DATE 10/25/2005

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000023762

APPLICANT ALLEN K. BATES PHONE 386.462.2884

ADDRESS 481 SW TURKEY CREEK ALACHUA FL 32615

OWNER ALICE Y. GREEN PHONE

ADDRESS 265 SW THORNE LANE FT. WHITE FL 32038

CONTRACTOR ALLEN K. BATES PHONE 386.462.2884

LOCATION OF PROPERTY 47-S TO C-138 TO MAPLETON,TL TO HEFLIN,TR TO THORNE LN,TL
THE JOB SITE IS ON THE R.(HEFLIN IS JUST PAST SANTA FE RIVER

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 111850.00

HEATED FLOOR AREA 2237.00 TOTAL AREA 3597.00 HEIGHT 26.00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 8'12 FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 30-7S-17-10058-110 SUBDIVISION SANTA FE RIVER PLANTATION

LOT 10 BLOCK PHASE UNIT TOTAL ACRES 2.91

000000862 CRC057572

Culvert Permit No. | Culvert Waiver | Contractor's License Number | Applicant/Owner/Contractor

WAIVER 05-1048-N BLK JTH N

Driveway Connection | Septic Tank Number | LU & Zoning checked by | Approved for Issuance | New Resident

COMMENTS: NOC ON FILE. SECTION 2.3.1 NON-CONFORMING LEGAL LOT OF RECORD.
1 FOOT ABOVE ROAD.

Check # or Cash 3855

FOR BUILDING & ZONING DEPARTMENT ONLY

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic (footer/Slab) _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 560.00 CERTIFICATION FEE $ 17.98 SURCHARGE FEE $ 17.98

MISC. FEES $ .00 ZONING CERT. FEE $ 50.00 FIRE FEE $ .00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 670.96

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0510-23 Date Received 10-7-05 By LH Permit # 862/ 23762
Application Approved by - Zoning Official BLK Date 24.10.05 Plans Examiner OK JTH Date 10-19-05
Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3
Comments Section 2.3.1 Nonconforming Legal Lot of Record
- CK# 3854-W / B.P. #3855-

Applicants Name GENESIS DESIGN & CONST., INC. : ALLEN BATES Phone 386-462-2884
Address 481 TURKEY CREEK - ALACHUA, FL 32615
Owners Name ALICE Y. GREEN Phone c/o GENESIS
911 Address 265 SW THORNE ~~RD.~~ LN, ~~High Springs, FL 32643~~ Fort White FL 32038
Contractors Name ALLEN K. BATES Phone 386-462-2884
Address 481 TURKEY CREEK - ALACHUA, FL 32615
Fee Simple Owner Name & Address
Bonding Co. Name & Address NONE
Architect/Engineer Name & Address NONE
Mortgage Lenders Name & Address NONE

Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progressive Energy (circled)
Property ID Number 30-7S-17-10058-110 Estimated Cost of Construction $150,000.-
Subdivision Name SANTA FE RIVER PLANTATIONS Lot 10 Block - Unit - Phase -
Driving Directions I-75 SOUTH TO US441 (EXIT 399) - NORTH - LEFT @ US 20/27 - THRU HIGH SPRINGS - CONTINUE TO 1ST. DIRT RD. PAST SANTA FE RIVER (MAPLETON) - LEFT ON MAPLETON TO HEFLIN - RIGHT TO THORNE - LEFT - JOBSITE ON RIGHT.
Type of Construction FRAME, w/ HARDI. SIDING Number of Existing Dwellings on Property -0-
Total Acreage 2.91 Lot Size ____ Do you need a - Culvert permit or Culvert Waiver (circled) or Have an Existing Drive
Actual Distance of Structure from Property Lines - Front 100' Side 144' Side 90' Rear 160'
Total Building Height 26' Number of Stories 1 Heated Floor Area 2237 Roof Pitch 8/12
Porch 896 Garage 472 TOTAL 3,597

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

<u>WARNING TO OWNER:</u> YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

[signature]
Owner Builder or Agent (Including Contractor)

Contractor Signature
Contractors License Number CRC057572
Competency Card Number
NOTARY STAMP/SEAL

MY COMMISSION EXPIRES Bonded Thru Notary Public Underwriters

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me
this 7th day of Oct 2005.
Personally known____ or Produced Identification DL

[signature]
Notary Signature

COLUMBIA COUNTY 9-1-1 ADDRESSING

263 NW Lake City Ave. * P. O. Box 1787 * Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE ISSUED: October 10, 2005

ENHANCED 9-1-1 ADDRESS:

265 SW THORNE LN (FORT WHITE, FL 32038)

Addressed Location 911 Phone Number: NOT AVAIL.

OCCUPANT NAME: NOT AVAIL.

OCCUPANT CURRENT MAILING ADDRESS:

PROPERTY APPRAISER PARCEL NUMBER: 30-7S-17-10058-110

Other Contact Phone Number (If any):

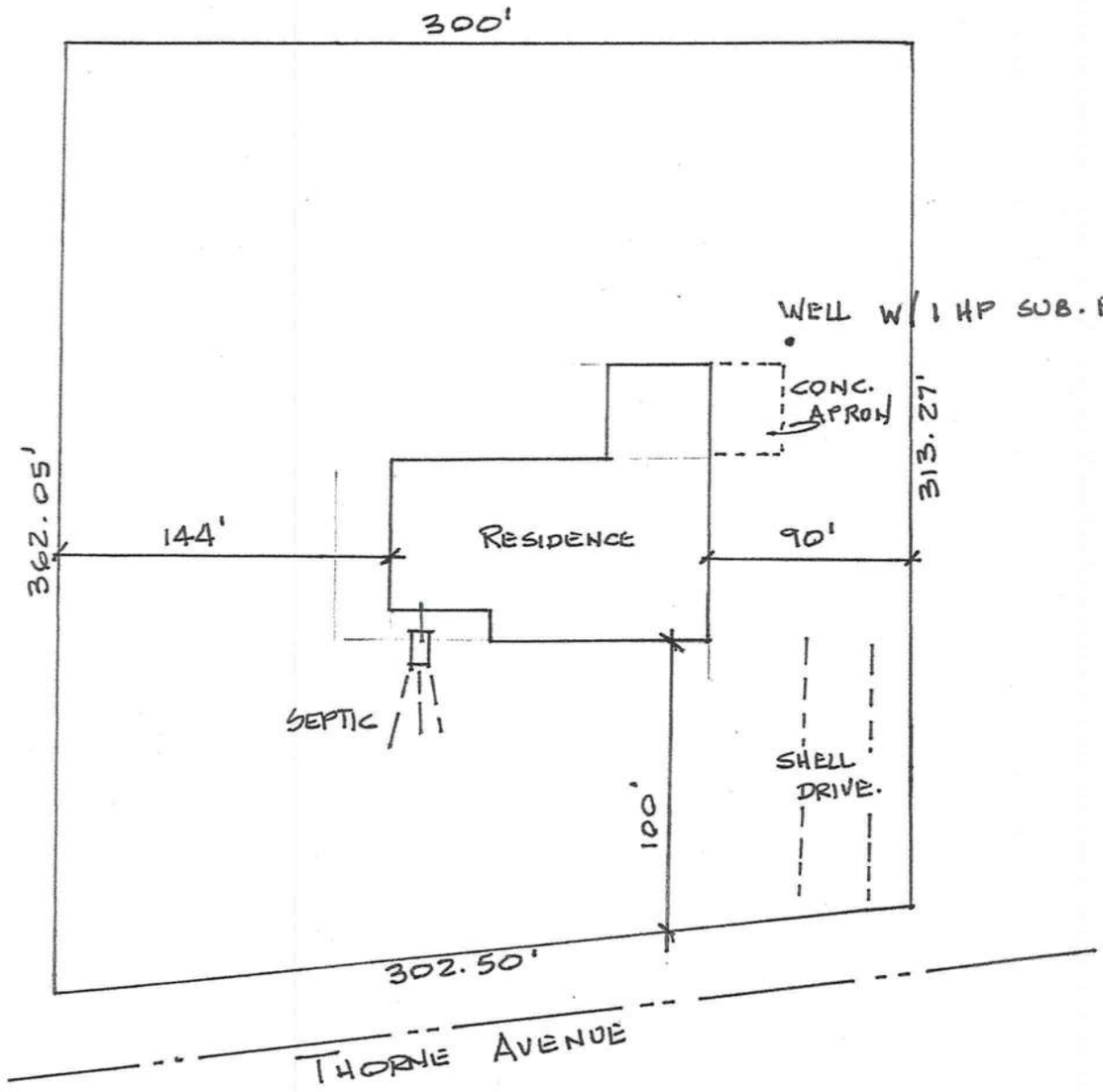
Building Permit Number (If known):

Remarks: LOT 10 SANTA FE RIVER PLANTATIONS

Address Issued By: _____
Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

COLUMBIA COUNTY
9-1-1 ADDRESSING
APPROVED



LOT #10 - SANTA FE
RIVER PLANTATIONS
COLUMBIA COUNTY, FL.

ALLEN BATES
GENESIS
DESIGN & CONSTRUCTION, INC.
(386) 462-2884

Columbia County Property Appraiser

DB Last Updated: 9/16/2005

2005 Proposed Values

Parcel: 30-7S-17-10058-110

Tax Record | Property Card | Interactive GIS Map | Print

Owner & Property Info

Search Result: 1 of 1

Owner's Name	GREEN ALICE Y
Site Address	
Mailing Address	10887 165TH RD NORTH JUPITER, FL 33478
Brief Legal	LOT 10 SANTA FE RIVER PLANTATIONS. ORB 993-2310,

Use Desc. (code)	VACANT (000000)
Neighborhood	30717.01
Tax District	3
UD Codes	MKTA02
Market Area	02
Total Land Area	2.290 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	$26,106.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$26,106.00

Just Value	$26,106.00
Class Value	$0.00
Assessed Value	$26,106.00
Exempt Value	$0.00
Total Taxable Value	$26,106.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
8/29/2003	993/2310	WD	V	Q		$28,000.00
11/1/1985	577/549	WD	V	U	01	$4,600.00
11/1/1980	458/489	03	V	Q		$6,800.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	2.290 AC	1.00/1.00/1.00/1.00	$11,400.00	$26,106.00

Columbia County Property Appraiser

DB Last Updated: 9/16/2005

1 of 1

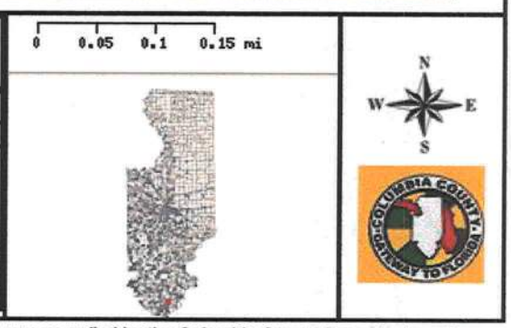
Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 30-7S-17-10058-110 - VACANT (000000)

LOT 10 SANTA FE RIVER PLANTATIONS. ORB 993-2310,

Name:	GREEN ALICE Y		LandVal	$26,106.00
Site:			BldgVal	$0.00
Mail:	10887 165TH RD NORTH JUPITER, FL 33478		ApprVal	$26,106.00
			JustVal	$26,106.00
Sales Info	8/29/2003	$28,000.00 V / Q	Assd	$26,106.00
	11/1/1985	$4,600.00 V / U	Exmpt	$0.00
	11/1/1980	$6,800.00 V / Q	Taxable	$26,106.00



This information, GIS Map Updated: 8/3/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

Inst:2003019107 Date:09/05/2003 Time:11:04
Doc Stamp-Deed : 196.00
DC,P.DeWitt Cason,Columbia County B:993 P:2310

Prepared by and Return to:
Mary T. Dotson, an employee of
Alachua Title Services, LLC,
P.O. Box 2408 (32616), 16407 N.W. 174th Drive, Suite C
Alachua, Florida 32615
386-418-8183

File Number:03-071

Warranty Deed

Made on August 29, 2003 A.D. by and between **Howard W. O'Steen, by his attorney-in-fact, Joan O'Steen,** whose address is 23804 NW 110th Avenue, Alachua, Florida 32615, hereinafter called the "grantor", to **Alice Y. Green**, whose post office address is 10887 - 165th Road North, Jupiter, Florida 33478, hereinafter called the "grantee":

(Whenever used herein the term "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations).

Witnesseth, that the grantor, for and in consideration of the sum of Ten Dollars, ($10.00) and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in **Columbia** County, Florida, to-wit:

Lot 10 of SANTA FE RIVER PLANTATIONS, a Subdivision, according to the Plat thereof as recorded in Plat Book A, Pages 55A and B, of the Public Records of Columbia County, Florida.

Parcel Identification Number: R10058-110

Subject to covenants, conditions, restrictions and easements of record.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances except taxes accruing subsequent to December 31, 2002.

In Witness Whereof, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in the presence of these witnesses:

Howard W. O'Steen, by Joan O'Steen his attorney-in-fact
23804 NW 110th Avenue, Alachua, Florida 32615
his attorney in fact

Witness Signature
Print Name: MARY T. DOTSON

Witness Signature
Print Name: DARRY E HUSBARD

Witness Signature
Print Name:

Witness Signature
Print Name:

Inst:2003019107 Date:09/05/2003 Time:11:04
Doc Stamp-Deed : 196.00
DC,P.DeWitt Cason,Columbia County B:993 P:2311

State of Florida
County of Alachua

THE FOREGOING INSTRUMENT WAS ACKNOWLEDGED before me on August 29, 2003, by Joan O'Steen, attorney in fact for Howard W. O'Steen,, who has produced a valid driver's license as identification.

NOTARY PUBLIC

OFFICIAL NOTARY SEAL
MARY T DOTSON
NOTARY PUBLIC STATE OF FLORIDA
COMMISSION NO. CC890398
MY COMMISSION EXP. DEC. 18,2003

Notary Print Name
My Commission Expires:

Inst:2005024993 Date:10/07/2005 Time:12:22
DC,P.DeWitt Cason,Columbia County B:1061 P:298

NOTICE OF COMMENCEMENT

STATE OF: FLORIDA
COUNTY OF: COLUMBIA

PERMIT #: **Columbia County**
PARCEL #: 30-7S-17-10058-110

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice Of Commencement.

DESCRIPTION OF PROPERTY

LOT: #10 BLOCK: UNIT: SECTION: TWP: RANGE:
SUBDIVISION: SANTA FE RIVER PLANTATIONS
MAP PG. PLAT BOOK A, PGS. 55A & B
STREET ADDRESS: TBD THORNE RD. – HIGH SPRINGS, FL

GENERAL DESCRIPTION OF IMPROVEMENTS: NEW SINGLE FAMILY RESIDENCE

OWNER INFORMATION
OWNER(S) NAME: ALICE Y. GREEN PHONE #: 386-462-2884
ADDRESS: C/O GEN'L. CONT.
INTEREST IN PROPERTY:
FEE SIMPLE TITLEHOLDER NAME:
FEE SIMPLE TITLEHOLDER ADDRESS:
CONTRACTOR NAME: BATES, ALLEN
ADDRESS: 481 TURKEY CREEK PHONE #: 386-462-2884
ALACHUA, FL 32615

LENDOR NAME: NONE PHONE #:

PERSONS WITHIN THE STATE OF FLORIDA DESIGNATED BY OWNER UPON WHOM NOTICES OR OTHER DOCUMENTS MAY BE SERVED AS PROVIDED BY SECTION 713.13(1)(A), FLORIDA STATUTES:
NAME:
ADDRESS:
IN ADDITION TO HIMSELF, OWNER DESIGNATES
_____ OF _____
TO RECEIVE A COPY OF THE LIENOR'S NOTICE AS PROVIDED IN SECTION 713.13(1)(B), FLORIDA STATUTES.

EXPIRATION DATE IS 1 YEAR FROM THE DATE OF RECORDING UNLESS A DIFFERENT DATE IS SPECIFIED: _____

SIGNATURE OF OWNER: Alice Y. Green
SWORN TO AND SUBSCRIBED BEFORE ME THIS 6th DAY OF September, 2005

KNOWN PERSONALLY / ID SHOWN: Florida Drivers license

NOTARY: Lynnette G Bennett

MY COMMISSION EXPIRES: January 16, 2009

NOTARY PUBLIC
LYNNETTE G. BENNETT
Notary Public. State of Florida
My comm. expires Jan. 16. 2009
No. DD 384561

ACORD CERTIFICATE OF LIABILITY INSURANCE

DATE (MM/DD/YYYY) 09/16/2005

PRODUCER (352)377-2002 FAX (352)376-8393
Scarborough Company Insurance, Inc.
2811 NW 41st Street
P. O. Box 147050
Gainesville, FL 32614-7050

THIS CERTIFICATE IS ISSUED AS A MATTER OF INFORMATION ONLY AND CONFERS NO RIGHTS UPON THE CERTIFICATE HOLDER. THIS CERTIFICATE DOES NOT AMEND, EXTEND OR ALTER THE COVERAGE AFFORDED BY THE POLICIES BELOW.

INSURED Genesis Design & Construction, Inc
481 Turkey Creek
Alachua, FL 32615

INSURERS AFFORDING COVERAGE	NAIC #
INSURER A: Mid-Continent Casualty	
INSURER B:	
INSURER C:	
INSURER D:	
INSURER E:	

COVERAGES

THE POLICIES OF INSURANCE LISTED BELOW HAVE BEEN ISSUED TO THE INSURED NAMED ABOVE FOR THE POLICY PERIOD INDICATED. NOTWITHSTANDING ANY REQUIREMENT, TERM OR CONDITION OF ANY CONTRACT OR OTHER DOCUMENT WITH RESPECT TO WHICH THIS CERTIFICATE MAY BE ISSUED OR MAY PERTAIN, THE INSURANCE AFFORDED BY THE POLICIES DESCRIBED HEREIN IS SUBJECT TO ALL THE TERMS, EXCLUSIONS AND CONDITIONS OF SUCH POLICIES. AGGREGATE LIMITS SHOWN MAY HAVE BEEN REDUCED BY PAID CLAIMS.

INSR LTR	ADD'L INSRD	TYPE OF INSURANCE	POLICY NUMBER	POLICY EFFECTIVE DATE (MM/DD/YY)	POLICY EXPIRATION DATE (MM/DD/YY)	LIMITS	
A		GENERAL LIABILITY [X] COMMERCIAL GENERAL LIABILITY [ ] CLAIMS MADE [X] OCCUR GEN'L AGGREGATE LIMIT APPLIES PER: [X] POLICY [ ] PRO-JECT [ ] LOC	04-GL-000580942	03/03/2005	03/03/2006	EACH OCCURRENCE	$ 500,000
						DAMAGE TO RENTED PREMISES (Ea occurence)	$ 100,000
						MED EXP (Any one person)	$ excluded
						PERSONAL & ADV INJURY	$ 500,000
						GENERAL AGGREGATE	$ 1,000,000
						PRODUCTS - COMP/OP AGG	$ 1,000,000
		AUTOMOBILE LIABILITY [ ] ANY AUTO [ ] ALL OWNED AUTOS [ ] SCHEDULED AUTOS [ ] HIRED AUTOS [ ] NON-OWNED AUTOS				COMBINED SINGLE LIMIT (Ea accident)	$
						BODILY INJURY (Per person)	$
						BODILY INJURY (Per accident)	$
						PROPERTY DAMAGE (Per accident)	$
		GARAGE LIABILITY [ ] ANY AUTO				AUTO ONLY - EA ACCIDENT	$
						OTHER THAN AUTO ONLY: EA ACC	$
						AGG	$
		EXCESS/UMBRELLA LIABILITY [ ] OCCUR [ ] CLAIMS MADE [ ] DEDUCTIBLE [ ] RETENTION $				EACH OCCURRENCE	$
						AGGREGATE	$
							$
							$
							$
		WORKERS COMPENSATION AND EMPLOYERS' LIABILITY ANY PROPRIETOR/PARTNER/EXECUTIVE OFFICER/MEMBER EXCLUDED? If yes, describe under SPECIAL PROVISIONS below				[ ] WC STATU-TORY LIMITS [ ] OTH-ER	
						E.L. EACH ACCIDENT	$
						E.L. DISEASE - EA EMPLOYEE	$
						E.L. DISEASE - POLICY LIMIT	$
		OTHER					

DESCRIPTION OF OPERATIONS / LOCATIONS / VEHICLES / EXCLUSIONS ADDED BY ENDORSEMENT / SPECIAL PROVISIONS

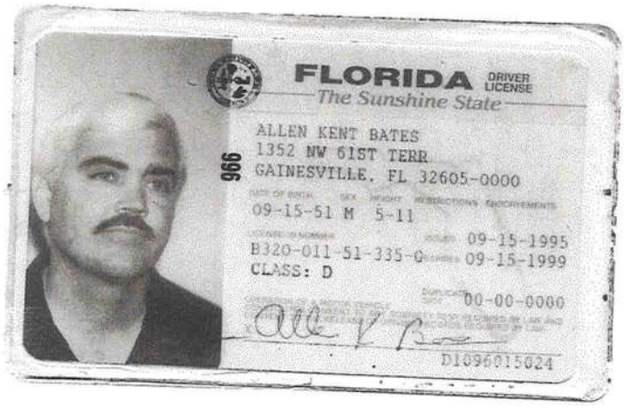
CERTIFICATE HOLDER

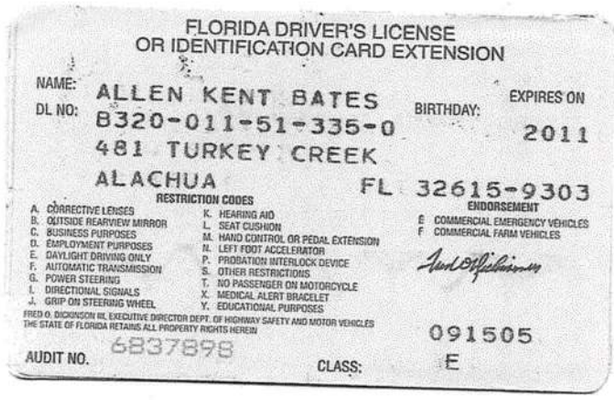
Columbia County Building Department
PO Drawer 1529
Lake City, FL 32056-1529

CANCELLATION

SHOULD ANY OF THE ABOVE DESCRIBED POLICIES BE CANCELLED BEFORE THE EXPIRATION DATE THEREOF, THE ISSUING INSURER WILL ENDEAVOR TO MAIL 10 DAYS WRITTEN NOTICE TO THE CERTIFICATE HOLDER NAMED TO THE LEFT, BUT FAILURE TO MAIL SUCH NOTICE SHALL IMPOSE NO OBLIGATION OR LIABILITY OF ANY KIND UPON THE INSURER, ITS AGENTS OR REPRESENTATIVES.

AUTHORIZED REPRESENTATIVE
Crystal Horne/CNH


FLORIDA DRIVER LICENSE
The Sunshine State
966
ALLEN KENT BATES
1352 NW 61ST TERR
GAINESVILLE, FL 32605-0000
09-15-51 M 5-11
09-15-1995
B320-011-51-335-0 09-15-1999
CLASS: D
00-00-0000
D1096015024



FLORIDA DRIVER'S LICENSE
OR IDENTIFICATION CARD EXTENSION

NAME: ALLEN KENT BATES
DL NO: B320-011-51-335-0
481 TURKEY CREEK
ALACHUA FL 32615-9303

BIRTHDAY: 091505
EXPIRES ON: 2011

RESTRICTION CODES
A. CORRECTIVE LENSES
B. OUTSIDE REARVIEW MIRROR
C. BUSINESS PURPOSES
D. EMPLOYMENT PURPOSES
E. DAYLIGHT DRIVING ONLY
F. AUTOMATIC TRANSMISSION
G. POWER STEERING
I. DIRECTIONAL SIGNALS
J. GRIP ON STEERING WHEEL
K. HEARING AID
L. SEAT CUSHION
M. HAND CONTROL OR PEDAL EXTENSION
N. LEFT FOOT ACCELERATOR
P. PROBATION INTERLOCK DEVICE
S. OTHER RESTRICTIONS
T. NO PASSENGER ON MOTORCYCLE
X. MEDICAL ALERT BRACELET
Y. EDUCATIONAL PURPOSES

ENDORSEMENT
E COMMERCIAL EMERGENCY VEHICLES
F COMMERCIAL FARM VEHICLES

FRED O. DICKINSON III, EXECUTIVE DIRECTOR DEPT. OF HIGHWAY SAFETY AND MOTOR VEHICLES
THE STATE OF FLORIDA RETAINS ALL PROPERTY RIGHTS HEREIN

AUDIT NO. 6837898
CLASS: E

06-20-2005

TOM GALLAGHER
CHIEF FINANCIAL OFFICER

STATE OF FLORIDA
DEPARTMENT OF FINANCIAL SERVICES
DIVISION OF WORKERS' COMPENSATION

**** CERTIFICATE OF EXEMPTION FROM FLORIDA WORKERS' COMPENSATION LAW ****

CONSTRUCTION INDUSTRY EXEMPTION

This certifies that the individual listed below has elected to be exempt from Florida Workers' Compensation Law.

EFFECTIVE DATE: 06/20/2005 ** EXPIRATION DATE: 06/20/2007

PERSON: BATES ALLEN K

FEIN: 593519238

BUSINESS NAME AND ADDRESS: GENESIS DESIGN & CONSTRUCTION INC
481 TURKEY CREEK
ALACHUA FL 32615

SCOPE OF BUSINESS OR TRADE: 1 - CERTIFIED RESIDENTIAL CONTRACT

IMPORTANT: Pursuant to Chapter 440.05(14), F.S., an officer of a corporation who elects exemption from this chapter by filing a certificate of election under this section may not recover benefits or compensation under this chapter.

DWC-252 CERTIFICATE OF ELECTION TO BE EXEMPT REVISED 01-04 QUESTIONS? (850) 413-1609

PLEASE CUT OUT THE CARD BELOW AND RETAIN FOR FUTURE REFERENCE

STATE OF FLORIDA
DEPARTMENT OF FINANCIAL SERVICES
DIVISION OF WORKERS' COMPENSATION

CONSTRUCTION INDUSTRY
CERTIFICATE OF EXEMPTION FROM FLORIDA WORKERS' COMPENSATION LAW

EFFECTIVE: 06/20/2005
** EXPIRATION DATE: 06/20/2007

PERSON: BATES ALLEN K

FEIN: 593519238

BUSINESS NAME AND ADDRESS: GENESIS DESIGN & CONSTRUCTION
481 TURKEY CREEK
ALACHUA FL 32615

SCOPE OF BUSINESS OR TRADE:
1- CERTIFIED RESIDENTIAL CONTRACT

FOLD HERE

IMPORTANT

Pursuant to Chapter 440.05(14), F.S., an officer of a corporation who elects exemption from this chapter by filing a certificate of election under this section may not recover benefits or compensation under this chapter.

QUESTIONS? (850) 413-1609

CUT HERE

* Carry bottom portion on the job, keep upper portion for your records.

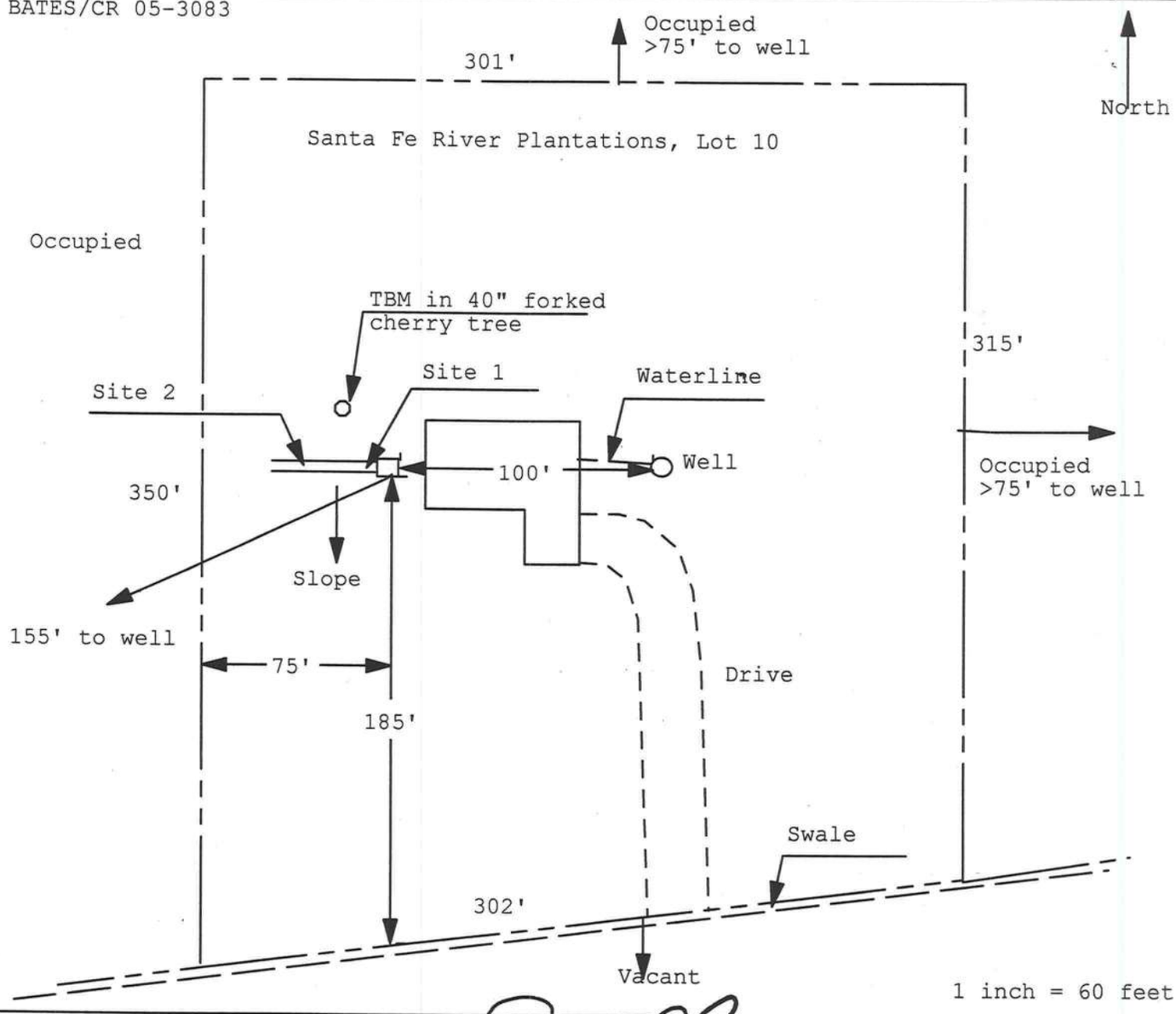
DWC-252 CERTIFICATE OF ELECTION TO BE EXEMPT REVISED 01-04

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 05-1048N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

BATES/CR 05-3083



Site Plan Submitted By Paul Lloyd **Date** 9/13/05

Plan Approved ✓ **Not Approved** ____ **Date** 10/14/05

By [illegible] Columbia **CPHU**

Notes: _____

14405 Peggy Road
Alachua, FL 32615
(386) 462-2845

To: Columbia Co

For: Allen Bates

Genesis Design & Construction Inc
Santa Fe River Plantation Lot 10
Building Permit 0510-33

Re: Well 4", 1 HP Sub Pump, 220 Gal Tank
meeting the So Building Code Requirements
THERE WILL BE NO CYCLE STOP VALVE

Yours

SH Griffis
State Lic # 1105
MGWC # 199346
CWS, CI CSR # 1009578

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
X	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
X	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
X	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
X	☐	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
X	☐	**Elevations including:**
X	☐	a) All sides
X	☐	b) Roof pitch
X	☐	c) Overhang dimensions and detail with attic ventilation
☐ N/A	☐	d) Location, size and height above roof of chimneys
☐ N/A	☐	e) Location and size of skylights
X	☐	f) Building height
X (1)	☐	e) Number of stories

Floor Plan including:

☒ ☐ a) Rooms labeled and dimensioned

☒ ☐ b) Shear walls

☒ ☐ c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)

☒ ☐ d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth

☐ N/A ☐ e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails

☒ ☐ f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

☒ ☐ a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing

☒ ☐ b) All posts and/or column footing including size and reinforcing

☒ ☐ c) Any special support required by soil analysis such as piling

☒ ☐ d) Location of any vertical steel

Roof System:

☒ ☐ a) Truss package including:

1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

☐ N/A ☐ b) Conventional Framing Layout including:

1. Rafter size, species and spacing
2. Attachment to wall and uplift
3. Ridge beam sized and valley framing and support details
4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

☐ N/A ☐ a) Masonry wall

1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termicide or alternative method)
10. Slab on grade
    a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☒ ☐ **b) Wood frame wall**

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
    a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ N/A ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☐ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

N/A

☐ ☐ b) Floor joist size and spacing

☐ ☐ c) Girder size and spacing

☐ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

☒ ☐ **Plumbing Fixture layout**

Electrical layout including:

☒ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☒ ☐ b) Ceiling fans

☒ ☐ c) Smoke detectors

☒ ☐ d) Service panel and sub-panel size and location(s)

☒ ☐ e) Meter location with type of service entrance (overhead or underground)

☒ ☐ f) Appliances and HVAC equipment

☒ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

☒ ☐ a) Manual J sizing equipment or equivalent computation

☒ ☐ b) Exhaust fans in bathroom

☒ ☐ **Energy Calculations** (dimensions shall match plans)

☐ ☐ **Gas System** Type (LP or Natural) Location and BTU demand of equipment

☐ ☐ **Disclosure Statement for Owner Builders**

☐ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done**

☒ ☐ **Private Potable Water**

a) Size of pump motor

b) Size of pressure tank

c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

OK 2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

OK 3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued.
(386) 758-1058 **(Toilet facilities shall be provided for construction workers)**

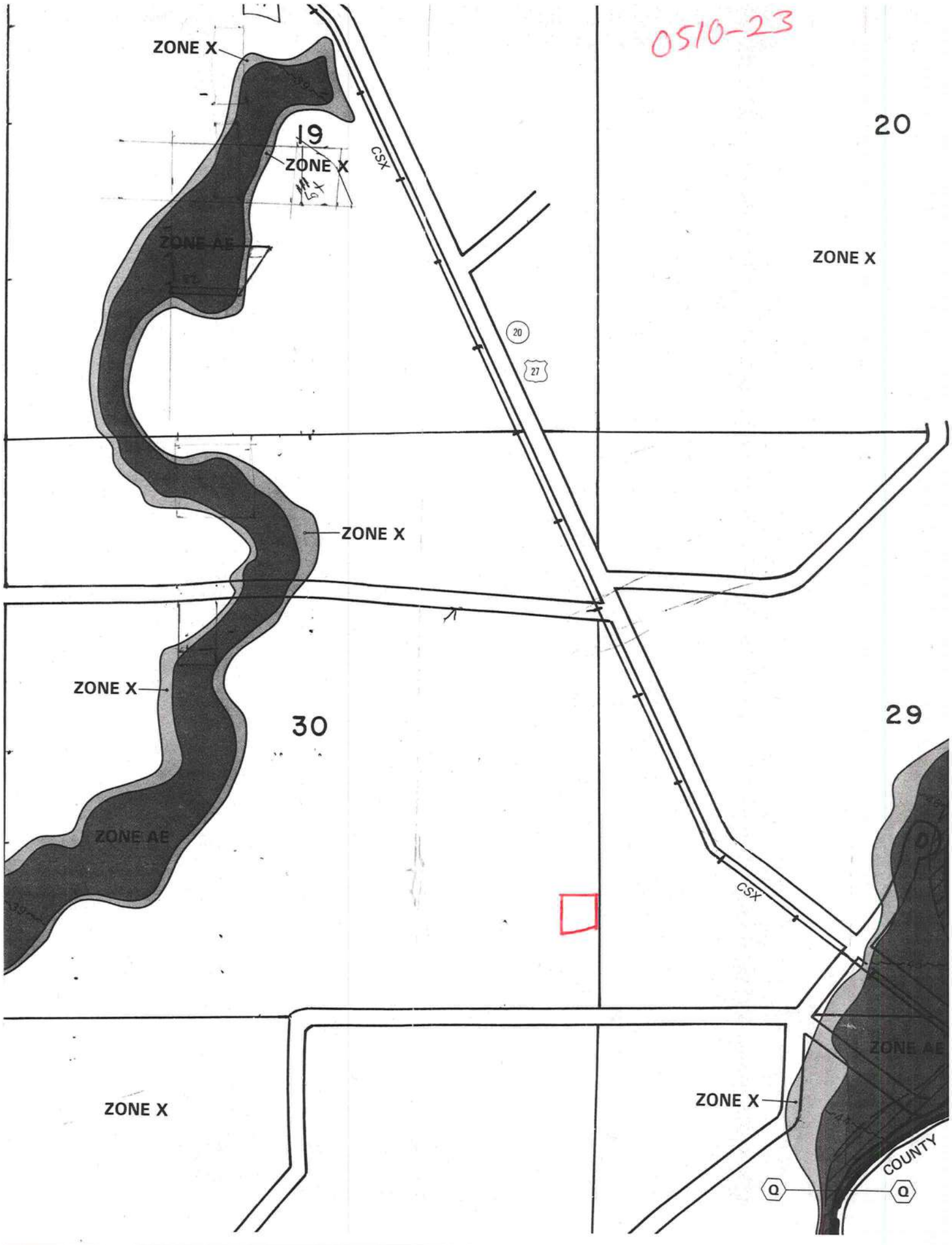
N/A 4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations.
CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.
A development permit will also be required. Development permit cost is **$50.00**

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit **($25.00)** must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver **($50.00)**. All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK


0510-23
ZONE X
19
ZONE X
ZONE AE
20
ZONE X
CSX
20
27
ZONE X
ZONE X
30
29
ZONE AE
CSX
ZONE X
ZONE X
ZONE AE
COUNTY
Q
Q

Genesis Design & Construction, Inc.
481 Turkey Creek
Alachua, FL 32615
386-462-2884 - Phone
386-462-4637 - FAX

New Residence For
Ms. Alice Y. Green
Thorne Rd
High Springs, FL

Lot #10, Santa Fe River Plantations Sub.
Columbia County, FL

OFFICE COPY

1 Truss Engineering

2 Wind Load Calcs.

3 Manual J & D

4 Energy Calculations

5 Window Engineering

6 Door Engineering

7 Garage Door Eng.

8

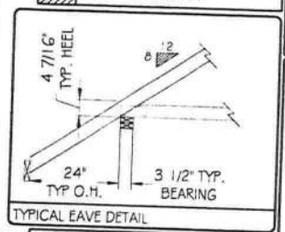
wilsonjones.

www.wilsonjones.com

LEDGEND
BEAM BY OTHERS
SUPPLIED BEAM
VALLEY/FIELD FRAME
PLUMBING DROP
SLOPED CEILING
RAISED CEILING
12'-0" BEARING HT.

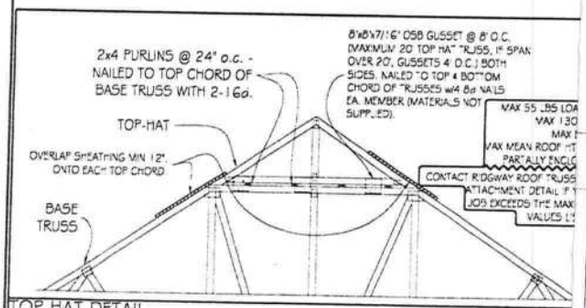
ROOF TRUSS LAYOUT

SCALE: 1/8" = 1'-0"

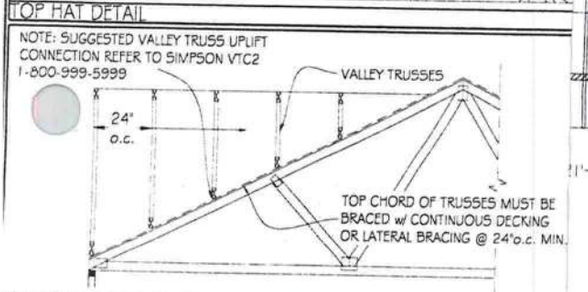


TYPICAL EAVE DETAIL

SYMBOL: J L
QTY: 3
For Nailing See Simpson Catalog or Ca. 1-800-999-5099
SIMPSON HUS26



TOP HAT DETAIL



TRUSSED VALLEY DETAIL

"ATTENTION"

1. LIMITED RESPONSIBILITY OF ROOF TRUSS MANUFACTURER. SEE WTCA 1-1995, SECTION 6.0, FOUND IN DESIGN MANUAL.
2. WARNING FAILURE TO FOLLOW RECOMMENDATIONS IN "BCSI-B1" MAY RESULT IN SERIOUS INJURY, LOSS OF LIFE, OR PROPERTY DAMAGE.
3. CUSTOMER TO VERIFY ALL DIMENSIONS, BEARING HEIGHTS, PITCHES AND ALL INFORMATION REGARDING THE DESIGN AND FABRICATION OF TRUSS SYSTEM.
4. DO NOT OVERLOAD TRUSSES w/ SHEETROCK / SHEATHING OR OTHER BUILDING MATERIALS.
5. TRUSSES ARE NOT MARKED IN ANY WAY TO IDENTIFY THE FREQUENCY, OR LOCATION OF TEMPORARY ERECTION BRACING. ALL TEMPORARY BRACING SHALL COMPLY WITH THE LATEST EDITION OF BCSI-B1 PUBLISHED BY THE TRUSS PLATE INSTITUTE FOUND IN THE DESIGN MANUAL.
6. NON-LOAD BEARING WALLS TO BE SINGLE TOP PLATED, TO PREVENT LOAD TRANSFER TO THESE WALLS. A 1x4 MAY BE USED AS SECOND TOP PLATE.

