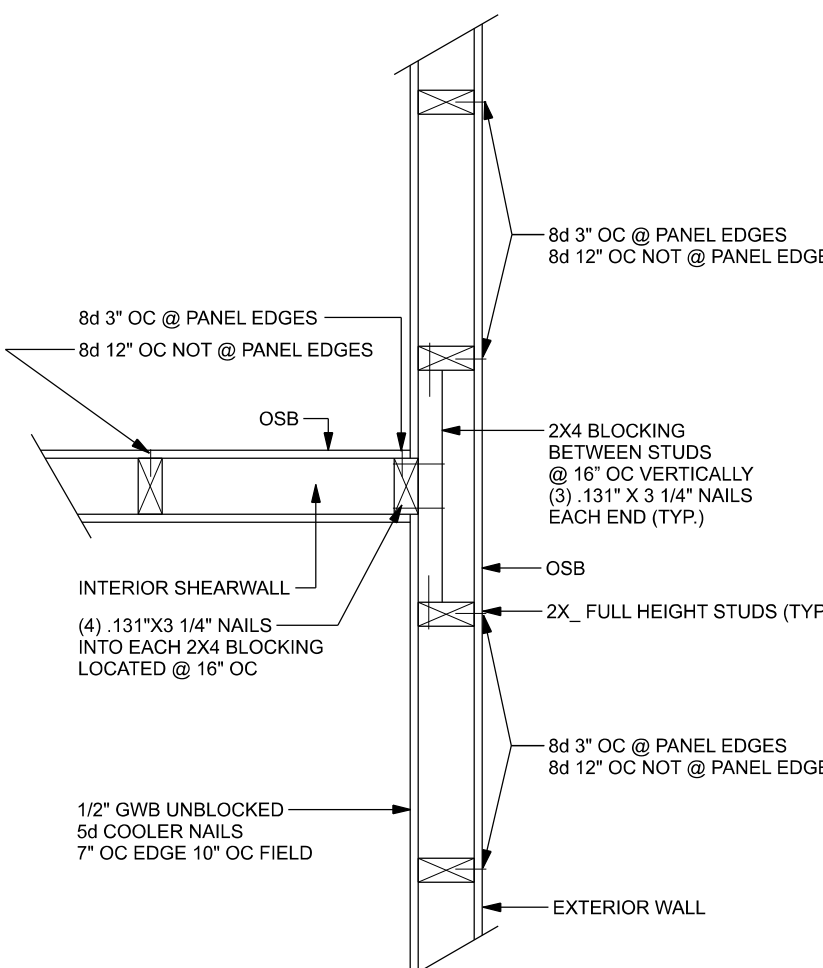
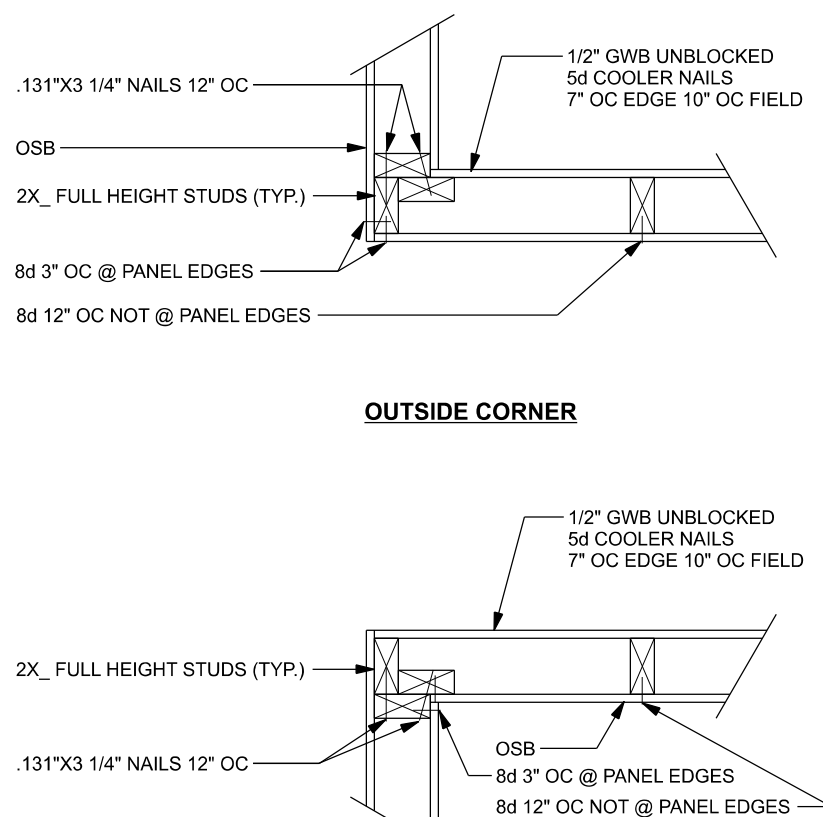


