DATE 06/14/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024629

APPLICANT KELLY O'NEIL PHONE 454-2476

ADDRESS PO BOX 1633 HIGH SPRINGS FL 32643

OWNER ANDREW TAYLOR PHONE 454-2476

ADDRESS 818 SW DOWNING DRIVE HIGH SPRINGS FL 32643

CONTRACTOR O'NEIL CONSTRUCTION PHONE 454-2476

LOCATION OF PROPERTY 441 S, L ADAMS RD, R DOWNING DR, TO END
PROPERTY ON RIGHT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 76900.00

HEATED FLOOR AREA 1538.00 TOTAL AREA 1965.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 5/12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT 15

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X PS DEVELOPMENT PERMIT NO.

PARCEL ID 10-7S-17-09977-014 SUBDIVISION DOWNING ACRES

LOT 14 BLOCK PHASE UNIT TOTAL ACRES 5.00

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
			Kelly L O'Neil

Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident
EXISTNG	~~06-0329-N~~	BK	JH	Y

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE
MH TO BE REMOVED 30 DAYS AFTER CO IS ISSUED

Check # or Cash 18276

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 385.00 CERTIFICATION FEE $ 9.83 SURCHARGE FEE $ 9.83

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 479.66

INSPECTORS OFFICE _____ CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

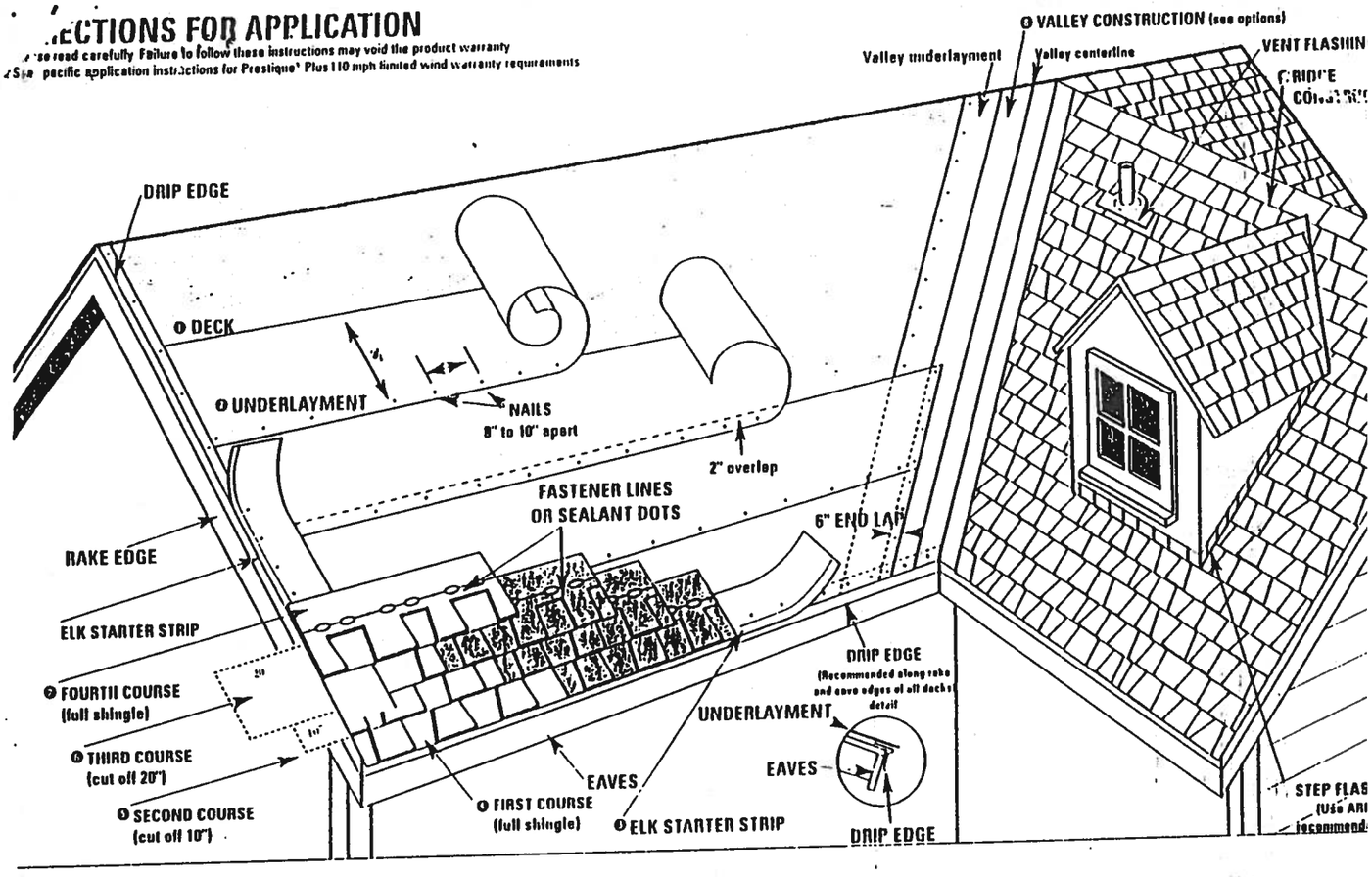
This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

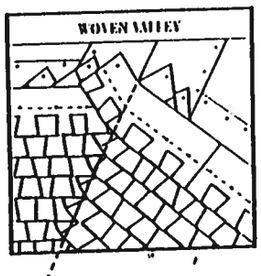
The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

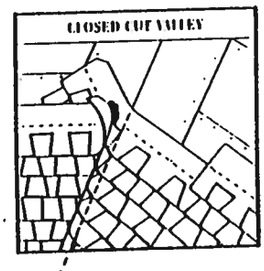
ECTIONS FOR APPLICATION

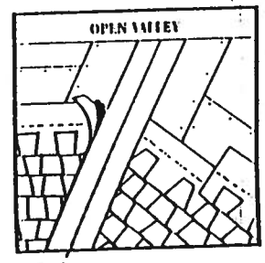
...so read carefully. Failure to follow these instructions may void the product warranty.
...See specific application instructions for Prestique® Plus 110 mph limited wind warranty requirements.



❻ VALLEY CONSTRUCTION OPTION (California Open and California Closed are also acceptable) NOTE: For complete ARMA valley installation details, see ARMA Residential Asphalt Roofing M







DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All attics should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

❶ DECK PREPARATION

Roof decks should be dry, well seasoned 1" x 6" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

❷ UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt). Cover drip edge at eaves only.

For low slope (2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 19". Begin by fastening a 19" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

EAVE FLASHING FOR ICE DAMS (IF REQUIRED)

For standard slope (4/12 to less than 21/12), use ice and water shield or coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 12 inches beyond the inside wall of the living space below.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below.

Consult the Elk Field Service Department for application specifications over other decks and other slopes.

❸ STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR A STRIP SHINGLE INVERTED WITH THE HEADLAP APPLIED AT THE EAVE EDGE. With at least 4" trimmed from the end of the first shingle, start at the rake edge overhanging the eave 1/2" to 3/4". Fasten 2" from the lower edge and 1" from each side. Shingles may be applied with a course alignment of 45° on the roof.

❸ FIRST COURSE

Start at rake and continue course with full shingles and flush with the starter course.

❺ SECOND COURSE

Start at the rake with the shingle having 10" trimmed off and continue across roof with full shingles.

❹ THIRD COURSE

Start at the rake with the shingle having 20" trimmed off and continue across roof with full shingles.

❼ FOURTH COURSE

Start at the rake and continue with full shingles across roof.

FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. Do not rack shingles straight up the roof.

❻ VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 36" wide vertical underlayment prior to applying 18" metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

❾ RIDGE CONSTRUCTION

For ridge construction use Class "A" Seal-A-Ridge® with formula *FLX* ® (See ridge wrapper for installation instructions.)

FASTENERS

While nailing is the preferred method for Prestique shingles, Elk will accept fastening methods according to the following instructions.

Always nail or staple through the fastener line or on products without fastener lines, nail or staple between and in line with sealant dots.

NAILS: Corrosive resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for roof-overs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only, 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-roof.

STAPLES: Corrosive resistant, 16 gauge minimum, crown width minimum of 15/16". Note: An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

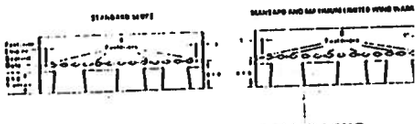
Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less.

MANSARD APPLICATIONS

Correct fastening is critical to the performance of the roof. For slopes exceeding 60° (or 21/12) use six fasteners per shingle. Locate fasteners in the fastener area 1" from each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

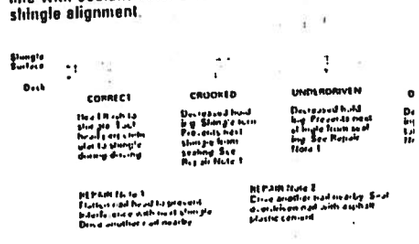
LIMITED WIND WARRANTY

- For a Limited Wind Warranty, Prestique shingles must be applied with 4 properly placed fasteners, or in the case of mansard applications, 6 properly placed fasteners per shingle.
- For a Limited Wind Warranty up to 110 MPH, Prestique Plus 40 shingles must be applied with 6 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 3/4 of an inch.



HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DI THICKNESS (laminated) area of the shingle. Nails or s must be placed along – and through – the fastener line products without fastener lines, nail or staple between line with sealant dots. CAUTION: Do not use fastener l shingle alignment.



Refer to local codes which in some areas may require application techniques beyond those Elk has specified. All Elk Prestique shingles have a U.L.® Wind Resistance when applied in accordance with these instructions using staples on re-roofs as well as new construction.

CAUTION TO WHOLESALER: Careless and im storage or handling can harm fiberglass sh Keep these shingles completely covered reasonably cool, and protected from the we Do not store near various sources of heat. D store in direct sunlight until applied. DO DOUBLE STACK. Systematically rotate all st that the material that has been stored the lo will be the first to be moved out.

ELK

www.elkcorp

ecord and Return to:
Sunbelt Title Agency
2211 Lee Rd, Suite 218
Winter Park FL 32789
File No. ST1050183

This Warranty Deed

Inst:2005029919 Date:12/02/2005 Time:13:35
Doc Stamp-Deed : 588.00
DC,P.DeWitt Cason,Columbia County B:1066 P:2442

Made this **18th day of November, 2005** by
LOIS A. CARROLL, UNMARRIED

hereinafter called the grantor, to
ANDREW V. TAYLOR

whose post office address is:
1075 WOODFIELD ROAD
GREENACRES, FL 33415

hereinafter called the grantee:
(Whenever used herein the term "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations)

Witnesseth, that the grantor, for and in consideration of the sum of **$10.00** and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in **COLUMBIA** County, Florida, viz:

A PART OF THE SOUTHEAST QUARTER (SE1/4) OF SECTION 10, TOWNSHIP 7 SOUTH, RANGE 17 EAST MORE PARTICULARLY DESCRIBED AS FOLLOWS:

BEGIN AT THE SOUTHEAST CORNER OF SAID SECTION 10 AND RUN SOUTH 88°25'08" WEST ALONG THE SOUTH LINE OF SAID SECTION 10, 364.28 FEET; THENCE NORTH 2°40'08" WEST, 584.10 FEET TO THE SOUTH RIGHT-OF-WAY LINE OF DOWNING DRIVE; THENCE NORTH 88°23'50" EAST ALONG THE SAID RIGHT-OF-WAY LINE, 73.72 FEET; THENCE ALONG THE ARC OF A CUL-DE-SAC CURVE TO THE LEFT HAVING A RADIUS OF 50.0 FEET, AN INCLUDED ANGLE OF 143°07'48" FOR AN ARC DISTANCE OF 124.90 FEET; THENCE NORTH 88°23'50" EAST, 200.0 FEET TO THE EAST LINE OF SAID SECTION 10, THENCE SOUTH 2°40'08" EAST, 614.25 FEET TO THE POINT OF BEGINNING. COLUMBIA COUNTY, FLORIDA. ALSO KNOWN AS LOT 14 OF DOWNING ACRES SUBDIVISION, UNRECORDED.

TOGETHER WITH A 1985 DRIF HS ID #SSMFLAC112234 LOCATED ON SUBJECT PROPERTY.

Subject to covenants, restrictions, easements of record and taxes for the current year.

Parcel Identification Number: 10-7S-17-09977-014

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.
To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances except taxes accruing subsequent to **December 31, 2005**

In Witness Whereof, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in our presence:

Witness: (Signature)
Print Name: Don A. Thomas

Witness: (Signature)
Print Name: Charlotte C. Dixon

Lois A Carroll
LOIS A. CARROLL
27718 NW 182ND AVENUE
HIGH SPRINGS, FL 32643

Incident to the issuance of a title insurance contract.
warrantydeed.dot
rev. 09/12/05
sm

Inst:2005029919 Date:12/02/2005 Time:13:35
Doc Stamp-Deed : 588.00
DC,P.DeWitt Cason,Columbia County B:1066 P:2443

State of Florida
County of Alachua

The foregoing instrument was acknowledged before me this **18th day of November, 2005**, by **LOIS A. CARROLL, UNMARRIED,** who is personally known to me or who has produced **drivers license** as identification.

NOTARY PUBLIC (signature)
Print Name:
My Commission Expires:
Stamp/Seal:

CHARLOTTE C. DIXON
Notary Public - State of Florida
My Commission Expires Jan 22, 2007
Commission # DD179033
Bonded By National Notary Assn.

Prepared by :
Charlotte Dixon
Professionals' Title Company, LLC
4141 NW 37th Pl
Gainesville, FL 32606
File Number: **581050183**

Incident to the issuance of a title insurance contract.
warrantydeed.dot
rev. 09/12/05

RESUBMISSION 07

A **Columbia County Building Permit Application** Revised 9-23-04

For Office Use Only Application # 0606-27 Date Received 6/9/06 By JW Permit # 24629

Application Approved by - Zoning Official BLK Date 6.12.06 Plans Examiner OK JTH Date 6/12/06

Flood Zone X Per survey Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments - [illegible] (Signed Site plan - EH) As per Mark Lander to issue Permit w/ signed MH to be removed prior to CO. being issued. 0603-29

Applicants Name ONEIL CONSTRUCTION, KELLY ONEIL Fax: 3864544244 Phone 3864542476

Address P.O. BOX 1633 HIGH Springs, Fl 32655

Owners Name ANDREW TAYLOR Phone 4542476

911 Address 818 Downing Drive, High Springs, Fl 32643

Contractors Name Dennis ONeil Phone 3525389515

Address P.O. Box 1633, High Springs Fl 32655

Fee Simple Owner Name & Address SAME AS OWNER / 1015 Woodfield, Green Acres Fl. 33415

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address PHIL Colacino 1223 SW 186th St. Newberry Fl.

Mortgage Lenders Name & Address NONE

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progressive Energ

Property ID Number 10-7S-17E 09977-014 Estimated Cost of Construction 150,000

Subdivision Name DOWNING ACRES UNREC. Lot 14 Block ____ Unit ____ Phase ____

Driving Directions From HIGH Springs Go N ON 441 to ADAMS RD T-R Go To Downing Drive T-R to END property ON R

Type of Construction Concrete Block SFD Number of Existing Dwellings on Property 1

Total Acreage 5 Lot Size ______ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Dri

Actual Distance of Structure from Property Lines - Front 309 Side 279 Side 79 Rear 218

Total Building Height 15' Number of Stories 1 Heated Floor Area 1538 total 1965 Roof Pitch 5/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Kelly L ONeil

Owner Builder or Agent (Including Contractor)

Kelly L ONeil

Contractor Signature

Contractors License Number CGC 061581

Competency Card Number ______

NOTARY STAMP/SEAL

GALE TEDDER
MY COMMISSION # DD 33[illegible]
EXPIRES: June 28, 2008
Bonded Thru Notary Public Underwriters

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me

this 9th day of June 2006.

Personally known _____ or Produced Identification DL

Gale Tedder

Notary Signature

[illegible]

Notice of Treatment 40842

Applicator: Florida Pest Control & Chemical Co. (www.flapest.com)
Address: 116 NW 16 AVE Phone 376 2661
City G. Ville

Site Location: Subdivision
Lot # Block# Permit #
Address 818 SW Downing Dr H. Springs

Product used	Active Ingredient	% Concentration
☑ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☐ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☐ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
M/B	1965	183	165.3

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

6.28.06 Date | 9:45 Time | Richard Print Technician's Name

Remarks: Permit Number 24629

Applicator - White Permit File - Canary Permit Holder - Pink 10/05 ©

FORM 600A-2004 EnergyGauge® 4.0

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	TAYLOR RESIDENCE	Builder:	O'NEIL CONSTRUCTION
Address:		Permitting Office:	COLUMBIA COUNTY
City, State:	,	Permit Number:	24629
Owner:	TAYLOR	Jurisdiction Number:	221000
Climate Zone:	North		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 30.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	No		
6. Conditioned floor area (ft²)	1538 ft²	c. N/A	
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)			
a. U-factor: (or Single or Double DEFAULT)	Description Area 7a. (Dble Default) 121.0 ft²	13. Heating systems	
b. SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 219.0 ft²	a. PTHP	Cap: 30.0 kBtu/hr
8. Floor types			COP: 3.70
a. Slab-On-Grade Edge Insulation	R=0.0, 147.0(p) ft	b. N/A	
b. N/A			
c. N/A		c. N/A	
9. Wall types			
a. Concrete, Int Insul, Exterior	R=5.0, 1083.0 ft²	14. Hot water systems	
b. Concrete, Int Insul, Adjacent	R=5.0, 271.0 ft²	a. Electric Resistance	Cap: 40.0 gallons
c. N/A			EF: 0.93
d. N/A		b. N/A	
e. N/A			
10. Ceiling types		c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)	
a. Under Attic	R=30.0, 1537.9 ft²	15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 50.0 ft		
b. N/A			

Glass/Floor Area: 0.14 Total as-built points: 20596 Total base points: 24610 **PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Larry Resmondo a/c

DATE: 3-24-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: O'Neil Construction

DATE: 3/27/06

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1538.0	20.04	5547.9	Double, Clear	N	5.0	4.0	12.0	19.20	0.68	155.7	
				Double, Clear	W	1.5	6.0	20.0	38.52	0.91	703.7	
				Single, Clear	S	1.5	8.0	98.0	40.81	0.92	3681.8	
				Double, Clear	N	5.0	8.0	7.0	19.20	0.79	106.0	
				Double, Clear	W	1.5	5.0	16.0	38.52	0.88	539.7	
				Double, Clear	N	1.5	6.0	30.0	19.20	0.94	540.7	
				Double, Clear	E	1.5	6.0	30.0	42.06	0.91	1151.8	
				Double, Clear	W	1.5	4.0	6.0	38.52	0.81	188.1	
				As-Built Total:				**219.0**			**7067.4**	
WALL TYPES	Area X	BSPM	= Points	Type			R-Value	Area X	SPM	=	Points	
Adjacent	271.0	0.70	189.7	Concrete, Int Insul, Exterior			5.0	1083.0	1.00		1083.0	
Exterior	1083.0	1.70	1841.1	Concrete, Int Insul, Adjacent			5.0	271.0	0.70		189.7	
Base Total:	**1354.0**		**2030.8**	**As-Built Total:**				**1354.0**			**1272.7**	
DOOR TYPES	Area X	BSPM	= Points	Type				Area X	SPM	=	Points	
Adjacent	21.0	2.40	50.4	Exterior Wood				21.0	6.10		128.1	
Exterior	21.0	6.10	128.1	Adjacent Wood				21.0	2.40		50.4	
Base Total:	**42.0**		**178.5**	**As-Built Total:**				**42.0**			**178.5**	
CEILING TYPES	Area X	BSPM	= Points	Type			R-Value	Area X	SPM X SCM	=	Points	
Under Attic	1537.9	1.73	2660.6	Under Attic			30.0	1537.9	1.73 X 1.00		2660.6	
Base Total:	**1537.9**		**2660.6**	**As-Built Total:**				**1537.9**			**2660.6**	
FLOOR TYPES	Area X	BSPM	= Points	Type			R-Value	Area X	SPM	=	Points	
Slab	147.0(p)	-37.0	-5439.0	Slab-On-Grade Edge Insulation			0.0	147.0(p	-41.20		-6056.4	
Raised	0.0	0.00	0.0									
Base Total:			**-5439.0**	**As-Built Total:**				**147.0**			**-6056.4**	
INFILTRATION	Area X	BSPM	= Points					Area X	SPM	=	Points	
	1538.0	10.21	15703.0					1538.0	10.21		15703.0	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 20681.7			**Summer As-Built Points:**					**20825.8**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 30000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)					
20826			20826	1.00	(1.09 x 1.147 x 1.00)	0.263	1.000	6835.7
20681.7	**0.4266**	**8822.8**	**20825.8**	**1.00**	**1.250**	**0.263**	**1.000**	**6835.7**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1538.0	12.74	3526.9	Double, Clear	N	5.0	4.0	12.0	24.58	1.02	301.1	
				Double, Clear	W	1.5	6.0	20.0	20.73	1.02	424.3	
				Single, Clear	S	1.5	8.0	98.0	20.24	1.04	2071.1	
				Double, Clear	N	5.0	8.0	7.0	24.58	1.01	174.2	
				Double, Clear	W	1.5	5.0	16.0	20.73	1.03	343.2	
				Double, Clear	N	1.5	6.0	30.0	24.58	1.00	739.1	
				Double, Clear	E	1.5	6.0	30.0	18.79	1.04	583.8	
				Double, Clear	W	1.5	4.0	6.0	20.73	1.05	131.1	
				As-Built Total:				**219.0**			**4767.8**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	271.0	3.60	975.6	Concrete, Int Insul, Exterior			5.0	1083.0	5.70		6173.1	
Exterior	1083.0	3.70	4007.1	Concrete, Int Insul, Adjacent			5.0	271.0	4.20		1138.2	
Base Total:	**1354.0**		**4982.7**	**As-Built Total:**				**1354.0**			**7311.3**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	21.0	11.50	241.5	Exterior Wood				21.0	12.30		258.3	
Exterior	21.0	12.30	258.3	Adjacent Wood				21.0	11.50		241.5	
Base Total:	**42.0**		**499.8**	**As-Built Total:**				**42.0**			**499.8**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X WCM =		Points	
Under Attic	1537.9	2.05	3152.7	Under Attic			30.0	1537.9	2.05 X 1.00		3152.7	
Base Total:	**1537.9**		**3152.7**	**As-Built Total:**				**1537.9**			**3152.7**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	147.0(p)	8.9	1308.3	Slab-On-Grade Edge Insulation			0.0	147.0(p	18.80		2763.6	
Raised	0.0	0.00	0.0									
Base Total:			**1308.3**	**As-Built Total:**				**147.0**			**2763.6**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	1538.0	-0.59	-907.4					1538.0	-0.59		-907.4	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**12563.0**	**Winter As-Built Points:**					**17587.8**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: PTHP 30000 btuh ,EFF(3.7) Ducts:Unc(S),Unc(R),Gar(AH),R6.0					
			17587.8	1.000	(1.069 x 1.169 x 1.00)	0.270	1.000	5940.2
12563.0	**0.6274**	**7882.0**	**17587.8**	**1.00**	**1.250**	**0.270**	**1.000**	**5940.2**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2635.00	7905.0	40.0	0.93	3	1.00	2606.67	1.00	7820.0	
				As-Built Total:						**7820.0**	

CODE COMPLIANCE STATUS

BASE					AS-BUILT				
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points		Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	
8823	**7882**	**7905**	**24610**		**6836**	**5940**	**7820**	**20596**	

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.8

The higher the score, the more efficient the home.

TAYLOR, , , ,

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	3	
5.	Is this a worst case?	No	
6.	Conditioned floor area (ft²)	1538 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
	a. U-factor: (or Single or Double DEFAULT)	7a. (Dble Default) 121.0 ft²	
	b. SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 219.0 ft²	
8.	Floor types		
	a. Slab-On-Grade Edge Insulation	R=0.0, 147.0(p) ft	
	b. N/A		
	c. N/A		
9.	Wall types		
	a. Concrete, Int Insul, Exterior	R=5.0, 1083.0 ft²	
	b. Concrete, Int Insul, Adjacent	R=5.0, 271.0 ft²	
	c. N/A		
	d. N/A		
	e. N/A		
10.	Ceiling types		
	a. Under Attic	R=30.0, 1537.9 ft²	
	b. N/A		
	c. N/A		
11.	Ducts		
	a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 50.0 ft	
	b. N/A		

12.	Cooling systems		
	a. Central Unit	Cap: 30.0 kBtu/hr SEER: 13.00	
	b. N/A		
	c. N/A		
13.	Heating systems		
	a. PTHP	Cap: 30.0 kBtu/hr COP: 3.70	
	b. N/A		
	c. N/A		
14.	Hot water systems		
	a. Electric Resistance	Cap: 40.0 gallons EF: 0.93	
	b. N/A		
	c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

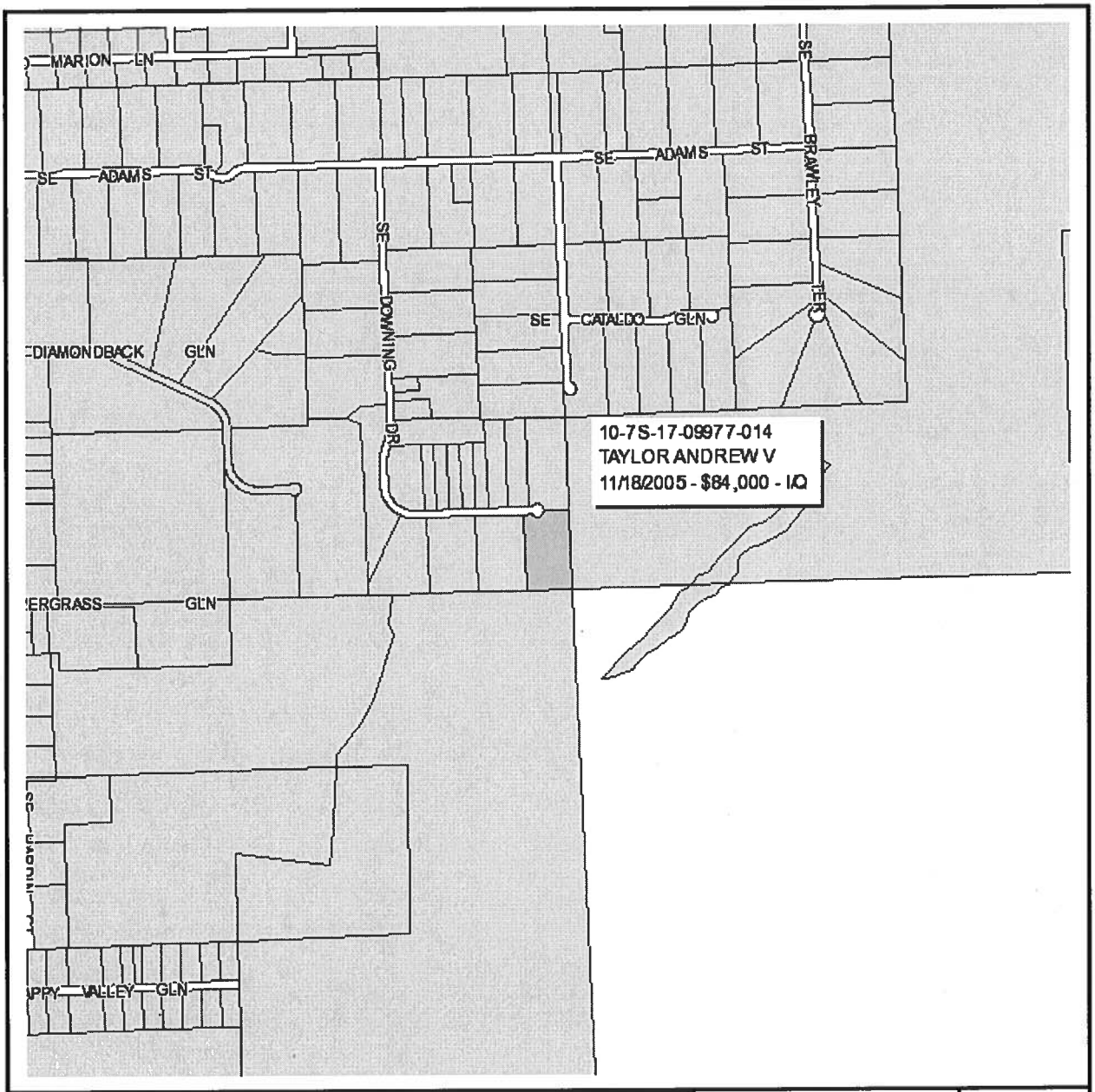
I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: 3/27/06

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.



