278

04/30/2024

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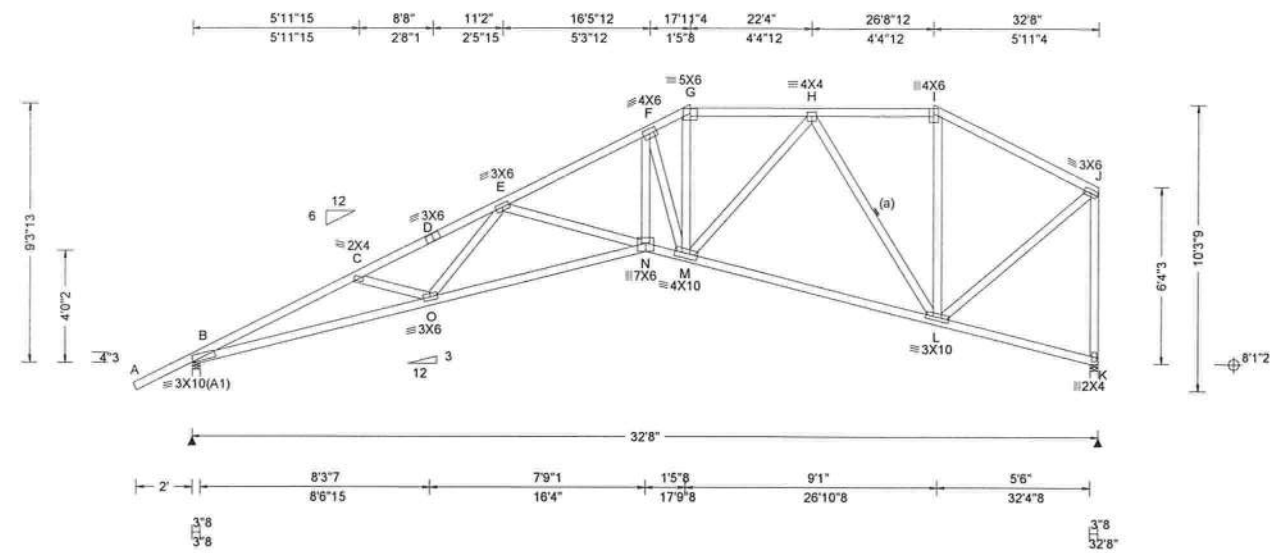
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199382 / COMN FROM: RNB	Ply: 1 Qty: 2	**Job Number:** B59010a Jones Residence **Truss Label:** HT16	Cust: R 857 JRef: 1XZG8570004 T29 / DrwNo: 121.24.1009.06119 SSB / WHK 04/30/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.296 E 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.557 E 700 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.193 K - -
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.363 K - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.611
Load Duration: 1.25	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.766
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.976
	C&C Dist a: 3.27 ft	FT/RT:20(0)/0(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1380	/-	/-	/763	/-	/253
K	1236	/-	/-	/596	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.7 (Support)
K Brg Wid = 3.5 Min Req = 1.6 (Support)
Bearings B & K Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	604	-4102	F - G	488	-2160
C - D	544	-3798	G - H	449	-1926
D - E	554	-3730	H - I	251	-821
E - F	572	-2769	I - J	230	-975

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - O	3715	-805	N - M	2441	-640
O - N	3227	-762	M - L	1428	-434

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
O - E	528	0	M - H	852	-143
E - N	112	-732	H - L	322	-1054
N - F	1615	-343	L - J	1050	-230
F - M	381	-1531	J - K	254	-1196
M - G	750	-118			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

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04/30/2024

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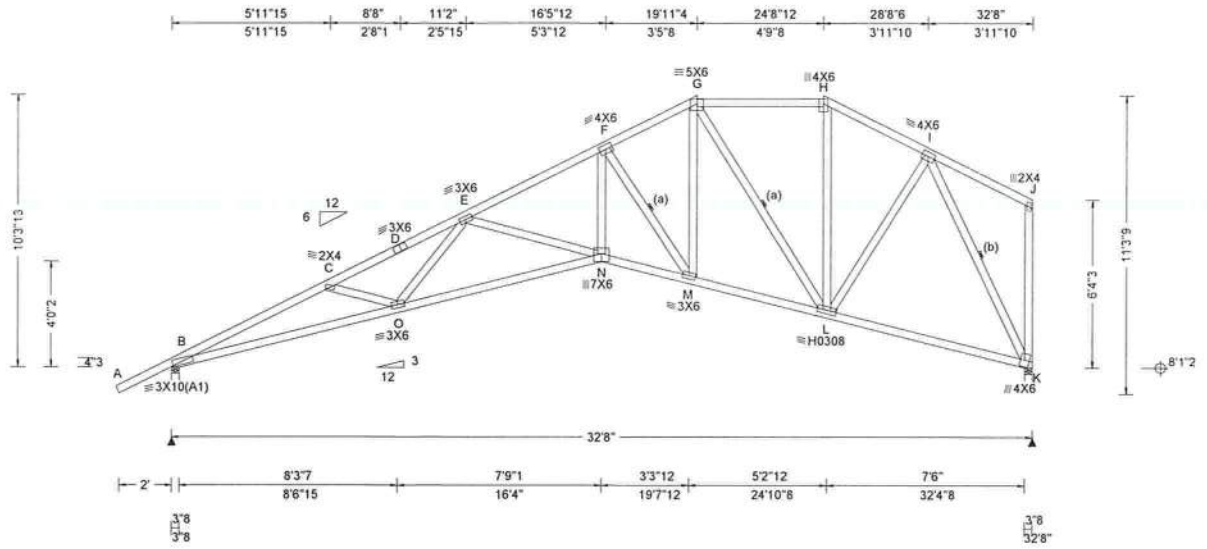
Glenview, IL 60025

