ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V4Z487-Z0125164634

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/25/2014

Truss Fabricator	Anderson Truss Company
Job Identification	14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL
Truss Count	36
Model Code	Florida Building Code 2010
Truss Criteria	FBC2010Res/TPI-2007(STD)
Engineering Software	Alpine Software,Versions 12.03, 13.02.
Structural Engineer of Record	The identity of the structural EOR did not exist as of
Address	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Details: BRCLBSUB-PB16010-

Douglas Fleming
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

#	Ref	Description	Drawing#	Date
1	20881	A 13'9"8 Common	14084001	03/25/14
2	20882	A1 13'9"8 Common	14084001	03/25/14
3	20883	A2 36'3"8 Mono H	14084031	03/25/14
4	20884	A3 36'3"8 Mono H	14084002	03/25/14
5	20885	A4 36'3"8 Specia	14084003	03/25/14
6	20886	A5 36'3"8 Mono H	14084004	03/25/14
7	20887	A6 36'3"8 Mono H	14084005	03/25/14
8	20888	A7 36'3"8 Mono H	14084006	03/25/14
9	20889	A8 36'3"8 Mono H	14084007	03/25/14
10	20890	A9 36'3"8 Mono H	14084012	03/25/14
11	20891	B 11'8" Common	14084008	03/25/14
12	20892	B1 11'8" Common	14084002	03/25/14
13	20893	B2 11'8" Mono	14084009	03/25/14
14	20894	B3 12'6"8 Mono	14084010	03/25/14
15	20895	B4 12'6"8 Mono	14084034	03/25/14
16	20896	B5 12'6"8 Mono	14084013	03/25/14
17	20897	B6 12'6"8 Mono	14084014	03/25/14
18	20898	B7 12'6"8 Mono	14084015	03/25/14
19	20899	B8 12'6"8 Mono	14084016	03/25/14
20	20900	B9 12'6"8 Mono	14084017	03/25/14
21	20901	B10 12'6"8 Mono	14084018	03/25/14
22	20902	CJ1 1'0"15 Jack	14084019	03/25/14
23	20903	CJ2A 1'4"15 Jack	14084020	03/25/14
24	20904	CJ3 3'0"15 Jack	14084021	03/25/14
25	20905	CJ3A 2'10"15 Jac	14084022	03/25/14
26	20906	DGA 13'9"8 Gable	14084023	03/25/14
27	20907	DGB 11'8" Gable	14084024	03/25/14
28	20908	EJ5 5'2" End Jac	14084025	03/25/14
29	20909	EJ5A 5' End Jack	14084026	03/25/14
30	20910	HJ5 7'2"15 Hip J	14084032	03/25/14
31	20911	HJ5A 7'0"2 Hip J	14084033	03/25/14
32	20912	MH5 12'6"8 Mono	14084011	03/25/14
33	20913	MH7 12'6"8 Mono	14084027	03/25/14
34	20914	MH9 12'6"8 Mono	14084028	03/25/14
35	20915	MH11 12'6"8 Mono	14084029	03/25/14
36	20916	PBG1 18'8"5 Comm	14084030	03/25/14

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
 Document ID 1V4Z487-Z0125164634

Truss Fabricator	**Anderson Truss Company**
Job Identification	**14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL**
Truss Count	**1**
Model Code	**Florida Building Code 2010**
Truss Criteria	**FBC2010Res/TPI-2007(STD)**
Engineering Software	**Alpine Software,Versions 12.03, 13.02.**
Structural Engineer of Record	
Address	
Minimum Design Loads	**Roof - 37.0 PSF @ 1.25 Duration** **Floor - N/A** **Wind - 120 MPH ASCE 7-10 -Closed**

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/25/2014

Notes

1. **Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1**
2. **The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.**
3. **As shown on attached drawings; the drawing number is preceded by: HCUSR9114**

-Truss Design Engineer-
Douglas Fleming

1950 Marley Drive
Haines City, FL 33844

Revised Trusses

#	Ref	Description	Drawing#	Date
1	20903-CJ2A	1.4"15 Jack	14084020	03/25/14

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - A 13'9"8 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

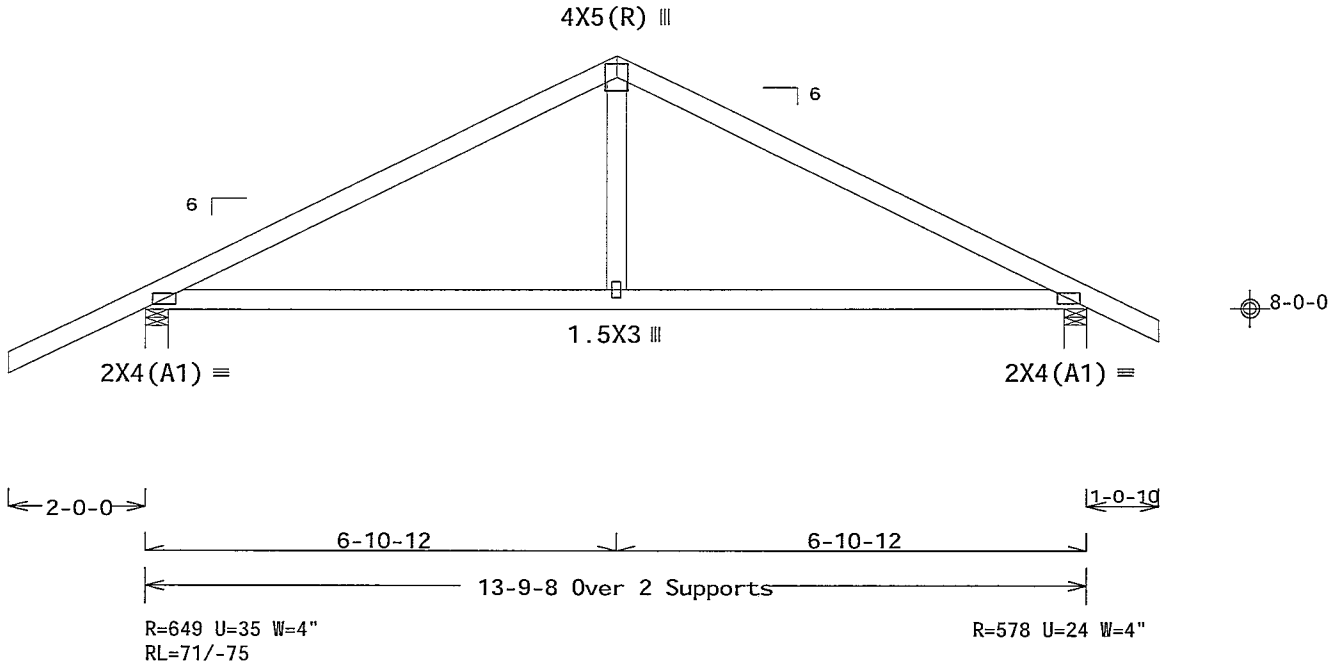
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.0326 14 QTY:1 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating handling sh pping installing and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices pr or to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipping installation bracing of trusses Apply plates to each face of truss and pos tion as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitab lity and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20881
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084001
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361286
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - A1 13'9"8 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

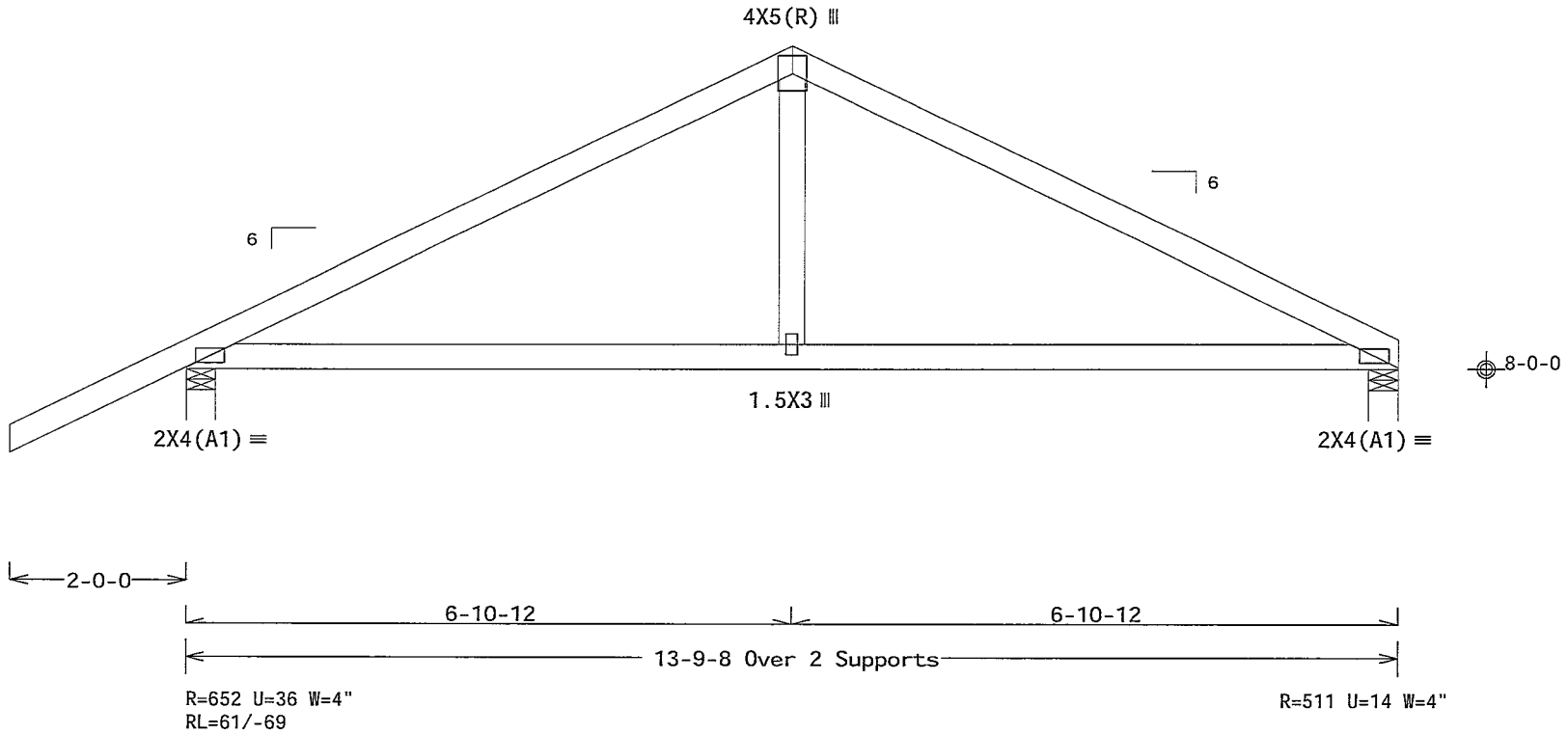
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5.0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.0 QTY:4 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS B. LEMINS LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/25/2014

TC LL	20.0 PSF	REF	R9114- 20882
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084001
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	360252
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - A2 36'3"8 Mono Hip Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B ·B2 2x4 SP 2850f-2 3E·
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

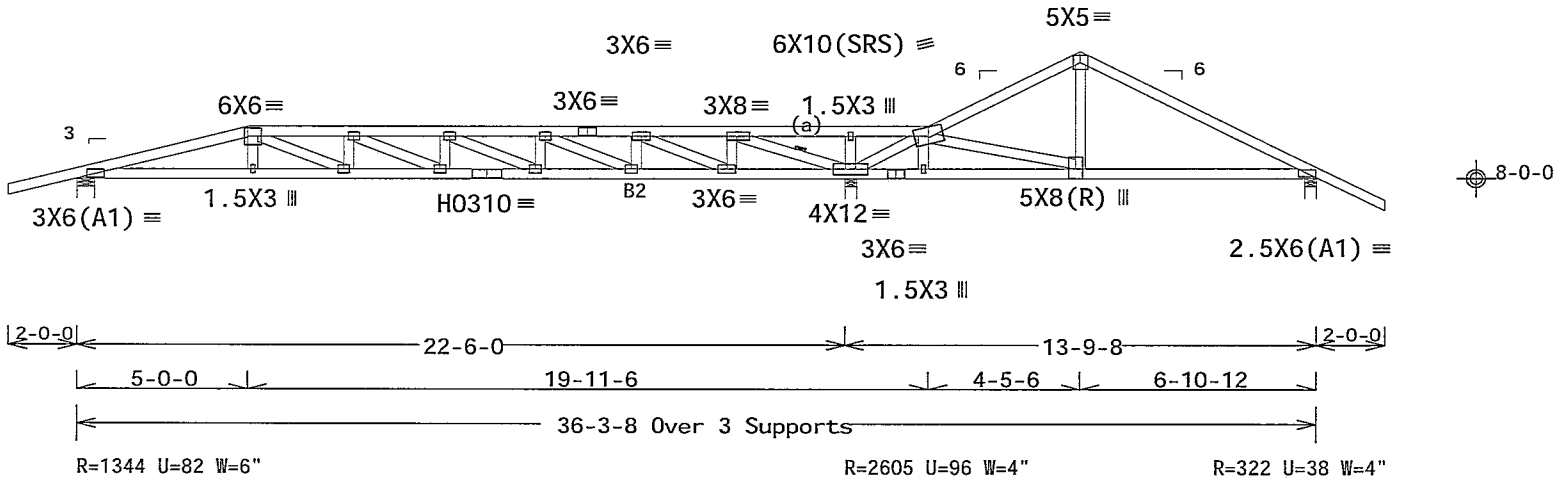
(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Special loads
------(Lumber Dur Fac =1.25 / Plate Dur Fac =1 25)
TC- From 54 plf at -2 00 to 54 plf at 5 00
TC- From 27 plf at 5.00 to 27 plf at 14 95
TC- From 27 plf at 14 95 to 27 plf at 21 06
TC- From 54 plf at 21 06 to 54 plf at 24 95
TC- From 56 plf at 24 95 to 56 plf at 29 40
TC- From 56 plf at 29 40 to 56 plf at 38 29
BC- From 4 plf at -2 00 to 4 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 5 03
BC- From 10 plf at 5 03 to 10 plf at 12 00
BC- From 10 plf at 12 00 to 10 plf at 21 06
BC- From 20 plf at 21 06 to 20 plf at 24 00
BC- From 20 plf at 24 00 to 20 plf at 36 29
BC- From 4 plf at 36 29 to 4 plf at 38 29
TC- 169 95 lb Conc Load at 5 03
TC- 106 82 lb Conc. Load at 7 06, 9 06,11.06,13 06 15 06,17 06,19 06,21 06
BC- 190 54 lb Conc Load at 5 03
BC- 79 47 lb Conc. Load at 7 06, 9 06,11 06,13 06 15 06,17 06,19 06,21.06



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03.04.0328 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this desi any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipping installation bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing ndicates acceptance of professional engineering responsibil ty solely for the design shown The suitability and use of this design for any structure is the responsibility of the Bu lding Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20883
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084031
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361305
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

03/25/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - A3 36'3"8 Mono Hip)

Top chord 2x4 SP #1-13B ·T1 2x4 SP M-30
Bot chord 2x4 SP #1-13B B2 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member.