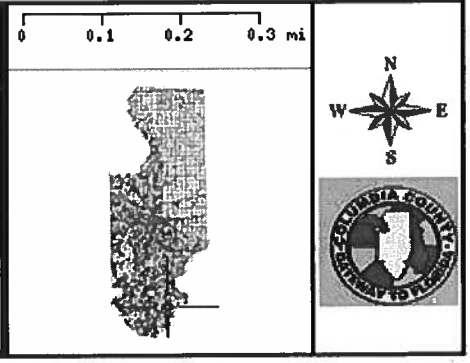
Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 10-7S-17-09977-014 - MOBILE HOM (000200)

BEG SE COR OF SEC, RUN W 364.28 FT, N 584.10 FT, E ALONG RD R/W 198.62 FT, E 200

Name:	TAYLOR ANDREW V	LandVal	$44,500.00
Site:	DOWNING	BldgVal	$7,674.00
Mail:	1075 WOODFIELD RD GREEN ACRES, FL 33415	ApprVal	$52,674.00
		JustVal	$52,674.00
Sales Info	11/18/2005 $84,000.00I / Q	Assd	$52,674.00
		Exmpt	$0.00
		Taxable	$52,674.00



This information, GIS Map Updated: 4/6/2006, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

PERMIT # 0604-24

PARCEL # 10-7S-17-09977-014

NOTICE OF COMMENCEMENT

STATE OF: FLORIDA COUNTY OF: Columbia CITY OF: _____

THE UNDERSIGNED hereby gives notice that improvement(s) will be made to certa real property, and in accordance with Chapter 713, Florida Statutes, th following information is provided in this Notice of Commencement.

DESCRIPTION OF PROPERTY:

LOT: 14 BLOCK: _____ SECTION: 10 TOWNSHIP: 7S RANGE: 17E

SUBDIVISION: _____ PLATBOOK: _____ MAP PAGE #: _____

STREET ADDRESS: 818 SE DOWNING DR. HIGH Springs, FL

GENERAL DESCRIPTION OF IMPROVEMENT:

TO CONSTRUCT: SINGLE FAMILY RESIDENCE

OWNER INFORMATION:

OWNER(S) NAME: ANDREW TAYLOR
ADDRESS: 1075 WOODFIELD RD PHONE NO.: 386 454 2476
CITY: Green Acres STATE: FL ZIP CODE: 33415
INTEREST IN THE PROPERTY: Owner
FEE SIMPLE TITLEHOLDER NAME: SAME
FEE SIMPLE TITLEHOLDER ADDRESS SAME
(if other than owner)

CONTRACTOR NAME: ONEIL Construction
ADDRESS: P.O. BOX 1633 PHONE NO.: 386 454 2476
CITY: HIGH Springs STATE: FL ZIP CODE: 32655

BONDING COMPANY: NONE
ADDRESS: —
CITY: — STATE: —

Inst:2006013934 Date:06/09/2006 Time:10:22
DC,P.DeWitt Cason,Columbia County B:1086 P:684

LENDER NAME: NONE
ADDRESS: —
CITY: — STATE: — ZIP CODE: —

Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes:
NAME: ANDREW TAYLOR ADDRESS: 1075 Woodfield Rd. Green Acres FL 33415
In addition to himself, Owner designates DENNIS ONEIL of ONEIL CONSTRUCTION to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

Expiration date is 1 year from date of recording unless a different date is specified.

SIGNATURE OF OWNER: Andrew Taylor

Sworn to and subscribed before me this 30 day of MAY A.D. 2006

Notary Public _____ My Commission Expires: MAY 2009

MARANTEL MACLEAN
Notary Public, State of Florida
My Commission Expires Mar 4, 2009
Bonded By National Notary Assn.

FLDLT460018611780

REVISED 5/2

STATE OF FLORIDA
DEPARTMENT OF HEALTH

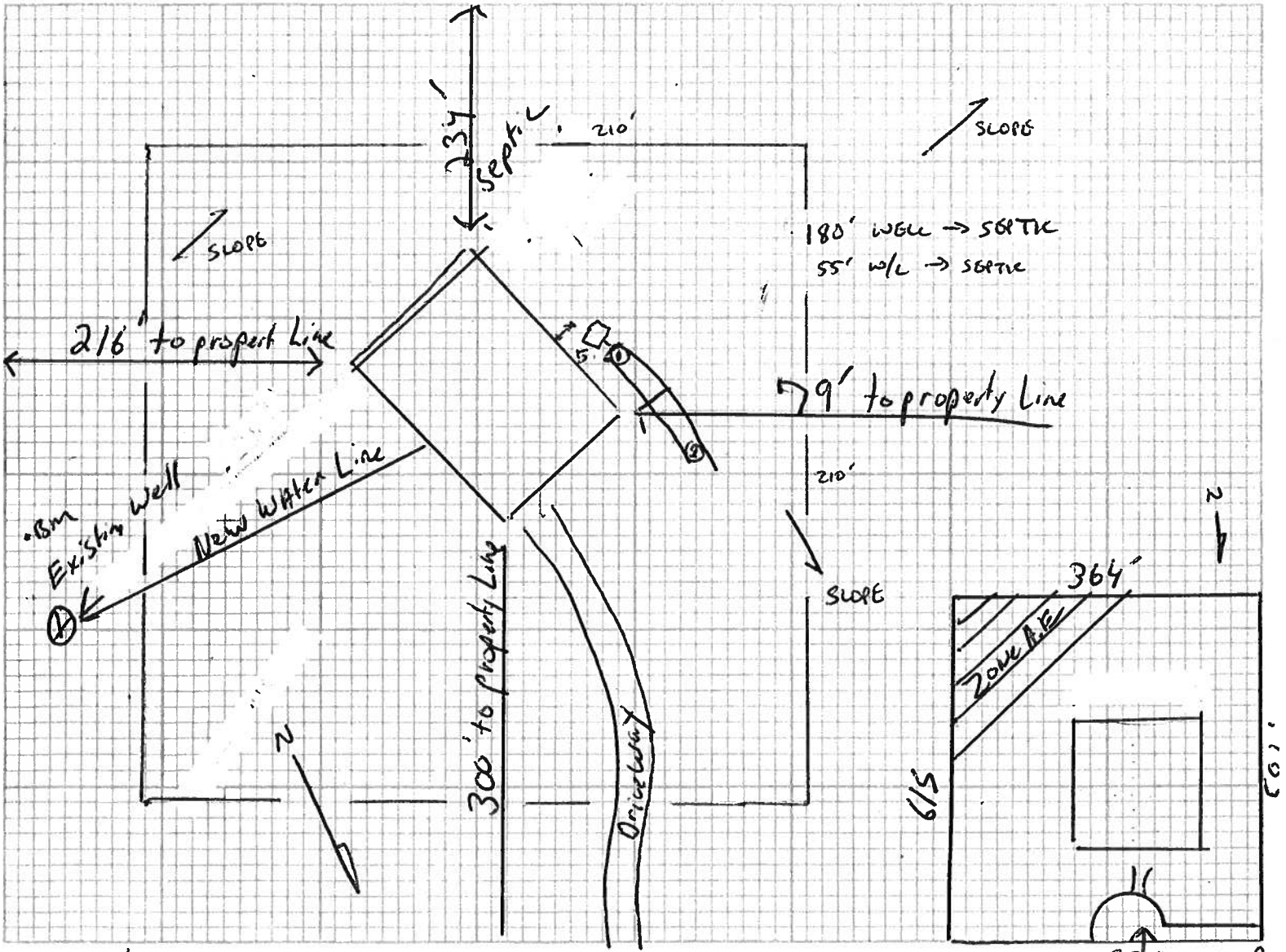
X _____

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-0329N

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes: this is one Acre of a 5 acre piece
see attached Survey

Site Plan submitted by: Dan O'Neil Constution (Signature) President (Title)

Plan Approved X Not Approved _____ Date 5/2/6

By _____ County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0604-24
O'Neil Construction Owner Andrew Taylor 818 SE Downing Drive

On the date of April 12, 2006 application 0604-24 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0604-24when making reference to this application.

1. In the master bath tub area please verify compliance with the FRC-2004 section R308.4 Hazardous locations: Glazing in doors and enclosures for hot tubs, whirlpools, saunas, steam rooms, bathtubs and showers. Glazing in any part of a building wall enclosing these compartments where the bottom exposed edge of the glazing is less than 60 inches (1524 mm) measured vertically above any standing or walking surface. Each pane of glazing installed in hazardous locations as defined in Section R308.4 shall

be provided with a manufacturer's or installer's label, designating the type and thickness of glass and the safety glazing standard with which it complies, which is visible in the final installation. The label shall be acid etched, sandblasted, ceramic-fired, embossed mark, or shall be of a type which once applied cannot be removed without being destroyed.

2. The door which enters the dwelling from the garage area shall comply with the FRC-2004 sections R309 Garage: R309.1 A: Opening protection: Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors.
3. The attic access opening (pull down ladder type attic egress door) in the garage ceiling shall have the same protection requirements as required in FRC-2004 C: R309.2 Separation required. The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side.
4. The electrical service location as shown on the plans does not show an overcurrent protection device. This device shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

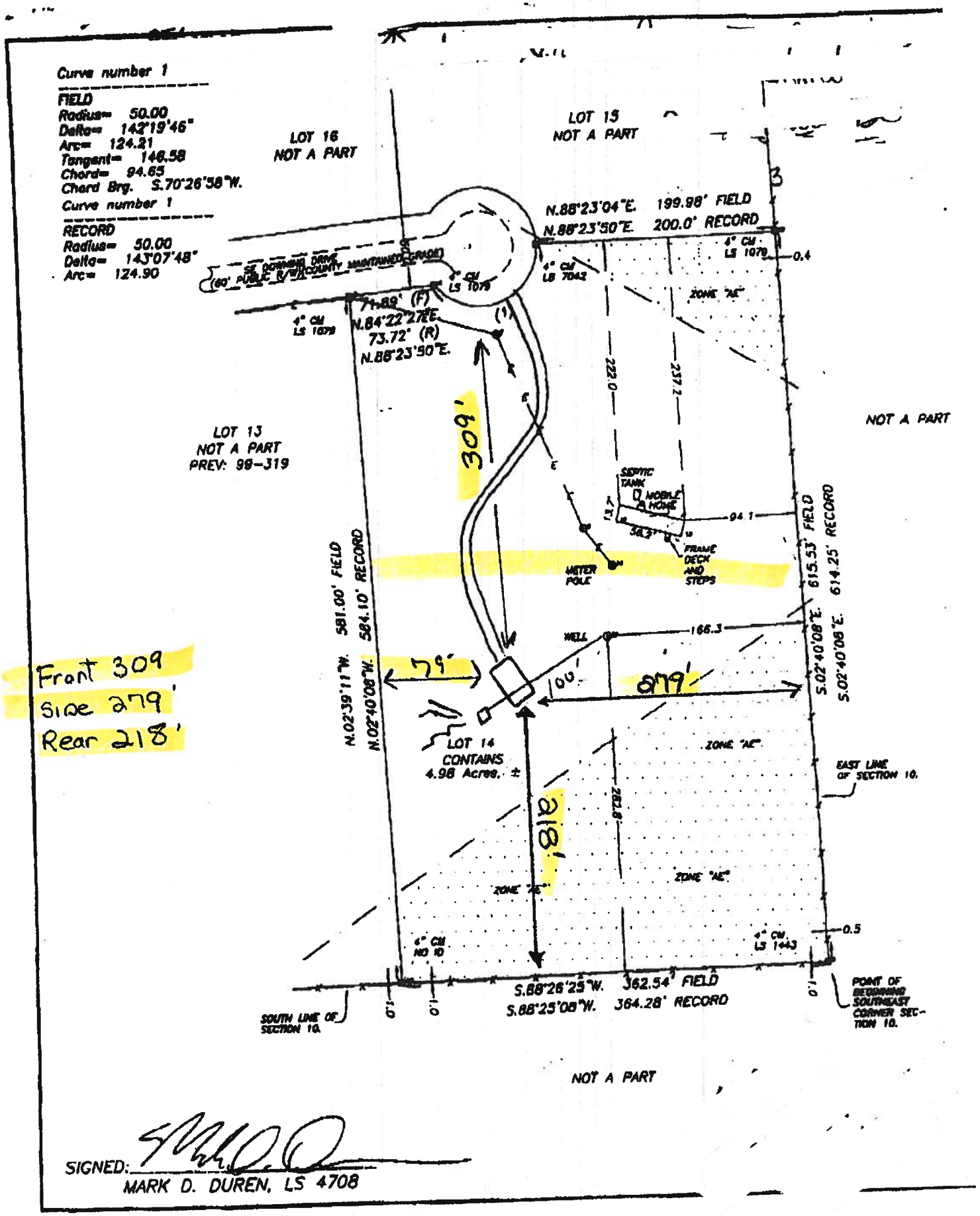
The air handling unit shown in the garage shall meet the requirements of the FRC-200-4 sections: R309.1.1 Duct penetration. Ducts in the garage and ducts penetrating the walls or ceilings separating the dwelling from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other approved material and shall have no openings into the garage.

5. The area within the garage which will house the washer, dryer and water heater shall comply the Florida Mechanical Code, Sections: 303.4: Protection from damage: Appliances shall not be installed in a location where subject to mechanical damage unless protected by approved barriers.

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

FROM :COLUMBIA CO BUILDING + ZONING FAX NO. :386-758-2160 Apr. 11 2006 01:13PM P1



Front 309
Side 279'
Rear 218'

SIGNED: _____
MARK D. DUREN, LS 4708

O'NEIL
CONSTRUCTION
Of High Springs, Inc.
PHONE (386) 454-2476 • FAX (386)454-4244

TAYLOR RESIDENCE
JOB #377

ADDRESS: 818 SE DOWNING DRIVE
HIGH SPRINGS, FLORIDA 32643
COLUMBIA COUNTY

FROM HIGH SPRINGS GO NORTH ON 441 APPROX 5 MILES TO ADANS STREET TURN RIGHT GO ½ MILE TO DOWNING DRIVE TURN RIGHT, FOLLOW TO END (4/10 MILE) PROPERTY ON RIGHT. SEE SIGN

FORM 600A-2004 EnergyGauge® 4.0

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	TAYLOR RESIDENCE	Builder:	O'NEIL CONSTRUCTION
Address:		Permitting Office:	COLUMBIA COUNTY
City, State:	,	Permit Number:	
Owner:	TAYLOR	Jurisdiction Number:	221000
Climate Zone:	North		

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	3	
5.	Is this a worst case?	No	
6.	Conditioned floor area (ft²)	1538 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
a.	U-factor: (or Single or Double DEFAULT)	7a. Description (Dble Default)	Area 121.0 ft²
b.	SHGC: (or Clear or Tint DEFAULT)	7b. (Clear)	219.0 ft²
8.	Floor types		
a.	Slab-On-Grade Edge Insulation	R=0.0, 147.0(p) ft	
b.	N/A		
c.	N/A		
9.	Wall types		
a.	Concrete, Int Insul, Exterior	R=5.0, 1083.0 ft²	
b.	Concrete, Int Insul, Adjacent	R=5.0, 271.0 ft²	
c.	N/A		
d.	N/A		
e.	N/A		
10.	Ceiling types		
a.	Under Attic	R=30.0, 1537.9 ft²	
b.	N/A		
c.	N/A		
11.	Ducts		
a.	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 50.0 ft	
b.	N/A		
12.	Cooling systems		
a.	Central Unit	Cap: 30.0 kBtu/hr SEER: 13.00	
b.	N/A		
c.	N/A		
13.	Heating systems		
a.	PTHP	Cap: 30.0 kBtu/hr COP: 3.70	
b.	N/A		
c.	N/A		
14.	Hot water systems		
a.	Electric Resistance	Cap: 40.0 gallons EF: 0.93	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.14

Total as-built points: 20596

Total base points: 24610

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Larry Resmondo a/c

DATE: 3-24-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: O'Neil Construction

DATE: 3/27/06

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1538.0	20.04	5547.9	Double, Clear	N	5.0	4.0	12.0	19.20	0.68	155.7	
				Double, Clear	W	1.5	6.0	20.0	38.52	0.91	703.7	
				Single, Clear	S	1.5	8.0	98.0	40.81	0.92	3681.8	
				Double, Clear	N	5.0	8.0	7.0	19.20	0.79	106.0	
				Double, Clear	W	1.5	5.0	16.0	38.52	0.88	539.7	
				Double, Clear	N	1.5	6.0	30.0	19.20	0.94	540.7	
				Double, Clear	E	1.5	6.0	30.0	42.06	0.91	1151.8	
				Double, Clear	W	1.5	4.0	6.0	38.52	0.81	188.1	
				As-Built Total:				**219.0**			**7067.4**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	271.0	0.70	189.7	Concrete, Int Insul, Exterior			5.0	1083.0	1.00		1083.0	
Exterior	1083.0	1.70	1841.1	Concrete, Int Insul, Adjacent			5.0	271.0	0.70		189.7	
Base Total:	**1354.0**		**2030.8**	**As-Built Total:**				**1354.0**			**1272.7**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	21.0	2.40	50.4	Exterior Wood				21.0	6.10		128.1	
Exterior	21.0	6.10	128.1	Adjacent Wood				21.0	2.40		50.4	
Base Total:	**42.0**		**178.5**	**As-Built Total:**				**42.0**			**178.5**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1537.9	1.73	2660.6	Under Attic			30.0	1537.9	1.73 X 1.00		2660.6	
Base Total:	**1537.9**		**2660.6**	**As-Built Total:**				**1537.9**			**2660.6**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	147.0(p)	-37.0	-5439.0	Slab-On-Grade Edge Insulation			0.0	147.0(p	-41.20		-6056.4	
Raised	0.0	0.00	0.0									
Base Total:			**-5439.0**	**As-Built Total:**				**147.0**			**-6056.4**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1538.0	10.21	15703.0					1538.0	10.21		15703.0	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 20681.7			**Summer As-Built Points:**					**20825.8**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 30000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)					
			20826	1.00	(1.09 x 1.147 x 1.00)	0.263	1.000	6835.7
20681.7	**0.4266**	**8822.8**	**20825.8**	**1.00**	**1.250**	**0.263**	**1.000**	**6835.7**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1538.0	12.74	3526.9	Double, Clear	N	5.0	4.0	12.0	24.58	1.02	301.1	
				Double, Clear	W	1.5	6.0	20.0	20.73	1.02	424.3	
				Single, Clear	S	1.5	8.0	98.0	20.24	1.04	2071.1	
				Double, Clear	N	5.0	8.0	7.0	24.58	1.01	174.2	
				Double, Clear	W	1.5	5.0	16.0	20.73	1.03	343.2	
				Double, Clear	N	1.5	6.0	30.0	24.58	1.00	739.1	
				Double, Clear	E	1.5	6.0	30.0	18.79	1.04	583.8	
				Double, Clear	W	1.5	4.0	6.0	20.73	1.05	131.1	
				As-Built Total:				**219.0**			**4767.8**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	271.0	3.60	975.6	Concrete, Int Insul, Exterior			5.0	1083.0	5.70		6173.1	
Exterior	1083.0	3.70	4007.1	Concrete, Int Insul, Adjacent			5.0	271.0	4.20		1138.2	
Base Total:	**1354.0**		**4982.7**	**As-Built Total:**				**1354.0**			**7311.3**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	21.0	11.50	241.5	Exterior Wood				21.0	12.30		258.3	
Exterior	21.0	12.30	258.3	Adjacent Wood				21.0	11.50		241.5	
Base Total:	**42.0**		**499.8**	**As-Built Total:**				**42.0**			**499.8**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1537.9	2.05	3152.7	Under Attic			30.0	1537.9	2.05 X	1.00	3152.7	
Base Total:	**1537.9**		**3152.7**	**As-Built Total:**				**1537.9**			**3152.7**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	147.0(p)	8.9	1308.3	Slab-On-Grade Edge Insulation			0.0	147.0(p	18.80		2763.6	
Raised	0.0	0.00	0.0									
Base Total:			**1308.3**	**As-Built Total:**				**147.0**			**2763.6**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	1538.0	-0.59	-907.4					1538.0	-0.59		-907.4	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**12563.0**	**Winter As-Built Points:**					**17587.8**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: PTHP 30000 btuh ,EFF(3.7) Ducts:Unc(S),Unc(R),Gar(AH),R6.0					
			17587.8	1.000	(1.069 x 1.169 x 1.00)	0.270	1.000	5940.2
12563.0	**0.6274**	**7882.0**	**17587.8**	**1.00**	**1.250**	**0.270**	**1.000**	**5940.2**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT						
WATER HEATING										
Number of Bedrooms	X Multiplier	= Total		Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3	2635.00	7905.0		40.0	0.93	3	1.00	2606.67	1.00	7820.0
				As-Built Total:						**7820.0**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
8823	**7882**	**7905**	**24610**	**6836**	**5940**	**7820**	**20596**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.8

The higher the score, the more efficient the home.

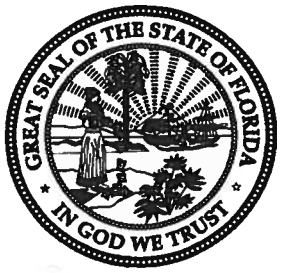
TAYLOR, , , ,

1.	New construction or existing	New		12. Cooling systems	
2.	Single family or multi-family	Single family		a. Central Unit	Cap: 30.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 13.00
4.	Number of Bedrooms	3		b. N/A	
5.	Is this a worst case?	No			
6.	Conditioned floor area (ft²)	1538 ft²		c. N/A	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)				
	a. U-factor:	Description Area		13. Heating systems	
	(or Single or Double DEFAULT)	7a. (Dble Default) 121.0 ft²		a. PTHP	Cap: 30.0 kBtu/hr
	b. SHGC:				COP: 3.70
	(or Clear or Tint DEFAULT)	7b. (Clear) 219.0 ft²		b. N/A	
8.	Floor types				
	a. Slab-On-Grade Edge Insulation	R=0.0, 147.0(p) ft		c. N/A	
	b. N/A				
	c. N/A			14. Hot water systems	
9.	Wall types			a. Electric Resistance	Cap: 40.0 gallons
	a. Concrete, Int Insul, Exterior	R=5.0, 1083.0 ft²			EF: 0.93
	b. Concrete, Int Insul, Adjacent	R=5.0, 271.0 ft²		b. N/A	
	c. N/A				
	d. N/A			c. Conservation credits	
	e. N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
	a. Under Attic	R=30.0, 1537.9 ft²		15. HVAC credits	
	b. N/A			(CF-Ceiling fan, CV-Cross ventilation,	
	c. N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
	a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 50.0 ft		MZ-C-Multizone cooling,	
	b. N/A			MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: 3/27/06

Address of New Home: _____ City/FL Zip: _____



**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

RESIDENTIAL WIND DESIGN & ANALYSIS

NO COPIES ARE TO BE PERMITTED \ FBC2004

PREPARED FOR:

THE TAYLOR RESIDENCE

PREPARED BY:

MARTY R. ESKRIDGE
14952 MAIN ST
ALACHUA FL 32615
386-462-1340 / 352-375-6329

MARTY R. ESKRIDGE & ASSOCIATES

14952 MAIN ST ALACHUA FL 32615 PH: 386-462-1340

March 22, 2006

SUMMARY: Wind Load Analysis for The Taylor Residence
Wind Speed: 110 M.P.H. \ No Copies Permitted \ Florida Building Code \ Latest Edition

Foundation:
20" wide x 10" deep stemwall footing with (2) #5 rebar continuous minimum. CMU walls must have #5 dowels at 72" o.c. with a standard 90 degree ACI hook in footing and a 4" slab on grade. Monolithic slab to be 12" wide x 20" deep minimum with (2) #5 rebar continuous with 12" minimum coverage on face of foundation. It is assumed that ideal soil conditions and pad preparation are provided.

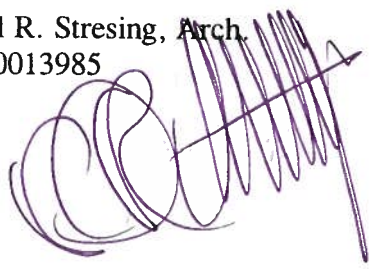
Walls:
8" CMU block with vertical #5 reinforcing bar in grout filled cell at 72" o.c. maximum spacing. Wall heights are 8' nominal. Provide an 8" x 8" bond beam with 1-#5 rebar horizontal continuous at the top course. Install pre-cast, pre-engineered lintels or pre-engineered steel lintels spanning over all openings. One #5 rebar each corner. One #5 rebar each side of door and window openings. Two #5 rebar in openings wider than 12'-0". One #5 rebar where girders or girder trusses bear on masonry wall.

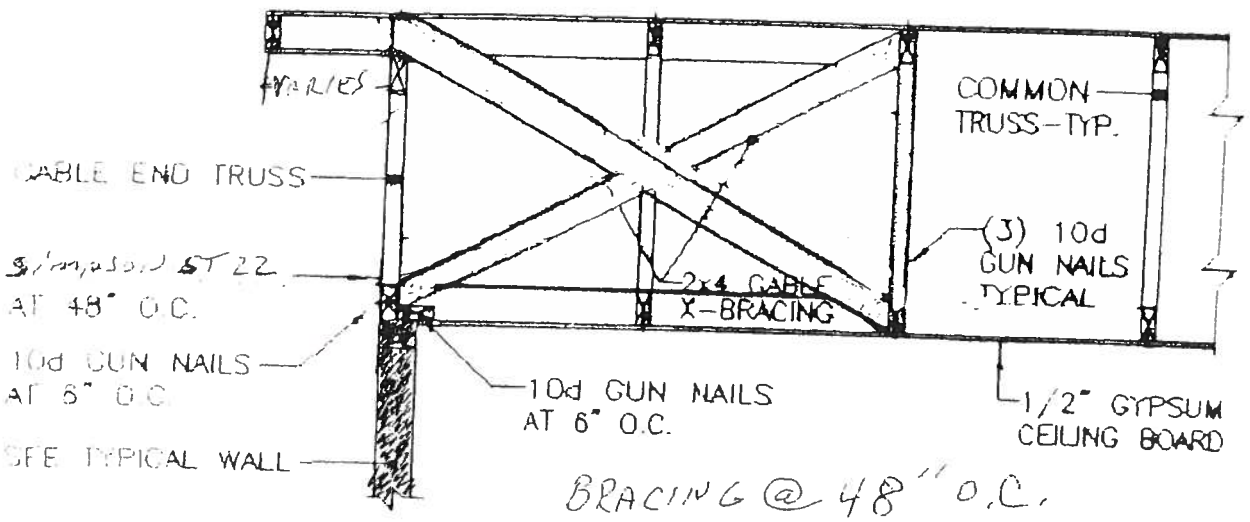
Shearwalls:
Transverse: 39'-0"
Longitudinal: 47'-0"

Trusses:
Pre-engineered Pre-fabricated with the bracing system designed by the manufacturer. Trusses must be anchored according to the truss engineering. Trusses must bear on all exterior walls and then porch headers.

Roof Sheathing:
7/16" osb minimum attached to the top chords of the trusses with 8d/131 gauge nails spaced at 4" o.c. edges and 8" interior.