SEQN: 199391 / COMN FROM: RNB	Ply: 1 Qty: 3	**Job Number:** B59010a Jones Residence **Truss Label:** HT18	Cust: R 857 JRef: 1XZG8570004 T8 / DrwNo: 121.24.1009.05993 SSB / WHK 04/30/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.27 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.301	E	999	360
VERT(CL): 0.566	E	689	240
HORZ(LL): 0.200	K	-	-
HORZ(TL): 0.376	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.609
Max BC CSI: 0.767
Max Web CSI: 0.961

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1380	/-	/-	/763	/-	/271
K	1236	/-	/-	/601	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.7 (Support)
K Brg Wid = 3.5 Min Req = 1.6 (Support)
Bearings B & K Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	468	-4102	F - G	309	-1672
C - D	404	-3796	G - H	223	-938
D - E	414	-3728	H - I	217	-1093
E - F	426	-2774			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - O	3714	-665	M - L	1458	-332
O - N	3226	-611	L - K	637	-189
N - M	2469	-488			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
O - E	526	0	M - G	1252	-168
E - N	111	-725	G - L	188	-904
N - F	1716	-224	L - I	637	-72
F - M	282	-1711	I - K	260	-1367

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

(b) Continuous lateral restraint equally spaced on member. Or 2x6 #3 or better "T" reinforcement. 80% length of web member. Attached with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

COA #6278
04/30/2024
Florida Certificate of Product Approval #FL 1999

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****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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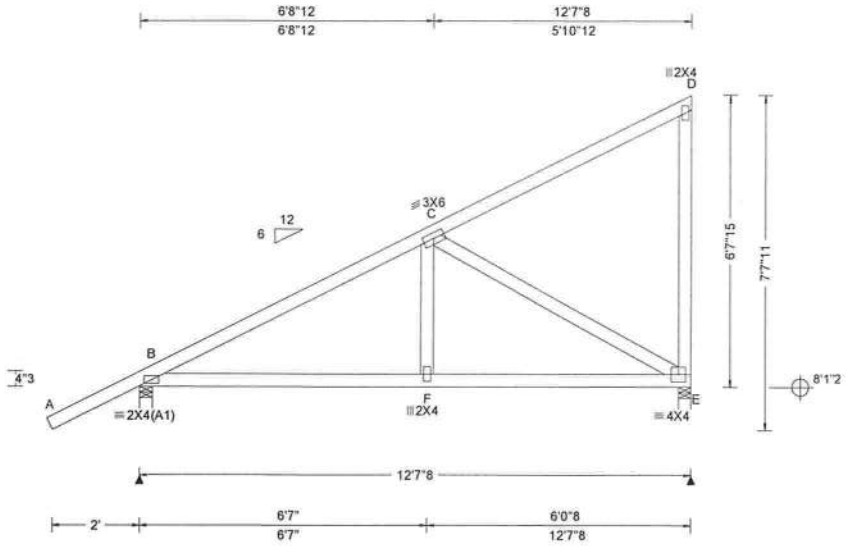
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199429 / MONO FROM: RNB	Ply: 1 Qty: 11	**Job Number:** B59010a Jones Residence **Truss Label:** M1	Cust: R 857 JRef: 1XZG8570004 T6 / DrwNo: 121.24.1009.06467 SSB / WHK 04/30/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.012	F	999	360
VERT(CL): 0.021	F	999	240
HORZ(LL): 0.005	E	-	-
HORZ(TL): 0.008	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.404
Max BC CSI: 0.359
Max Web CSI: 0.602

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity		Non-Gravity		
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL
B	619 / -	/ -	/346	/ -	/207
E	460 / -	/ -	/294	/22	/ -

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Support)
E Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings B & E Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - C	85	-635

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - F	506	-188	F - E	503	-190

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
C - E	146	-577

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes
Plates sized for a minimum of 3.50 sq.in./piece.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