Ridgway Roof Truss Company
Mailing: P.O. Box 1309 Gainesville, Florida 32602
Physical: 235 sw 11th Place Gainesville, Florida 32601
Telephone (352) 376-4436 FAX (352) 371-3316

GENESIS
GREEN RESIDENCE

SALESMAN:	WELLS THELOSEN		
DESIGN:	PL	CAD:	CHRIS
JOB #:	050984	DATE:	8-22-05
FILE:	K:\Jobs\050\050984		
PAGE	1 of 1		

g, 09/02/05 07:18:16 AM, LORENZO

Ridgway Roof Truss Company

(Trusses and Prefabricated Building Components)

Mailing: P.O. Box 1309 – Gainesville, Florida 32602
Physical: 235 SW 11th Place – Gainesville, Florida 32601

Telephone: (352) 376-4436
FAX: (352) 371-3316

Email: Sales@RidgwayTruss.com
www.RidgwayTruss.com

WARNING

THESE TRUSSES MUST BE HANDLED ACCORDING TO BCSI-B1 SUMMARY SHEETS

SEE TABLE OF CONTENTS

SEALED
DESIGN MANUAL

PROJECT NAME: Green Residence
JOB NUMBER: 05-0984
CONTRACTOR: Genesis
DATE: 9-6-05
REVISIONS:
COMMENTS:

RIDGWAY ROOF TRUSS COMPANY

P.O. BOX 1309 GAINESVILLE, FL 32602, TELEPHONE: (352) 376-4436 FAX: (352) 371-3316

Job: **Green Residence**

Customer: **Genesis**

Jurisdiction: **Columbia**

Structural Eng. of Record: none License # : none building designer

Address of Eng. Of Record: none

SIGNATURE of Building Designer: *single family home*

I, Gary G. Dounson, Florida Professional Engineer No.35054, hereby certify that I am the truss engineer of record with responsibility for the design of the metal plate connected wood trusses listed herein. The design of these trusses have been performed under my responsible supervision, direction and control and are in accordance with the National Design Standard for Metal Plate Connected Wood Truss Construction (ANSI TPI 1). Building Designer of Record responsibilities shall be required in Article 2.2 of the aforementioned TPI 1 document and shall include but are not limited to the design and detailing of truss supports, anchorage, and permanent truss bracing.

Engineering Program is **Robbins Engineering Online Plus**

Live load: 20 lbs **Dead load:** 17 lbs Roof **Building code and chapter:** ANSI/ASCE 7-98

Mean Roof Height: 25 ' **Exposure:** B **Wind speed:** 110 mph **Enclosure:** Enclosed **Design criteria:** TPI 95 **Category:** II

#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date	#	Mark	Run Date
1	R1	8/19/05	29			57			85			107		
2	R2	8/19/05	30			58			86			108		
3	R3	8/19/05	31			59			87			109		
4	R4	9/1/05	32			60			88			110		
5	R5	9/1/05	33			61			89			111		
6	R6	8/19/05	34			62			90			112		
7	R6B	8/19/05	35			63			91			113		
8	R7	8/19/05	36			64			92			114		
9	R8	8/19/05	37			65			93			115		
10	R9	8/19/05	38			66			94			116		
11	R10	8/19/05	39			67			95			117		
12	T1	8/19/05	40			68			96			118		
13	T2	8/19/05	41			69			97			119		
14	T3	8/19/05	42			70			98			120		
15	T4	8/19/05	43			71			99			121		
16	V1	8/19/05	44			72			100			122		
17	V2	8/19/05	45			73			101			123		
18	V3	8/19/05	46			74			102			124		
19	V4	8/19/05	47			75			103			125		
20	V5	8/19/05	48			76			104			126		
21			49			77			105			127		
22			50			78			106			128		
23			51			79								
24			52			80								
25			53			81								
26			54			82								
27			55			83								
28			56			84								

This Package includes 20 individual, dated Truss Design Drawings. With my Seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003 section 5 of the Florida Board of Professional Engineers.

Delegated Engineer (Truss Designer)
GARY DOUNSON, PE 35054
GARY DOUNSON & ASSOCIATES, INC.
2830 NW 41st STREET SUITE D
GAINESVILLE, FL 32606
(352) 375-8593
CA 5201
9/6/2005

Note: The seal on this index sheet indicates acceptance of professional engineering responsibility solely for the Truss Design Drawings listed above and attached. The suitability and use of each component for any particular building is the responsibility of the Building Designer, per ANSI/TPI 1-1995 Section 2.

FILE SealSheet

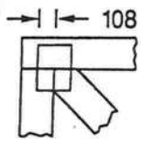
9-6-05

C1

TABLE OF CONTENTS

ROBBINS ENG. GENERAL NOTES & SYMBOLS

PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108).

PLATE SIZE

6.3 x 8.8

The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots.

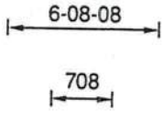
LATERAL BRACING

1x4 continuous lateral bracing attached with (2) 8d nails each member where indicated or 2x4 "T" or "L" brace stiffener if applicable nailed flat to edge of web with 12d nails spaced 8" o.c. "T" or "L" brace must be extended at least 90% of web length.

PLATE ORIENTATION

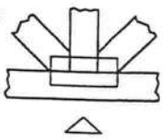
When shown, indicates direction of slots in connector plate.

DIMENSIONS



All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).

BEARING



When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS LOCK connector plates (20 ga. galv. steel ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates, unless shown otherwise by circles (o) or dimensions. No loose knots or wanes in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrication shall be performed on equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and these designs are not applicable for use with fire retardant lumber. This design was prepared in accordance with "National Design Specifications for Stress - Grade Lumber and Its Fastenings" (AFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for Trussed Rafters. Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to HIB-91 as published by the Truss Plate Institute, 583 D'Onofrio Drive, Suite 200, Madison, Wisconsin 53719. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF BUILDING DESIGNER TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.

Robbins Engineering, Inc.

CORPORATE HEADQUARTERS

P.O. Box 280055
Tampa, FL 33682-0055
800-282-1299 • Fax: 813-971-6117

Wood Truss Council of America
5937 Meadowood Dr., Ste. 14 • Madison, WI 53711-4125 • 608/274-3329 (fax)

Standard Responsibilities in the Design Process Involving Metal Plate Connected Wood Trusses

WTCA 1-1995

Developed by the WTCA Engineering Review Committee
in cooperation with the Truss Plate Institute

The Wood Truss Council of America publishes standard practice materials prepared and edited by knowledgeable authors from the construction industry to give as much assistance as possible to owners, architects, engineers, contractors, building officials, and others involved in the metal plate connected wood truss industry. The competence of the authors ensures accurate and authoritative information in regard to the subject matter covered, but, of course, neither the Wood Truss Council of America, nor the authors make either express or implied warranties in regard to the use of the materials.

1.0 SCOPEAND DEFINITIONS

1.1 This standard defines the design responsibilities of the individuals and organizations involved in the preparation, submittal, review and approval of each Truss Design Drawing and Truss Placement Plan associated with the use of metal plate connected wood trusses. These guidelines are presented as industry standard practice. The guidelines are not intended to preclude alternate provisions as agreed upon by the parties involved.

1.2 The following definitions shall apply:

1.2.1 "Architect" shall mean the individual registered architect responsible for the architectural design of the structure and who produces the architectural drawings included in the Construction Design Documents.

1.2.2 "Building Designer" is the individual or organization having responsibility for the overall building or structure design in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. This responsibility includes but is not limited to foundation design, structural member sizing, load transfer, bearing conditions, and the structure's compliance with the applicable building codes. Also referred to as registered architect or engineer, building designer, and registered building designer, but hereinafter will be referred to as Building Designer.

1.2.3 "Construction Design Documents" are the architectural drawings, structural drawings, mechanical drawings, electrical drawings, and any other drawings, specifications, and addenda which set forth the overall design of the structure and issued by the Building Designer.

1.2.4 "Contractor" shall mean the individual or organization responsible for the field storage, handling, and installation of trusses including, but not limited to, temporary bracing, permanent bracing, anchorage, connections and field assembly. The term "Contractor" shall include those subcontractors who have a direct contract with the Contractor to perform all or a portion of the storage, handling, and installation of the trusses.

1.2.5 "Engineer-of-Record" shall mean the registered professional engineer responsible for the structural design of the structure and who produces the structural drawings included in the Construction Design Documents.

1.2.6 "Owner" shall mean the individual or organization for whom the structure is designed.

1.2.7 "Truss" is an individual metal plate connected wood structural component manufactured by the Truss Manufacturer.

1.2.8 "Truss Designer" is the design professional, individual or organization, having responsibility for the design of metal plate connected wood trusses. This responsibility shall be in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. Also referred to as truss engineer, design engineer, registered engineer, and engineer, but hereinafter will be referred to as Truss Designer.

1.2.9 "Truss Design Drawing" shall mean the graphic depiction of an individual Truss prepared by the Truss Designer.

1.2.10 "Truss Manufacturer" shall mean an individual or organization regularly engaged in the manufacturing of Trusses.

1.2.11 "Truss Placement Plan" is the drawing identifying the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.

2.0 OWNER RESPONSIBILITIES

2.1 Directly or through its representatives, which may include the Contractor and/or Building Designer; (a) review and approve each Truss Design Drawing; (b) review and approve the Truss Placement Plan; (c) resolve and approve all design issues arising out of the preparation of each Truss Design Drawing and Truss Placement Plan; and (d) coordinate the return of each approved Truss Design Drawing and Truss Placement Plan to the Truss Manufacturer prior to truss manufacturing.

3.0 BUILDING DESIGNER RESPONSIBILITIES

3.1 Design a structure suitable to ensure that the intended function of each Truss is not affected by adverse influences including, but not limited to, moisture, temperature, corrosive chemicals and gases;

3.2 Prepare the Construction Design Documents, showing all trussed areas, which must provide as a minimum the following:

3.2.1 All truss orientations and locations;

3.2.2 Information to fully determine all truss profiles;

3.2.3 Adequate support of the Truss and all truss bearing conditions;

3.2.4 Permanent bracing design for the structure including the Trusses, except as provided in 3.4 and 6.2.12.

3.2.5 The location, direction and magnitude of all dead and live loads applicable to each Truss including, but not limited to, loads attributable to: roof, floor, partition, mechanical, fire sprinkler, attic, storage, wind, snow drift and seismic;

3.2.6 All Truss anchorage designs required to resist uplift, gravity, and lateral loads;

3.2.7 Allowable vertical and horizontal deflection criteria;

3.2.8 Proper transfer of design loads affecting the Truss; and

3.2.9 Adequate connections between Truss and non-Truss components, except as noted in Section 6.2.9.

3.3 Review and approve the Truss Placement Plan and each Truss Design Drawing for conformance with the requirements and intent of the Construction Design Documents, the effect of each Truss Design Drawing and Truss Placement Plan on other parts of the structure, and the effect of the structure on each Truss.

3.4 Specify permanent lateral bracing where indicated by the Truss Designer on the Truss Design Drawings, to prevent buckling of the individual truss members due to design loads. The Building Designer shall specify how the permanent lateral bracing is to be anchored or restrained to prevent lateral movement if all truss members, so braced, buckle together. This shall be accomplished by: (a) anchorage to solid end walls; (b) permanent diagonal bracing in the plane of the web members; or (c) other means when demonstrated by the Building Designer to provide equivalent bracing.

4.0 CONTRACTOR RESPONSIBILITIES

4.1 Provide to the Truss Manufacturer the Construction Design Documents and all revisions and supplements thereto.

4.2 Review and approve the Truss Placement Plan and each Truss Design Drawing for conformance with the requirements and intent of the Construction Design Documents, and the effect of the Truss Placement Plan and each Truss Design Drawing on other trades involved in the construction of the structure and the effect of the other trades on the Trusses.

4.3 Coordinate the review, approval and return of each Truss Design Drawing and the Truss Placement Plan by the Owner and Building Designer.

4.4 Provide the approved Truss Design Drawings, approved Truss Placement Plans, and any supplemental information provided by the Truss Manufacturer to the individual or organization responsible for the installation of the Trusses.

4.5 Comply with the field storage, handling, installation, permanent bracing, anchorage, connections and field assembly requirements of the Construction Design Documents.

4.6 Determine and install the temporary bracing for the structure, including the Trusses.

5.0 TRUSS MANUFACTURER RESPONSIBILITIES

5.1 Communicate the design criteria from the Construction Design Documents to the Truss Designer.

5.2 Where required by the Construction Design Documents, prepare the Truss Placement Plan, providing as a minimum the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.

5.3 Submit to the Contractor the Truss Placement Plan, as may be required, and each Truss Design Drawing for review and approval.

5.4 Manufacture the Trusses in accordance with the final approved Truss Design Drawings using the quality criteria for Metal Plate Connected Wood Trusses established by the ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."

6.0 TRUSS DESIGNER RESPONSIBILITIES

6.1 Prepare the Truss Design Drawings in conformance with the requirements set forth in the latest approved edition of ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."

6.2 For each Truss Design Drawing, set forth as a minimum the following:

6.2.1 Slope or depth, span and spacing;

6.2.2 Location of all joints;

6.2.3 Required bearing widths;

6.2.4 Design loads as applicable:

6.2.4.1 Top chord live load (including snow loads);

6.2.4.2 Top chord dead load;

6.2.4.3 Bottom chord live load;

6.2.4.4 Bottom chord dead load;

6.2.4.5 Concentrated loads and their points of application; and

6.2.4.6 Controlling wind and earthquake loads;

6.2.5 Adjustments to lumber and metal connector plate design values for conditions of use;

6.2.6 Each reaction force and direction;

6.2.7 Metal connector plate type, size, thickness or gage, and the dimensioned location of each metal connector plate except where symmetrically located relative to the joint interface;

6.2.8 Lumber size, species, and grade for each member;

6.2.9 Connection Requirements for:

(a) Truss to Truss girder;

(b) Truss ply to ply; and

(c) Field splices;

6.2.10 Calculated deflection ratio and/or maximum deflection for live and total load;

6.2.11 Maximum axial compression forces in the Truss members to enable the Building Designer to design the size, connections and anchorage of the permanent continuous lateral bracing. Forces may be shown on the Truss Design Drawing or on supplemental documents; and

6.2.12 Required permanent Truss member bracing location.

7.0 OTHER RESPONSIBILITIES

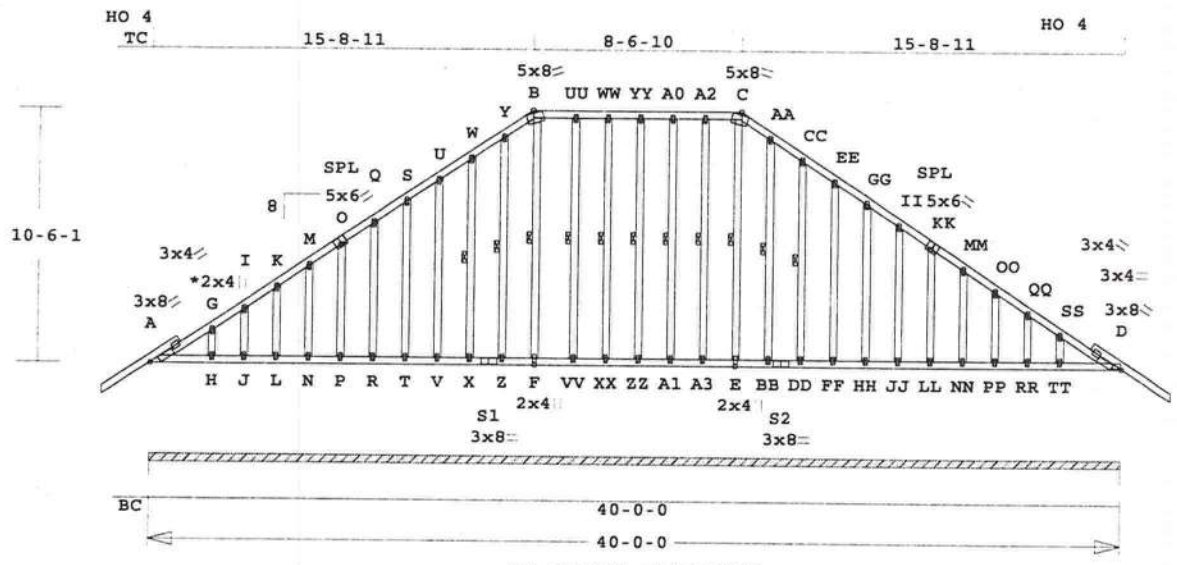
7.1 Any party who cuts or damages a truss shall be responsible for securing the engineering required for the repair and for subsequent costs.

Wood Truss Council of America's Objective

WTCA is committed to promoting the common interests of all engaged in the manufacture of wood trusses and related components to ensure growth, continuity, and increased professionalism in our industry. Fundamental to this is promoting the safe, economic, and structurally sound use of trusses in all applications.

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
050984	R1	1	HIPP	400000	8	0	0	

GREEN RESIDENCE



ALL PLATES ARE LOCK20

See Joint G For Typical Gable Plate Size and Placement

Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 502.4 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.05	2x 4	SP-#2
BC	0.06	2x 4	SP-#2
WB	0.02	2x 4	SP-#3
GW	0.06	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	40- 0- 0
BC	Cont.	0- 0- 0	40- 0- 0

WB 1 rows CLB on F -B
WB 1 rows CLB on E -C
WB 1 rows CLB on X -W
WB 1 rows CLB on Z -Y
WB 1 rows CLB on VV-UU
WB 1 rows CLB on XX-WW
WB 1 rows CLB on ZZ-YY
WB 1 rows CLB on A1-A0
WB 1 rows CLB on A3-A2
WB 1 rows CLB on BB-AA
WB 1 rows CLB on DD-CC
Attach CLB with (2)-8d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0	to	40- 0- 0	
	2893	717	Hz =	434

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -G	0.05	323 C	0.02 0.03
G -I	0.03	248 C	0.02 0.01
I -K	0.02	206 C	0.02 0.00
K -M	0.02	177 C	0.02 0.00
M -O	0.01	149 C	0.00 0.01
O -Q	0.01	125 T	0.00 0.01
Q -S	0.01	107 T	0.00 0.01
S -U	0.02	113 T	0.01 0.01
U -W	0.02	141 T	0.01 0.01
W -Y	0.03	189 T	0.02 0.01
Y -B	0.03	222 T	0.02 0.01
B -UU	0.03	210 T	0.02 0.01
UU-WW	0.02	210 T	0.02 0.00
WW-YY	0.02	210 T	0.02 0.00
YY-A0	0.02	210 T	0.02 0.00
A0-A2	0.02	210 T	0.02 0.00
A2-C	0.02	210 T	0.02 0.00
C -AA	0.03	222 T	0.02 0.01
AA-CC	0.03	189 T	0.02 0.01
CC-EE	0.02	141 T	0.01 0.01
EE-GG	0.02	113 T	0.01 0.01
GG-II	0.01	104 T	0.00 0.01
II-KK	0.01	122 T	0.00 0.01
KK-MM	0.01	146 C	0.00 0.01
MM-OO	0.02	174 C	0.02 0.00
OO-QQ	0.02	204 C	0.02 0.00
QQ-SS	0.03	248 C	0.02 0.01
SS-D	0.05	323 C	0.02 0.03
--------Bottom Chords---------			
A -H	0.06	0 T	0.00 0.06
H -J	0.01	0 T	0.00 0.01
J -L	0.00	0 T	
L -N	0.00	0 T	
N -P	0.00	0 T	
P -R	0.00	0 T	
R -T	0.00	0 T	
T -V	0.00	0 T	
V -X	0.00	0 T	
X -S1	0.00	0 T	
S1-Z	0.00	0 T	
Z -F	0.00	0 T	
F -VV	0.00	0 T	
VV-XX	0.00	0 T	
XX-ZZ	0.00	0 T	
ZZ-A1	0.00	0 T	
A1-A3	0.00	0 T	
A3-E	0.00	0 T	
E -BB	0.00	0 T	
BB-S2	0.00	0 T	
S2-DD	0.00	0 T	
DD-FF	0.00	0 T	
FF-HH	0.00	0 T	
HH-JJ	0.00	0 T	
JJ-LL	0.00	0 T	
LL-NN	0.00	0 T	
NN-PP	0.00	0 T	
PP-RR	0.00	0 T	
RR-TT	0.01	0 T	0.00 0.01
TT-D	0.06	0 T	0.00 0.06
----------Webs----------			
F -B	0.02	65 C	1 Br
E -C	0.02	58 C	1 Br
----------Gable Webs----------			
H -G	0.03	122 T	
J -I	0.01	72 C	
L -K	0.01	80 C	
N -M	0.02	80 C	
P -O	0.02	80 C	
R -Q	0.04	80 C	
T -S	0.05	80 C	
V -U	0.06	80 C	
X -W	0.02	85 C	1 Br
Z -Y	0.02	70 C	1 Br
VV-UU	0.03	80 C	1 Br
XX-WW	0.02	71 C	1 Br
ZZ-YY	0.02	71 C	1 Br
A1-A0	0.03	72 C	1 Br
A3-A2	0.02	71 C	1 Br
BB-AA	0.02	70 C	1 Br
DD-CC	0.02	85 C	1 Br
FF-EE	0.06	80 C	
HH-GG	0.05	80 C	
JJ-II	0.04	80 C	
LL-KK	0.02	80 C	
NN-MM	0.02	80 C	
PP-OO	0.01	80 C	
RR-QQ	0.01	72 C	
TT-SS	0.03	122 T	

LL Defl 0.00" in A -H L/999
TL Defl 0.00" in A -H L/999
Shear // Grain in A -G 0.06

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	Ctr	Ctr	0.79
G	LOCK	2.0x 4.0	Ctr	Ctr	0.00
I	LOCK	2.0x 4.0	Ctr	Ctr	0.00
K	LOCK	2.0x 4.0	Ctr	Ctr	0.00
M	LOCK	2.0x 4.0	Ctr	Ctr	0.00
O	LOCK	5.0x 6.0	0.4-0.6		0.75
Q	LOCK	2.0x 4.0	Ctr	Ctr	0.00
S	LOCK	2.0x 4.0	Ctr	Ctr	0.00
U	LOCK	2.0x 4.0	Ctr	Ctr	0.00
W	LOCK	2.0x 4.0	Ctr	Ctr	0.00
Y	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B	LOCK	5.0x 8.0	1.1-3.6		0.75
UU	LOCK	2.0x 4.0	Ctr	Ctr	0.00
WW	LOCK	2.0x 4.0	Ctr	Ctr	0.00
YY	LOCK	2.0x 4.0	Ctr	Ctr	0.00
A0	LOCK	2.0x 4.0	Ctr	Ctr	0.00
A2	LOCK	2.0x 4.0	Ctr	Ctr	0.00
C	LOCK	5.0x 8.0	-1.1-3.6		0.75
AA	LOCK	2.0x 4.0	Ctr	Ctr	0.00
CC	LOCK	2.0x 4.0	Ctr	Ctr	0.00

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering Cont.
050984	R1	1	HIPP	400000	8	0	0	

GREEN RESIDENCE

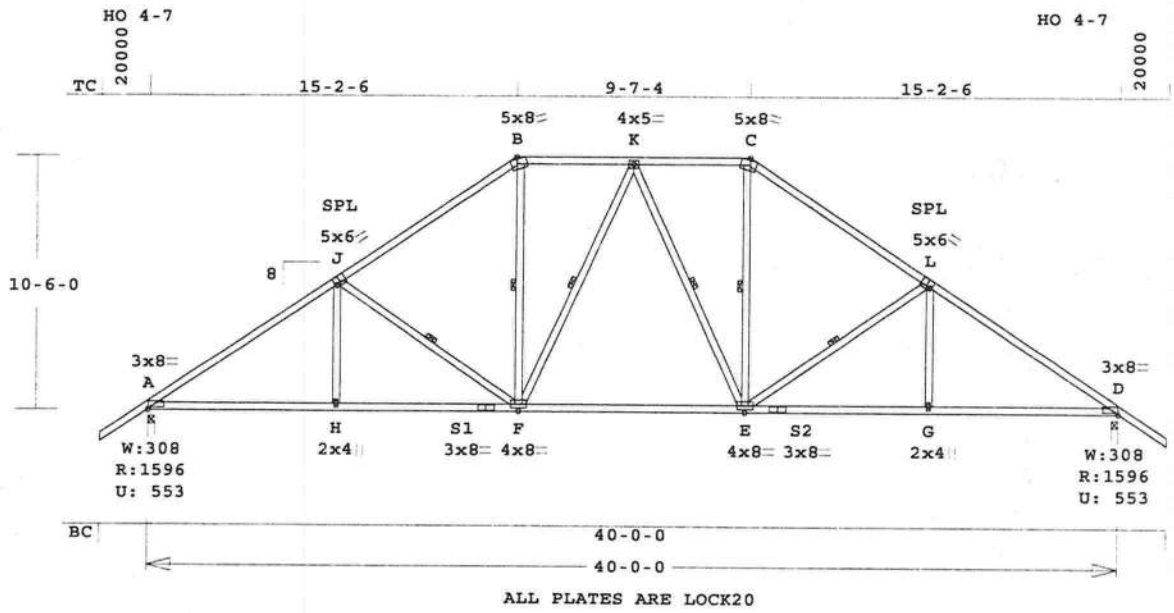
EE	LOCK	2.0x 4.0	Ctr Ctr	0.00
GG	LOCK	2.0x 4.0	Ctr Ctr	0.00
II	LOCK	2.0x 4.0	Ctr Ctr	0.00
KK	LOCK	5.0x 6.0-0.4-0.6		0.75
MM	LOCK	2.0x 4.0	Ctr Ctr	0.00
OO	LOCK	2.0x 4.0	Ctr Ctr	0.00
QQ	LOCK	2.0x 4.0	Ctr Ctr	0.00
SS	LOCK	2.0x 4.0	Ctr Ctr	0.00
D	LOCK	3.0x 8.0	Ctr Ctr	0.79
H	LOCK	2.0x 4.0	Ctr Ctr	0.00
J	LOCK	2.0x 4.0	Ctr Ctr	0.00
L	LOCK	2.0x 4.0	Ctr Ctr	0.00
N	LOCK	2.0x 4.0	Ctr Ctr	0.00
P	LOCK	2.0x 4.0	Ctr Ctr	0.00
R	LOCK	2.0x 4.0	Ctr Ctr	0.00
T	LOCK	2.0x 4.0	Ctr Ctr	0.00
V	LOCK	2.0x 4.0	Ctr Ctr	0.00
X	LOCK	2.0x 4.0	Ctr Ctr	0.00
S1	LOCK	3.0x 8.0	Ctr Ctr	0.88
Z	LOCK	2.0x 4.0	Ctr Ctr	0.00
F	LOCK	2.0x 4.0	Ctr Ctr	0.47
VV	LOCK	2.0x 4.0	Ctr Ctr	0.00
XX	LOCK	2.0x 4.0	Ctr Ctr	0.00
ZZ	LOCK	2.0x 4.0	Ctr Ctr	0.00
A1	LOCK	2.0x 4.0	Ctr Ctr	0.00
A3	LOCK	2.0x 4.0	Ctr Ctr	0.00
E	LOCK	2.0x 4.0	Ctr Ctr	0.47
BB	LOCK	2.0x 4.0	Ctr Ctr	0.00
S2	LOCK	3.0x 8.0	Ctr Ctr	0.88
DD	LOCK	2.0x 4.0	Ctr Ctr	0.00
FF	LOCK	2.0x 4.0	Ctr Ctr	0.00
HH	LOCK	2.0x 4.0	Ctr Ctr	0.00
JJ	LOCK	2.0x 4.0	Ctr Ctr	0.00
LL	LOCK	2.0x 4.0	Ctr Ctr	0.00
NN	LOCK	2.0x 4.0	Ctr Ctr	0.00
PP	LOCK	2.0x 4.0	Ctr Ctr	0.00
RR	LOCK	2.0x 4.0	Ctr Ctr	0.00
TT	LOCK	2.0x 4.0	Ctr Ctr	0.00

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
Provide connection to bearing
for 433 Lbs Horiz Reaction
Prevent truss rotation at all
bearing locations.
Refer to Gen Det 3 series for
web bracing and plating.
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 323 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
050984	R2	6	HIPP	400000	8	2- 0- 0	2- 0- 0	

GREEN RESIDENCE



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 303.5 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.54	2x 4	SP-#2
BC	0.53	2x 4	SP-#2
WB	0.19	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	40- 0- 0
BC Cont.	0- 0- 0	40- 0- 0

WB 1 rows CLB on J -F
WB 1 rows CLB on F -B
WB 1 rows CLB on F -K
WB 1 rows CLB on K -E
WB 1 rows CLB on E -C
WB 1 rows CLB on E -L
Attach CLB with (2)-8d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1596	554	3- 8	1-14
			Hz =	-429
D	1596	554	3- 8	1-14
			Hz =	430

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -J	0.54	2153 C	0.08 0.46
J -B	0.53	1658 C	0.07 0.46
B -K	0.20	1371 C	0.07 0.13
K -C	0.20	1371 C	0.07 0.13
C -L	0.53	1658 C	0.07 0.46
L -D	0.54	2153 C	0.08 0.46
Bottom Chords			
A -H	0.46	1798 T	0.30 0.16
H -S1	0.46	1798 T	0.30 0.16
S1-F	0.53	1798 T	0.30 0.23
F -E	0.46	1428 T	0.23 0.23
E -S2	0.53	1798 T	0.30 0.23
S2-G	0.46	1798 T	0.30 0.16
G -D	0.46	1798 T	0.30 0.16
Webs			
H -J	0.04	151 T	
J -F	0.17	536 C	1 Br
F -B	0.19	596 T	1 Br
F -K	0.08	322 T	1 Br
K -E	0.06	261 T	1 Br
E -C	0.19	596 T	1 Br
E -L	0.17	536 C	1 Br
G -L	0.04	151 T	

LL Defl -0.09" in F -E L/999
TL Defl -0.29" in F -E L/999
Shear // Grain in A -J 0.23

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

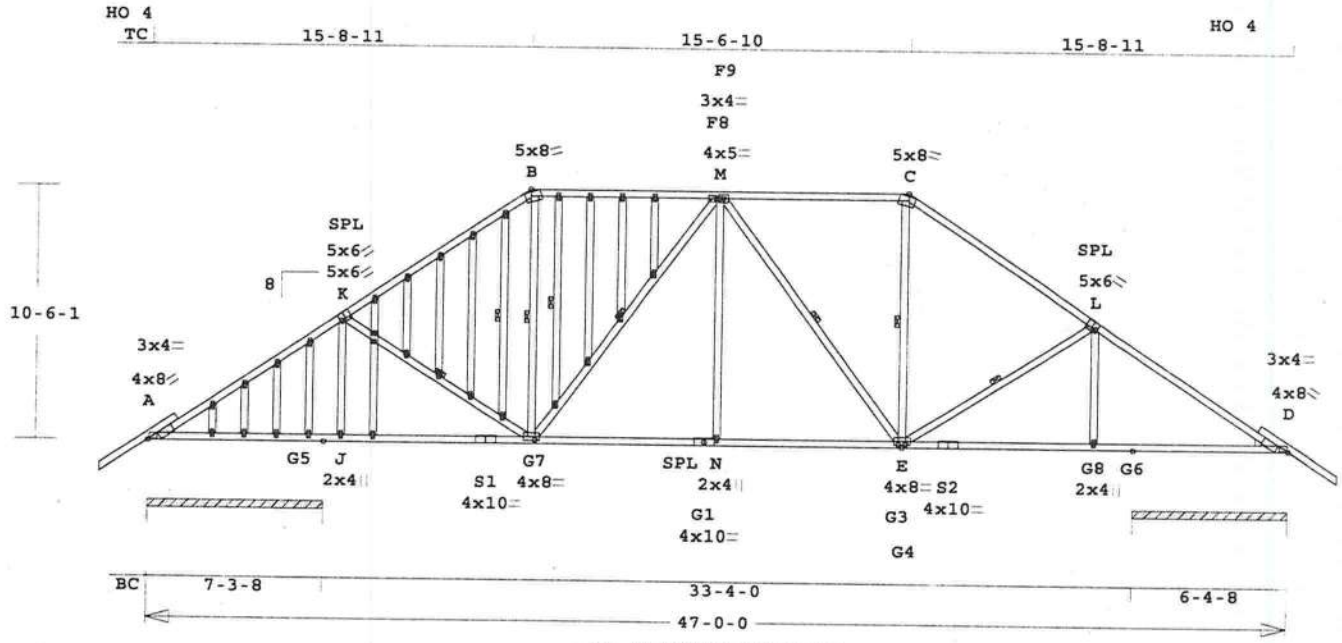
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 8.0	1.5	0.7	0.99
J	LOCK	5.0x 6.0	0.4-0.6		0.75
B	LOCK	5.0x 8.0	1.1-3.6		0.75
K	LOCK	4.0x 5.0	Ctr	Ctr	0.58
C	LOCK	5.0x 8.0-1.1-3.6			0.75
L	LOCK	5.0x 6.0-0.4-0.6			0.75
D	LOCK	3.0x 8.0-1.5	0.7		0.99
H	LOCK	2.0x 4.0	Ctr	Ctr	0.47
S1	LOCK	3.0x 8.0	Ctr	Ctr	0.88
F	LOCK	4.0x 8.0	Ctr	Ctr	0.58
E	LOCK	4.0x 8.0	Ctr	Ctr	0.58
S2	LOCK	3.0x 8.0	Ctr	Ctr	0.88
G	LOCK	2.0x 4.0	Ctr	Ctr	0.47

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
OH Loading
Soffit psf 2.0
Provide connection to bearing for 429 Lbs Horiz Reaction
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2153 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
050984	R3	1	HIPP	470000	8	0	0	

GREEN RESIDENCE



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 478.6 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.80	2x 4	SP-#2
BC	0.92	2x 4	SP-#2
WB	0.21	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	47- 0- 0
BC Cont.	0- 0- 0	47- 0- 0

WB 1 rows CLB on K -G7
WB 1 rows CLB on G7-B
WB 1 rows CLB on G7-F8
WB 1 rows CLB on M -E
WB 1 rows CLB on E -C
WB 1 rows CLB on E -L
WB 1 rows CLB on PP-OO
WB 1 rows CLB on B9-B8
Attach CLB with (2)-8d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to		7- 3- 8	
	1726	564	Hz =	434
Cont. Brg	40- 7- 8 to		47- 0- 0	
	1694	526	Hz =	434

Membr	CSI	P Lbs	Axl	CSI-Bnd
----------Top Chords----------				
A -O	0.29	1750	C	0.01 0.28
O -Q	0.15	1688	C	0.14 0.01
Q -S	0.15	1706	C	0.14 0.01
S -U	0.33	1621	C	0.01 0.32
U -K	0.33	1765	C	0.01 0.32
K -Y	0.21	1755	C	0.01 0.20
CC-K	0.40	1747	T	0.29 0.11
Y -CC	0.15	1706	C	0.14 0.01
K -GG	0.18	1723	C	0.08 0.10
GG-KK	0.17	1679	C	0.09 0.08
KK-OO	0.21	1655	C	0.07 0.14
OO-B	0.27	1563	C	0.01 0.26
B -B8	0.18	1399	C	0.08 0.10
B8-C2	0.24	1407	C	0.01 0.23
C2-C6	0.23	1419	C	0.01 0.22
C6-F8	0.23	1435	C	0.01 0.22
F8-D0	0.75	1554	T	0.26 0.49
D0-M	0.80	1601	C	0.01 0.79
M -C	0.64	1435	C	0.04 0.60
C -L	0.51	1725	C	0.08 0.43
L -D	0.51	1869	C	0.08 0.43
--------Bottom Chords---------				
A -P	0.33	281	C	0.00 0.33
P -R	0.05	281	C	0.00 0.05
R -T	0.04	281	C	0.00 0.04
T -V	0.41	281	C	0.00 0.41
V -J	0.64	1413	T	0.23 0.41
J -BB	0.39	1413	T	0.23 0.16
BB-S1	0.42	1428	T	0.23 0.19
S1-G7	0.59	1428	T	0.23 0.36
G7-G1	0.62	1601	T	0.26 0.36
G1-N	0.27	1601	T	0.26 0.01
N -E	0.59	1601	T	0.26 0.33
E -S2	0.59	1554	T	0.26 0.33
S2-G8	0.58	1554	T	0.26 0.32
G8-G6	0.92	1554	T	0.26 0.66
G6-D	0.92	1554	T	0.26 0.66
D -D	0.00	0	T	
------------Webs------------				
J -K	0.02	54	C	
K -AA	0.01	41	C	0.01 0.00
AA-DD	0.02	65	C	0.02 0.00
DD-HH	0.02	72	C	0.02 0.00
HH-LL	0.03	108	C	0.03 0.00
LL-PP	0.04	134	C	0.04 0.00
PP-G7	0.04	140	C	0.04 0.00
G7-B	0.14	452	T	1 Br
G7-B9	0.19	326	C	0.19 0.00
B9-C3	0.21	358	C	0.21 0.00
C3-C7	0.19	320	C	0.19 0.00
C7-D1	0.15	258	C	0.15 0.00
D1-F8	0.11	188	C	0.11 0.00
N -M	0.03	111	T	
M -E	0.16	277	C	1 Br
E -C	0.16	515	T	1 Br
E -L	0.10	305	C	1 Br
G8-L	0.09	243	C	

LL Defl -0.21" in G1-N L/999
TL Defl -0.43" in S2-G8 L/936
Shear // Grain in G8-G6 0.41

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
Plate - RHS 20 Ga, Gross Area					
A	LOCK	4.0x 8.0	1.0	0.4	0.74
O	LOCK	2.0x 4.0	Ctr	Ctr	0.52
Q	LOCK	2.0x 4.0	Ctr	Ctr	0.52
S	LOCK	2.0x 4.0	Ctr	Ctr	0.52
U	LOCK	2.0x 4.0	Ctr	Ctr	0.52
K	LOCK	5.0x 6.0	0.4-0.6		0.75
Y	LOCK	2.0x 4.0	Ctr	Ctr	0.52
CC	LOCK	2.0x 4.0	Ctr	Ctr	0.52
K	LOCK	5.0x 6.0	0.4-0.6		0.75
GG	LOCK	2.0x 4.0	Ctr	Ctr	0.52
KK	LOCK	2.0x 4.0	Ctr	Ctr	0.52
OO	LOCK	2.0x 4.0	Ctr	Ctr	0.52
B	LOCK	5.0x 8.0	1.1-3.6		0.75
B8	LOCK	2.0x 4.0	Ctr	Ctr	0.52
C2	LOCK	2.0x 4.0	Ctr	Ctr	0.52
C6	LOCK	2.0x 4.0	Ctr	Ctr	0.52
F8	LOCK	3.0x 4.0	-0.9	Ctr	0.39
D0	LOCK	2.0x 4.0	Ctr	Ctr	0.52
M	LOCK	4.0x 5.0	Ctr	Ctr	0.60
C	LOCK	5.0x 8.0	-1.1-3.6		0.75
L	LOCK	5.0x 6.0	-0.4-0.6		0.75
D	LOCK	4.0x 8.0	-1.0	0.4	0.74
P	LOCK	2.0x 4.0	Ctr	Ctr	0.52
R	LOCK	2.0x 4.0	Ctr	Ctr	0.52
T	LOCK	2.0x 4.0	Ctr	Ctr	0.52
V	LOCK	2.0x 4.0	Ctr	Ctr	0.52
J	LOCK	2.0x 4.0	Ctr	Ctr	0.52
BB	LOCK	2.0x 4.0	Ctr	Ctr	0.52
S1	LOCK	4.0x10.0	Ctr	0.2	0.75
G7	LOCK	4.0x 8.0	Ctr	Ctr	0.52
G1	LOCK	4.0x10.0	Ctr	0.2	0.75
N	LOCK	2.0x 4.0	Ctr	Ctr	0.52
E	LOCK	4.0x 8.0	Ctr	Ctr	0.52
S2	LOCK	4.0x10.0	Ctr	0.2	0.75
G8	LOCK	2.0x 4.0	Ctr	Ctr	0.52
AA	LOCK	4.0x 6.0	Ctr	Ctr	0.61
DD	LOCK	2.0x 4.0	Ctr	Ctr	0.52
HH	LOCK	2.0x 4.0	Ctr	Ctr	0.52
LL	LOCK	2.0x 4.0	Ctr	Ctr	0.52
PP	LOCK	2.0x 4.0	Ctr	Ctr	0.52
B9	LOCK	2.0x 4.0	Ctr	Ctr	0.52
C3	LOCK	2.0x 4.0	Ctr	Ctr	0.52
C7	LOCK	2.0x 4.0	Ctr	Ctr	0.52
D1	LOCK	2.0x 4.0	Ctr	Ctr	0.52

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
Provide connection to bearing for 433 Lbs Horiz Reaction
Wind Loads - ANSI / ASCE 7-98
Truss is designed as

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering Cont.*
050984	*R3*	1	HIPP	470000	8	0	0	