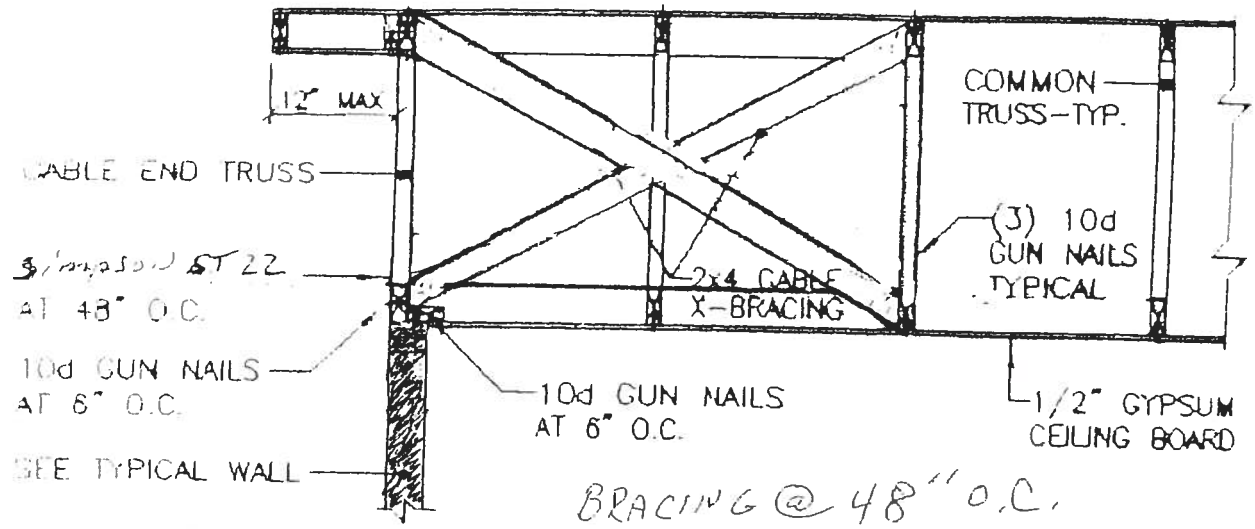
Paul R. Stresing, Arch.
AR0013985





GABLE END DETAIL

SCALE: NTS



GABLE END DETAIL

SCALE: NTS

TAYLOR
AM013985
3/24/06

ASCE 7-98

3/22/06

Wind Load Design per ASCE 7-98

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	22.6	Deg
Type of Roof	Hipped	
Eave Height (Eht)	8.00	ft
Ridge Height (RHt)	16.62	ft
Mean Roof Height (Ht)	12.38	ft
Width Perp. to Wind (B)	36.67	ft
Width Parallel to Wind (L)	54.00	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.34
Flexible Structure	No

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	0.9139	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8742	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	V*(5280/3600)	161.33	ft/s
Vzm	bm*(zm/33)^Am*Vhref	70.89	ft/s
NF1	NatFreq*Lzm/Vzm	4.37	Hz
Rn	(7.47*NF1)/(1+10.302*NF1)^1.667	0.0552	
Nh	4.6*NatFreq*Ht/Vzm	0.80	
Nb	4.6*NatFreq*B/Vzm	2.38	
Nd	15.4*NatFreq*Depth/Vzm	11.73	
Rh	1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))	0.6254	
Rb	1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))	0.3327	
Rd	1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))	0.0816	
RR	((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^0.5	0.8078	
gg	+(2*LN(3600*n1))^0.5+0.577/(2*LN(3600*n1))^0.5	4.19	
Gust3	0.925*((1+1.7*Izm*(3.4^2*Q^2+GG^2*RR^2)^0.5)/(1+1.7*3.4*Izm))	1.13	

Gust Factor Summary			
Main Wind-force resisting system:		**Components and Cladding:**	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.87	Gust Factor (G)	0.87

ASCE 7-98

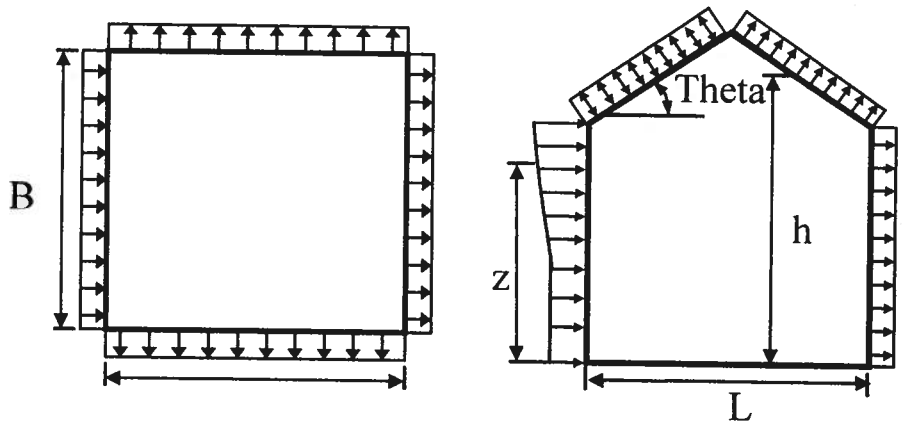
Wind Load Design per ASCE 7-98

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev. ft	Kz	Kzt	Kd 1.00	qz lb/ft^2	Pressure (lb/ft^2) Windward Wall* +GCpi	-GCpi
16.62	0.70	1.00	1.00	21.70	11.97	18.38
15	0.70	1.00	1.00	21.70	11.97	18.38

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(15/zg)^(2/Alpha)	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*ImpFac*Kh*Kht*Kd	17.80	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf) +GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 36.67 ft wall)	-0.41	-9.52	-3.11
Leeward Walls (Wind Dir Parallel to 54 ft wall)	-0.50	-10.99	-4.58
Side Walls	-0.70	-14.10	-7.69
Roof - Normal to Ridge (Theta>=10)			
Windward - Max Negative	-0.25	-7.06	-0.66
Windward - Max Positive	0.25	0.72	7.13
Leeward Normal to Ridge	-0.60	-12.54	-6.13
Overhang Top	-0.25	-3.86	-3.86
Overhang Bottom	0.80	0.70	0.70
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 6.19 ft	-0.90	-17.21	-10.80
Dist from Windward Edge: 6.19 ft to 12.38 ft	-0.90	-17.21	-10.80
Dist from Windward Edge: 12.38 ft to 24.76 ft	-0.50	-10.99	-4.58

ASCE 7-98

3/22/06

Wind Load Design per ASCE 7-98

Dist from Windward Edge: > 24.76 ft	-0.30	-7.87	-1.46

* Horizontal distance from windward edge

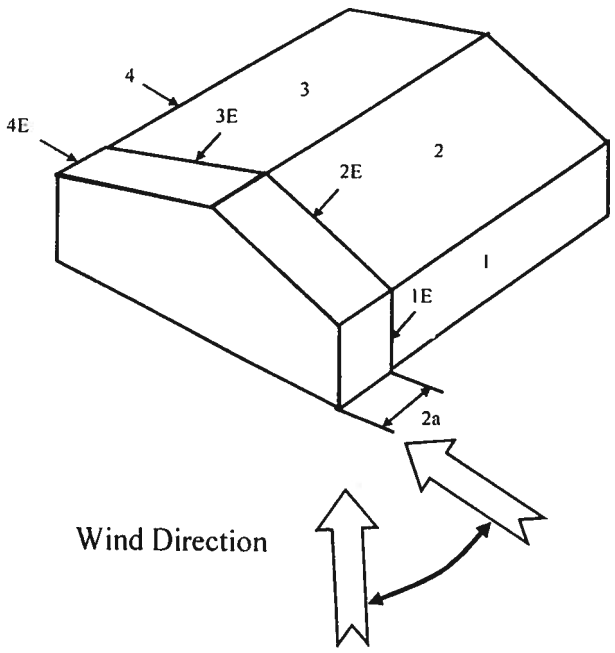
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

Case A						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	0.54	0.18	-0.18	21.70	7.76	15.58
2	-0.46	0.18	-0.18	21.70	-13.80	-5.99
3	-0.47	0.18	-0.18	21.70	-14.04	-6.23
4	-0.41	0.18	-0.18	21.70	-12.90	-5.09
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.77	0.18	-0.18	21.70	12.83	20.65
2E	-0.72	0.18	-0.18	21.70	-19.57	-11.75
3E	-0.65	0.18	-0.18	21.70	-17.98	-10.16
4E	-0.60	0.18	-0.18	21.70	-16.89	-9.08
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

* p = qh * (GCpf - GCpi)



ASCE 7-98

Wind Load Design per ASCE 7-98

Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

* p = qh * (GCpf - GCpi)

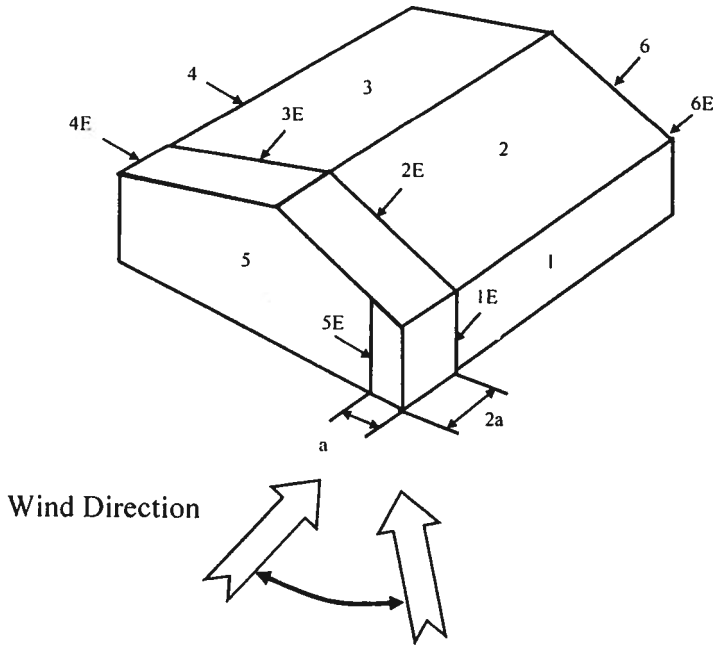
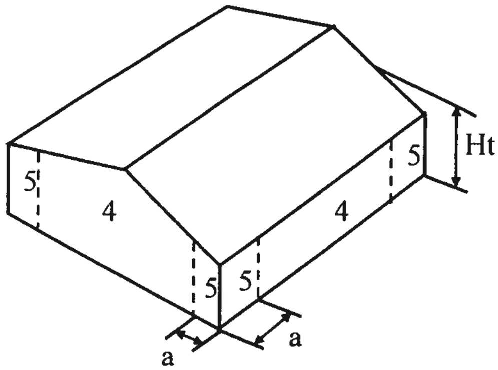


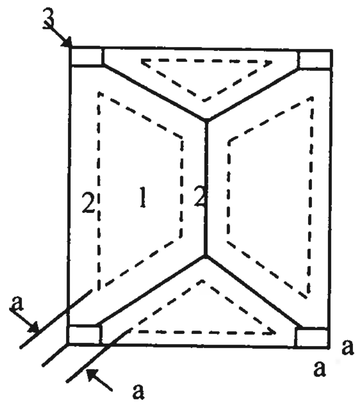
Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft

ASCE 7-98

Wind Load Design per ASCE 7-98





Hipped Roof
10 < Theta <= 30

a = 3.667 ==> 3.67 ft

Component	Width (ft)	Length (ft)	Area (ft^2)	Zone	GCp Max	GCp Min	Wind Press (lb/ft^2) Max	Wind Press (lb/ft^2) Min
	16	7	112.00	5	0.81	-1.03	17.71	-21.53
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
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	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

Condition	Gcpi Max +	Gcpi Max -

Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

Table 6-8 External Pressure Coefficients for Arched Roofs, Cp

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	Cp		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Cp	0.13	-1	-0.5
	P (+GCpi) - psf	-1.26	-18.77	-10.99
	P (-GCpi) -psf	5.15	-12.36	-4.58
Roof Springing from Ground	Cp	0.42	-1	-0.5
	P (+GCpi) - psf	3.33	-18.77	-10.99
	P (-GCpi) -psf	3.33	-18.77	-10.99

Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf

Variable	Description	Value	
L	Roof dimension normal to wind direction	54.00	ft
B	Roof dimension parallel to wind direction	36.67	ft
L/B	Ratio of L to B	1.473	
Theta	Slope of Roof	22.6	Deg
Cf	Force Coefficient	0.95	
X	Distance to center of pressure from windward edge	0.34	ft

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Flange
TYPE: Aluminum Single Hung Window

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	12 lb max.
Air Infiltration	0.14 cfm/ft^2
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.02 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:nlb

Allen N. Reeves
8 APRIL 2002

ALLEN N. REEVES
CERTIFICATE
NO. 12254
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.02
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Flange, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description

Series/Model: 650 Flange

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allen H. Reeves
4 APRIL 2002

ALLEN H. REEVES
No. 41059
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper	1	Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

Allen N. Reeves
4 APRIL 2002

ALLEN N. REEVES
CERTIFICATE NO. 15334
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 3" installation screws through the jambs. The installation screws were located 3" from the head and sill and one midspan on both jambs. The exterior was sealed with polyurethane.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	12 lbs	30 lbs max.
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.14 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
@ 25.9 psf (positive)	0.45"*	0.26" max.
@ 34.7 psf (negative)	0.53"*	0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.02"	0.18" max.
	@ 52.1 psf (negative)	0.01"	0.18" max.

Allen N. Reeves
* APRIL 2002

ALLEN N. REEVES
CERTIFICATE NO. 19264
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987)		
	In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.55"*	0.26" max.
	@ 47.2 psf (negative)	0.64"*	0.26" max.
4.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.04"	0.18" max.
	@ 70.8 psf (negative)	0.05"	0.38" max.

Exceeds L/175 for deflection, but passes all other test requirements.

Allen N. Reeves
4 APRIL 2002

ALLEN N. REEVES
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

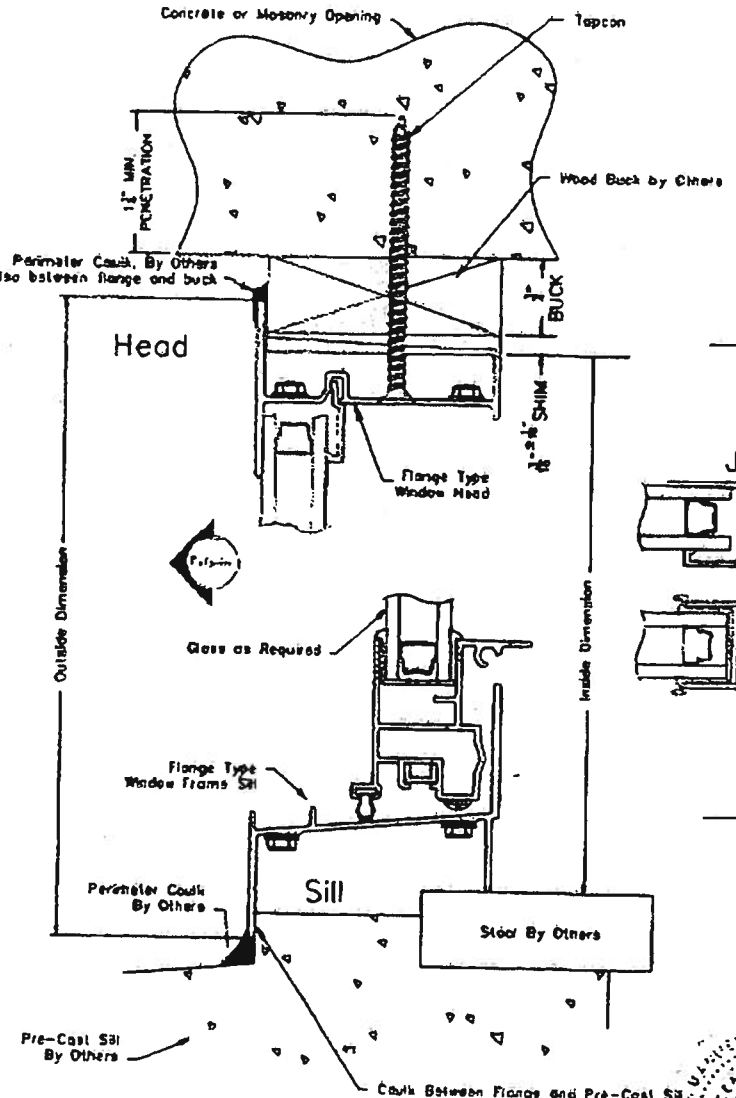
For ARCHITECTURAL TESTING, INC:

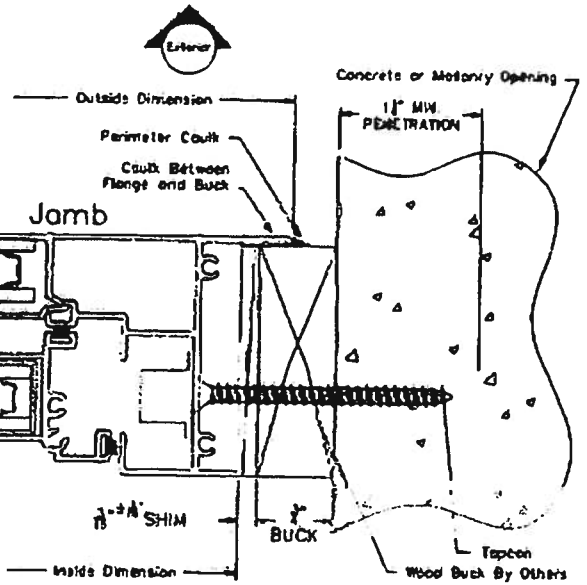
Mark A. Hess
Technician

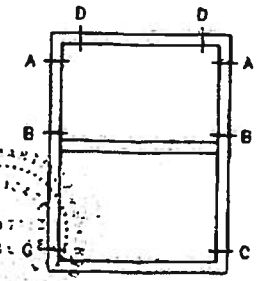
Allen N. Reeves, P.E.
Director - Engineering Services
4 APRIL 2002

MAH:nlb
01-41134.02

ALLEN N. REEVES
CERTIFICATE NO. 17554
STATE OF FLORIDA
REGISTERED PROFESSIONAL ENGINEER







1. Shim as required at each installation anchor as shown.
2. Anchor must be of a length to have 1 1/4" penetration into masonry or concrete.
3. Caulk between window flange and buck.
4. Caulk full perimeter of window.
5. If exact window size is not given, use anchor quantity for next larger window in chart.
6. Glass thickness will vary with window size and design load.
7. Letter designations on the tapcon location chart indicate where anchors are to be installed using the elevation as a key.
8. All factory applied holes not designated for tapcon anchor should be filled with #8 screws of sufficient length to provide min. 5/8" embedment into wood buck.

TAPCON * LOCATION CHART			
CODE SIZE	WINDOW ID SIZE	FASTENER LOCATIONS	
		UP TO DP35	DP35.1 TO DP50
12	18 1/8 x 25	C, D	C, D
13	18 1/8 x 37 3/8	C, D	C, D
14	18 1/8 x 49 5/8	C, D	C, D
15	18 1/8 x 62	C, D	C, D
16	18 1/8 x 71 1/4	C, D	C, D
1/2 32	25 1/2 x 25	C, D	C, D
1/2 33	25 1/2 x 37 3/8	C, D	C, D
1/2 34	25 1/2 x 49 5/8	C, D	C, D
1/2 35	25 1/2 x 62	C, D	C, D
1/2 36	25 1/2 x 71 1/4	C, D	B, C, D
22	36 x 25	C, D	C, D
23	36 x 37 3/8	C, D	C, D
24	36 x 49 5/8	C, D	C, D
245	36 x 55 1/4	C, D	B, C, D
25	36 x 62	C, D	B, C, D
26	36 x 71 1/4	B, C, D	B, C, D
32	52 1/8 x 25	B, C, D	B, C, D
33	52 1/8 x 37 3/8	B, C, D	B, C, D
34	52 1/8 x 49 5/8	B, C, D	B, C, D
345	52 1/8 x 55 1/4	B, C, D	B, C, D
35	52 1/8 x 62	B, C, D	B, C, D
36	52 1/8 x 71 1/4	B, C, D	A, B, C, D

TAPCON * LOCATION CHART			
CODE SIZE	WINDOW ID SIZE	FASTENER LOCATIONS	
		UP TO DP35	DP35.1 TO DP50
2'-0" x 3'-0"	23 1/8 x 35 5/8	C, D	C, D
2'-0" x 4'-0"	23 1/8 x 47 5/8	C, D	C, D
3'-0" x 3'-0"	35 1/8 x 35 5/8	C, D	C, D
3'-0" x 4'-0"	35 1/8 x 47 5/8	C, D	C, D
3'-0" x 4'-4"	35 1/8 x 51 5/8	C, D	B, C, D
3'-0" x 5'-0"	35 1/8 x 59 5/8	C, D	B, C, D
3'-0" x 6'-0"	35 1/8 x 71 5/8	B, C, D	B, C, D
3'-4" x 3'-0"	39 1/8 x 35 5/8	C, D	C, D
3'-4" x 4'-0"	39 1/8 x 47 5/8	C, D	B, C, D
3'-4" x 4'-4"	39 1/8 x 51 5/8	C, D	B, C, D
3'-4" x 5'-0"	39 1/8 x 59 5/8	C, D	B, C, D
3'-4" x 6'-0"	39 1/8 x 71 5/8	B, C, D	B, C, D
3'-8" x 4'-0"	43 1/8 x 47 5/8	C, D	B, C, D
3'-8" x 4'-4"	43 1/8 x 51 5/8	C, D	B, C, D
3'-8" x 5'-0"	43 1/8 x 59 5/8	C, D	B, C, D
3'-8" x 6'-0"	43 1/8 x 71 5/8	B, C, D	B, C, D
4'-0" x 4'-0"	47 1/8 x 47 5/8	C, D	B, C, D
4'-8" x 4'-4"	47 1/8 x 51 5/8	C, D	B, C, D
4'-0" x 5'-0"	47 1/8 x 59 5/8	B, C, D	B, C, D
4'-0" x 6'-0"	47 1/8 x 71 5/8	B, C, D	B, C, D

*"TAPCON" TYPE HARDENED MASONRY SCREWS INCLUDE TAPCON, RAWL, & SIMPSON

05-2-02

INSTALLATION INSTRUCTIONS & FASTENER SCHEDULE

650 FLANGE SINGLE HUNG

DRAWN BY	DATE
Tony C	02/28/02
CHECKED	DATE
SCALE	SHEET
NONE	1 of 1
DWG NO	REV
	-

STRUCTURAL VERTICAL MULLION - FLANGE type

MULLION PART # CM–65129

650 SH / PW

STEP 1. Caulk inside mull grooves as shown to seal frame jambs.

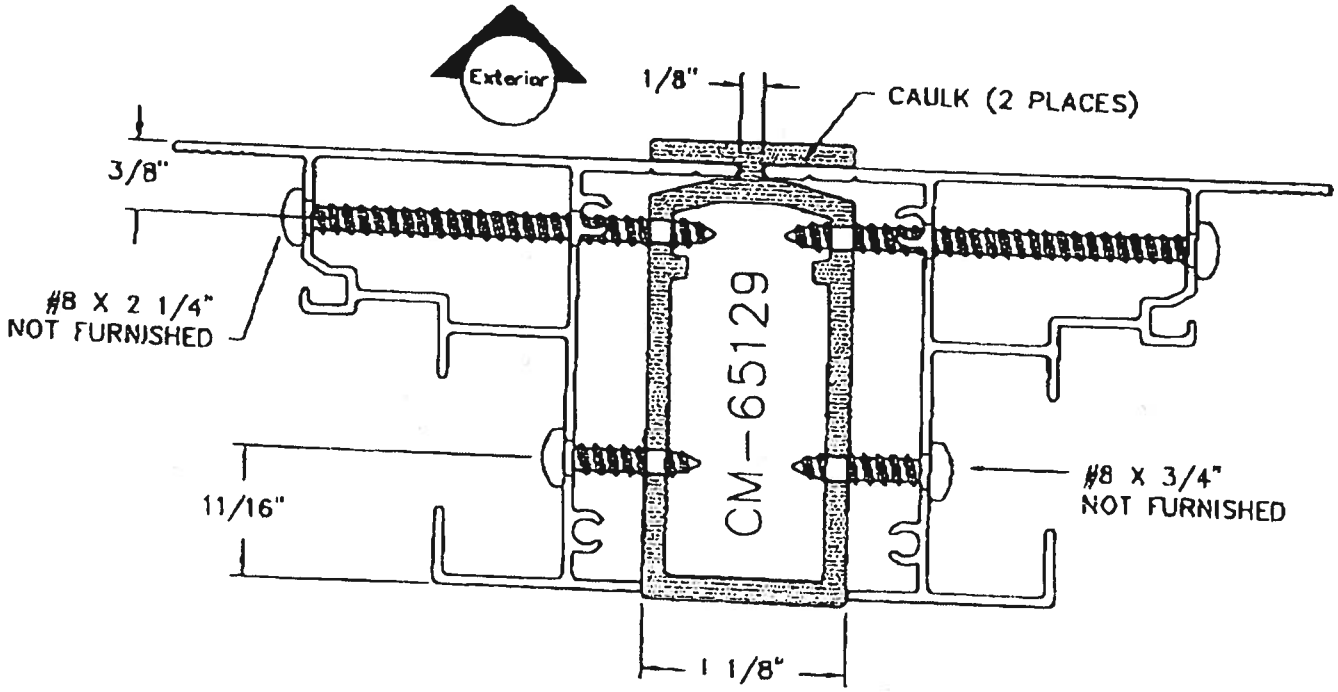
STEP 2. Place windows and mullions together as shown.

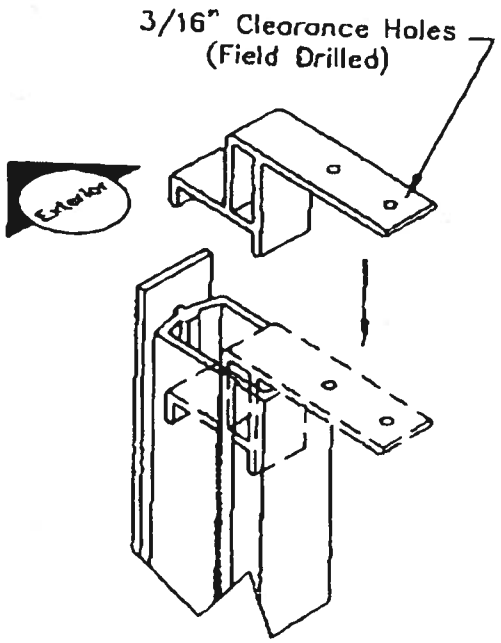
STEP 3. Using holes in jamb as a guide, drill 1/8" dia. holes through mullion.

STEP 4. Attach windows to mullions using #8 x 2 1/4" screws through holes in exterior track and #8 x 3/4" screws through holes in interior track. (Screws not included) To avoid distortion, do not over tighten screws.

STEP 5. Caulk any voids to prevent any leakage.

STEP 6. Before lifting into rough opening, drill two holes in each clip and insert into each end of mull as shown below with tab pointing to inside. Fasten each clip tab to construction with two #10 x 1 1/2" screws for structural integrity.

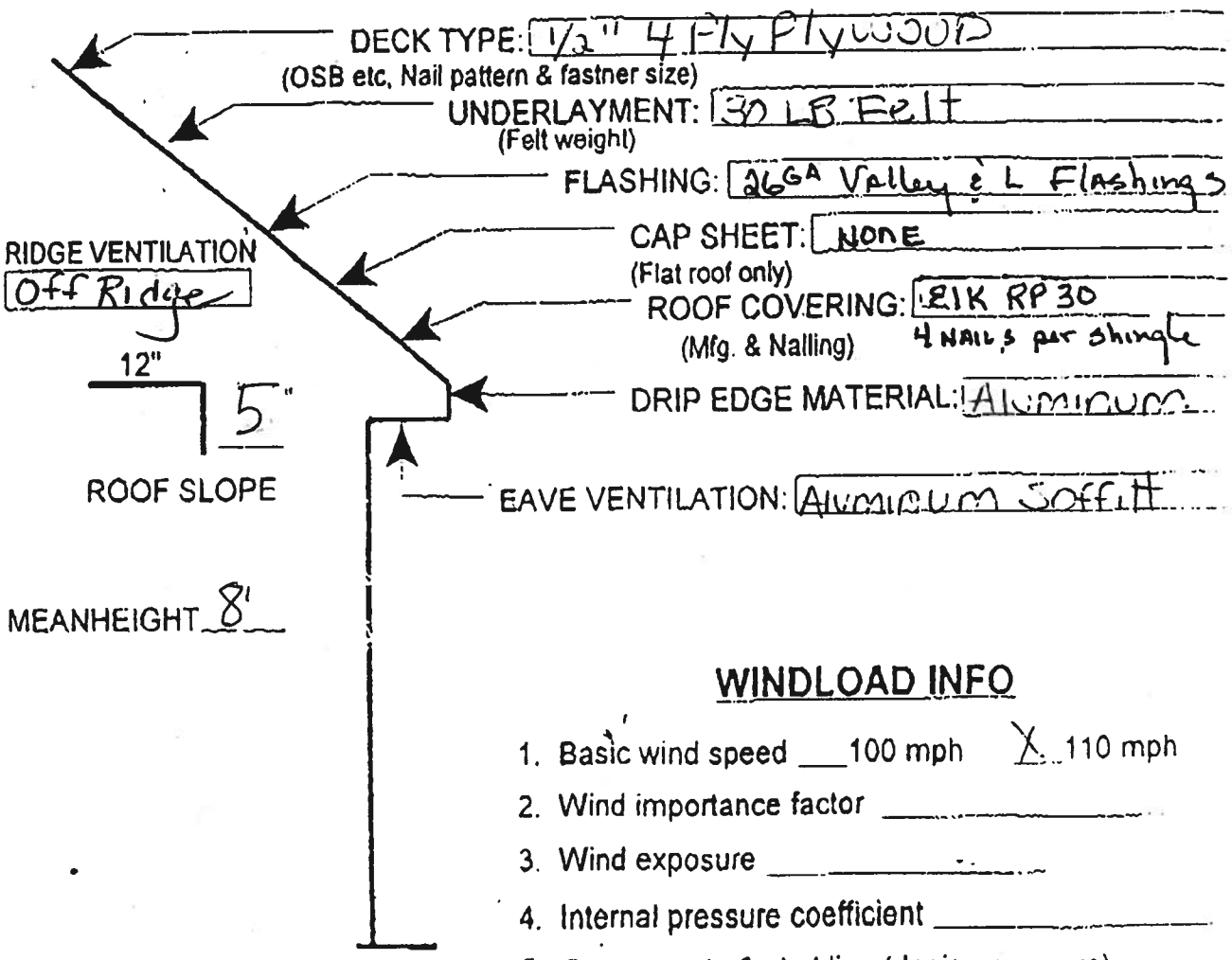




Attach clip to construction using #10 x 1 1/2" screws as described on reverse side of sheet.