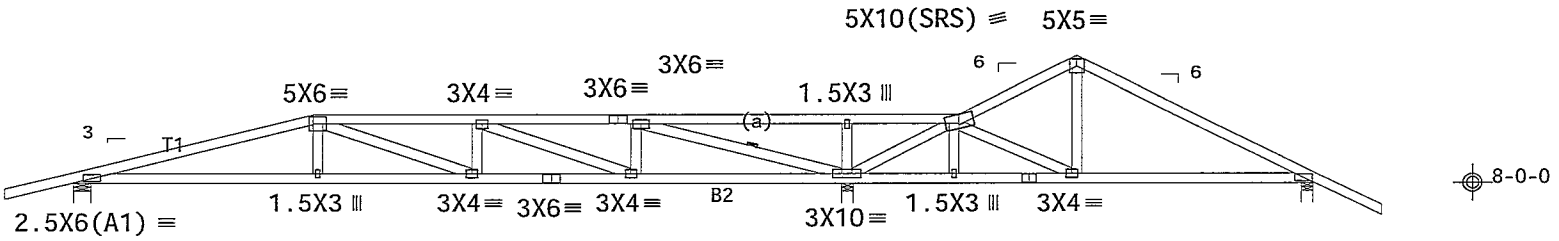
Bottom chord checked for 10 00 psf non-concurrent live load.

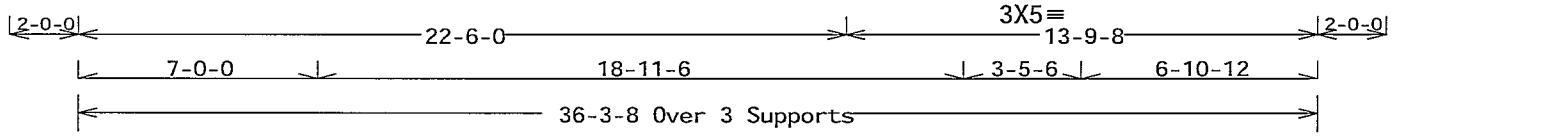
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf. GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=855 U=52 W=6"
RL=71/-56

R=1663 U=67 W=4"

R=540 U=28 W=4"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04-0906.14 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20884
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084002
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361309
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - A4 36'3"8 Special)

Top chord 2x4 SP #1-13B ·T1 2x4 SP 2850f-2 3E
Bot chord 2x4 SP #1-13B B1 2x4 SP M-30
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

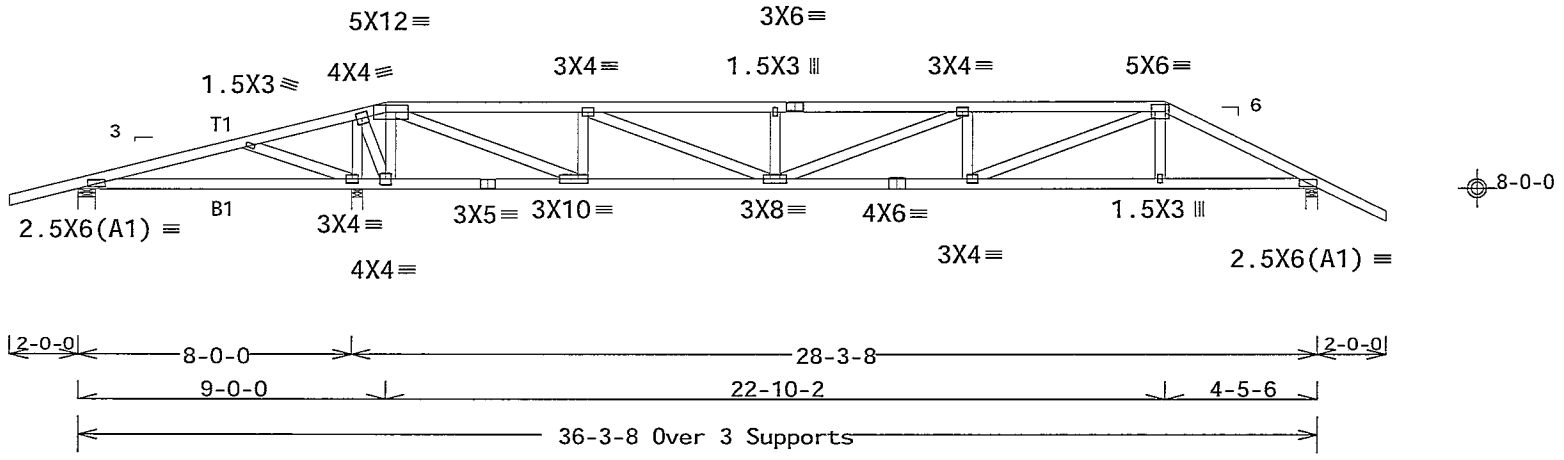
MWFRS loads based on trusses located at least 7 50 ft from roof edge.

Negative reaction(s) of -270# MAX (See below) from a non-wind load case requires uplift connection

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5 0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04-0006.14 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
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Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20885
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084003
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361313
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

03/25/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - A5 36'3"8 Mono Hip)

Top chord 2x4 SP #1-13B T1 2x4 SP M-30
Bot chord 2x4 SP #1-13B B1 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

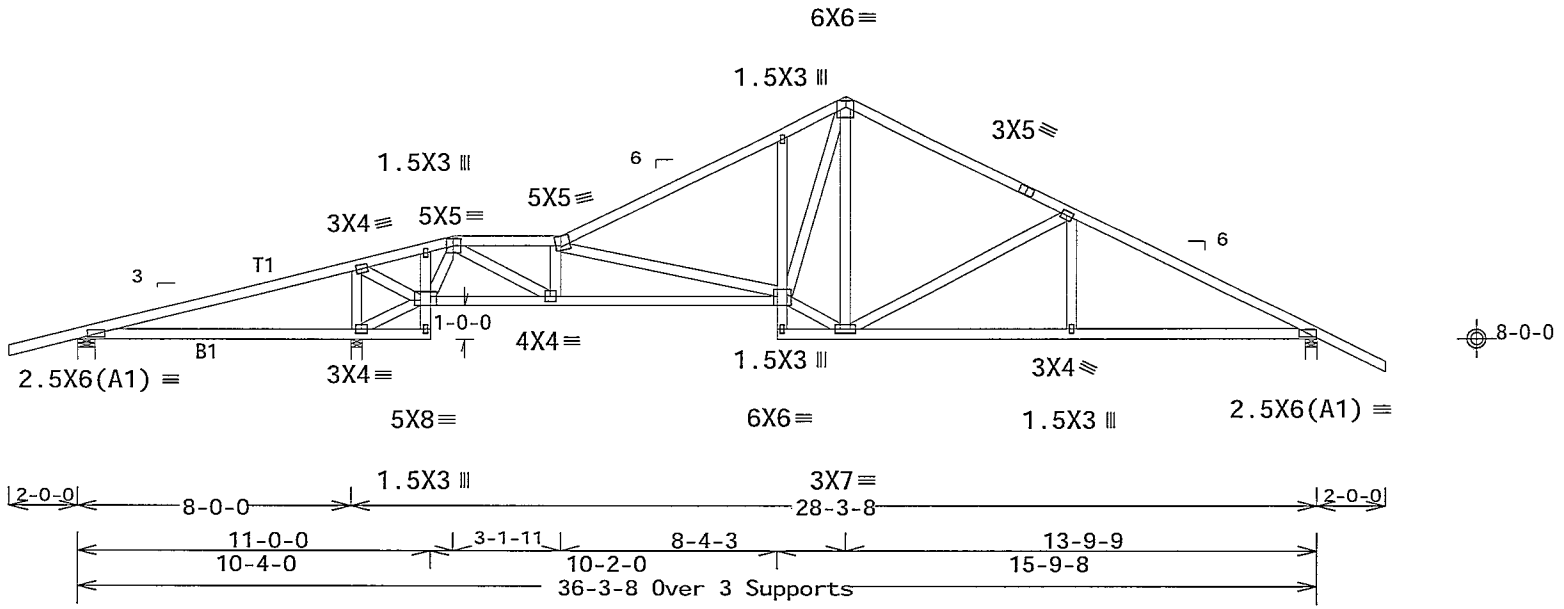
MWFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.14 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
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Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20886
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084004
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361364
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

03/25/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - A6 36'3"8 Mono Hip)

Top chord 2x4 SP #1-13B .T1 2x4 SP 2850f-2 3E·
Bot chord 2x4 SP #1-13B ·B1 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

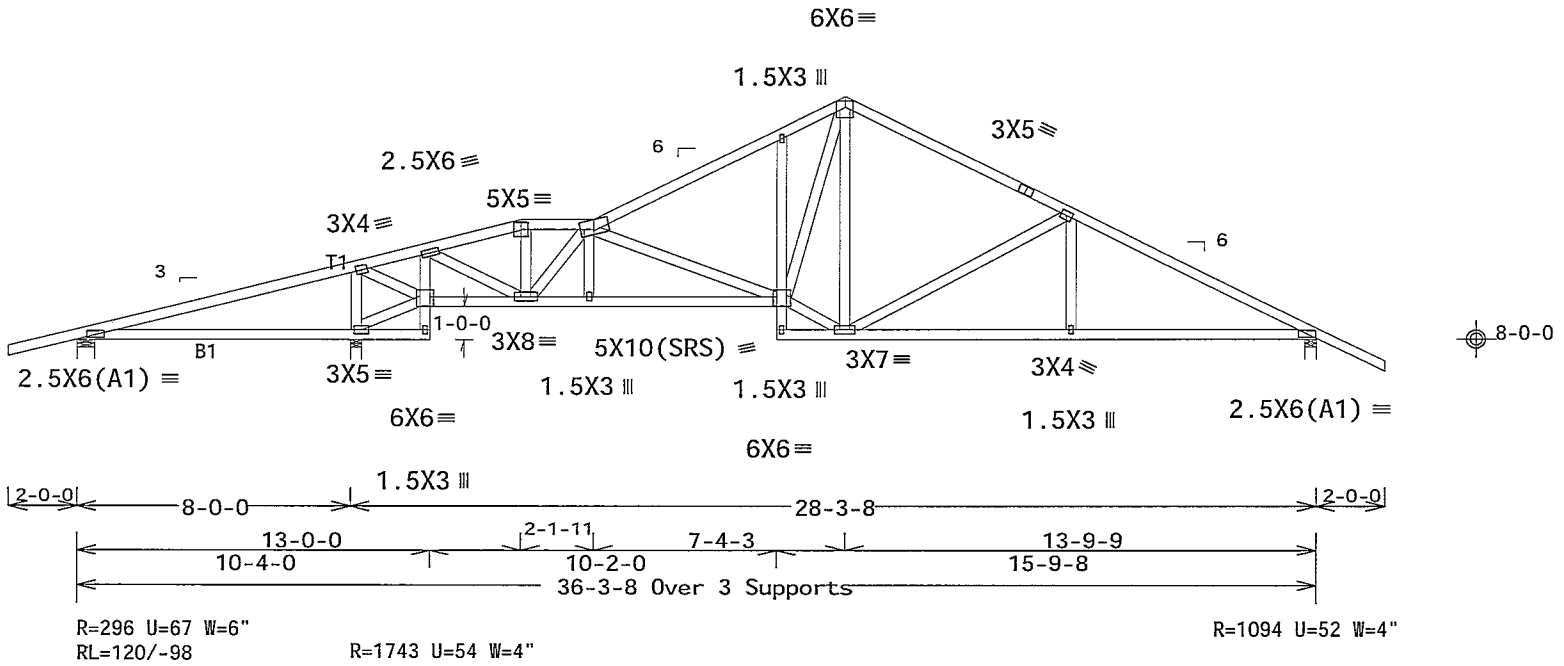
MWFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1 50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.0826.14 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20887
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084005
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361370
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - A7 36'3"8 Mono Hip)