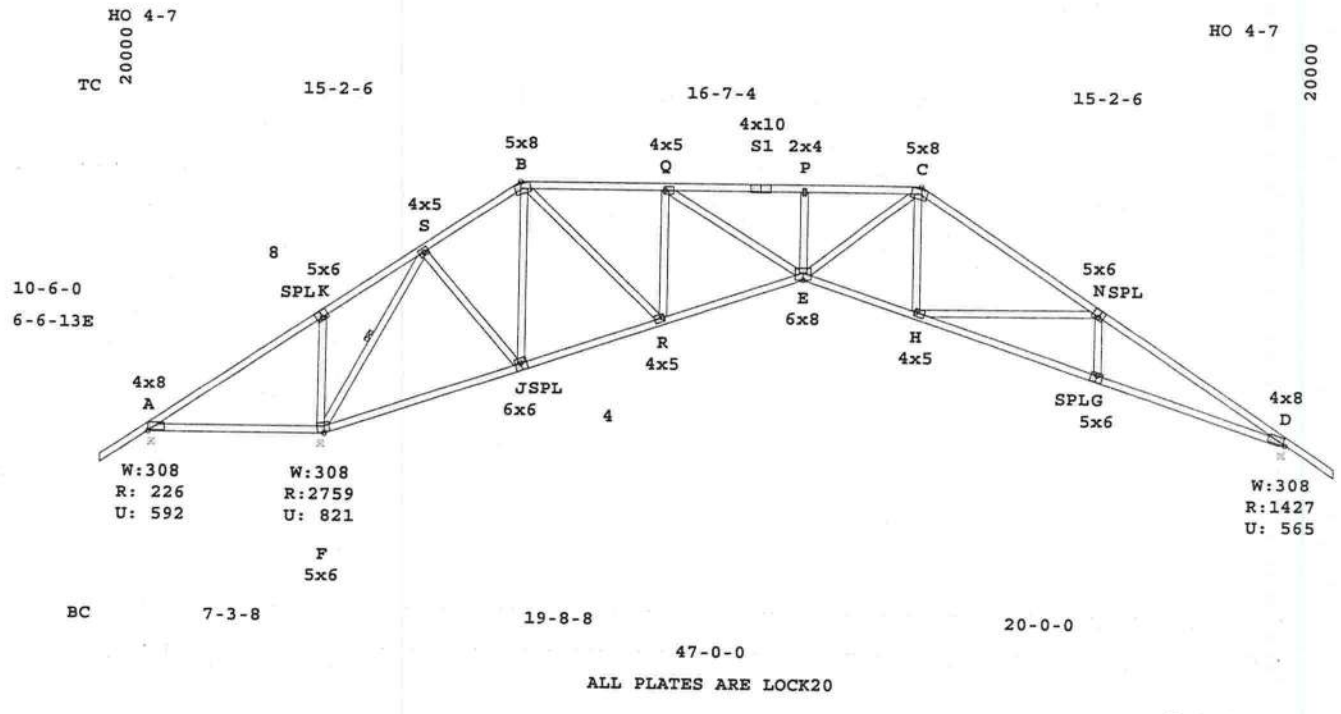
GREEN RESIDENCE

Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1869 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
050984	R4	14	SP	470000	8	2- 0- 0	2- 0- 0	

GREEN RESIDENCE



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 333.9 LBS

Online Plus -- Version 17.7.008
RUN DATE: 01-SEP-05

	CSI	-Size-	----Lumber----
TC	0.83	2x 4	SP-#2
BC	0.80	2x 4	SP-#2
WB	0.76	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	47- 0- 0
BC Cont.		0- 0- 0	47- 0- 0

WB 1 rows CLB on F -S
Attach CLB with (2)-8d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	226	593G	3- 8	1- 0
F	2760	821	3- 8	2-15
D	1427	565	3- 8	1-11

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -K	0.83	1512 T	0.27		0.56
K -S	0.79	1447 T	0.23		0.56
S -B	0.17	673 C	0.00		0.17
B -Q	0.28	1669 C	0.01		0.27
Q -S1	0.36	3160 C	0.12		0.24
S1-P	0.35	3160 C	0.35		0.00
P -C	0.30	3161 C	0.29		0.01
C -N	0.76	2636 C	0.23		0.53
N -D	0.61	3521 C	0.25		0.36
Bottom Chords					
A -F	0.61	1230 C	0.02		0.59
F -J	0.80	445 T	0.00		0.80
J -R	0.27	607 T	0.10		0.17
R -E	0.46	1751 T	0.29		0.17
E -H	0.53	2295 T	0.38		0.15
H -G	0.67	3085 T	0.52		0.15
G -D	0.73	3077 T	0.51		0.22
Webs					
F -K	0.15	424 C			
F -S	0.73	2403 C			1 Br
S -J	0.32	1004 T			
J -B	0.76	924 C			
B -R	0.48	1498 T			
R -Q	0.57	1228 C			
Q -E	0.55	1744 T			
E -P	0.06	262 C			
E -C	0.38	1211 T			
H -C	0.13	409 T			
H -N	0.65	801 C			
G -N	0.03	123 T			

LL Defl -0.16" in A -F L/500
TL Defl -0.33" in A -F L/245
Shear // Grain in A -K 0.26

Hz Disp	LL	DL	TL
Jt D	0.27"	0.22"	0.49"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

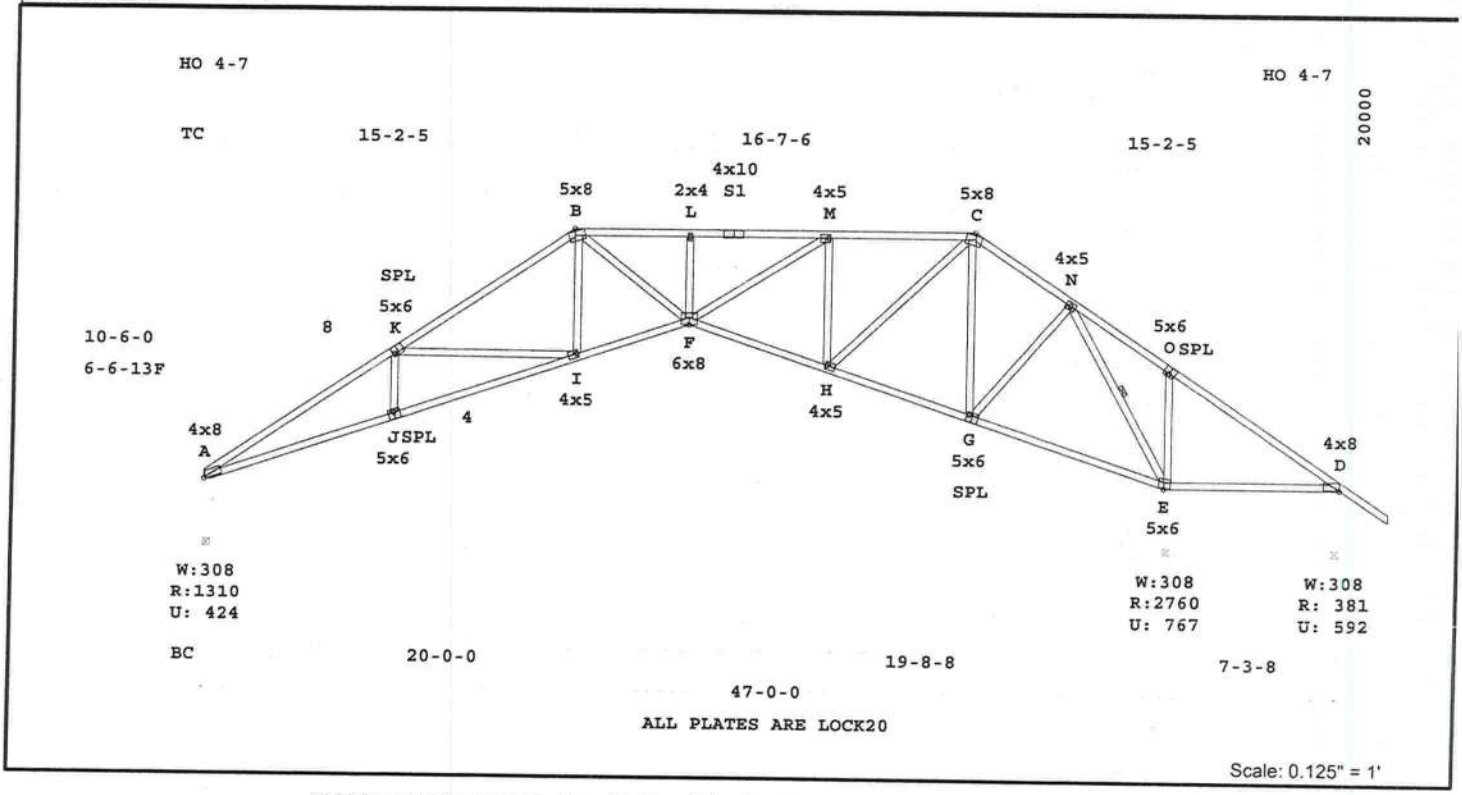
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 8.0	1.5	0.6	0.85
K	LOCK	5.0x 6.0	0.4	-0.6	0.75
S	LOCK	4.0x 5.0	Ctr	Ctr	0.68
B	LOCK	5.0x 8.0	1.1	-3.6	0.96
Q	LOCK	4.0x 5.0	Ctr	Ctr	0.65
S1	LOCK	4.0x10.0	Ctr	-0.2	0.75
P	LOCK	2.0x 4.0	Ctr	Ctr	0.52
C	LOCK	5.0x 8.0	-1.1	-3.6	0.75
N	LOCK	5.0x 6.0	-0.8	-0.4	0.75
D	LOCK	4.0x 8.0	-1.0	0.6	0.93
F	LOCK	5.0x 6.0	-0.5	2.9	0.75
J	LOCK	6.0x 6.0	-0.4	1.2	0.75
R	LOCK	4.0x 5.0	Ctr	Ctr	0.63
E	LOCK	6.0x 8.0	Ctr	-0.7	0.74
H	LOCK	4.0x 5.0	Ctr	Ctr	0.63
G	LOCK	5.0x 6.0	0.2	0.7	0.75

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
OH Loading
Soffit psf 2.0
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3521 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
050984	R5	1	SP	470000	8	0	2- 0- 0	

GREEN RESIDENCE



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 329.4 LBS

Online Plus -- Version 17.7.008
RUN DATE: 01-SEP-05

	CSI	Size	Lumber
TC	0.83	2x 4	SP-#2
BC	0.80	2x 4	SP-#2
WB	0.76	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	47- 0- 0
BC Cont.	0- 0- 0	47- 0- 0

WB 1 rows CLB on N -E
Attach CLB with (2)-8d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	1311	425	3- 8	1- 9
			Hz =	-429
E	2760	768	3- 8	2-15
D	381	593G	3- 8	1- 0
			Hz =	431

G = Gravity Uplift

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -K	0.60	3522 C	0.25		0.35
K -B	0.76	2636 C	0.23		0.53
B -L	0.30	3164 C	0.29		0.01
L -S1	0.35	3164 C	0.35		0.00
S1-M	0.36	3164 C	0.12		0.24
M -C	0.28	1673 C	0.01		0.27
C -N	0.17	671 C	0.00		0.17
N -O	0.79	1448 T	0.23		0.56
O -D	0.83	1513 T	0.27		0.56
Bottom Chords					
A -J	0.73	3078 T	0.51		0.22
J -I	0.67	3086 T	0.51		0.16
I -F	0.53	2297 T	0.38		0.15
F -H	0.46	1756 T	0.29		0.17
H -G	0.27	607 T	0.10		0.17
G -E	0.80	404 T	0.00		0.80
E -D	0.61	1230 C	0.02		0.59
Webs					
J -K	0.03	123 T			
K -I	0.65	801 C			
I -B	0.13	407 T			
B -F	0.38	1215 T			
F -L	0.06	262 C			
F -M	0.55	1743 T			
H -M	0.57	1227 C			
H -C	0.48	1501 T			
G -C	0.76	924 C			
G -N	0.32	1003 T			
N -E	0.73	2405 C			1 Br
E -O	0.15	425 C			

LL Defl -0.16" in E -D L/500
TL Defl -0.33" in E -D L/245
Shear // Grain in O -D 0.26

Hz Disp	LL	DL	TL
Jt E	0.28"	0.23"	0.51"

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

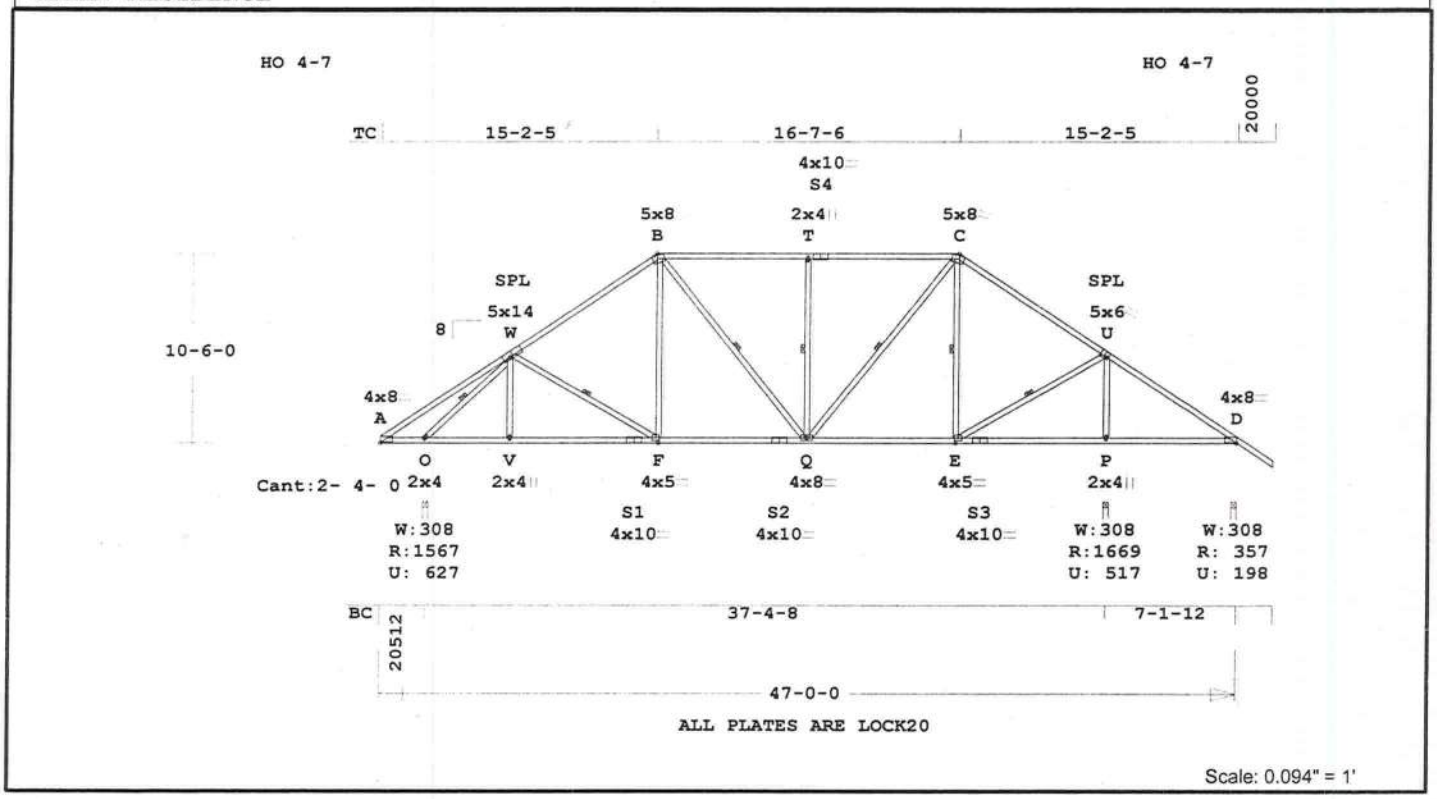
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 8.0	1.0	0.6	0.93
K	LOCK	5.0x 6.0	0.4-0.6		0.75
B	LOCK	5.0x 8.0	1.1-3.6		0.76
L	LOCK	2.0x 4.0	Ctr Ctr		0.52
S1	LOCK	4.0x10.0	Ctr-0.2		0.75
M	LOCK	4.0x 5.0	Ctr Ctr		0.65
C	LOCK	5.0x 8.0	1.1-3.6		0.96
N	LOCK	4.0x 5.0	Ctr Ctr		0.68
O	LOCK	5.0x 6.0	0.4-0.6		0.75
D	LOCK	4.0x 8.0	1.5	0.6	0.85
J	LOCK	5.0x 6.0	0.2	0.7	0.75
I	LOCK	4.0x 5.0	Ctr Ctr		0.63
F	LOCK	6.0x 8.0	Ctr-0.7		0.74
H	LOCK	4.0x 5.0	Ctr Ctr		0.63
G	LOCK	5.0x 6.0	0.2	0.7	0.75
E	LOCK	5.0x 6.0	0.5	2.9	0.75

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
OH Loading
Soffit psf 2.0
Provide connection to bearing for 430 Lbs Horiz Reaction
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 3522 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
050984	R6	7	HIPP	470000	8	0	2- 0- 0	

GREEN RESIDENCE



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 362.5 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.64	2x 4	SP-#2
BC	0.43	2x 4	SP-#2
WB	0.54	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	47- 0- 0
BC Cont.	0- 0- 0	47- 0- 0

WB 1 rows CLB on O -W
WB 1 rows CLB on W -F
WB 1 rows CLB on B -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on Q -C
WB 1 rows CLB on E -C
WB 1 rows CLB on E -U
Attach CLB with (2)-8d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
O	1567	627	3- 8	1-11
			Hz =	-427
P	1670	518	3- 8	1-12
D	357	198	3- 8	1- 0
			Hz =	429

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -W	0.54	162 T	0.04 0.50
W -B	0.54	1388 C	0.07 0.47
B -T	0.64	1269 C	0.01 0.63
T -S4	0.64	1269 C	0.01 0.63
S4-C	0.40	1269 C	0.03 0.37
C -U	0.58	1058 C	0.00 0.58
U -D	0.58	151 C	0.00 0.58
--------Bottom Chords---------			
A -O	0.33	194 T	0.00 0.33
O -V	0.37	1196 T	0.20 0.17
V -S1	0.34	1196 T	0.20 0.14
S1-F	0.43	1196 T	0.20 0.23
F -S2	0.42	1148 T	0.19 0.23
S2-Q	0.39	1148 T	0.19 0.20
Q -E	0.34	874 T	0.14 0.20
E -S3	0.19	367 T	0.00 0.19
S3-P	0.26	367 T	0.00 0.26
P -D	0.26	367 T	0.00 0.26
------------Webs------------			
O -W	0.43	1900 C	1 Br
V -W	0.03	113 T	
W -F	0.10	310 C	1 Br
F -B	0.06	260 T	
B -Q	0.21	335 C	1 Br
Q -T	0.19	481 C	1 Br
Q -C	0.20	631 T	1 Br
E -C	0.13	320 C	1 Br
E -U	0.30	946 T	1 Br
P -U	0.54	1483 C	

LL Defl -0.03" in P -D L/999
TL Defl -0.08" in P -D L/959
LL Cant -0.01" in A -O L/999
Shear // Grain in B -T 0.31

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

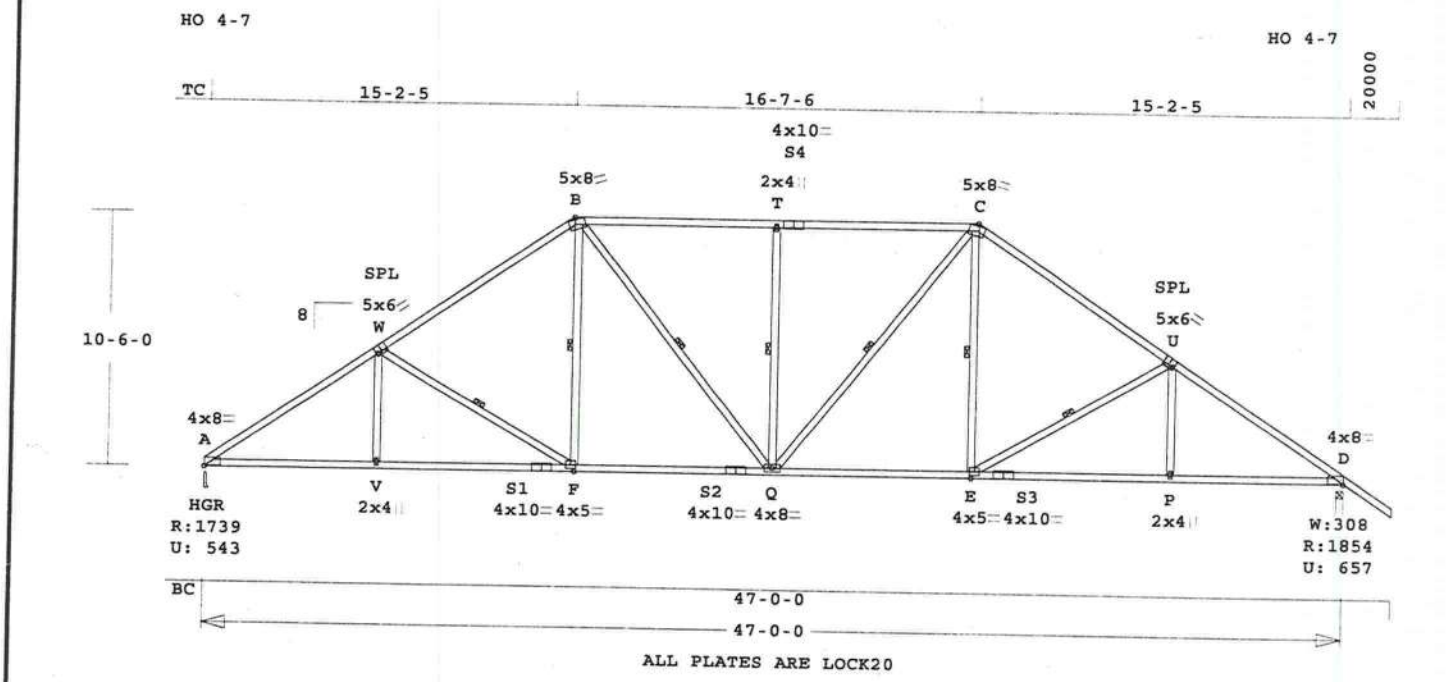
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 8.0	1.5	0.6	0.85
W	LOCK	5.0x14.0	0.4-0.6		0.98
B	LOCK	5.0x 8.0	1.1-3.6		0.75
T	LOCK	2.0x 4.0	Ctr	Ctr	0.52
S4	LOCK	4.0x10.0	Ctr-0.2		0.75
C	LOCK	5.0x 8.0	1.1-3.6		0.75
U	LOCK	5.0x 6.0	0.4-0.6		0.75
D	LOCK	4.0x 8.0	1.5	0.6	0.85
O	LOCK	2.0x 4.0	Ctr	Ctr	0.77
V	LOCK	2.0x 4.0	Ctr	Ctr	0.52
S1	LOCK	4.0x10.0	Ctr	0.2	0.75
F	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S2	LOCK	4.0x10.0	Ctr	0.2	0.75
Q	LOCK	4.0x 8.0	Ctr	Ctr	0.50
E	LOCK	4.0x 5.0	Ctr	Ctr	0.60
S3	LOCK	4.0x10.0	Ctr	0.2	0.75
P	LOCK	2.0x 4.0	Ctr	Ctr	0.52

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
OH Loading
Soffit psf 2.0
Provide connection to bearing for 429 Lbs Horiz Reaction
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1900 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
050984	R6B	3	HIPP	470000	8	0	2- 0- 0	

GREEN RESIDENCE



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 350.3 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.68	2x 4	SP-#2
BC	0.58	2x 4	SP-#2
WB	0.23	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	47- 0- 0
BC Cont.	0- 0- 0	47- 0- 0

WB 1 rows CLB on W -F
WB 1 rows CLB on F -B
WB 1 rows CLB on B -Q
WB 1 rows CLB on Q -T
WB 1 rows CLB on Q -C
WB 1 rows CLB on E -C
WB 1 rows CLB on E -U
Attach CLB with (2)-8d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1739	544	3- 8	2- 1
			Hz =	-428
D	1855	658	3- 8	2- 3
			Hz =	430

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -W	0.57	2643 C	0.11 0.46
W -B	0.67	2121 C	0.16 0.51
B -T	0.68	2023 C	0.02 0.66
T -S4	0.68	2023 C	0.02 0.66
S4-C	0.58	2023 C	0.09 0.49
C -U	0.67	2121 C	0.16 0.51
U -D	0.57	2642 C	0.11 0.46
--------Bottom Chords---------			
A -V	0.49	2203 T	0.36 0.13
V -S1	0.49	2203 T	0.36 0.13
S1-F	0.58	2203 T	0.36 0.22
F -S2	0.51	1759 T	0.29 0.22
S2-Q	0.48	1759 T	0.29 0.19
Q -E	0.51	1759 T	0.29 0.22
E -S3	0.58	2203 T	0.36 0.22
S3-P	0.49	2203 T	0.36 0.13
P -D	0.49	2203 T	0.36 0.13
------------Webs------------			
V -W	0.04	142 T	
W -F	0.18	539 C	1 Br
F -B	0.13	428 T	1 Br
B -Q	0.23	419 T	1 Br
Q -T	0.20	494 C	1 Br
Q -C	0.15	419 T	1 Br
E -C	0.13	428 T	1 Br
E -U	0.18	539 C	1 Br
P -U	0.04	142 T	

LL Defl -0.15" in S2-Q L/999
TL Defl -0.32" in Q -E L/999
Shear // Grain in B -T 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

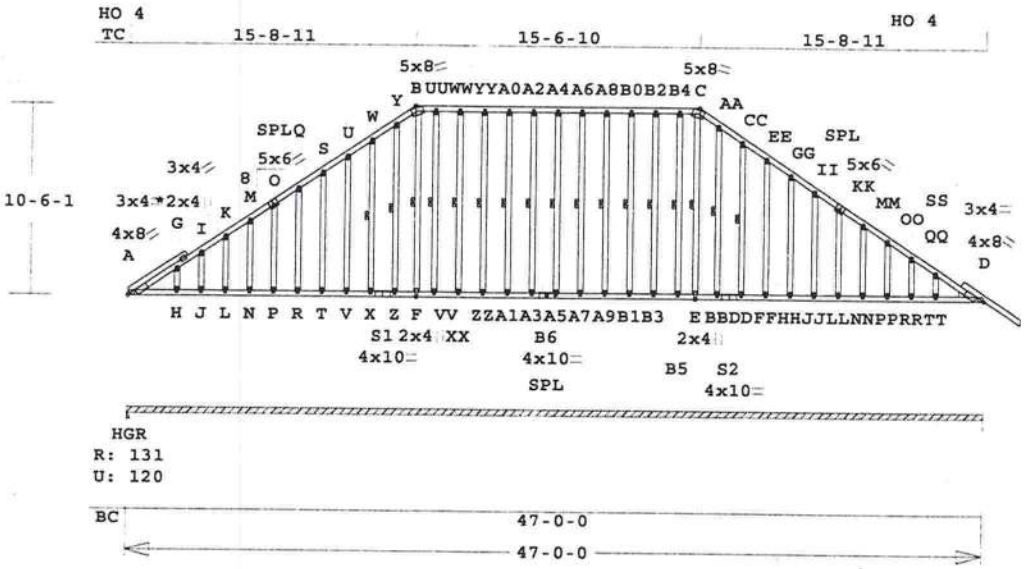
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 8.0	1.5	0.6	0.85
W	LOCK	5.0x 6.0	0.4-0.6		0.75
B	LOCK	5.0x 8.0	1.1-3.6		0.75
T	LOCK	2.0x 4.0	Ctr Ctr		0.52
S4	LOCK	4.0x10.0	Ctr-0.2		0.75
C	LOCK	5.0x 8.0	-1.1-3.6		0.75
U	LOCK	5.0x 6.0	-0.4-0.6		0.75
D	LOCK	4.0x 8.0	-1.5 0.6		0.85
V	LOCK	2.0x 4.0	Ctr Ctr		0.52
S1	LOCK	4.0x10.0	Ctr 0.2		0.75
F	LOCK	4.0x 5.0	Ctr Ctr		0.60
S2	LOCK	4.0x10.0	Ctr 0.2		0.75
Q	LOCK	4.0x 8.0	Ctr Ctr		0.50
E	LOCK	4.0x 5.0	Ctr Ctr		0.60
S3	LOCK	4.0x10.0	Ctr 0.2		0.75
P	LOCK	2.0x 4.0	Ctr Ctr		0.52

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
OH Loading
Soffit psf 2.0
Provide connection to bearing for 429 Lbs Horiz Reaction
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 2643 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
050984	R7	1	HIPP	470000	8	0	0	

GREEN RESIDENCE



ALL PLATES ARE LOCK20

See Joint G For Typical Gable Plate Size and Placement

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 645.4 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.05	2x 4	SP-#2
BC	0.06	2x 4	SP-#2
WB	0.03	2x 4	SP-#3
GW	0.07	2x 4	SP-#3
PB	---	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	47- 0- 0
BC Cont.	0- 0- 0	47- 0- 0

WB 1 rows CLB on F -B
WB 1 rows CLB on E -C
WB 1 rows CLB on X -W
WB 1 rows CLB on Z -Y
WB 1 rows CLB on VV-UU
WB 1 rows CLB on XX-WW
WB 1 rows CLB on ZZ-YY
WB 1 rows CLB on A1-A0
WB 1 rows CLB on A3-A2
WB 1 rows CLB on A5-A4
WB 1 rows CLB on A7-A6
WB 1 rows CLB on A9-A8
WB 1 rows CLB on B1-B0
WB 1 rows CLB on B3-B2
WB 1 rows CLB on B5-B4
WB 1 rows CLB on BB-AA
WB 1 rows CLB on DD-CC
Attach CLB with (2)-8d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	132	120	3- 8	1- 0
		Hz =	-433	
Cont. Brg	0- 0- 0 to		47- 0- 0	
	3425	1026	Hz =	434

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -G	0.05	323 T	0.04 0.01
G -I	0.04	299 T	0.03 0.01
I -K	0.03	283 T	0.03 0.00
K -M	0.03	265 T	0.03 0.00
M -O	0.03	247 T	0.03 0.00
O -Q	0.03	230 T	0.03 0.00
Q -S	0.02	212 T	0.02 0.00
S -U	0.03	194 T	0.02 0.01
U -W	0.03	221 T	0.02 0.01
W -Y	0.04	272 T	0.03 0.01
Y -B	0.05	295 T	0.03 0.02
B -UU	0.03	274 T	0.03 0.00
UU-WW	0.04	274 T	0.03 0.01
WW-YY	0.03	274 T	0.03 0.00
YY-A0	0.03	274 T	0.03 0.00
A0-A2	0.03	274 T	0.03 0.00
A2-A4	0.03	274 T	0.03 0.00
A4-A6	0.03	274 T	0.03 0.00
A6-A8	0.03	274 T	0.03 0.00
A8-B0	0.03	274 T	0.03 0.00
B0-B2	0.03	274 T	0.03 0.00
B2-B4	0.04	274 T	0.03 0.01
B4-C	0.03	274 T	0.03 0.00
C -AA	0.05	295 T	0.03 0.02
AA-CC	0.04	272 T	0.03 0.01
CC-EE	0.03	221 T	0.02 0.01
EE-GG	0.03	174 T	0.02 0.01
GG-II	0.02	127 T	0.01 0.01
II-KK	0.02	104 T	0.01 0.01
KK-MM	0.01	123 C	0.00 0.01
MM-OO	0.02	172 C	0.01 0.01
OO-QQ	0.02	222 C	0.02 0.00
QQ-SS	0.03	265 C	0.02 0.01
SS-D	0.04	343 C	0.02 0.02
Bottom Chords			
A -H	0.02	170 T	0.01 0.01
H -J	0.02	170 T	0.02 0.00
J -L	0.02	170 T	0.02 0.00
L -N	0.02	170 T	0.02 0.00
N -P	0.02	170 T	0.02 0.00
P -R	0.02	170 T	0.02 0.00
R -T	0.02	170 T	0.02 0.00
T -V	0.02	170 T	0.02 0.00
V -X	0.02	170 T	0.02 0.00
X -S1	0.02	170 T	0.02 0.00
S1-Z	0.02	170 T	0.02 0.00
Z -F	0.02	170 T	0.02 0.00
F -VV	0.02	170 T	0.02 0.00
VV-XX	0.02	170 T	0.02 0.00
XX-ZZ	0.02	170 T	0.02 0.00
ZZ-A1	0.02	170 T	0.02 0.00
A1-A3	0.02	170 T	0.02 0.00
A3-B6	0.02	170 T	0.02 0.00
B6-A5	0.02	170 T	0.02 0.00
A5-A7	0.02	170 T	0.02 0.00
A7-A9	0.02	170 T	0.02 0.00
A9-B1	0.02	170 T	0.02 0.00
B1-B3	0.02	170 T	0.02 0.00
B3-B5	0.02	170 T	0.02 0.00
B5-E	0.02	170 T	0.02 0.00
E -BB	0.02	170 T	0.02 0.00
BB-S2	0.02	170 T	0.02 0.00
S2-DD	0.02	170 T	0.02 0.00
DD-FF	0.02	170 T	0.02 0.00
FF-HH	0.02	170 T	0.02 0.00
HH-JJ	0.02	170 T	0.02 0.00
JJ-LL	0.02	170 T	0.02 0.00
LL-NN	0.02	170 T	0.02 0.00
NN-PP	0.02	170 T	0.02 0.00
PP-RR	0.02	170 T	0.02 0.00
RR-TT	0.02	170 T	0.02 0.00
TT-D	0.06	170 T	0.00 0.06
Webs			
F -B	0.03	88 C	1 Br
E -C	0.03	88 C	1 Br
Gable Webs			
H -G	0.02	103 C	
J -I	0.01	74 C	
L -K	0.01	80 C	
N -M	0.02	80 C	
P -O	0.03	80 C	
R -Q	0.04	80 C	
T -S	0.05	80 C	
V -U	0.07	81 C	
X -W	0.02	87 C	1 Br
Z -Y	0.02	67 C	1 Br
VV-UU	0.02	61 C	1 Br
XX-WW	0.03	73 C	1 Br
ZZ-YY	0.03	72 C	1 Br
A1-A0	0.02	71 C	1 Br
A3-A2	0.02	71 C	1 Br
A5-A4	0.02	71 C	1 Br
A7-A6	0.02	71 C	1 Br
A9-A8	0.02	71 C	1 Br
B1-B0	0.03	72 C	1 Br
B3-B2	0.03	73 C	1 Br
B5-B4	0.02	61 C	1 Br
BB-AA	0.02	67 C	1 Br
DD-CC	0.02	87 C	1 Br
FF-EE	0.07	81 C	
HH-GG	0.05	80 C	
JJ-II	0.04	80 C	
LL-KK	0.03	80 C	
NN-MM	0.02	80 C	
PP-OO	0.01	80 C	
RR-QQ	0.01	71 C	
TT-SS	0.03	128 T	

LL Defl 0.00" in
TL Defl 0.00" in
Shear // Grain in SS-D 0.06

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering Cont.
050984	R7	1	HIPP	470000	8	0	0	

GREEN RESIDENCE

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 8.0	1.0	0.4	0.74
G	LOCK	2.0x 4.0	Ctr	Ctr	0.00
I	LOCK	2.0x 4.0	Ctr	Ctr	0.00
K	LOCK	2.0x 4.0	Ctr	Ctr	0.00
M	LOCK	2.0x 4.0	Ctr	Ctr	0.00
O	LOCK	5.0x 6.0	0.4-0.6		0.75
Q	LOCK	2.0x 4.0	Ctr	Ctr	0.00
S	LOCK	2.0x 4.0	Ctr	Ctr	0.00
U	LOCK	2.0x 4.0	Ctr	Ctr	0.00
W	LOCK	2.0x 4.0	Ctr	Ctr	0.00
Y	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B	LOCK	5.0x 8.0	1.1-3.6		0.75
UU	LOCK	2.0x 4.0	Ctr	Ctr	0.00
WW	LOCK	2.0x 4.0	Ctr	Ctr	0.00
YY	LOCK	2.0x 4.0	Ctr	Ctr	0.00
A0	LOCK	2.0x 4.0	Ctr	Ctr	0.00
A2	LOCK	2.0x 4.0	Ctr	Ctr	0.00
A4	LOCK	2.0x 4.0	Ctr	Ctr	0.00
A6	LOCK	2.0x 4.0	Ctr	Ctr	0.00
A8	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B0	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B2	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B4	LOCK	2.0x 4.0	Ctr	Ctr	0.00
C	LOCK	5.0x 8.0	-1.1-3.6		0.75
AA	LOCK	2.0x 4.0	Ctr	Ctr	0.00
CC	LOCK	2.0x 4.0	Ctr	Ctr	0.00
EE	LOCK	2.0x 4.0	Ctr	Ctr	0.00
GG	LOCK	2.0x 4.0	Ctr	Ctr	0.00
II	LOCK	2.0x 4.0	Ctr	Ctr	0.00
KK	LOCK	5.0x 6.0	-0.4-0.6		0.75
MM	LOCK	2.0x 4.0	Ctr	Ctr	0.00
OO	LOCK	2.0x 4.0	Ctr	Ctr	0.00
QQ	LOCK	2.0x 4.0	Ctr	Ctr	0.00
SS	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	4.0x 8.0	-1.0	0.4	0.74
H	LOCK	2.0x 4.0	Ctr	Ctr	0.00
J	LOCK	2.0x 4.0	Ctr	Ctr	0.00
L	LOCK	2.0x 4.0	Ctr	Ctr	0.00
N	LOCK	2.0x 4.0	Ctr	Ctr	0.00
P	LOCK	2.0x 4.0	Ctr	Ctr	0.00
R	LOCK	2.0x 4.0	Ctr	Ctr	0.00
T	LOCK	2.0x 4.0	Ctr	Ctr	0.00
V	LOCK	2.0x 4.0	Ctr	Ctr	0.00
X	LOCK	2.0x 4.0	Ctr	Ctr	0.00
S1	LOCK	4.0x10.0	Ctr	0.2	0.75
Z	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	2.0x 4.0	Ctr	Ctr	0.52
VV	LOCK	2.0x 4.0	Ctr	Ctr	0.00
XX	LOCK	2.0x 4.0	Ctr	Ctr	0.00
ZZ	LOCK	2.0x 4.0	Ctr	Ctr	0.00
A1	LOCK	2.0x 4.0	Ctr	Ctr	0.00
A3	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B6	LOCK	4.0x10.0	Ctr	0.2	0.75
A5	LOCK	2.0x 4.0	Ctr	Ctr	0.00
A7	LOCK	2.0x 4.0	Ctr	Ctr	0.00
A9	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B1	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B3	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B5	LOCK	2.0x 4.0	Ctr	Ctr	0.00
E	LOCK	2.0x 4.0	Ctr	Ctr	0.52
BB	LOCK	2.0x 4.0	Ctr	Ctr	0.00
S2	LOCK	4.0x10.0	Ctr	0.2	0.75
DD	LOCK	2.0x 4.0	Ctr	Ctr	0.00
FF	LOCK	2.0x 4.0	Ctr	Ctr	0.00
HH	LOCK	2.0x 4.0	Ctr	Ctr	0.00
JJ	LOCK	2.0x 4.0	Ctr	Ctr	0.00
LL	LOCK	2.0x 4.0	Ctr	Ctr	0.00
NN	LOCK	2.0x 4.0	Ctr	Ctr	0.00
PP	LOCK	2.0x 4.0	Ctr	Ctr	0.00
RR	LOCK	2.0x 4.0	Ctr	Ctr	0.00
TT	LOCK	2.0x 4.0	Ctr	Ctr	0.00

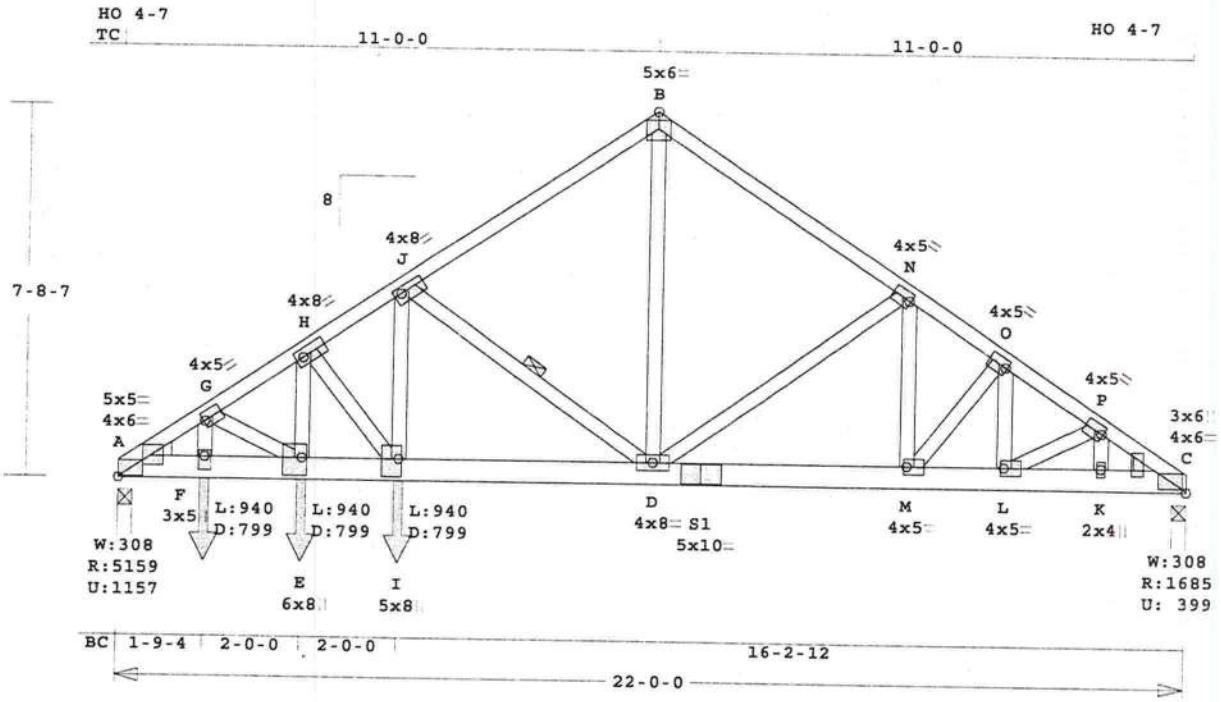
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
Provide connection to bearing
for 433 Lbs Horiz Reaction
Prevent truss rotation at all
bearing locations.