M.I. Home Products

ROOF SYSTEM DESCRIPTION



WINDLOAD INFO

1. Basic wind speed ___100 mph X 110 mph
2. Wind importance factor _____
3. Wind exposure _____
4. Internal pressure coefficient _____
5. Components & cladding (design pressure)_____

DUCT SYSTEM SUMMARY
Entire House
LARRY RESMONDO A/C

Job: TAYLOR RESIDENCE
3/24/06

715 NW 1ST AVE, HIGH SPRINGS, FL 32643 Phone: 386-454-4433 Fax: 386-454-8843 Email: resmondoac@netcommander.com

Project Information

For: O'NEIL CONSTRUCTION
P.O. BOX 1633, HIGH SPRINGS, FL 32655
Phone: 386-454-2476 Fax: 386-454-4244

	HEATING	COOLING
External Static Pressure:	***0.10*** in H2O	***0.10*** in H2O
Pressure Losses:	0.25 in H2O	0.25 in H2O
Available Static Pressure:	-0.2 in H2O	-0.2 in H2O
Friction Rate:	***0.100*** in/100ft	***0.100*** in/100ft
Actual AVF:	1150 cfm	1150 cfm

Total Effective Length (TEL): 230 ft

Supply Branch Detail Table

Name	Htg (Btuh)	Clg (Btuh)	Htg (cfm)	Clg (cfm)	Dsn FR	Vel (fpm)	Dia (in)	Rect Sz (in)	Duct Matl	Trnk
WHOLE HOUSE	4715	3650	192	192	0.100	434	9	0x 0	VlFx	st1A
WHOLE HOUSE-A	4714	3648	192	192	0.100	434	9	0x 0	VlFx	st1
WHOLE HOUSE-B	4714	3648	192	192	0.100	434	9	0x 0	VlFx	st1
WHOLE HOUSE-C	4714	3648	192	192	0.100	434	9	0x 0	VlFx	st1
WHOLE HOUSE-D	4714	3648	192	192	0.100	434	9	0x 0	VlFx	st1
WHOLE HOUSE-E	4714	3648	192	192	0.100	434	9	0x 0	VlFx	st1

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Vel (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	1150	1150	824	16	0 x 0	RectFbg	
st1A	Peak AVF	192	192	549	8	0 x 0	RectFbg	st1

Return Branch Detail Table

Name	Diffus Sz (in)	Htg (Btuh)	Clg (Btuh)	Htg (cfm)	Clg (cfm)	Dsn FR	Vel (fpm)	Dia (in)	Rect Sz (in)	Duct Matl	Trunk
rb1	0 x 0	28286	21892	1150	1150	0.100	651	18	0x 0	VlFx	

Bold/italic values have been manually overridden

RIGHT-J BUILDING ANALYSIS REPORT
Entire House
LARRY RESMONDO A/C

Job: TAYLOR RESIDENCE
3/24/06

715 NW 1ST AVE, HIGH SPRINGS, FL 32643 Phone: 386-454-4433 Fax: 386-454-8843 Email: resmondoac@netcommander.com

Project Information

For: O'NEIL CONSTRUCTION
P.O. BOX 1633, HIGH SPRINGS, FL 32655
Phone: 386-454-2476 Fax: 386-454-4244

Design Information

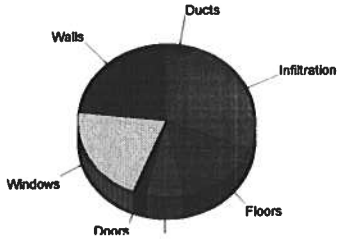
	Htg	Clg
Outside db (°F)	33	92
Inside db (°F)	70	75
Design TD (°F)	37	17
Daily range	-	M
Inside humidity (%)	-	50
Moisture difference (gr/lb)	-	52

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

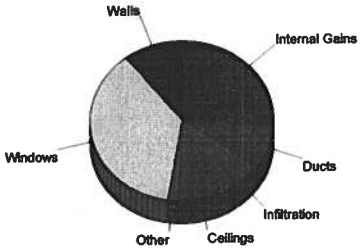
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	4.8	6551	23.2
Windows	26.8	5863	20.7
Doors	17.0	715	2.5
Ceilings	1.2	1878	6.6
Floors	30.0	4406	15.6
Infiltration	28.8	7526	26.6
Ducts		1347	4.8
Total		**28285**	**100.0**



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	1.7	2289	10.5
Windows	37.0	8110	37.0
Doors	9.5	398	1.8
Ceilings	1.1	1675	7.7
Floors	0.0	0	0.0
Infiltration	6.6	1729	7.9
Ducts		1990	9.1
Internal gains		5700	26.0
Total		**21891**	**100.0**



Cooling at 85 % SHR = 2.1 ton
Cooling at 70 % SHR = 2.5 ton
Cooling air flow = 548 cfm/ton
Cooling at 400 cfm/ton = 2.9 ton
Overall U-Value = 0.164 Btuh/ft²-°F

Data entries checked.

RIGHT-J SHORT FORM
Entire House

LARRY RESMONDO A/C

715 NW 1ST AVE, HIGH SPRINGS, FL 32643 Phone: 386-454-4433 Fax: 386-454-8843 Email: resmondoac@netcommander.com

Job: TAYLOR RESIDENCE
3/24/06

Project Information

For: O'NEIL CONSTRUCTION
P.O. BOX 1633, HIGH SPRINGS, FL 32655
Phone: 386-454-2476 Fax: 386-454-4244

Design Information

	Htg	Clg
Outside db (°F)	33	92
Inside db (°F)	70	75
Design TD (°F)	37	17
Daily range	-	M
Inside humidity (%)	-	50
Moisture difference (gr/lb)	-	52

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

HEATING EQUIPMENT

Make RUUD AIR COND
Trade Ruud UPMB Series
UPMB-030JA

Efficiency	3.7 HSPF	
Heating input	0	Btuh
Heating output	0	Btuh
Heating temperature rise	0	°F
Actual heating fan	1150	cfm
Heating air flow factor	0.041	cfm/Btuh

Space thermostat

COOLING EQUIPMENT

Make RUUD AIR COND
Trade Ruud UPMB Series
UPMB-030JA
UBHJ-21+RCHJ-36A1

Efficiency	13.0 SEER	
Sensible cooling	19600	Btuh
Latent cooling	8400	Btuh
Total cooling	28000	Btuh
Actual cooling fan	1150	cfm
Cooling air flow factor	0.053	cfm/Btuh
Load sensible heat ratio	85	%

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
WHOLE HOUSE	1538	28285	21891	1150	1150
Entire House d	1538	28285	21891	1150	1150
Ventilation air		0	0		
Equip. @ 0.97 RSM			21234		
Latent cooling			3931		
TOTALS	1538	28285	25165	1150	1150

Printout certified by ACCA to meet all requirements of Manual J 7th Ed.

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-7S-17-09977-014 **Building permit No.** 000024629

Use Classification SFD,UTILITY **Fire:** 0.00

Permit Holder O'NEIL CONSTRUCTION **Waste:** 0.00

Owner of Building ANDREW TAYLOR **Total:** 0.00

Location: 818 SW DOWNING DRIVE, HIGH SPRINGS, FL

Date: 12/27/2006

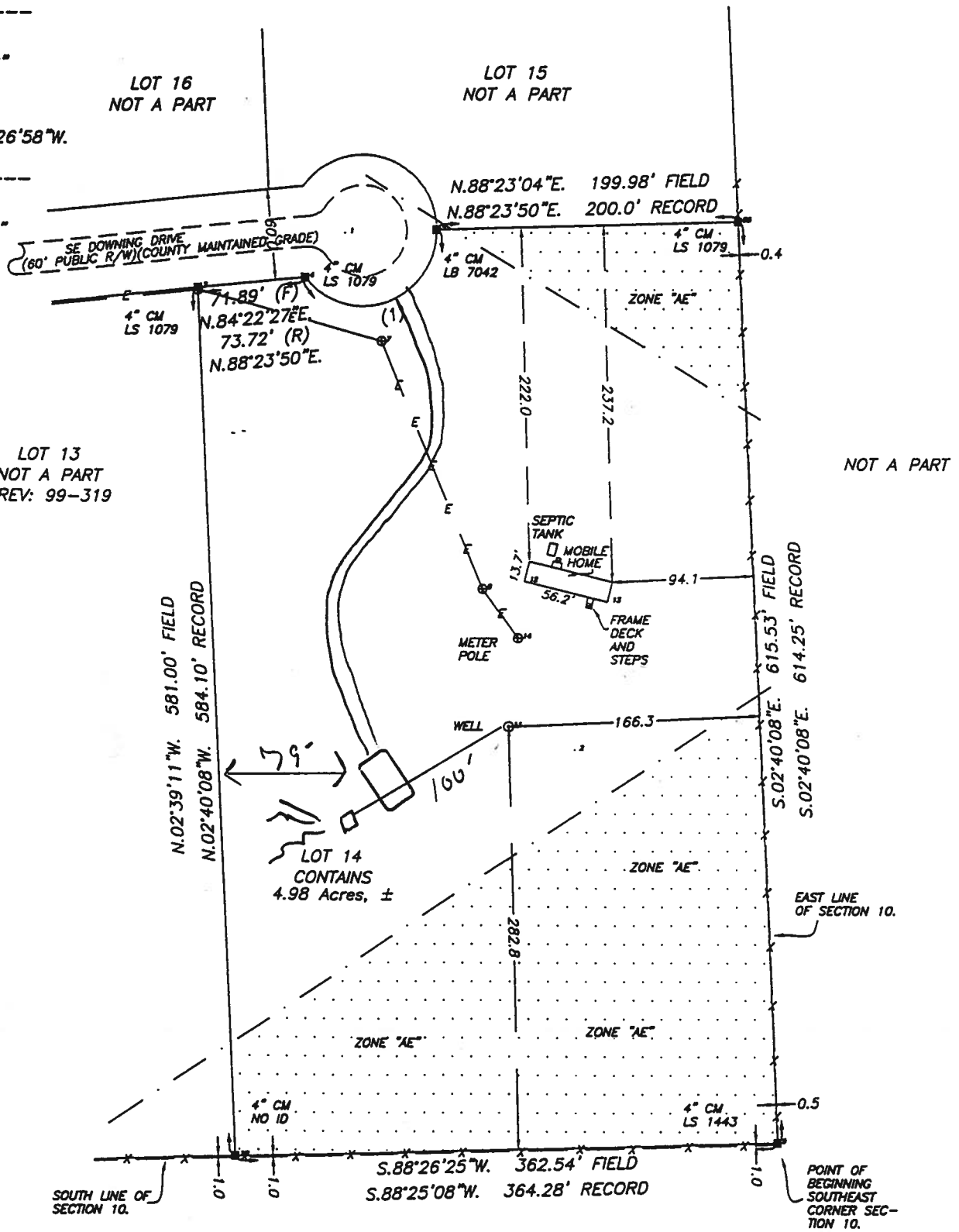
Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

BOUNDARY SURVEY
IN SECTION 10, TOWNSHIP 7 SOUTH, RANGE 17 EAST. COLUMBIA COUNTY, FLA.

Curve number 1
FIELD
Radius= 50.00
Delta= 142°19'46"
Arc= 124.21
Tangent= 146.58
Chord= 94.65
Chord Brg. S.70°26'58"W.

Curve number 1
RECORD
Radius= 50.00
Delta= 143°07'48"
Arc= 124.90



NOT A PART

SIGNED: _____
MARK D. DUREN, LS 4708

DESCRIPTION: LOT "14"
A PART OF THE SOUTHEAST QUARTER (SE 1/4) OF SECTION 10, TOWNSHIP 7 SOUTH, RANGE 17 EAST, MORE PARTICULARLY DESCRIBED AS FOLLOWS: BEGIN AT THE SOUTHEAST CORNER OF SAID SECTION 10 AND RUN SOUTH 88°25'08" WEST ALONG THE SOUTH LINE OF SAID SECTION 10, 364.28 FEET; THENCE NORTH 2°40'08" WEST, 584.10 FEET TO THE SOUTH RIGHT-OF-WAY LINE OF DOWNING DRIVE; THENCE NORTH 88°23'50" EAST ALONG THE SAID RIGHT-OF-WAY LINE, 73.72 FEET; THENCE ALONG THE ARC OF A CUL-DE-SAC CURVE TO THE LEFT HAVING A RADIUS OF 50.0 FEET, AN INCLUDED ANGLE OF 143°07'48" FOR AN ARC DISTANCE OF 124.90 FEET; THENCE NORTH 88°23'50" EAST, 200.0 FEET TO THE EAST LINE OF SAID SECTION 10, THENCE SOUTH 2°40'08" EAST, 614.25 FEET TO THE POINT OF BEGINNING. COLUMBIA COUNTY, FLORIDA.
CONTAINING 5.0 ACRES MORE OR LESS.
ALSO KNOWN AS LOT 14 OF DOWNING ACRES SUBDIVISION, UNRECORDED.

SURVEYOR'S NOTES:
1. BOUNDARY BASED ON MONUMENTATION FOUND IN ACCORDANCE WITH THE RETRACEMENT OF THE ORIGINAL SURVEY FOR SAID DEED OF RECORD.
2. BEARINGS BASED ON DEED OF RECORD USING MONUMENTS FOUND ON THE EAST BOUNDARY LINE OF LOT 14.
3. PART OF THIS PARCEL IS IN SHADED ZONE "X" AND IS DETERMINED TO BE INSIDE AREAS OF 500 YEAR FLOOD; AREAS OF 100 YEAR FLOOD WITH AVERAGE DEPTHS OF LESS THAN 1 FOOT AS PER FLOOD INSURANCE RATE MAP, DATED JANUARY 6, 1988, COMMUNITY PANEL NO. 120070 0280 B. AND PART OF THIS PARCEL IS IN ZONE "AE" AND HAS A BASE FLOOD ELEVATION OF ABOUT 53 FEET.
4. NO EASEMENT FOR UTILITY AND/OR DRAINAGE IS SHOWN ON THIS LOT IN RECORDS IN THE POSSESSION OF THIS OFFICE.
5. THE IMPROVEMENTS, IF ANY, INDICATED ON THIS SURVEY DRAWING ARE AS LOCATED ON DATE OF FIELD SURVEY AS SHOWN HEREON.
6. IF THEY EXIST, NO UNDERGROUND ENCROACHMENTS AND/OR UTILITIES WERE LOCATED FOR THIS SURVEY EXCEPT AS SHOWN HEREON.
7. "NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER."
8. CLOSURE OF FIELD SURVEY IS 1/67,219.
9. THIS SURVEY WAS PREPARED WITHOUT THE BENEFIT OF AN ABSTRACT OR TITLE POLICY. THEREFORE EXCEPTION IS MADE HEREON REGARDING EASEMENTS, RESERVATIONS, RESTRICTIONS, AND/OR TITLE CONFLICTS OF RECORD, IF ANY, NOT PROVIDED BY THE CLIENT OR HIS AGENTS.
10. CERTIFIED TO:
    ANDREW VINCENT TAYLOR

SYMBOL LEGEND

■	CONCRETE MONUMENT FOUND
□	CONCRETE MONUMENT SET, LS 4708
●	IRON PIN OR PIPE FOUND
○	5/8" IRON ROD SET, LS 4708
—x—	WIRE FENCE
—E—	ELECTRIC UTILITY LINE (OVERHEAD)
– UGE –	UNDERGROUND ELECTRIC SERVICE
— CTV —	CABLE TV LINE (OVERHEAD)
—o—	CHAIN LINK FENCE
—o—	WOODEN FENCE
CMP	CORRUGATED METAL PIPE
RCP	REINFORCED CONCRETE PIPE
LS	LAND SURVEYOR
LB	LICENSED BUSINESS
ORB	OFFICIAL RECORD BOOK
PRM	PERMANENT REFERENCE MONUMENT
PCP	PERMANENT CONTROL POINT
⊗	UTILITY POLE
R/W	RIGHT-OF-WAY
NO ID.	NO IDENTIFICATION
FLA. D.O.T.	FLA. DEPT. OF TRANSPORTATION
℄	CENTERLINE
C.M.	CONCRETE MONUMENT
I.R.	IRON ROD
I.P.	IRON PIPE

0 25 50 100 200
GRAPHIC SCALE

MARK D. DUREN, P.S.M.
LS 4708

1604 SW SISTERS WELCOME ROAD
LAKE CITY, FLA. 32025
(386) 758-9831 OFFICE
(386) 758-8010 FAX

FIELD SURVEY DATE NOVEMBER 11, 2005
DATE DRAWN NOVEMBER 14, 2005
FOR TAYLOR

FIELD BOOK 159 PAGE 55-56
DRAWN BY M. DUREN/S. COONER

WO# 05-888

RE: ONTAYLOR - 0 0

MiTek Industries, Inc.
1801 Massaro Blvd.
Tampa, Fl 33619
Phone: 813/675-1200
Fax: 813/675-1148

Site Information:
Project Customer: Project Name:
Lot/Block: Subdivision:
Address:
City: State:

Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI200 Design Program: MiTek 20/20 6.2
Wind Code: ASCE 7/02 Wind Speed: 110 mph Design Method: User defined
Roof Load: 40 psf, nonconcurrent BCLL=10 psf Floor Load: N/A psf

This package includes 30 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Job ID#	Truss Name	Date	No.	Seal#	Job ID#	Truss Name	Date
1	T2113543	ONTAYLOR	A	3/30/06	17	T2113559	ONTAYLOR	CJO1	3/30/06
2	T2113544	ONTAYLOR	AET	3/30/06	18	T2113560	ONTAYLOR	CJO2	3/30/06
3	T2113545	ONTAYLOR	B	3/30/06	19	T2113561	ONTAYLOR	D	3/30/06
4	T2113546	ONTAYLOR	B1	3/30/06	20	T2113562	ONTAYLOR	D1	3/30/06
5	T2113547	ONTAYLOR	B2	3/30/06	21	T2113563	ONTAYLOR	D2	3/30/06
6	T2113548	ONTAYLOR	B3	3/30/06	22	T2113564	ONTAYLOR	D3	3/30/06
7	T2113549	ONTAYLOR	B4	3/30/06	23	T2113565	ONTAYLOR	D4	3/30/06
8	T2113550	ONTAYLOR	B5	3/30/06	24	T2113566	ONTAYLOR	D5	3/30/06
9	T2113551	ONTAYLOR	B6	3/30/06	25	T2113567	ONTAYLOR	DET	3/30/06
10	T2113552	ONTAYLOR	BET	3/30/06	26	T2113568	ONTAYLOR	EJ5	3/30/06
11	T2113553	ONTAYLOR	C	3/30/06	27	T2113569	ONTAYLOR	J1	3/30/06
12	T2113554	ONTAYLOR	C1	3/30/06	28	T2113570	ONTAYLOR	J1A	3/30/06
13	T2113555	ONTAYLOR	C2	3/30/06	29	T2113571	ONTAYLOR	J3	3/30/06
14	T2113556	ONTAYLOR	C3	3/30/06	30	T2113572	ONTAYLOR	J3A	3/30/06
15	T2113557	ONTAYLOR	C4	3/30/06					
16	T2113558	ONTAYLOR	C5	3/30/06					

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Zhang, Guo-jie
My license renewal date for the state of is February 28, 2007.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

Guo-Jie Zhang, FL Lic #47744
MiTek Industries, Inc.
1801 Massaro Blvd
Tampa FL 33619
FL Cert.#6634

March 30,2006

Zhang, Guo-jie

Job	Truss	Truss Type	Qty	Ply	0 0 T2113543
ONTAYLOR	A	ROOF TRUSS	15	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:02 2006 Page 1

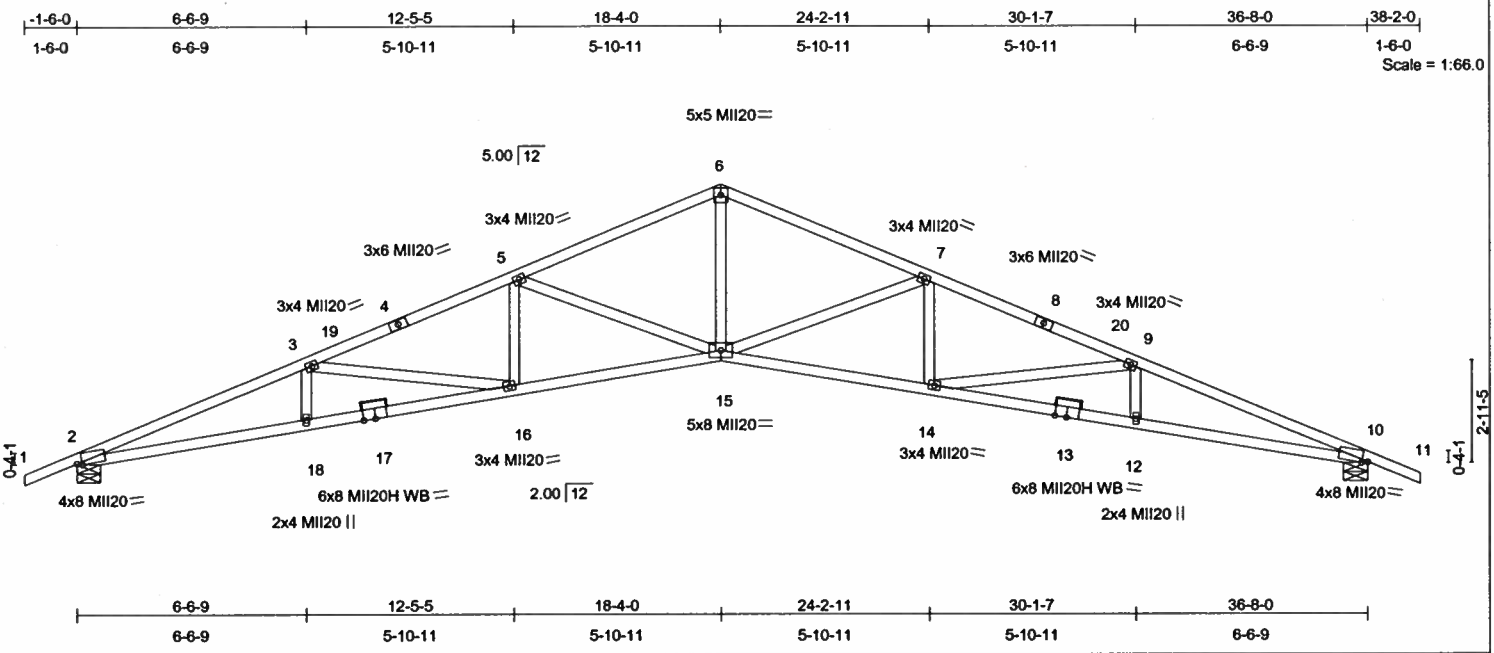


Plate Offsets (X,Y): [2:0-1-14,0-0-8], [10:0-1-14,0-0-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	0.72	15-16	>599	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.99	14-15	>436	180	MII20H	187/143
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.58	10	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 174 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-7-13 oc bracing.

REACTIONS (lb/size) 2=1550/0-8-0, 10=1550/0-8-0
Max Horz 2=-83(load case 6)
Max Uplift2=-1180(load case 5), 10=-1180(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/31, 2-3=-4657/3179, 3-19=-3954/2588, 4-19=-3894/2596, 4-5=-3888/2608, 5-6=-3067/1987, 6-7=-3067/1987, 7-8=-3888/2559, 8-20=-3894/2547, 9-20=-3954/2539, 9-10=-4657/3128, 10-11=0/31
BOT CHORD 2-18=-2868/4239, 17-18=-2836/4229, 16-17=-2832/4242, 15-16=-2171/3648, 14-15=-2126/3648, 13-14=-2701/4242, 12-13=-2706/4229, 10-12=-2737/4239
WEBS 3-18=-140/242, 3-16=-635/631, 5-16=-326/377, 5-15=-914/718, 6-15=-1333/1946, 7-15=-914/720, 7-14=-327/377, 9-14=-635/636, 9-12=-139/242

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are MT20 plates unless otherwise indicated.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Bearing at joint(s) 2, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1180 lb uplift at joint 2 and 1180 lb uplift at joint 10.

LOAD CASE(S) Standard

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March 30,2006

Job	Truss	Truss Type	Qty	Ply	0 0 T2113544
ONTAYLOR	AET	ROOF TRUSS	1	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:05 2006 Page 1

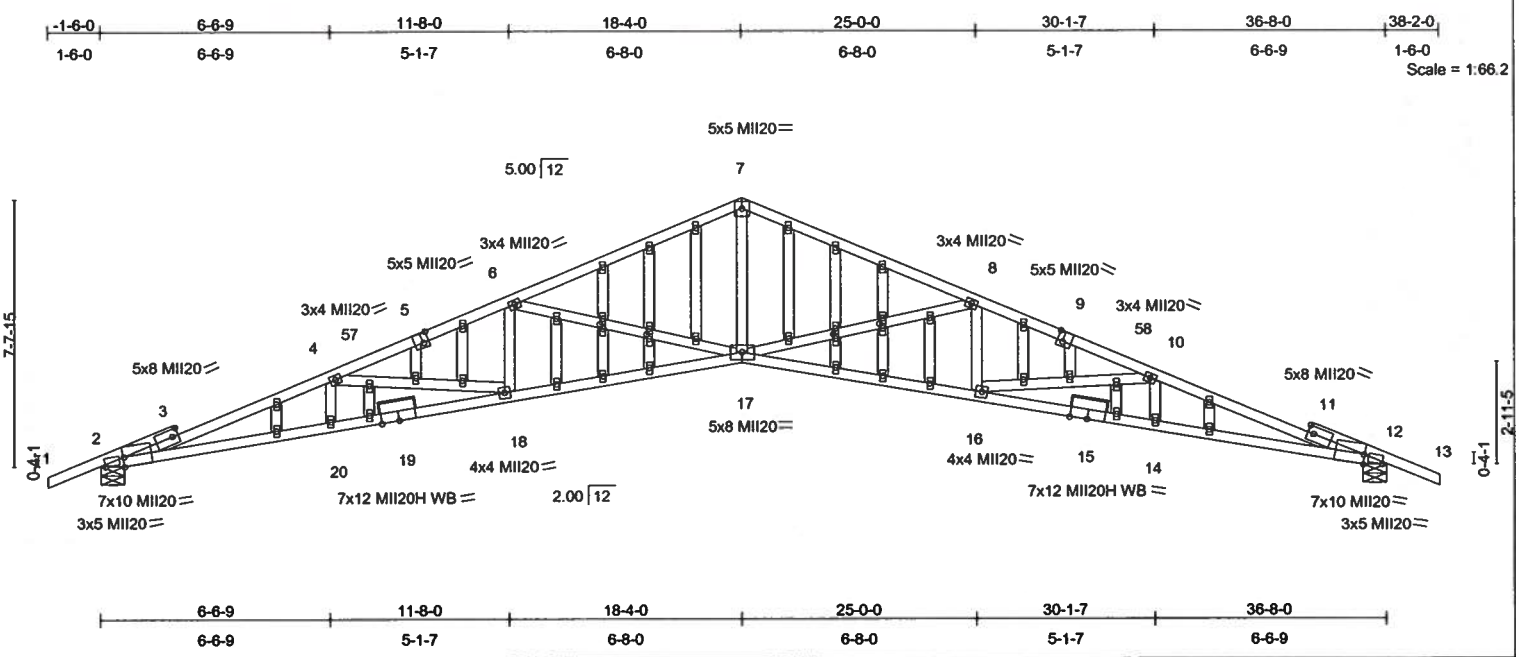

Plate Offsets (X,Y): [2:0-6-13,Edge], [2:0-0-7,Edge], [5:0-2-0,0-3-4], [9:0-2-0,0-3-4], [12:0-0-7,Edge], [12:0-6-13,Edge], [23:0-1-9,0-1-0], [26:0-1-9,0-1-0], [43:0-1-9,0-1-0], [46:0-1-9,0-1-0]

LOADING (psf)		SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0		Plates Increase 1.25	TC 0.98	Vert(LL) 0.97 17-18 >446 240	MII20 249/190
TCDL 10.0		Lumber Increase 1.25	BC 0.92	Vert(TL) -1.32 16-17 >328 180	MII20H 187/143
BCLL 10.0	*	Rep Stress Incr YES	WB 0.97	Horz(TL) 0.72 12 n/a n/a	
BCDL 10.0		Code FBC2004/TPI2002	(Matrix)		Weight: 222 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
2-19 2 X 4 SYP SS, 12-15 2 X 4 SYP SS
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 2=1550/0-8-0, 12=1550/0-8-0
Max Horz 2=-77(load case 6)
Max Uplift2=-1180(load case 5), 12=-1180(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/31, 2-3=-5526/3763, 3-4=-5486/3784, 4-57=-4503/2965, 5-57=-4444/2967, 5-6=-4443/2983, 6-7=-3312/2126, 7-8=-3312/2127, 8-9=-4443/2926, 9-58=-4444/2910, 10-58=-4503/2908, 10-11=-5486/3728, 11-12=-5526/3707, 12-13=0/31
BOT CHORD 2-20=-3496/5145, 19-20=-3464/5134, 18-19=-3461/5145, 17-18=-2544/4170, 16-17=-2487/4171, 15-16=-3330/5145, 14-15=-3332/5134, 12-14=-3364/5145
WEBS 4-20=-133/222, 4-18=-969/884, 6-18=-386/450, 6-17=-1165/911, 7-17=-1400/2056, 8-17=-1165/911, 8-16=-386/450, 10-16=-969/884, 10-14=-133/222

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) All plates are 2x4 MII20 unless otherwise indicated.
7) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
8) Gable studs spaced at 1-4-0 oc.
9) Bearing at joint(s) 2, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1180 lb uplift at joint 2 and 1180 lb uplift at joint 12.