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



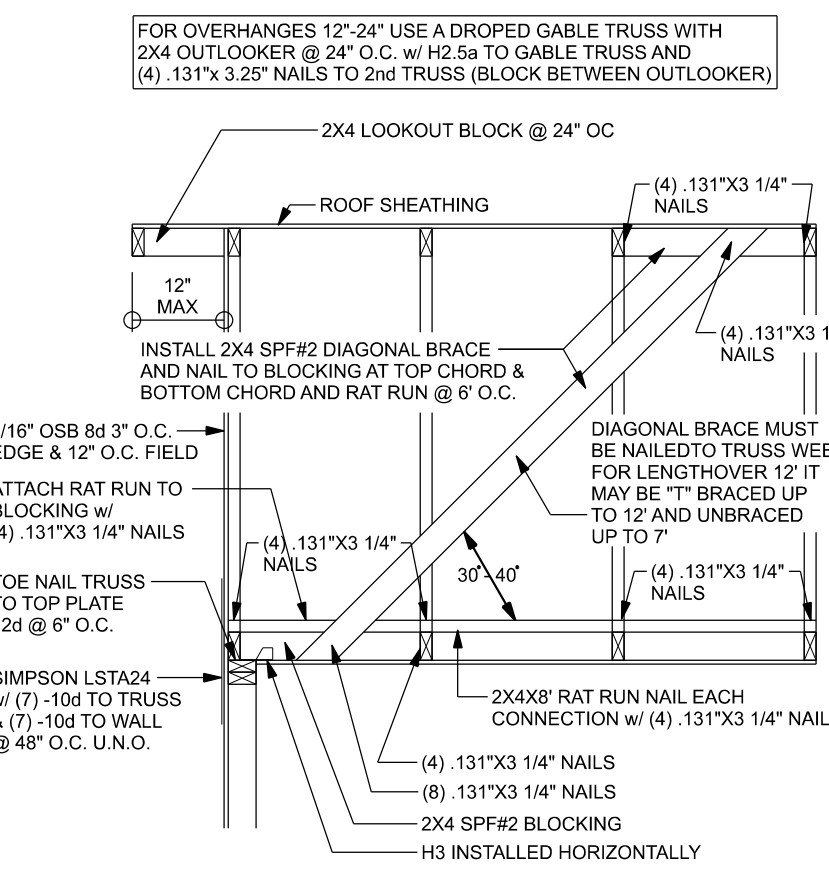
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



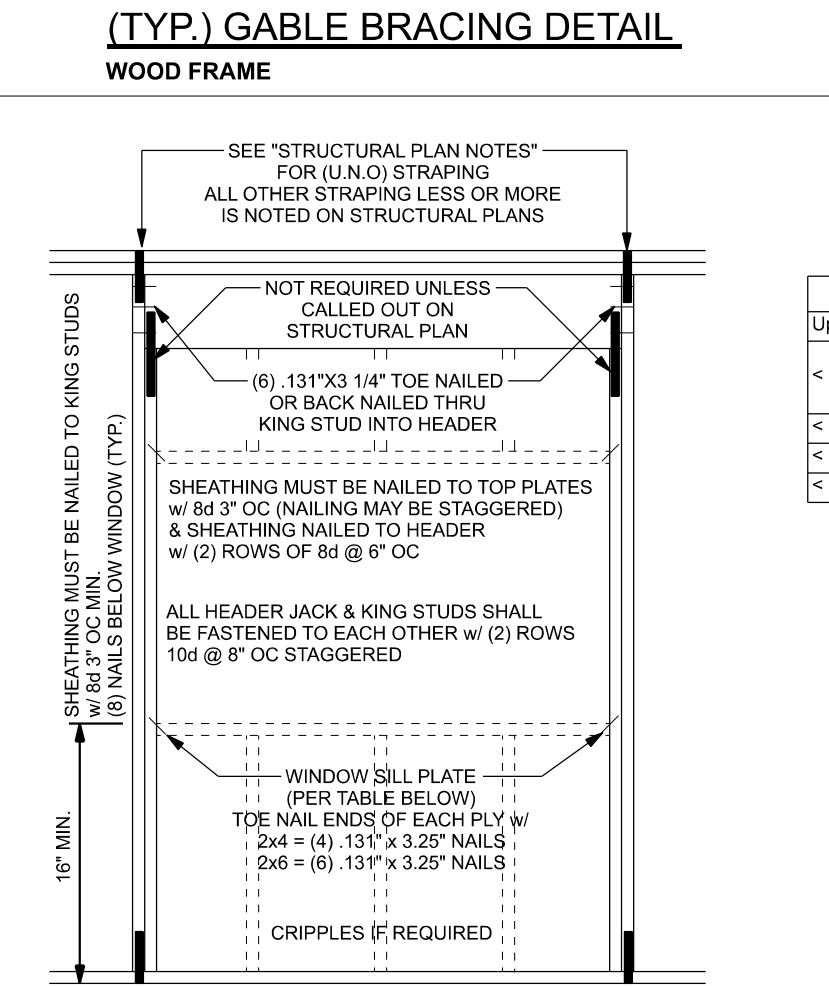
(TYP.) CORNER FRAMING
WOOD FRAME

ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)					
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field	
120 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc	12" oc	
120 mph Exp. C	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc	6" oc	
120 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc	
130 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc	6" oc	
130 mph Exp. C	15/32"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc	6" oc	
130 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc	6" oc	
140 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc	
150 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc	
150 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	4" oc	4" oc	