Refer to Gen Det 3 series for
web bracing and plating.
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 343 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
050984	R8	1	TR	220000	8	0	0	

GREEN RESIDENCE



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 192.8 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.78	2x 4	SP-#2
BC	0.61	2x 6	SP-2400
EX	S1-C	2x 6	SP-#2
WB	0.74	2x 4	SP-#3
EX	I -J	2x 4	SP-#2
WG	---	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	22- 0- 0
BC	Cont.	0- 0- 0	22- 0- 0

WB 1 rows CLB on J -D
Attach CLB with (2)-8d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	14	0.0'	22.0'
BC V	0	20	0.0'	22.0'
BC V	940	799	1.8'	CL-LB
BC V	940	799	3.8'	CL-LB
BC V	940	799	5.8'	CL-LB

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	5160	1158	3- 8	4- 4 **
			Hz =	-306
C	1685	399	3- 8	2- 0
			Hz =	307

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -G	0.78	8052 C	0.54	0.24
G -H	0.70	6953 C	0.43	0.27
H -J	0.51	5423 C	0.51	0.00
J -B	0.28	2306 C	0.08	0.20
B -N	0.28	2306 C	0.08	0.20
N -O	0.27	2593 C	0.09	0.18
O -P	0.25	2672 C	0.24	0.01
P -C	0.24	2608 C	0.24	0.00
Bottom Chords				
A -F	0.54	6652 T	0.33	0.21
F -E	0.61	6652 T	0.33	0.28
E -I	0.43	5802 T	0.29	0.14
I -D	0.28	4539 T	0.22	0.06
D -S1	0.14	2176 T	0.10	0.04
S1-M	0.33	2176 T	0.29	0.04
M -L	0.34	2227 T	0.29	0.05
L -K	0.40	2159 T	0.28	0.12
K -C	0.37	2159 T	0.28	0.09
Webs				
F -G	0.35	1120 T		
G -E	0.16	979 C		
E -H	0.74	2321 T		
H -I	0.40	2011 C		
I -J	0.62	3409 T		
J -D	0.69	3240 C		1 Br
D -B	0.72	2261 T		
D -N	0.18	322 C		
M -N	0.04	128 T		
M -O	0.01	87 C		
L -O	0.01	44 T		
L -P	0.02	78 T		
K -P	0.01	126 C		

LL Defl -0.12" in E -I L/999
TL Defl -0.22" in E -I L/999
Shear // Grain in F -F 0.40

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

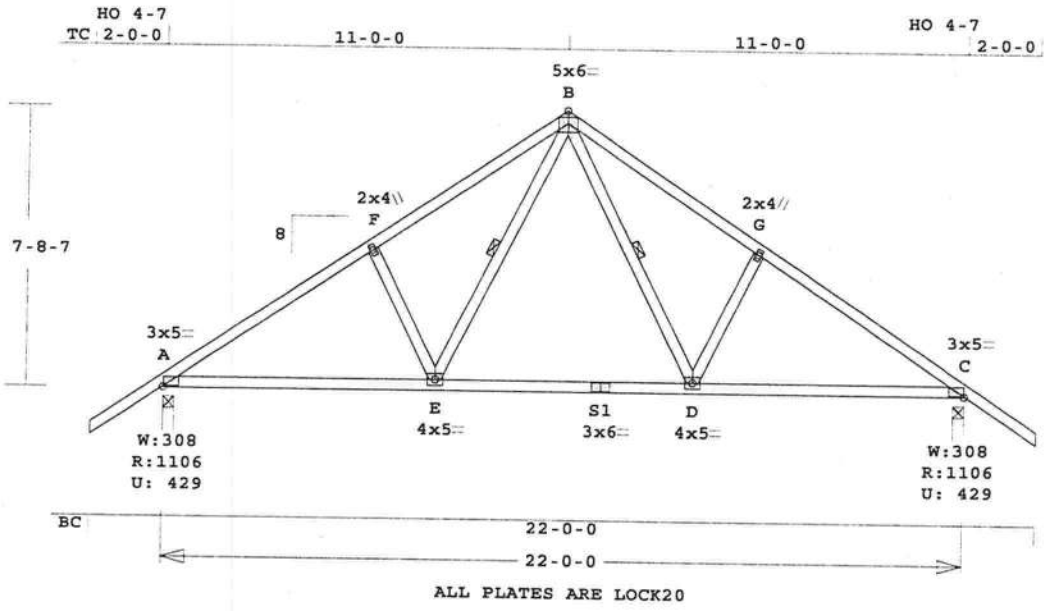
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	1.0	0.8	0.90
A	LOCK	5.0x 5.0	2.6	1.5	0.87
G	LOCK	4.0x 5.0	Ctr	Ctr	0.72
H	LOCK	4.0x 8.0	Ctr	Ctr	0.78
J	LOCK	4.0x 8.0	0.3	-0.4	0.86
B	LOCK	5.0x 6.0	Ctr	-0.3	0.75
N	LOCK	4.0x 5.0	Ctr	Ctr	0.72
O	LOCK	4.0x 5.0	Ctr	Ctr	0.72
P	LOCK	4.0x 5.0	Ctr	Ctr	0.72
C	LOCK	4.0x 6.0	Ctr	Ctr	0.60
C	LOCK	3.0x 6.0	Ctr	Ctr	0.00
F	LOCK	3.0x 5.0	Ctr	-1.0	0.80
E	LOCK	6.0x 8.0	Ctr	-0.6	0.87
I	LOCK	5.0x 8.0	Ctr	-0.6	0.88
D	LOCK	4.0x 8.0	Ctr	Ctr	0.76
S1	LOCK	5.0x10.0	Ctr	Ctr	0.82
M	LOCK	4.0x 5.0	Ctr	Ctr	0.60
L	LOCK	4.0x 5.0	Ctr	Ctr	0.60
K	LOCK	2.0x 4.0	Ctr	Ctr	0.40

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
Provide connection to bearing for 306 Lbs Horiz Reaction
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 8052 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
050984	R9	12	TR	220000	8	2- 0- 0	2- 0- 0	

GREEN RESIDENCE



Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 146.1 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.31	2x 4	SP-#2
BC	0.65	2x 4	SP-#2
WB	0.20	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	22- 0- 0
BC Cont.	0- 0- 0	22- 0- 0

WB 1 rows CLB on E -B
WB 1 rows CLB on B -D
Attach CLB with (2)-8d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	14	0.0'	22.0'
BC V	0	20	0.0'	22.0'
BC V	40	10	7.5'	14.5'

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1107	430	3- 8	1- 5
			Hz =	-308
C	1107	430	3- 8	1- 5
			Hz =	309

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -F	0.31	1404	C	0.07	0.24
F -B	0.31	1281	C	0.07	0.24
B -G	0.31	1281	C	0.07	0.24
G -C	0.31	1404	C	0.07	0.24
Bottom Chords					
A -E	0.65	1169	T	0.21	0.44
E -S1	0.60	779	T	0.14	0.46
S1-D	0.58	779	T	0.14	0.44
D -C	0.65	1169	T	0.21	0.44
Webs					
F -E	0.07	281	C		
E -B	0.20	634	T		1 Br
B -D	0.20	634	T		1 Br
D -G	0.07	281	C		

LL Defl -0.16" in E -S1 L/999
TL Defl -0.24" in E -S1 L/999
Shear // Grain in E -S1 0.32

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.68
F	LOCK	2.0x 4.0	Ctr	Ctr	0.37
B	LOCK	5.0x 6.0	Ctr	-0.3	0.75
G	LOCK	2.0x 4.0	Ctr	Ctr	0.37
C	LOCK	3.0x 5.0	Ctr	Ctr	0.68
E	LOCK	4.0x 5.0	Ctr	Ctr	0.57
S1	LOCK	3.0x 6.0	Ctr	Ctr	0.95
D	LOCK	4.0x 5.0	Ctr	Ctr	0.57

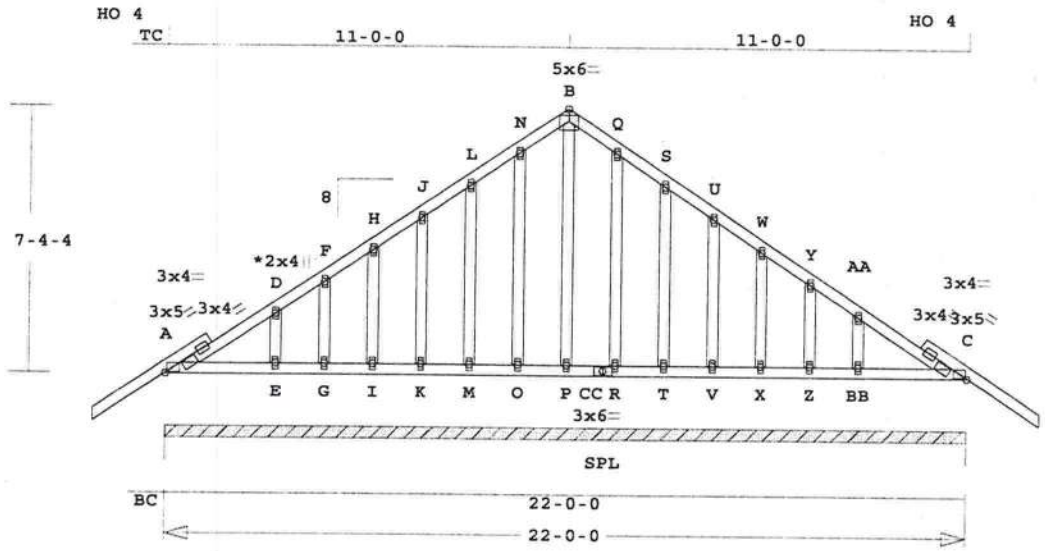
NOTES:
Trusses Manufactured by:

RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
OH Loading
Soffit psf 2.0
Provide connection to bearing for 308 Lbs Horiz Reaction
Wind Loads - ANSI / ASCE 7-98
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1404 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineerin
050984	R10	1	TR	220000	8	0	0	

GREEN RESIDENCE



ALL PLATES ARE LOCK20
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.188" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 201.0 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.04	2x 4	SP-#2
BC	0.05	2x 4	SP-#2
GW	0.08	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	22- 0- 0
BC	Cont.	0- 0- 0	22- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg	0- 0- 0	to 22- 0- 0	
	1561	341	Hz =	297

Membr	CSI	P Lbs	Axl	CSI-Bnd
----------Top Chords----------				
A -D	0.04	180	C	0.01 0.03
D -F	0.03	130	C	0.00 0.03
F -H	0.01	107	C	0.00 0.01
H -J	0.01	79	C	0.00 0.01
J -L	0.02	91	T	0.01 0.01
L -N	0.02	130	T	0.01 0.01
N -B	0.03	167	T	0.02 0.01
B -Q	0.03	167	T	0.02 0.01
Q -S	0.02	130	T	0.01 0.01
S -U	0.02	91	T	0.01 0.01
U -W	0.01	79	C	0.00 0.01
W -Y	0.01	107	C	0.00 0.01
Y -AA	0.03	130	C	0.00 0.03
AA-C	0.04	180	C	0.01 0.03
--------Bottom Chords---------				
A -E	0.05	0	T	0.00 0.05
E -G	0.01	0	T	0.00 0.01
G -I	0.00	0	T	
I -K	0.00	0	T	
K -M	0.00	0	T	
M -O	0.00	0	T	
O -P	0.00	0	T	
P -CC	0.00	0	T	
CC-R	0.00	0	T	
R -T	0.00	0	T	
T -V	0.00	0	T	
V -X	0.00	0	T	
X -Z	0.00	0	T	
Z -BB	0.01	0	T	0.00 0.01
BB-C	0.05	0	T	0.00 0.05
----------Gable Webs----------				
E -D	0.03	130	T	
G -F	0.01	67	C	
I -H	0.01	81	C	
K -J	0.02	80	C	
M -L	0.03	85	C	
O -N	0.04	70	C	
P -B	0.08	124	C	
R -Q	0.04	70	C	
T -S	0.03	85	C	
V -U	0.02	80	C	
X -W	0.01	81	C	
Z -Y	0.01	67	C	
BB-AA	0.03	130	T	

LL Defl 0.00" in A -E L/999
TL Defl 0.00" in A -E L/999
Shear // Grain in A -D 0.06

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

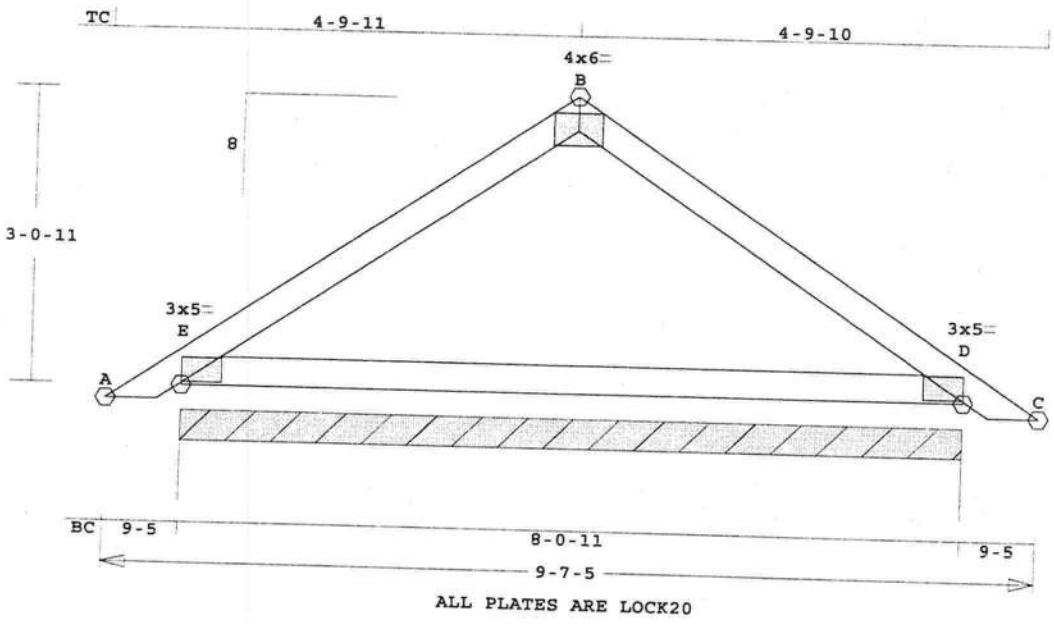
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.68
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	2.0x 4.0	Ctr	Ctr	0.00
H	LOCK	2.0x 4.0	Ctr	Ctr	0.00
J	LOCK	2.0x 4.0	Ctr	Ctr	0.00
L	LOCK	2.0x 4.0	Ctr	Ctr	0.00
N	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B	LOCK	5.0x 6.0	Ctr-0.3		0.75
Q	LOCK	2.0x 4.0	Ctr	Ctr	0.00
S	LOCK	2.0x 4.0	Ctr	Ctr	0.00
U	LOCK	2.0x 4.0	Ctr	Ctr	0.00
W	LOCK	2.0x 4.0	Ctr	Ctr	0.00
Y	LOCK	2.0x 4.0	Ctr	Ctr	0.00
AA	LOCK	2.0x 4.0	Ctr	Ctr	0.00
C	LOCK	3.0x 5.0	Ctr	Ctr	0.68
E	LOCK	2.0x 4.0	Ctr	Ctr	0.00
G	LOCK	2.0x 4.0	Ctr	Ctr	0.00
I	LOCK	2.0x 4.0	Ctr	Ctr	0.00
K	LOCK	2.0x 4.0	Ctr	Ctr	0.00
M	LOCK	2.0x 4.0	Ctr	Ctr	0.00
O	LOCK	2.0x 4.0	Ctr	Ctr	0.00
P	LOCK	2.0x 4.0	Ctr	Ctr	0.00
CC	LOCK	3.0x 6.0	Ctr	Ctr	0.88
R	LOCK	2.0x 4.0	Ctr	Ctr	0.00
T	LOCK	2.0x 4.0	Ctr	Ctr	0.00
V	LOCK	2.0x 4.0	Ctr	Ctr	0.00
X	LOCK	2.0x 4.0	Ctr	Ctr	0.00
Z	LOCK	2.0x 4.0	Ctr	Ctr	0.00
BB	LOCK	2.0x 4.0	Ctr	Ctr	0.00

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 180 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
050984	T1	6	PB	90705	8	0	0	

GREEN RESIDENCE



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 37.0 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.16	2x 4	SP-#2
BC	0.16	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 7- 5
BC Cont.	0- 0- 0	9- 7- 5

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 686	0- 0- 0 to 242 Hz =	8- 0-11	110

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
E -B	0.16	223	C	0.00	0.16
B -D	0.16	223	C	0.00	0.16
--------Bottom Chords---------					
E -D	0.16	3	T	0.00	0.16

LL Defl -0.02" in E -D L/999
TL Defl -0.09" in E -D L/999
Shear // Grain in E -B 0.13

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

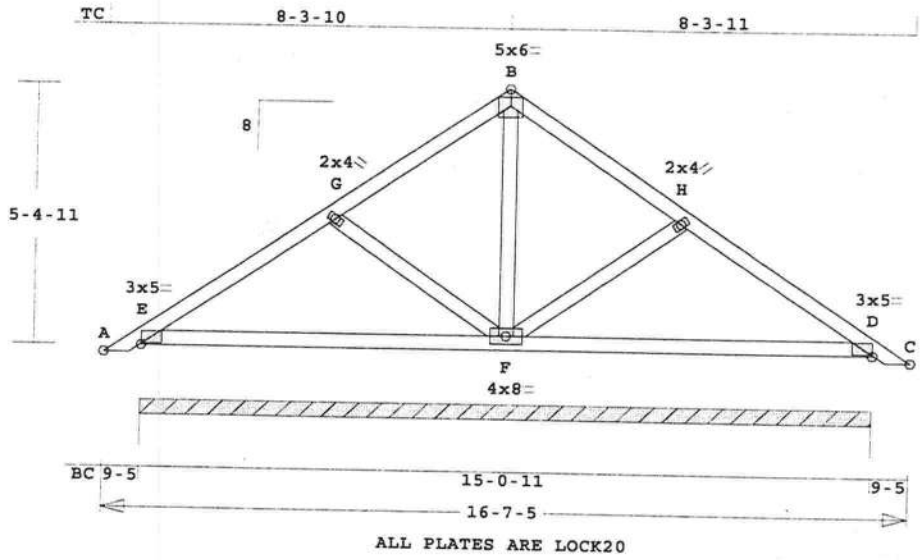
Jt Type	Plt Size	X	Y	JSI
E LOCK	3.0x 5.0	0.1 Ctr		0.49
B LOCK	4.0x 6.0	Ctr-1.9		0.76
D LOCK	3.0x 5.0	-0.1 Ctr		0.49

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
OH Loading
Soffit psf 2.0
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 223 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineerin
050984	T2	25	PB	160705	8	0	0	

GREEN RESIDENCE



Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 91.1 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.15	2x 4	SP-#2
BC	0.21	2x 4	SP-#2
WB	0.06	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	16- 7- 5
BC Cont.	0- 0- 0	16- 7- 5

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 15- 0-11			
	1204	355	Hz =	210

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
E -G	0.15	293 C	0.00	0.15
G -B	0.15	119 C	0.00	0.15
B -H	0.15	119 C	0.00	0.15
H -D	0.15	293 C	0.00	0.15
--------Bottom Chords---------				
E -F	0.21	2 T	0.00	0.21
F -D	0.21	2 T	0.00	0.21
------------Webs------------				
G -F	0.06	229 C		
F -B	0.02	87 T		
F -H	0.06	229 C		

LL Defl 0.00" in F -D L/999
TL Defl -0.05" in F -D L/999
Shear // Grain in E -G 0.13

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
E	LOCK	3.0x 5.0	0.1 Ctr		0.59
G	LOCK	2.0x 4.0	Ctr Ctr		0.37
B	LOCK	5.0x 6.0	Ctr-0.3		0.76
H	LOCK	2.0x 4.0	Ctr Ctr		0.37
D	LOCK	3.0x 5.0	-0.1 Ctr		0.59
F	LOCK	4.0x 8.0	Ctr Ctr		0.51

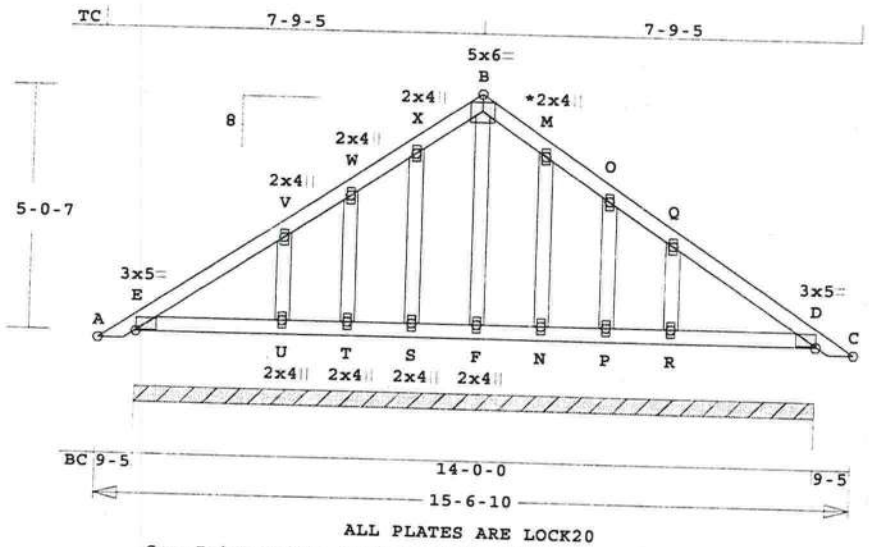
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
OH Loading
Soffit psf 2.0
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings*
for Exterior zone location.

Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 293 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
050984	T3	2	PB	150610	8	0	0	

GREEN RESIDENCE



ALL PLATES ARE LOCK20
See Joint M For Typical Gable Plate Size and Placement

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 100.8 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.05	2x 4	SP-#2
BC	0.05	2x 4	SP-#2
WB	0.03	2x 4	SP-#3
GW	0.03	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	15- 6-10
BC Cont.	0- 0- 0	15- 6-10

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to 14- 0- 0			
	1126	337 Hz =	195	

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
E -V	0.05	108 C	0.00	0.05
V -W	0.05	66 C	0.00	0.05
W -X	0.01	82 C	0.00	0.01
X -B	0.01	117 C	0.00	0.01
B -M	0.01	117 C	0.00	0.01
M -O	0.01	82 C	0.00	0.01
O -Q	0.05	66 C	0.00	0.05
Q -D	0.05	108 C	0.00	0.05
--------Bottom Chords---------				
E -U	0.05	3 T	0.00	0.05
U -T	0.03	0 T	0.00	0.03
T -S	0.00	0 T		
S -F	0.00	0 T		
F -N	0.00	0 T		
N -P	0.00	0 T		
P -R	0.03	0 T	0.00	0.03
R -D	0.05	3 T	0.00	0.05
----------Webs----------				
U -V	0.03	152 C		
T -W	0.01	61 C		
S -X	0.01	74 C		
F -B	0.02	70 C		
----------Gable Webs----------				
N -M	0.01	74 C		
P -O	0.01	61 C		
R -Q	0.03	152 C		

LL Defl 0.00" in E -U L/999
TL Defl 0.00" in E -U L/999
Shear // Grain in E -V 0.08

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

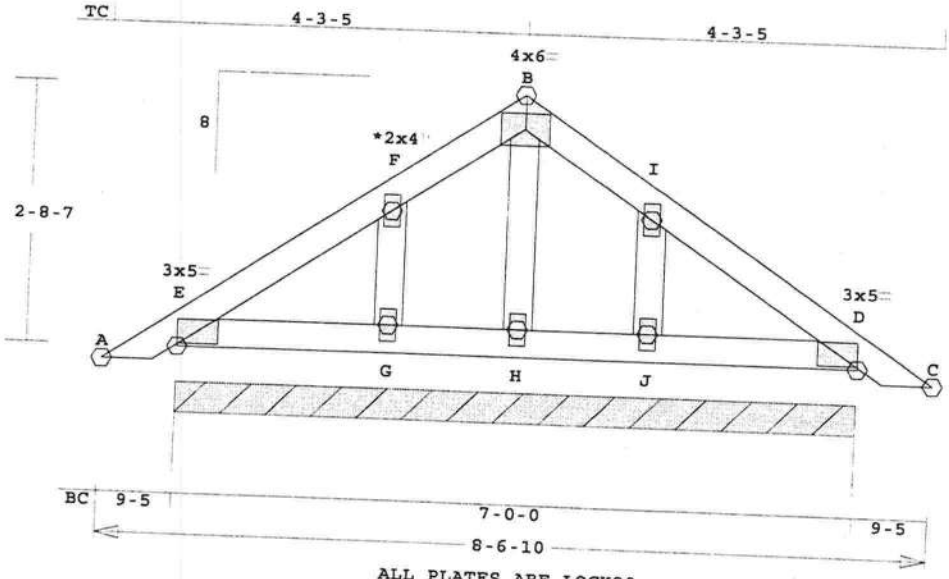
Jt	Type	Plt Size	X	Y	JSI
E	LOCK	3.0x 5.0	0.1 Ctr		0.57
V	LOCK	2.0x 4.0	Ctr Ctr		0.38
W	LOCK	2.0x 4.0	Ctr Ctr		0.38
X	LOCK	2.0x 4.0	Ctr Ctr		0.38
B	LOCK	5.0x 6.0	Ctr-0.3		0.75
M	LOCK	2.0x 4.0	Ctr Ctr		0.00
O	LOCK	2.0x 4.0	Ctr Ctr		0.00
Q	LOCK	2.0x 4.0	Ctr Ctr		0.00
D	LOCK	3.0x 5.0	-0.1 Ctr		0.57
U	LOCK	2.0x 4.0	Ctr Ctr		0.38
T	LOCK	2.0x 4.0	Ctr Ctr		0.38
S	LOCK	2.0x 4.0	Ctr Ctr		0.38
F	LOCK	2.0x 4.0	Ctr Ctr		0.38
N	LOCK	2.0x 4.0	Ctr Ctr		0.00
P	LOCK	2.0x 4.0	Ctr Ctr		0.00
R	LOCK	2.0x 4.0	Ctr Ctr		0.00

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
OH Loading
Soffit psf 2.0
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 152 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineerin
050984	T4	1	PB	80610	8	0	0	

GREEN RESIDENCE



ALL PLATES ARE LOCK20
See Joint F For Typical Gable Plate Size and Placement

Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 41.3 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.02	2x 4	SP-#2
BC	0.02	2x 4	SP-#2
GW	0.02	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8- 6-10
BC Cont.	0- 0- 0	8- 6-10

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg	0- 0- 0 to	7- 0- 0	
	608	224 Hz =	95	

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
E -F	0.02	46 C	0.00	0.02
F -B	0.02	73 C	0.00	0.02
B -I	0.02	73 C	0.00	0.02
I -D	0.02	46 C	0.00	0.02
--------Bottom Chords---------				
E -G	0.02	2 T	0.00	0.02
G -H	0.01	0 T	0.00	0.01
H -J	0.01	0 T	0.00	0.01
J -D	0.02	2 T	0.00	0.02
----------Gable Webs----------				
G -F	0.02	114 C		
H -B	0.00	29 C		
J -I	0.02	114 C		

LL Defl 0.00" in J -D L/999
TL Defl 0.00" in J -D L/999
Shear // Grain in E -F 0.06

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
E	LOCK	3.0x 5.0	0.1 Ctr		0.48
F	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B	LOCK	4.0x 6.0	Ctr	Ctr	0.79
I	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	3.0x 5.0	-0.1 Ctr		0.48
G	LOCK	2.0x 4.0	Ctr	Ctr	0.00
H	LOCK	2.0x 4.0	Ctr	Ctr	0.00
J	LOCK	2.0x 4.0	Ctr	Ctr	0.00

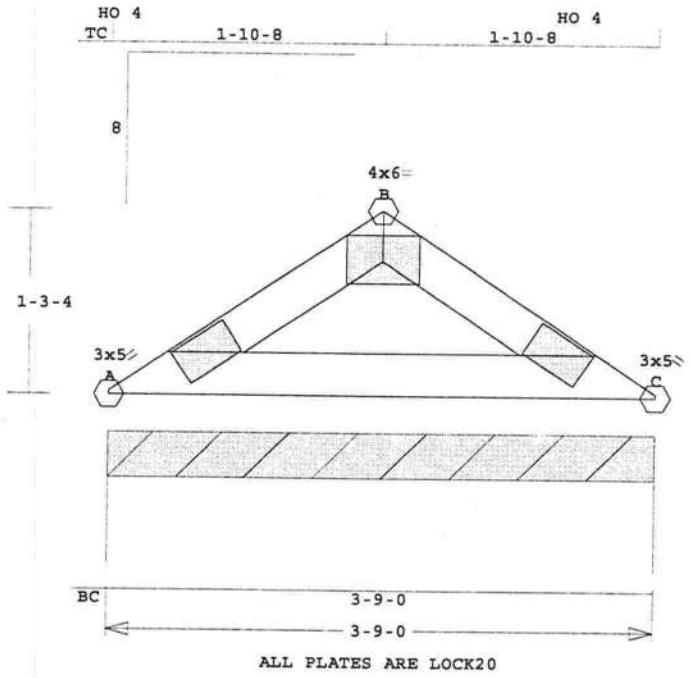
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
OH Loading
Soffit psf 2.0

Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-98
Truss is designed as Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 114 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineeri
050984	V1	1	VL	30900	8	0	0	

GREEN RESIDENCE



ALL PLATES ARE LOCK20

Scale: 0.750" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 13.8 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.01	2x 4	SP-#2
BC	0.01	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 9- 0
BC Cont.	0- 0- 0	3- 9- 0

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 3- 9- 0			
	211	46	Hz =	37

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.01	50 C	0.00	0.01
B -C	0.01	50 C	0.00	0.01
--------Bottom Chords---------				
A -C	0.01	0 T	0.00	0.01

LL Defl 0.00" in A -C L/999
TL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.03

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.44
B	LOCK	4.0x 6.0	Ctr	-1.9	0.74
C	LOCK	3.0x 5.0	Ctr	Ctr	0.44

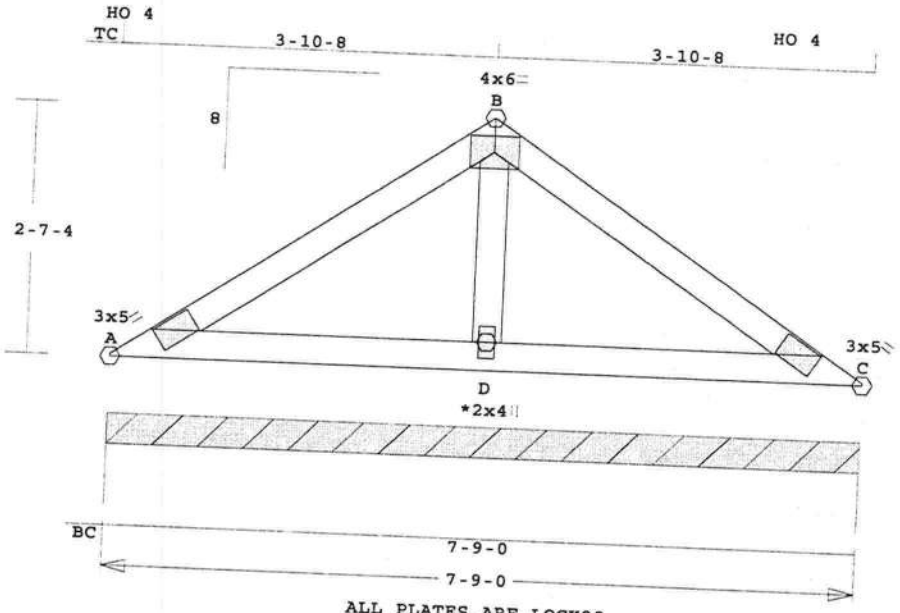
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 50 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
050984	V2	1	VL	70900	8	0	0	

GREEN RESIDENCE



ALL PLATES ARE LOCK20
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 34.1 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.06	2x 4	SP-#2
BC	0.04	2x 4	SP-#2
GW	0.00	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 9- 0
BC	Cont.	0- 0- 0	7- 9- 0

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 507	0- 0- 0 to 111	Hz =	7- 9- 0 94

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.06	136 C	0.01	0.05
B -C	0.06	136 C	0.01	0.05
--------Bottom Chords---------				
A -D	0.04	0 T	0.00	0.04
D -C	0.04	0 T	0.00	0.04
----------Gable Webs----------				
D -B	0.00	43 C		

LL Defl 0.00" in A -D L/999
TL Defl 0.00" in A -D L/999
Shear // Grain in A -B 0.08

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.48
B	LOCK	4.0x 6.0	Ctr	Ctr	0.79
C	LOCK	3.0x 5.0	Ctr	Ctr	0.48
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00

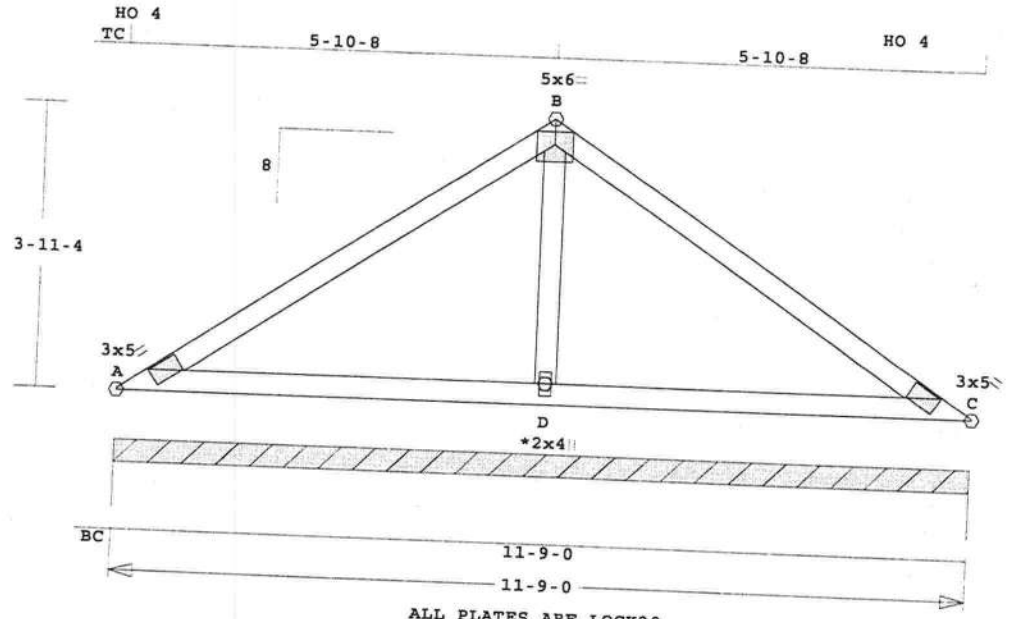
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 136 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
050984	V3	1	VL	110900	8	0	0	

GREEN RESIDENCE



ALL PLATES ARE LOCK20
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.375" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 53.3 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.17	2x 4	SP-#2
BC	0.13	2x 4	SP-#2
GW	0.03	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	11- 9- 0
BC Cont.		0- 0- 0	11- 9- 0

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 11- 9- 0			
	803	175	Hz =	151

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.17	335 C	0.02	0.15
B -C	0.17	335 C	0.02	0.15
--------Bottom Chords---------				
A -D	0.13	0 T	0.00	0.13
D -C	0.13	0 T	0.00	0.13
----------Gable Webs----------				
D -B	0.03	135 T		

LL Defl -0.01" in A -D L/999
TL Defl -0.02" in A -D L/999
Shear // Grain in A -B 0.14

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.53
B	LOCK	5.0x 6.0	Ctr	-0.3	0.75
C	LOCK	3.0x 5.0	Ctr	Ctr	0.53
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00

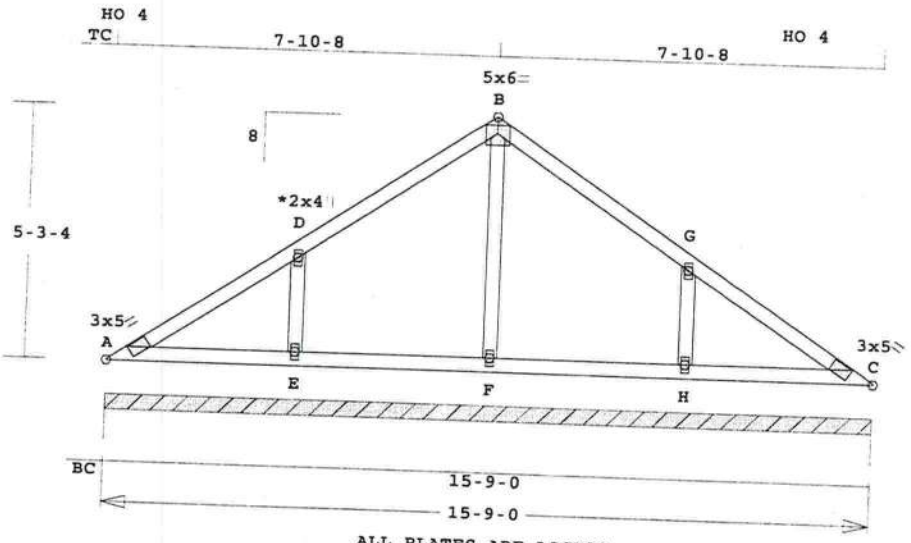
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 335 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
050984	V4	1	VL	150900	8	0	0	

GREEN RESIDENCE



ALL PLATES ARE LOCK20
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 80.2 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.12	2x 4	SP-#2
BC	0.05	2x 4	SP-#2
GW	0.06	2x 4	SP-#3

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	15- 9- 0
BC Cont.		0- 0- 0	15- 9- 0

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to 15- 9- 0			
	1099	240	Hz =	208

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -D	0.12	116 C	0.00	0.12
D -B	0.12	142 C	0.00	0.12
B -G	0.12	142 C	0.00	0.12
G -C	0.12	116 C	0.00	0.12
--------Bottom Chords--------				
A -E	0.04	0 T	0.00	0.04
E -F	0.05	0 T	0.00	0.05
F -H	0.05	0 T	0.00	0.05
H -C	0.04	0 T	0.00	0.04
----------Gable Webs----------				
E -D	0.06	241 C		
F -B	0.04	115 C		
H -G	0.06	241 C		

LL Defl 0.00" in A -E L/999
TL Defl 0.00" in F -H L/999
Shear // Grain in D -B 0.12

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.59
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B	LOCK	5.0x 6.0	Ctr-0.3		0.75
G	LOCK	2.0x 4.0	Ctr	Ctr	0.00
C	LOCK	3.0x 5.0	Ctr	Ctr	0.59
E	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	2.0x 4.0	Ctr	Ctr	0.00
H	LOCK	2.0x 4.0	Ctr	Ctr	0.00

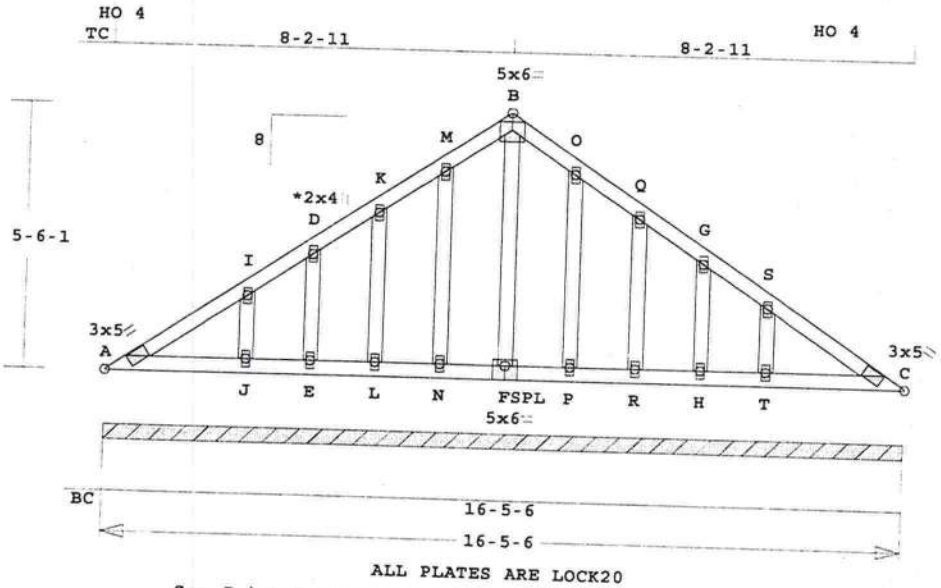
NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
Wind Loads - ANSI / ASCE 7-98
Truss is designed as

Components and Claddings* for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 241 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	*Engineering*
050984	V5	1	VL	160506	8	0	0	

GREEN RESIDENCE



ALL PLATES ARE LOCK20

See Joint I For Typical Gable Plate Size and Placement

Scale: 0.250" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 117.1 LBS

Online Plus -- Version 17.7.008
RUN DATE: 19-AUG-05

	CSI	-Size-	----Lumber----
TC	0.03	2x 4	SP-#2
BC	0.03	2x 4	SP-#2
GW	0.03	2x 4	SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	16- 5- 6
BC Cont.	0- 0- 0	16- 5- 6

Loading	Live	Dead	(psf)
TC	20.0	7.0	
BC	0.0	10.0	
Total	20.0	17.0	37.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 7 Wind Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	1150	251	0- 0- 0 to Hz =	16- 5- 6 218

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -I	0.03	124	C	0.00	0.03
I -D	0.03	80	C	0.00	0.03
D -K	0.01	58	T	0.00	0.01
K -M	0.02	90	T	0.01	0.01
M -B	0.01	131	T	0.00	0.01
B -O	0.01	131	T	0.00	0.01
O -Q	0.02	90	T	0.01	0.01
Q -G	0.01	58	T	0.00	0.01
G -S	0.03	80	C	0.00	0.03
S -C	0.03	124	C	0.00	0.03
Bottom Chords					
A -J	0.03	0	T	0.00	0.03
J -E	0.01	0	T	0.00	0.01
E -L	0.00	0	T		
L -N	0.00	0	T		
N -F	0.00	0	T		
F -P	0.00	0	T		
P -R	0.00	0	T		
R -H	0.00	0	T		
H -T	0.01	0	T	0.00	0.01
T -C	0.03	0	T	0.00	0.03
Gable Webs					
J -I	0.03	121	C		
E -D	0.01	69	C		
L -K	0.02	85	C		
N -M	0.02	71	C		
F -B	0.03	89	C		
P -O	0.02	71	C		
R -Q	0.02	85	C		
H -G	0.01	69	C		
T -S	0.03	121	C		

LL Defl 0.00" in A -J L/999
TL Defl 0.00" in A -J L/999
Shear // Grain in A -I 0.06

Plates for each ply each face.
ALL CONNECTOR PLATES
TO BE MANUFACTURED BY
ROBBINS ENGINEERING, INC.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 5.0	Ctr	Ctr	0.60
I	LOCK	2.0x 4.0	Ctr	Ctr	0.00
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00
K	LOCK	2.0x 4.0	Ctr	Ctr	0.00
M	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B	LOCK	5.0x 6.0	Ctr-0.3		0.75
O	LOCK	2.0x 4.0	Ctr	Ctr	0.00
Q	LOCK	2.0x 4.0	Ctr	Ctr	0.00
G	LOCK	2.0x 4.0	Ctr	Ctr	0.00
S	LOCK	2.0x 4.0	Ctr	Ctr	0.00
C	LOCK	3.0x 5.0	Ctr	Ctr	0.60
J	LOCK	2.0x 4.0	Ctr	Ctr	0.00
E	LOCK	2.0x 4.0	Ctr	Ctr	0.00
L	LOCK	2.0x 4.0	Ctr	Ctr	0.00
N	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	5.0x 6.0	Ctr	0.8	0.75
P	LOCK	2.0x 4.0	Ctr	Ctr	0.00
R	LOCK	2.0x 4.0	Ctr	Ctr	0.00
H	LOCK	2.0x 4.0	Ctr	Ctr	0.00
T	LOCK	2.0x 4.0	Ctr	Ctr	0.00

NOTES:
Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
ANSI/TPI 95 & 02
Wind Loads - ANSI / ASCE 7-98
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 124 Lbs
Quality Control Factor 1.25

FABRICATOR NOTES:
1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

BCSI-B1 SUMMARY SHEET 1 of 2

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary bracing. Follow the recommendations for handling, installing and temporary bracing of trusses. Refer to BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent bracing on individual compression members. Refer to the BCSI-B3 Summary Sheet - Web Member Permanent Bracing/Web Reinforcement for more information. All other permanent bracing design is the responsibility of the Building Designer.