LOAD CASE(S) Standard

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March 30,2006

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Tampa, FL 33619

MiTek®

Job	Truss	Truss Type	Qty	Ply	0 0
ONTAYLOR	B	ROOF TRUSS	2	1	T2113545
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:06 2006 Page 1

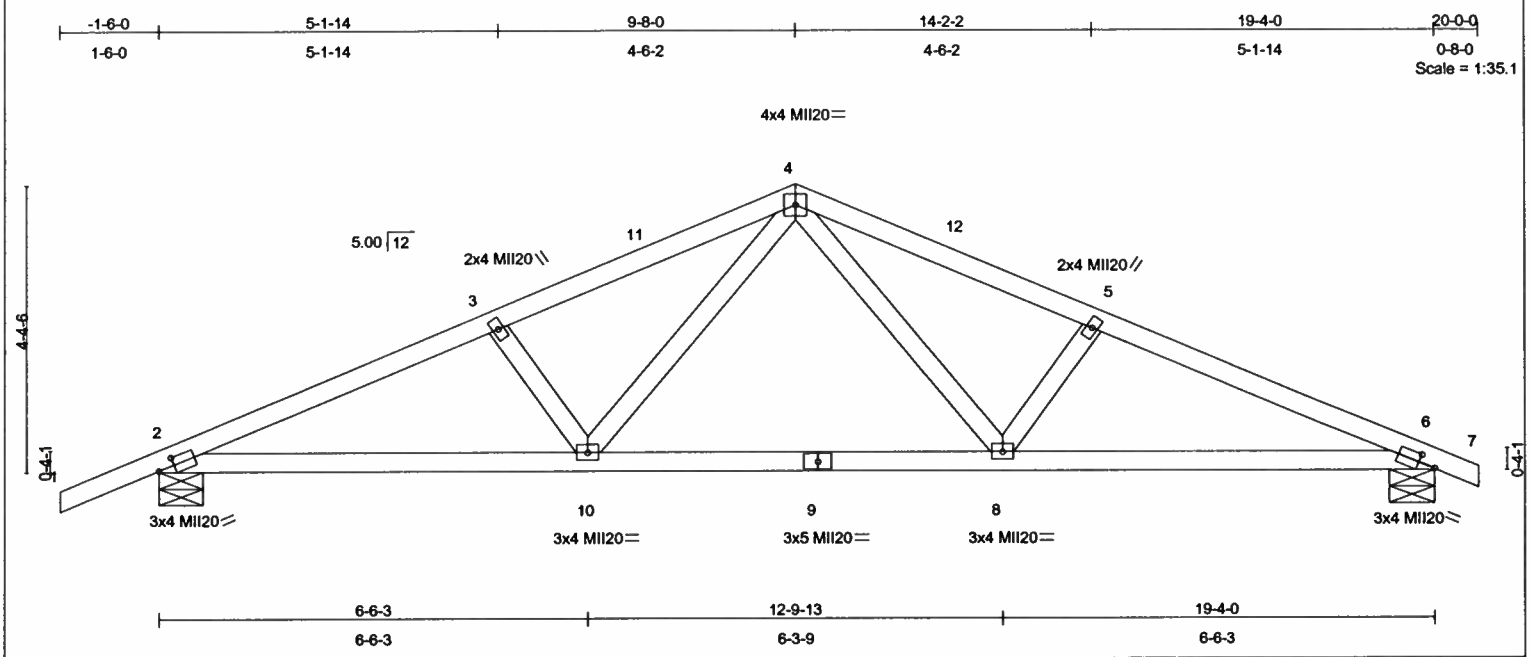

Plate Offsets (X,Y): [2:0-3-0,0-1-8], [6:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	0.10	8-10	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.27	8-10	>842	180		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.19	Horz(TL)	0.04	6	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 85 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-1-8 oc bracing.

REACTIONS (lb/size) 2=986/0-8-0, 6=929/0-8-0
Max Horz 2=96(load case 5)
Max Uplift2=-594(load case 5), 6=-528(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-1736/678, 3-11=-1582/635, 4-11=-1527/651, 4-12=-1544/682, 5-12=-1599/666, 5-6=-1755/711, 6-7=0/14
BOT CHORD 2-10=-597/1527, 9-10=-311/1073, 8-9=-311/1073, 6-8=-565/1547
WEBS 3-10=-221/243, 4-10=-246/577, 4-8=-277/599, 5-8=-232/258

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 594 lb uplift at joint 2 and 528 lb uplift at joint 6.
6) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-60, 4-7=-60, 2-10=-20, 8-10=-60(F=-40), 6-8=-20
2) IBC BC Live: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-20, 4-7=-20, 2-10=-40, 8-10=-80(F=-40), 6-8=-40
3) MWFRS Wind Left: Lumber Increase=1.33, Plate Increase=1.33

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March 30,2006

Continued on page 2

MiTek®

LOAD CASE(S) Standard

Uniform Loads (plf)
Vert: 1-2=55, 2-4=32, 4-6=28, 6-7=20, 2-10=22, 8-10=-18(F=-40), 6-8=22
Horz: 1-2=-65, 2-4=-42, 4-6=38, 6-7=30

4) MWFRS Wind Right: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=20, 2-4=28, 4-6=32, 6-7=55, 2-10=22, 8-10=-18(F=-40), 6-8=22
Horz: 1-2=-30, 2-4=-38, 4-6=42, 6-7=65

5) MWFRS 1st Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=71, 2-11=48, 4-11=30, 4-6=30, 6-7=22, 2-10=22, 8-10=-18(F=-40), 6-8=22
Horz: 1-2=-81, 2-11=-58, 4-11=-40, 4-6=40, 6-7=32

6) MWFRS 2nd Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=22, 2-4=30, 4-12=30, 6-12=48, 6-7=71, 2-10=22, 8-10=-18(F=-40), 6-8=22
Horz: 1-2=-32, 2-4=-40, 4-12=40, 6-12=58, 6-7=81

7) MWFRS 3rd Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=46, 2-11=23, 4-11=16, 4-6=16, 6-7=7, 2-10=22, 8-10=-18(F=-40), 6-8=22
Horz: 1-2=-56, 2-11=-33, 4-11=-26, 4-6=26, 6-7=17

8) MWFRS 4th Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=7, 2-4=16, 4-12=16, 6-12=23, 6-7=46, 2-10=22, 8-10=-18(F=-40), 6-8=22
Horz: 1-2=-17, 2-4=-26, 4-12=26, 6-12=33, 6-7=56

9) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-60, 4-7=-20, 2-10=-20, 8-10=-60(F=-40), 6-8=-20

10) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-20, 4-7=-60, 2-10=-20, 8-10=-60(F=-40), 6-8=-20

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Job	Truss	Truss Type	Qty	Ply	0 0 T2113546
ONTAYLOR	B1	ROOF TRUSS	4	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:07 2006 Page 1

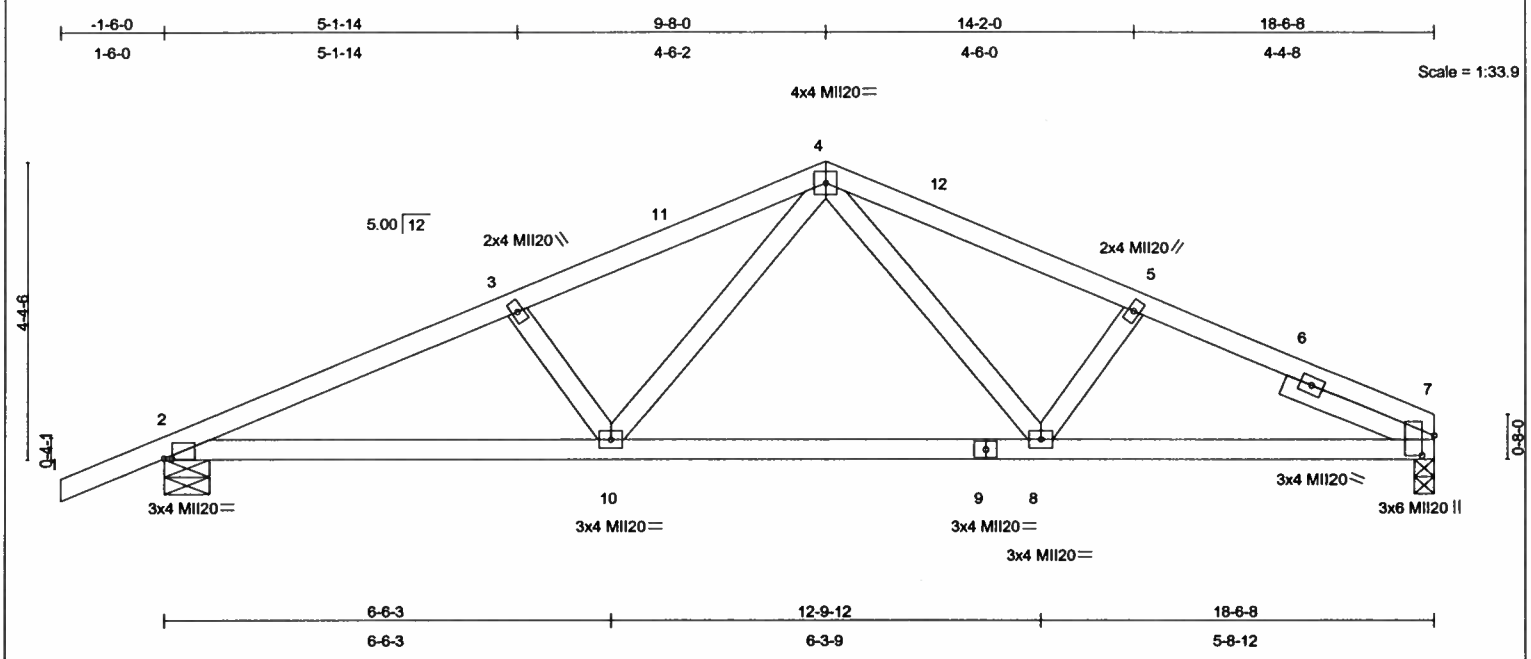

Plate Offsets (X,Y): [2:0-1-6,0-0-2], [7:0-3-7,0-2-3]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	0.10	8-10	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.26	8-10	>842	180		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.19	Horz(TL)	0.04	7	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 85 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 4 SYP No.3 2-3-13

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-2-7 oc bracing.

REACTIONS (lb/size) 7=852/0-3-8, 2=967/0-8-0
Max Horz 2=104(load case 5)
Max Uplift7=-441(load case 6), 2=-585(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-1688/658, 3-11=-1535/616, 4-11=-1440/632, 4-12=-1370/633, 5-12=-1455/624, 5-6=-1541/660, 6-7=-1603/640
BOT CHORD 2-10=-586/1482, 9-10=-299/1031, 8-9=-299/1031, 7-8=-526/1382
WEBS 3-10=-218/240, 4-10=-250/577, 4-8=-221/493, 5-8=-154/232

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 441 lb uplift at joint 7 and 585 lb uplift at joint 2.
6) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S)
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-60, 4-7=-60, 2-10=-20, 8-10=-60(F=-40), 7-8=-20
2) IBC BC Live: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-20, 4-7=-20, 2-10=-40, 8-10=-80(F=-40), 7-8=-40
3) MWFRS Wind Left: Lumber Increase=1.33, Plate Increase=1.33

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Continued on page 2

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LOAD CASE(S)

Uniform Loads (plf)
Vert: 1-2=55, 2-4=32, 4-7=28, 2-10=22, 8-10=-18(F=-40), 7-8=22
Horz: 1-2=-65, 2-4=-42, 4-7=38

4) MWFRS Wind Right: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=20, 2-4=28, 4-7=32, 2-10=22, 8-10=-18(F=-40), 7-8=22
Horz: 1-2=-30, 2-4=-38, 4-7=42

5) MWFRS 1st Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=71, 2-11=48, 4-11=30, 4-7=30, 2-10=22, 8-10=-18(F=-40), 7-8=22
Horz: 1-2=-81, 2-11=-58, 4-11=-40, 4-7=40

6) MWFRS 2nd Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=22, 2-4=30, 4-12=30, 7-12=48, 2-10=22, 8-10=-18(F=-40), 7-8=22
Horz: 1-2=-32, 2-4=-40, 4-12=40, 7-12=58

7) MWFRS 3rd Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=46, 2-11=23, 4-11=16, 4-7=16, 2-10=22, 8-10=-18(F=-40), 7-8=22
Horz: 1-2=-56, 2-11=-33, 4-11=-26, 4-7=26

8) MWFRS 4th Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: 1-2=7, 2-4=16, 4-12=16, 7-12=23, 2-10=22, 8-10=-18(F=-40), 7-8=22
Horz: 1-2=-17, 2-4=-26, 4-12=26, 7-12=33

9) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-60, 4-7=-20, 2-10=-20, 8-10=-60(F=-40), 7-8=-20

10) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-20, 4-7=-60, 2-10=-20, 8-10=-60(F=-40), 7-8=-20

Job	Truss	Truss Type	Qty	Ply	0 0 T2113547
ONTAYLOR	B2	ROOF TRUSS	1	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:08 2006 Page 1

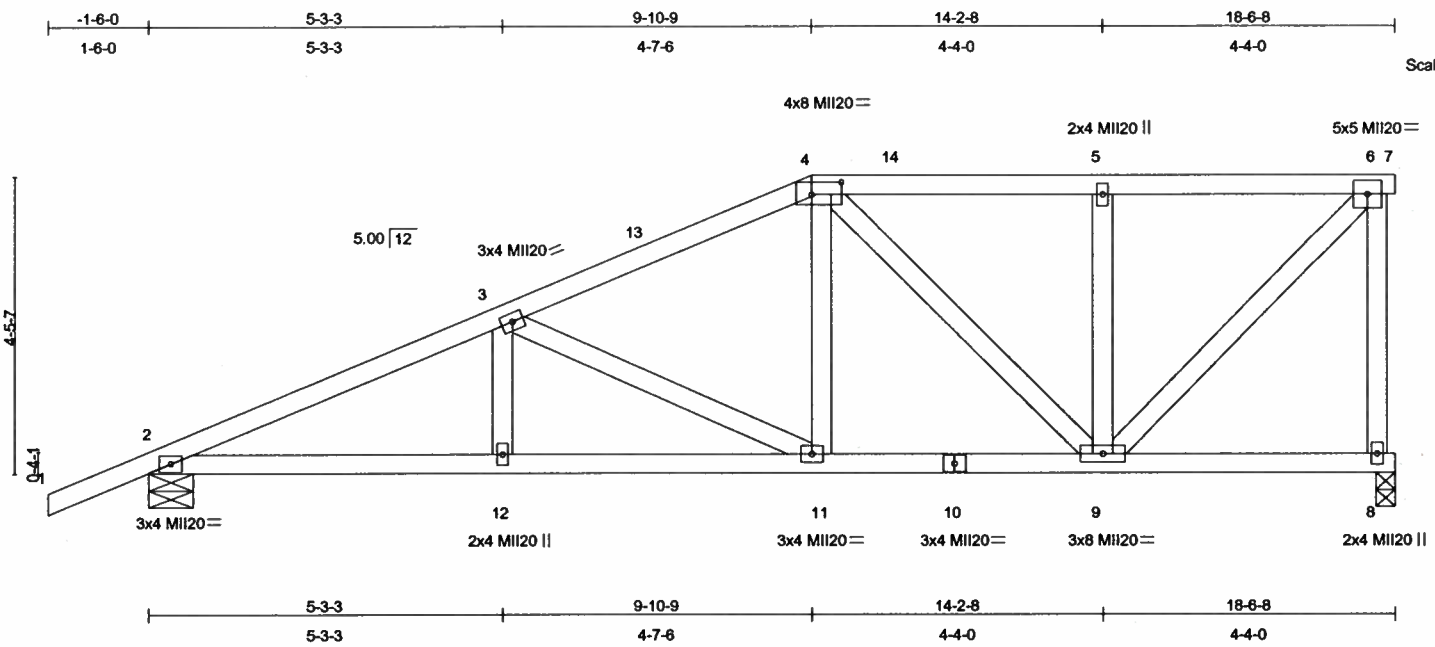

Plate Offsets (X,Y): [4:0-5-4,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	0.07	11-12	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.08	11-12	>999	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.02	8	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 103 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-6-4 oc bracing.

REACTIONS (lb/size) 8=717/0-3-8, 2=833/0-8-0
Max Horz 2=277(load case 4)
Max Uplift8=-614(load case 4), 2=-695(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-1352/962, 3-13=-903/682, 4-13=-849/691, 4-14=-562/517, 5-14=-561/517, 5-6=-561/517, 6-7=0/0, 6-8=-673/561
BOT CHORD 2-12=-987/1177, 11-12=-987/1177, 10-11=-702/794, 9-10=-702/794, 8-9=-66/51
WEBS 3-12=-114/204, 3-11=-434/433, 4-11=-281/325, 4-9=-322/261, 5-9=-257/219, 6-9=-653/775

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 614 lb uplift at joint 8 and 695 lb uplift at joint 2.

LOAD CASE(S) Standard

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March 30,2006

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Job	Truss	Truss Type	Qty	Ply	0 0 T2113548
ONTAYLOR	B3	ROOF TRUSS	1	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:09 2006 Page 1

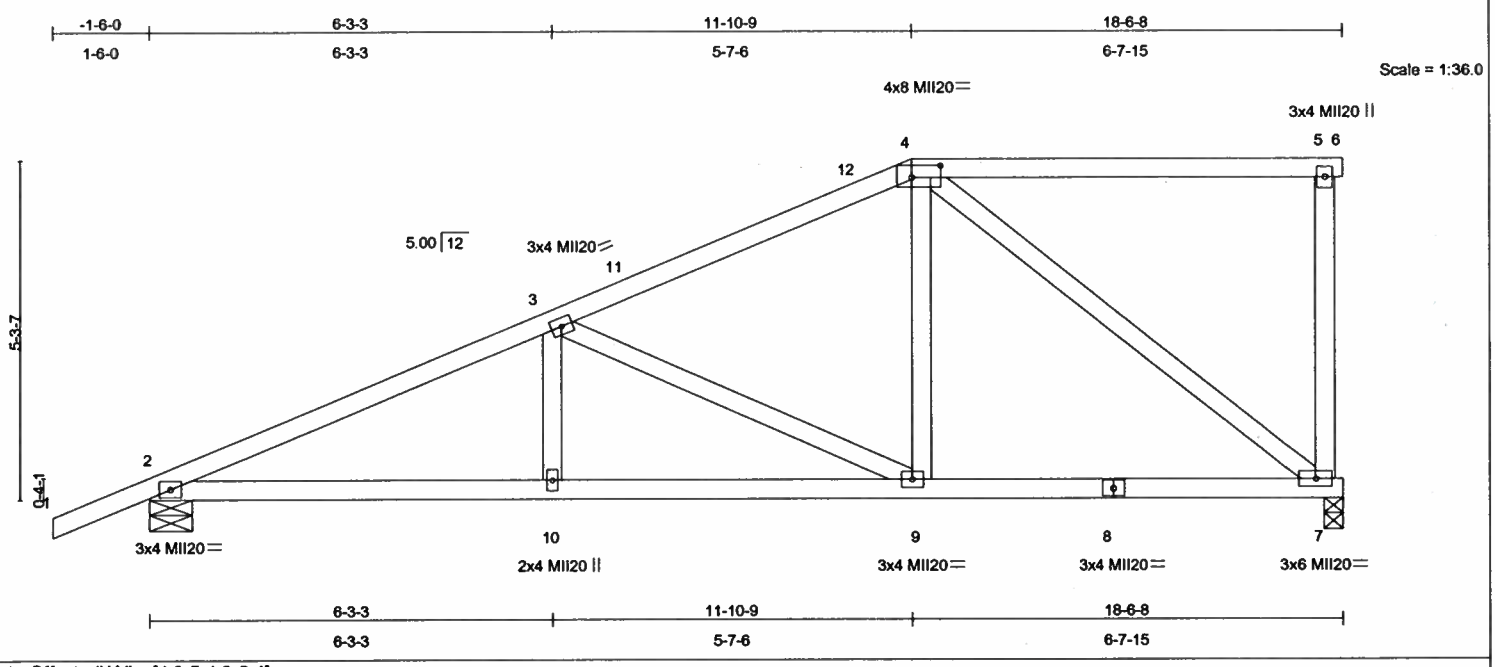

Plate Offsets (X,Y): [4:0-5-4,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.09	2-10	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.11	2-10	>999	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.84	Horz(TL)	0.03	7	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 97 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-6-12 oc bracing.

REACTIONS (lb/size) 7=717/0-3-8, 2=833/0-8-0
Max Horz 2=328(load case 4)
Max Uplift7=-604(load case 4), 2=-693(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-1310/912, 3-11=-741/537, 11-12=-629/553, 4-12=-624/556, 4-5=-40/111, 5-6=0/0, 5-7=-186/166
BOT CHORD 2-10=-936/1133, 9-10=-936/1133, 8-9=-575/637, 7-8=-575/637
WEBS 3-10=-142/243, 3-9=-557/531, 4-9=-363/431, 4-7=-757/610

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 604 lb uplift at joint 7 and 693 lb uplift at joint 2.

LOAD CASE(S) Standard

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March 30,2006

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Job	Truss	Truss Type	Qty	Ply	0 0
ONTAYLOR	B4	ROOF TRUSS	1	1	T2113549
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:09 2006 Page 1

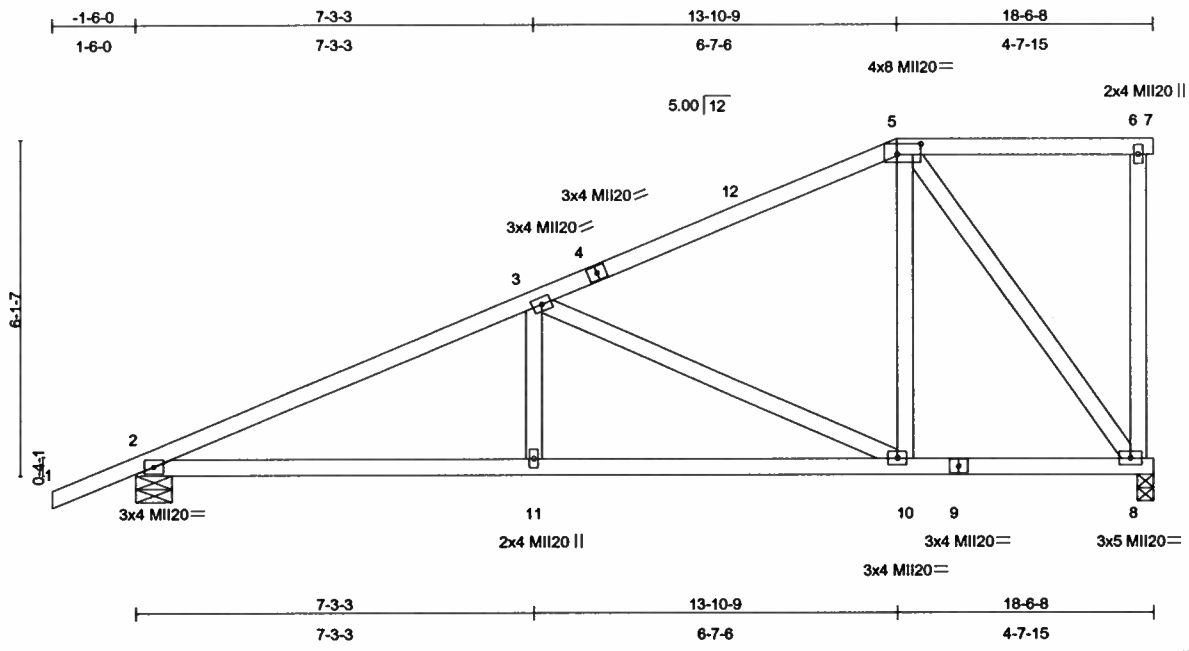

Plate Offsets (X,Y): [5:0-5-4,0-2-4]

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.36	Vert(LL)	0.14	2-11	>999	240	MII20	249/190
TCDL	10.0		Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.16	2-11	>999	180		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.03	8	n/a	n/a		
BCDL	10.0		Code FBC2004/TPI2002		(Matrix)							Weight: 101 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-7-7 oc bracing.

REACTIONS (lb/size) 8=717/0-3-8, 2=833/0-8-0
Max Horz 2=380(load case 4)
Max Uplift8=-593(load case 6), 2=-688(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-1270/858, 3-4=-551/386, 4-12=-470/398, 5-12=-463/409, 5-6=-60/98, 6-7=0/0, 6-8=-105/106
BOT CHORD 2-11=-895/1092, 10-11=-895/1092, 9-10=-421/444, 8-9=-421/444
WEBS 3-11=-175/303, 3-10=-722/643, 5-10=-405/460, 5-8=-721/584

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 593 lb uplift at joint 8 and 688 lb uplift at joint 2.

LOAD CASE(S) Standard

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March 30,2006

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Job	Truss	Truss Type	Qty	Ply	0 0
ONTAYLOR	B5	ROOF TRUSS	1	1	T2113550
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:10 2006 Page 1

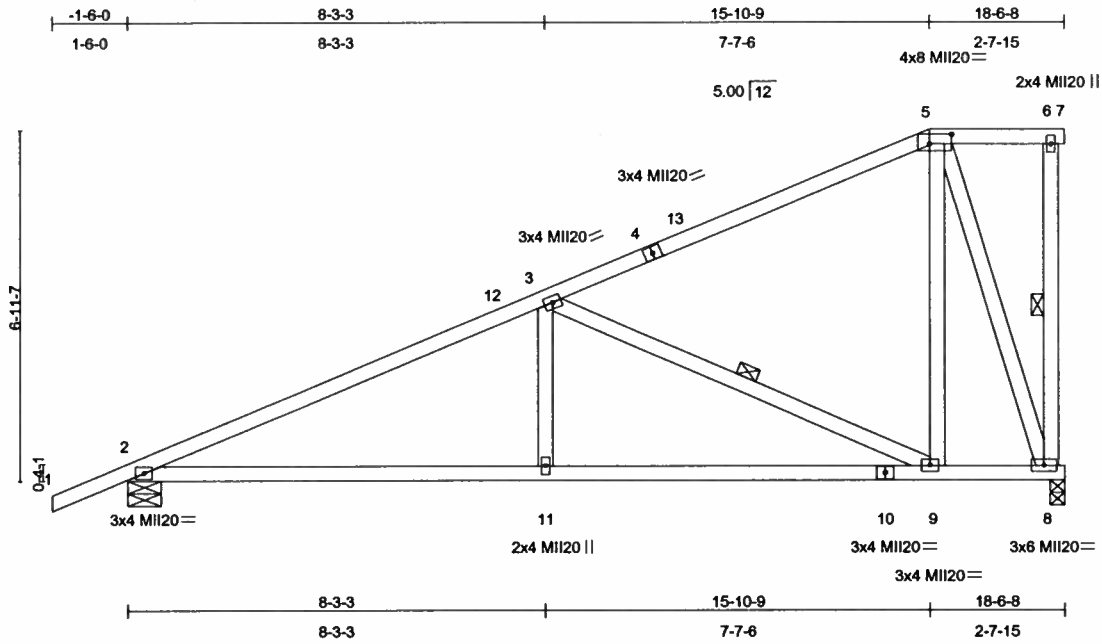

Plate Offsets (X,Y): [5:0-5-4,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	0.22	2-11	>984	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.24	2-11	>888	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.03	8	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 105 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-8-0 oc bracing.
WEBS 1 Row at midpt 6-8, 3-9

REACTIONS (lb/size) 8=717/0-3-8, 2=833/0-8-0
Max Horz 2=432(load case 4)
Max Uplift8=-601(load case 6), 2=-687(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-12=-1219/788, 3-12=-1047/805, 3-4=-368/234, 4-13=-303/242, 5-13=-273/260, 5-6=-78/102, 6-7=0/0, 6-8=-16/47
BOT CHORD 2-11=-847/1037, 10-11=-847/1037, 9-10=-847/1037, 8-9=-281/262
WEBS 3-11=-204/352, 3-9=-861/743, 5-9=-475/516, 5-8=-777/670

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 601 lb uplift at joint 8 and 687 lb uplift at joint 2.