Top chord 2x4 SP #1-13B -T1, T2 2x4 SP 2850f-2 3E
Bot chord 2x4 SP #1-13B B1 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

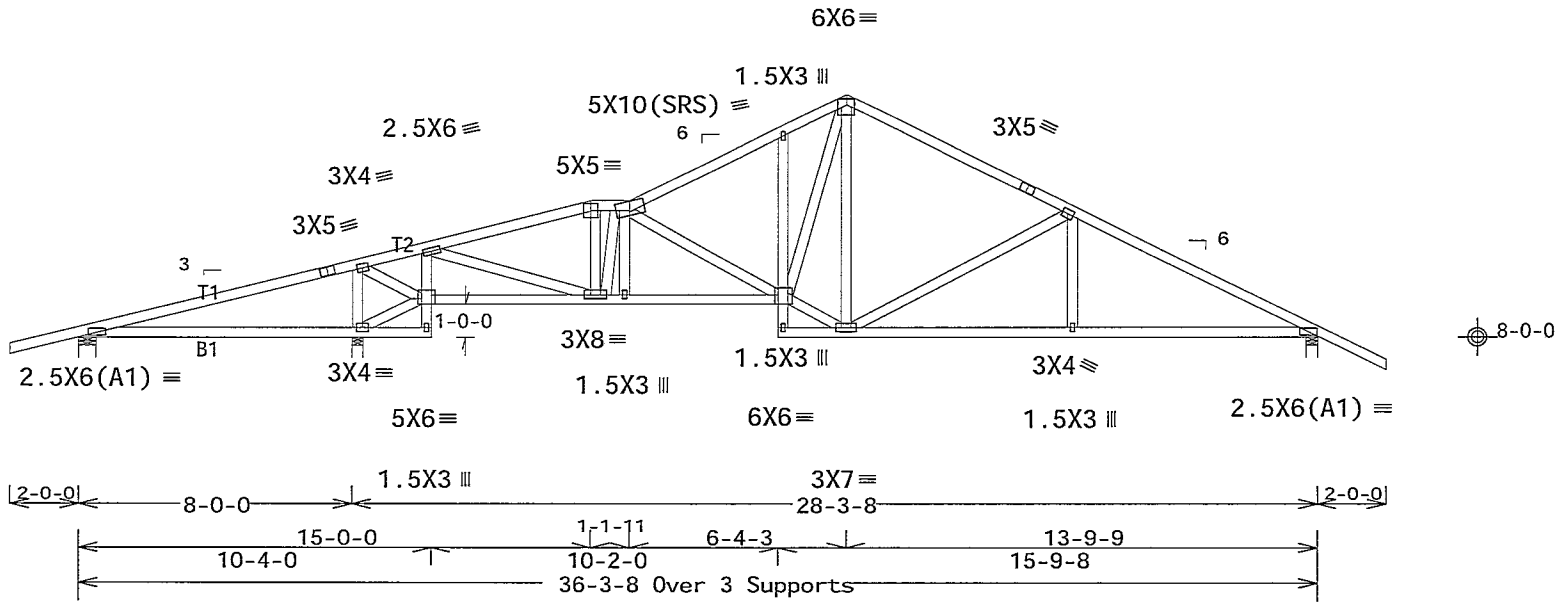
MWFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5.0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.14 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20888
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084006
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361380
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - A8 36'3"8 Mono Hip)

Top chord 2x4 SP #1-13B ·T1, T2 2x4 SP M-30
Bot chord 2x4 SP #1-13B B1 2x4 SP 2850f-2 3E·
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

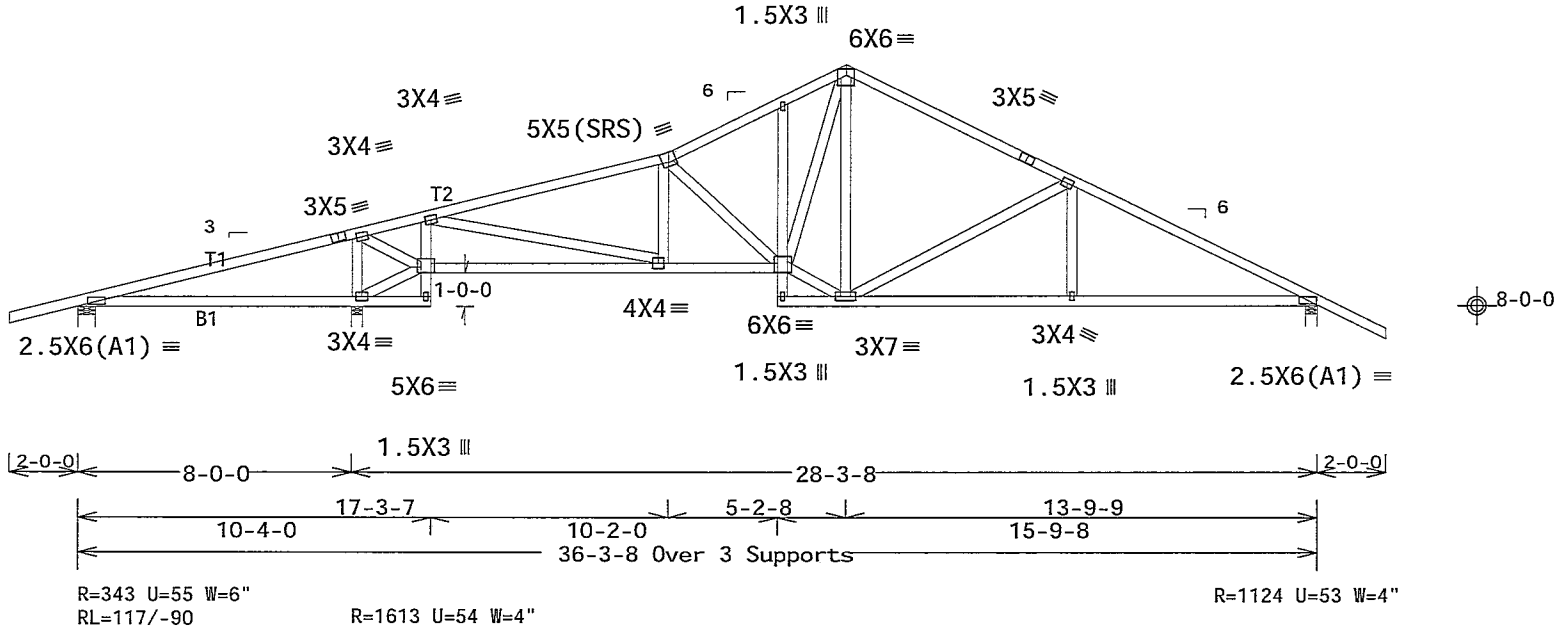
Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5.0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 C320 14 QTY:2 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20889
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084007
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361394
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - A9 36'3"8 Mono Hip Girder)

Top chord 2x4 SP #1-13B .T1, T6 2x4 SP 2850f-2 3E
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B ·W2 2x4 SP #2-13B
·Rt Wedge 2x4 SP #3-13B·

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Negative reaction(s) of -370# MAX (See below) from a non-wind load case requires uplift connection

(++) - This plate works for both joints covered.

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

(a) Continuous lateral restraint equally spaced on member

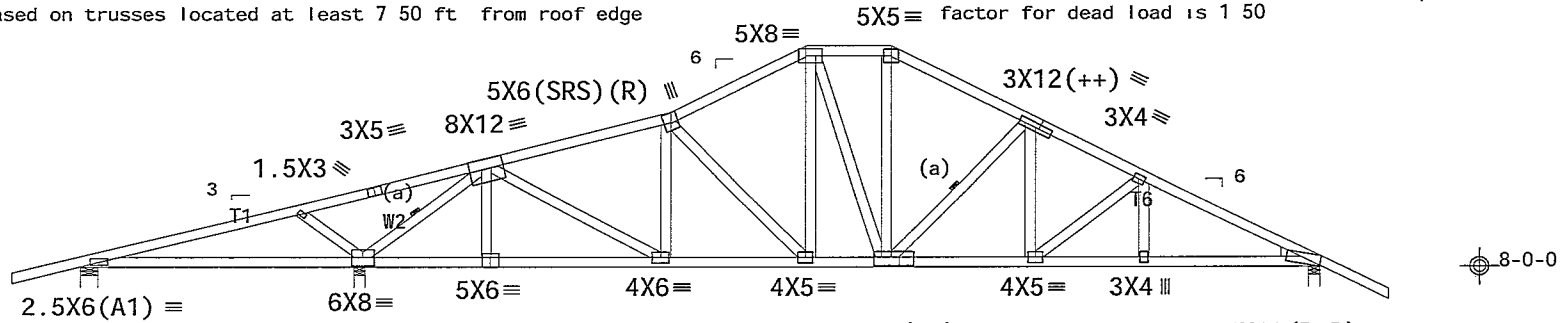
In lieu of structural panels use purlins to brace all flat TC @ 24" OC

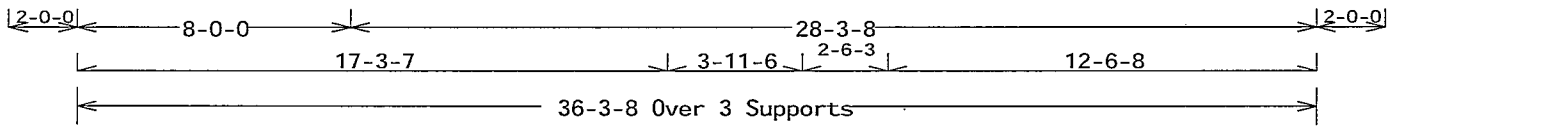
MWFRS loads based on trusses located at least 7 50 ft from roof edge

Special loads
------(Lumber Dur Fac =1.25 / Plate Dur Fac.=1 25)
TC- From 54 plf at -2 00 to 54 plf at 8 57
TC- From 27 plf at 8.57 to 27 plf at 17 29
TC- From 28 plf at 17.29 to 28 plf at 21 24
TC- From 27 plf at 21 24 to 27 plf at 23 75
TC- From 28 plf at 23.75 to 28 plf at 28 20
TC- From 28 plf at 28 20 to 28 plf at 31 06
TC- From 56 plf at 31.06 to 56 plf at 38 29
BC- From 4 plf at -2 00 to 4 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 9 06
BC- From 10 plf at 9 06 to 10 plf at 12 00
BC- From 10 plf at 12 00 to 10 plf at 24 00
BC- From 10 plf at 24 00 to 10 plf at 31 06
BC- From 20 plf at 31 06 to 20 plf at 36 29
BC- From 4 plf at 36 29 to 4 plf at 38 29
BC- 457 71 lb Conc Load at 9 06,11 06,13 06,15 06 17 06,19 06,23 06,25 06,27 06,29 06
BC- 458 75 lb Conc Load at 21 06
BC- 863 47 lb Conc. Load at 31 06

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase 5X5≡ factor for dead load is 1 50





R=148/-370 U=47 W=6"
RL=108/-80
R=4824 U=168 W=4"
R=3330 U=169 W=4"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03.04 C32 14 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20890
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084012
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361433
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

03/25/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - B 11'8" Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

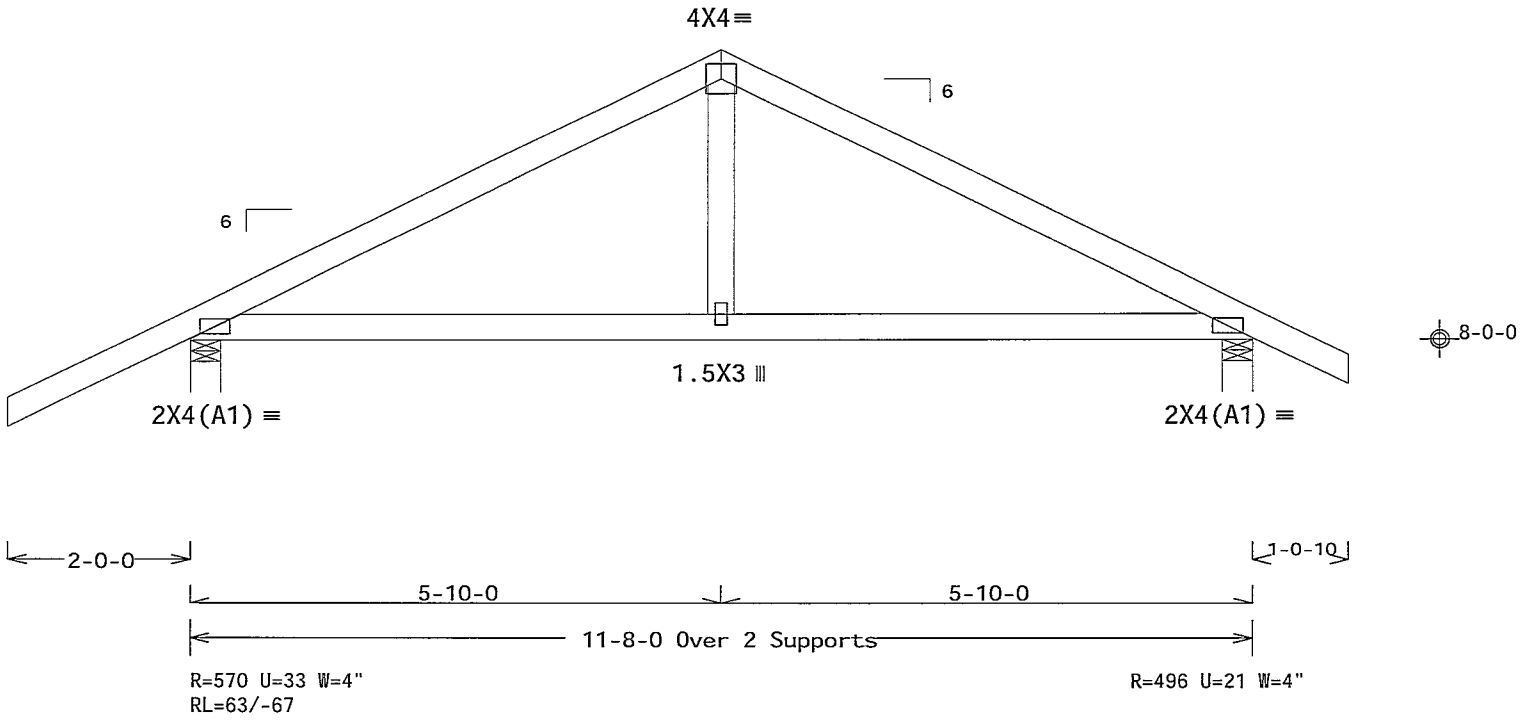
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 USE 14 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20891
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084008
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD	37.0 PSF	SEQN-	361268
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - B1 11'8" Common)