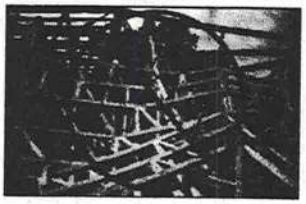
NOTAS GENERALES

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de los arriostres (bracing) temporales. Use las recomendaciones de manejo, instalación y arriostre temporal de los trusses. Vea el folleto BCSI 1-03 Guía de Buena Práctica para el Manejo, Instalación y Arriostre de los Trusses de Madera Conectados con Placas de Metalpara para mayor información.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de los arriostres permanentes en los miembros individuales en compresión. Vea la hoja resúmen BCSI-B3 para los arriostres permanentes y refuerzos de los miembros secundarios (webs) para mayor información. El resto de arriostres permanentes son la responsabilidad del Diseñador del Edificio.

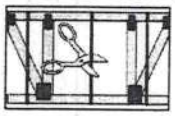
⚠ The consequences of improper handling, installing and bracing may be a collapse of the structure, or worse, serious personal injury or death.

El resultado de un manejo, instalacion y arriostre inadecuados, puede ser la caida de la estructura o aún peor, muertos o heridos.



⚠ Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

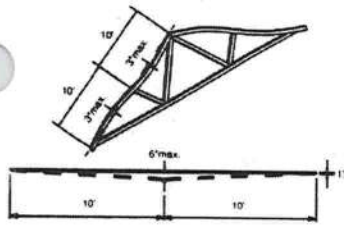
Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.



HANDLING — MANEJO

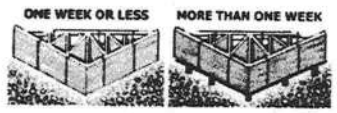
☑ Allow no more than 3" of deflection for every 10' of span.

No permita mas de 3 pulgadas de pandeo por cada 10 pies de tramo.



☑ Pick up vertical bundles at the top chord.

Levante de la cuerda superior los grupos verticales de trusses.



☑ Bundles stored on the ground for one week or more should be raised by blocking at 8' to 10' on center.

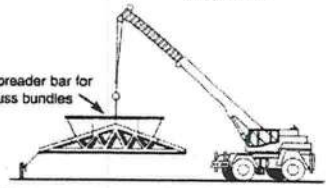
Los paquetes almacenados en la tierra por una semana o más deben ser elevados con bloques a cada 8 o 10 pies.

☑ For long term storage, cover bundles to prevent moisture gain but allow for ventilation.

Para almacen-amiento por mayor tiempo, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

⚠ Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



☑ Check banding prior to moving bundles.

Revise los empaques antes de mover los paquetes de trusses.

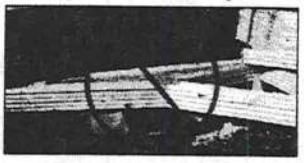
⚠ Avoid lateral bending. — Evite la flexión lateral.

⊘ Do not store unbraced bundles upright.

No almacene verticalmente los trusses sueltos.

⊘ Do not store on uneven ground.

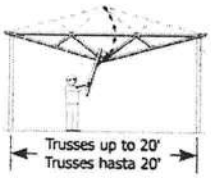
No almacene en tierra desigual.



HAND ERECTION — LEVANTAMIENTO A MANO

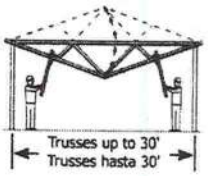
☑ Trusses 20' or less, support at peak.

Levante del pico los trusses de 20 pies o menos.



☑ Trusses 30' or less, support at quarter points.

Levante de los cuartos de tramo los trusses de 30 pies o menos.



HOISTING — LEVANTAMIENTO

☑ Hold each truss in position with the erection equipment until temporary bracing is installed and truss is fastened to the bearing points.

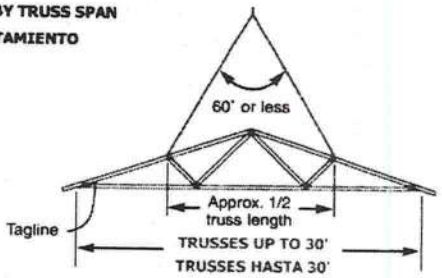
Sostenga cada truss en posición con la grúa hasta que el arriostre temporal esté instalado y el truss asegurado en los soportes.

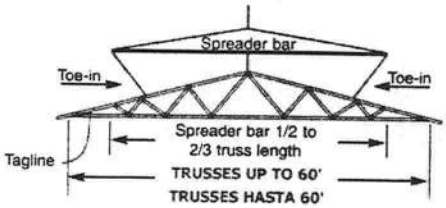
⊘ Do not lift trusses over 30' by the peak.

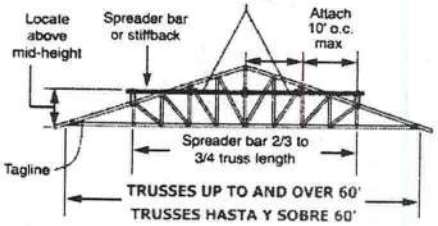
No levante del pico los trusses de más de 30 pies.

HOISTING RECOMMENDATIONS BY TRUSS SPAN

RECOMMENDACIONES DE LEVANTAMIENTO POR LONGITUD DEL TRUSS



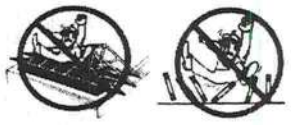




BRACING — ARRIOSTRE

⚠ Refer to BCSI-B2 Summary Sheet - Truss Installation and Temporary Bracing for more information.

Vea el resúmen BCSI-B2 - Instalación de Trusses y Arriostre Temporal para mayor información.

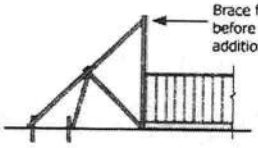


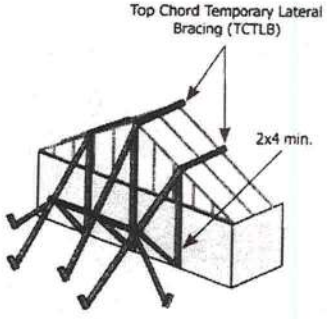
⊘ Do not walk on unbraced trusses.

No camine en trusses sueltos.

☑ Locate ground braces for first truss directly in line with all rows of top chord temporary lateral bracing.

Coloque los arriostres de tierra para el primer truss directamente en linea con cada una de las filas de arriostres laterales temporales de la cuerda superior.





BCSI-B1 SUMMARY SHEET 2 of 2

BRACING FOR THREE PLANES OF ROOF
EL ARRIOSTRE EN TRES PLANOS DE TECHO

☑ This bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.
Este metodo de arriostre es para todo trusses excepto trusses de cuerdas paralelas 3x2 y 4x2

1) TOP CHORD — CUERDA SUPERIOR

Truss Span Longitud de Tramo	Top Chord Temporary Lateral Brace (TCTLB) Spacing Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30' Hasta 30 pies	10' o.c. max. 10 pies máximo
30' to 45' 30 a 45 pies	8' o.c. max. 8 pies máximo
45' to 60' 45 a 60 pies	6' o.c. max. 6 pies máximo
60' to 80'* 60 a 80 pies*	4' o.c. max. 4 pies máximo

*Consult a Professional Engineer for trusses longer than 60'.
*Consulte a un ingeniero para trusses de mas de 60 pies.

☑ See BCSI-B2 for TCTLB options.
Vea el BCSI-B2 para las opciones de TCTLB.

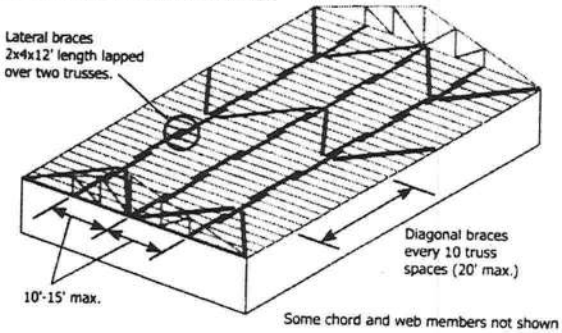
⚠ Refer to BCSI-B6 Summary Sheet - Gable End Frame Bracing.
Vea el resúmen BCSI-B6 - Arriostre del truss terminal de un techo a dos aguas.

☑ Repeat diagonal braces.
Repita los arriostres diagonales.

☑ Set first five trusses with spacer pieces, then add diagonals. Repeat process on groups of four trusses until all trusses are set.

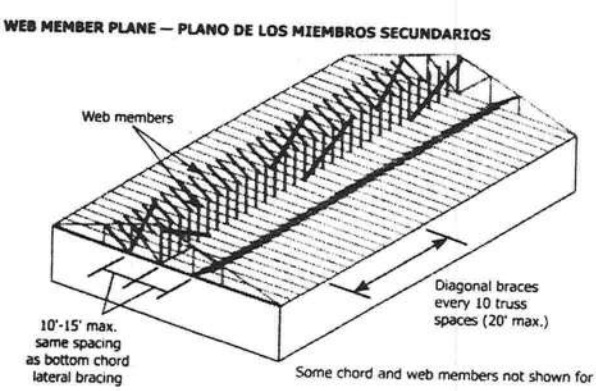
Instale los cinco primeros trusses con espaciadores, luego los arriostres diagonales. Repita este procedimiento en grupos de cuatro trusses hasta que todos los trusses esten instalados.

2) BOTTOM CHORD — CUERDA INFERIOR



Some chord and web members not shown for clarity.

3) WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS

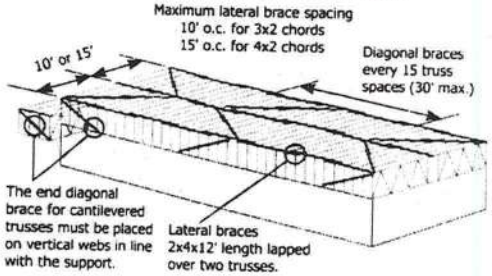


Some chord and web members not shown for clarity.

⚠ **DIAGONAL BRACING IS VERY IMPORTANT**
¡EL ARRIOSTRE DIAGONAL ES MUY IMPORTANTE! ⚠

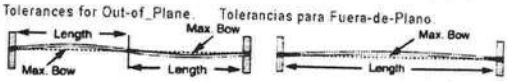
BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES
EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3x2 Y 4x2

⚠ Refer to BCSI-B7 Summary Sheet - Temporary and Permanent Bracing for Parallel Chord Trusses for more information.
Vea el resumen BCSI-B7 - Arriostre temporal y permanente de trusses de cuerdas paralelas para mayor información.



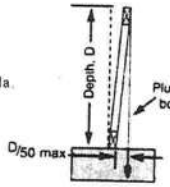
INSTALLING — INSTALACION

☑ Tolerances for Out-of_Plane. Tolerancias para Fuera-de-Plano.



Max. Bow	Truss Length
3/4"	12.5'
7/8"	14.6'
1"	16.7'
1-1/8"	18.8'
1-1/4"	20.8'
1-3/8"	22.9'
1-1/2"	25.0'
1-3/4"	29.2'
2"	≥ 33.3'

☑ Tolerances for Out-of-Plumb
Tolerances para Fuerta-de-Plomada.



D/50	D (ft.)
1/4"	1'
1/2"	2'
3/4"	3'
1"	4'
1-1/4"	5'
1-1/2"	6'
1-3/4"	7'
2"	≥ 8'

CONSTRUCTION LOADING — CARGA DE CONSTRUCCION

⚠ Do not proceed with construction until all bracing is securely and properly in place.
No proceda con la construcción hasta que todos los arriostres estén colocados en forma apropiada y segura.

⊘ Do not exceed maximum stack heights. Refer to BCSI-B4 Summary Sheet - Construction Loading for more information.
No exceda las máximas alturas recomendadas. Vea el resúmen BCSI-B4 Carga de Construcción para mayor información.

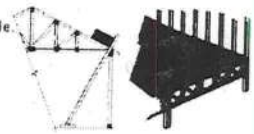
Maximum Stack Height for Materials on Trusses

Material	Height (h)
Gypsum Board	12'
Plywood or OSB	16'
Asphalt Shingles	2 bundles
Concrete Block	8'
Clay Tile	3-4 tiles high

⊘ Do not overload small groups or single trusses.
No sobrecargue pequeños grupos o trusses individuales.

☑ Place loads over as many trusses as possible
Cologue las cargas sobre tantos trusses como sea posible.

☑ Position loads over load bearing walls.
Coloque las cargas sobre las paredes soportantes.



ALTERATIONS — ALTERACIONES

⚠ Refer to BCSI-B5 Summary Sheet - Truss Damage, Jobsite Modifications and Installation Errors.
Vea el resúmen BCSI-B5 Daños de trusses, Modificaciones en la Obra y Errores de Instalación.

⊘ Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.
No corte, altere o perfore ningún miembro estructural de los trusses, a menos que esté especificamente permitido en el dibujo del diseño del truss.

⚠ Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void
Trusses que se han sobrecargado durante la construccion o han sido alterados sin una autorización previa del Fabricante de Trusses, pueden reducir o eliminar la garantia del Fabricante de Trusses.

WOOD TRUSS COUNCIL OF AMERICA
One WTCA Center • 6300 Enterprise Lane • Madison, WI 53719
608/274-4849 • www.woodtruss.com

TRUSS PLATE INSTITUTE
583 D'Onofrio Drive • Madison, WI 53719
608/833-5900 • www.tpinst.org

General detail for repair of broken webs that meet the following conditions.

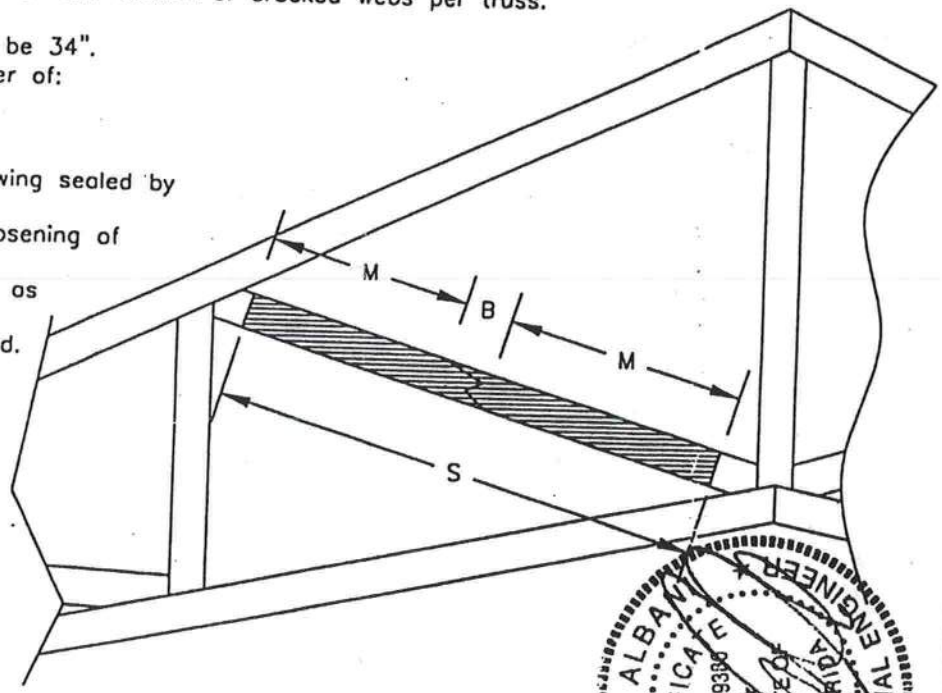
* Web size and grade must be maximum 2x4, #3, standard, or stud grade of any species.
* Truss must be single ply.
* No more than one crack or break per web and no more than two broken or cracked webs per truss.

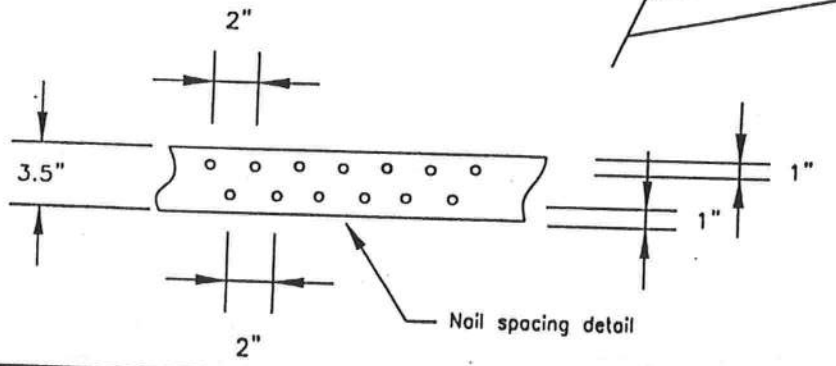
B = Maximum length of damaged area not to exceed 12".
M = Minimum length of scab at each end of break area to be 34".
S = Overall length of scab member (shaded) must be greater of:
S = 68" + B
S = 80% of original web length.

For all lumber, plates, web bracing, etc. refer to original drawing sealed by Robbins Engineering Inc.

1) Apply all nails so as to avoid damaging of lumber and loosening of plates at joints.
2) Attach 2x4 scab (shaded), minimum same size and grade as original web member, to one face of truss using 2 rows of 10d common nails 2" on center each row and staggered.

Truss manufacturer must be notified if field conditions do not match this general detail.





THOMAS ALBANESE CERTIFICATE No. 39386 STATE OF FLORIDA PROFESSIONAL ENGINEER

ROBBINS

Engineering Inc.

P.O.Box 280055, Tampa, FL 33882

ROBBINS LOCK connector plates (20 ga. galv. steel-ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates,unless shown otherwise by circles (o) or dimensions. Unless otherwise indicated by a "+", all plate in plates run parallel with the chords or horizontally at the peak and / or heel. No loose knots or voids in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrications shall be performed on equipment which produces snug-fitting joints and plates. This design was prepared in accordance with "National Design Specifications for Stress-Grade lumber and its Fastenings" (AFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for Trussed Rafters.

Robbins Mfg., Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to HIB-91 as published by the Truss Plate Institute,583 D'Onofrio Drive,Suite 200, Madison, Wisconsin 53719. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR
IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FAB. TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.

Designed By: MG
Checked By: TAA
Date: 5/23/02
Dwg. No: GD-41

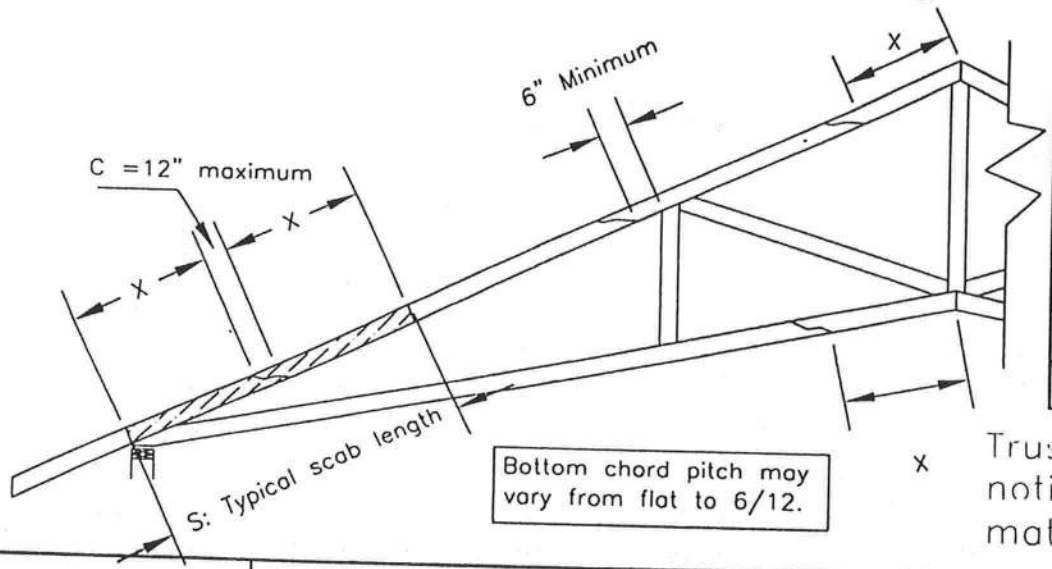
General detail for repair of broken chords and damaged or missing chord splices that meet the following conditions:

* Chord size must be maximum 2x6 and southern pine species.
* Scab must be the same size and grade as broken chord.
* Truss must be single ply.
* No more than two broken or cracked chords per truss.
* Perimeter of break area must be minimum of X distance from any heel or peak, and minimum of 6" away from any interior joint locations.

C = Maximum length of damaged area not to exceed 12".
S = Overall length of scab member (shaded) must be equal or greater than 2(X)+C.
X = Minimum length of scab member at each end of break area.
Refer to following table for minimum length of scab, and minimum number of nails at each end of break area and maximum axial force of broken member.
For all lumber, plates, web bracing, etc. refer to original drawing sealed by Robbins Engineering Inc.

1) Apply all nails so as to avoid damaging of lumber and loosening of plates at joints.
2) Use 2 rows of 10d common nails 3" on center each row and staggered into 2x4 members.
   Use 3 rows of 10d common nails 3" on center each row and staggered into 2x6 members.
   Use 6" spacing If scabs applied to each face of truss.
   Minimum end distance of 2" must be provided for all scab members.

THOMAS ALBANI
CERTIFICATE
No. 39381
STATE OF
FLORIDA
PROFESSIONAL ENGINEER



Bottom chord pitch may vary from flat to 6/12.

TOTAL NUMBER OF NAILS ON EACH SIDE OF BREAK		-X (INCHES)	MAXIMUM FORCE (LBS.) AT 33% LOAD DURATION	
2X4 SYP	2X6 SYP		2X4 SYP	2X6 SYP
13	20	24"	1924	2886
17	26	30"	2501	3752
21	32	36"	3079	4618
25	38	42"	3656	5484
29	44	48"	4233	6349

Truss manufacturer must be notified if field conditions do not match this general detail.

ROBBINS
Engineering Inc.
P.O.Box 280055, Tampa, FL 33682

ROBBINS LOCK connector plates (20 ga. galv. steel-ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates,unless shown otherwise by circles (o) or dimensions. Unless otherwise indicated by a "V", all slots in plates run parallel with the chords or horizontally at the peak and / or heel. No loose knots or wanes in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrications shall be performed on equipment which produces snug-fitting joints and plates. This design was prepared in accordance with "National Design Specifications for Stress-Grade lumber and its Fastenings" (NFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for Trussed Rafters.

Robbins Mfg., Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to HIB-91 as published by the Truss Plate Institute,583 D'Onofrio Drive,Suite 200, Madison, Wisconsin 53719. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively it is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR
IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FAB. TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.

Designed By: MG
Checked By: TAA
Rev. Date: 12/01/03
Dwg. No: GD-62-33

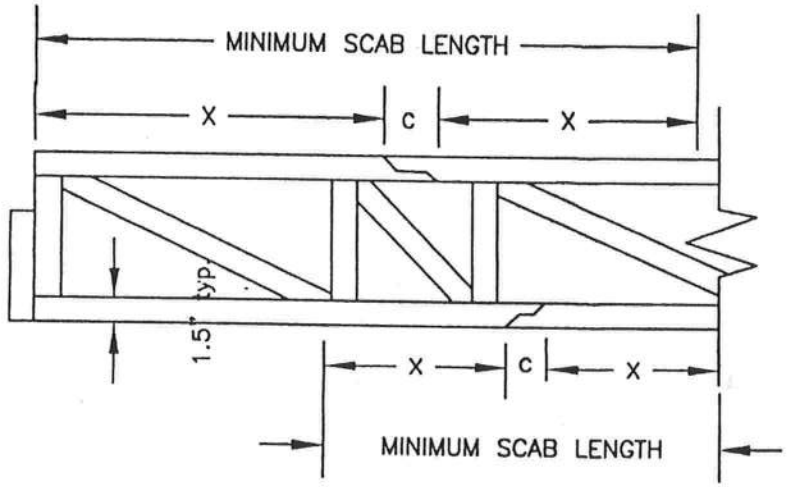
General detail for repair of broken, damaged or cut chords of PC42 floor trusses that meet the following conditions.

* Chord size must be maximum 4x2 and southern pine species.
* Scab must be at minimum the same size and grade as broken chord. Wide face of scab must be attached to narrow face of chord member.
* Truss must be single ply.
* No more than two broken or cracked chords per truss.
* Perimeter of break area must be minimum of X distance from end of truss. and minimum of 4" away from any plates at panel points. All plates must be intact and fully embedded.

C = Maximum length of damaged area or cut section not to exceed 12".
S = Overall length of scab member must be equal or greater than 2(X)+C.
X = Minimum length of scab member at each end of break area.
Refer to following table for minimum length of scab, and minimum number of nails at each end of break area and maximum axial force of broken member.
For all lumber, plates, web bracing, etc. refer to original drawing sealed by Robbins Engineering Inc.

1) Apply all nails so as to avoid damaging of lumber and loosening of plates at joints.
2) Attach the scab with one row of 10d common nails at 3" on center into 4x2 chord members. Axial force and number of nails may be doubled or X distance divided by 2, if scabs applied to each face of truss at the same location. Minimum end distance of 3" must be provided for all members.

THOMAS ALBANI
CERTIFICATE
No. 39380
STATE OF FLORIDA
PROFESSIONAL ENGINEER



TOTAL NUMBER OF NAILS AT EACH END OF	X (INCHES)	MAXIMUM AXIAL FORCE IN CHORD MEMBER (LBS.)
3	16	384
6	24	768
8	30	1024
10	36	1280
12	42	1536
14	48	1792
16	54	2048
18	60	2304
20	66	2560
22	72	2816

Truss manufacturer must be notified if field conditions do not match this general detail.

DEC 10 2003

ROBBINS
Engineering Inc.
P.O.Box 280055, Tampa, FL 33682

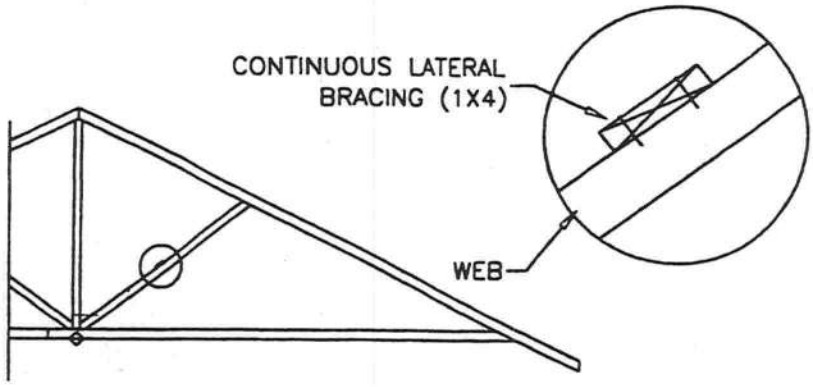
ROBBINS LOCK connector plates (20 ga. galv. steel-ASTM A653 SS Grade 40) shall be applied on both faces of truss at each joint. Center the plates, unless shown otherwise by circles (o) or dimensions. Unless otherwise indicated by a "V", all slots in plates run parallel with the chords or horizontally at the peak and / or heel. No loose knots or wanes in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrications shall be performed on equipment which produces snug-fitting joints and plates. This design was prepared in accordance with "National Design Specifications for Stress-Grade lumber and its Fastenings" (AFPA), "Design Specifications for Light Metal Plate Connected Wood Trusses" (TPI), and HUD Design Criteria for Trussed Rafters.

Robbins Mfg., Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to HIB-81 as published by the Truss Plate Institute, 583 D'Onofrio Drive, Suite 200, Madison, Wisconsin 53719. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR
IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FAB. TO REVIEW THIS DRWG. & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS LAYOUTS.

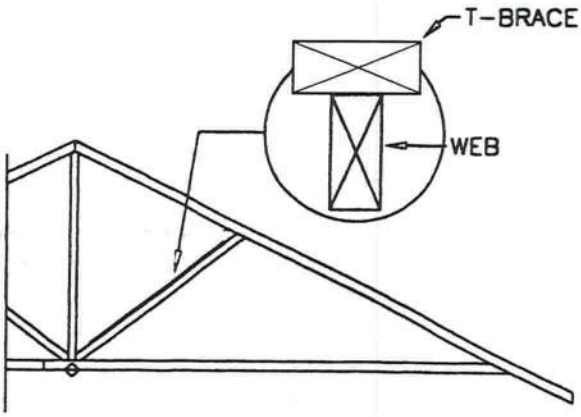
Designed By: MG
Checked By: TAA
Rev. Date: 12/09/03
Dwg. No: GD-62-FL

PERMANENT WEB BRACING



CONTINUOUS LATERAL BRACING

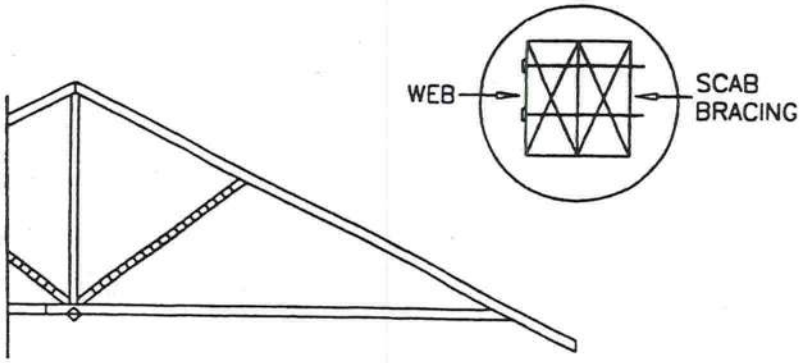
1x4 #3 HEM–FIR OR BETTER CONTINUOUS LATERAL BRACING TO BE EQUALLY SPACED. ATTACH WITH (2) 8d NAILS. BRACING MATERIAL TO BE SUPPLIED AND ATTACHED AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.



T-BRACE

THESE DETAILS APPLY TO 1.5" WIDE WOOD TRUSSES.

- USE A 2x4 T-BRACE IF THE TRUSS DESIGN SPECIFIES ONE LATERAL BRACE (MID POINT OF WEB).
- USE A 2x6 T-BRACE IF THE TRUSS DESIGN SPECIFIES TWO LATERAL BRACES (AT THE THIRD POINTS OF THE WEB).
- USE A CONTINUOUS PIECE FOR THE T-BRACE, OF THE SAME GRADE AS THE WEB AND COVERING AT LEAST 90% OF THE WEB LENGTH.
- CENTER THE T-BRACE ON THE WEB AND FASTEN WITH 10d COMMON NAILS SPACED 4" ON CENTER.



SCAB BRACE

SCAB BRACE SAME SIZE, GRADE, AND LENGTH AS WEB MEMBER. ATTACH WITH 10d NAILS @ 4" O.C. BRACING MATERIAL TO BE SUPPLIED BY ERECTION CONTRACTOR.

RESIDENTIAL WIND DESIGN & ANALYSIS

NO COPIES ARE TO BE PERMITTED \ FBC2004

PREPARED FOR:

GENESIS CONSTRUCTION \ THE GREEN RESIDENCE

PREPARED BY:

MARTY R. ESKRIDGE
14952 MAIN ST
ALACHUA FL 32615
386-462-1340 / 352-375-6329

Genesis Design & Construction, Inc.

481 Turkey Creek
Alachua, FL 32615
(386) 462-2884 - PH.

Allen K. Bates
CRC057572
(386) 462-4637 - Fax

October 18, 2005

Columbia County Building & Zoning Dept.
135 NE Hernando Ave. – Ste. B-21
Lake City, FL 32055

Attn. Mr. Joe Haltiwanger – Plans Examiner

RE: Ms. Alice Green – Permit #0510-23

Dear Joe,

As per your request in your previous fax regarding header sizes on the above referenced project, please be advised that ALL headers in exterior walls will be 2"x12", with the exception of the header over the garage door. The engineering for that opening is attached.

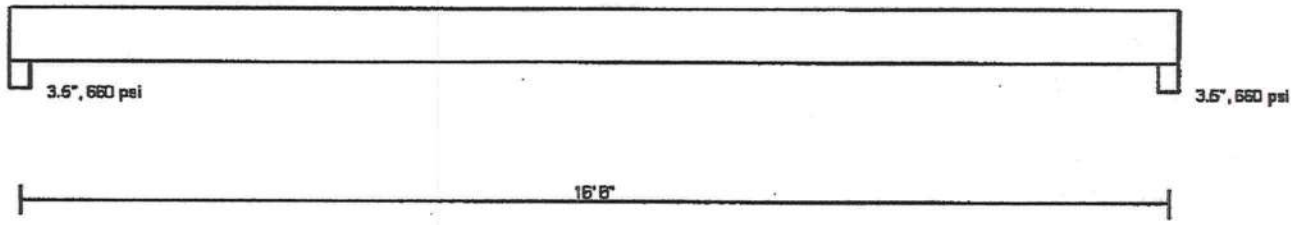
Thank you for your help and cooperation.

Sincerely,

Allen K. Bates
Genesis Design & Construction, Inc.

RIDGWAY ROOF TRUSS *Seth Schrum* 18 Oct 2005 1:40 pm
235 SW 11th PL , Gainesville, Fl. 32601 (352)376-4436
 Version: 5.0

Project : **050984.FBD**
Mark # : **LVL**
Usage : **Beam (Roof)** Repetitive : **No** Spacing (in.) : **0.0**
Max Defl : **LL = L/240 TL = L/180** Composite Action : **No** Slope : **0/12**



LOADS

Project Design Loads : Roof: Live=20 psf, Dead=17 psf;

#	Shape	Live+Dead Ld(T) @Start	@End	Live Ld(L) @Start	@End	LDF	Span#	Location* Starts	Ends	Additional Info
1	Span Carried(psf)	37		20		115%	0	0' 0"	16' 8"	22" 0" s.c. - R9's
	Uniform(plf)	11		0			0	0	16' 8"	Self Weight

**Dimensions measured from left end when span# is 0, otherwise, from left end of the specified span.*

SUPPORTS(lbs)

	1	2	
Max R'n	3482	3482	
Max 115%	1833	1833	
Min R'n	1649	1649	
Min 115%	1833	1833	
DL R'n	1649	1649	
Min Brg(in.)	1.51	1.51	[Based on bearing stress below]
Brg Str(psi)	660	660	

DESIGN

	Value	Span	X	Group	Allow	LDF	Ratio	
V(lbs)	3007	1	0' 2"	31	9080	115%	0.33	
M(ft-lbs)	14508	1	8' 4"	31	22887	115%	0.63	
LtRn(lbs)	3482	0	0' 0"	31	8085	100%	0.43	See Note #5
RtRn(lbs)	3482	0	16' 8"	31	8085	100%	0.43	See Note #5
LLDefl(in.)	0.41	1	8' 4"	31	0.83		L/485	
TLDefl(in.)	0.78	1	8' 4"	31	1.11		L/255	

USE: **GPLAM 2.0E 1.75x11.88" 2 Plies** *Grade, Depth, Plies selected by User*
G-P LAM tm Georgia-Pacific Corp.

NOTES :

1. *Designed in accordance with National Design Specifications for Wood Construction and applicable Approvals or Research Reports.*
2. *Provide lateral support at the bearing location nearest each end of the member. Continuous lateral support required for compression edge.*
3. *Loads have been input by the user and have not been verified by Georgia-Pacific Corporation.*
4. *Design valid for dry use only.*
5. *This reaction is based on the combination of loads & duration factors that produces the highest stress ratio and may be less than maximum reaction. Therefore, when reaction values are required, use Max R'n from 'Supports' section above.*
6. *Bearing length based on design material; support material capacity shall be verified (by others).*
7. *Roof Usage: Install with minimum 1/4:12 slope for adequate drainage.*
8. *When required by the building code, a registered design professional or building official should verify the input loads and product application.*
9. *This engineered lumber product has been sized for residential use. A concentrated load check, per the building code, must be performed for commercial uses.*
10. *Verify that load is applied at top or equally from both sides.*
11. *Nail plies together with 16d nails @ 12" o/c along top and bottom edges. Nail from alternate faces, 2" from edges.*
12. *Max/Min reactions are based on the applicable load combinations outlined in the notes. Summation of max/min reactions for various DOL may not match total max/min reaction.*
13. *Company, product or brand names referenced are trademarks or registered trademarks of their respective owners.*
14. *Load Combinations: 10 = D, 20 = D + 100%, 30 = D + 115%, 40 = D + 125%, 50 = D + 133%, 60 = D + 100% + 115%, 70 = D + 100% + 125% , 80 = D + 100% + 133%, 90 = D + 100% + 115% + 133%/2 , 100 = D + 100% + 115%/2 + 133%, 110 = D + Commercial Ld (100%)*
15. *Group = Load Combination Number + Load Pattern number. (For simple span, Load pattern = 1 for LL, 0 for DL).*

RESIDENTIAL WIND DESIGN & ANALYSIS

FBC 2001 SECTION 1606 \ V=110 \ ***NO COPIES ARE TO BE PERMITTED***

PREPARED FOR:

GENESIS CONSTRUCTION

PREPARED BY:

MARTY R. ESKRIDGE
14952 MAIN ST
ALACHUA FL 32615
386-462-1340 / 352-375-6329

SUMMARY
OF WIND DESIGN & ANALYSIS

Trusses: Lumber type So. Pine Grade #1 #2 #3 Size 2 x 4 Spacing 24 in.

Hurricane anchors: Interior: Mfr * Model *
End: Mfr * Model *

Roof sheathing: Type OSB Size 7/16 Fastener type Nails Size 8d/131 GA.
Interior zone spacing: Interior 8 in. Periphery 4 in.
Edge and end zone spacing: Interior 8 in. Periphery 4 in.

Top double pl: Type Spruce Grade #1 #2 Size 2 x 4 Nail spacing 12 in.

Studs: Wood or Steel: Wood Type Spruce Grade #1 #2 Size 2 x 4
Interior stud spacing 16 in. Composite (yes or no) Y
End stud spacing 16 in. Composite (yes or no) Y

Shearwall siding: Type OSB Thickness 7/16 in.
71' — Trans: Fastener 8d/131 Spacing: Int 8 in. Edge 4 in.
50' — Long: Fastener 8d/131 Spacing: Int 8 in. Edge 4 in.

Wall tension transferred by: Siding nails 8d/131 @ 4 O.C. edges

Foundation anchor bolts: Concrete strength 3000 psi
Size 1/2 in. Shape L Washer 2" Embedment 6 in.
Location of first anchor bolt from corner 8 in.
Anchor Bolts @ 48" O.C.
Hold-down device: Mfr _____ Model A307 Loc. from corner 8 in.

Type of foundation: 1 #5 rebar continuous required in bond beam./FBC-1804.6.2.6.2

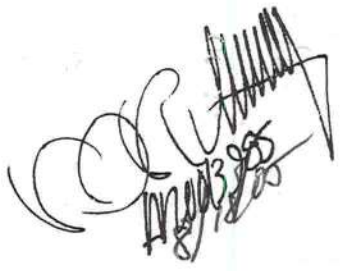
Floor slab 4 in. CMU: Size 8 x 16 in. Height 24 in. Reinf. # 5 at 96 in.
Monolithic footing: Depth 20 in. Bottom width 12 in.