LOAD CASE(S) Standard

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March 30,2006

Job	Truss	Truss Type	Qty	Ply	0 0 T2113551
ONTAYLOR	B6	ROOF TRUSS	1	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:11 2006 Page 1

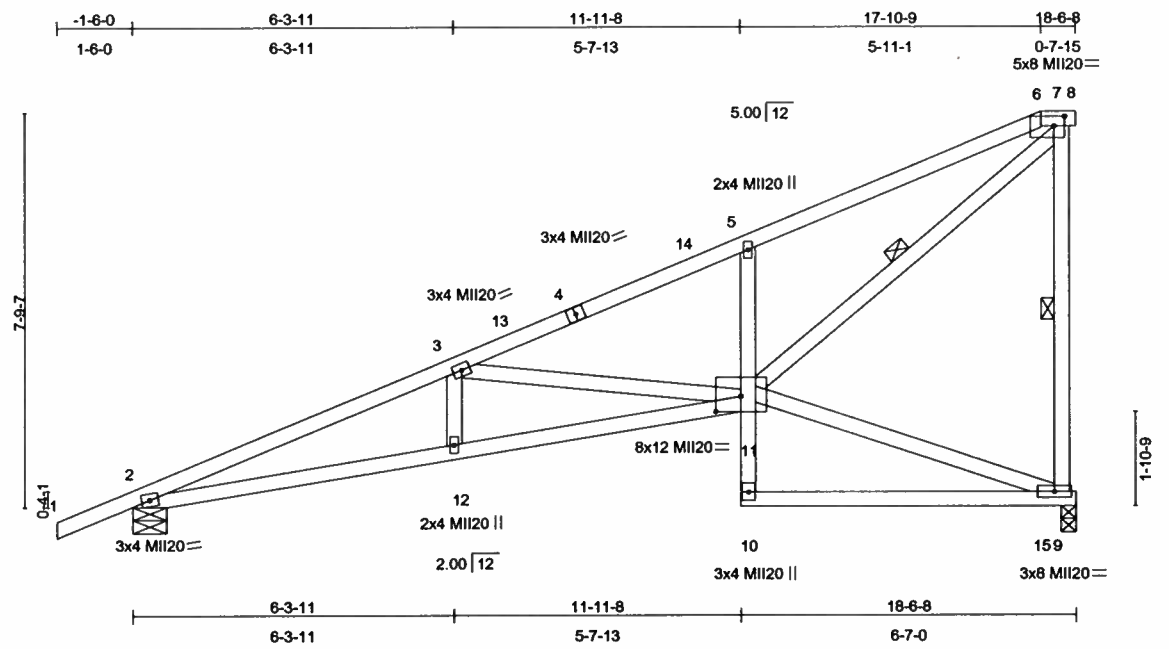

Plate Offsets (X,Y): [7:0-2-8,0-2-4], [11:0-6-0,0-3-9]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	0.19	11-12	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.20	11-12	>999	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.10	9	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 109 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except* 5-10 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-10-9 oc bracing.
WEBS 1 Row at midpt 7-9, 7-11

REACTIONS (lb/size) 9=717/0-3-8, 2=833/0-8-0
Max Horz 2=664(load case 4)
Max Uplift9=-621(load case 6), 2=-672(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/31, 2-3=-1976/1465, 3-13=-1110/774, 4-13=-1055/781, 4-14=-1045/783, 5-14=-990/794, 5-6=-1088/893, 6-7=-940/891, 7-8=0/0, 7-9=-648/544
BOT CHORD 2-12=-1675/1769, 11-12=-1641/1766, 10-11=-93/132, 5-11=-349/270, 10-15=-18/69, 9-15=-18/69
WEBS 3-12=-156/260, 3-11=-775/759, 9-11=-138/129, 7-11=-1052/1254

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 621 lb uplift at joint 9 and 672 lb uplift at joint 2.

LOAD CASE(S) Standard

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FL Cert.#6634

March 30,2006

Job	Truss	Truss Type	Qty	Ply	0 0 T2113552
ONTAYLOR	BET	FINK	1	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC

6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:13 2006 Page 1

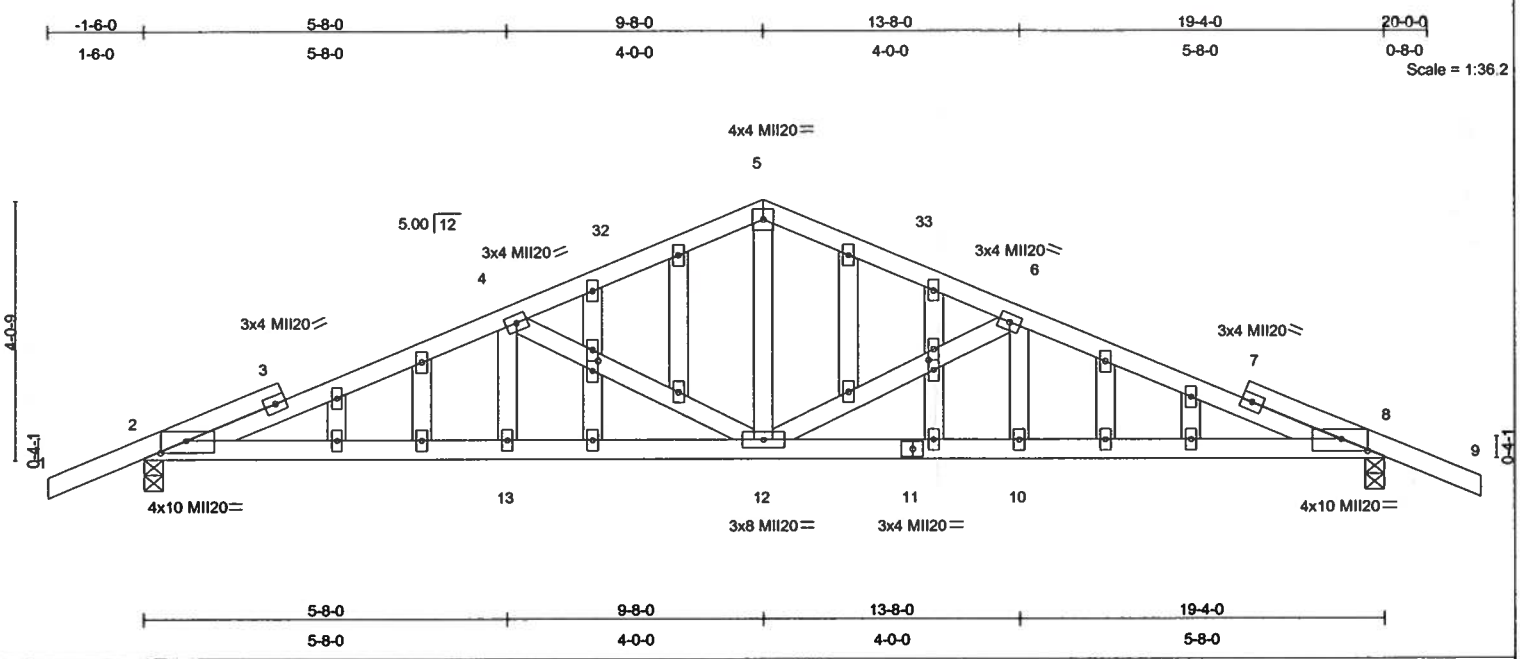

Plate Offsets (X,Y): [2:0-4-12,0-2-4], [8:0-4-12,0-2-4], [16:0-1-14,0-1-0], [27:0-1-14,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.13	2-13	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.16	8-10	>999	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.22	Horz(TL)	0.05	8	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 112 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-8 oc bracing.

REACTIONS (lb/size) 2=860/0-3-8, 8=860/0-3-8
Max Horz 2=-79(load case 6)
Max Uplift2=-717(load case 5), 8=-717(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-1645/1181, 3-4=-1612/1199, 4-32=-1100/798, 5-32=-1042/812, 5-33=-1042/812, 6-33=-1100/799, 6-7=-1612/1200, 7-8=-1645/1182, 8-9=0/33
BOT CHORD 2-13=-1089/1497, 12-13=-1089/1497, 11-12=-1011/1497, 10-11=-1011/1497, 8-10=-1011/1497
WEBS 4-13=-125/212, 4-12=-603/553, 5-12=-535/623, 6-12=-603/554, 6-10=-125/212

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MII20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable studs spaced at 1-4-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 717 lb uplift at joint 2 and 717 lb uplift at joint 8.

LOAD CASE(S) Standard

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March 30,2006

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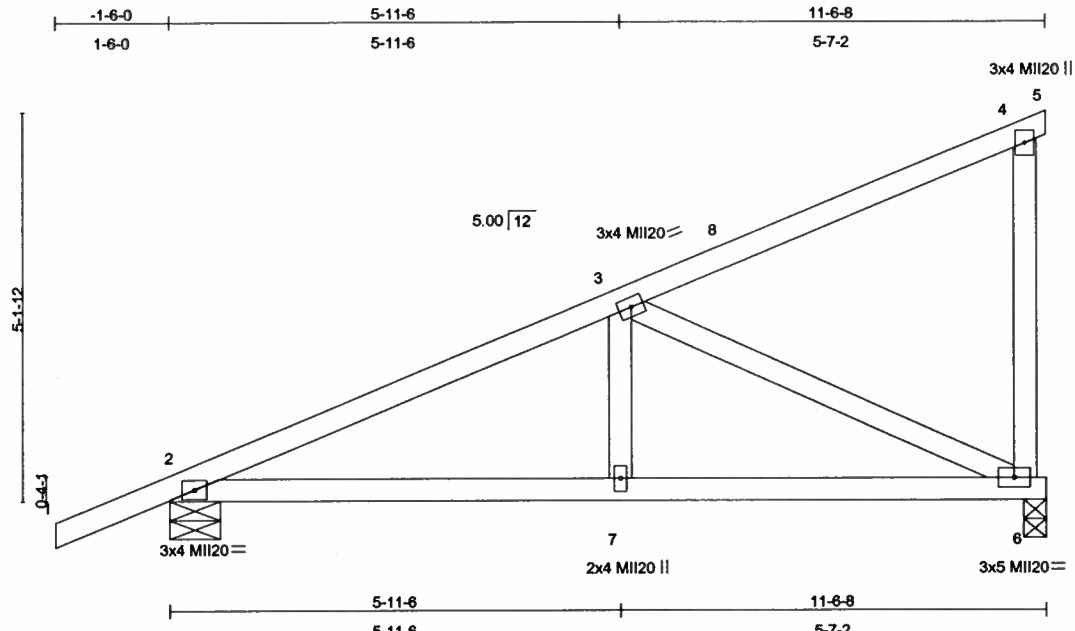
MiTek®

Job	Truss	Truss Type	Qty	Ply	0 0
ONTAYLOR	C	ROOF TRUSS	4	1	T2113553
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:13 2006 Page 1



LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.33	Vert(LL)	0.05	2-7	>999	240	MII20	249/190
TCDL	10.0		Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.06	2-7	>999	180		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.35	Horz(TL)	0.01	6	n/a	n/a		
BCDL	10.0		Code FBC2004/TPI2002		(Matrix)							Weight: 57 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-5-5 oc bracing.

REACTIONS (lb/size) 6=434/0-3-8, 2=557/0-8-0
Max Horz 2=315(load case 4)
Max Uplift6=-341(load case 4), 2=-491(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-661/445, 3-8=-95/46, 4-8=-83/65, 4-5=-2/0, 4-6=-137/99
BOT CHORD 2-7=-461/545, 6-7=-461/545
WEBS 3-7=-147/246, 3-6=-575/538

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 341 lb uplift at joint 6 and 491 lb uplift at joint 2.

LOAD CASE(S) Standard

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March 30,2006

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Job	Truss	Truss Type	Qty	Ply	0 0
ONTAYLOR	C1	ROOF TRUSS	1	1	T2113554
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:16 2006 Page 1

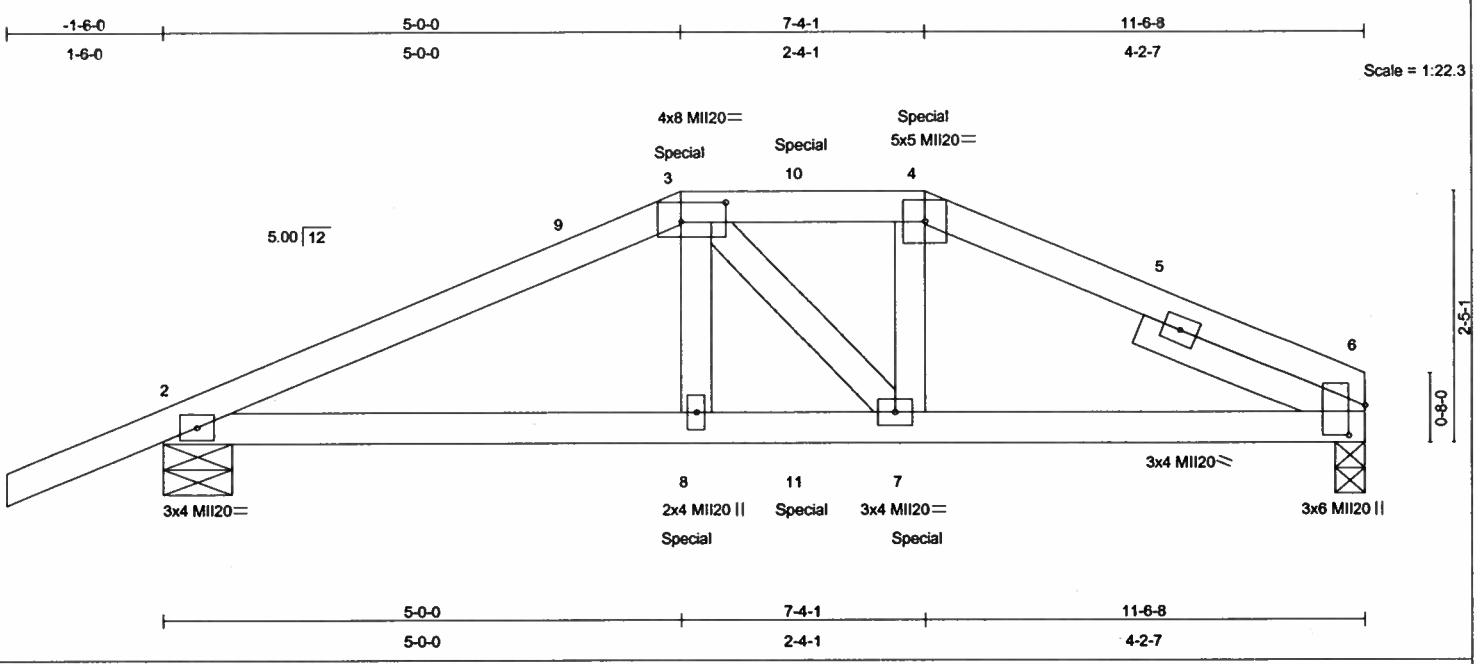

Plate Offsets (X,Y): [3:0-5-4,0-2-4], [6:0-3-8,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	0.06	2-8	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.06	2-8	>999	180		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.13	Horz(TL)	0.02	6	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 51 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 4 SYP No.3 2-3-10

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-4-14 oc bracing.

REACTIONS (lb/size) 6=730/0-3-8, 2=845/0-8-0
Max Horz 2=86(load case 5)
Max Uplift6=-664(load case 6), 2=-780(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-9=-1377/1184, 3-9=-1272/1194, 3-10=-1131/1110, 4-10=-1131/1110, 4-5=-1238/1161, 5-6=-1291/1145
BOT CHORD 2-8=-1025/1200, 8-11=-1046/1225, 7-11=-1046/1225, 6-7=-966/1110
WEBS 3-8=-315/391, 3-7=-206/110, 4-7=-328/357

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 664 lb uplift at joint 6 and 780 lb uplift at joint 2.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 46 lb down and 65 lb up at 5-0-0, and 53 lb down and 83 lb up at 6-2-0, and 53 lb down and 83 lb up at 7-4-1 on top chord, and 270 lb down and 214 lb up at 5-0-0, and 52 lb down and 28 lb up at 6-2-0, and 160 lb down and 183 lb up at 7-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-60, 3-4=-60, 4-6=-60, 2-6=-20
Concentrated Loads (lb)
Vert: 3=-6(B) 4=-53(B) 8=-270(B) 7=-160(B) 10=-53(B) 11=-26(B)

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March 30,2006

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Job	Truss	Truss Type	Qty	Ply	0 0 T2113555
ONTAYLOR	C2	ROOF TRUSS	1	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:17 2006 Page 1

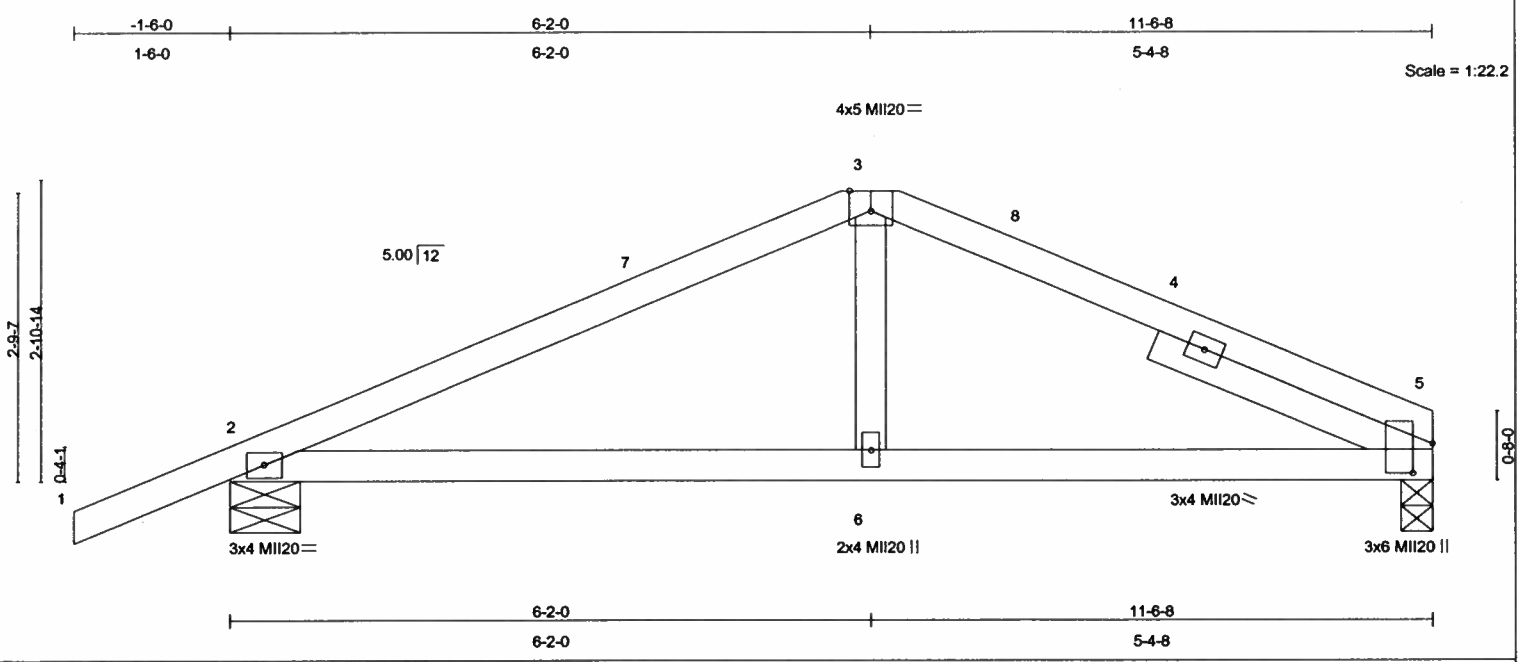

Plate Offsets (X,Y): [3:0-2-8,Edge], [5:0-3-7,0-2-3]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.06	2-6	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.07	2-6	>999	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.01	5	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 46 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 4 SYP No.3 2-10-4

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=439/0-3-8, 2=567/0-8-0
Max Horz 2=84(load case 5)
Max Uplift5=-373(load case 6), 2=-516(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-7=-672/519, 3-7=-580/532, 3-8=-549/532, 4-8=-597/522, 4-5=-659/514
BOT CHORD 2-6=-398/551, 5-6=-398/551
WEBS 3-6=-150/247

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 373 lb uplift at joint 5 and 516 lb uplift at joint 2.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	0 0 T2113556
ONTAYLOR	C3	ROOF TRUSS	2	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:18 2006 Page 1

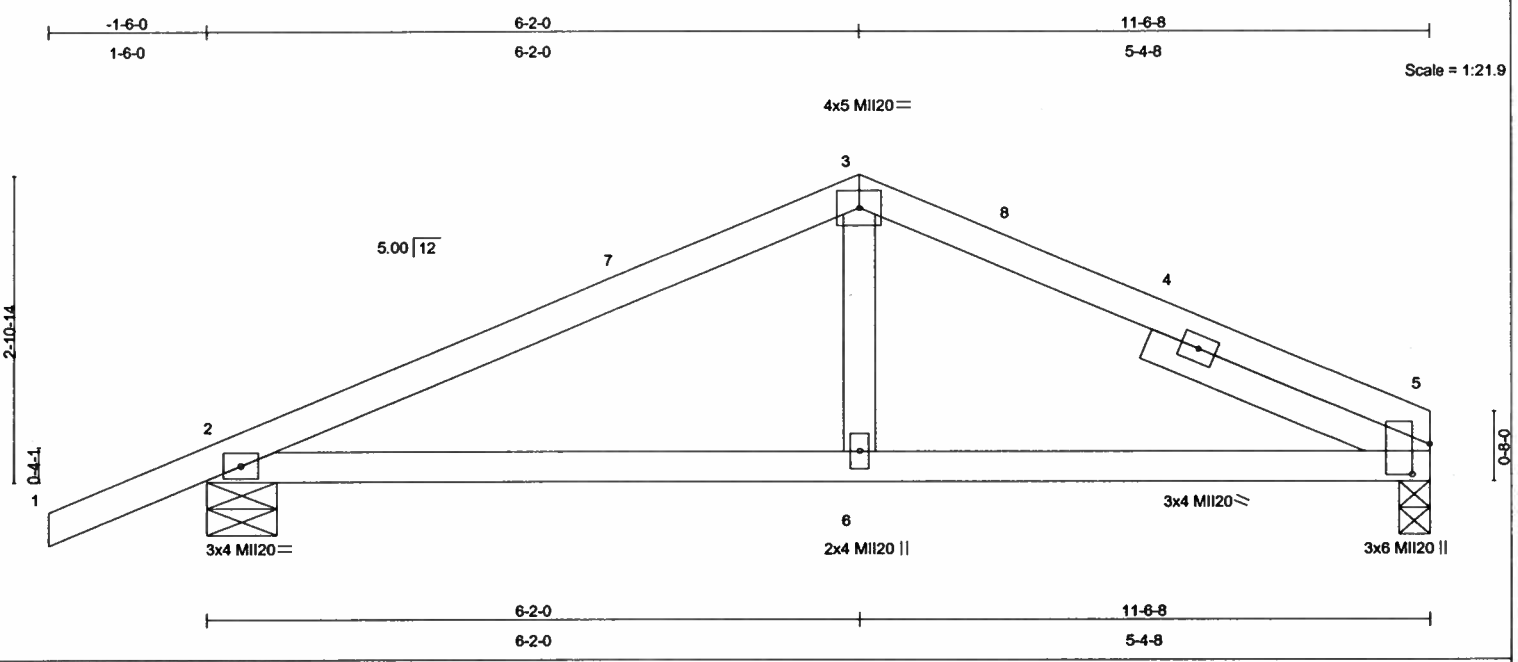

Plate Offsets (X,Y): [5:0-3-8,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.06	2-6	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.07	2-6	>999	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.01	5	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 46 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 4 SYP No.3 2-10-4

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=439/0-3-8, 2=567/0-8-0
Max Horz 2=85(load case 5)
Max Uplift5=-373(load case 6), 2=-516(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-7=-671/519, 3-7=-580/532, 3-8=-549/532, 4-8=-597/522, 4-5=-658/514
BOT CHORD 2-6=-398/551, 5-6=-398/551
WEBS 3-6=-150/247

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 373 lb uplift at joint 5 and 516 lb uplift at joint 2.

LOAD CASE(S) Standard

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March 30,2006

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MiTek®

Job	Truss	Truss Type	Qty	Ply	0 0 T2113557
ONTAYLOR	C4	ROOF TRUSS	1	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:19 2006 Page 1

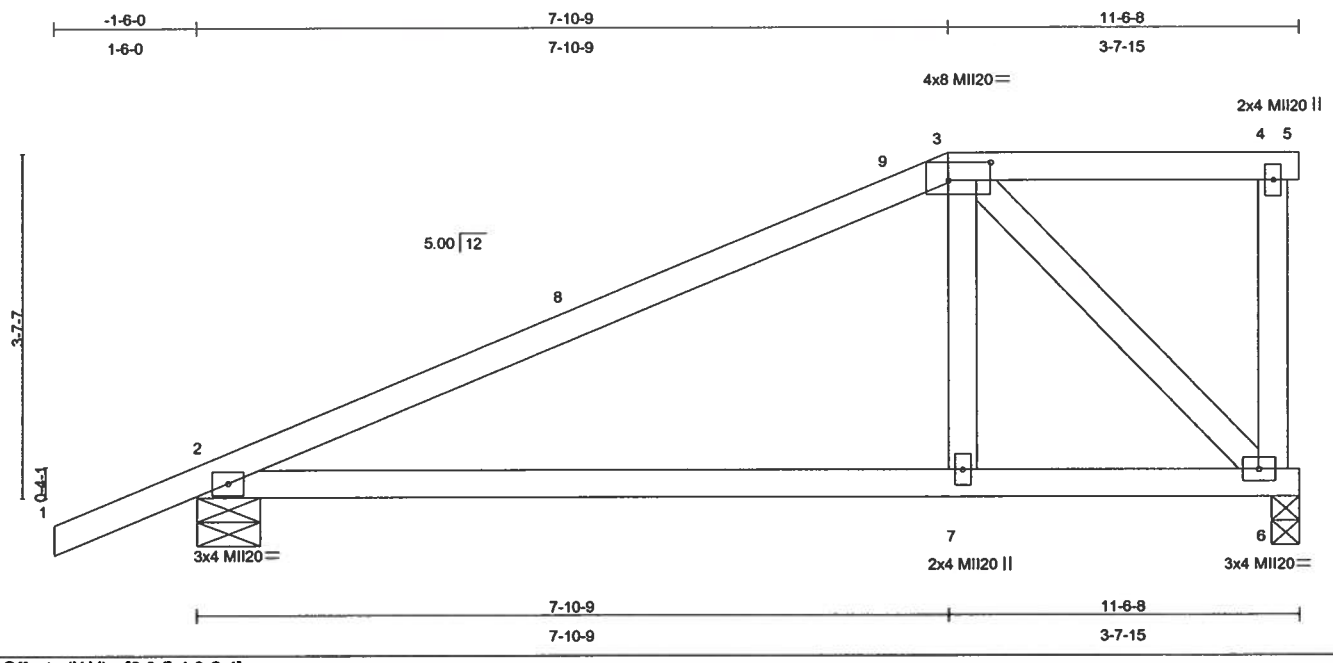

Plate Offsets (X,Y): [3:0-5-4,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.18	2-7	>733	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.21	2-7	>618	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.23	Horz(TL)	0.01	6	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 53 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=434/0-3-8, 2=557/0-8-0
Max Horz 2=225(load case 4)
Max Uplift6=-389(load case 6), 2=-498(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-8=-536/354, 8-9=-446/368, 3-9=-354/370, 3-4=-46/44, 4-5=0/0, 4-6=-22/35
BOT CHORD 2-7=-370/413, 6-7=-377/421
WEBS 3-7=-195/301, 3-6=-606/552

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 389 lb uplift at joint 6 and 498 lb uplift at joint 2.

LOAD CASE(S) Standard

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March 30,2006

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MiTek®

Job	Truss	Truss Type	Qty	Ply	0 0
ONTAYLOR	C5	ROOF TRUSS	1	1	T2113558
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:20 2006 Page 1

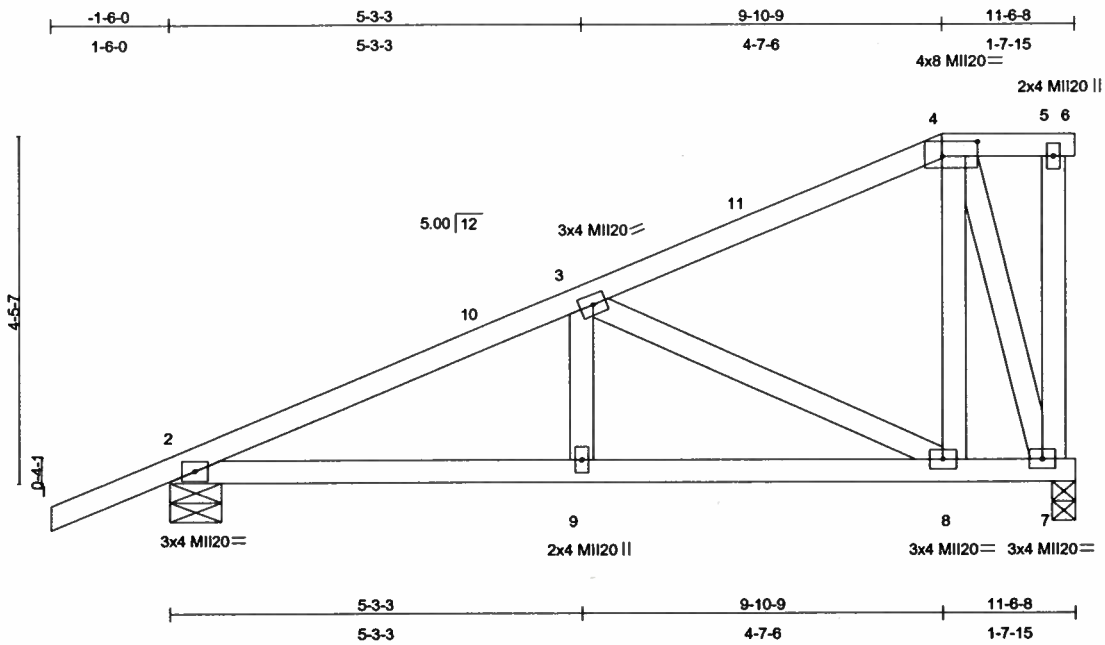

Plate Offsets (X,Y): [4:0-5-4,0-2-4]

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.20	Vert(LL)	0.04	2-9	>999	240	MII20	249/190
TCDL	10.0		Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.04	2-9	>999	180		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.23	Horz(TL)	0.01	7	n/a	n/a		
BCDL	10.0		Code FBC2004/TPI2002		(Matrix)							Weight: 66 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-2-14 oc bracing.