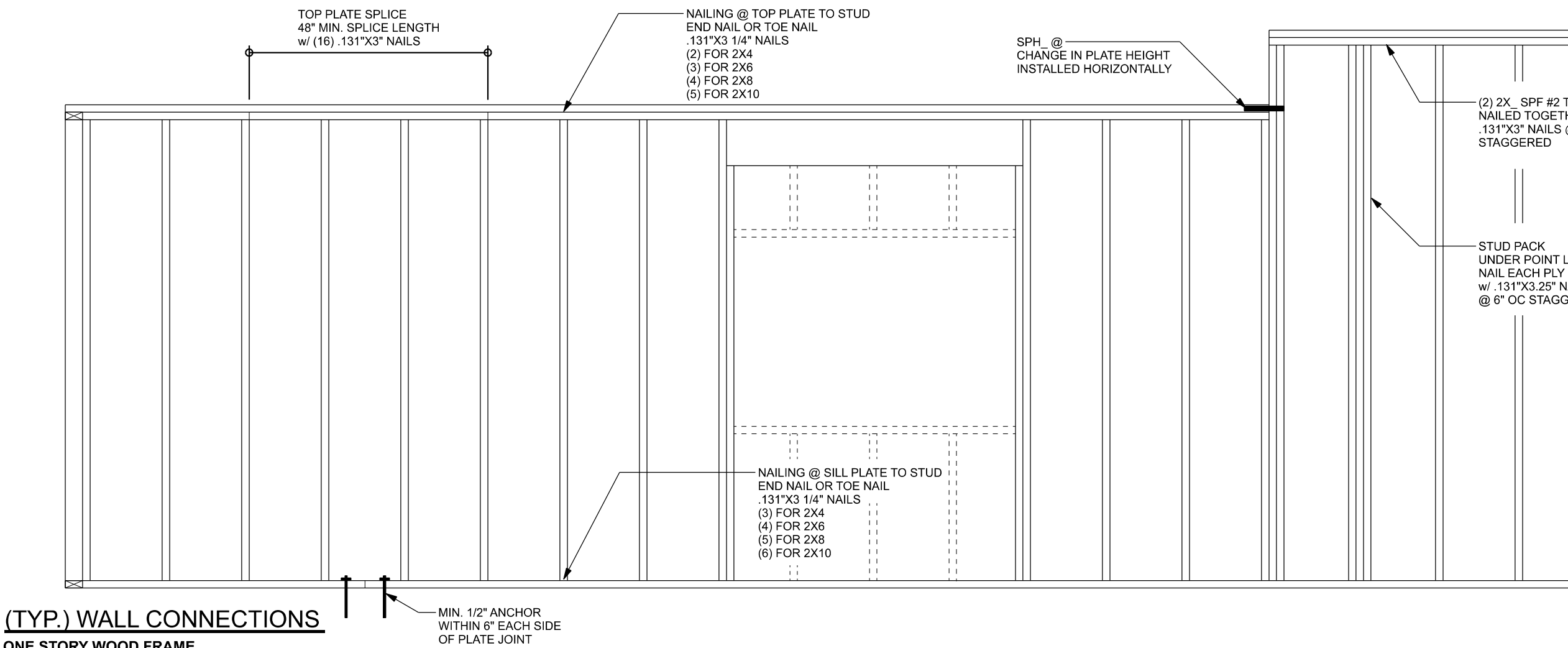
Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



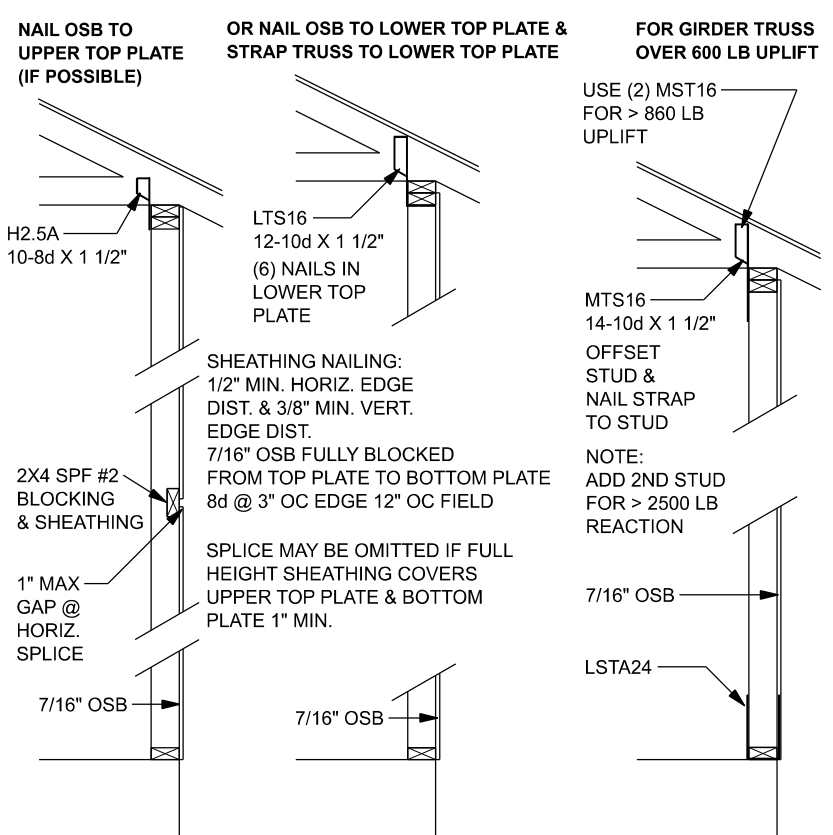
SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.
FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. C, ENCLOSED



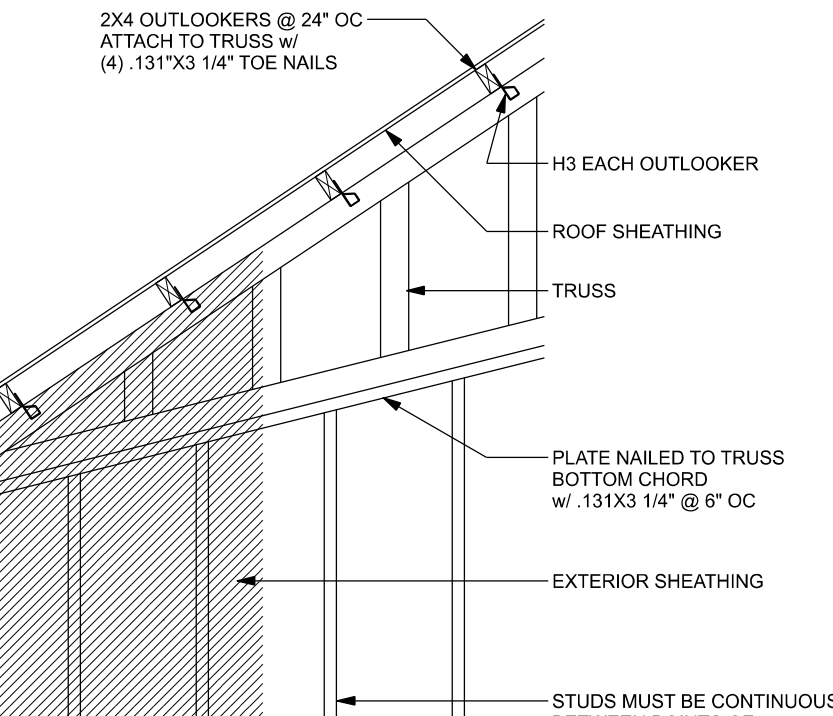
TYPICAL HEADER STRAPING DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



