

ERECTION NOTES

1. All bracing shown and provided by the Metal Building Provider (MBP) for this building is required and shall be installed by the erector as a permanent part of the structure ("Code of Standard Practice for Steel Buildings" in the ANSI/AISC 303–16; Section 7.10).
2. Temporary supports, such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined and furnished by the erector ("Code of Standard Practice for Steel Buildings and Bridges " in the ANSI/AISC 303–16; Section 7.10.3).
3. Normal erection operations include the correction of minor misfits by moderate amounts of reaming, grinding, welding or cutting, and the drawing of elements into line through use of drift pins. Errors which require major changes in the member configuration are to be reported immediately to the Metal Building Provider by the customer to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others ("Code of Standard Practice for Steel Buildings and Bridges "in the ANSI/AISC 303–16; Section 7.14).
4. Erection tolerances are set forth in the "Code of Standard Practice for Steel Buildings and Bridges "in the ANSI/AISC 303–16; Section 7.13 note that individual members are considered plump, level and aligned if the deviation does not exceed 1:500. Variations in finished overall dimensions of structure steel framing are deemed within the limits of good practice when they do not exceed the cumulative effect of rolling, fabricating, and erection tolerances.
- 4.1. When crane support systems are part of the metal building system erection tolerances Section 6.8, Erection Tolerances, 2018 MBMA Metal Building Systems manual shall apply. To achieve the required tolerances grouting of the columns and shimming of the runway beams may be required. The customer shall provide grout if required. The contractor erecting the runway beams is responsible for shimming, plumbing, and leveling of the runway system. When aligning the runway beams the alignment shall be with respect to the beam webs so that the center of the aligned rail is over the runway web.
5. As a general rule field welding is not used to assemble a metal building system. In cases where the drawings indicate field welding and in cases where approved corrections are to be made by field welding the following requirements shall be met;
- 5.1. welders must be qualified by an independent testing agency, with suitable documentation to AWS D1.1 Structural Welding Code – Steel or AWS D1.3 Structural Welding Code – Sheet as applicable, for the processes, positions, and materials involved.
- 5.2. All welds must be made in conformance to a documented and approved Welding Procedure Specification (WPS). All joints which are not prequalified must be supported by a certified Procedure Qualification Record (PQR) by an independent testing agency.
6. All documentation and records shall be the responsibility of the customer.
7. Any claims or shortages by buyer must be made to the Metal Building Provider within seven (7) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed. All claims should be directed to the Metal Building Provider's Customer Service Department.
8. Claims for correction of alleged misfits will be disallowed unless the Metal Building Provider shall have received prior notice thereof and allowed reasonable inspection of such misfits. Ordinary inaccuracies of shop work shall not be construed as misfits. No part of the building may be returned or charges assessed for alleged misfits without prior approval from the Metal Building Provider.
9. Neither the Metal Building Provider nor the customer will cut, drill or otherwise alter their work, or the work of other trades to accommodate other trades unless such work is clearly specified in the contract documents. Whenever such work is specified the customer is responsible for furnishing complete information as to materials, size, location, and number of alterations prior to preparation of shop drawings ("Code of Standard Practice for Steel Buildings and Bridges "in the ANSI/AISC 303–16, Section 7.15).
10. The Metal Building Provider Field Modifications Policy:
- 10.1. The Metal Building Provider will only be responsible for the field–modified parts designed and approved by the Metal Building Provider's Customer Service Department.
- 10.2. Any field modifications designed by third parties may not be approved by the Metal Building Provider and may limit the Metal Building Provider's warranty and liability.
- 10.3. The Metal Building Provider makes no warranty and hereby disclaims any responsibility with respect to the design, engineering, or construction of any field–modified parts performed by third parties.
11. WARNING – SOME PANELS AND TRIM PARTS ARE FURNISHED WITH A PROTECTIVE PEEL–OFF FILM. PARTS PROVIDED WITH THIS FILM CANNOT BE EXPOSED TO SUNLIGHT WITHOUT FIRST REMOVING THE FILM. THIS FILM MUST BE REMOVED PRIOR TO INSTALLATION. FILM MUST ALSO BE REMOVED FROM ALL NON EXPOSED PARTS WITHIN SIX MONTHS FROM FILM APPLICATION OR IRREPARABLE DAMAGE WILL OCCUR TO THE SURFACE CLAIMS WILL NOT BE ACCEPTED FOR THIS ISSUE.