Top chord 2x4 SP 2850f-2 3E T2 2x4 SP M-30
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

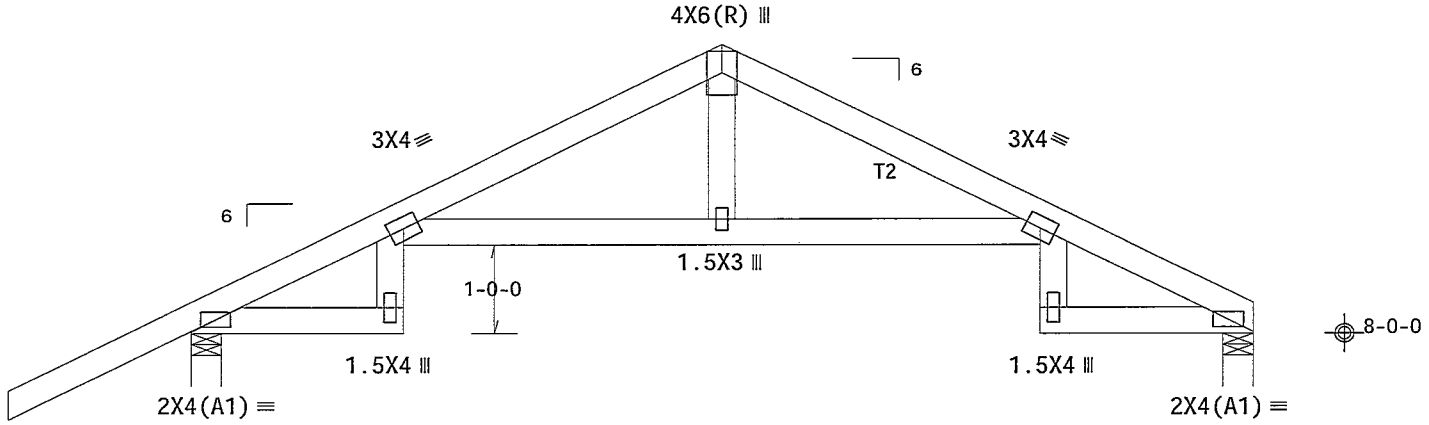
Calculated horizontal deflection is 0 28" due to live load and 0 35" due to dead load

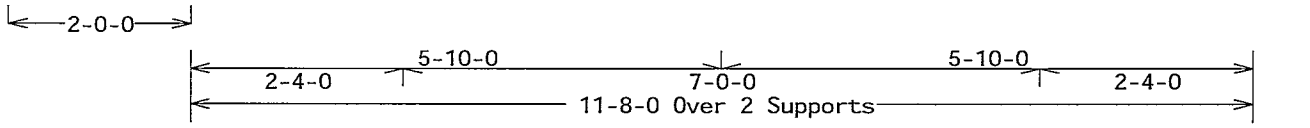
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=573 U=33 W=4"
RL=54/-61

R=429 U=11 W=4"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 MSC 14 QTY:3 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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TC LL	20.0 PSF	REF	R9114- 20892
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084002
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	360260
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - B2 11'8" Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

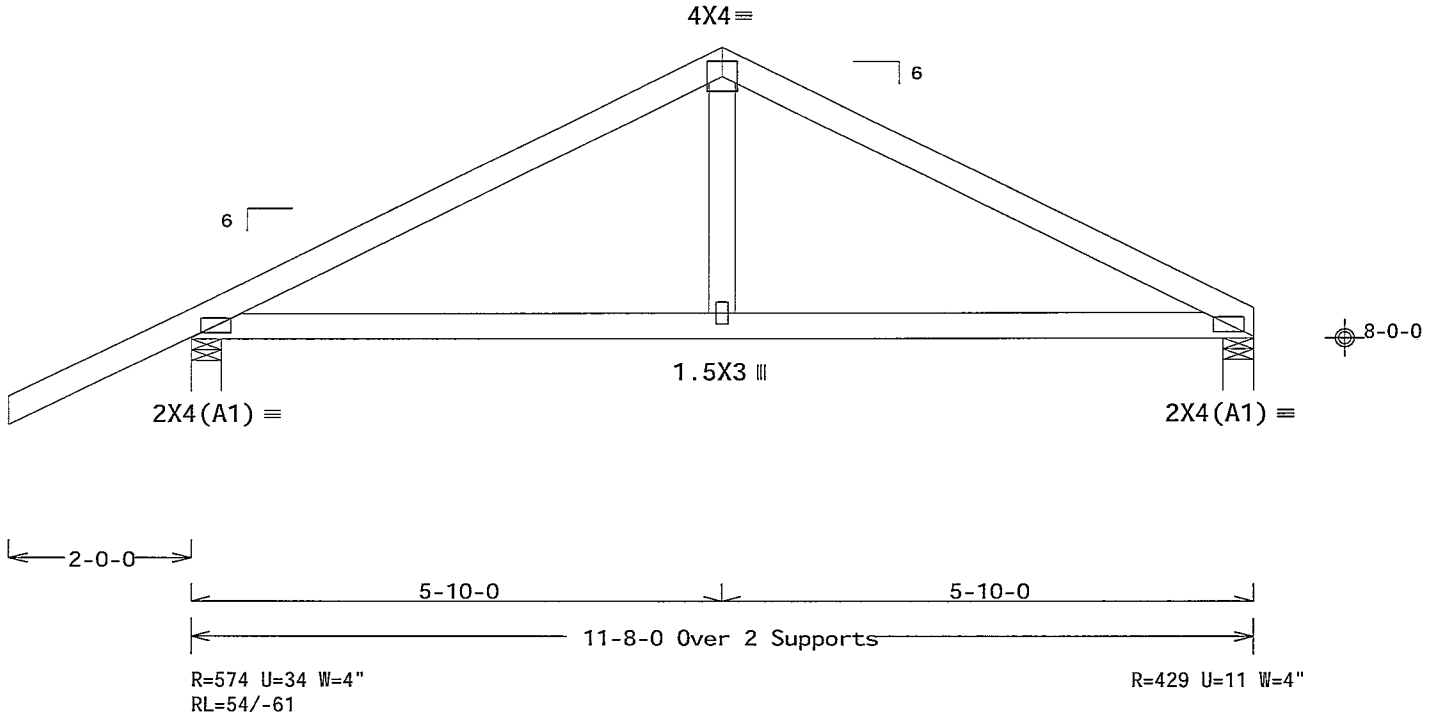
Bottom chord checked for 10.00 psf non-concurrent live load

MWFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 USR6 14 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20893
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084009
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361427
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - B3 12'6"8 Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LU24 w/ (1)2x4 SP 2850f-2 3E supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5.0 psf GCpi(+/-)=0.18

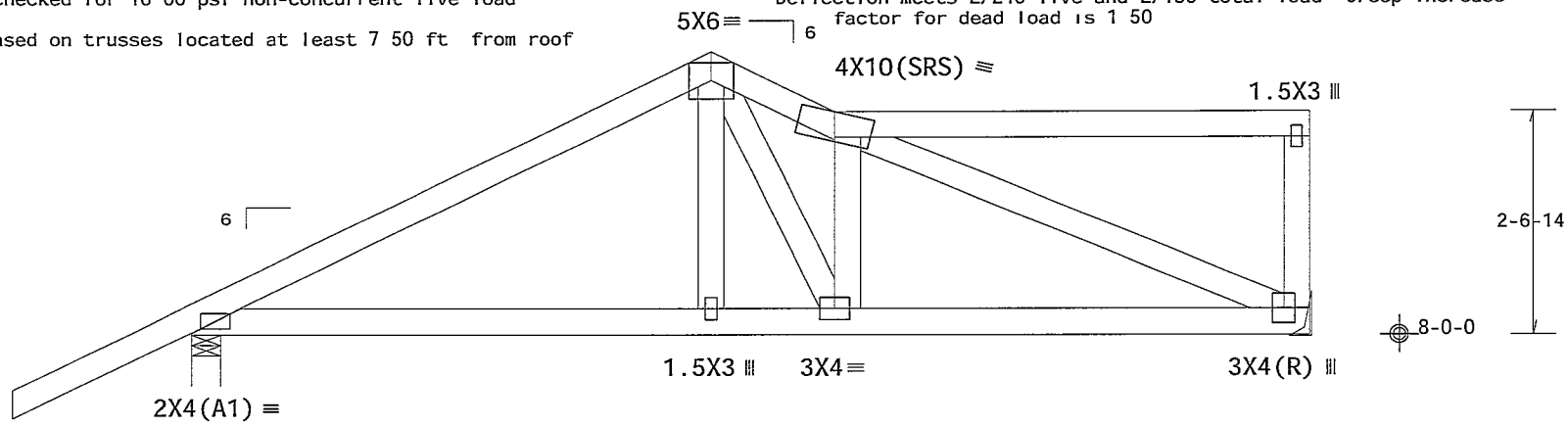
Wind loads and reactions based on MWFRS with additional C&C member design

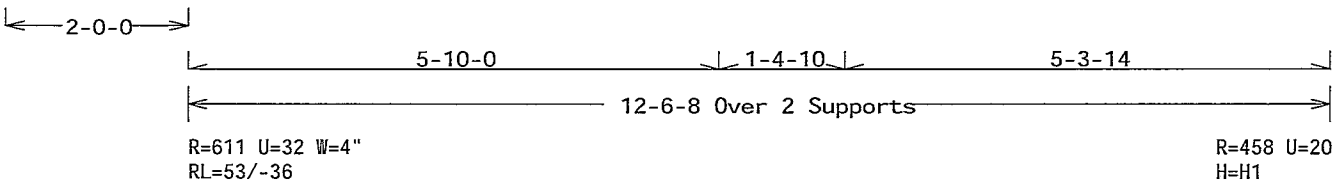
Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 12.06.14 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating handling shipping installing and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Informat on by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipping installation bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20894
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084010
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361426
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - B4 12'6"8 Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LU24 w/ (1)2x4 SP 2850f-2 3E supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member

Bottom chord checked for 10.00 psf non-concurrent live load

MWFRS loads based on trusses located at least 15 00 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf. GCpi(+/-)=0 18

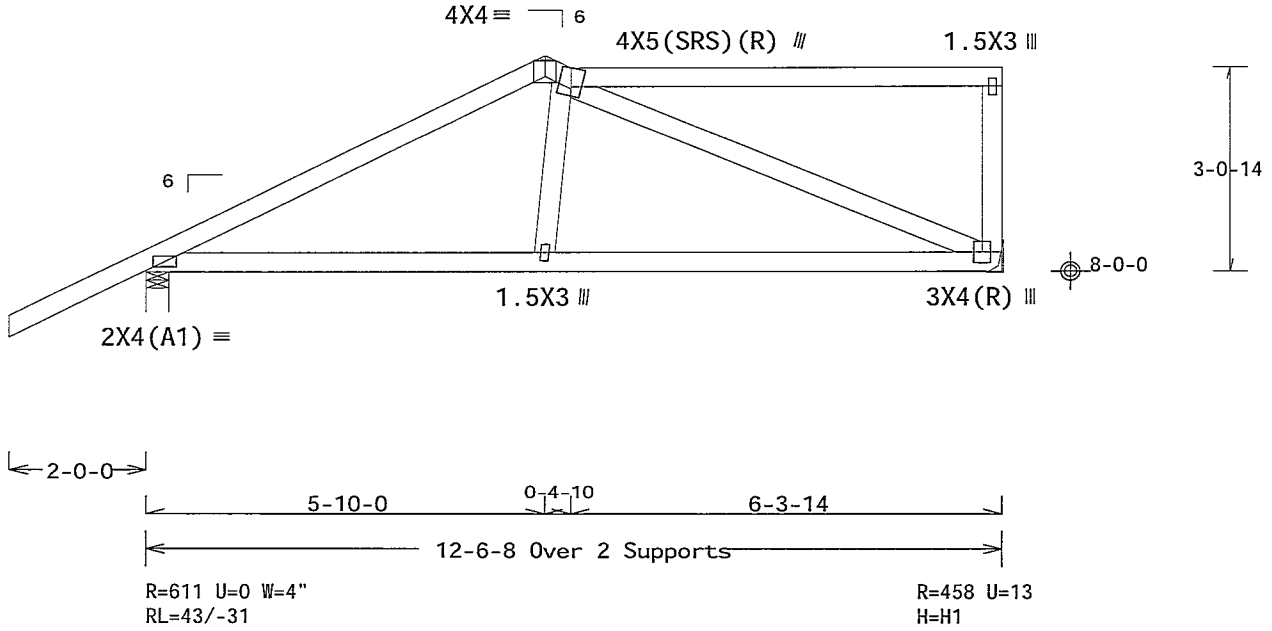
Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.0 0349 4 QTY:1 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care in fabricating handling shipping installing and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Informat on by TPI and WTCA) for safety pract ces prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of web shall have bracing nstalled per BCSI sections B3 B7 or B10 as applicable