COA #6778
04/30/2024
Florida Certificate of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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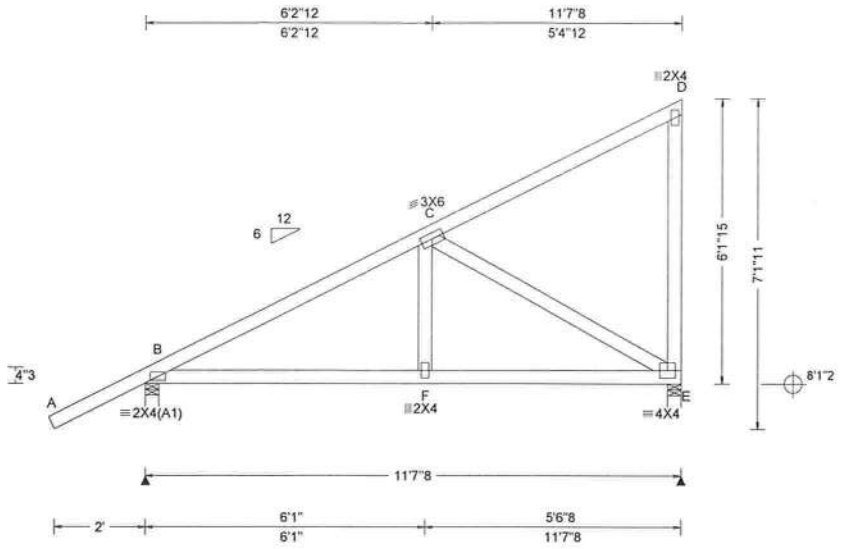
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199437 / MONO FROM: RNB	Ply: 1 Qty: 11	**Job Number:** B59010a Jones Residence **Truss Label:** M2	Cust: R 857 JRef: 1XZG8570004 T25 / DrwNo: 121.24.1009.06403 SSB / WHK 04/30/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.010	F	999	360
VERT(CL): 0.018	F	999	240
HORZ(LL): 0.004	E	-	-
HORZ(TL): 0.007	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.328
Max BC CSI: 0.300
Max Web CSI: 0.509

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	582	/-	/-	/326	/32	/192
E	421	/-	/-	/269	/20	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Support)
E Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings B & E Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - C	78	-575

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - F	456	-178	F - E	453	-180

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
C - E	139	-522

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes
Plates sized for a minimum of 3.50 sq.in./piece.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

COA #0278
04/30/2024
Florida Certificate of Product Approval #FL 1999

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****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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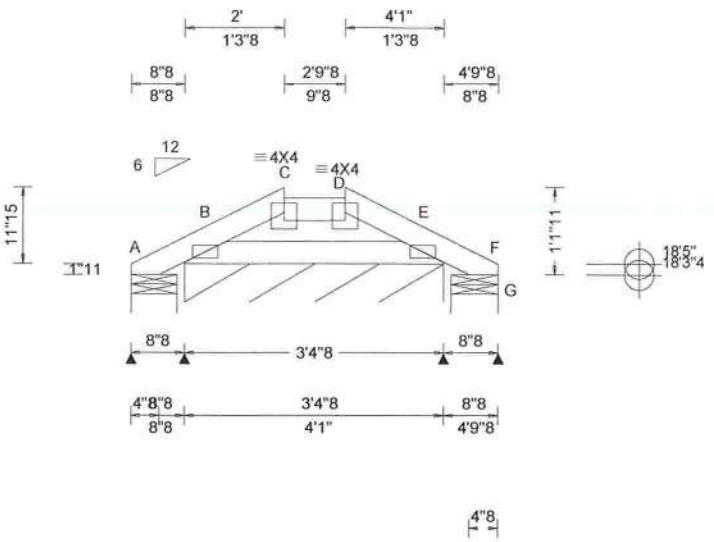
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199393 / COMN FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** PB1	Cust: R 857 JRef: 1XZG8570004 T9 / DrwNo: 121.24.1009.07002 SSB / WHK 04/30/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 18.91 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.001	C	999	360
VERT(CL): 0.001	B	999	240
HORZ(LL): 0.000	E	-	-
HORZ(TL): 0.000	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.013
Max BC CSI: 0.042
Max Web CSI: 0.000

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	14	/-2	/-	/7	/8	/17
B*	89	/-	/-	/43	/-	/-
G	14	/-2	/-	/4	/5	/-

Wind reactions based on MWFRS
A Brg Wid = 7.3 Min Req = 1.5 (Truss)
B Brg Wid = 40.5 Min Req = -
G Brg Wid = 7.3 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;

Plating Notes

All plates are 2X4(A1) except as noted.

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Refer to DWG PB160220723 for piggyback details.

COA #6178
04/30/2024
Florida Certificate of Product Approval #FL 1999

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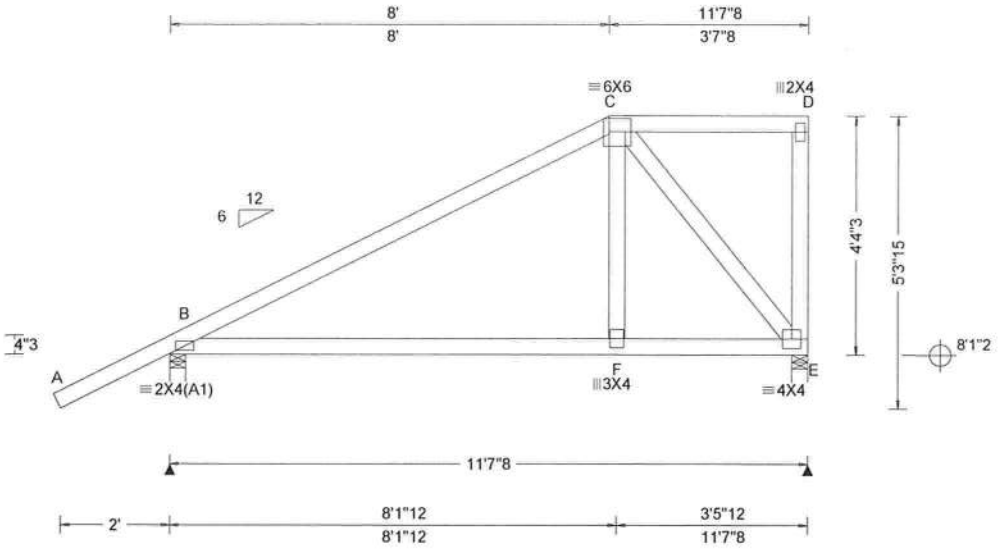
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199452 / SPEC FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** SGT1	Cust: R 857 JRef: 1XZG8570004 T18 / DrwNo: 121.24.1009.07532 SSB / WHK 04/30/2024