RESPONSIBILITIES

1. The Metal Building Provider Customer, hereafter referred to as the "customer, " obtains and pays for all building permits, licenses, public assessments, paving or utility pro rata, utility connections, occupancy fees and other fees required by any governmental authority or utility in connection with the work provided for in the Contract Documents. The customer provides at his expense all plans and specifications required to obtain a building permit. it is the customer's responsibility to ensure that all plans and specifications comply with the applicable requirements of any governing building authorities.
2. The customer is responsible for identifying all applicable building codes, zoning codes, or other regulations applicable to the Construction Project, including the Metal Building system.
3. It is the responsibility of the customer to interpret all aspects of the End User's specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to the Metal Building Provider.
4. It is the responsibility of the Metal Building Provider to furnish the metal building system to meet the specifications including the design criteria and design loads incorporated by the Contractor into the Order Documents. The Metal Building Provider is not responsible for making an independent determination of any local codes or any other requirements not part of the Order Document.
5. The Metal Building Provider's standard specifications apply unless stipulated otherwise in the Contract Documents. The Metal Building Provider design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work any other interpretations to the contrary not with standing. it is understood by both parties that the customer is responsible for clarifications of inclusions or exclusions from the Architectural plans.
6. In case of discrepancies between the Metal Building Provider's structural steel plans and plans for other trades, the Metal Building Provider's shall govern ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303–16; Section 3.3).
7. The customer is responsible for overall project coordination. All interface, compatibility and design considerations concerning any materials not furnished by the Metal Building Provider and the Metal Building Provider's steel system are to be considered and coordinated by the customer. Specific design criteria concerning this interface between materials must be furnished by the customer before release for fabrication or the Metal Building Provider's assumptions will govern.
8. Foundations, anchor rods, and anchor rod embedment are designed, furnished, and set by the customer in accordance with an approved drawing. Dimensional accuracy shall satisfy the requirements of Section 7.5 1 of "Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303–16.
9. All other embedded items or connection materials between the structural steel and the work of other trades are located and set by the customer in accordance with approved location on erection drawings. Accuracy of these items must satisfy the erection tolerance requirements.
10. The Metal Building Provider does not investigate the influence of the metal building system on existing buildings or structures. The End Customer assures that such buildings and structures are adequate to resist snow drifts, wind loads, or other conditions as a result of the presence of the metal building system.

GENERAL SPECIFICATIONS

1. Wall and liner panels are an integral part of the structural system. Unauthorized removal of panels or cutting panels for framed openings not shown is prohibited.
2. Oil–canning, a perceived waviness inherent to light gauge metal, may exist. This condition does not affect the structural integrity or the finish of the panel, and therefore is not a cause for rejection.
3. The Metal Building Provider's red–oxide and gray–oxide primer are designed for short term field protection from exposure to ordinary atmospheric conditions. Primed steel which is stored in the field pending erection should be kept free of the ground, and so positioned as to minimize water–holding pockets, dust, mud, and other contamination of the primer film. Repairs of damage to primed surfaces and/or removal of foreign material due to transportation (e.g. road salt, de–icing chemicals and other substances encountered during transportation that may accelerate deterioration of the primer or corrosion of the underlying steel), improper field storage, or site conditions are not the responsibility of the Metal Building Provider. (MBMA, 2018 MBSM, Section 4.2.4)
4. All bolts are 1/2" x 1–1/4" A307 unless noted. Refer to the erection drawings for specific framing connections and the cross–section(s) for main frame connections.
5. Unless noted otherwise on the frame cross section(s), all bolted joints with ASTM F3125 Grade A325 bolts are specified as snug–tightened joints in accordance with the specification for Structural Joints Using High–Strength Bolts, June 11, 2020. Installation inspection requirements for Snug–Tight Bolts (Specification for Structural joints, Section 9.1) is suggested.
6. Unless noted otherwise, all bolted connections are designed as bearing type connections with bolt threads not excluded from the shear plane.
7. Any type of suspended or load inducing system(s) is prohibited if zero collateral and zero sprinkler loads are designated on the contract. This would include lights, duct work, piping, and insulation types other than 3" standard duty fiberglass blanket insulation, etc.