REACTIONS (lb/size) 7=434/0-3-8, 2=557/0-8-0
Max Horz 2=277(load case 4)
Max Uplift7=-397(load case 6), 2=-495(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-10=-692/479, 3-10=-582/491, 3-11=-208/146, 4-11=-151/156, 4-5=-45/66, 5-6=0/0, 5-7=-8/33
BOT CHORD 2-9=-488/577, 8-9=-488/577, 7-8=-164/149
WEBS 3-9=-120/212, 3-8=-483/466, 4-8=-304/305, 4-7=-456/429

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 397 lb uplift at joint 7 and 495 lb uplift at joint 2.

LOAD CASE(S) Standard

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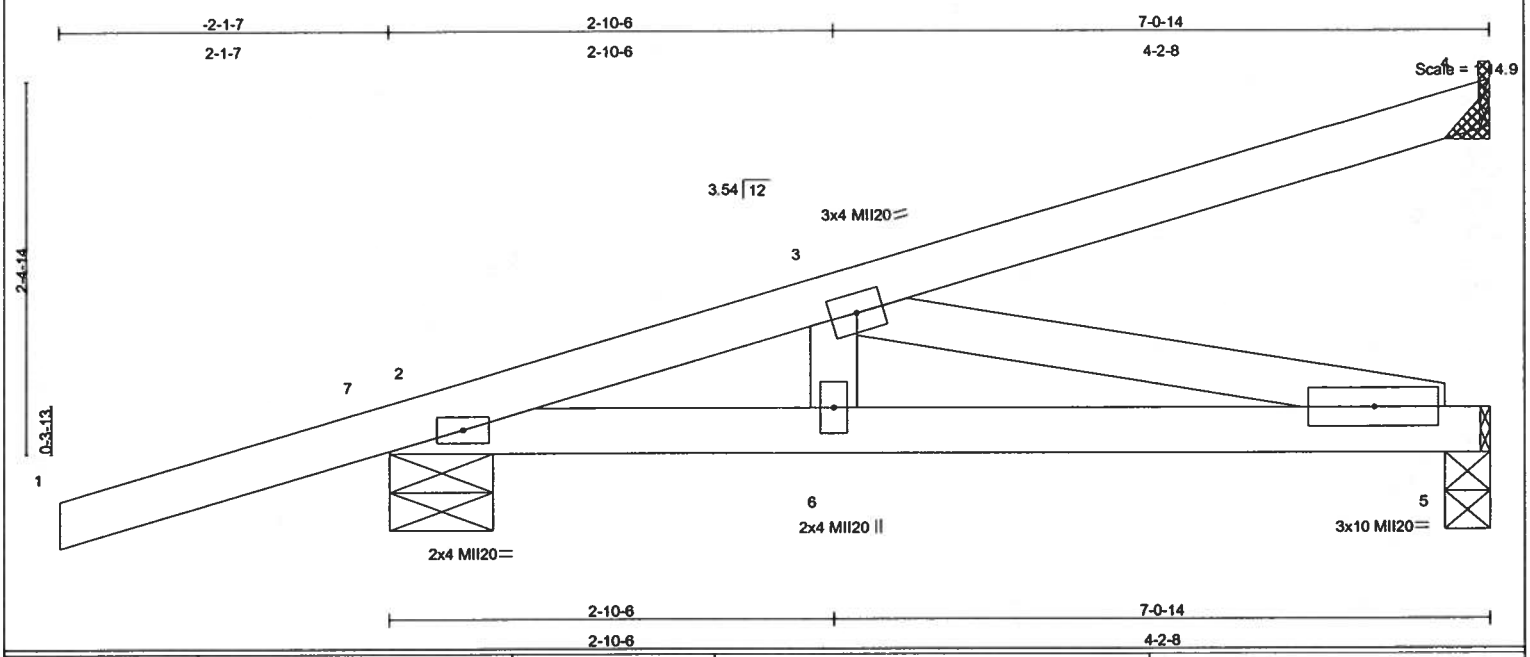
March 30,2006

Job	Truss	Truss Type	Qty	Ply	0 0
ONTAYLOR	CJO1	JACK	1	1	T2113559
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:20 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.05	5-6	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.06	5-6	>999	180		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.10	Horz(TL)	0.00	5	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 31 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=12/Mechanical, 2=307/0-8-0, 5=264/0-3-8
Max Horz 2=-72(load case 7)
Max Uplift4=-5(load case 3), 2=-362(load case 3), 5=-207(load case 3)
Max Grav4=64(load case 7), 2=307(load case 1), 5=264(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-7=0/30, 2-7=0/30, 2-3=-360/296, 3-4=-1/18
BOT CHORD 2-6=-232/313, 5-6=-232/313
WEBS 3-6=-284/278, 3-5=-321/237

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 4, 362 lb uplift at joint 2 and 207 lb uplift at joint 5.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-7=-60
Trapezoidal Loads (plf)
Vert: 2=-0(F=10, B=10)-to-5=-141(F=-61, B=-61)

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March 30,2006

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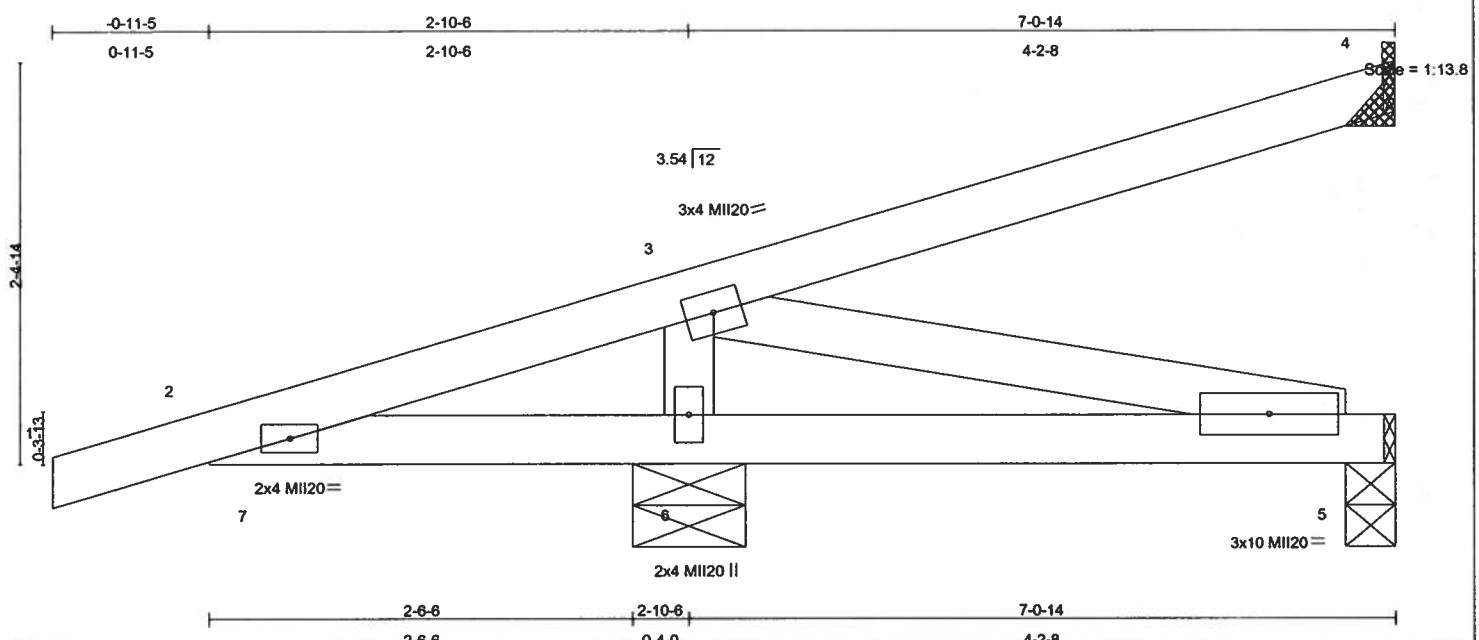
Job	Truss	Truss Type	Qty	Ply	0 0 T2113560
ONTAYLOR	CJO2	JACK	1	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:21 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	0.04	5-6	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	-0.04	5-6	>999	180		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.05	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 30 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=-1/Mechanical, 6=366/0-8-0, 5=154/0-3-8
Max Horz 6=-73(load case 7)
Max Uplift 4=-1(load case 2), 6=-289(load case 3), 5=-176(load case 7)
Max Grav 4=65(load case 7), 6=366(load case 1), 5=154(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/15, 2-3=-153/163, 3-4=-0/18
BOT CHORD 2-7=-159/170, 6-7=-159/170, 5-6=-159/159
WEBS 3-6=-231/47, 3-5=-163/162

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1 lb uplift at joint 4, 289 lb uplift at joint 6 and 176 lb uplift at joint 5.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-60, 2-7=-20
Trapezoidal Loads (plf)
Vert: 7=0(F=10, B=10)-to-5=-141(F=-61, B=-61)

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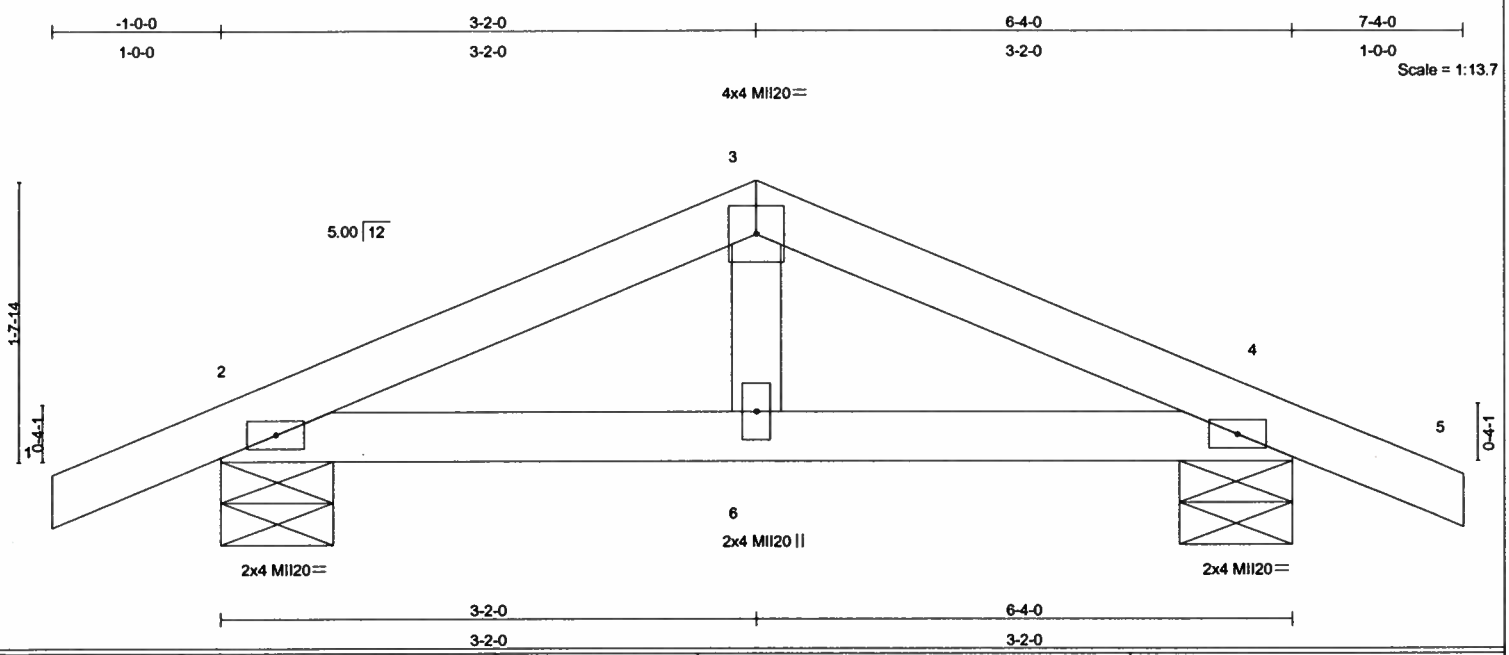
Job	Truss	Truss Type	Qty	Ply	0 0
ONTAYLOR	D	ROOF TRUSS	3	1	T2113561
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:22 2006 Page 1



LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.11	Vert(LL)	0.01	6	>999	240	MII20	249/190
TCDL	10.0		Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-6	>999	180		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	4	n/a	n/a		
BCDL	10.0		Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=307/0-8-0, 4=307/0-8-0
Max Horz 2=-18(load case 4)
Max Uplift2=-298(load case 5), 4=-298(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/22, 2-3=-274/213, 3-4=-274/213, 4-5=0/22
BOT CHORD 2-6=-120/207, 4-6=-120/207
WEBS 3-6=-73/136

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS automatic zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 298 lb uplift at joint 2 and 298 lb uplift at joint 4.

LOAD CASE(S) Standard

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March 30,2006

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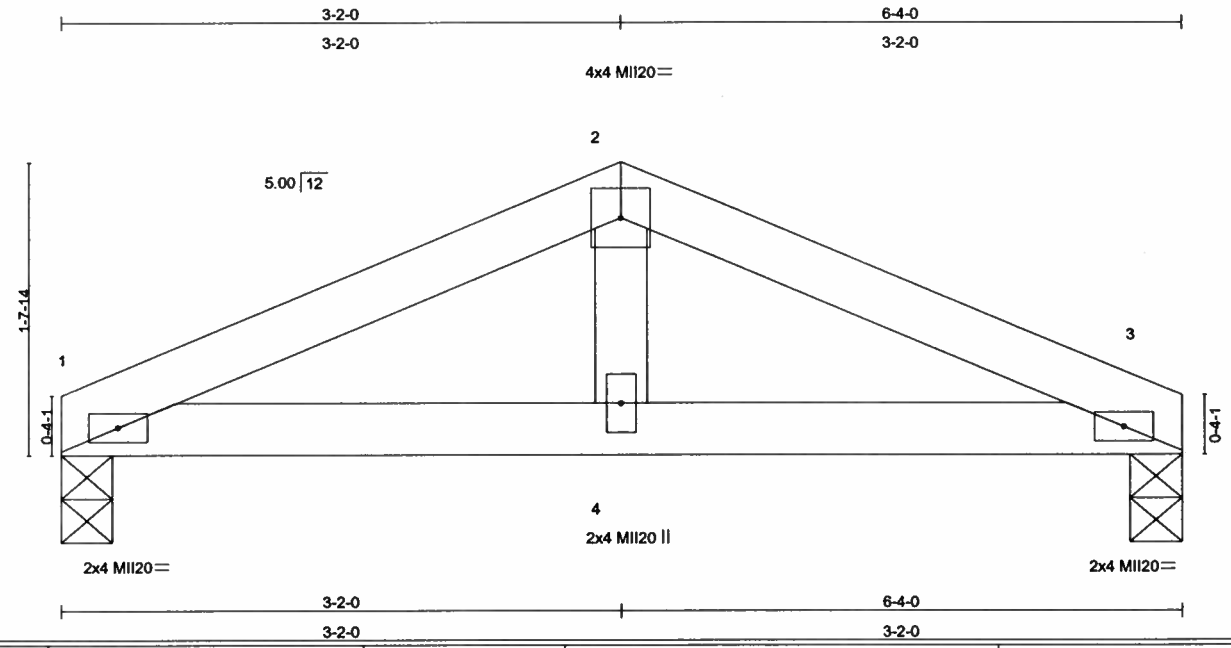
Job	Truss	Truss Type	Qty	Ply	0 0 T2113562
ONTAYLOR	D1	ROOF TRUSS	2	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:23 2006 Page 1



LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.14	Vert(LL)	0.01	4	>999	240	MII20	249/190
TCDL	10.0		Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	1-4	>999	180		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	3	n/a	n/a		
BCDL	10.0		Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=242/0-3-8, 3=242/0-3-8
Max Horz 1=4(load case 3)
Max Uplift1=-210(load case 5), 3=-210(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-335/275, 2-3=-335/275
BOT CHORD 1-4=-208/279, 3-4=-208/279
WEBS 2-4=-78/145

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS automatic zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 210 lb uplift at joint 1 and 210 lb uplift at joint 3.

LOAD CASE(S) Standard

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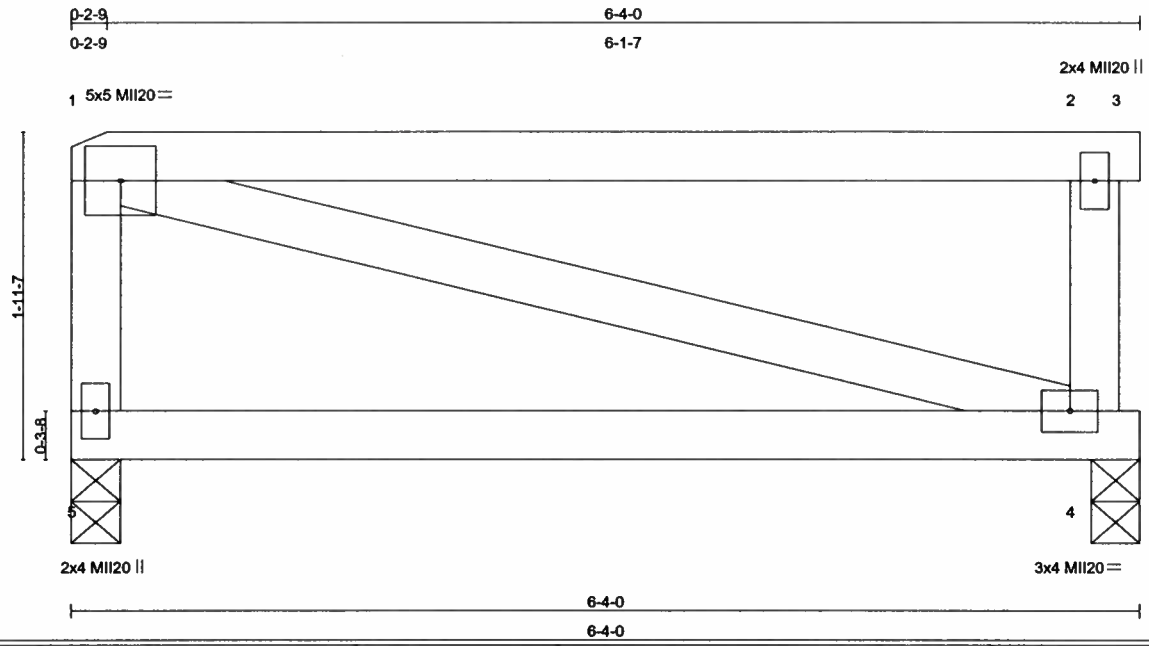
Job	Truss	Truss Type	Qty	Ply	0 0
ONTAYLOR	D2	ROOF TRUSS	1	1	T2113563
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:23 2006 Page 1



LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.42	Vert(LL)	0.10	4-5	>743	240	MII20	249/190
TCDL	10.0		Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.15	4-5	>469	180		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	4	n/a	n/a		
BCDL	10.0		Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-4-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=242/0-3-8, 5=237/0-3-8
Max Horz 5=-90(load case 3)
Max Uplift4=-230(load case 4), 5=-219(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-20/25, 2-3=0/0, 2-4=-183/153, 1-5=-177/155
BOT CHORD 4-5=-65/71
WEBS 1-4=-47/47

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS automatic zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 4 and 219 lb uplift at joint 5.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	0 0
ONTAYLOR	D3	ROOF TRUSS	1	1	T2113564
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:24 2006 Page 1

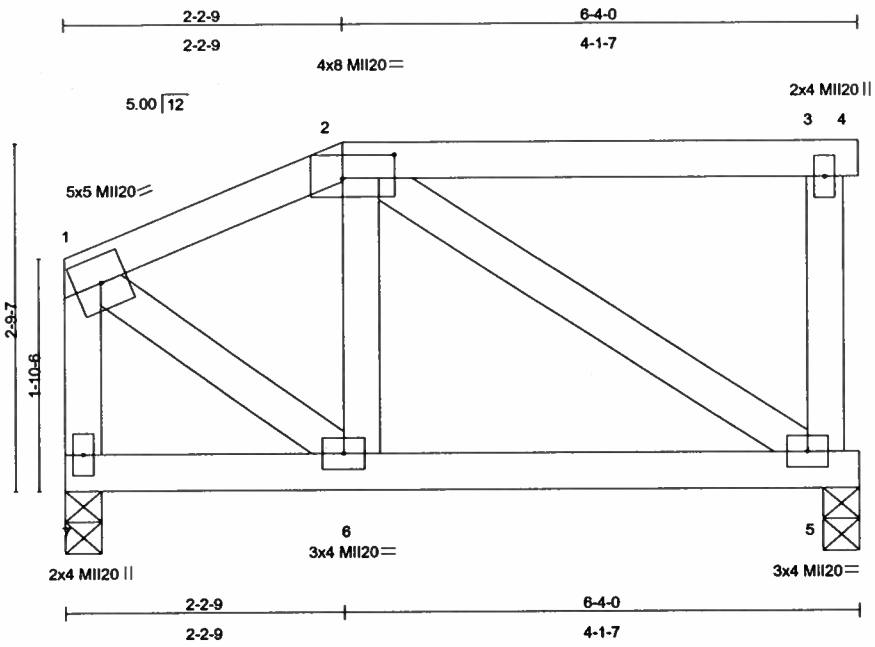

Plate Offsets (X,Y): [2:0-5-0,0-2-4]

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.18	Vert(LL)	0.01	5-6	>999	240	MII20	249/190
TCDL	10.0		Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.02	5-6	>999	180		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.06	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	10.0		Code FBC2004/TPI2002		(Matrix)							Weight: 37 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=242/0-3-8, 7=237/0-3-8
Max Horz 7=143(load case 4)
Max Uplift5=-241(load case 4), 7=-197(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-172/163, 2-3=-30/38, 3-4=0/0, 3-5=-121/104, 1-7=-224/184
BOT CHORD 6-7=-118/59, 5-6=-190/139
WEBS 2-6=-32/68, 2-5=-166/191, 1-6=-152/174

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS automatic zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 241 lb uplift at joint 5 and 197 lb uplift at joint 7.

LOAD CASE(S) Standard

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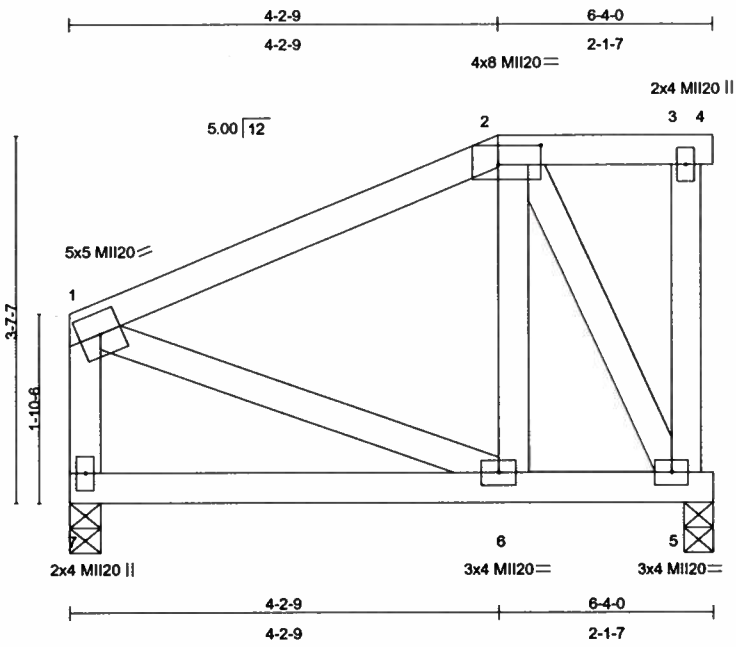
Job	Truss	Truss Type	Qty	Ply	0 0
ONTAYLOR	D4	ROOF TRUSS	1	1	T2113565
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:25 2006 Page 1



Scale = 1:22.8

Plate Offsets (X,Y): [2:0-5-0,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	0.02	6-7	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.02	6-7	>999	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.07	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 42 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=242/0-3-8, 7=237/0-3-8
Max Horz 7=194(load case 4)
Max Uplift5=-246(load case 4), 7=-185(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-145/111, 2-3=-40/51, 3-4=0/0, 3-5=-61/56, 1-7=-201/147
BOT CHORD 6-7=-169/50, 5-6=-146/101
WEBS 2-6=-102/138, 2-5=-207/218, 1-6=-77/106

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS automatic zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 246 lb uplift at joint 5 and 185 lb uplift at joint 7.

LOAD CASE(S) Standard

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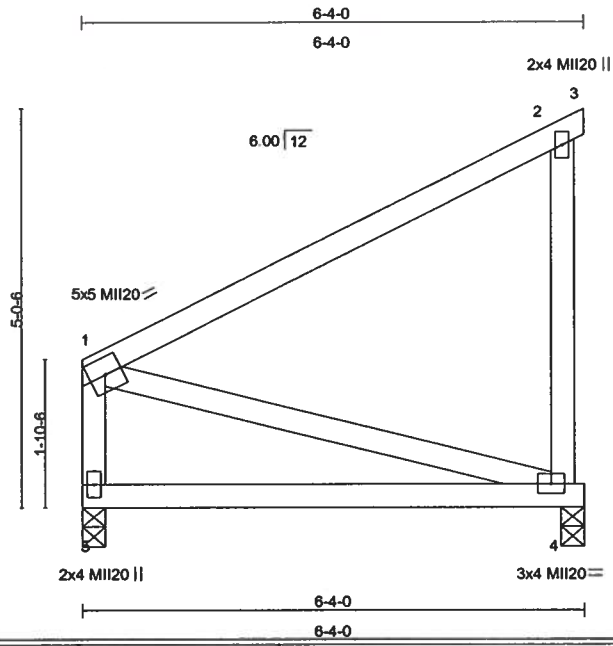
Job	Truss	Truss Type	Qty	Ply	0 0 T2113566
ONTAYLOR	D5	MONO TRUSS	1	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL , PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:27 2006 Page 1



Scale = 1:29.2

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.42	Vert(LL)	0.10	4-5	>743	240	MII20	249/190
TCDL	10.0		Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.15	4-5	>469	180		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.06	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	10.0		Code FBC2004/TPI2002		(Matrix)							Weight: 37 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=242/0-3-8, 5=237/0-3-8
Max Horz 5=267(load case 4)
Max Uplift4=-267(load case 5), 5=-156(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-134/64, 2-3=-2/0, 2-4=-183/166, 1-5=-177/92
BOT CHORD 4-5=-242/108
WEBS 1-4=-39/195

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 267 lb uplift at joint 4 and 156 lb uplift at joint 5.

