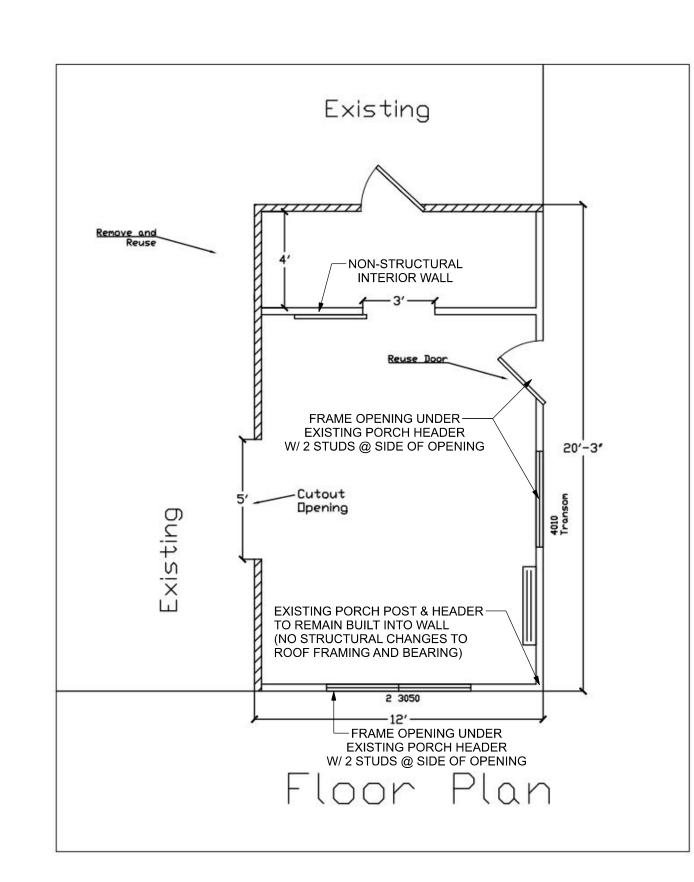
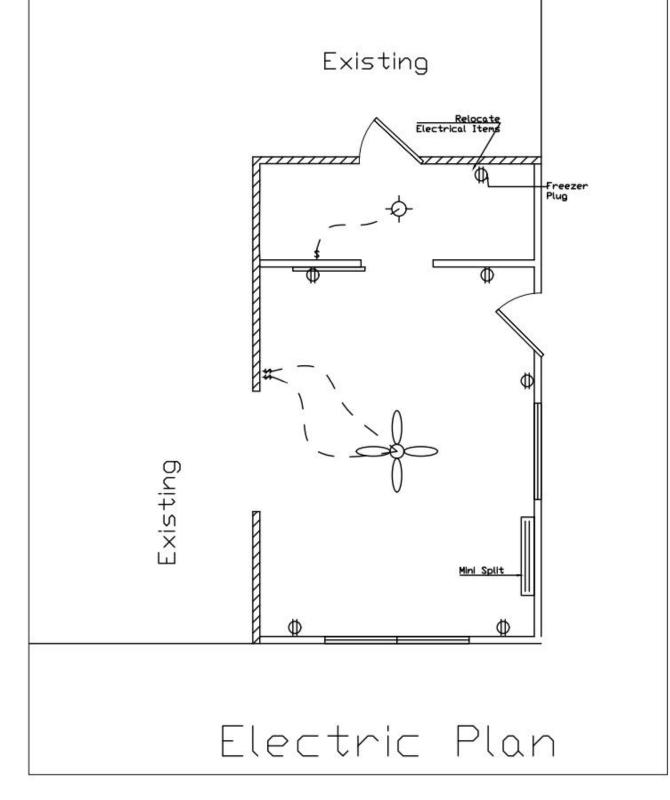


**EXISTING ELEVATION** 





ALL NEW OUTLETS TO BE PROTECTED BY A LISTED ARC-FAULT CIRCUIT INTERRUPTER, COMBINATION-TYPE INSTALLED TO PROVIDE PROTECTION OF THE BRANCH CIRCUIT.