BUILDING DESIGN CODES

Building Code: Florida Building Code 2023
Hot–rolled version: AISC 360–16
Cold–formed version: AISI S100–16

GENERAL LOADS

Dead Load: 2.00 psf (Building A&B)
Roof Collateral Load: 1.00 psf (Misc.) (Building A&B)
Sprinkler Load: 0.00 psf
Roof Live Load: 20.00 psf
Tributary Live Load Reduction: YES
Rainfall Intensity: 10.00 in/hr (5–minute duration 5–year recurrence)

WIND LOAD

Wind Load (3–sec gust) Vult: 120 mph
Vasd: 93 mph
V service: 75 mph
Exposure Factor: B
Wind Condition: Enclosed (Building A)
Wind Condition: Partially Enclosed (Building B)
Internal Pressure Coefficient : +/- 0.18 (Building A)
Internal Pressure Coefficient : +/- 0.55 (Building B)
Edge Zone Width: 5.20 Ft (Building A)
Edge Zone Width: 3.00 Ft (Building B)

SNOW LOAD

Ground Snow Load : 0.00 psf

SEISMIC LOAD

Risk Category: II – Normal
Seismic Importance Factor : 1.0000
Structural Response Acceleration (Ss): 0.0810
Structural Response Acceleration(S1): 0.0480
Site Class: D
Design Spectral Response (Sds): 0.0864
Design Spectral Response (Sd1): 0.0768
Seismic Design Category: B

Framing Direction: Lateral Longitudinal
Structural Syst: 'Structural Steel Systems Not Specifically Detailed for Seismic Resistance'

Response Modification Factor(s) : 3.0 3.0
Deflection Amplification: 3.0 3.0

Sesimic Response Coefficient(s) (Cs): 0.0288 0.0288
Design Base Shear V : 1.24 kips 1.37 kips

Analysis Procedure: Equivalent Lateral Force

OTHER LOADS:

1. Building–B is supported by Building–A.

DEFLECTION CRITERIA

Main Frames Horizontal: H/60
Main Frames Vertical: L/180
Bearing Frame Rafter: L/180
Endwall Columns: L/120
Wind Frame Horizontal : H/60
Roof Panels: L/60
Purlins: L/150
Wall Panels: L/60
Girts: L/90

For components,claddings and MWFRS, deflections involving wind are based on 10 year serviceability wind pressures.

ROOF PANEL

Profile: Super Span X Gauge: 26 Color: Galvalume Plus (Building A&B)
UL580 Class 90: Yes
Clip Type if Standing Seam: NO

WALL PANEL

Profile: Super Span X Gauge: 26 Color: SMP Surfsand (Building A)

PRIMARY FRAMING

Built–Up & Hot–Rolled: Gray Oxide Primer

SECONDARY FRAMING

Purlins, Eave Struts: Pre–Galvanized
Girts, Light Gage Columns: Pre–Galvanized
Light Gage Jamb's & Headers: Pre–Galvanized
Base Angle Finish: Pre–Galvanized

Hot–Dip Galvanizing conforms to the ASTM A123 specification.
Pre–Galvanized members conform to the ASTM A653, Grade 50,
Coating G–90 specification.

APPROVAL SPECIFICATIONS

1. Approval of the Metal Building Provider drawings and/or calculations indicate that the Metal Building Provider has correctly interpreted the contact requirements. This approval constitutes the customer acceptance of the Metal Building Provider design, concepts, assumptions, and loadings.
2. Failure to respond to clouded areas and areas to verify may result in additional costs and/or schedule delays for which the Metal Building Provider will not be responsible.
3. Any changes made after the Metal Building Provider's customer has signed and returned the Metal Building Provider drawings and/or calculations and the project is released for fabrication shall be billed to the Metal Building Provider customer including material, engineering, and other costs. An additional fee may be charged if the project must be moved in the fabrication and/or the shipping schedule.
4. It is the responsibility of the customer to field verify all existing conditions prior to fabrication.
5. It is imperative that any changes to these drawings:
- 5.1. Be made in contrasting ink.
- 5.2. Be legible and unambiguous.
- 5.3. Have all instances of changes clearly indicated.
6. A dated signature, in the designated areas, is required on all pages. The signature must be from the person authorized on the contract or a person authorized, in writing, by the Metal Building Provider customer.
7. The Metal Building Provider reserves the right to resubmit drawings with extensive or complex changes required to avoid misfabrication. This may impact the delivery schedule.
8. Any changes noted on the drawings not in conformance with the terms and requirements of the contract between the Metal Building Provider and its customer are not binding on the Metal Building Provider unless subsequently acknowledged and agreed to in writing by change order or separate documentation.
9. Waiving the approval process by designating the order "For Production" supercedes notes 1,2,5,6, and 8 in this section, and constitutes the customer acceptance of the Metal Building Provider's design, concepts, assumptions, and loadings.