LOAD CASE(S) Standard

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March 30,2006

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Job	Truss	Truss Type	Qty	Ply	0 0 T2113567
ONTAYLOR	DET	KINGPOST	1	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:28 2006 Page 1

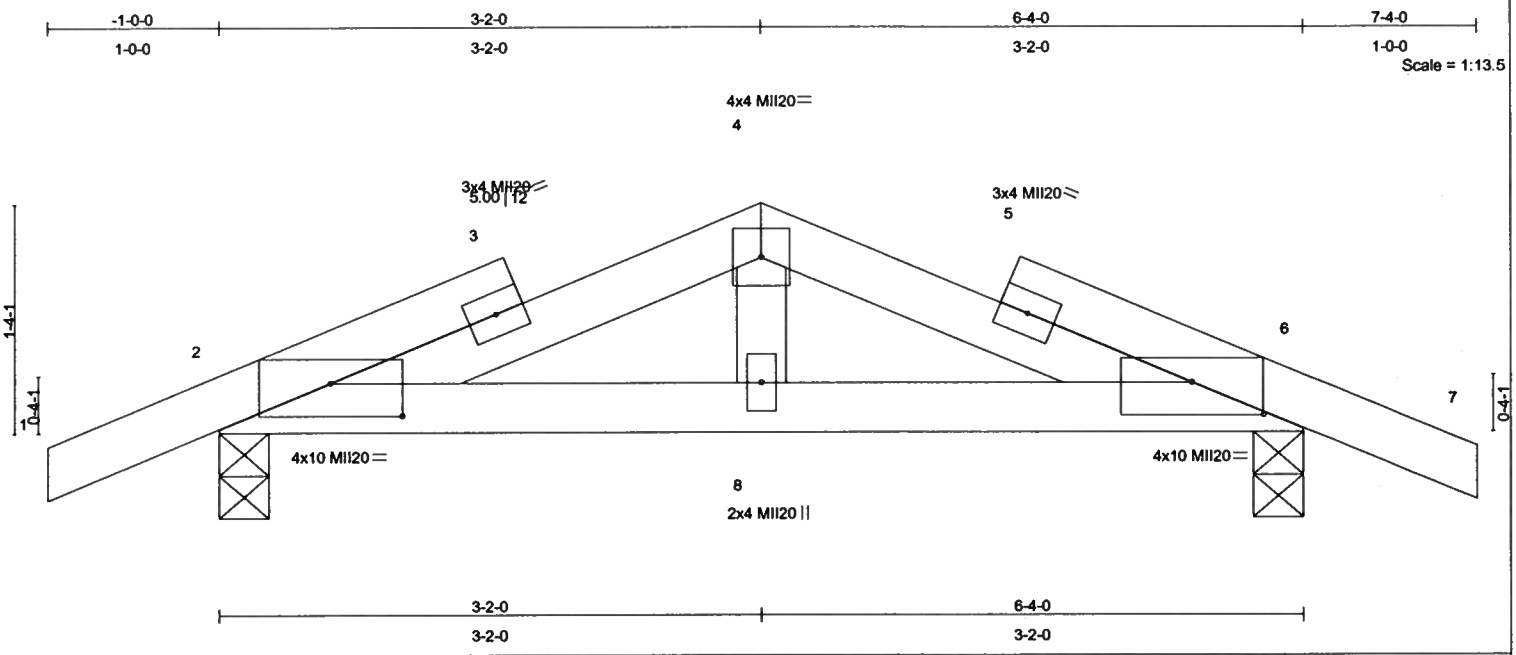


Plate Offsets (X,Y): [2:0-5-0,0-2-5], [6:0-5-0,0-2-5]

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.16	Vert(LL)	0.01	8	>999	240	MII20	249/190
TCDL	10.0		Lumber Increase	1.25	BC	0.10	Vert(TL)	0.01	8	>999	180		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	6	n/a	n/a		
BCDL	10.0		Code FBC2004/TPI2002		(Matrix)							Weight: 28 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=310/0-3-8, 6=310/0-3-8
Max Horz 2=-17(load case 4)
Max Uplift2=-296(load case 5), 6=-296(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/22, 2-3=-365/292, 3-4=-338/297, 4-5=-338/298, 5-6=-365/293, 6-7=0/22
BOT CHORD 2-8=-228/319, 6-8=-228/319
WEBS 4-8=-75/142

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS automatic zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 2 and 296 lb uplift at joint 6.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744
MiTek Industries, Inc.
1801 Massaro Blvd
Tampa FL 33619
FL Cert.#6634

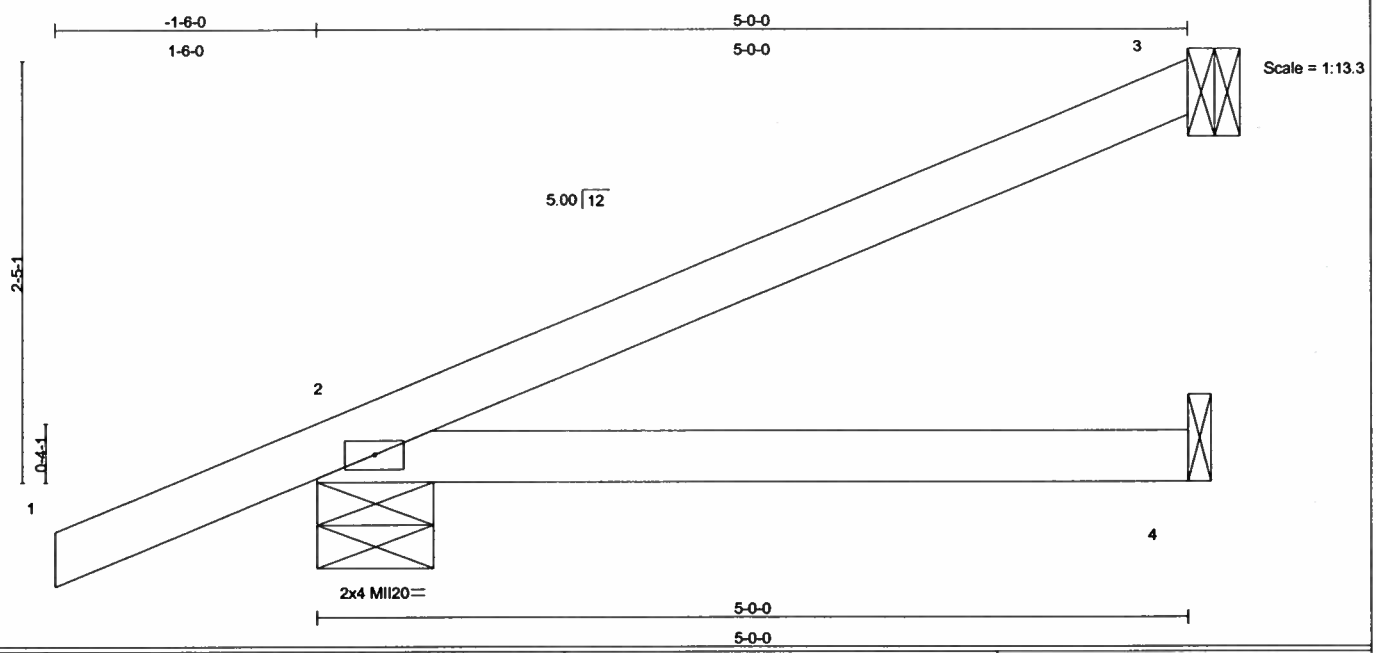
March 30,2006

1801 Massaro Blvd.
Tampa, FL 33619

MiTek®

Job	Truss	Truss Type	Qty	Ply	0 0
ONTAYLOR	EJ5	ROOF TRUSS	3	1	T2113568
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:29 2006 Page 1



LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.20	Vert(LL)	0.04	2-4	>999	240	MII20	249/190
TCDL	10.0		Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.06	2-4	>996	180		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0		Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=113/Mechanical, 2=314/0-8-0, 4=46/Mechanical
Max Horz 2=167(load case 5)
Max Uplift 3=-106(load case 5), 2=-292(load case 5), 4=-50(load case 3)
Max Grav 3=113(load case 1), 2=314(load case 1), 4=92(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-65/36
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 106 lb uplift at joint 3, 292 lb uplift at joint 2 and 50 lb uplift at joint 4.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744
MiTek Industries, Inc.
1801 Massaro Blvd
Tampa FL 33619
FL Cert #6634

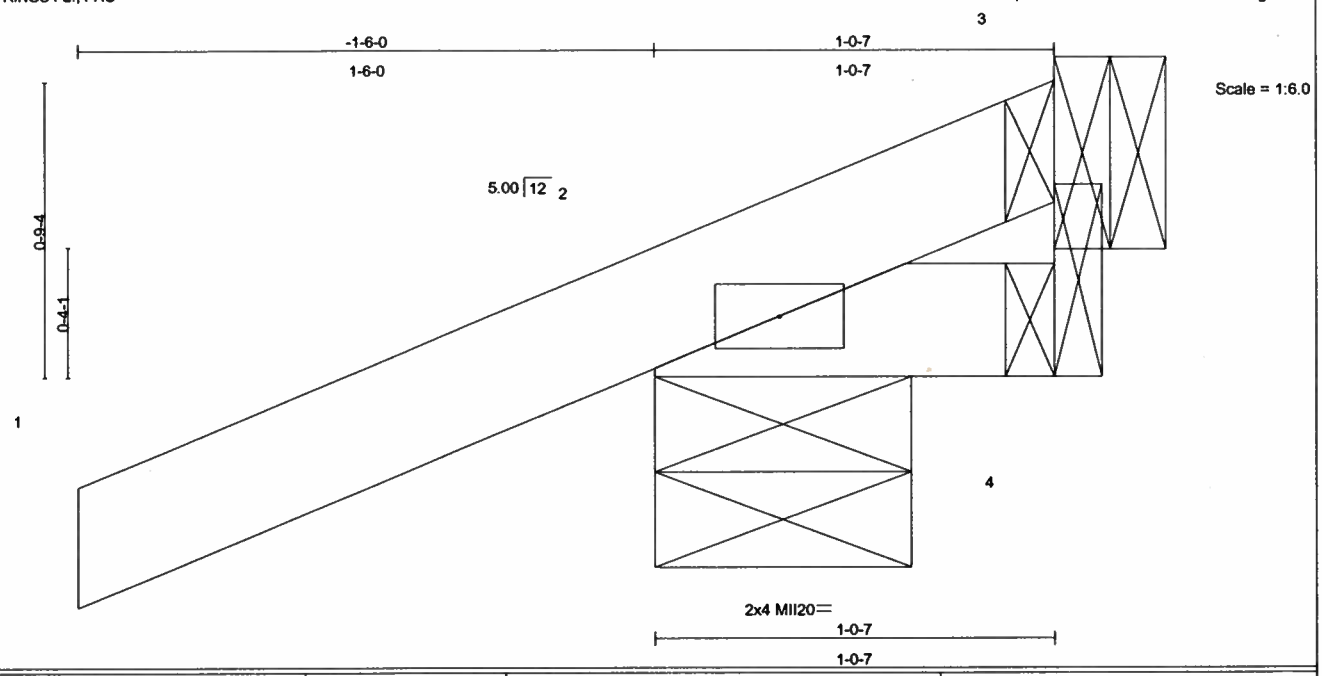
March 30,2006

1801 Massaro Blvd.
Tampa, FL 33619

MiTek®

Job	Truss	Truss Type	Qty	Ply	0 0
ONTAYLOR	J1	ROOF TRUSS	2	1	T2113569
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:30 2006 Page 1



LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.16	Vert(LL)	-0.00	2	>999	240	MII20	249/190
TCDL	10.0		Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0		Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=210/0-8-0, 4=10/Mechanical, 3=-50/Mechanical
Max Horz 2=74(load case 5)
Max Uplift2=-231(load case 5), 3=-50(load case 1)
Max Grav 2=210(load case 1), 4=19(load case 2), 3=79(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-46/34
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 2 and 50 lb uplift at joint 3.

LOAD CASE(S) Standard

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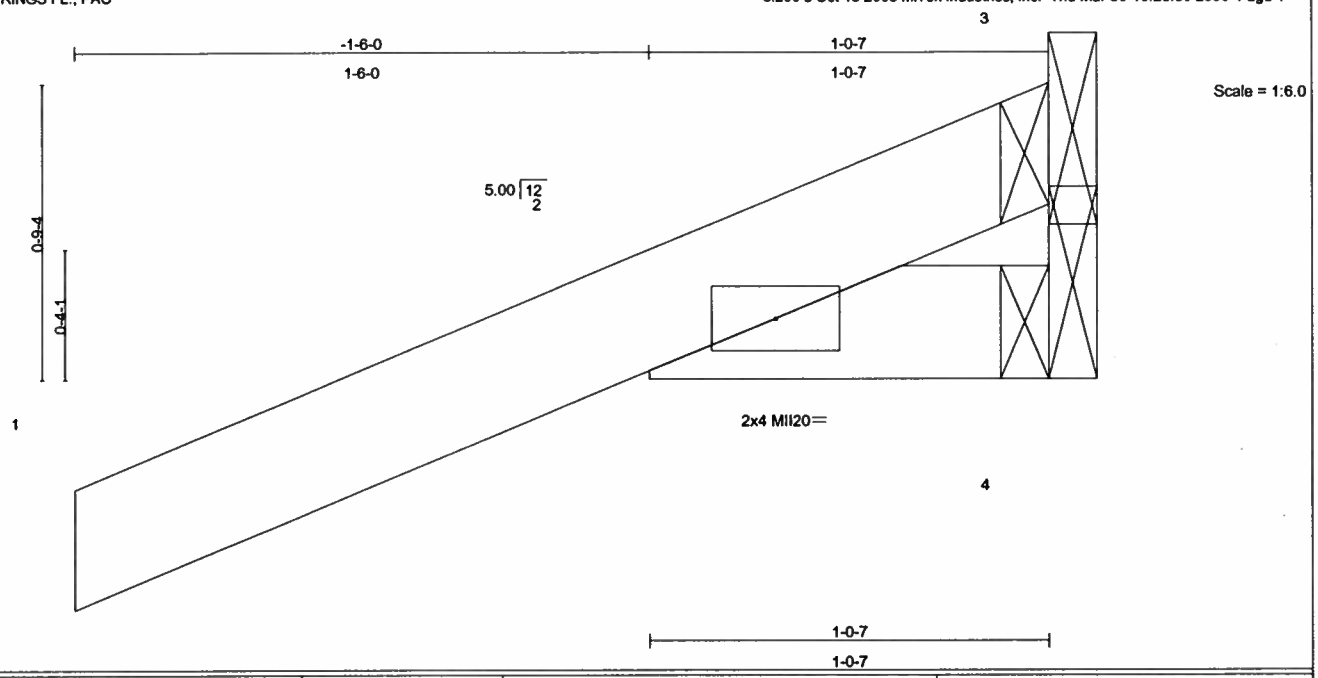
1801 Massaro Blvd
Tampa FL 33619
FL Cert #6634

March 30,2006

Job	Truss	Truss Type	Qty	Ply	0 0
ONTAYLOR	J1A	ROOF TRUSS	2	1	T2113570
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:30 2006 Page 1



LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.18	Vert(LL)	-0.00	2	n/r	120	MII20	249/190
TCDL	10.0		Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.00	2	n/r	120		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00		n/a	n/a		
BCDL	10.0		Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=10/Mechanical, 3=162/Mechanical
Max Horz 4=549(load case 5), 3=-474(load case 5)
Max Uplift3=-151(load case 5)
Max Grav 4=20(load case 2), 3=162(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-498/464
BOT CHORD 2-4=-439/549

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 151 lb uplift at joint 3.
6) Non Standard bearing condition. Review required.
7) Provide a horizontal connection for +/- 681 lbs at joint 3&4.
LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744
MiTek Industries, Inc.
1801 Massaro Blvd
Tampa FL 33619
FL Cert.#6634

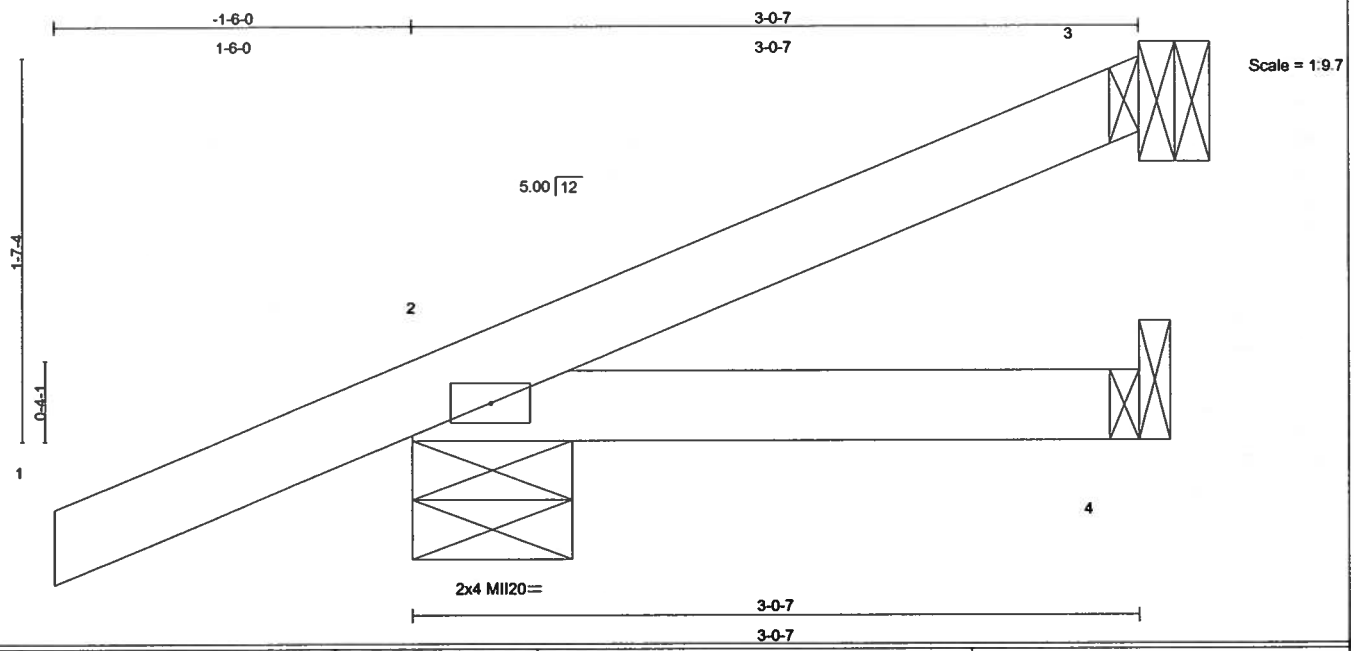
March 30,2006

1801 Massaro Blvd.
Tampa, FL 33619

MiTek®

Job	Truss	Truss Type	Qty	Ply	0 0 T2113571
ONTAYLOR	J3	ROOF TRUSS	2	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL , PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:31 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.00	2-4	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=37/Mechanical, 2=253/0-8-0, 4=26/Mechanical
Max Horz 2=120(load case 5)
Max Uplift3=-34(load case 6), 2=-256(load case 5), 4=-29(load case 3)
Max Grav 3=37(load case 1), 2=253(load case 1), 4=53(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-51/10
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 34 lb uplift at joint 3, 256 lb uplift at joint 2 and 29 lb uplift at joint 4.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744
MiTek Industries, Inc.
1801 Massaro Blvd
Tampa FL 33619
FL Cert.#6634

March 30,2006

Job	Truss	Truss Type	Qty	Ply	0 0
ONTAYLOR	J3A	ROOF TRUSS	2	1	T2113572
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., PAC — 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Mar 30 10:23:32 2006 Page 1

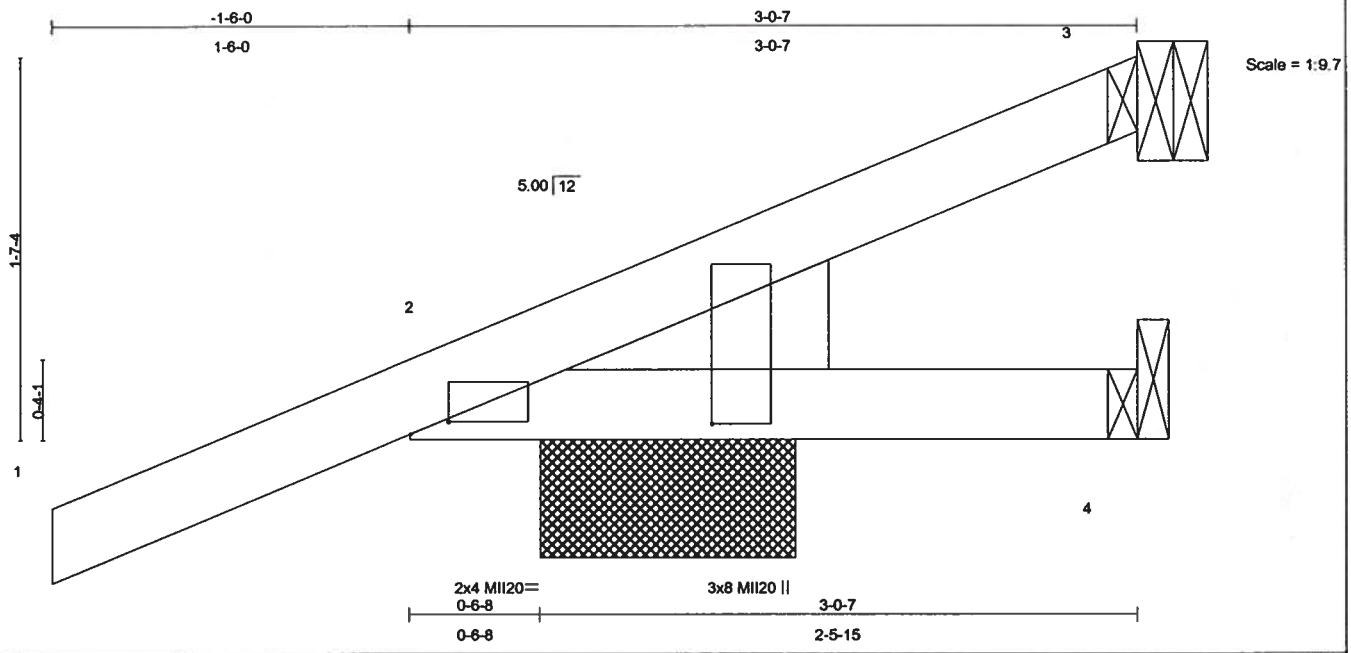


Plate Offsets (X,Y): [2:0-1-14,0-0-10], [2:0-0-8,1-3-2]

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.21	Vert(LL)	-0.00	2-4	>999	240	MII20	249/190
TCDL	10.0		Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0		Code FBC2004/TPI2002		(Matrix)							Weight: 15 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEDGE
Left: 2 X 6 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=37/Mechanical, 2=253/1-0-13, 4=26/Mechanical
Max Horz 2=120(load case 5)
Max Uplift3=-34(load case 6), 2=-256(load case 5), 4=-29(load case 3)
Max Grav3=37(load case 1), 2=253(load case 1), 4=53(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-51/10
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 34 lb uplift at joint 3, 256 lb uplift at joint 2 and 29 lb uplift at joint 4.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744
MiTek Industries, Inc.
1801 Massaro Blvd
Tampa FL 33619
FL Cert.#6634

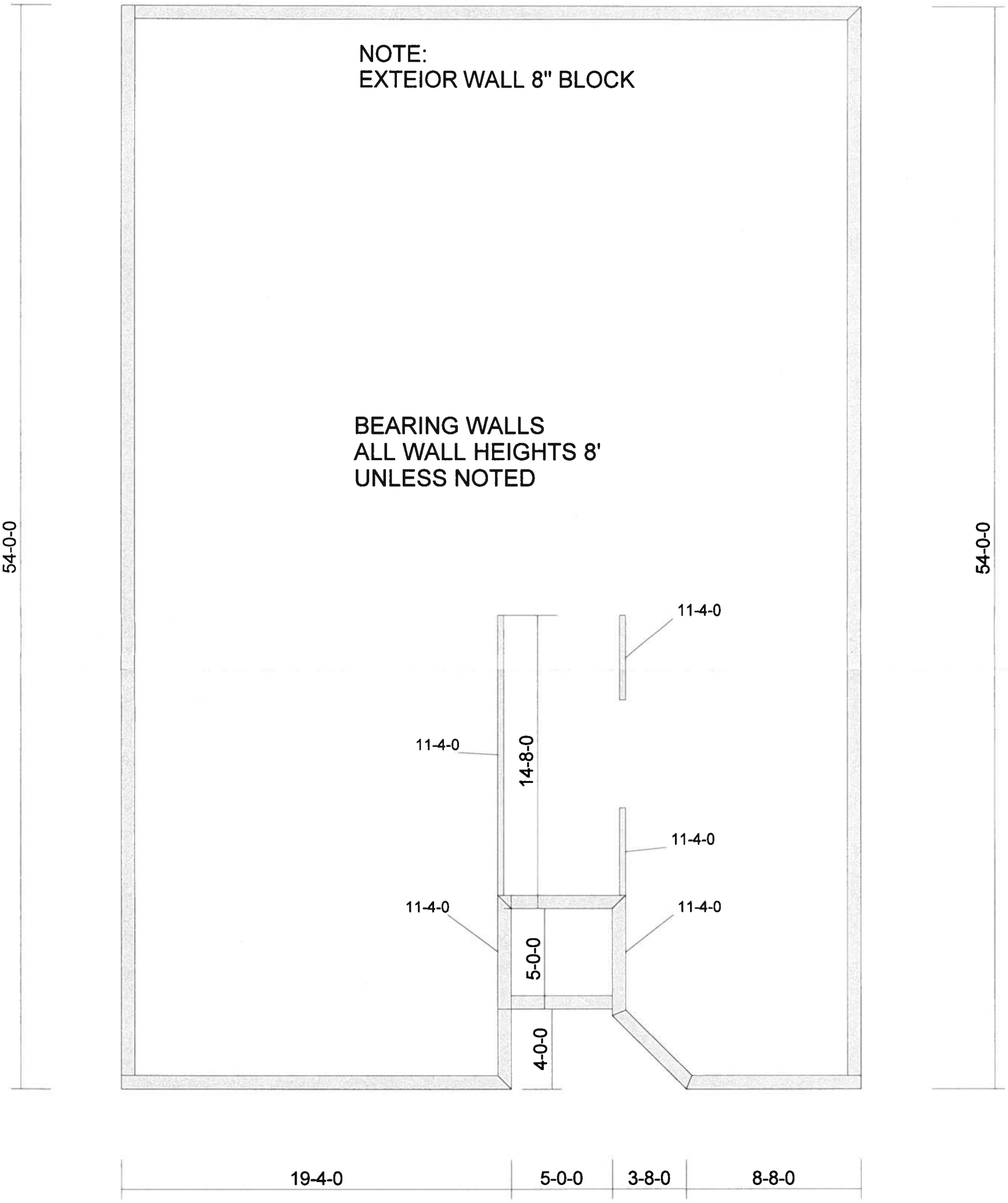
March 30,2006

1801 Massaro Blvd.
Tampa, FL 33619

MiTek®

NOTE:
EXTEIOR WALL 8" BLOCK

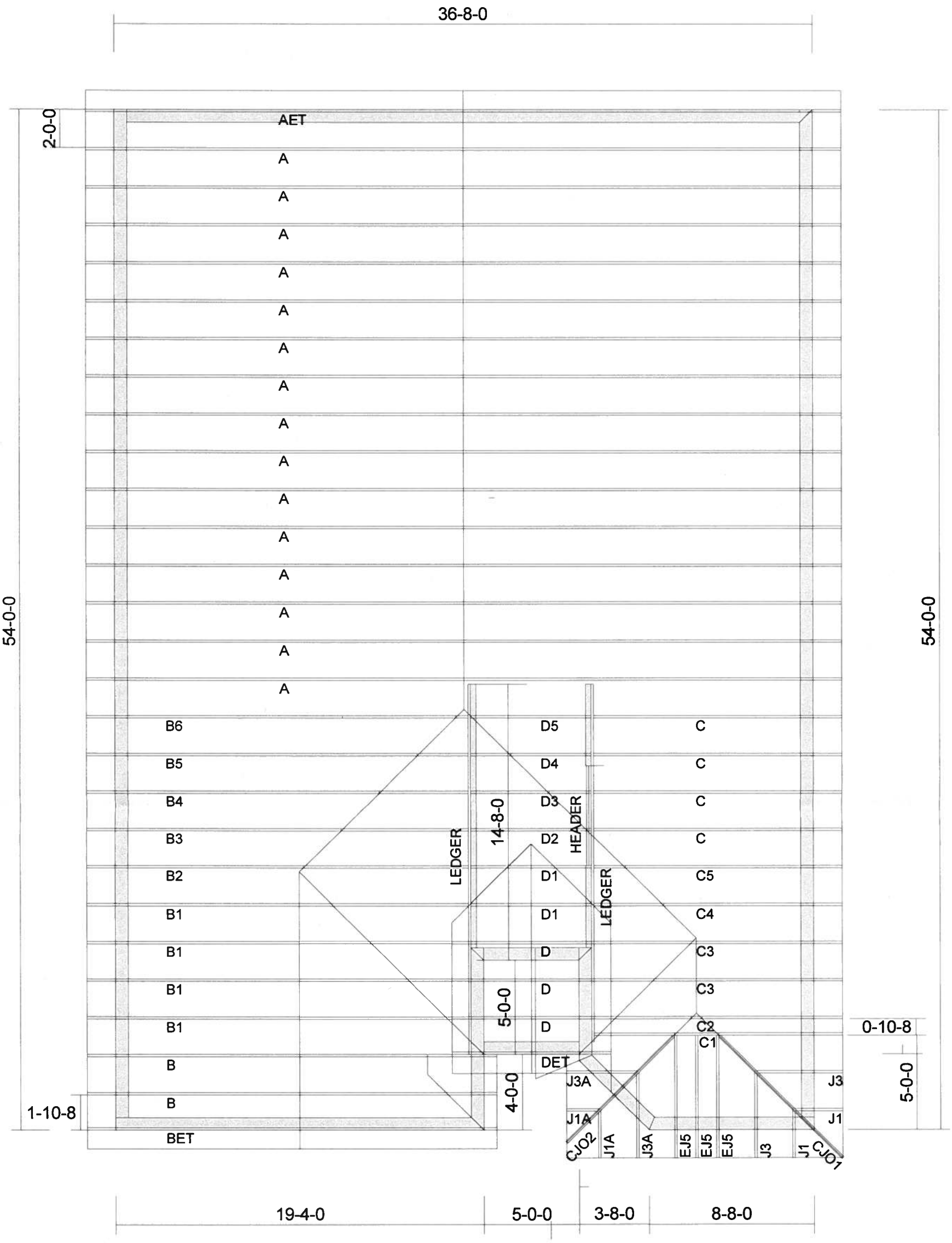
BEARING WALLS
ALL WALL HEIGHTS 8'
UNLESS NOTED



Santa Fe Truss

410 SW PCE SPRINGS RD.
HIGH SPRINGS, FL 32655
FX#(386)454-1055 PH#(386)454-7711

CUSTOMER	ONEIL CONSTRUCTION
DATE	3/30/2006
JOB NAME	TAYLOR (ONTAYLOR)
DESIGNER	P.A.C.



5/12 PITCH 1-6" OVERHANGS
ALL TRUSSES BEAR @ 8' EXCEPT
D TRUSSES BEAR @ 11'-4"
HANGERS HUS26 (20 COUNT)

Santa Fe Truss

410 SW POE SPRINGS RD.
HIGH SPRINGS, FL 32655
FX#(386)454-1055 PH#(386)454-7711

CUSTOMER	ONEIL CONSTRUCTION
DATE	3/30/2006
JOB NAME	TAYLOR (ONTAYLOR)
DESIGNER	P.A.C.