SHEATHING FOR UPLIFT
ATTACHMENT DETAILS
ONE STORY WOOD FRAME

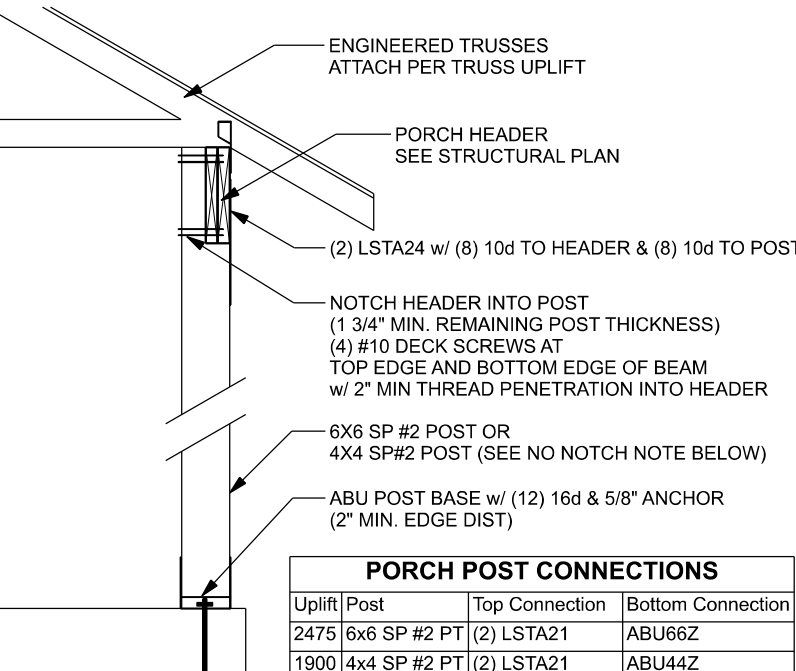


(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

HEADER STRAP TABLE		
Uplift	Top Connection	Bottom Connection
< 1235	LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate
< 1485	MSTA24, 18-10d header to joists	DTT22
< 1800	MSTA24, 18-10d header to joists	DTT22
< 2910	MSTA24, 18-10d header to joists	HTT4

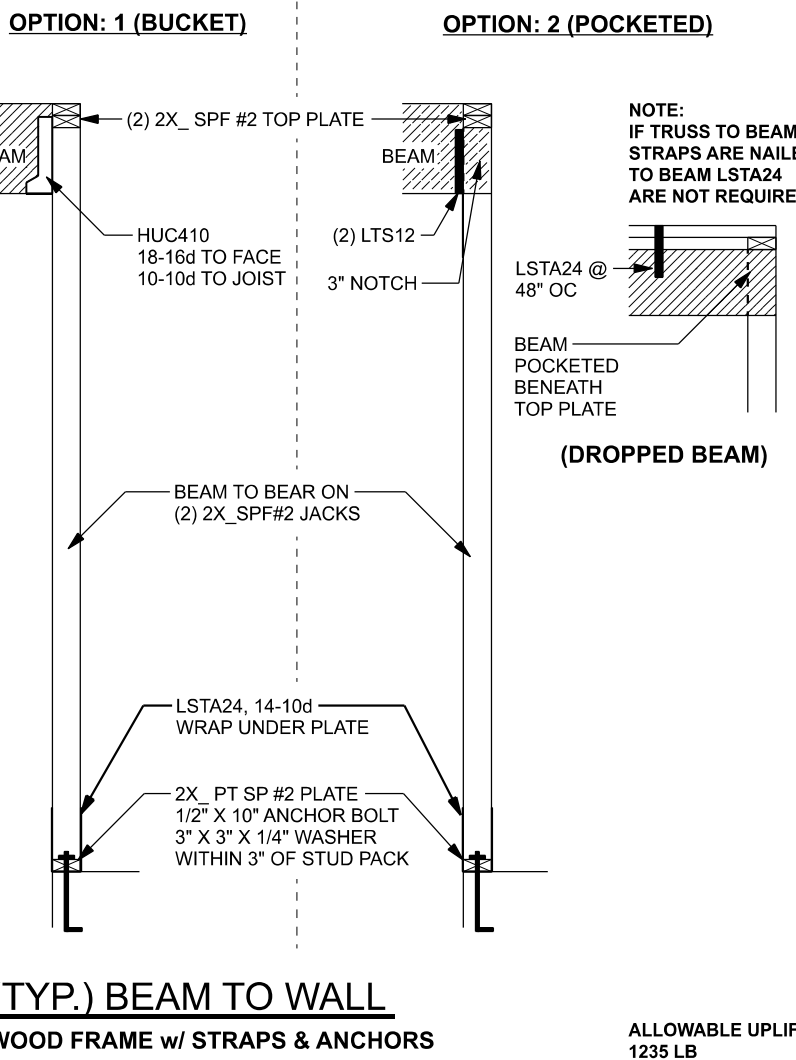
SILL PLATE SPANS FOR 10'-0" WALL HEIGHT				
DESIGN WIND SPEED	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"