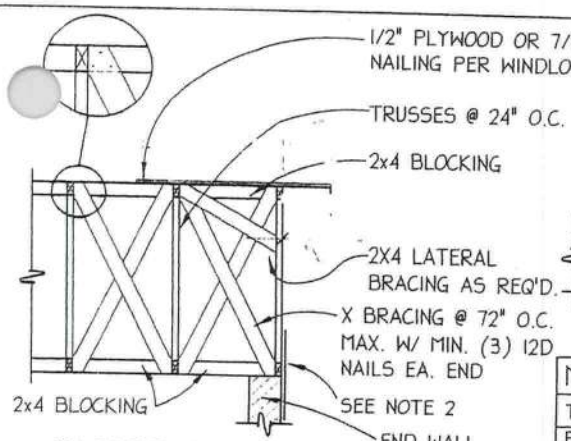
Footing: Width 20 in. Depth 10 in. Reinforcing 2 --# 5 bars
Interior Footings: 16" W X 10" D

Porch Columns: 4X4X8' SYP #2 PT @ 144" O.C.

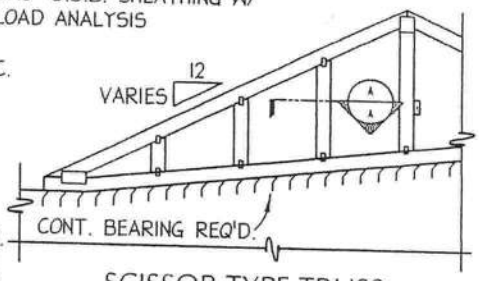
Porch Column Fasteners: Simpson AJU44/CC44 OR EQUAL

NOTE:
1. Balloon frame ALL gable ends unless this summary is accompanied by Gable End Wall Brace detail.
2. All trusses must bear on exterior walls & porch beams.
3. All walls to be nailed with same nailing pattern as shearwalls.
4. This is a windload only, NOT a structural analysis.
5. This windload is not valid without a raised, embossed seal.
6. It is assumed that ideal soil conditions and pad preparations are provided.
7. Fiber mesh or WWM may be used in concrete slab.
8. Trusses must be anchored and supported in accordance to the truss engineering.
9. Wind design and analysis valid for one use only, no copies permitted.

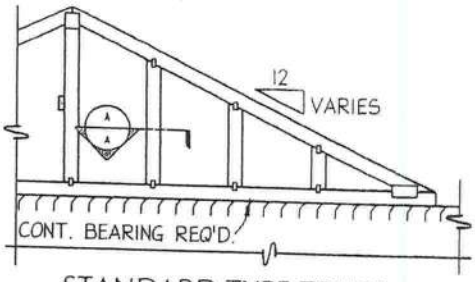




GABLE END SECTION

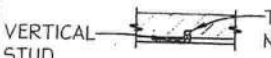


SCISSOR TYPE TRUSS



STANDARD TYPE TRUSS

MINIMUM GRADE OF LUMBER
TOP CHORD - 2X4 NO. 2 19 SP
BOTT. CHORD - 2X4 NO. 2 19 SP
WEBS - 2X4 NO. 3 19 SP



SECTION A-A

MAXIMUM VERTICAL STUDS HEIGHT			
SPACING OF VERTICALS	WITHOUT BRACE	WITH LATERAL BRACE	WITH L OR T BRACE
12" O.C.	5'-7"	14'-3"	10'-5"
16" O.C.	5'-1"	12'-5"	9'-6"
24" O.C.	4'-5"	10'-2"	8'-0"

LOADING (P.S.F.)		
LOAD TYPE	LIVE	DEAD
TOP	20	7
BOTTOM	0	10
SPACING 24" O.C.		

STR. INCR.: 33%
REP. STRESS: YES
TPI-95 CRIT.
SSBC-97

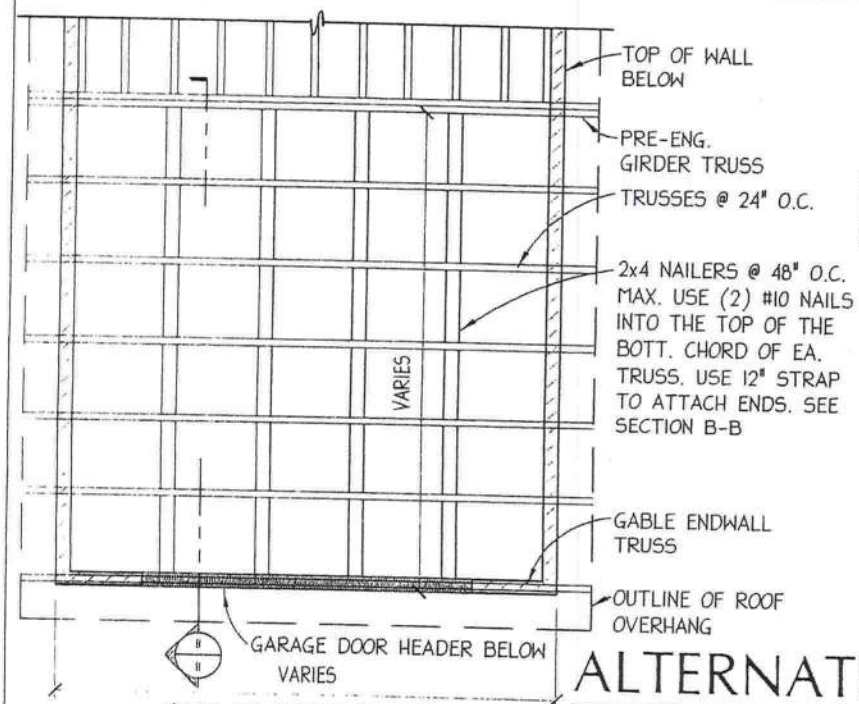
LATERAL BRACING NAILING SCHEDULES	
VERTICAL HEIGHT	# NAILS AT END
UP TO 7'-0"	2 - 16D
7'-0" TO 8'-8"	3 - 16D
OVER 8'-8"	4 - 16D

NOTES:

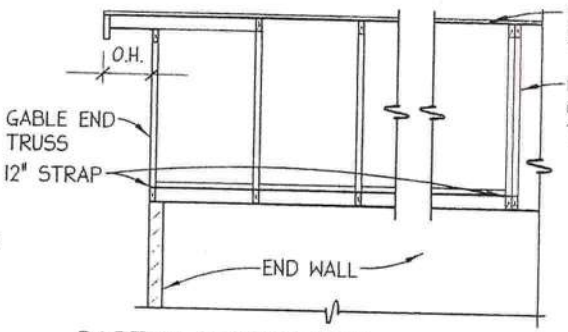
1. VERT. HAS BEEN CHECKED FOR 110 M.P.H. WIND LOAD, 25'-0" MEAN WALL HEIGHT.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END MUST BE MADE BY INSTALLING SIMPSON ST22 STRAPS OR EQUIV., PLACE STRAP ON EVERY OTHER VERT., OR 48" O.C. USING 10d NAILS ON STRAP.
3. REFER TO MANUF. NAILING RECOMMENDATIONS FOR ALL STRAPPING & HARDWARE
4. NAIL BLOCKING TO TRUSS W/ (2) 16d EA. END. NAIL X-BRACING TO BLOCKING W/ (3) 16d EA. END

STANDARD GABLE END DETAIL

NOT TO SCALE (NON BALLOON FRAMED GABLE END, 2x4 #2 GRADE MARKED LUMBER MIN.)



PLAN VIEW

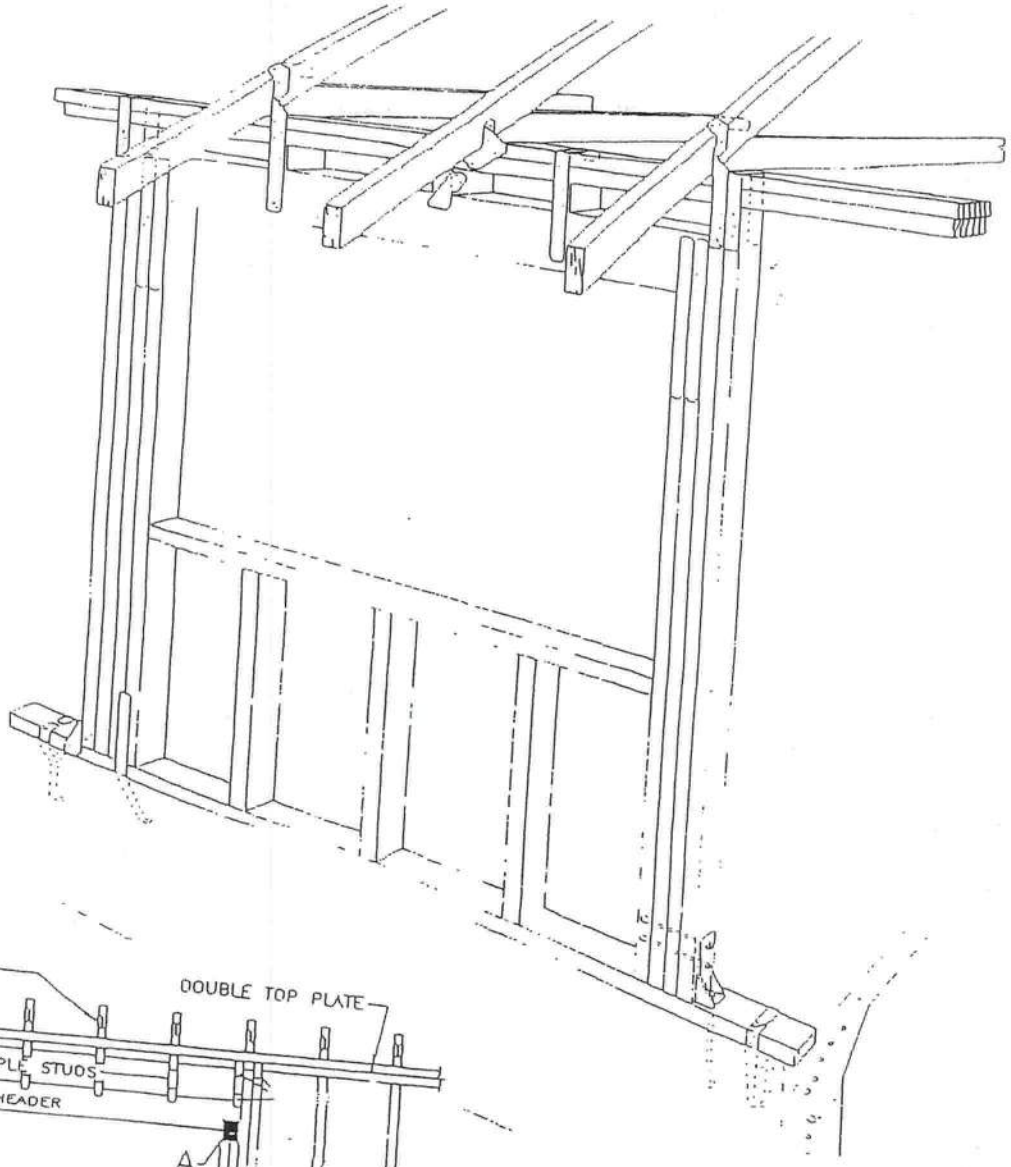


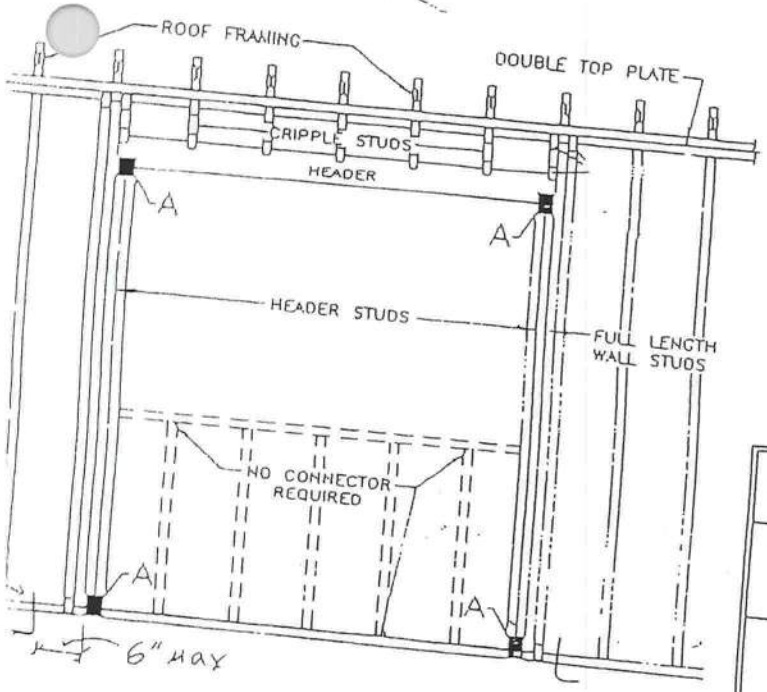
PARTIAL SECTION B-B

NOTE:
INSTALL AND ERECT ALL TRUSS MEMBERS IN STRICT CONFORMANCE WOTH THE PRE-ENGINEERED ROOF TRUSS MANUFACTURERS ERECTION SHEET ACCOMPANYING THE TRUSS PACKAGE. IF NOT AVAILABLE, IT IS THE CONTRACTOR'S RESPONSIBILITY TO CONTACT THE SUPPLIER TO OBTAIN ERECTION & BRACING SHEET.

ALTERNATE GABLE END DETAIL

NOT TO SCALE (NON BALLOON FRAMED GABLE END - UPLIFT ONLY, 2x4 #2 GRADE MARKED LUMBER MIN.)





Unsupported Wall Height	Stud Spacing	Maximum Header Span (ft.)					
		3'	6'	9'	12'	15'	18'
		Number of Header Studs Supporting End of Header					
		1[1]	2	2	2	2	2
		Number of Full-Length Studs at Each End of Header					
10' or less	12 in.	2	2	3	3	3	3
	16 in.	2	2	3	3	3	3
	24 in.	1	2	2	2	2	2
greater than 10'	12 in.	2	2	3	4	5	5
	16 in.	2	2	3	3	4	4
	24 in.	1	2	2	2	3	3

Total each truss uplift on the header divide by 2 for header anchorage

ASCE 7-98

Wind Load Design per ASCE 7-98

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	26.6	Deg
Type of Roof	Gabled	
Eave Height (Eht)	8.00	ft
Ridge Height (RHt)	21.64	ft
Mean Roof Height (Ht)	17.38	ft
Width Perp. to Wind (B)	73.33	ft
Width Parallel to Wind (L)	64.83	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.27
Flexible Structure	No

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	0.8803	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8544	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	V*(5280/3600)	161.33	ft/s
Vzm	bm*(zm/33)^Am*Vhref	70.89	ft/s
NF1	NatFreq*Lzm/Vzm	4.37	Hz
Rn	(7.47*NF1)/(1+10.302*NF1)^1.667	0.0552	
Nh	4.6*NatFreq*Ht/Vzm	1.13	
Nb	4.6*NatFreq*B/Vzm	4.76	
Nd	15.4*NatFreq*Depth/Vzm	14.08	
Rh	1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))	0.5348	
Rb	1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))	0.1881	
Rd	1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))	0.0685	
RR	((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^0.5	0.5586	
gg	+(2*LN(3600*n1))^0.5+0.577/(2*LN(3600*n1))^0.5	4.19	
Gust3	0.925*((1+1.7*Izm*(3.4^2*Q^2+GG^2*RR^2)^0.5)/(1+1.7*3.4*Izm))	0.99	

Gust Factor Summary			
Main Wind-force resisting system:		**Components and Cladding:**	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.85	Gust Factor (G)	0.85

ASCE 7-98

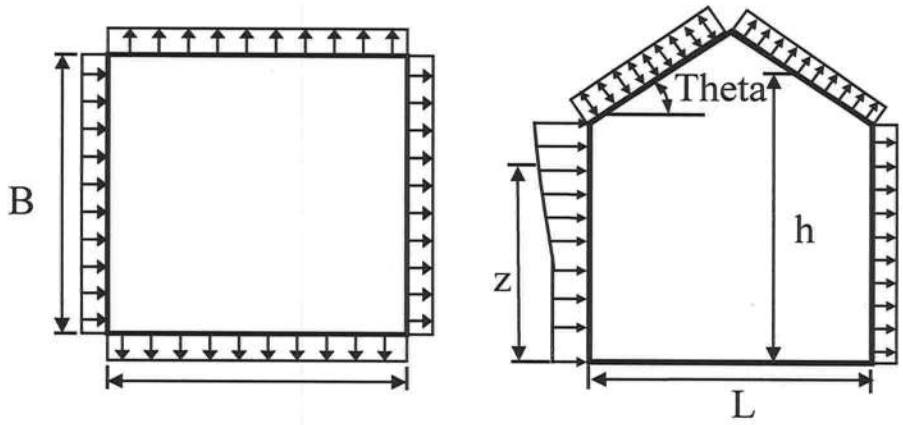
Wind Load Design per ASCE 7-98

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev.	Kz	Kzt	Kd	qz	Pressure (lb/ft^2)	
					Windward Wall*	
ft			1.00	lb/ft^2	+GCpi	-GCpi
21.64	0.70	1.00	1.00	21.70	11.49	18.17
20	0.70	1.00	1.00	21.70	11.49	18.17
17.38	0.70	1.00	1.00	21.70	11.49	18.17
15	0.70	1.00	1.00	21.70	11.49	18.17

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(Ht/zg)^(2/Alpha)	0.60	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*ImpFac*Kh*Kht*Kd	18.57	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 73.33 ft wall)	-0.50	-11.27	-4.59
Leeward Walls (Wind Dir Parallel to 64.83 ft wall)	-0.47	-10.86	-4.17
Side Walls	-0.70	-14.45	-7.76
Roof - Normal to Ridge (Theta>=10)			
Windward - Max Negative	-0.20	-6.59	0.09
Windward - Max Positive	0.29	1.30	7.99
Leeward Normal to Ridge	-0.60	-12.86	-6.18
Overhang Top	-0.20	-3.25	-3.25
Overhang Bottom	0.80	0.68	0.68
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 8.69 ft	-0.90	-17.62	-10.93

ASCE 7-98

Wind Load Design per ASCE 7-98

Dist from Windward Edge: 8.69 ft to 17.38 ft	-0.90	-17.62	-10.93
Dist from Windward Edge: 17.38 ft to 34.76 ft	-0.50	-11.27	-4.59
Dist from Windward Edge: > 34.76 ft	-0.30	-8.10	-1.42

* Horizontal distance from windward edge

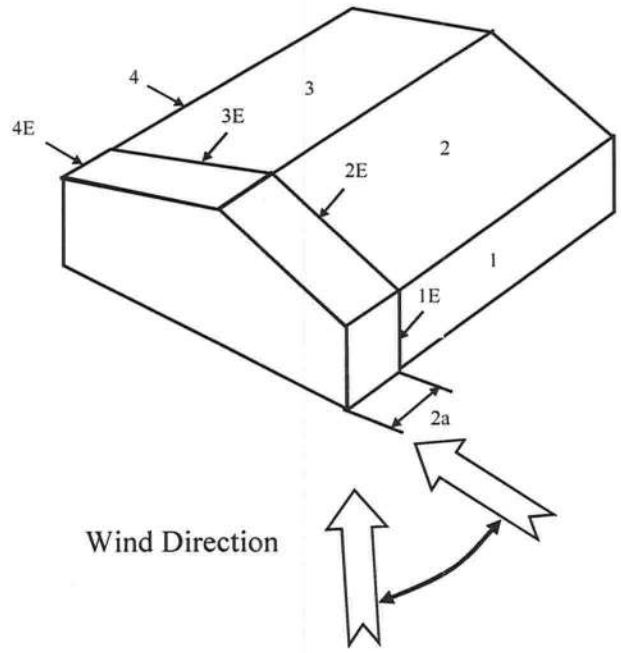
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.60
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	18.57

Case A						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	0.55	0.18	-0.18	21.70	8.03	15.84
2	-0.10	0.18	-0.18	21.70	-5.99	1.82
3	-0.45	0.18	-0.18	21.70	-13.61	-5.79
4	-0.39	0.18	-0.18	21.70	-12.38	-4.57
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.73	0.18	-0.18	21.70	11.88	19.69
2E	-0.19	0.18	-0.18	21.70	-7.93	-0.12
3E	-0.58	0.18	-0.18	21.70	-16.59	-8.78
4E	-0.53	0.18	-0.18	21.70	-15.50	-7.69
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

* p = qh * (GCpf - GCpi)



ASCE 7-98

Wind Load Design per ASCE 7-98

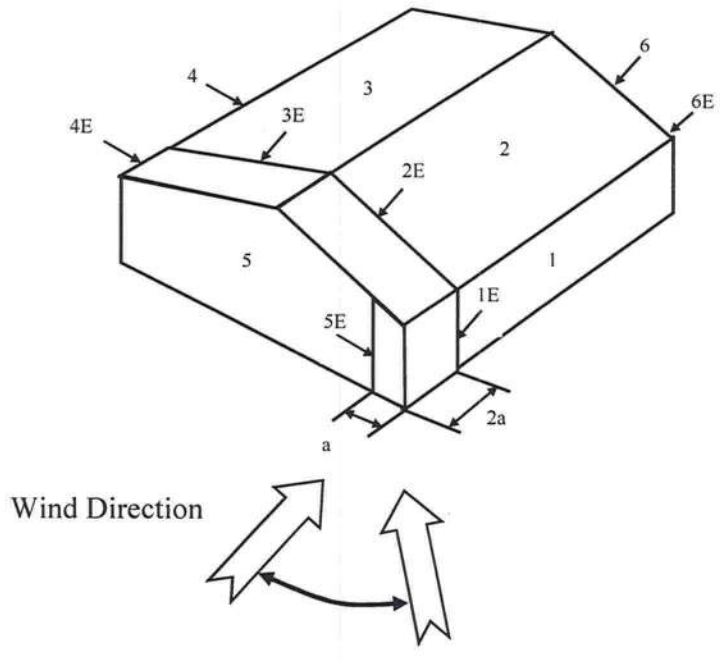
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.60
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	18.57

Case B						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

* p = qh * (GCpf - GCpi)

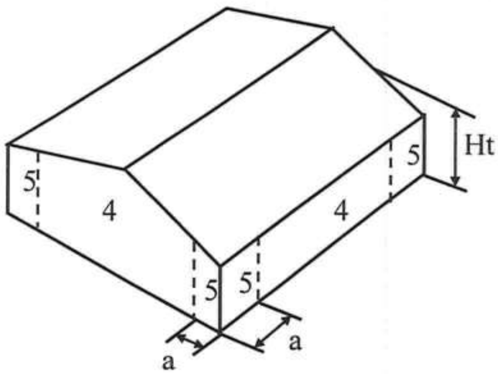


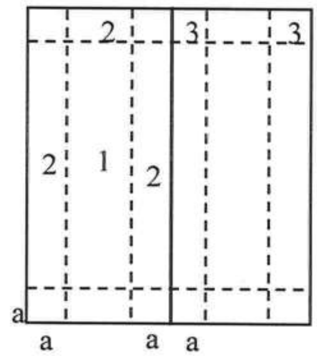
ASCE 7-98

Wind Load Design per ASCE 7-98

Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft





Gabled Roof
10 < Theta <= 45

a = 6.483 ==> 6.48 ft

Component	Width (ft)	Length (ft)	Area (ft^2)	Zone	GCp		Wind Press (lb/ft^2)	
					Max	Min	Max	Min
	16	7	112.00	5	0.81	-1.03	18.47	-22.46
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
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	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

ASCE 7-98

Wind Load Design per ASCE 7-98

Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

Table 6-8 External Pressure Coefficients for Arched Roofs, Cp

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	Cp		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Cp	0.13	-1	-0.5
	P (+GCpi) - psf	-1.36	-19.21	-11.27
	P (-GCpi) -psf	5.33	-12.52	-4.59
Roof Springing from Ground	Cp	0.42	-1	-0.5
	P (+GCpi) - psf	3.32	-19.21	-11.27
	P (-GCpi) -psf	3.32	-19.21	-11.27

Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf

Variable	Description	Value	
L	Roof dimension normal to wind direction	64.83	ft
B	Roof dimension parallel to wind direction	73.33	ft
L/B	Ratio of L to B	0.884	
Theta	Slope of Roof	26.6	Deg
Cf	Force Coefficient	1.18	
X	Distance to center of pressure from windward edge	0.42	ft

wrightsoft Version 6

Project Summary
Entire House
Bertie Heating & Air Conditioning

Job:
Date: Aug 18, 2005
By: Fred Townsend

1730 NE 23rd Avenue, Gainesville, Fl 32609 Phone: 352-371-2005 Fax: 352-371-4942

Project Information

For: Genesis Design & Construction
481 Turkey Creek, Alachua, Fl 32615
Phone: 352-514-5055 Fax: 386-462-4637

Notes: Green Residence
12 SEER Trane HP
All Venting

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33	°F
Inside db	70	°F
Design TD	37	°F

Summer Design Conditions

Outside db	***95***	°F
Inside db	75	°F
Design TD	20	°F
Daily range	M	
Relative humidity	50	%
Moisture difference	47	gr/lb

Heating Summary

Structure	35248	Btuh
Ducts	0	cfm
Central vent (0 cfm)	0	Btuh
Humidification	0	Btuh
Piping	0	Btuh
Equipment load	35248	Btuh

Sensible Cooling Equipment Load Sizing

Structure	30963	Btuh
Ducts	0	Btuh
Central vent (0 cfm)	0	Btuh
Blower	0	Btuh
Use manufacturer's data	n	
Rate/swing multiplier	1.00	
Equipment sensible load	30963	Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft²)	2190	2190
Volume (ft³)	19032	19032
Air changes/hour	0.70	0.40
Equiv. AVF (cfm)	222	127

Latent Cooling Equipment Load Sizing

Structure	4024	Btuh
Ducts	0	Btuh
Central vent (0 cfm)	0	Btuh
Equipment latent load	4024	Btuh
Equipment total load	34988	Btuh
Req. total capacity at 0.70 SHR	3.7	ton

Heating Equipment Summary

Make Trane
Trade XB12 Weathertron
Model 2TWB2048A1

Efficiency	8.2 HSPF	
Heating input		
Heating output	47000	Btuh @ 47°F
Temperature rise	26	°F
Actual air flow	1656	cfm
Air flow factor	0.047	cfm/Btuh
Static pressure	0.00	in H2O
Space thermostat		

Cooling Equipment Summary

Make Trane
Trade XB12 Weathertron
Cond 2TWB2048A1
Coil TWE048P13

Efficiency	12 SEER	
Sensible cooling	32900	Btuh
Latent cooling	14100	Btuh
Total cooling	47000	Btuh
Actual air flow	1656	cfm
Air flow factor	0.053	cfm/Btuh
Static pressure	0.00	in H2O
Load sensible heat ratio	0.88	

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 7th Ed.

wrightsoft Version 6

Short Form
Entire House
Bertie Heating & Air Conditioning

Job:
Date: Aug 18, 2005
By: Fred Townsend

1730 NE 23rd Avenue, Gainesville, Fl 32609 Phone: 352-371-2005 Fax: 352-371-4942

Project Information

For: Genesis Design & Construction
481 Turkey Creek, Alachua, Fl 32615
Phone: 352-514-5055 Fax: 386-462-4637

Design Information

	Htg	Clg
Outside db (°F)	33	***95***
Inside db (°F)	70	75
Design TD (°F)	37	20
Daily range	-	M
Inside humidity (%)	-	50
Moisture difference (gr/lb)	-	47

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

HEATING EQUIPMENT

Make Trane
Trade XB12 Weathertron
Model 2TWB2048A1

Efficiency	8.2 HSPF	
Heating input		
Heating output	47000	Btuh @ 47°F
Temperature rise	26	°F
Actual air flow	1656	cfm
Air flow factor	0.047	cfm/Btuh
Static pressure	0.00	in H2O
Space thermostat		

COOLING EQUIPMENT

Make Trane
Trade XB12 Weathertron
Cond 2TWB2048A1
Coil TWE048P13

Efficiency	12 SEER	
Sensible cooling	32900	Btuh
Latent cooling	14100	Btuh
Total cooling	47000	Btuh
Actual air flow	1656	cfm
Air flow factor	0.053	cfm/Btuh
Static pressure	0.00	in H2O
Load sensible heat ratio	0.88	

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
master bedroom	312	4787	5157	225	276
master wic 2	30	37	44	2	2
master wic 1	43	590	223	28	12
library	155	2171	1382	102	74
master bath	143	1786	1155	84	62
dining room	156	2113	1411	99	75
hall	148	0	0	0	0
family room	332	8150	9178	383	491
bedroom 3	170	2372	1602	111	86
bath 2	60	798	503	37	27
kitchen	161	1250	1446	59	77
breakfast nook	116	2947	3902	138	209
bedroom 2	161	3074	1758	144	94
laundry room	95	2784	1170	131	63
foyer	87	1760	1236	83	66
master t	21	629	795	30	43

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 7th Ed.

Entire House Other equip loads Equip. @ 1.00 RSM Latent cooling	d	2190	35248 0	30963 0 30963 4024	1656	1656
TOTALS		2190	35248	34988	1656	1656

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 7th Ed.

wrightsoft Version 6

Duct System Summary
Entire House
Bertie Heating & Air Conditioning

Job:
Date: Aug 18, 2005
By: Fred Townsend

1730 NE 23rd Avenue, Gainesville, Fl 32609 Phone: 352-371-2005 Fax: 352-371-4942

Project Information

For: Genesis Design & Construction
481 Turkey Creek, Alachua, Fl 32615
Phone: 352-514-5055 Fax: 386-462-4637

	Heating	Cooling
External static pressure	0.00 in H2O	0.00 in H2O
Pressure losses	0.00 in H2O	0.00 in H2O
Available static pressure	0.00 in H2O	0.00 in H2O
Supply / return available pressure	0.00 / 0.00 in H2O	0.00 / 0.00 in H2O
Lowest friction rate	***0.100*** in/100ft	***0.100*** in/100ft
Actual air flow	1656 cfm	1656 cfm
Total effective length (TEL)	0 ft	

Supply Branch Detail Table

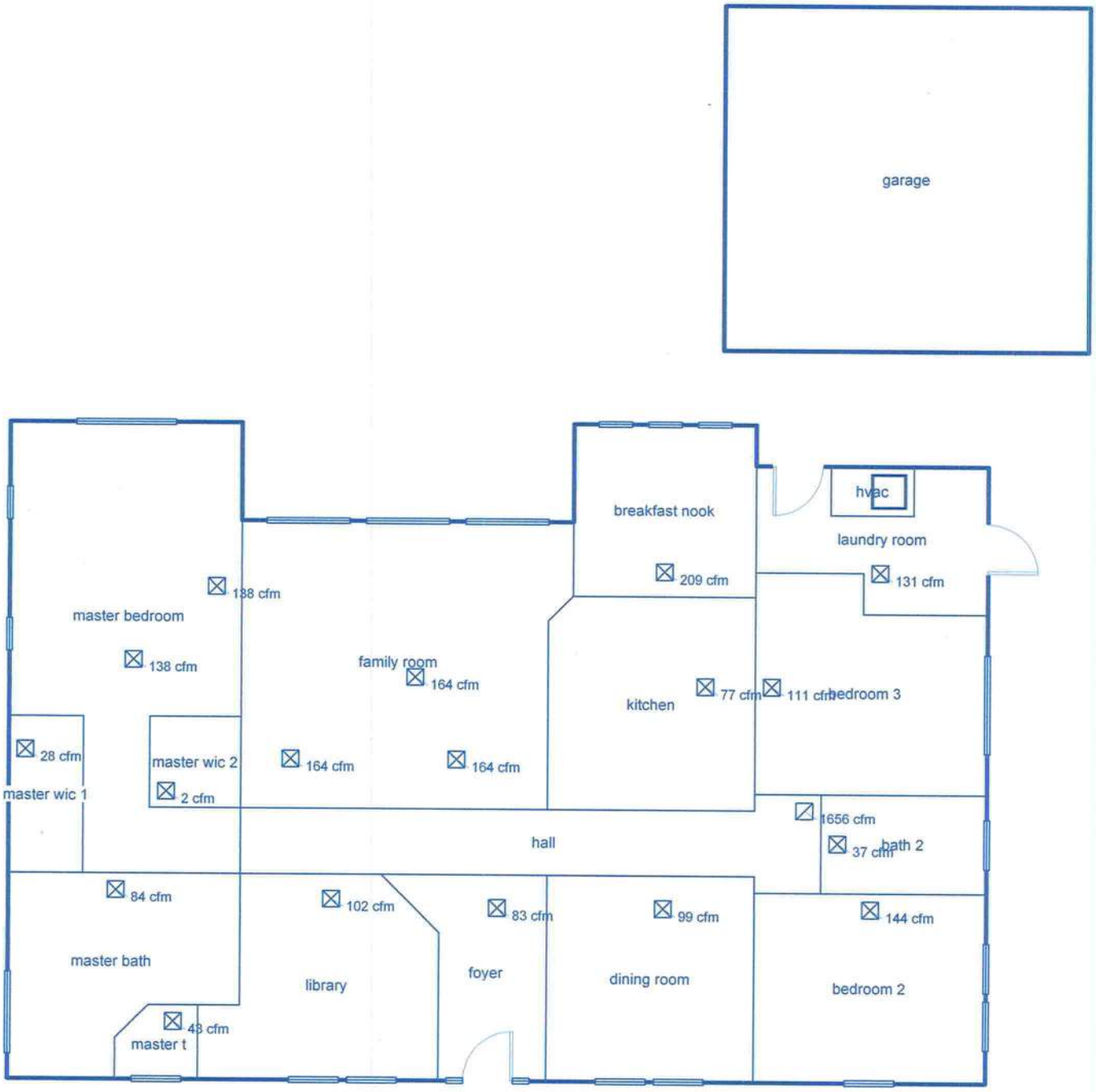
Name		Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
master bedroom-A	c	2579	112	138	0.100	7	0x0	ShMt	0.0	0.0	
master bedroom	c	2579	112	138	0.100	7	0x0	ShMt	0.0	0.0	
master wic 2	c	44	2	2	0.100	4	0x0	ShMt	0.0	0.0	
master wic 1	h	590	28	12	0.100	4	0x0	ShMt	0.0	0.0	
library	h	2171	102	74	0.100	6	0x0	ShMt	0.0	0.0	
master bath	h	1786	84	62	0.100	5	0x0	ShMt	0.0	0.0	
dining room	h	2113	99	75	0.100	6	0x0	ShMt	0.0	0.0	
family room-A	c	3059	128	164	0.100	7	0x0	ShMt	0.0	0.0	
family room-B	c	3059	128	164	0.100	7	0x0	ShMt	0.0	0.0	
family room	c	3059	128	164	0.100	7	0x0	ShMt	0.0	0.0	
bedroom 3	h	2372	111	86	0.100	6	0x0	ShMt	0.0	0.0	
bath 2	h	798	37	27	0.100	4	0x0	ShMt	0.0	0.0	
kitchen	c	1446	59	77	0.100	5	0x0	ShMt	0.0	0.0	
breakfast nook	c	3902	138	209	0.100	8	0x0	ShMt	0.0	0.0	
bedroom 2	h	3074	144	94	0.100	7	0x0	ShMt	0.0	0.0	
laundry room	h	2784	131	63	0.100	6	0x0	ShMt	0.0	0.0	
foyer	h	1760	83	66	0.100	5	0x0	ShMt	0.0	0.0	
master t	c	795	30	43	0.100	4	0x0	ShMt	0.0	0.0	

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	1656	1656	0.0	0.000	0	0	0x 0		ShMt	

Bold/italic values have been manually overridden

first floor



Job #: **Performed by Fred Townsend for:** Genesis Design & Construction 481 Turkey Creek Alachua, Fl 32615 Phone: 352-514-5055 Fax: 386-462-4637	**Bertie Heating & Air Conditioning** 1730 NE 23rd Avenue Gainesville, Fl 32609 Phone: 352-371-2005 Fax: 352-371-4942	Scale: 1 : 112 Page 1 Right-Suite Residential 6.0.22 RSR27178 2005-Aug-18 14:33:22 Z:\Public\Builders\Residential Contra...

FORM 600A-2001 EnergyGauge® 3.4

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Genesis - 2212**	Builder:	**Owner Builder**
Address:		Permitting Office:	
City, State:	,	Permit Number:	
Owner:		Jurisdiction Number:	
Climate Zone:	**North**		

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		3	
5. Is this a worst case?		Yes	
6. Conditioned floor area (ft²)		2212 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear glass, default U-factor	0.0 ft²	400.0 ft²	
b. Default tint, default U-factor	0.0 ft²	0.0 ft²	
c. Labeled U-factor or SHGC	0.0 ft²	0.0 ft²	
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 209.0(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 2090.0 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 2213.0 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior		Sup. R=6.0, 150.0 ft	
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 48.0 kBtu/hr	
	SEER: 12.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 48.0 kBtu/hr	
	HSPF: 8.20	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 40.0 gallons	
	EF: 0.89	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.18

Total as-built points: 31282
Total base points: 33944

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Gale Insulation

DATE: 8-29-05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT:

DATE: 9/30/05

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL:

DATE:

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	2212.0	20.04	7979.1	Double, Clear	E	0.0	0.0	155.0	42.06	1.00	6519.9	
				Double, Clear	S	0.0	0.0	35.0	35.87	1.00	1255.3	
				Double, Clear	W	0.0	0.0	157.0	38.52	1.00	6048.3	
				Double, Clear	N	0.0	0.0	53.0	19.20	1.00	1017.6	
				As-Built Total:				**400.0**			**14841.1**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	2090.0	1.50		3135.0	
Exterior	2090.0	1.70	3553.0									
Base Total:	**2090.0**		**3553.0**	**As-Built Total:**				**2090.0**			**3135.0**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	0.0	0.00	0.0	Exterior Insulated				154.0	4.10		631.4	
Exterior	154.0	6.10	939.4									
Base Total:	**154.0**		**939.4**	**As-Built Total:**				**154.0**			**631.4**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	2213.0	1.73	3828.5	Under Attic			30.0	2213.0	1.73 X 1.00		3828.5	
Base Total:	**2213.0**		**3828.5**	**As-Built Total:**				**2213.0**			**3828.5**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	209.0(p)	-37.0	-7733.0	Slab-On-Grade Edge Insulation			0.0	209.0(p	-41.20		-8610.8	
Raised	0.0	0.00	0.0									
Base Total:			**-7733.0**	**As-Built Total:**				**209.0**			**-8610.8**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	2212.0	10.21	22584.5					2212.0	10.21		22584.5	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE			AS-BUILT					
Summer Base Points:		**31151.5**	**Summer As-Built Points:**					**36409.7**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
31151.5	**0.4266**	**13289.2**	36409.7 **36409.7**	1.000 **1.00**	(1.090 x 1.147 x 0.91) **1.138**	0.284 **0.284**	1.000 **1.000**	11781.6 **11781.6**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	2212.0	12.74	5072.6	Double, Clear	E	0.0	0.0	155.0	18.79	1.00	2913.0	
				Double, Clear	S	0.0	0.0	35.0	13.30	1.00	465.3	
				Double, Clear	W	0.0	0.0	157.0	20.73	1.00	3254.3	
				Double, Clear	N	0.0	0.0	53.0	24.58	1.00	1302.6	
				As-Built Total:				**400.0**			**7935.2**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	2090.0	3.40		7106.0	
Exterior	2090.0	3.70	7733.0									
Base Total:	**2090.0**		**7733.0**	**As-Built Total:**				**2090.0**			**7106.0**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	0.0	0.00	0.0	Exterior Insulated				154.0	8.40		1293.6	
Exterior	154.0	12.30	1894.2									
Base Total:	**154.0**		**1894.2**	**As-Built Total:**				**154.0**			**1293.6**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X WCM =		Points	
Under Attic	2213.0	2.05	4536.6	Under Attic			30.0	2213.0	2.05 X 1.00		4536.6	
Base Total:	**2213.0**		**4536.6**	**As-Built Total:**				**2213.0**			**4536.6**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	209.0(p)	8.9	1860.1	Slab-On-Grade Edge Insulation			0.0	209.0(p	18.80		3929.2	
Raised	0.0	0.00	0.0									
Base Total:			**1860.1**	**As-Built Total:**				**209.0**			**3929.2**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	2212.0	-0.59	-1305.1					2212.0	-0.59		-1305.1	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE			AS-BUILT						
Winter Base Points:		**19791.4**	**Winter As-Built Points:**						**23495.6**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
19791.4	**0.6274**	**12417.1**	23495.6 **23495.6**	1.000 **1.00**	(1.069 x 1.169 x 0.93) **1.162**	0.416 **0.416**	1.000 **1.000**	11355.4 **11355.4**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2746.00	8238.0	40.0	0.89	3	1.00	2715.15	1.00	8145.4	
				As-Built Total:							**8145.4**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points +	Heating Points +	Hot Water Points =	Total Points	Cooling Points +	Heating Points +	Hot Water Points =	Total Points
13289	**12417**	**8238**	**33944**	**11782**	**11355**	**8145**	**31282**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 83.5

The higher the score, the more efficient the home.

, , , ,

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		3	
5. Is this a worst case?		Yes	
6. Conditioned floor area (ft²)		2212 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear glass, default U-factor	0.0 ft²	400.0 ft²	
b. Default tint, default U-factor	0.0 ft²	0.0 ft²	
c. Labeled U-factor or SHGC	0.0 ft²	0.0 ft²	
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 209.0(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 2090.0 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 2213.0 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior		Sup. R=6.0, 150.0 ft	
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 48.0 kBtu/hr	
	SEER: 12.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 48.0 kBtu/hr	
	HSPF: 8.20	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 40.0 gallons	
	EF: 0.89	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft^2
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:nlb

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INCORPORATED
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-37589.01
Test Date: 06/29/00
Report Date: 09/11/00
Expiration Date: 06/29/04

Project Summary: Architectural Testing, Inc. (ATI) was contracted to witness tests on a Series/Model 450, aluminum single hung window at the MI Home Products in-plant test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1 H-C30 54 x 90; Test Specimen #2 H-C40 52 x 72*. Test specimen descriptions and results are reported herein.