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20895
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084034
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361428
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

03/25/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - B5 12'6"8 Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LU24 w/ (1)2x4 SP 2850f-2 3E supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 15 00 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0.18

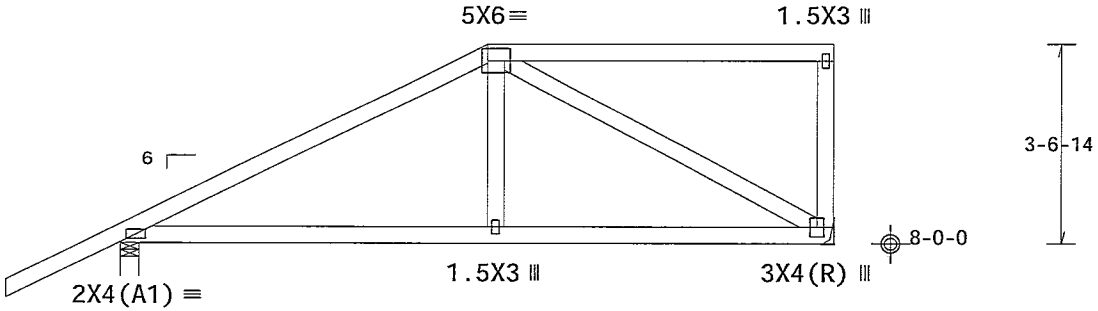
Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.0 0 32 4 QTY:1 FL/-/5/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care in fabricating handling shipping installing and brac ng Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of web shall have brac ng installed per BCSI sections B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any deviation from this design any failure to build the truss in conformance w th ANSI/TPI 1 or for handling sh pping installation bracing of trusses Apply plates to each face of truss and pos tion as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate pos tions A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20896
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084013
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361425
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - B6 12'6"8 Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LU24 w/ (1)2x4 SP 2850f-2 3E supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 15 00 ft from roof edge

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5.0 psf GCpi(+/-)=0.18

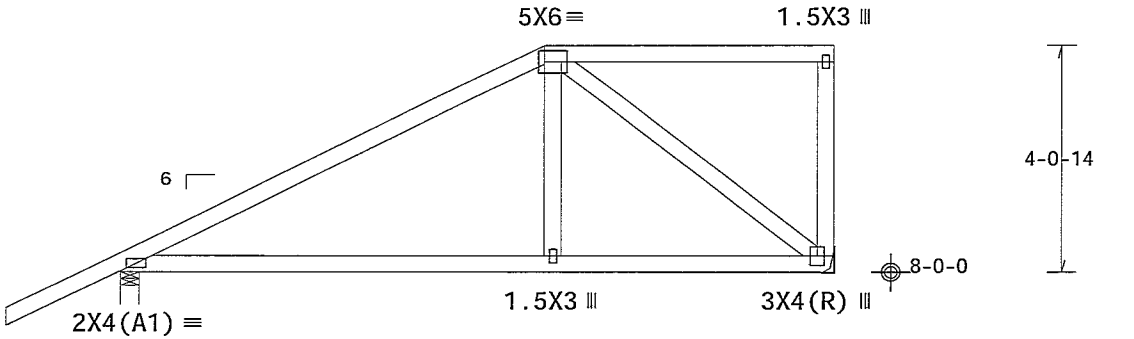
Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=611 U=0 W=4"
RL=52/-32

R=458 U=13
H=H1

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.0 QTY:1 FL/-/5/-/-/R/- Scale =.3125"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Bu lding Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipping nstallation bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering respons bility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Build ng Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

DOUGLAS E. LEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20897
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084014
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361424
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - B7 12'6"8 Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LU24 w/ (1)2x4 SP 2850f-2 3E supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 15 00 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf. GCpi(+/-)=0 18

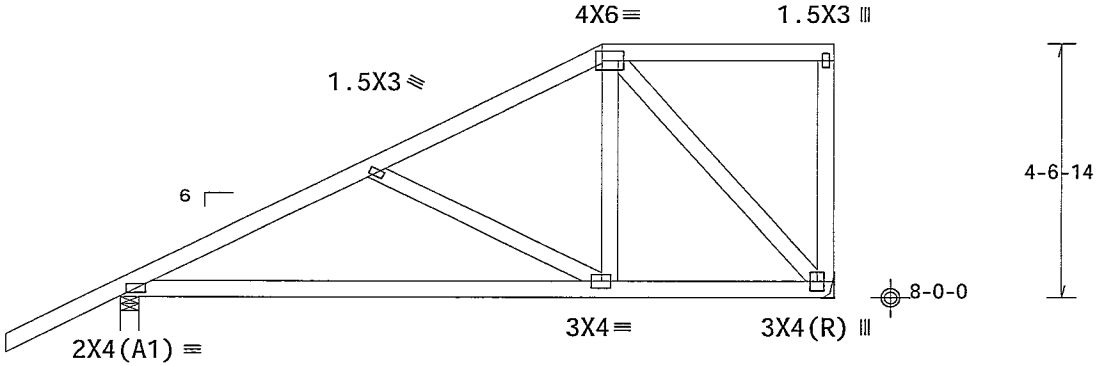
Wind loads and reactions based on MWFRS with additional C&C member design

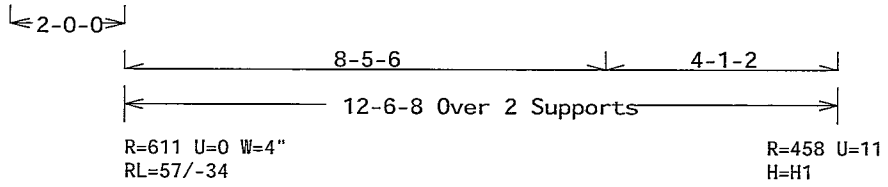
Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.0 | QTY:1 | FL/-/5/-/-/R/- | Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this des any failure to bu ld the truss in conformance with ANSI/TPI 1 or for handling shipping installation bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

DOUGLAS R. EMMS LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20898
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084015
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361423
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

03/25/2014

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - B8 12'6"8 Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LU24 w/ (1)2x4 SP 2850f-2.3E supporting member
(4) 0.148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 15 00 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

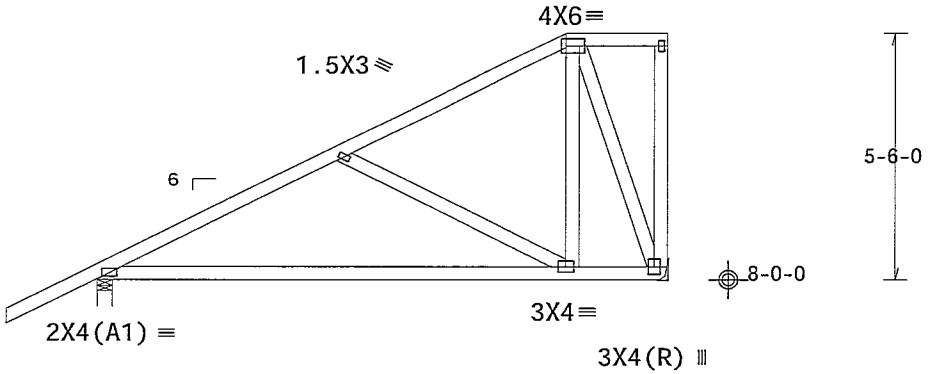
Right end vertical not exposed to wind pressure

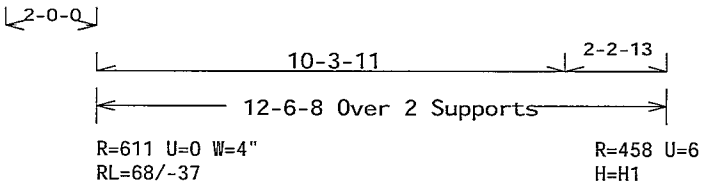
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

1.5X3 ≡





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.0 QTY:1 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS R. EVANS LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20899
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084016
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361422
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

03/25/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - B9 12'6"8 Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LU24 w/ (1)2x4 SP 2850f-2 3E supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5.0 psf GCpi(+/-)=0 18

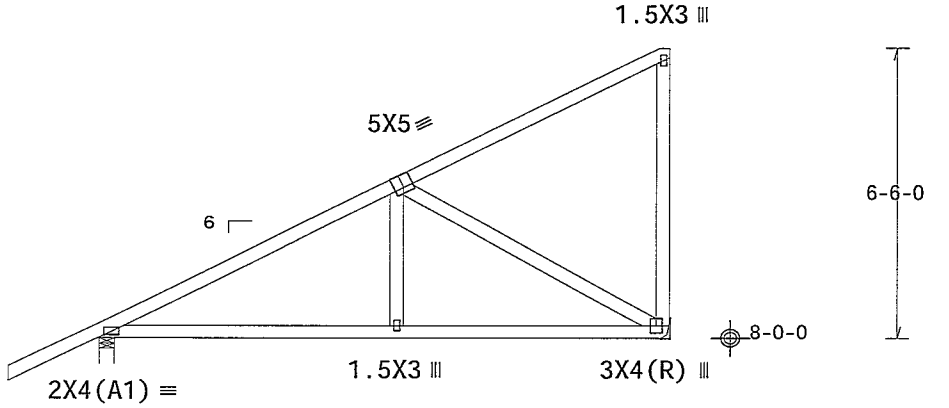
Wind loads and reactions based on MWFRS with additional C&C member design

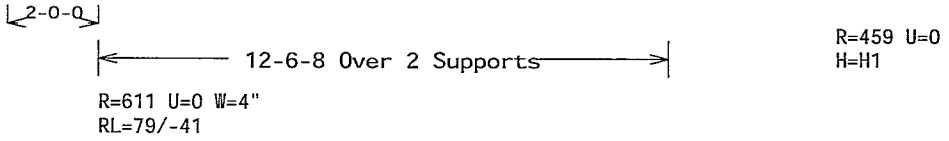
Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

Bottom chord checked for 10.00 psf non-concurrent live load

MWFRS loads based on trusses located at least 15 00 ft from roof edge.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 ms26 14 QTY:1 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20900
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084017
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361421
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - B10 12'6"8 Mono)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LU24 w/ (1)2x4 SP 2850f-2 3E supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

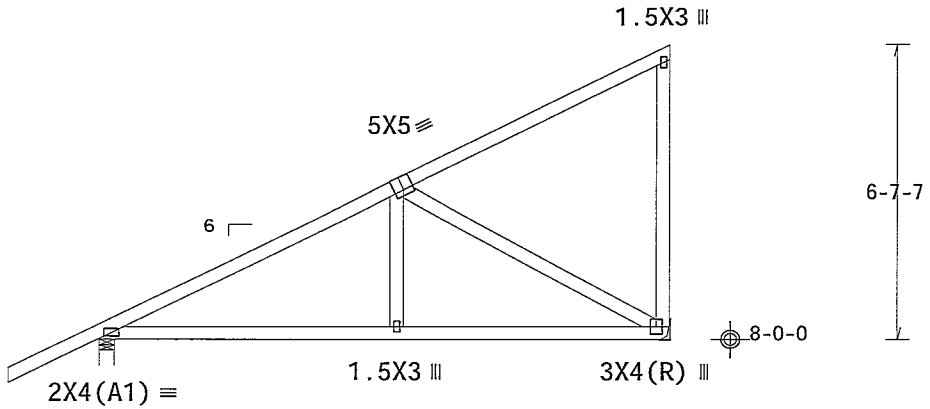
Wind loads and reactions based on MWFRS with additional C&C member design

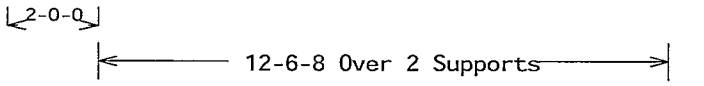
Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

Bottom chord checked for 10.00 psf non-concurrent live load

MWFRS loads based on trusses located at least 15.00 ft from roof edge





R=611 U=0 W=4"
RL=80/-41

R=458 U=0
H=H1

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 05:06:14 QTY:1 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20901
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084018
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361419
DUR.FAC.	1.25		
SPACING	24.0"		JREF- 1V4Z487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - CJ1 1'0"15 Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

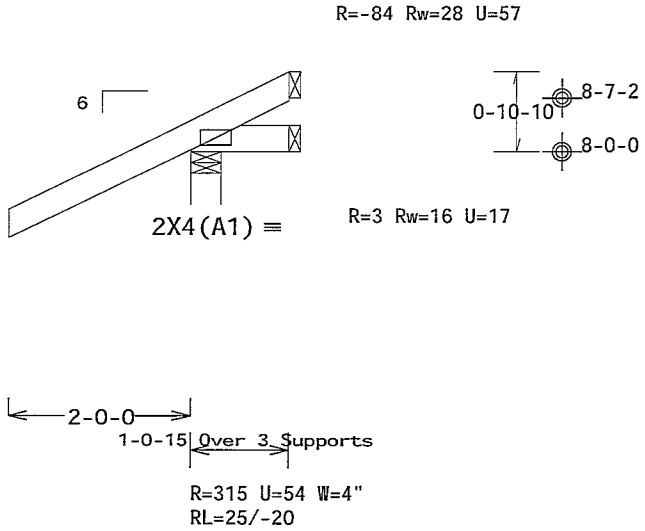
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10.00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5.0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 USG 14 QTY:2 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20902
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084019
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361414
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - CJ2A 1'4"15 Jack)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Lumber value set "13B" uses design values approved 1/30/2013 by ALSC