CONNECTOR TABLE				
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	-	-
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"
575	495	H2SA	5-8d x 1 1/2"	5-8d x 1 1/2"
1340	1015	H10A	9-10d x 1 1/2"	9-10d x 1 1/2"
720	820	LSTA20	6-10d x 1 1/2"	6-10d x 1 1/2"
1000	860	HTS20-30	7-10d x 1 1/2"	7-10d x 1 1/2"
1450	1245	HTS20-30	12-10d x 1 1/2"	12-10d x 1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	805	SP2	6-10d	6-10d
1771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stemwall	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16d x 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x8" Titen HD
4235	3640	HTT4	18-16d x 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stemwall	To Post	Anchor
1900		ABU44Z	12-16d	5/8"x12" Drill & Epoxy
2475		ABU66Z	12-16d	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
1900		ABU44Z	12-16d	5/8"x7" Drill & Epoxy
2475		ABU66Z	12-16d	5/8"x7" Drill & Epoxy



POSTS CUT FLUSH WITHOUT NOTCH - BRACE TO PREVENT ROTATION:
EAVE / HIP ROOF:
-STRAP TRUSS w/ H2 SA OR EQUAL TO EACH SIDE OF HEADER (FRONT & BACK)
USE LONGER STRAP AS NEEDED IF TOP PLATES ARE INSTALLED
-ATTACH HEADER TO POST w/ (8) 131"x 3.25" TOE-NAIL
-EXTEND GABLE SHEATHING TO BOTTOM OF HEADER NAILED WITH 8d NAILS @ 6" MIN INTO TRUSS BOTTOM CHORD AND INTO BEAM AT 1 1/2" FROM TOP & BOTTOM EDGE
-ATTACH HEADER TO POST w/ (8) 131"x 3.25" TOE-NAIL

(TYP.) PORCH POST
ONE STORY WOOD



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCL TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S DESIGN MEETS ALL REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 4x15LB EACH END, 2X6 RAFTERS 70LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI.

WELED WIRE REINFORCED SLAB: 6" x 6" W/ 4 x W/ 4, F_y = 80ksi, WELED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES, DOSAGE AMOUNTS FROM 0.175 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS.

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F_y = 40 KSI. ALL LAP SPLICES 40" DB (25" FOR 45 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-08, UNO.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH OF SLAB AREAS SHALL NOT EXCEED 15' AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO DRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCL REQUIRED GABLE END.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GROTTED CMU.

BUILDER'S RESPONSIBILITY: THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES WHICH COMPLY WITH FBCL REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PREVENT A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCL, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCL REQUIRED GABLE END AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEMS AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/40 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT	
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT	
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT	
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT	

GRADE & SPECIES TABLE

		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

DESIGN CRITERIA & LOADS:

BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WINDLOADS	BASIC WIND SPEED (ASCE 7-16, SS GUST) 130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	ROOMS OTHER THAN SLEEPING ROOM 40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	FLAT OR < 4:12 20 PSF LIVE LOAD
4:12 TO < 12:12 16 PSF LIVE LOAD	
12:12 & GREATER 12 PSF LIVE LOAD	
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C)			
EFFECTIVE WIND AREA (F ₂)	ZONE 1 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER	
0 - 20	+25.6(Vasd)	-27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)
0 - 20	+42.6(Vult)	-46.2(Vult)	+42.6(Vult) -57(Vult)