General Note: *An asterisk (*) next to the performance grade indicates that the size tested for optional performance was smaller than the minimum test size for the product type and class.*

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 450

Type: Aluminum Single Hung Window

Test Specimen #1 H-C30 54 x 90

Overall Size: 4' 6-1/2" wide by 7' 6-1/2" high

Sash Size: 4' 4" wide by 3' 9-3/4" high

Fixed Daylight Opening Size: 4' 1-1/2" wide by 3' 6-1/2" high

Screen Size: 4' 2-1/4" wide by 3' 8-1/2" high

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.testati.com

Test Specimen Description: (Continued)

Test Specimen #2: H-C40 52 x 72*

Overall Size: 4' 4-1/4" wide by 6' 0" high

Sash Size: 4' 2" wide by 3' 0-1/2" high

Fixed Daylight Opening Size: 3' 11-1/2" wide by 2' 9-1/2" high

Screen Size: 4' 0" wide by 2' 11" high

The following descriptions apply to all specimens.

Finish: All aluminum was painted.

Glazing Details: The lites utilized 5/8" thick sealed insulating glass units fabricated from two sheets of 3/32" thick clear annealed glass and an Intercept™ spacer system. The sash was channel glazed with a flexible gasket. The fixed lite was interior glazed onto single-sided adhesive foam tape and secured with extruded PVC glazing beads.

Weatherstripping:

Description	Quantity	Location
0.210" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Stiles
0.300" diameter by 0.187" backed foam-filled vinyl bulb gasket	Row	Bottom rail
0.400' high by 1/2" square polypile dust plug	4	One on each sash corner

Frame Construction: The main frame was constructed of thermally-broken extruded aluminum members with coped, butted and sealed corners. The fixed meeting rail was constructed of an extruded aluminum member with coped, butted and sealed ends fastened with two screws each.

Test Specimen Description: (Continued)

Sash Construction: The sash members were constructed of thermally-broken extruded aluminum members with coped, butted and sealed corners fastened with one screw each.

Screen Construction: The screen was constructed of rolled aluminum members with plastic keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Plastic snap latch	1	Midspan of bottom rail
Block and tackle balance system	2	One per jamb
Plastic tilt latch	2	One on each end of sash meeting rail
Metal pivot bar	2	One on each end of bottom rail

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test unit was installed into the nominal 2" x 8" Spruce-Pine-Fir #2 wood test buck utilizing the integral nailing fin secured with 1" long galvanized roofing nails, 6" from each corner and every 18" on center. The nailing fin was also bedded in polyurethane. The exterior perimeter was blindstopped with wood members and secured with #8 x 3" screws every 24" on center.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: H-C30 54 x 90			
2.2.1.6.1	Operating Force	20 lbs	45 lbs max.
	Air Infiltration per ASTM E 283 (See Note #1) @ 1.57 psf (25 mph)	0.27 cfm/ft^2	0.3 cfm/ft^2 max.
	Water Resistance per ASTM E 547 (with and without screen) WTP = 4.5 psf	No leakage	No leakage
2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the fixed meeting rail)		
	@ 45.0 psf (exterior)	0.03"	0.22" max.
	@ 45.0 psf (interior)	0.04"	0.22" max.
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction at 70 lbs		
	Meeting rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance per ASTM F 588-97		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Note #1: *The tested specimen meets (or exceeds) the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

Test Results:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: (Continued)			
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) WTP = 5.25 psf	No leakage	No leakage
Test Specimen #2: H-C40 52 X 72*			
Optional Performance			
4.3	Water Resistance per ASTM E 547 and 331 (with and without screen) WTP = 6.0 psf	No leakage	No leakage
4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the fixed meeting rail) (Loads held for 33 seconds)		
	@ 47.0 psf (exterior)	0.04"	N/A
	@ 47.0 psf (interior)	0.03"	N/A
	(Loads held for 10 seconds)		
	@ 70.5 psf (exterior)	0.07"	0.21" max.
	@ 70.5 psf (interior)	0.04"	0.21" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product which may only be granted by the certification program administrator.

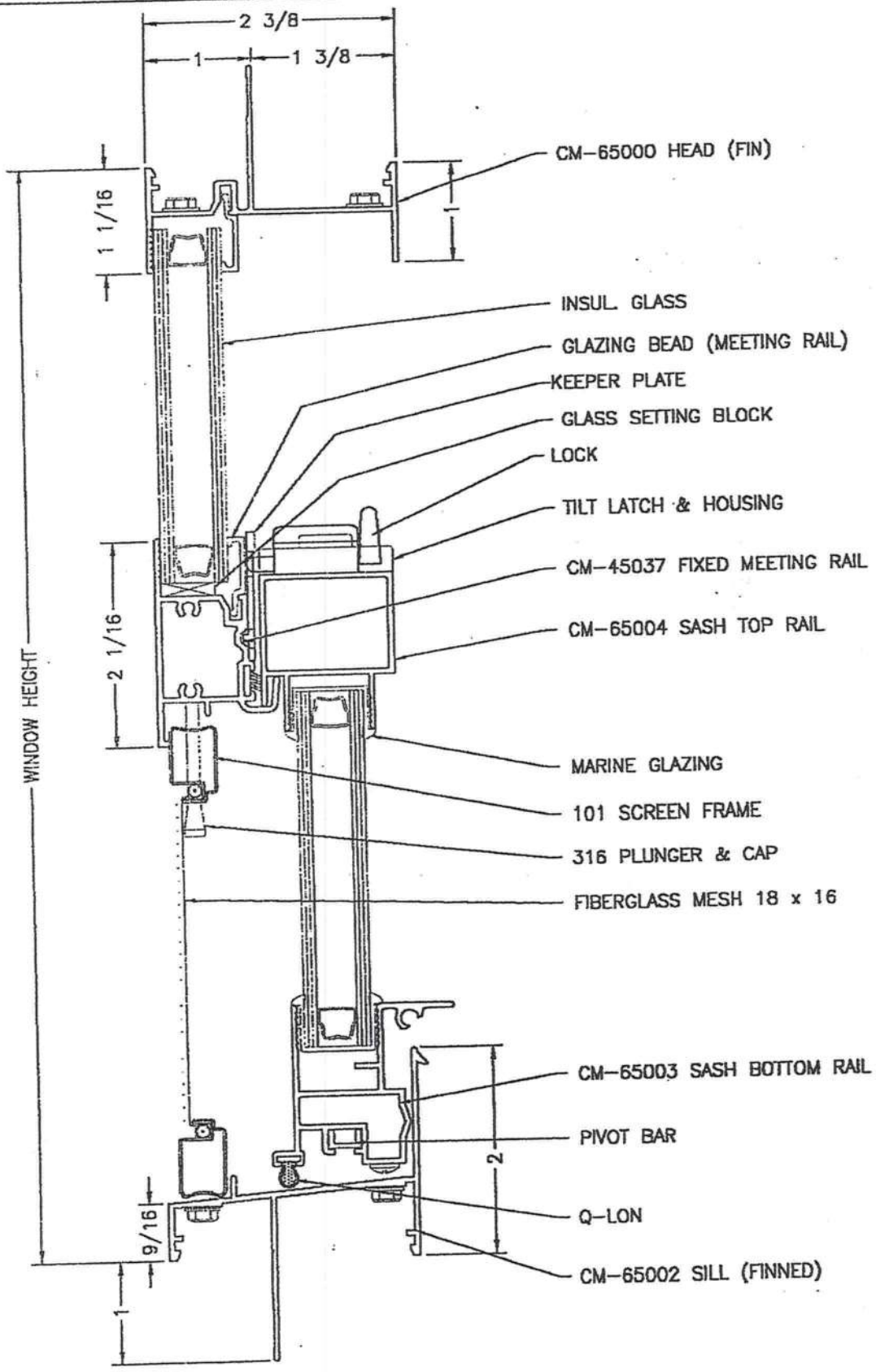
For ARCHITECTURAL TESTING, INC:

Adam A. Fodor
Technician

Bruce W. Croak
Director – Product/Physical Testing

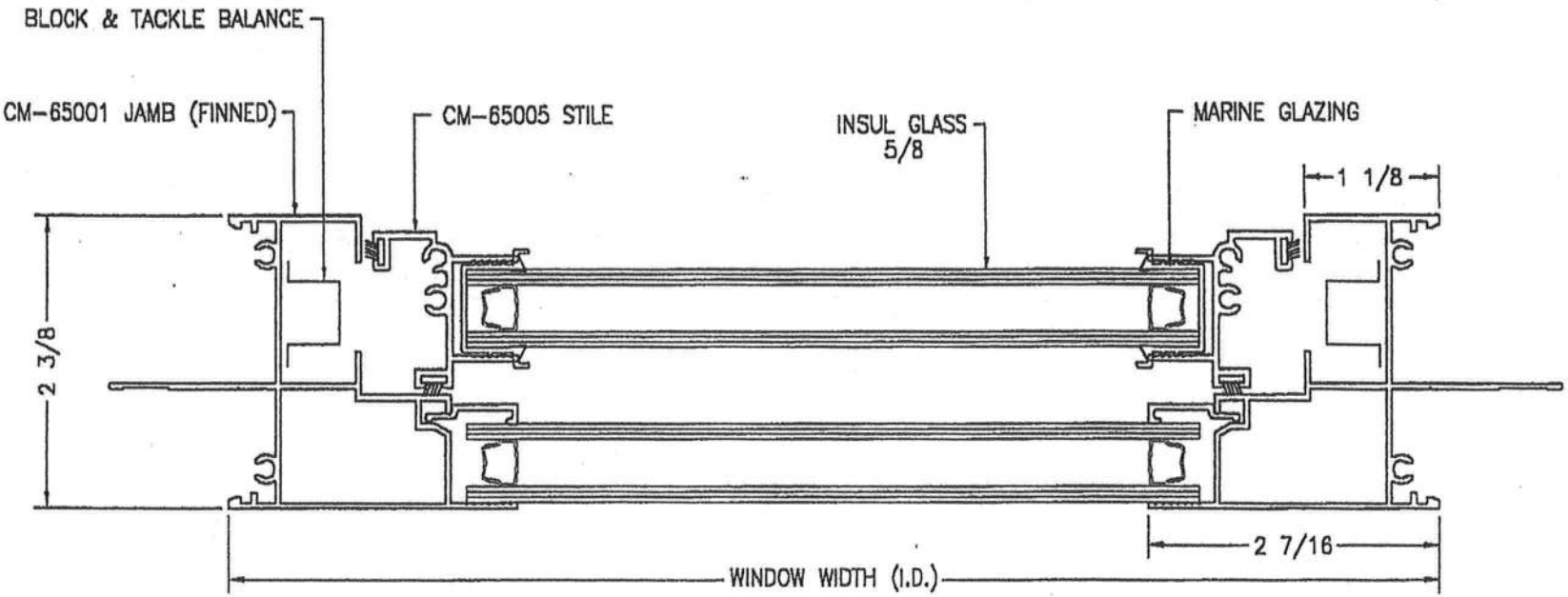
AAF:
01-37589.01

650-AS1 A



MI HOME PRODUCTS
650 WEST MARKET STREET • GRATZ, PA • 17030-0370

LTR	DESCRIPTION REVISIONS	BY	DATE	TITLE: 650 SH FIN MAIN FRAME VERTICAL CROSS SECTION			
				DFTM: V.M.R.	DATE: 4-7-92	SCALE: FULL	DWG. NO.: 650-AS1 REV: A



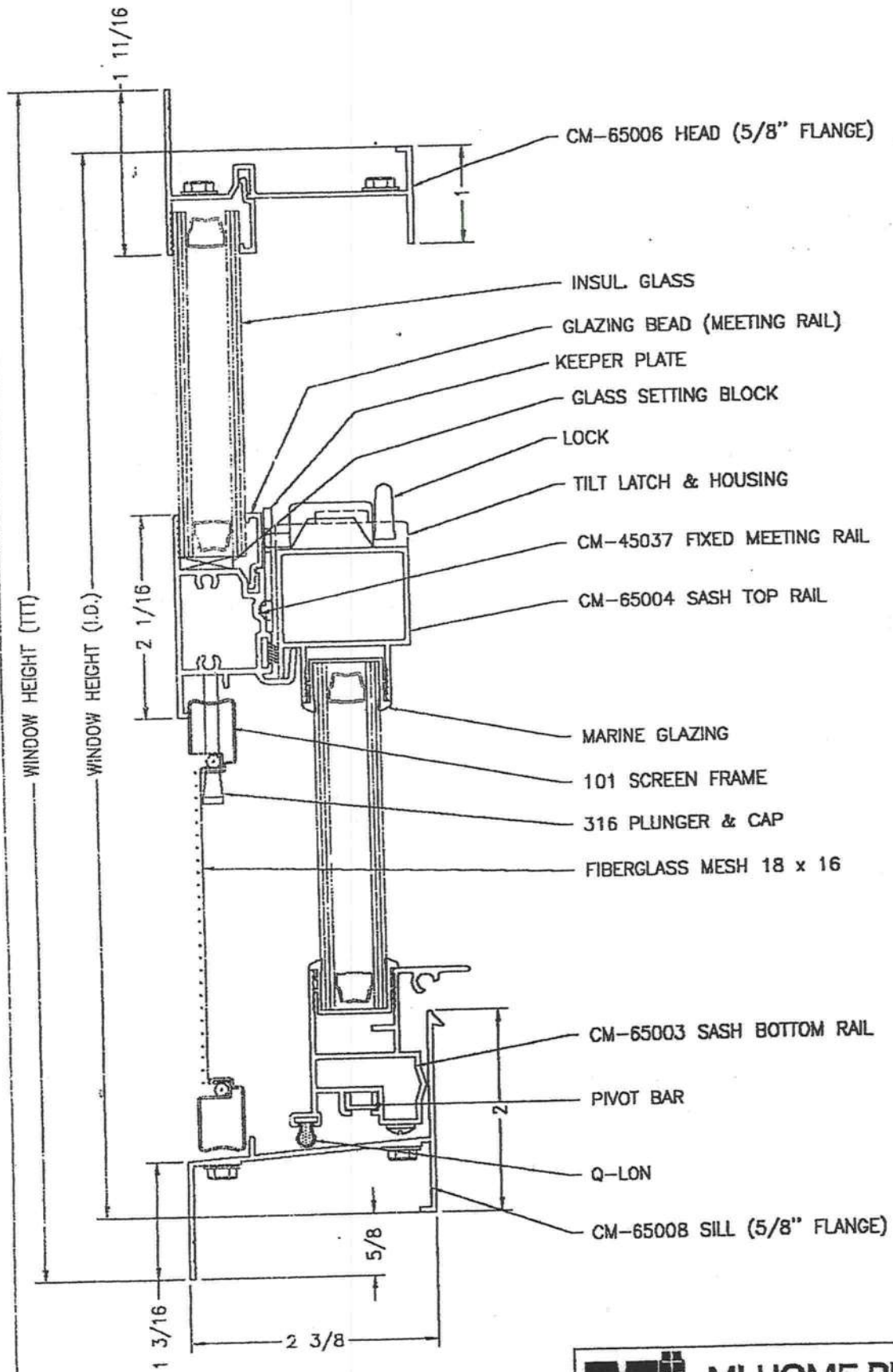
LTR	DESCRIPTION REVISIONS	BY	DATE

MI HOME PRODUCTS
650 WEST MARKET STREET • GRATZ, PA • 17030-0370

TITLE: 650 SH FIN MAIN FRAME INSULATED GLASS HORIZONTAL CROSS SECTION

DRWN	DATE	SCALE	DRAWING NO.	REV
V.M.R.	4-7-92	FULL	650-AS2	B

650-AS3 A

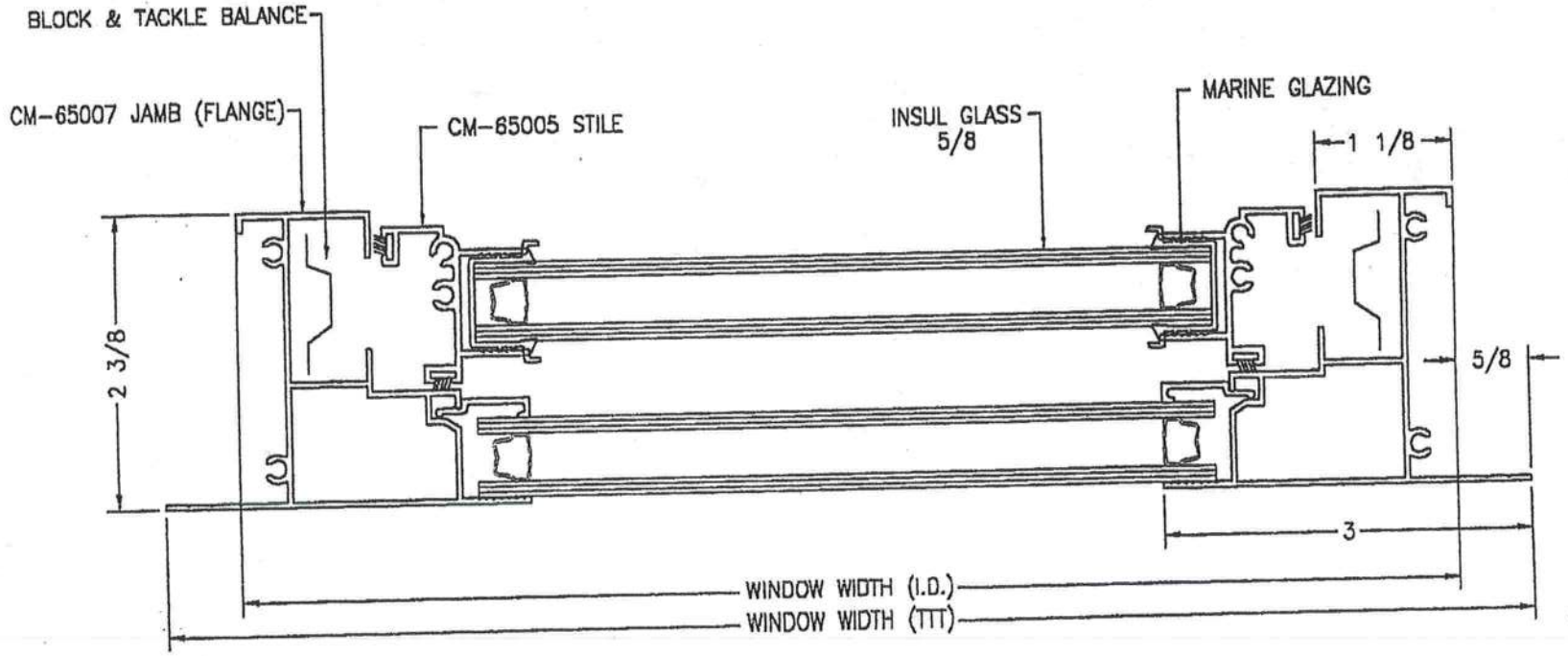


LTR	DESCRIPTION REVISIONS	BY	DATE

MI HOME PRODUCTS
650 WEST MARKET STREET • GRATZ, PA • 17030-0370

TITLE 650 SH L-FLANGE MAIN FRAME INSUL GLASS VERTICAL CROSS SECTION

DFTM	DATE	SCALE	DWG. NO.	REV.
V.M.R.	7-15-92	FULL	650-AS3	A



LTR.	DESCRIPTION REVISIONS	BY	DATE

MI HOME PRODUCTS
650 WEST MARKET STREET • GRATZ, PA • 17030-0370

TITLE: 650 SINGLE HUNG FLANGE FRAME HORIZONTAL ASSEMBLY

DRWN.	DATE	SCALE	DRAWING NO.	REV.
V.M.R.	7-27-93	FULL	650-AS4	

ARCHITECTURAL TESTING INC.
130 Derry Court • York, PA 17402-9405
web www.testati.com • Facsimile 717-764-4129 • Telephone 717-764-7700

STRUCTURAL TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650
TYPE: Twin Aluminum Single Hung Window

Title of Test	Results
Overall Design Pressure	35.0 psf
Operating Force	18 lb max.
Air Infiltration	0.29 cfm/ft^2
Water Resistance	5.25 psf
Structural Test Pressure	70.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-36060.01 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Scott D. Kramer

Scott D. Kramer, Technician

SDK:nlb

ARCHITECTURAL TESTING INC.
130 Derry Court • York, PA 17402-9405
web www.testati.com • Facsimile 717-764-4129 • Telephone 717-764-7700

OK FOR 120

5-10 x 5-0 TWIN
(QUALIFIES 30 x 5-0)

STRUCTURAL TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650
TYPE: Twin Aluminum Single Hung Window

CONTINUOUS HEAD & SILL

Title of Test	Results
Overall Design Pressure	35.0 psf
Operating Force	18 lb max
Air Infiltration	0.29 cfm/ft²
Water Resistance	5.25 psf
Structural Test Pressure	70.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

≈ DP 47

Reference should be made to Report No. 01-36060.01 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Scott D. Kramer

Scott D. Kramer, Technician

SDK:alb

ARCHITECTURAL TESTING INC.

130 Derry Court • York, PA 17402-9405
web www.testati.com • Facsimile 717-764-4129 • Telephone 717-764-7700

STRUCTURAL TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-36060.01
Test Date: 11/04/99
Report Date: 11/29/99
Expiration Date: 11/04/03

Project Summary: Architectural Testing, Inc. (ATI) was contracted to perform tests on a Series/Model 650, twin aluminum single hung window at MI Home Products' test facility in Elizabethville, Pennsylvania. Test specimen description and results are reported herein.

Test Specification: The test specimen was evaluated in accordance with the following:

ASTM E 283-91, *Standard Test Method for Determining the Rate of Air Leakage Through Exterior Windows, Curtain Walls, and Doors Under Specified Pressure Differences Across the Specimen*

ASTM E 330-97, *Standard Test Method for Structural Performance of Exterior Windows, Curtain Walls, and Doors by Uniform Static Air Pressure Difference*

ASTM E 547-96, *Standard Test Method for Water Penetration of Exterior Windows, Curtain Walls, and Doors by Cyclic Static Air Pressure Differential*

Test Specimen Description:

Series/Model: 650

Type: Twin Aluminum Single Hung Window

Overall Size: 5' 10-1/4" wide by 5' 0" high

Active Size (2): 2' 8-3/4" wide by 2' 6-1/4" high

Fixed Daylight Opening Size (2): 2' 6- 1/4" wide by 2' 3" high

Screen Size (2): 2' 7-3/4" wide by 2' 4-1/4" high

t Specimen Description: (Continued)

Finish: All aluminum was painted white.

Glazing Details: Both the active sash and fixed lites utilized 5/8" thick insulating glass fabricated from two sheets of 3/32" thick clear annealed glass and a desiccant filled metal spacer system. The active sash were channel glazed with a flexible wedge gasket. The fixed lites were interior glazed, back bedded with single sided adhesive foam tape and held-in-place with PVC snap-in glazing beads.

Weatherstripping:

Description	Quantity	Location
0.270" backed by 0.190" high polypile with center fin	1 Row	Fixed meeting stile
3/8" high vinyl wrapped foam bulb	1 Row	Bottom rail
0.187" backed by 0.250" high polypile with center fin	2 Rows	Stiles
1/4" high polypile dust plug	2 Rows	Ends of bottom rail, top of each stile

Frame Construction: Frame was constructed of extruded aluminum members and all corners were coped, butted, sealed, and fastened with two screws per corner. The fixed meeting rail was attached to the jambs with a plastic clip and two screws per end.

Mullion Construction: The mullion was constructed of an extruded aluminum member. It was fastened to the head and sill with four screws per end. All screw heads were sealed as well as the butt joint at the sill.

Sash Construction: The sash were constructed of extruded aluminum members and all corners were coped, butted, and fastened with one screw per corner.

Screen Construction: The screen was constructed of rolled aluminum members and the corners were keyed. The screen mesh was held-in-place with a flexible spline.

t Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Plastic tilt latches	4	Ends of interior meeting rail
Metal pivot bars	4	Ends of the bottom rails
Metal sweep lock	2	Midspan of interior meeting rail
Metal keeper	2	Midspan of fixed meeting rail
Sash stops	4	One per jamb
Block and tackle balance system	4	One per jamb
Spring loaded latch pins	2	6" from ends of screen top rail

Drainage:

Description	Quantity	Location
Sloped sill	1	Sill
1/4" wide by 3/16" high weepslot	4	Ends of exterior vertical sill leg

Installation: The test unit was installed into the 2" x 8" nominal Spruce-Pine-Fir #2 wood test buck utilizing the integral nailing fin and 1" roofing nails. Five per top, bottom, and sides of the nail fin were evenly spaced. The nail fin was bedded in a silicone sealant.

Test Results:

The results are tabulated as follows:

Title of Test - Test Method	Results
Air Infiltration per ASTM E 283	
@ 0.56 psf (15 mph)	0.15 cfm/ft^2
@ 1.57 psf (25 mph)	0.29 cfm/ft^2
Water Resistance per ASTM E 547 (with and without screen) WTP = 5.25 psf	No leakage
Uniform Load Structural (Measurements reported were taken on the meeting rail) (load held for 33 seconds)	
@ 47.0 psf (exterior)	0.010"
@ 47.0 psf (interior)	0.015"
Uniform Load Structural (Measurements reported were taken on the meeting rail) (load held for 10 seconds)	
@ 70.5 psf (exterior)	0.060"
@ 70.5 psf (interior)	0.040"

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Scott D. Kramer
Technician

Bruce W. Croak
Project Manager

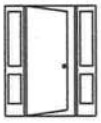
SDK:nlb
01-36060.01

Installation Instructions Pre-Hung Door Systems In High Wind Velocity Areas

These instructions apply to all Therma-Tru wood-framed door systems. Some apply specifically to:

Inswing Doors 

Outswing Doors 

Doors with sidelights 

Double Doors 

Patio Doors 

Read all instructions before starting.

THERMA-TRU® DOORS

The Most Preferred Brand in the Business™

P.O. Box 8780 Maumee, Ohio 43537

Printed 3/02 Questions? Comments? Call 1-800-THERMATRU (1-800-843-7628) Part No. MSDRISTH

KEY DIFFERENCES CHECKLIST

Rough openings are 1/4″ smaller on each side and 1/4″ smaller on the header than standard Therma-Tru rough opening dimensions.

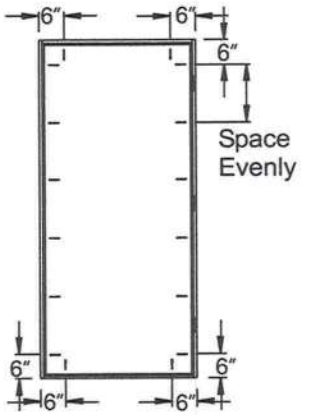
Use only a maximum of 1/4″ thick shims.

Predrill the jambs and headers using 1/8″ drill bit. Predrilling increases structural performance.

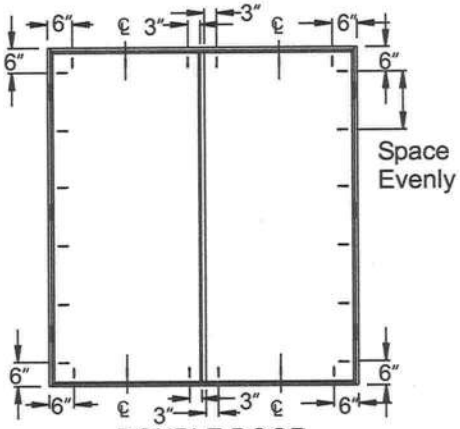
Use only 2-1/2″ wood screws (#8 and #10) when securing the pre-hung frame to the rough opening buck. Drywall screws, deck screws, and nails are insufficient anchors, and should not be substituted for wood screws.

Be certain of the number and placement of all screws securing the frame to the rough opening buck. Consult the architect or engineer of record concerning fastening the buck to the rest of the building structure.

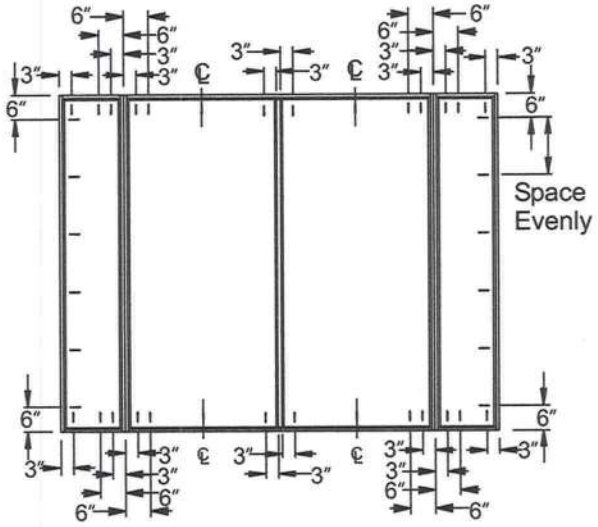
For structural strength, the rough opening buck must be either 2″ x 4″ or 2″ x 6″ construction, not ″1 by″ construction.



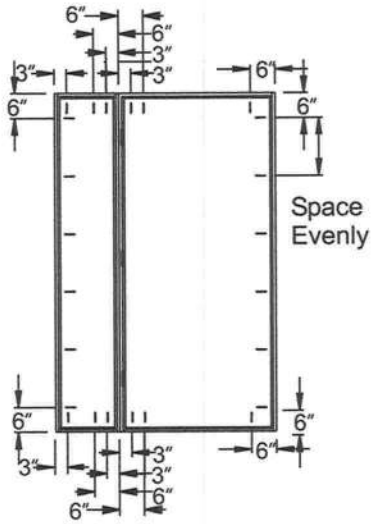
SINGLE DOOR



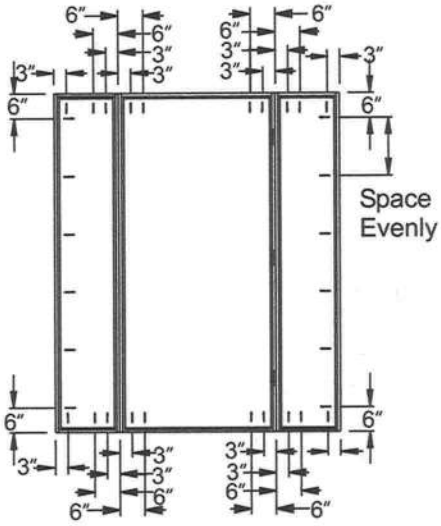
DOUBLE DOOR



DOUBLE DOOR W/SIDELITES



SINGLE DOOR W/SIDELITE



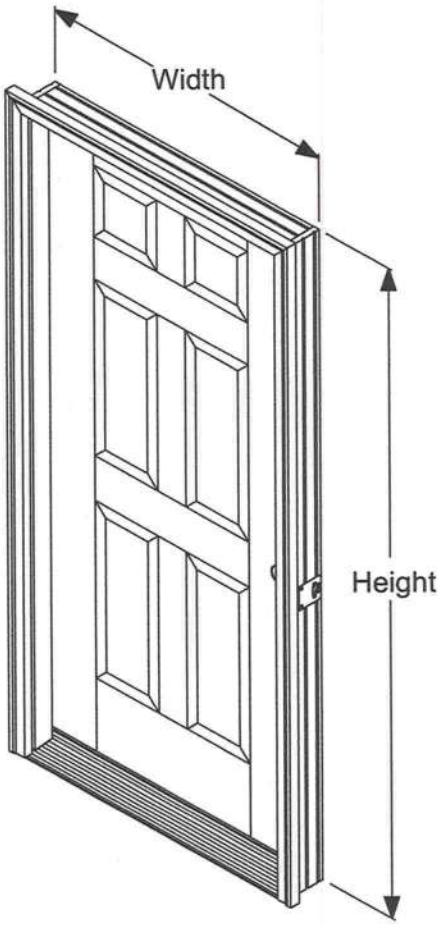
SINGLE DOOR W/SIDELITES

1 CHECK DOOR UNIT.

Check width and height.

Measure size of frame (width and height), not brickmold.

Remove cleats and packaging, but keep door fastened closed with transport clip. Do not remove clip or open door until instructed to do so.



2 CHECK AND PREPARE OPENING.

Is subfloor level and solid? *Provide a flat, level, clean bearing surface so the sill may be caulked and sealed to the opening. Scrape, sand, or fill as required.*

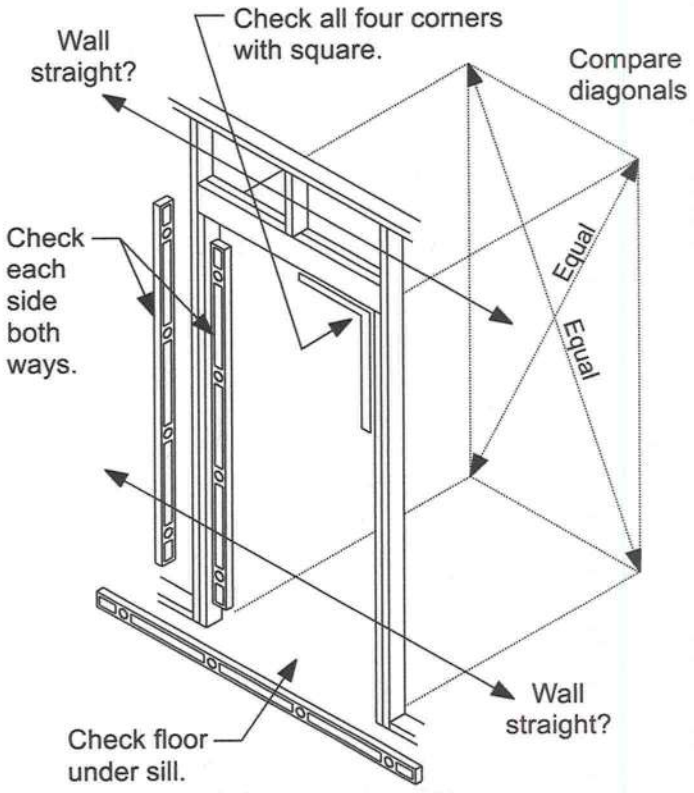
Shim subfloor for floor covering clearance, if required. *If shimming, caulk under shims.*

Is opening square? Check all corners with a framing square. Double-check by comparing diagonal measurements. *Fix any problems now.*

Are framing and walls plumb? Use a 6-foot level and check both sides of opening, both ways. *Fix any problems now.*

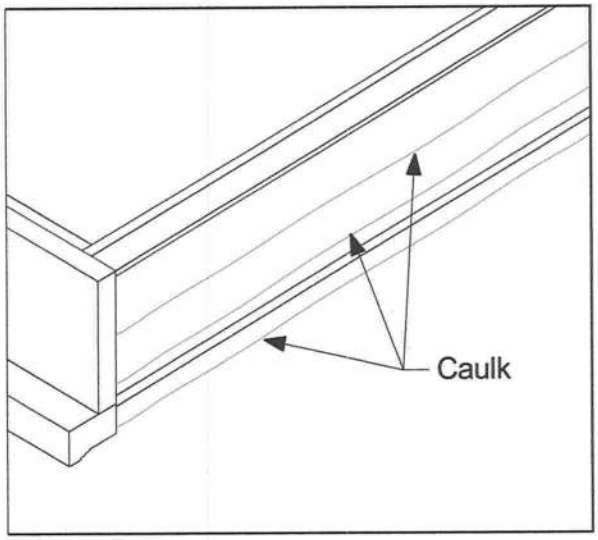
Are the wall surfaces around the opening in the same plane? There must be no "warps" or "jogs". *Fix any problems now.*

Is the opening the correct size? Check it against the door frame size now, before installation. Opening should be frame height plus **1/4"**, and frame width plus **1/2"**. Remember to use only 1/4" shims.



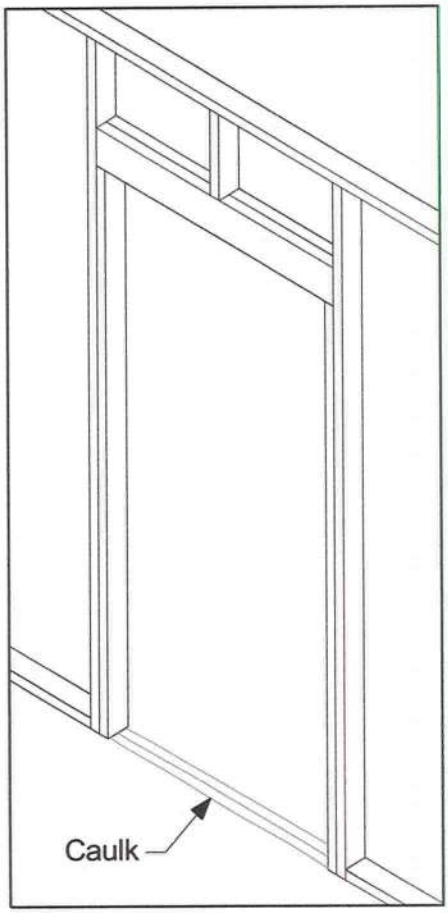
3 CAULK SUBFLOOR AND SILL BOTTOM.

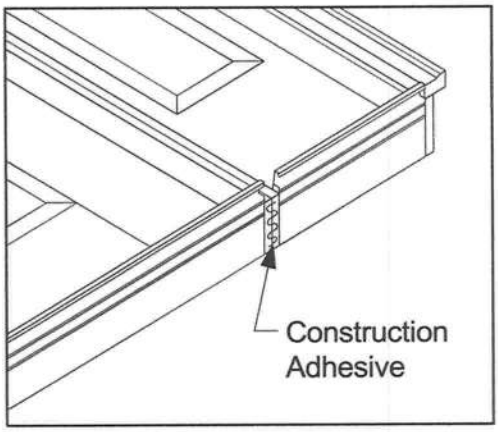
Lay door unit on edge or face so that bottom side of sill can be caulked. Place very large beads of caulk across full width at front edge of sill. Place one or more very large caulk beads at parallel lines across bottom surfaces which will bear on subfloor.



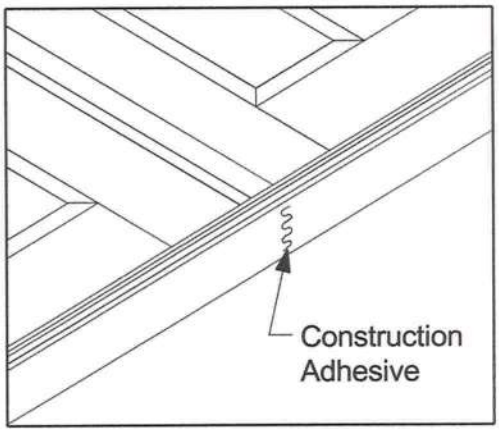
Avoid callbacks due to leaks! Use an entire tube of caulking to seal between sill and subfloor.

On subfloor at opening, place very large beads of caulk that will match with those placed on sill. Run beads full width of opening.





NOTE: If installing **units with sidelights,** place construction adhesive under points where door jambs will bear on floor. Complete installation before adhesive cures.



NOTE: If installing **double door** or **patio units,** place construction adhesive under mullion or point where doors meet at center. Complete installation before adhesive cures.

4 *PLACE UNIT IN OPENING AND TEMPORARILY FASTEN HINGE JAMB. DO NOT FASTEN THROUGH BRICKMOULD.*

If the jamb and head does not come with pilot holes, drill 1/8″ pilot holes before using screws.

For single or double doors, note hinge locations and mark jamb faces near door surface, for fastener placement later.

Lift unit up. With top edge tilted away from opening, center unit and place sill down onto caulk beads. Tilt into place.

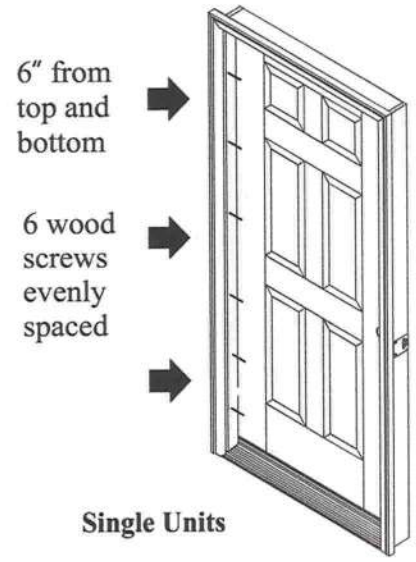
Work from the side of the door that is weather-stripped. (*outside* for **inswing** doors, *Inside* for **outswing** doors)

Plumb hinge side jamb both ways. Use a 6-foot level.

Use shims totaling a maximum of 1/4″ thickness, not the usual 1/2″.

Use 2-1/2″ wood screws. Do NOT substitute nails, deck screws, or drywall screws . Place **six** #8 wood screws through jambs into "two-by" studs, at each location where shown in diagrams. **For single or double doors,** refer to marks on jambs and place fasteners *below* each hinge location, so that shims may be placed behind hinges. Fasteners will keep shims from falling down while adjustments are made.

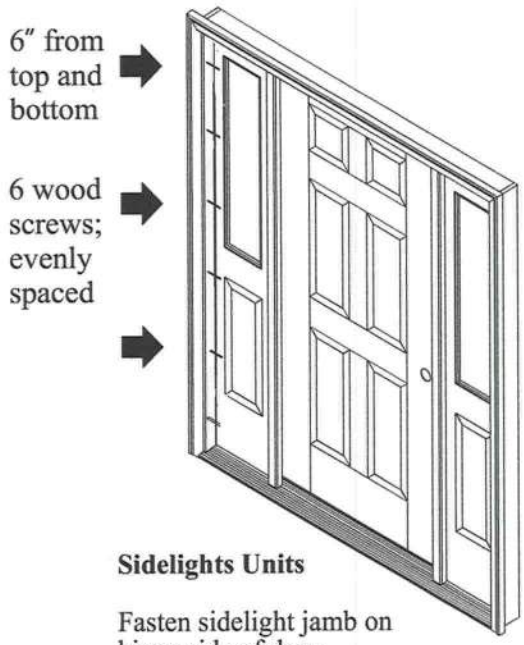
Do not drive screws completely in at this time.