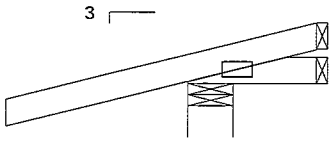
Bottom chord checked for 10 00 psf non-concurrent live load

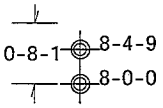
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design

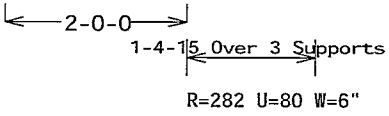
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

R=-31 Rw=21 U=16





2X4(A1) ≡ R=4 Rw=23 U=13



R=282 U=80 W=6"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 13.02.07 2006.14 QTY:2 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20903
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084020
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	2694 REV
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - CJ3 3'0"15 Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

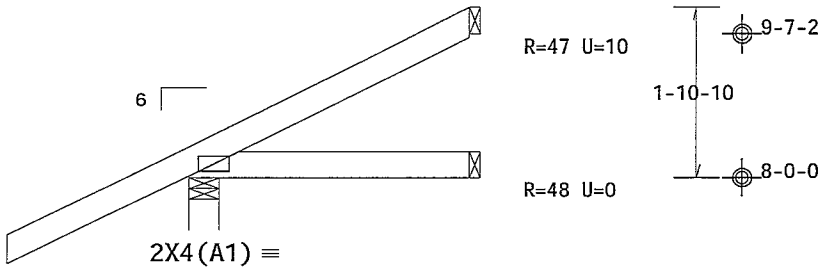
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

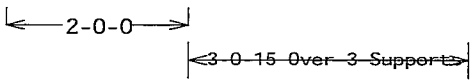
Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5.0 psf GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=289 U=22 W=4"
RL=42/-24

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04-0006.14 | QTY:2 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20904
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084021
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361413
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - CJ3A 2'10"15 Jack)

Top chord 2x4 SP 2850f-2 3E
Bot chord 2x4 SP #1-13B

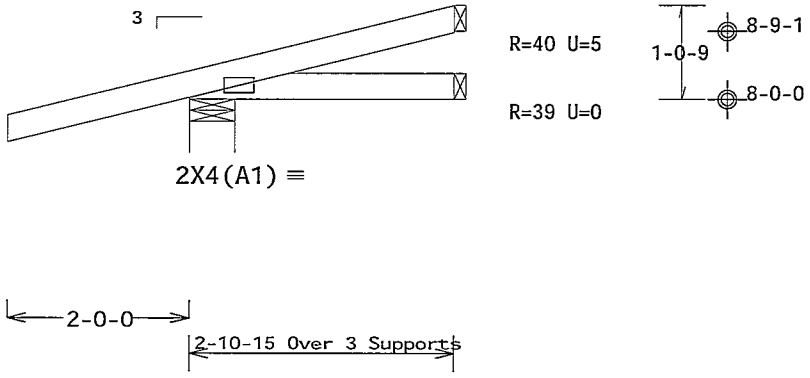
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04-0806.14 QTY:2 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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TC LL	20.0 PSF	REF	R9114- 20905
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084022
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361266
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - DGA 13'9"8 Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

Gable end supports 8" max rake overhang

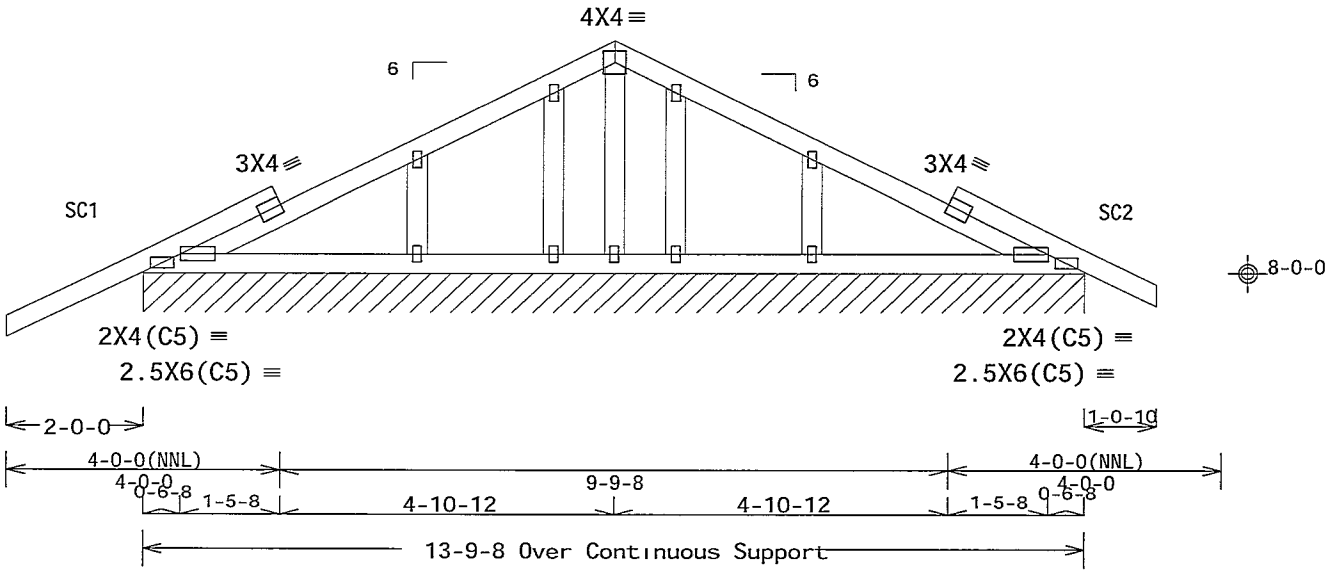
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

Bottom chord checked for 10 00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=89 PLF U=4 PLF W=13-9-8
RL=5/-5 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.0906.14 QTY:1 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20906
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084023
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361272
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - DGB 11'8" Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

Gable end supports 8" max rake overhang

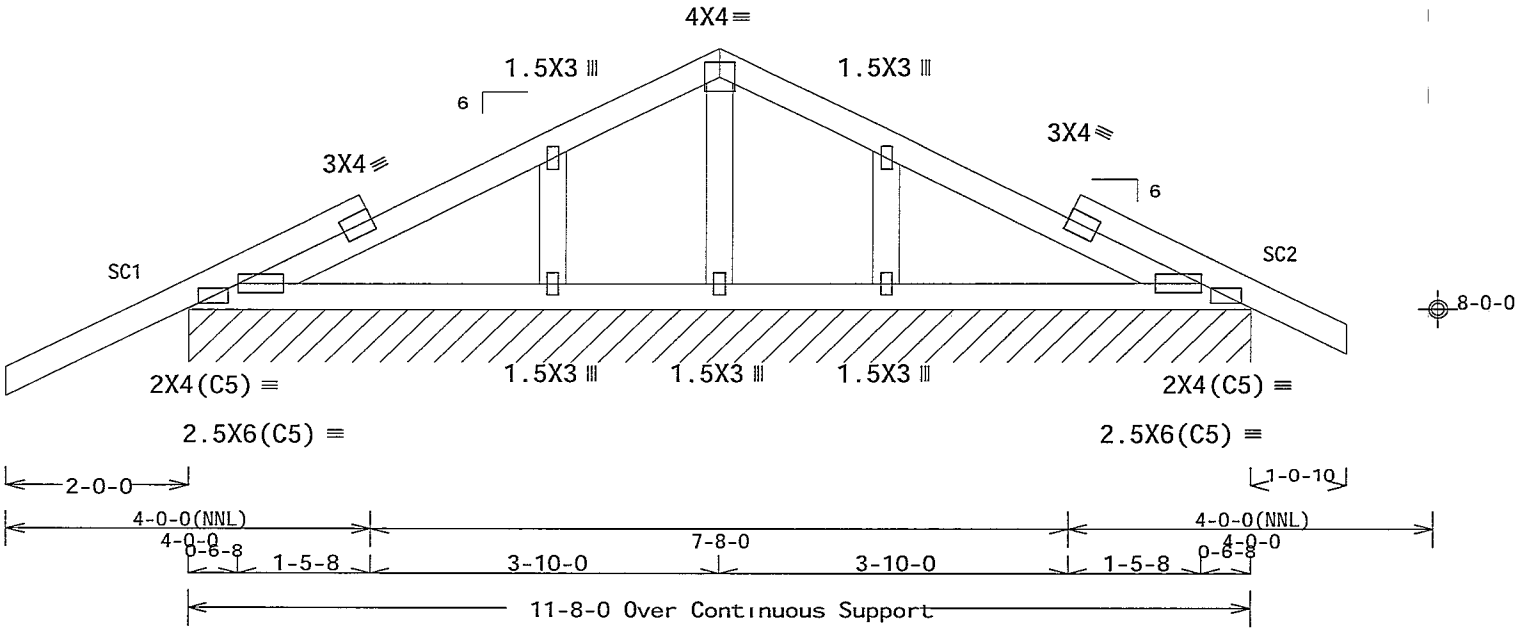
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=91 PLF U=4 PLF W=11-8-0
RL=5/-6 PLF

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.0328 44 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating handling shipping installing and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20 0 PSF	REF	R9114- 20907
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084024
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361280
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

03/25/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - EJ5 5'2" End Jack)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

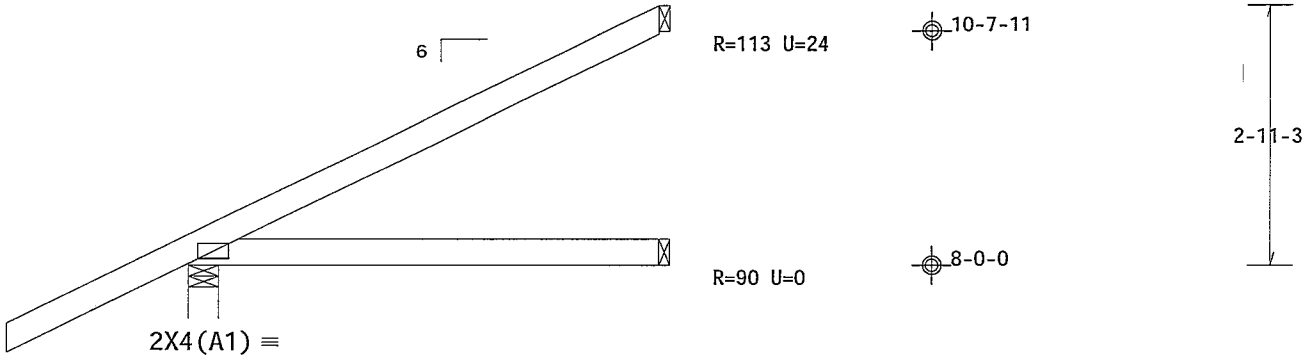
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

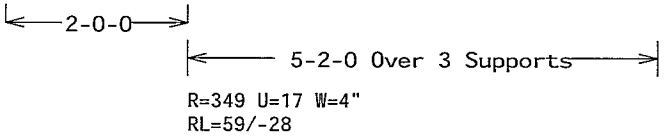
Bottom chord checked for 10.00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=349 U=17 W=4"
RL=59/-28

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.0 [illegible] QTY:4 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20908
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084025
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361412
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

03/25/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - EJ5A 5' End Jack)

Top chord 2x4 SP 2850f-2 3E
Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

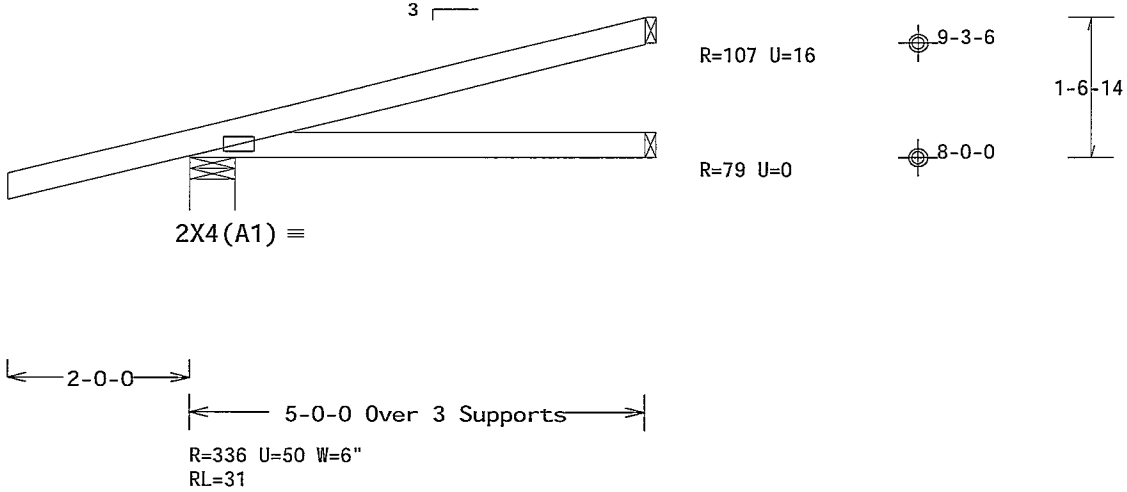
Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.0 03 [illegible] QTY:9 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20909
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084026
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361264
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - HJ5 7'2"15 Hip Jack Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