Bryan Zecher Construction

McDermott Res

PROJECT ADDRESS:
1365 SW 11th Ave
Lake City, FL 32024
Columbia County, Florida

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915

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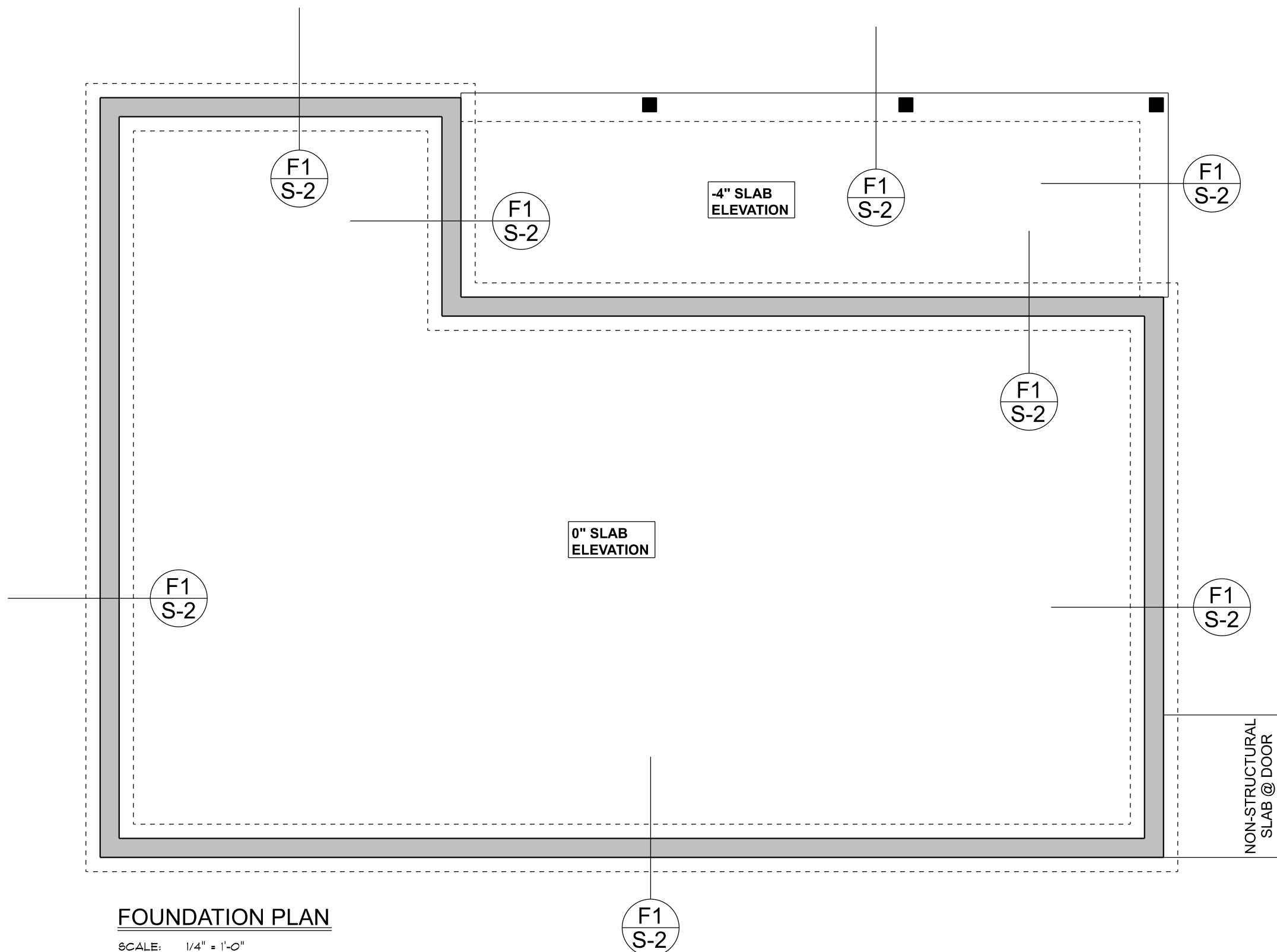
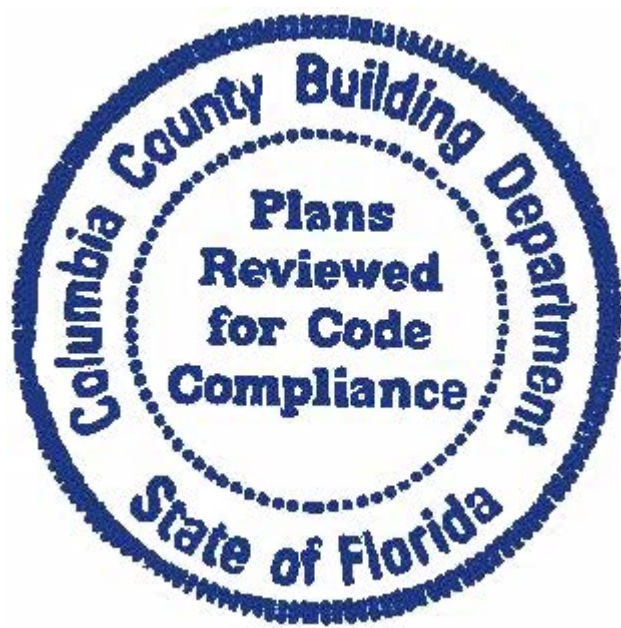
Thursday, August 5, 2021