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.019	B	999	360
VERT(CL): 0.031	B	999	240
HORZ(LL): 0.011	B	-	-
HORZ(TL): 0.021	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.761
Max BC CSI: 0.495
Max Web CSI: 0.681

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity		Non-Gravity		
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL
B	870 /-	/-	/-	/87	/82
E	1187 /-	/-	/-	/69	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Support)
E Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings B & E Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - C	110	- 1048

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - F	842	0	F - E	870	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - F	793	0	C - E	43	- 1305

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	56 plf at	-2.13 to	56 plf at	8.00
TC: From	28 plf at	8.00 to	28 plf at	11.63
BC: From	4 plf at	-2.13 to	4 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	8.06
BC: From	10 plf at	8.06 to	10 plf at	11.63

TC: 295 lb Conc. Load at 8.06
TC: 190 lb Conc. Load at 10.13
BC: 560 lb Conc. Load at 8.06
BC: 144 lb Conc. Load at 10.13

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

COA #0278
04/30/2024
Florida Certificate of Product Approval #FL 1999

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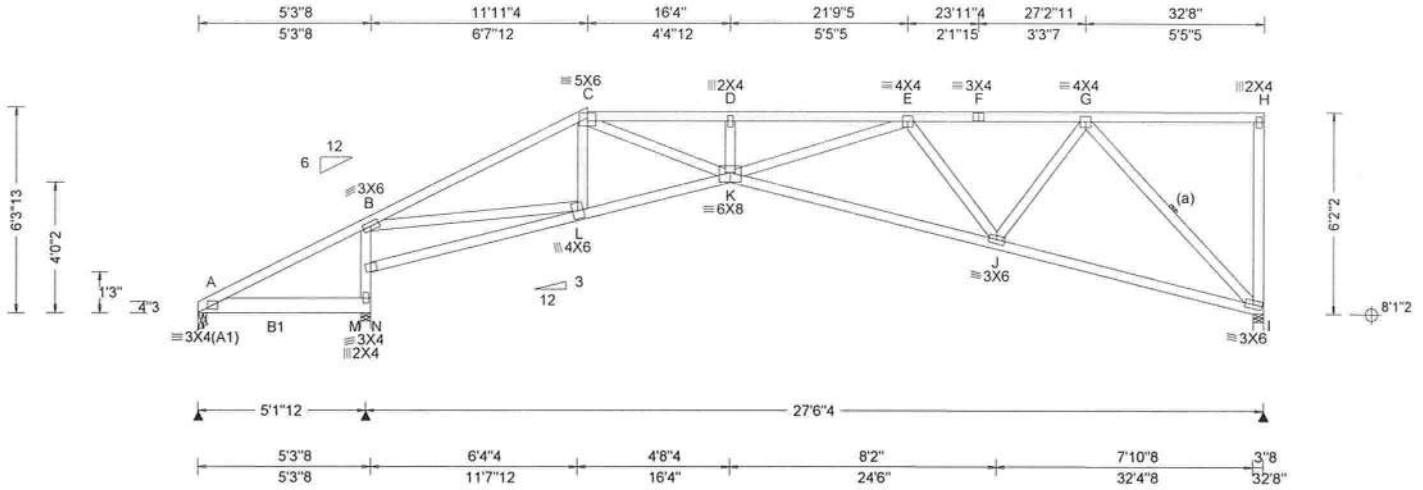
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199373 / COMN FROM: RNB	Ply: 1 Qty: 1	**Job Number:** B59010a Jones Residence **Truss Label:** SGT2	Cust: R 857 JRef: 1XZG8570004 T7 / DrwNo: 121.24.1009.05987 SSB / WHK 04/30/2024	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.219 D 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.408 D 808 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.120 I - -
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.223 I - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.650
Load Duration: 1.25	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.680
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Varies by Ld Case	Max Web CSI: 0.937
	C&C Dist a: 3.27 ft	FT/RT:20(0)/0(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	469	/-	/-	/-	/84	/118
N	1895	/-	/-	/-	/21	/-
I	1022	/-	/-	/-	/26	/-

Wind reactions based on MWFRS
A Brg Wid = 3.0 Min Req = 1.5 (Support)
N Brg Wid = 3.5 Min Req = 2.4 (Support)
I Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings A, N, & I Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	50	- 1892	E - F	37	- 1518
C - D	5	- 3517	F - G	37	- 1518
D - E	4	- 3515			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
L - K	1701	0	J - I	1045	0
K - J	2158	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
M - N	17	- 1396	K - E	1520	0
M - B	50	- 1259	E - J	66	- 988
B - L	1893	0	J - G	915	0
C - K	2049	0	G - I	34	- 1466

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B1 2x6 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 2x4 #3 or better "T" reinforcement. 80% length of web member. Attached with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	56 plf at	0.00 to	56 plf at	32.67	
BC: From	10 plf at	0.00 to	10 plf at	5.29	
BC: From	21 plf at	5.29 to	21 plf at	32.67	

BC: 435 lb Conc. Load at 2.00, 4.00

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

Shim all supports to solid bearing.