Single Units

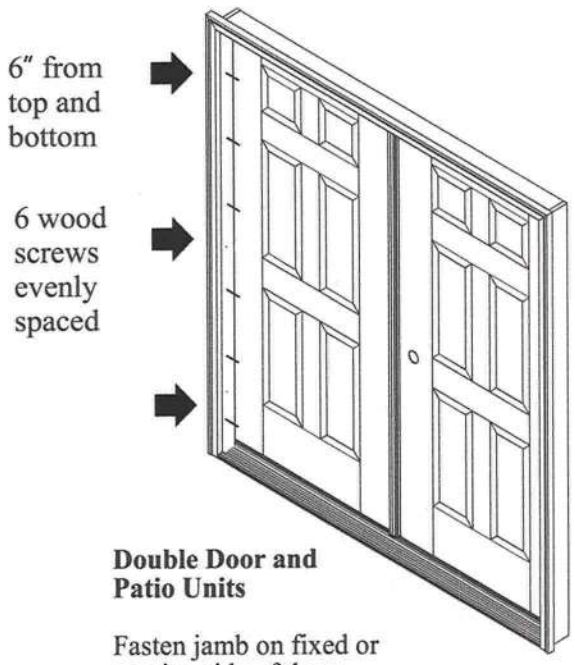
Fasten hinge jamb.

All screws used outside should be coated or galvanized to prevent rusting and staining.



Sidelights Units

Fasten sidelight jamb on hinge side of door.



Double Door and Patio Units

Fasten jamb on fixed or passive side of door.

5 SHIM BEHIND HINGES AND SECURE HINGE JAMB.

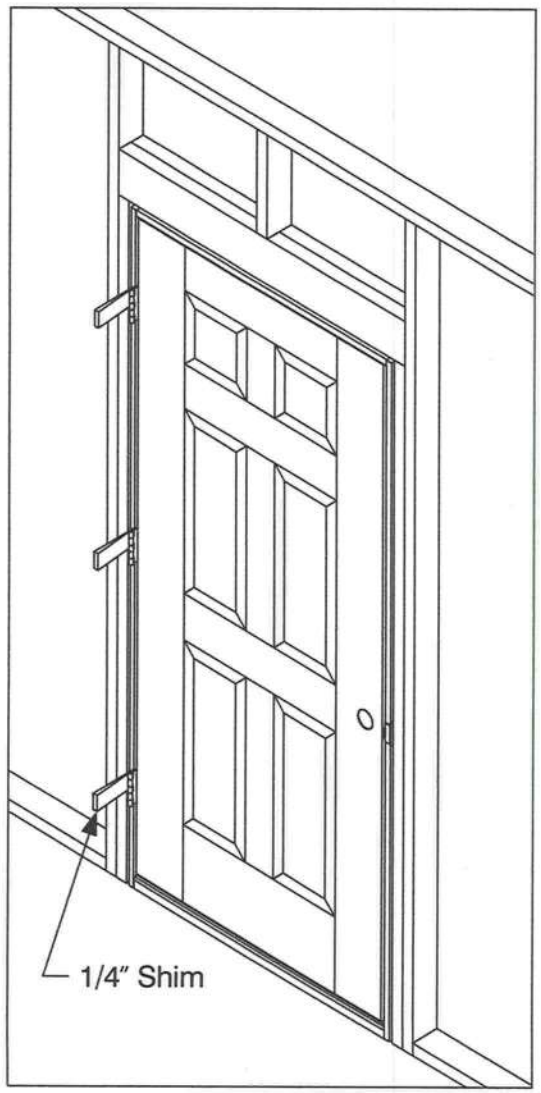
Go through another opening to work on swing side of door.

Leave door fastened closed with transport clip.

Shim above fasteners, behind each hinge, between jamb and opening. Shim a total of 1/4″ maximum per side.

Recheck hinge jamb to ensure it is plumb and straight. Use a 6-foot level.

Go back through another opening to other side of door to secure hinge jamb. Finish driving the wood screws.



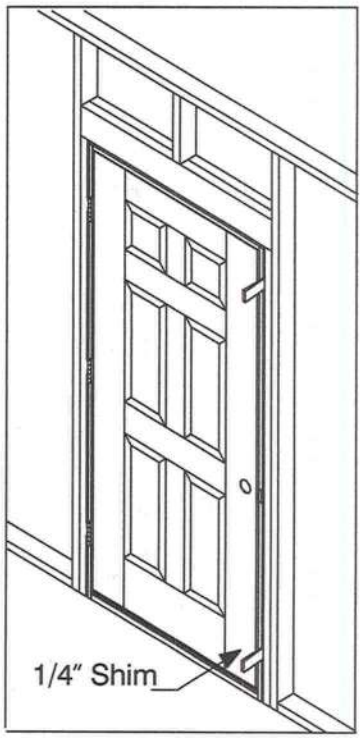
6 ADJUST REST OF FRAME AND FASTEN.

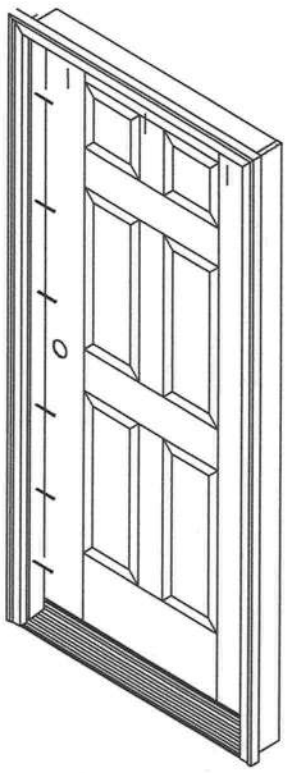
From swing side of door, shim near top and bottom of unfastened jamb.

Make frame adjustments so margins between door and frame are all even.

Adjust frame so frame face is flush with door face all around.

NOTE: For **double door units,** make adjustments that affect alignment and margins and weatherstrip contact *between doors.*





From weatherstrip side of door, check weatherstrip margins and contact.

Adjust frame as required so contact and margins are equal all around door.

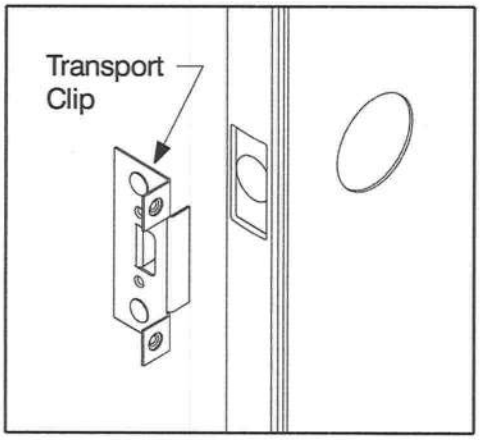
Evenly space six screws per side. Review the figures on page 3.

For single doors, on the head, use **three screws** for each door: 6″ from each end and at the center of the door.

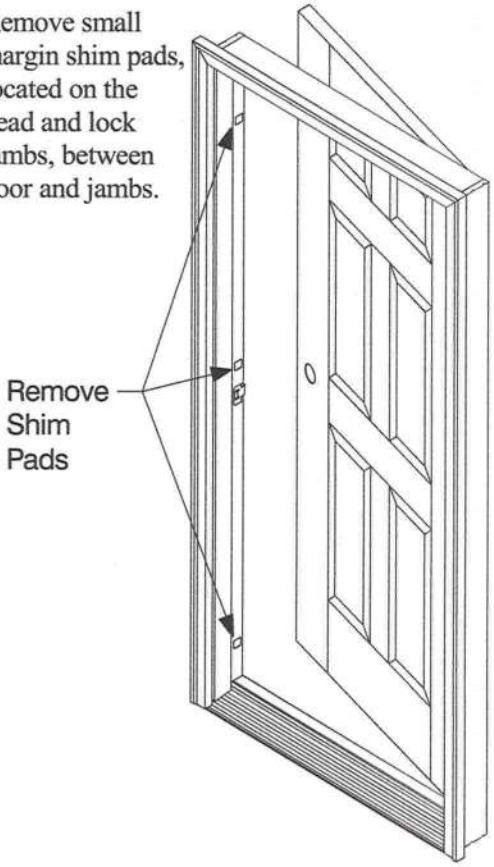
When a sidelite is present, add an additional screw 3″ from the end of each door and use three screws for each sidelite head: 3″ from each end and 6″ from door-side end. Finish driving the

7 *UNCLIP AND OPEN DOOR. INSTALL HARDWARE. COMPLETE JAMB FASTENING.*

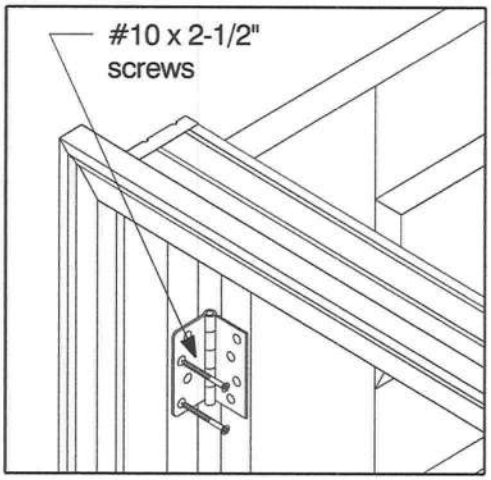
Remove transport clip. Open and close door to ensure smooth operation.



Remove small margin shim pads, located on the head and lock jambs, between door and jambs.



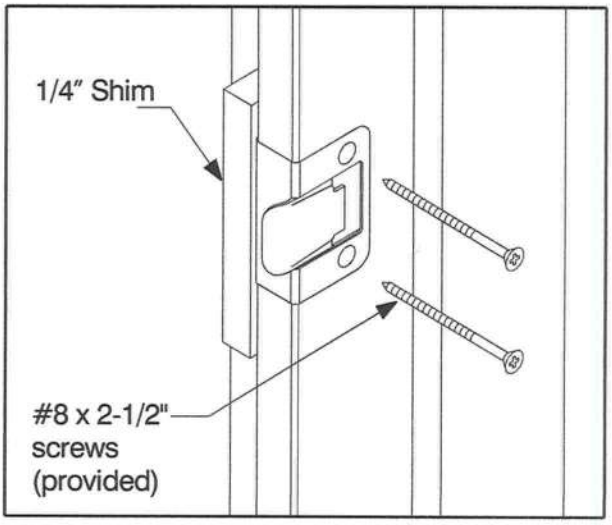
With door open, at hinges where labels indicate, drill 1/8" dia. pilot holes and fasten #10 x 2-1/2" screws (provided) through hinges to anchor door frame and prevent sagging.



Close door. Carefully shim between jamb and opening behind latch area.

Open door and install lockset hardware.

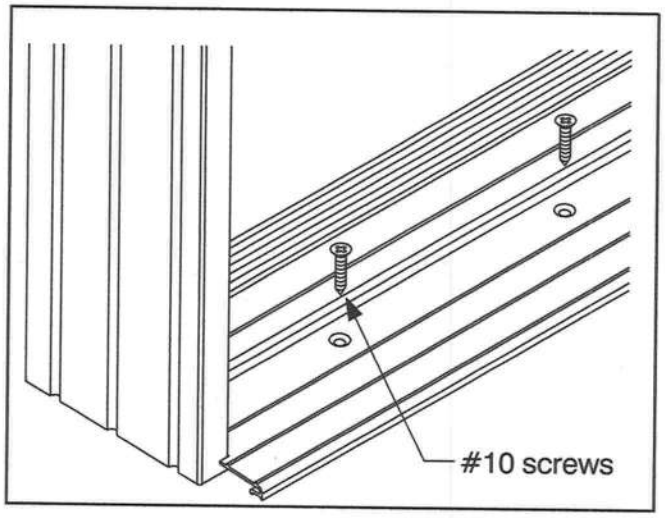
Drill 1/8" dia. pilot holes and place (2) #8 x 2-1/2" screws (provided) through strike mounting holes to secure lock jamb center and provide security. Adjust strike in or out for proper door operation and tighten screws.



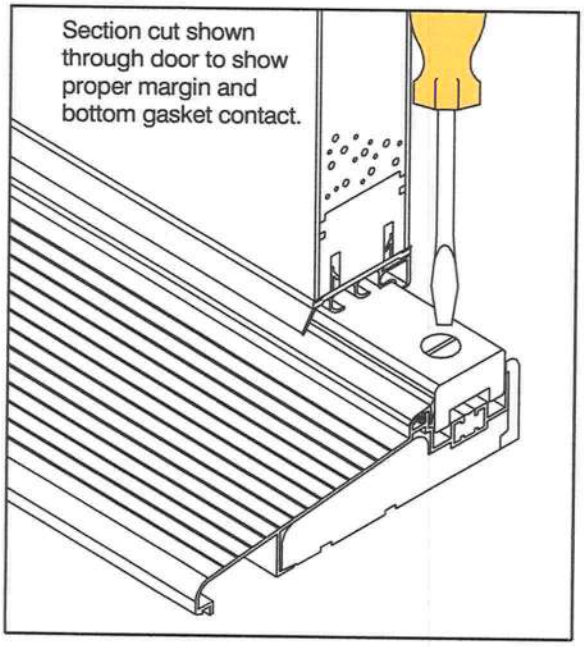
8 ANCHOR SILL, ADJUST SILL.

For sills prepared for anchor screws, place #10 screws through sill into subfloor. If not prepared, pre-drill 1/8″ holes 6″ from each end and at the center of each door. Countersink as needed.

If a sidelite is used, drill an additional pilot hole 3″ from end where the door and sidelight meet.



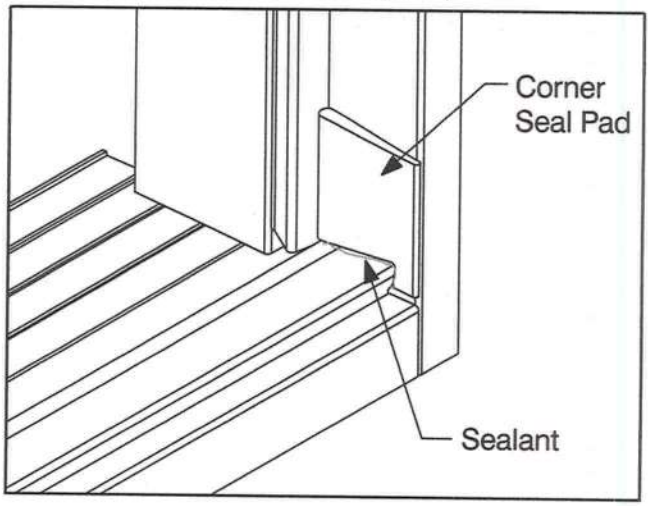
For sill with screw-adjustable thresholds, follow directions on sill to adjust threshold to meet door bottom gasket.



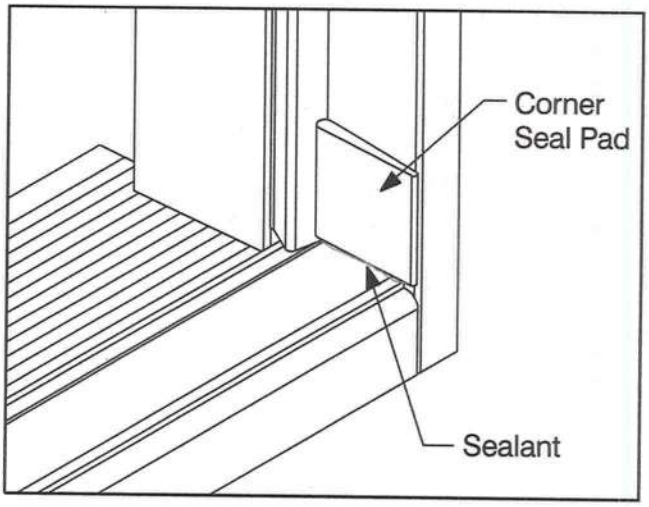
9 CHECK OR INSTALL CORNER SEAL PADS. (SWING-IN DOORS ONLY)

Corner seal pads are an important part of a swing-in door's weatherproofing integrity. Check to see that they are installed properly, in place and with sealant under the pad at the jamb and sill joint.

Self-Adjusting Sills (shop applied)



Adjustable Sills



For sills with adjustable thresholds, pads are sent in a poly bag with unit, not installed. They are installed after final threshold adjustment. Follow directions with pads to install.

PROCEED TO STEP 10.

10 WEATHERPROOF, FINISH, AND MAINTAIN ENTRY DOOR SYSTEM.

Place and set galvanized finish nails through brickmould around perimeter. Cover all countersunk fasteners with an exterior-grade putty.

Caulk around entire unit on "weather" side: seal brickmold to siding or facing, seal front bottom edge of sill, seal all joints between jambs and moldings.

Seal joints between exterior hardware trim and door face to prevent air and water infiltration.

Provide and maintain a properly-installed cap flashing to protect top surfaces from water damage.

Paint or stain according to Therma-Tru instructions. Do not paint gasketing or weatherstripping.

Bare unprotected wood will weather and degrade and change color. All bare wood surfaces exposed to weather must be primed or stained and painted or finished within two weeks of exposure.

Maintain or replace sealants and finishes as soon as any deterioration is evident. For semi-gloss and glossy paints or clearcoats, do this when surface becomes dull or rough. More severe exposures require more frequent maintenance.

Swing-out doors must have all edges - sides, top, and bottom - finished. Inspect and maintain these edges as regularly as all other surfaces.

FINISHING INSTRUCTIONS

TO PAINT A STEEL OR SMOOTH-STAR DOOR:
Clean first with detergent and water. Do not wash doorlite frames and moldings (see below). Rinse and let dry completely. Use only an exterior high-quality 100% acrylic latex paint following manufacturer's directions for application. Paint edges and exposed ends of door.

TO PAINT STEEL DOORLITE FRAMES AND MOLDINGS:
Clean first by lightly wiping with a clean cloth. The material has a factory-applied surface preparation to improve finish adhesion. (Do not use stripping solvents on doorlite frames. This will damage or remove the surface preparation.) Mask off glass, prime first with an alkyd-based primer, then finish with the same paint used for the door.

TO FINISH CLASSIC-CRAFT OR FIBER-CLASSIC DOORLITE FRAMES AND PANEL INSERTS:
Clean first by lightly wiping with a clean cloth. The material has a factory-applied surface preparation to improve finish adhesion. (Do not use stripping solvents on frames. This will damage or remove the surface preparation.) Mask off glass. Stain or paint using the same materials as for the door. (See below. If painting, prime first with an alkyd-based primer.) To balance color when staining, apply stain more lightly on doorlite frames and panels than on door. Topcoat when stain is completely dry.

TO PAINT OR STAIN ALL CLASSIC-CRAFT OR FIBER-CLASSIC DOORS:
Clean first. Use a dry rag or a clean cloth and mineral spirits or detergent and water. Allow to dry before finishing. Edges and exposed ends should also be finished. Only apply finish when the temperature is between 50 and 90 F. with humidity less than 85%. Do not work with the door in direct sunlight. Follow directions above for doorlite frames and panel inserts.

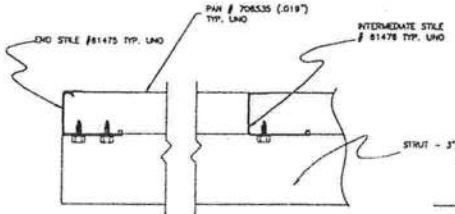
TO PAINT:
Prime first with an alkyd-based primer. Allow the primer to cure completely. Finish with an exterior-grade oil-based or alkyd or 100% acrylic latex paint.

TO STAIN:
For best results, use only the complete Therma-Tru Finishing System. Follow directions enclosed with Finishing System.

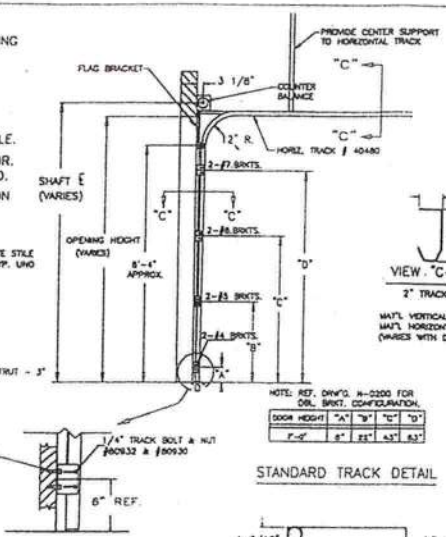
Issued: 3/02
Printed in U.S.A

NOTES:

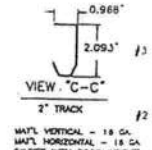
1. 5 STRUTS POSITIONED AS SHOWN WITH 4 THREAD FORMING SCREWS 1/4"-20 X 3/4" AT EACH END STILE & 2 AT INTERMEDIATE STILES PER SECTION "B-B".
2. FOUR 21" SECTIONS FORM THE 7'-0" HIGH DOOR.
3. ALL SECTIONS EMBOSSED 14.50"x20.375"x 25 GA.
4. TORSION SPRINGS SHOWN. EXTENSION SPRINGS AVAILABLE.
5. SLIDE LOCKS MUST BE ATTACHED TO EACH SIDE OF DOOR. CAM & BAR LOCKS OR AN OPENER MAY BE SUBSTITUTED.
6. WHERE THE DRAWING CONFLICTS WITH OTHER INSTALLATION INSTRUCTIONS, THIS DRAWING GOVERNS.



SECTION "B-B" 5/16"x 1 1/2" LAG SCREW #80908 TYP. UNO



DOOR HEIGHT	"A"	"B"	"C"	"D"
7'-0"	8"	22"	43"	83"



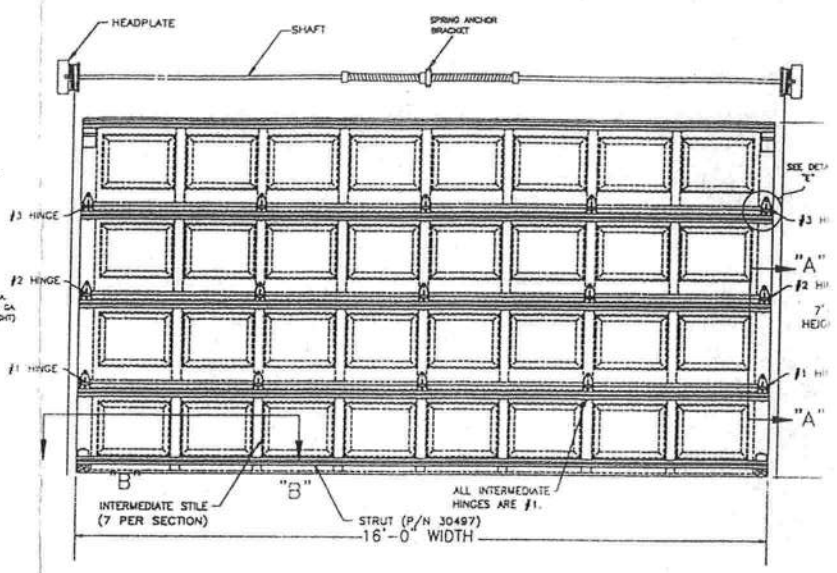
MAT'L VERTICAL - 18 GA.
MAT'L HORIZONTAL - 18 GA.
(VARIES WITH DOOR WEIGHT)

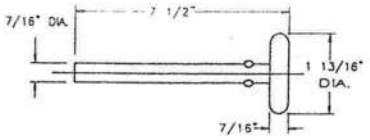
1/4" TRACK BOLT & NUT #80932 & #80930

6" REF.

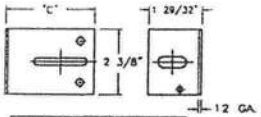
DETAIL "D"

STANDARD TRACK DETAIL



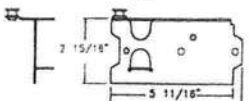


P/N 80441 ROLLER ASSEMBLY



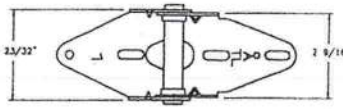
BRACKET #	"C"	PART NUMBER
4	3"	80900
5	3 1/4"	80901
6	3 1/2"	80902

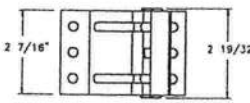
TRACK BRACKET

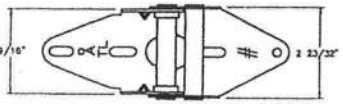


P/N 80262 R + L BOT. FIXTURE

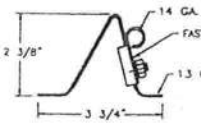
P/N 80298 SCREW
1/4"-20 x 5/8"



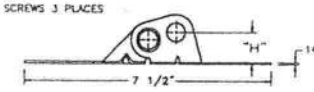




P/N 80428 CTR. HINGE (#1)

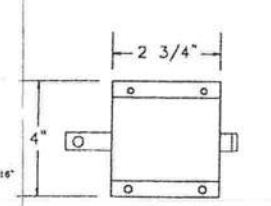


P/N 80448 TOP FIXTURE

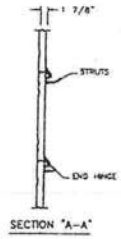


END HINGE
(#2 & #3)

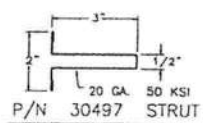
HINGE #	GAGE #	"H"	PART NUMBER
1	16	1"	80428
2	14	1 1/4"	80429
3	14	1 1/2"	80430



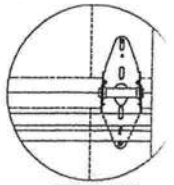
P/N 80100 LOCK



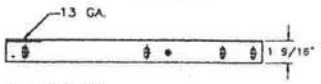
SECTION "A-A"

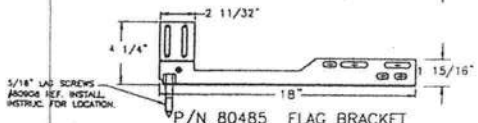


P/N 30497 STRUT



DETAIL "E"





P/N 80485 FLAG BRACKET

PROFESSIONAL ENGINEER'S SEAL PROVIDED ONLY FOR VERIFICATION OF DOOR TESTING TO LOADS (PSF) INDICATED ON THIS DRAWING.

07/17/01

SCALE	N.T.S.	DATE
DRAWN BY	JLG	07-14-98
INITIAL CHK.		"
FINAL CHK.		"
ENGR.		"
APPR. BY		"

Mid-America Door Company POPLAR CITY

DESCRIPTION: 16'x7' GARAGE DOOR + - 30 PSF DESIGN LOAD

TITLE: REGAL/CELEBRITY

DWG. NO. FL-1670110T

SHT. 1 OF 1

Lesters Garage Doors
P.O. Box 763
Gainesville, FL 32602

To calculate; multiply total square feet of door by 30 (p.s.f.)
Example; 16x7 = 112 s.f. x 30 (p.s.f.) =-3360 total load.

DOOR FRAME PLATE CONNECTION SCHEDULE 1.1

WOOD CONSTRUCTION		QUANT. OF NAILS PER JAMB**		
		SYP	DOUG.FIR	SPF
TOTAL DOOR LOAD (LBS.)	LOAD PER JAMB (LBS.)	SPECIFIC GRA.=0.55	SPECIFIC GRA.=0.48	SPECIFIC GRA.=0.42
1000	500	5	7	10
1250	625	7	9	12
1500	750	8	11	15
1750	875	9	13	17
2000	1000	10	14	20
2250	1125	12	16	22
2500	1250	13	18	24
2750	1375	14	20	27
3000	1500	15	21	29
3250	1625	17	23	32
3500	1750	18	25	34
3750	1875	19	26	36
4000	2000	20	28	39
4250	2125	22	30	41
4500	2250	23	32	44
4750	2375	24	33	46
5000	2500	25	35	48

** – BASED ON 16dx 3 1/2" LONG THREADED, HARDENED NAIL ASSUMING 2" MIN. PENETRATION.
1. ANCHORS TO BE EVENLY SPACED BETWEEN THE HEADER AND THE FLOOR.
2. PREDRILL NAIL HOLES TO PREVENT SPLITTING.
3. CHART IS BASED ON 8'-0" MAX. DOOR HEIGHT.

DOOR FRAME PLATE CONNECTION SCHEDULE 1.2

WOOD CONSTRUCTION		QUANT. OF LAG SCREWS PER JAMB**		
		SYP	DOUG.FIR	SPF
TOTAL DOOR LOAD (LBS.)	LOAD PER JAMB (LBS.)	SPECIFIC GRA.=0.55	SPECIFIC GRA.=0.48	SPECIFIC GRA.=0.42
1000	500	3	3	3
1250	625	3	3	3
1500	750	3	3	3
1750	875	3	3	3
2000	1000	3	3	3
2250	1125	3	3	3
2500	1250	3	3	3
2750	1375	3	3	3
3000	1500	3	3	3
3250	1625	3	3	3
3500	1750	3	3	4
3750	1875	3	3	4
4000	2000	3	3	4
4250	2125	3	4	4
4500	2250	3	4	5
4750	2375	3	4	5
5000	2500	4	4	5
5250	2625	4	4	5
5500	2750	4	5	6
5750	2875	4	5	6
6000	3000	4	5	6
6250	3125	4	5	6
6500	3250	4	5	6
6750	3375	5	6	7
7000	3500	5	6	7
7250	3625	5	6	7
7500	3750	5	6	7

** – BASED ON 3/8"ø LAG SCREW ASSUMING 2" MIN. PENETRATION SEASONED WOOD, DRY USE.
1. ANCHORS TO BE EVENLY SPACED BETWEEN THE HEADER AND THE FLOOR.
2. CHART IS BASED ON 8'-0" MAX. DOOR HEIGHT.

DOOR FRAME PLATE CONNECTION SCHEDULE 2.1

MASONRY/CONCRETE CONSTRUCTION		QUANTITY OF TAPCON ANCHORS PER JAMB**	
		HOLLOW BLOCK	3,000 PSI CONCRETE
TOTAL DOOR LOAD (LBS.)	LOAD PER JAMB (LBS.)		
1000	500	3	3
1250	625	3	3
1500	750	3	3
1750	875	4	3
2000	1000	4	3
2250	1125	5	3
2500	1250	5	3
2750	1375	6	4
3000	1500	6	4
3250	1625	6	4
3500	1750	7	5
3750	1875	7	5
4000	2000	8	5
4250	2125	8	6
4500	2250	9	6
4750	2375	9	6
5000	2500	10	6
5250	2625	10	7
5500	2750	11	7
5750	2875	11	7
6000	3000	11	8
6250	3125	12	8
6500	3250	12	8
6750	3375	13	9
7000	3500	13	9
7250	3625	14	9
7500	3750	14	9

**– BASED ON 1/4"ø TAPCON ANCHOR WITH A 1 1/2" MIN. EMBEDMENT.
1. ANCHORS TO BE EVENLY SPACED BETWEEN THE HEADER AND THE FLOOR.
2. 1/4" WASHERS REQUIRED WITH EACH TAPCON ANCHOR.
3. CHART IS BASED ON 8'-0" MAX. DOOR HEIGHT.

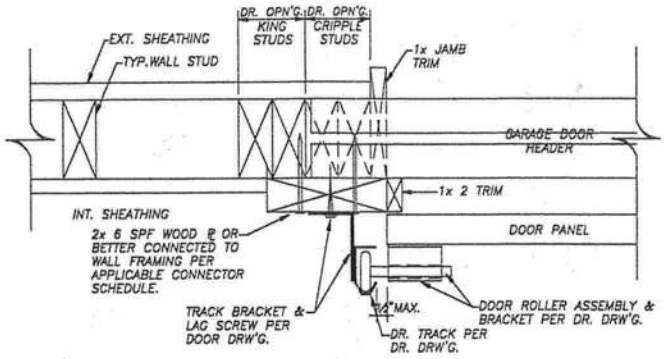
DOOR FRAME PLATE CONNECTION SCHEDULE 3.1

MASONRY/CONCRETE CONSTRUCTION		QUANTITY OF EXPANSION ANCHORS PER JAMB**		
		GROUT-FILLED BLOCK	2,000 PSI CONCRTE	3,000 PSI CONCRTE
TOTAL DOOR LOAD (LBS.)	LOAD PER JAMB (LBS.)			
1000	500	3	3	3
1250	625	3	3	3
1500	750	3	3	3
1750	875	3	3	3
2000	1000	3	3	3
2250	1125	3	3	3
2500	1250	3	3	3
2750	1375	3	3	3
3000	1500	3	3	3
3250	1625	3	3	3
3500	1750	4	3	3
3750	1875	4	3	3
4000	2000	4	3	3
4250	2125	4	3	3
4500	2250	4	3	3
4750	2375	5	3	3
5000	2500	5	3	3
5250	2625	5	3	3
5500	2750	5	3	3
5750	2875	6	3	3
6000	3000	6	3	3
6250	3125	6	3	3
6500	3250	6	3	3
6750	3375	6	3	3
7000	3500	7	4	3
7250	3625	7	4	4
7500	3750	7	4	4

**– BASED ON 3/8"ø HILTI STUD TYPE EXPANSION ANCHOR WITH A 2 1/2" MIN. EMBEDMENT.
1. ANCHORS TO BE EVENLY SPACED BETWEEN THE HEADER AND THE FLOOR.
2. MINIMUM DISTANCE FROM CONCRETE EDGE TO ANCHOR SHALL BE 3 3/4".
3. CHART IS BASED ON 8'-0" MAX. DOOR HEIGHT.

NOTES

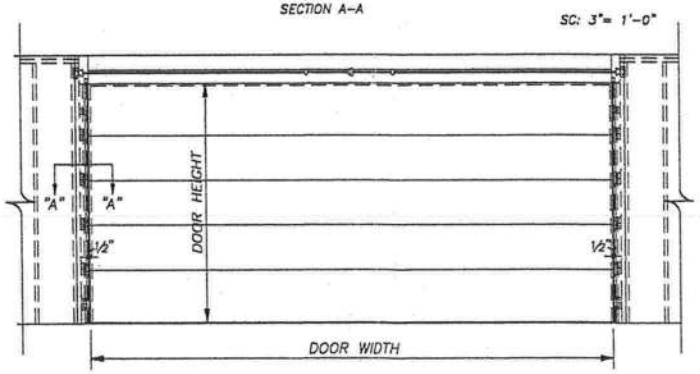
1. WIND LOAD PERFORMANCE LABEL MUST BE AFFIXED TO THE INSIDE OF THE DOOR IN A LOCATION THAT IS EASILY SEEN.
2. WHERE THIS DRAWING CONFLICTS WITH OTHER INSTALLATION INSTRUCTIONS, THIS DRAWING GOVERNS.
3. THE TESTING METHODS FOR THE DOORS PERTAINING TO DRW'S. LISTED HEREIN WERE IN ACCORDANCE WITH DASMA 108, SBC 1999 & ASCE 7-98.



JAMB FASTENER DETAIL

SECTION A-A

SC: 3"= 1'-0"



ELEVATION NOTES:
1. SPACE BETWEEN INSIDE OF VERTICAL TRACK AND END OF DOOR IS 1/2" MAX.

DOOR ELEVATION

(INTERIOR ELEVATION)

SCALE: 1/4"= 1'-0"

02/22/02

SCALE	N.T.S.	DATE	
DRAWN BY	JAR	02-22-02	
INITAIL CHK.			
FINAL CHK.			
ENGR.			
APPR. BY			

Mid-America Door Company PONCA CITY, OK

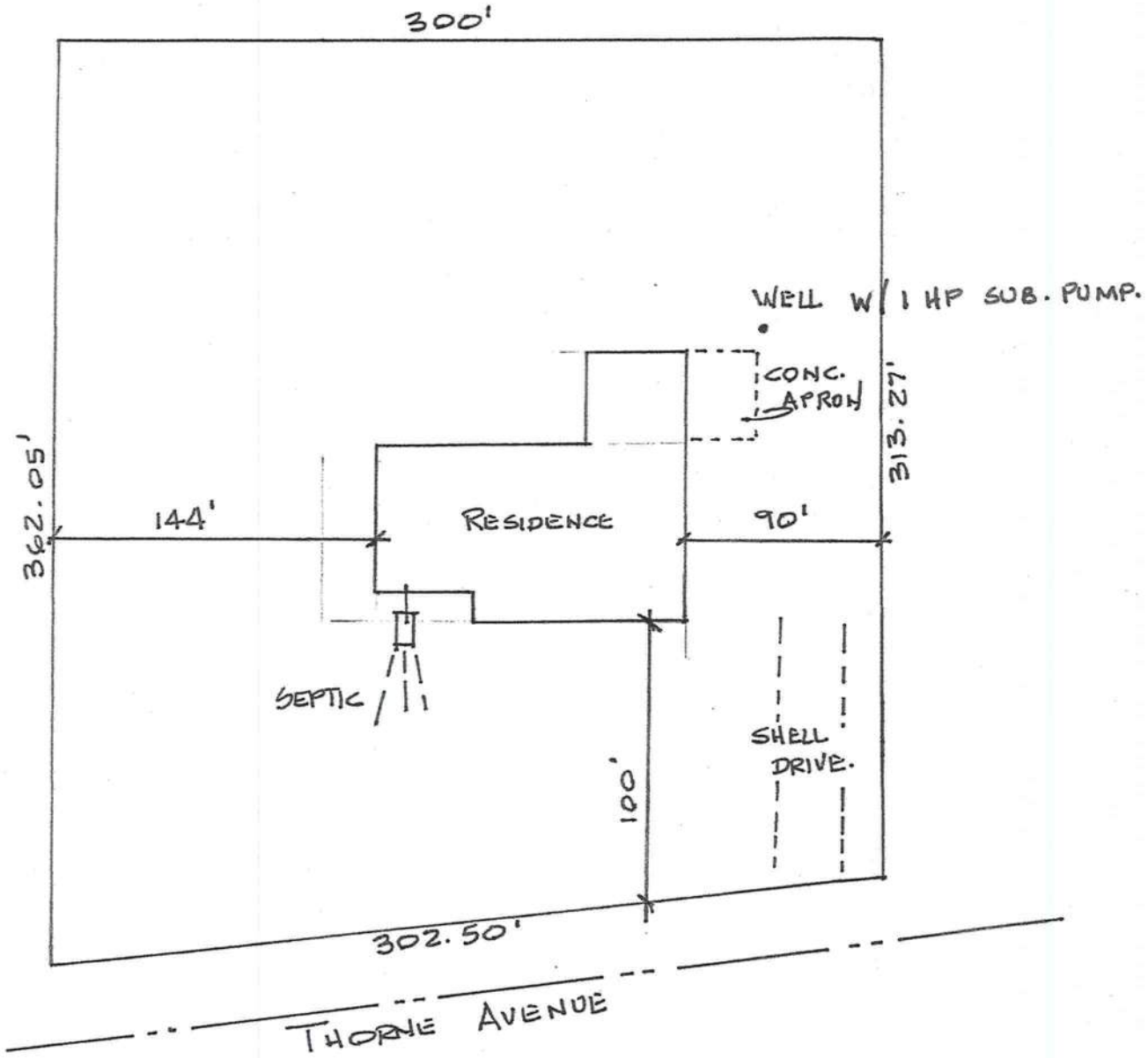
DESCRIPTION: INSTALLATION ADDENDUM FOR WIND LOAD DOORS

TITLE

DWG. NO. FL-0101

SHT. 1 OF 2

REV. -



LOT #10 - SANTA FE
RIVER PLANTATIONS
COLUMBIA COUNTY, FL.

ALLEN BATES
GENESIS
DESIGN & CONSTRUCTION, INC.
(386) 462-2884

attn: WEEBSITE

Columbia County Building Department
Culvert Waiver

Culvert Waiver No. 000000862

DATE: 10/25/2005 BUILDING PERMIT NO. 23762

APPLICANT ALLEN K. BATES PHONE 386.462.2884

ADDRESS 481 TURKEY CREEK ALACHUA FL 32615

OWNER ALICE Y.GREEN PHONE

ADDRESS 265 SW THORNE LANE FT. WHITE FL 32038

CONTRACTOR ALLEN K. BATES PHONE 386.462.2884

LOCATION OF PROPERTY 47-S TO C-138 TO MAPLETON,TL TO HEFLIN,TR TO THORNE,TL AND THE JOB SITE IS ON THE R.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT SANTA FE RIV. PLANT 10

PARCEL ID # 30-7S-17-10058-110

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE:

A SEPARATE CHECK IS REQUIRED
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

PUBLIC WORKS DEPARTMENT USE ONLY

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE CULVERT WAIVER IS:

✓ APPROVED ______ NOT APPROVED - NEEDS A CULVERT PERMIT

COMMENTS:

SIGNED: DATE: 10/27/05

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

COLUMBIA COUNTY
OCT 2 8 2005
PUBLIC WORKS DEPT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

COLUMBIA COUNTY · GATEWAY TO FLORIDA

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 30-7S-17-10058-110 **Building permit No.** 000023762

Use Classification SFD/UTILITY **Fire:** 17.76

Permit Holder ALLEN K. BATES **Waste:** 36.75

Owner of Building ALICE Y. GREEN **Total:** 54.51

Location: 265 SW THORNE LN, FORT WHITE, FL 32038

Date: 07/27/2006

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

ARROW EXTERMINATORS
TERMITE AND PEST CONTROL

5602 N.W. 13th STREET
GAINESVILLE, FLORIDA 32653-2198

P.O. BOX 5875
GAINESVILLE, FLORIDA 32627-5875

PHONE (352) 373-3642
FAX (352) 373-9037

#23762

CERTIFICATE OF PROTECTIVE TREATMENT

Builder: Genesis Design

Date: 1-25-06 Time: _____ AM _____ PM

Site Location: 265 SW Thorne Ft White

Area Treated: Living Porch Garage

Product Used: Bifen IT Chemical Used: Bifenthrin

% Concentration: .06% # Gallons Used: 700

Applicator: Jerry