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JOB NUMBER:
211095

S-1

OF 2 SHEETS

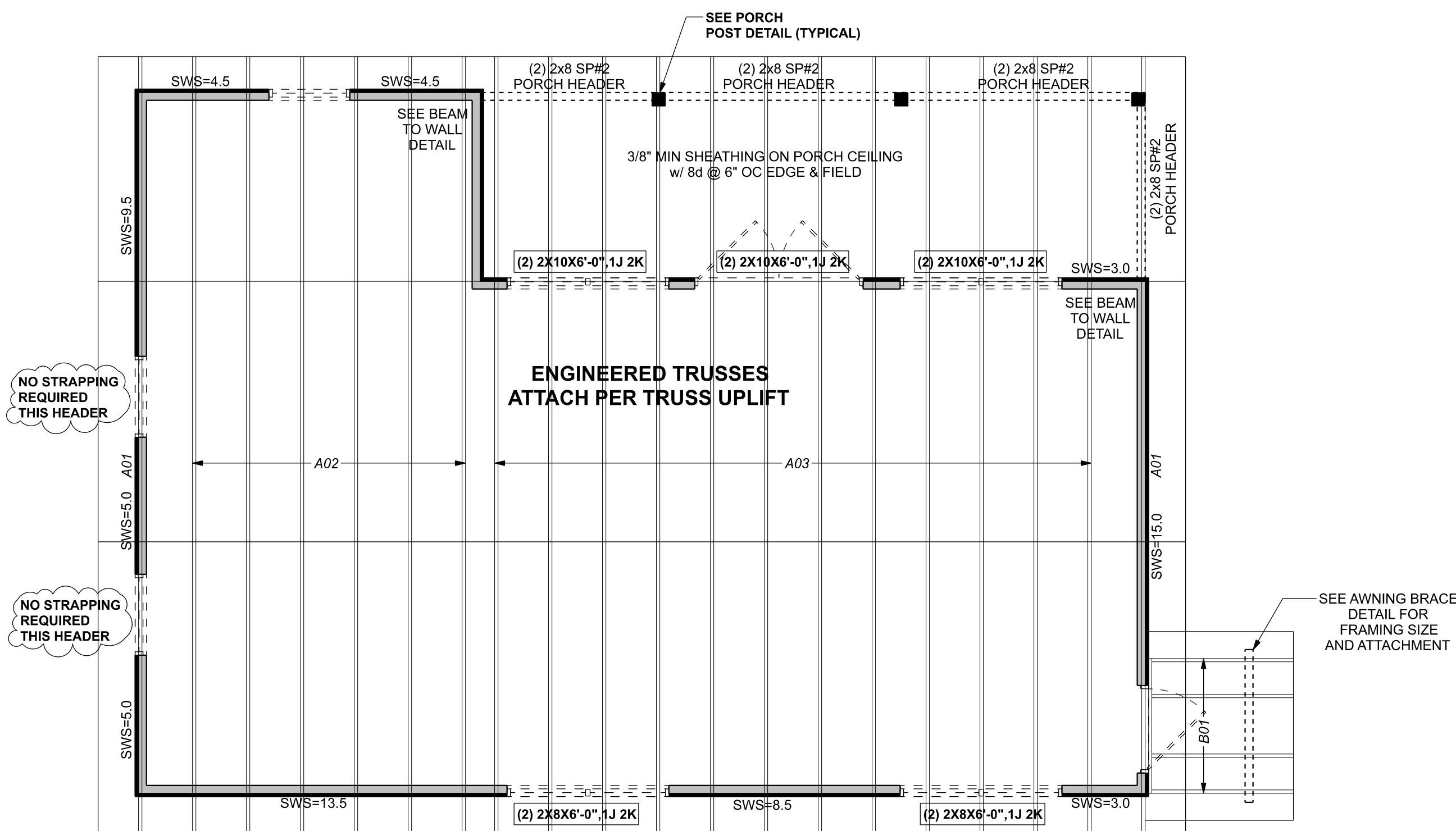


FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

- FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- FN - 2 CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARINGS IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
- FN - 3 THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED W/ 6X6-14114 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER W/ 0" LAPS SEALED W/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD).



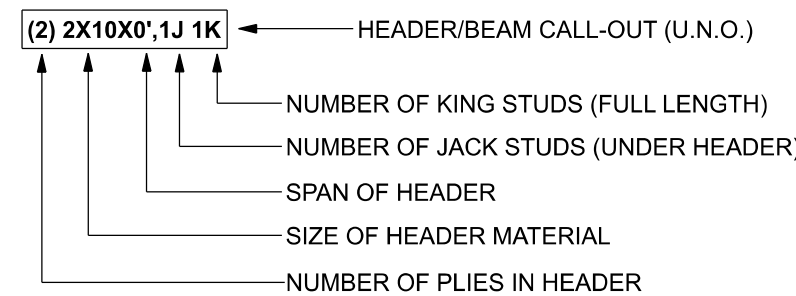
STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 ALL HEADERS W/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LST24: 14-100 @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2" X 10" ANCHOR BOLT W/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
- SN-4 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-5 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-6 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

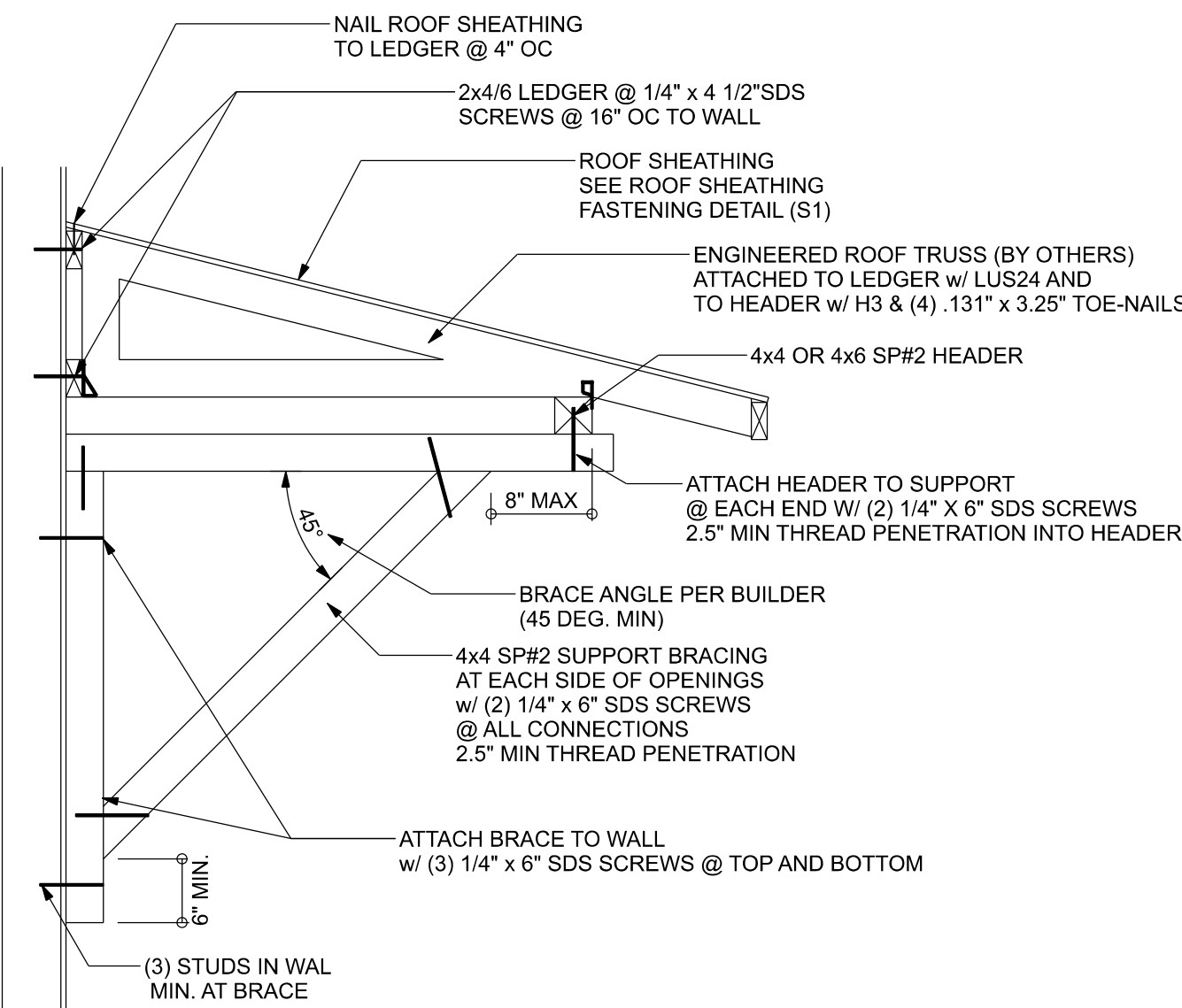
HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	13662 LBF	14652 LBF
REQUIRED	10602 LBF	7788 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, W.B. HOWLAND COMPANY INC. JOB #21-5841 (SEALED ON 2021-08-02)