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data,including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.

COA #0278

04/30/2024

Florida Certificate of Product Approval #FL 1999

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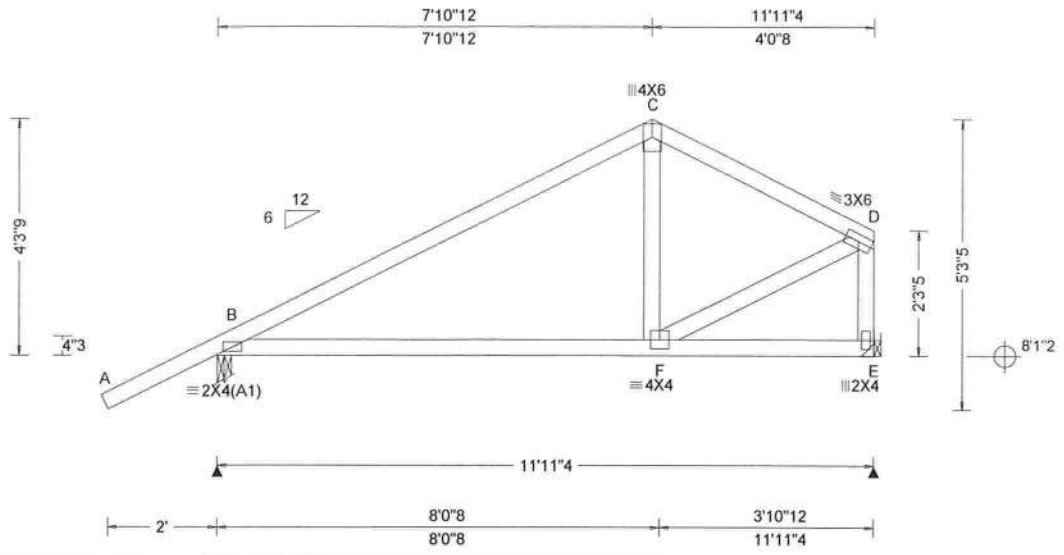
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199363 / COMN FROM: RNB	Ply: 1 Qty: 2	**Job Number:** B59010a Jones Residence **Truss Label:** T2	Cust: R 857 JRef: 1XZG8570004 T15 / DrwNo: 121.24.1009.07001 SSB / WHK 04/30/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.016	B	999	360
VERT(CL): 0.027	B	999	240
HORZ(LL): 0.009	B	-	-
HORZ(TL): 0.017	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.423
Max BC CSI: 0.359
Max Web CSI: 0.155

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	593	/-	/-	/333	/34	/82
E	435	/-	/-	/212	/14	/-

Wind reactions based on MWFRS
B Brg Wid = 3.0 Min Req = 1.5 (Support)
E Brg Wid = - Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	151	-496	C - D	155	-435

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
F - D	408	-112	D - E	161	-427

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bearing at location x=11'8"4 uses the following support conditions: 11'8"4
Bearing E (11'8"4, 8'1"2) HUS26
Supporting Member: (1)2x6 SP #1
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

COA #0278
04/30/2024
Florida Certificate of Product Approval #FL 1999

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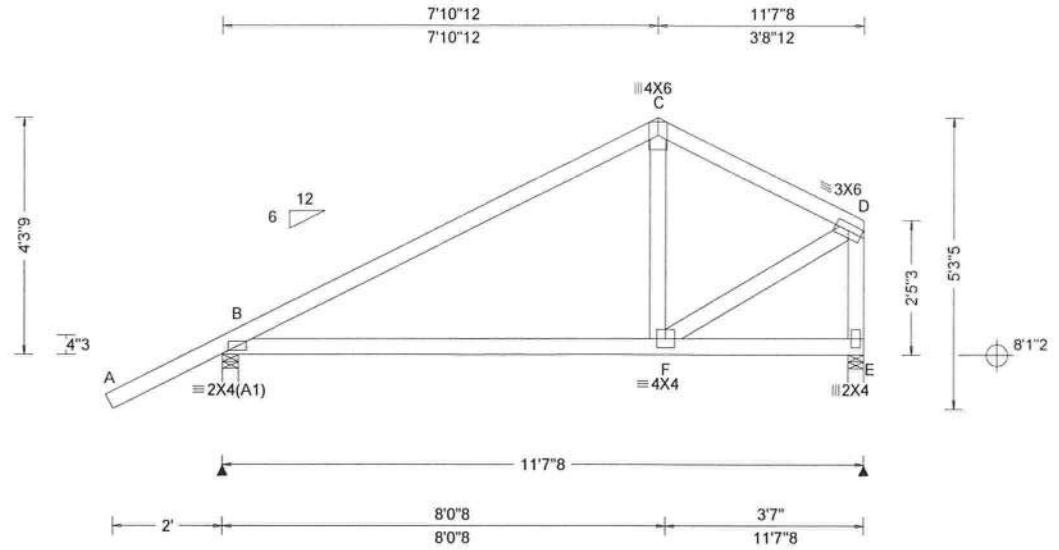
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 199366 / COMN FROM: RNB	Ply: 1 Qty: 2	**Job Number:** B59010a Jones Residence **Truss Label:** T3	Cust: R 857 JRef: 1XZG8570004 T12 DrwNo: 121.24.1009.06516 SSB / WHK 04/30/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.015	B	999	360
VERT(CL): 0.026	B	999	240
HORZ(LL): 0.009	B	-	-
HORZ(TL): 0.016	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.419
Max BC CSI: 0.355
Max Web CSI: 0.151