FOUNDATION NOTE:
- BUILDER TO VERIFY THAT EXISTING
FOUNDATION HAS A CONTIONUS FOOTER
ALONG EDGE OF PORCH
- NO STRUCTURAL CHANGES TO
FOUNDATION ARE PART OF THIS PLAN
- IF NEEDED AN NEW SLAB CAN BE ADDED
OVER TOP OF EXISTING SLAB
- IF NEW SLAB IS LESS THAN 4" ATTACHMENT
OF NEW WALLS MUST BE INTO EXISTING
- FIELD VERIFY THAT THE FINIAL FOOTER
WILL BE 12" WIDE MIN. AND A MIN. OF12"
BELOW GRADE AND FINISHED SLAB
ELEVATION IS 8" ABOVE GRADE

| CONNECTOR TABLE |            |                       |                   |                                                |  |  |  |  |  |
|-----------------|------------|-----------------------|-------------------|------------------------------------------------|--|--|--|--|--|
| Uplift SP       | Uplift SPF | Truss Connector       | To Plate          | To Truss/Rafter                                |  |  |  |  |  |
| 805             | 505        | SDWC15600             | -                 | -                                              |  |  |  |  |  |
| 415             | 290        | H3                    | 4-8dx1 1/2"       | 4-8dx1 1/2"                                    |  |  |  |  |  |
| 615             | 540        | H2.5A                 | 5-8dx1 1/2"       | 5-8dx1 1/2"                                    |  |  |  |  |  |
| 1340            | 1015       | H10A                  | 9-10d1 1/2"       | 9-10d1 1/2"                                    |  |  |  |  |  |
| 720             | 620        | LTS12-20              | 6-10d1 1/2"       | 6-10d1 1/2"                                    |  |  |  |  |  |
| 1000            | 860        | MTS12-30              | 7-10d1 1/2"       | 7-10d1 1/2"                                    |  |  |  |  |  |
| 1450            | 1245       | HTS20-30              | 12-10d1 1/2"      | 12-10d1 1/2"                                   |  |  |  |  |  |
| Uplift SP       | Uplift SPF | Strap Ties            | To One Member     | To Other Member                                |  |  |  |  |  |
| 1235            | 1235       | LSTA21                | 8-10d             | 8-10d                                          |  |  |  |  |  |
| 1640            | 1455       | MSTA24                | 9-10d             | 9-10d                                          |  |  |  |  |  |
| 1030            | 1030       | CS20                  | 7-10d             | 7-10d                                          |  |  |  |  |  |
| Uplift SP       | Uplift SPF | Stud Plate Ties       | To Stud           | To Plate                                       |  |  |  |  |  |
| 585             | 535        | SP1                   | 6-10d             | 4-10d                                          |  |  |  |  |  |
| 1065            | 605        | SP2                   | 6-10d             | 6-10d                                          |  |  |  |  |  |
| 771             | 771        | LSTA24                | 10-10d            | wrap under or over plat                        |  |  |  |  |  |
| 1235            | 1235       | LSTA24                | 14-10d            | wrap under or over plate                       |  |  |  |  |  |
| Uplift SP       | Uplift SPF | Holdowns @ Stemwall   | To Stud / Post    | Anchor                                         |  |  |  |  |  |
| 1825            | 1800       | DTT2Z                 | 8-SDS 1/4"x1 1/2" | 1/2"x12" Titen HD                              |  |  |  |  |  |
| 4235            | 3640       | HTT4                  | 18-16dx2 1/2"     | 1/2"x12" Titen HD                              |  |  |  |  |  |
| Uplift SP       | Uplift SPF | Holdowns @ Mono       | To Stud / Post    | Anchor                                         |  |  |  |  |  |
| 1825            | 1800       | DTT2Z                 | 8-SDS 1/4"x1 1/2" | 1/2"x6" Titen HD                               |  |  |  |  |  |
| 4235            | 3640       | HTT4                  | 18-16dx2 1/2"     | 1/2"x12" Titen HD                              |  |  |  |  |  |
| Uplift SP       | Uplift SPF | Post Bases @ Stemwall | To Post           | Anchor                                         |  |  |  |  |  |
| 1900            |            | ABU44Z                | 12-16d            | 5/8"x12" Drill & Epoxy                         |  |  |  |  |  |
| 2475            |            | ABU66Z                | 12-16d            | 5/8"x12" Drill & Epoxy                         |  |  |  |  |  |
| Uplift SP       | Uplift SPF | Post Bases @ Mono     | To Post           | Anchor                                         |  |  |  |  |  |
|                 |            |                       |                   |                                                |  |  |  |  |  |
| 1900            |            | ABU44Z                | 12-16d            | 5/8"x7" Drill & Epoxy                          |  |  |  |  |  |
| 1900<br>2475    |            | ABU44Z<br>ABU66Z      | 12-16d<br>12-16d  | 5/8"x7" Drill & Epoxy<br>5/8"x7" Drill & Epoxy |  |  |  |  |  |

## **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE
REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE
OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

## BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

## EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC TO 10'-1" STUD HEIGHT

(1) 2x4 @ 12" OC TO 11'-2" STUD HEIGHT

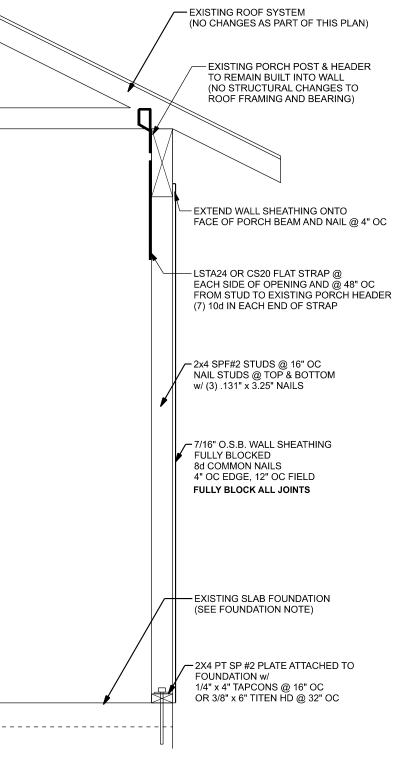
(1) 2x6 @ 16" OC TO 15'-7" STUD HEIGHT

TO 17'-3" STUD HEIGHT

## GRADE & SPECIES TABLE Fb E 2x8 SP #2 925 1.4 2x10 SP #2 800 1.4 2x12 SP #2 750 1.4 GLB 24F-V3 SP 2600 1.9

LSL TIMBERSTRAND 1700 1.7 LVL MICROLAM 2950 2.0 PSL PARALAM 2900 2.0

(1) 2x6 @ 12" OC



SCALE: 3/4" = 1'-0"

| DESIGN CRITERIA & LOADS:                                   |                                                           |  |
|------------------------------------------------------------|-----------------------------------------------------------|--|
| 8TH EDITION<br>FLORIDA BUILDING CODE RESIDENTIAL<br>(2023) | 8TH EDITION<br>FLORIDA BUILDING CODE RESIDENTI/<br>(2023) |  |
| CODE FOR DESIGN LOADS                                      | ASCE 7-22                                                 |  |
| WINDLOADS                                                  |                                                           |  |
| BASIC WIND SPEED<br>(ASCE 7-22, 3S GUST)                   | 130 MPH                                                   |  |
| WIND EXPOSURE<br>(BUILDER MUST FIELD VERIFY)               | С                                                         |  |
| TOPOGRAPHIC FACTOR<br>(BUILDER MUST FIELD VERIFY)          | I                                                         |  |
| RISK CATEGORY                                              | II                                                        |  |
| ENCLOSURE CLASSIFICATION                                   | ENCLOSED                                                  |  |
| INTERNAL PRESSURE<br>COEFFICIENT                           | 0.18                                                      |  |
| ROOF ANGLE                                                 | 7-45 DEGREES                                              |  |
| MEAN ROOF HEIGHT                                           | 30 FT                                                     |  |
| C&C DESIGN PRESSURES                                       | SEE TABLE                                                 |  |
| FLOOR LOADING                                              |                                                           |  |
| ROOMS OTHER THAN<br>SLEEPING ROOM                          | 40 PSF LIVE LOAD                                          |  |
| SLEEPING ROOMS                                             | 30 PSF LIVE LOAD                                          |  |
| ROOF LOADING                                               |                                                           |  |
| FLAT OR < 4:12                                             | 20 PSF LIVE LOAD                                          |  |
| 4:12 TO < 12:12                                            | 16 PSF LIVE LOAD                                          |  |
| 12:12 & GREATER                                            | 12 PSF LIVE LOAD                                          |  |
| SOIL BEARING CAPACITY                                      | 1500 PSF                                                  |  |
| FLOOD ZONE                                                 | THIS BUILDING IS NOT IN THE FLOOD                         |  |

| COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C) |             |                                             |             |             |  |  |  |  |
|------------------------------------------------------|-------------|---------------------------------------------|-------------|-------------|--|--|--|--|
| EFFECTIVE ZONE 4<br>WIND AREA (FT2) INTERIOR         |             | ZONE 5<br>END 4' FROM ALL<br>OUTSIDE CORNER |             |             |  |  |  |  |
| 0 - 20                                               | +25.6(Vasd) | -27.8(Vasd)                                 | +25.6(Vasd) | -34.2(Vasd) |  |  |  |  |
| 0 - 20                                               | +42.6(Vult) | -46.2(Vult)                                 | +42.6(Vult) | -57(Vult)   |  |  |  |  |

FL PE 53915

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida

Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E. 163 SW Midtown Place Suite 103 Lake City, Florida 32025 386.754.5419 disoswaydesign@gmail.com

JOB NUMBER: 250612 **S-1**OF 1 SHEET