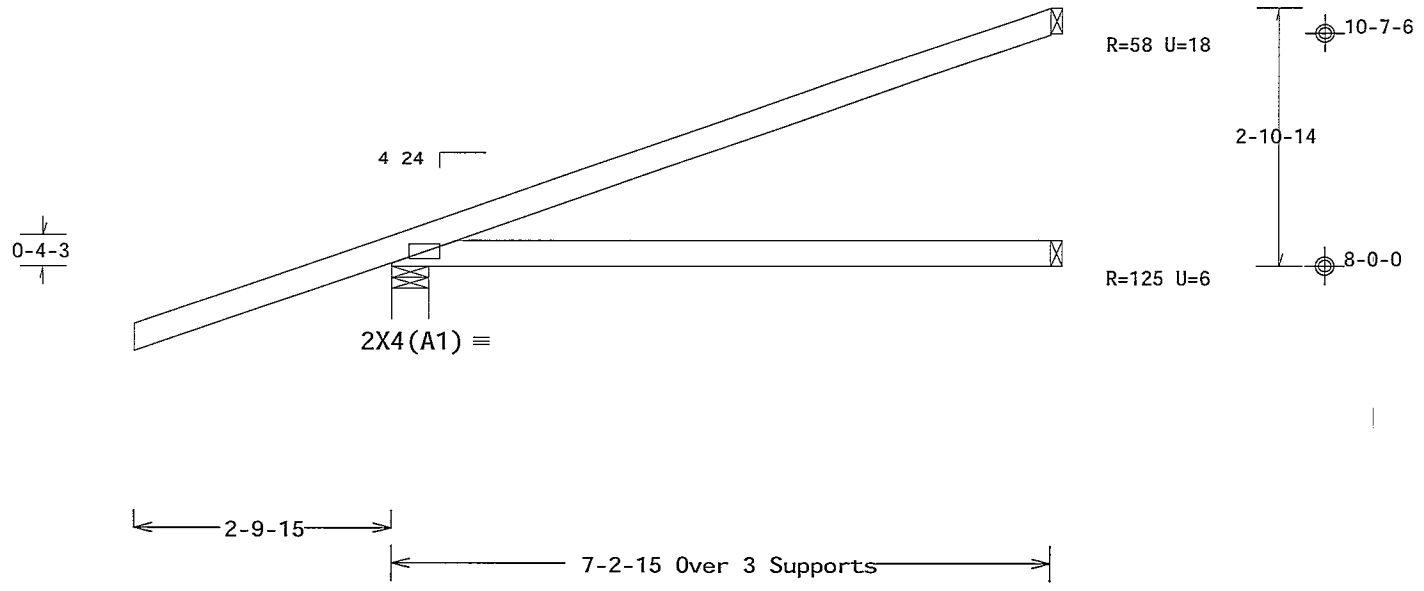
Special loads

------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)

TC- From	0 plf at	-2 83 to	55 plf at	0 00
TC- From	2 plf at	0 00 to	2 plf at	7 24
BC- From	0 plf at	-2.83 to	4 plf at	0 00
BC- From	2 plf at	0.00 to	2 plf at	7 24

TC- -54 18 lb Conc Load at 1 57
TC- 93 56 lb Conc Load at 4 40
BC- 5.88 lb Conc Load at 1 57
BC- 96 24 lb Conc. Load at 4 40

Bottom chord checked for 10 00 psf non-concurrent live load



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 03:20:14 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20 0 PSF	REF	R9114- 20910
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084032
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361429
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - HJ5A 7'0"2 Hip Jack Girder)

Top chord 2x4 SP 2850f-2 3E
Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

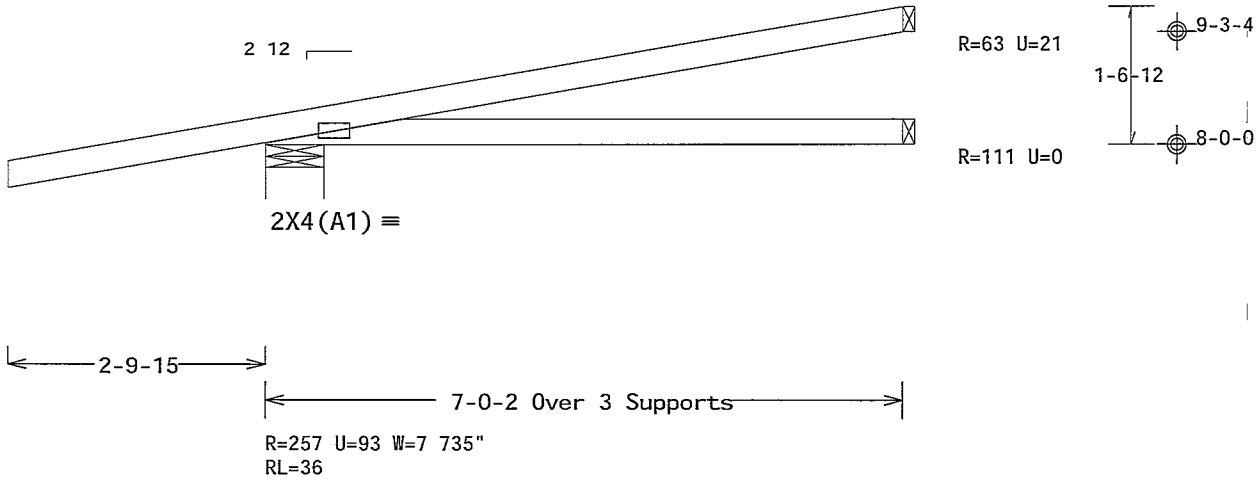
Wind loads and reactions based on MWFRS

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Special loads
------(Lumber Dur.Fac =1.25 / Plate Dur Fac =1 25)
TC- From 0 plf at -2 83 to 54 plf at 0.00
TC- From 2 plf at 0.00 to 2 plf at 7 01
BC- From 0 plf at -2.83 to 4 plf at 0 00
BC- From 2 plf at 0 00 to 2 plf at 7 01
TC- -7 35 lb Conc Load at 2.04
TC- 80 10 lb Conc Load at 4.16
BC- 8 15 lb Conc. Load at 2.04
BC- 77 94 lb Conc Load at 4 16

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 7 50 ft from roof edge



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.0 US2014 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20911
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084033
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361301
DUR.FAC.	1.25		
SPACING	24.0"		JREF- 1V4Z487_Z01

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - MH5 12'6"8 Mono Hip Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LUS24 w/ (1)2x4 SP 2850f-2 3E supporting member
(4) SD9112, 0 131"x1 5" into supporting member,
(2) SD9212, 0 131"x2 5" into supported member

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

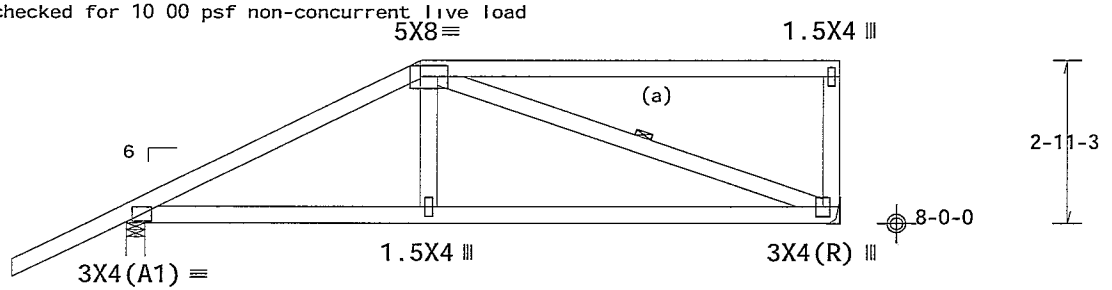
Special loads
------(Lumber Dur Fac.=1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at -2.00 to 56 plf at 5 17
TC- From 28 plf at 5 17 to 28 plf at 12 54
BC- From 4 plf at -2.00 to 4 plf at 0.00
BC- From 20 plf at 0 00 to 20 plf at 5 20
BC- From 10 plf at 5.20 to 10 plf at 12 54
TC- 170 28 lb Conc Load at 5 20
TC- 112.62 lb Conc Load at 7 23, 9 23,11 23
BC- 214 73 lb Conc. Load at 5 20
BC- 89 67 lb Conc Load at 7.23, 9 23,11 23

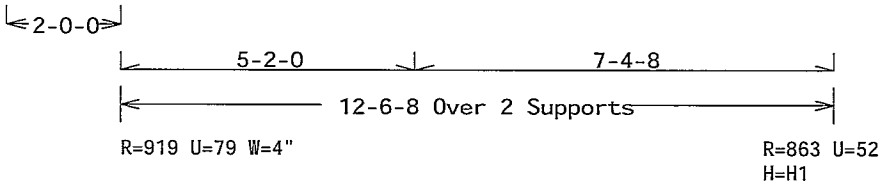
Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.0320.14 QTY:1 FL/-/5/-/-/R/- Scale =.3125"/Ft

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20912
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084011
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361430
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - MH7 12'6"8 Mono Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LU24 w/ (1)2x4 SP 2850f-2 3E supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

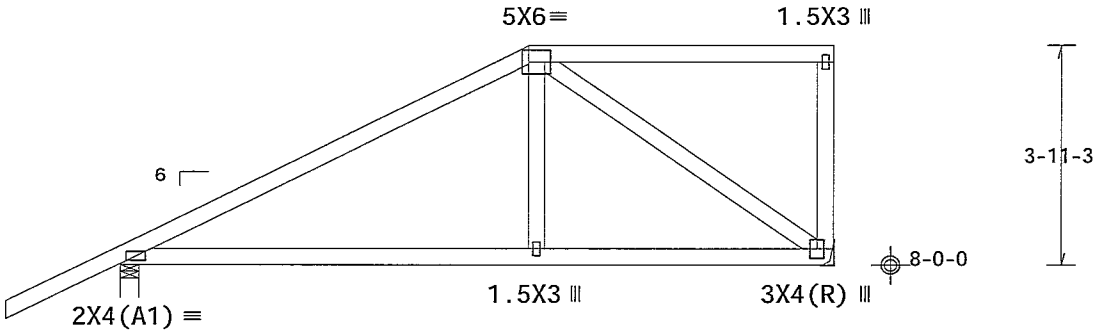
Wind loads and reactions based on MWFRS with additional C&C member design

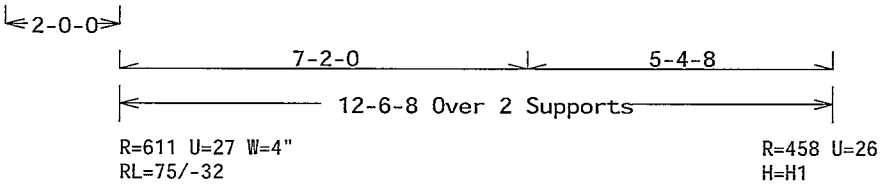
Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.0 10.34.4 QTY:1 FL/-/5/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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DOUGLAS A FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 20913
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084027
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361415
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

03/25/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - MH9 12'6"8 Mono Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LU24 w/ (1)2x4 SP 2850f-2.3E supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

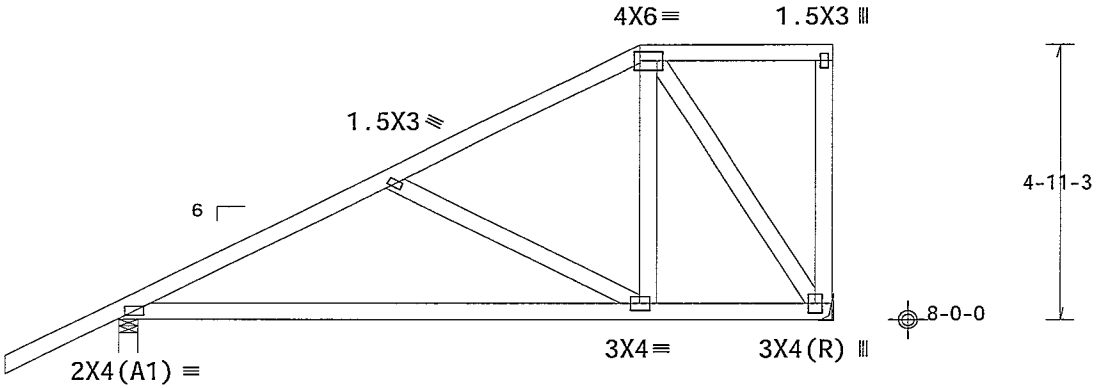
Wind loads and reactions based on MWFRS with additional C&C member design

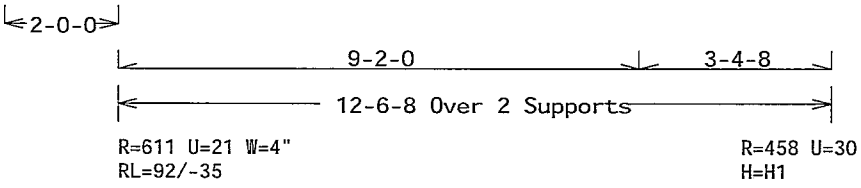
Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.0324.14 QTY:1 FL/-/5/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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TC LL	20.0 PSF	REF	R9114- 20914
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084028
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361416
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