VIEW Ver: 23.02.01A.1204.18

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	582	/-	/-	/326	/38	/115
E	421	/-	/-	/209	/10	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Support)
E Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings B & E Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	163	-470	C - D	170	-402

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
F - D	395	-134	D - E	157	-420

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes
Plates sized for a minimum of 3.50 sq.in./piece.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

COA #0778
04/30/2024
Florida Certificate of Product Approval #FL 1999

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH,GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

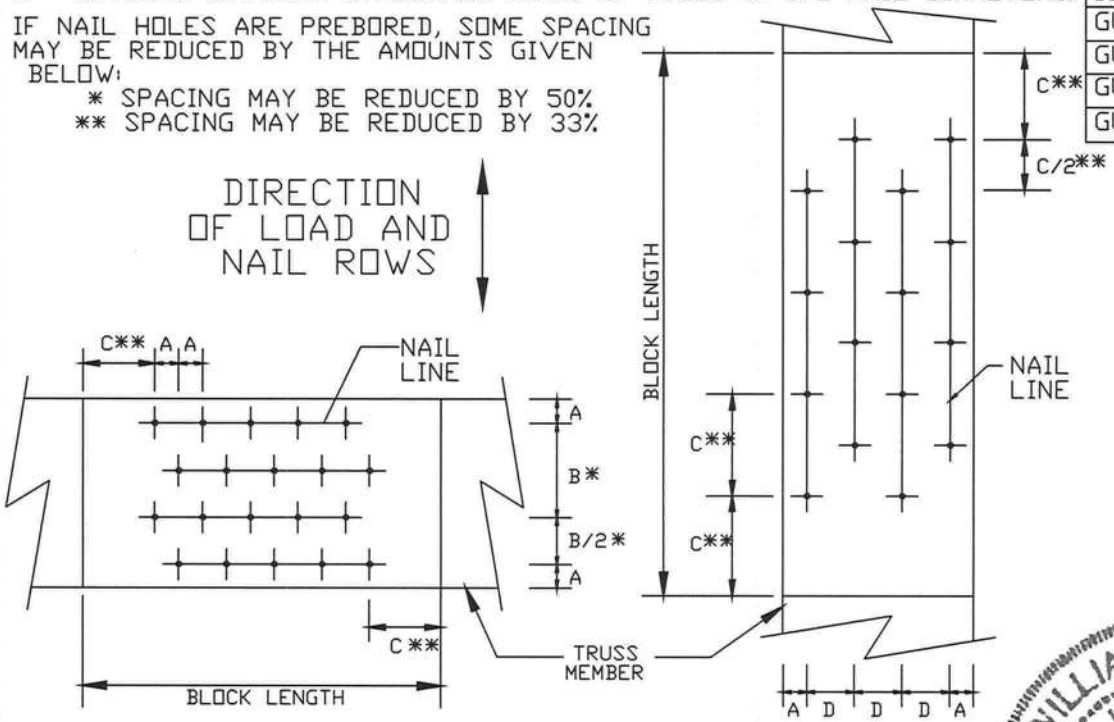
LOAD PERPENDICULAR TO GRAIN
A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN
A - EDGE DISTANCE (6 NAIL DIAMETERS)
C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:
* SPACING MAY BE REDUCED BY 50%
** SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3.",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1'	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3.",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3.",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN LOAD APPLIED PARALLEL TO GRAIN

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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COA #0 278
Florida Certificate of Product Approval #FL 1999

04/30/2024

REF	NAIL SPACE
DATE	10/01/14
DRWG	CNNAILSP1014

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

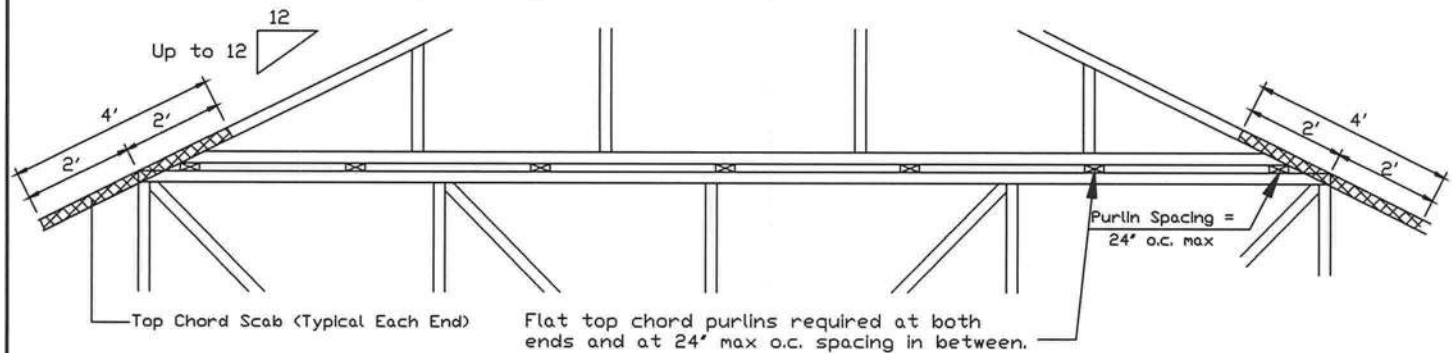
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