SCREWS ARE TO BE TOE-SCREWED OR ANGLED DO NOT SCREW INTO END GRAIN
LOG HOG SCREWS OR EQUAL 1/4" FRAMING SCREWS CAN BE USED INPLACE OF SDS SCREWS

AWNING BRACE DETAIL

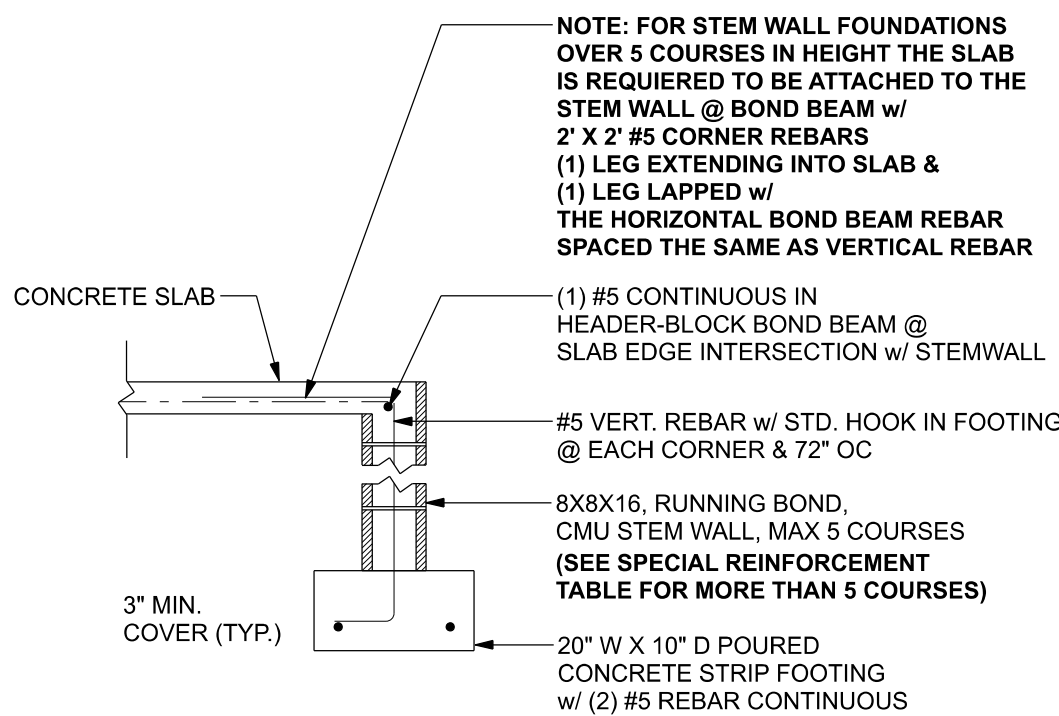
MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF SPECIFICATION FOR MASONRY STRUCTURES* (ACI 530.1/ASCE 6/MS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 475, admixtures require approval
2.3	CMU standard	ASTM C 90-02: Normal weight, hollow medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02: Grade SW, Type FBS, 5.5x2.75x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM A615, Grade 40, Fy = 40 ksi. Lap splices min 40 bar dia. (25" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

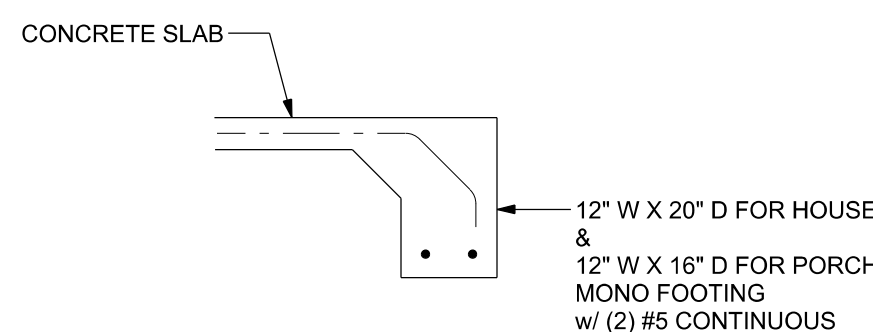
TALL STEM WALL TABLE:
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16" OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



STEM WALL FOOTING



MONOLITHIC FOOTING

Bryan Zecher Construction

McDermott Res

PROJECT ADDRESS:
163 SW Midtown Place
Suite 103
Lake City, FL 32024
Columbia County, Florida

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915
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OF 2 SHEETS