03/25/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - MH11 12'6"8 Mono Hip)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = LU24 w/ (1)2x4 SP 2850f-2 3E supporting member
(4) 0 148"x3" nails into supporting member,
(2) 0 148"x1 5" nails into supported member

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 50 ft from roof edge

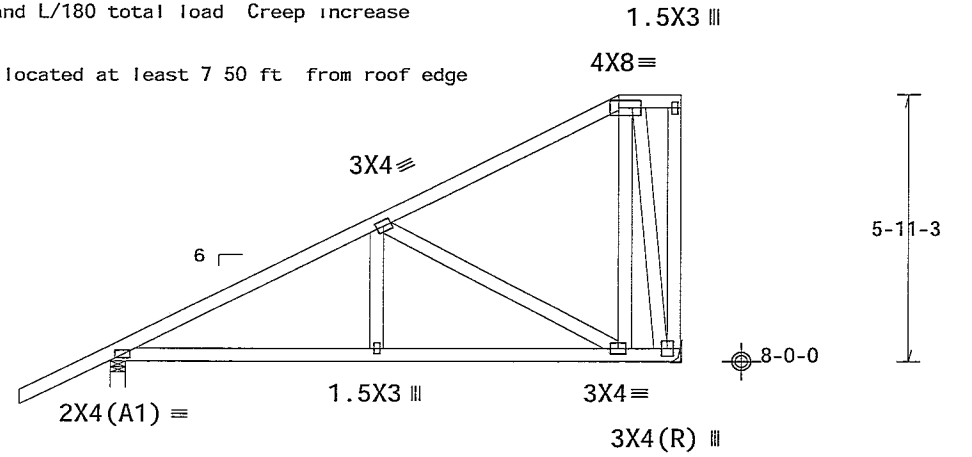
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf. GCpi(+/-)=0 18

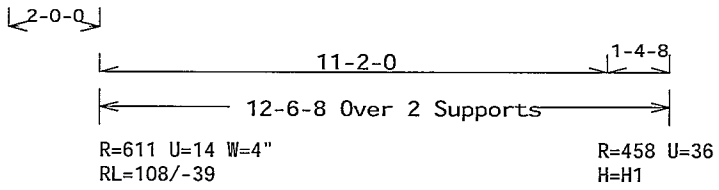
Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

In lieu of structural panels use purlins to brace all flat TC @ 24" OC





R=611 U=14 W=4"
RL=108/-39

R=458 U=36
H=H1

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.0 [illegible]4 QTY:1 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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TC LL	20.0 PSF	REF	R9114- 20915
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084029
BC LL	0.0 PSF		HC-ENG JB/DF
TOT.LD.	37.0 PSF	SEQN-	361417
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

03/25/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-041--BRYAN ZECHER /David & Donna Hutchinson -- Columbia County , FL - PBG1 18'8"5 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
.Stack Chord SC1 2x4 SP #1-13B··Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=2 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

In lieu of rigid ceiling use purlins to brace BC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Refer to DWG PB160100212 for piggyback details

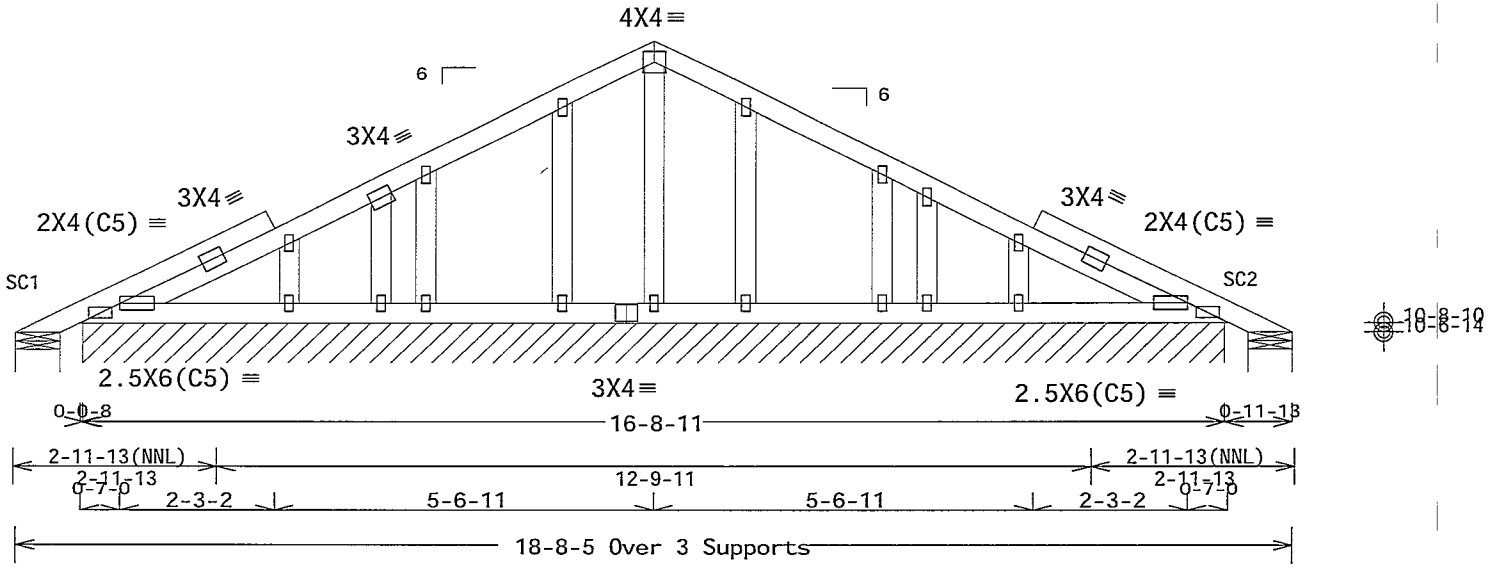
Special loads
------(Lumber Dur Fac =1.25 / Plate Dur Fac =1 25)

TC- From	56 plf at	-0 00 to	56 plf at	3 66
TC- From	56 plf at	3 66 to	56 plf at	9 35
TC- From	56 plf at	9.35 to	56 plf at	17 17
TC- From	56 plf at	17.17 to	56 plf at	18 69
BC- From	4 plf at	0.00 to	4 plf at	8 94
BC- From	4 plf at	8.94 to	4 plf at	18 69

Gable end supports 8" max rake overhang

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24" o c. intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6.



R=-19 Rw=23 U=32 W=7 826"
RL=59/-59

R=67 PLF U=7 PLF W=16-8-11

R=-19 U=7 W=7 826"

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.0 0329 44 QTY:1 FL/-/5/-/-/R/- Scale =.375"/Ft.

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Orlando FL, 32837
FL COA #0 278

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TC LL	20.0 PSF	REF	R9114- 20916
TC DL	7.0 PSF	DATE	03/25/14
BC DL	10.0 PSF	DRW	HCUSR9114 14084030
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	37.0 PSF	SEQN-	361216
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V4Z487_Z01

03/25/2014

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired

Notes

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement

Alternative reinforcement specified in chart below may be conservatve For minimum alternative reinforcement, re-run design with appropriate reinforcement type

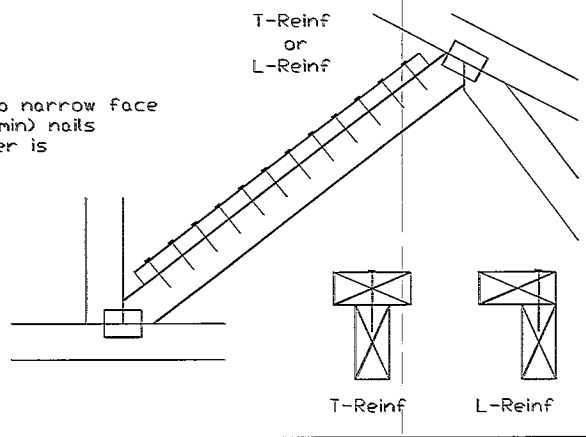
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf	Scab Reinf
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web Apply (1) scab to each face of web

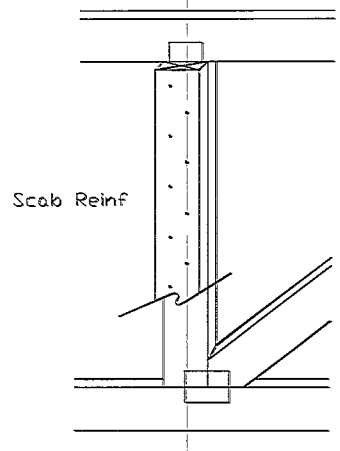
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face Attach with 10d (0.128"x3.0",min) nails at 6" oc Reinforcing member is a minimum 80% of web member length



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face Attach with 10d (0.128"x3.0",min) nails at 6" o.c Reinforcing member is a minimum 80% of web member length.



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Building Components Group Inc.

Earth City MO 63045

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
3/25/14

TC LL	PSF	REF CLR Subst
TC DL	PSF	DATE 8/15/13
BC DL	PSF	DRWG BRCLBSUB0813
BC LL	PSF	
TOT LD	PSF	
DUR FAC		
SPACING		

Piggyback Detail – ASCE 7-10 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

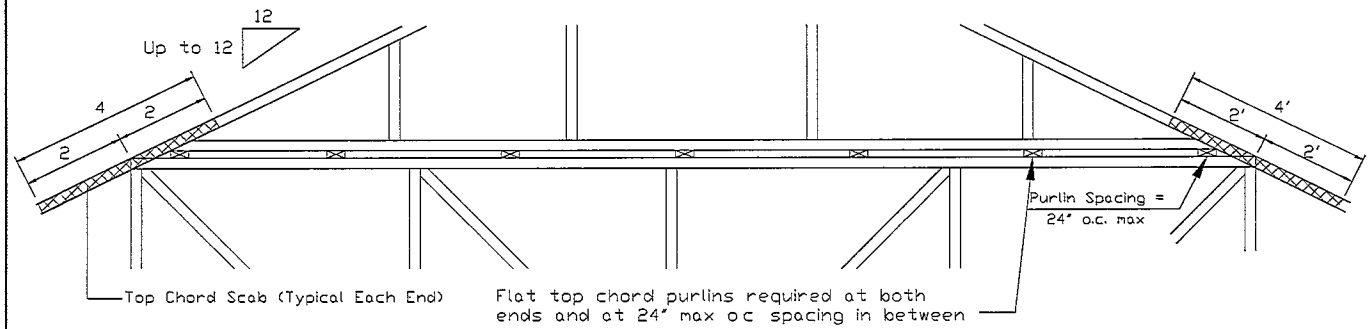
160 mph Wind 30.00 ft Mean Hgt, ASCE 7-10 Enclosed Bldg located anywhere in roof Exp C Wind DL= 5.0 psf (min) Kzt=1.0
Or 140 mph wind 30.00 ft Mean Hgt ASCE 7-10 Enclosed Bldg located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0

Note Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends

Maximum truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola steeple, chimney or drag strut loads

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications

Detail A Purlin Spacing = 24" o.c. or less



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

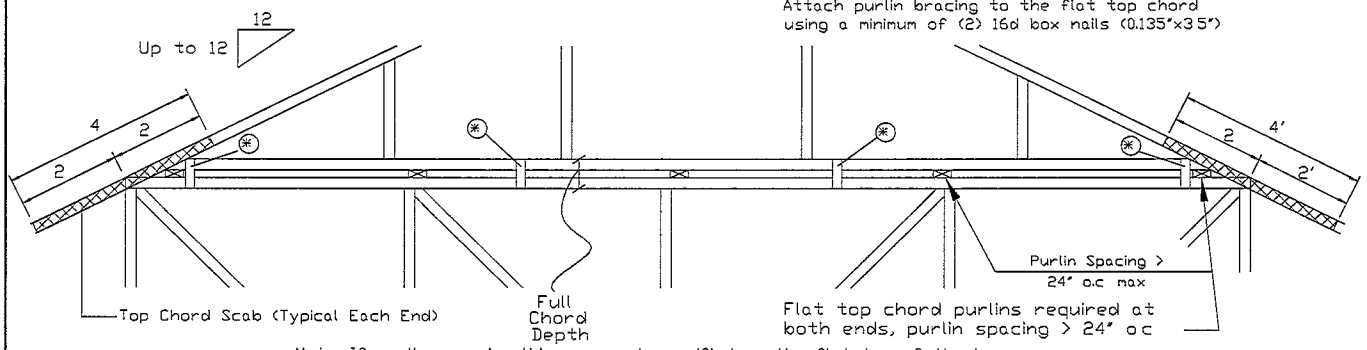
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails Note Nailing thru holes of wave plate is acceptable

Detail B Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5")



Note If purlins or sheathing are not specified on the flat top of the base truss purlins must be installed at 24" o.c. max and use Detail A

* In addition provide connection with one of the following methods.

Trulox
Use 3X8 Trulox plates for 2x4 chord member and 3X10 Trulox plates for 2x6 and larger chord members Attach to each face @ 8" o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord Trulox plates may be staggered 4" o.c. front to back faces.

APA Rated Gusset
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8" o.c. with (8) 6d common (0.113"x2") nails per gusset (4) in cap bottom chord and (4) in base truss top chord Gussets may be staggered 4" o.c. front to back faces

2x4 Vertical Scabs
2x4 SPF #2, full chord depth scabs (each face) Attach @ 8" o.c. with (6) 10d box nails (0.128"x3") per scab (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces

28PB Wave Piggyback Plate
One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply Piggyback plates may be staggered 4' o.c. front to back faces.

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'14

SPACING 24.0"

REF	PIGGYBACK
DATE	2/14/12
DRWG	PB160100212