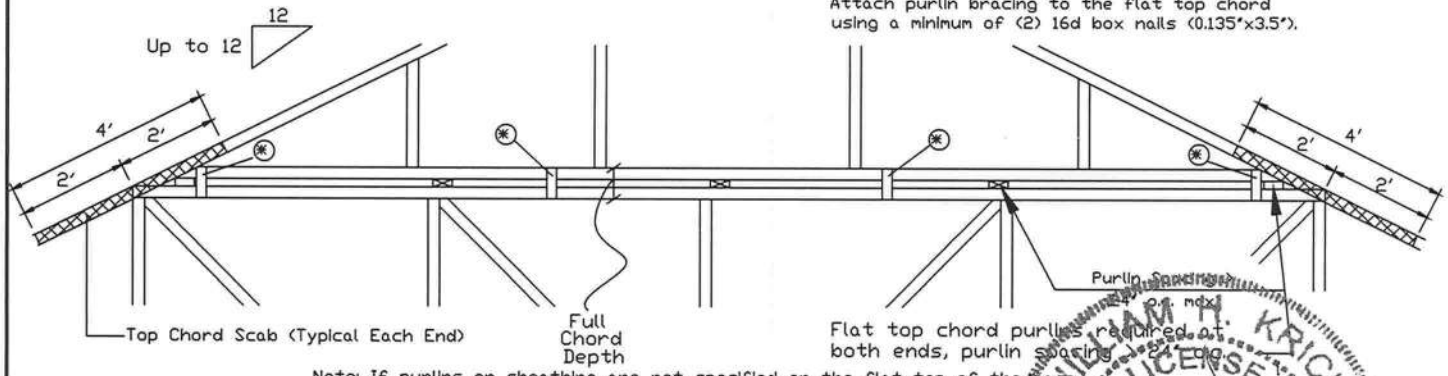
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the above truss, purlins must be installed at 24" o.c. max. and use Detail A

* In addition, provide connection with one of the following methods:

Trulox
Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gusset
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

28PB Wave Piggyback Plate
One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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WILLIAM H. KRICK
LICENSE
No. 70861
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

04/30/2024

COA #0 278
Florida Certificate of Product Approval #FL 1999

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118

SPACING- 24.0"

Piggyback Detail - ASCE 7-16: 180 mph, 30' Mean Hgt, Partially Enclosed, Exp. C, Kzt=1.00

180 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Part. Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 160 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Part. Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

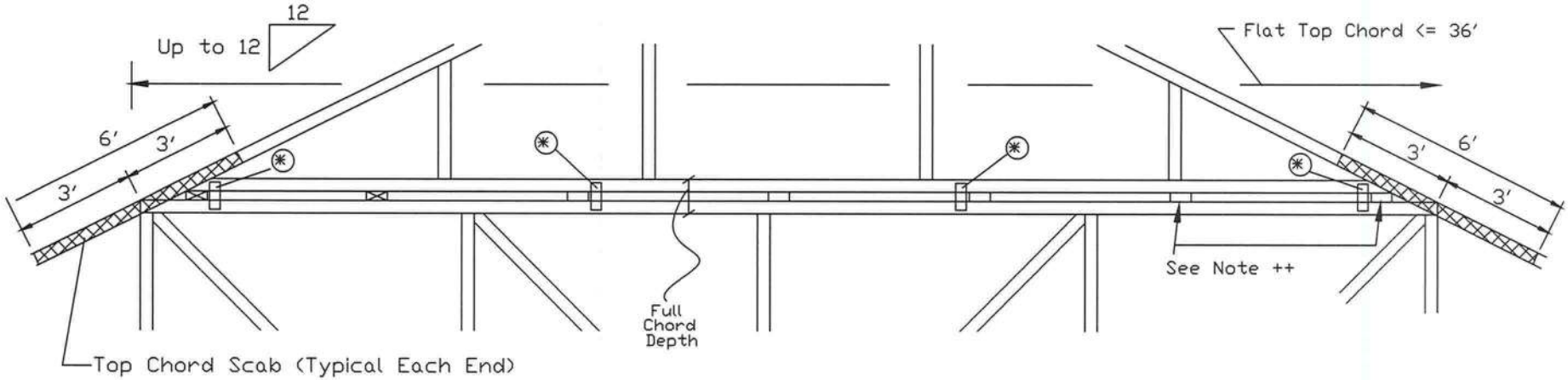
Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

++ Flat top chord purlins required at both ends and at a maximum of 24" intervals unless otherwise noted on base truss design drawing. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



* In addition, provide connection with one of the following methods:	
Trulox Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.	28PB Wave Piggyback Plate One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.
APA Rated Gusset 8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.	2x4 Vertical Scabs 2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/30/2024

COA #0 278
Florida Certificate of Product Approval #FL 1999

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB180160118
SPACING	24.0"

Cracked or Broken Member Repair Detail

This drawing specifies repairs for a truss with broken chord or web member.

This design is valid only for single ply trusses with 2x4 or 2x6 broken members. No more than one break per chord panel and no more than two breaks per truss are allowed. Contact the truss manufacturer for any repairs that do not comply with this detail.

(B) = Damaged area, 12" max length of damaged section
(L) = Minimum nailing distance on each side of damaged area (B)
(S) = Two 2x4 or two 2x6 side members, same size, grade, and species as damaged member. Apply one scab per face. Minimum side member length(s) = (2)(L) + (B)

Scab member length (S) must be within the broken panel.

Nail into 2x4 members using two (2) rows at 4" o.c., rows staggered.
Nail into 2x6 members using three (3) rows at 4" o.c., rows staggered.

Nail using 10d box or gun nails (0.128"x3", min) into each side member.

The maximum permitted lumber grade for use with this detail is limited to Visual grade #1 and MSR grade 1650f.

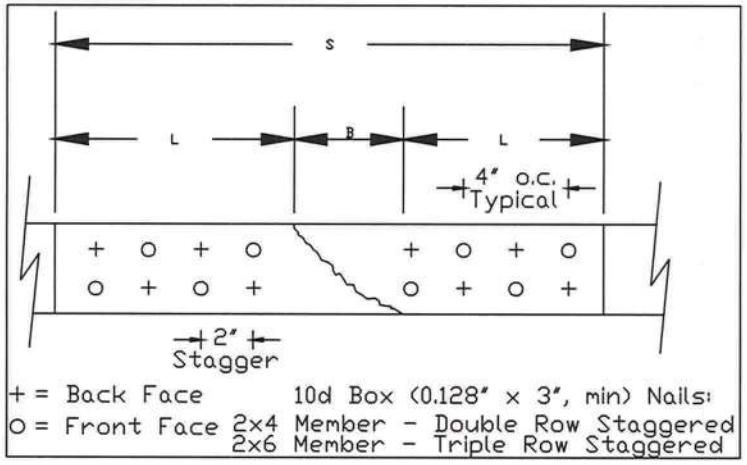
This repair detail may be used for broken connector plate at mid-panel splices.

This repair detail may not be used for damaged chord or web sections occurring within the connector plate area.

Broken chord may not support any tie-in loads.

Load Duration = 0%
Member forces may be increased for Duration of Load

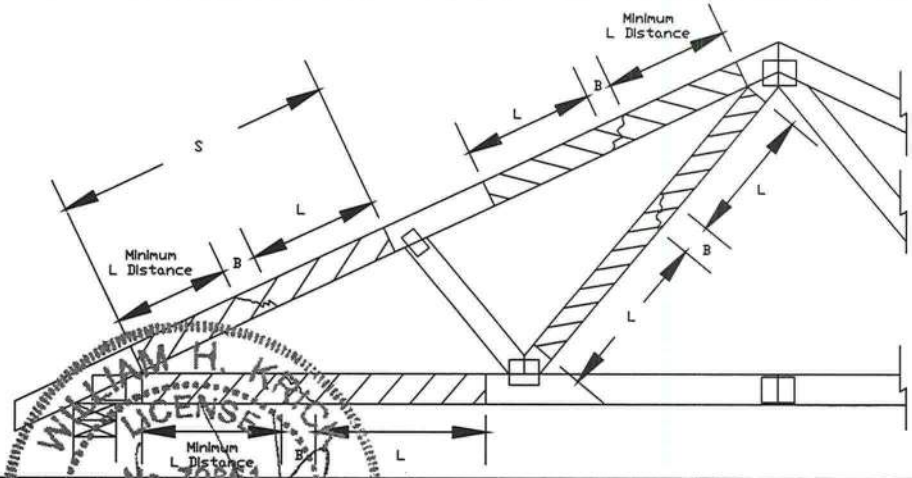
Member	Size	L	Maximum Member Axial Force SPF-C	HF	DF-L	SYP
Web Only	2x4	12"	620#	635#	730#	800#
Web Only	2x4	18"	975#	1055#	1295#	1415#
Web or Chord	2x4	24"	975#	1055#	1495#	1745#
Web or Chord	2x6		1465#	1585#	2245#	2620#
Web or Chord	2x4	30"	1910#	1960#	2315#	2555#
Web or Chord	2x6		2230#	2365#	3125#	3575#
Web or Chord	2x4	36"	2470#	2530#	2930#	3210#
Web or Chord	2x6		3535#	3635#	4295#	4745#
Web or Chord	2x4	42"	2975#	3045#	3505#	3835#
Web or Chord	2x6		4395#	4500#	5225#	5725#
Web or Chord	2x4	48"	3460#	3540#	4070#	4445#
Web or Chord	2x6		5165#	5280#	6095#	6660#



+ = Back Face
O = Front Face

10d Box (0.128" x 3", min) Nails:
2x4 Member - Double Row Staggered
2x6 Member - Triple Row Staggered

Nail Spacing Detail



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COA #0 278
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04/30/2024

SPACING 24.0" MAX

REF	MEMBER REPAIR
DATE	10/01/14
DRWG	REPCHRD1014