DRAWING SCHEDULE

| DWG NO. | ISSUE | DATE | DESCRIPTION |
|---------|-------|----------|-------------------------------------|
| C1 | P2 | 12.20.24 | COVER SHEET |
| F1 | 1 | 12.20.24 | ANCHOR BOLT PLAN |
| F2 | 1 | 12.20.24 | ANCHOR BOLT DETAILS |
| F3 | 1 | 12.20.24 | ANCHOR BOLT REACTIONS |
| F4 | 1 | 12.20.24 | ANCHOR BOLT REACTIONS |
| P1 | P2 | 12.20.24 | RIGID FRAME ELEVATION |
| P2 | P2 | 12.20.24 | RIGID FRAME ELEVATION |
| E1 | P2 | 12.20.24 | ROOF FRAMING PLAN |
| E2 | P2 | 12.20.24 | ROOF SHEETING PLAN |
| E3 | P2 | 12.20.24 | ENDWALL FRAME & SHEETING ELEVATION |
| E4 | P2 | 12.20.24 | ENDWALL FRAME & SHEETING ELEVATION |
| E5 | P2 | 12.20.24 | SIDEWALL FRAME & SHEETING ELEVATION |
| E6 | P2 | 12.20.24 | SIDEWALL FRAME & SHEETING ELEVATION |
| E7 | P2 | 12.20.24 | SIDEWALL FRAME & SHEETING ELEVATION |
| E8 | P2 | 12.20.24 | BUILDING SECTIONS |
| E9 | P2 | 12.20.24 | BUILDING SECTIONS |
| D1 | P2 | 12.20.24 | STANDARD DETAIL PAGE |
| D2 | P2 | 12.20.24 | STANDARD DETAIL PAGE |
| D3 | P2 | 12.20.24 | STANDARD DETAIL PAGE |

TRIM COLOR:

SHADOW EAVE: SMP SURFSAND GAUGE: 26
SHADOW RAKE: SMP SURFSAND GAUGE: 26
CORNER: SMP SURFSAND GAUGE: 26
ACCESSORY: SMP SURFSAND GAUGE: 26
BASE TRIM: SMP SURFSAND GAUGE: 26

Roof and wall panels have been designed in accordance with section 2222.4 of the Florida Building Code, Eighth Edition (2023). Product approval numbers for the State of Florida, Department of Community Affairs per Product Rule 9B–72:

Roof Deck
Super Span X Roof Panel 17700.5 26 ga. Super Span X Roof Panel over open framing HVHZ:
No

Structural Wall
Super Span X Wall Panel 17702.4 26 ga. Super Span X Wall Panel over open framing HVHZ:
No

The rigid frames at lines 1 and 5 are designed as non–expandable rigid frames. Corresponding frame reactions are calculated based upon actual tributary area.



This item has been digitally signed and sealed by
Jeremy Mathews, PE on 12/23/2024

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

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☒ FOR ERECTOR INSTALLATION: Final drawings for construction.



| ISSUE | DATE | DESCRIPTION | BY | CHK |
|-------|----------|-----------------------------|-----|-----|
| P1 | 08.23.23 | FOR CONSTRUCTION PERMIT | PND | PNC |
| 0 | 08.29.23 | FOR ERECTOR INSTALLATION | PND | PNC |
| P2 | 12.20.24 | REV FOR CONSTRUCTION PERMIT | PND | PNC |
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| SHEET DESCRIPTION: | | BLDG SIZE: | |
| COVER SHEET | | VARIES | |
| CUSTOMER: | | CUSTOMER LOCATION: | |
| MARCO BUILDER INC. | | FORT WHITE, FL 32038 | |
| PROJECT REFERENCE: | | | |
| MARCO BUILDER INC. | | | |
| JOB SITE LOCATION: | | JOB SITE COUNTY: | |
| FORT WHITE, FL 32038 | | COLUMBIA | |
| DWN: | CHK: | DATE: | ENG: |
| PND | PNC | 08.29.23 | MAH |
| JOB NO: | | DWG NO: | |
| 11463–32450 | | C1 | |
| | | ISSUE: | |
| | | P2 | |

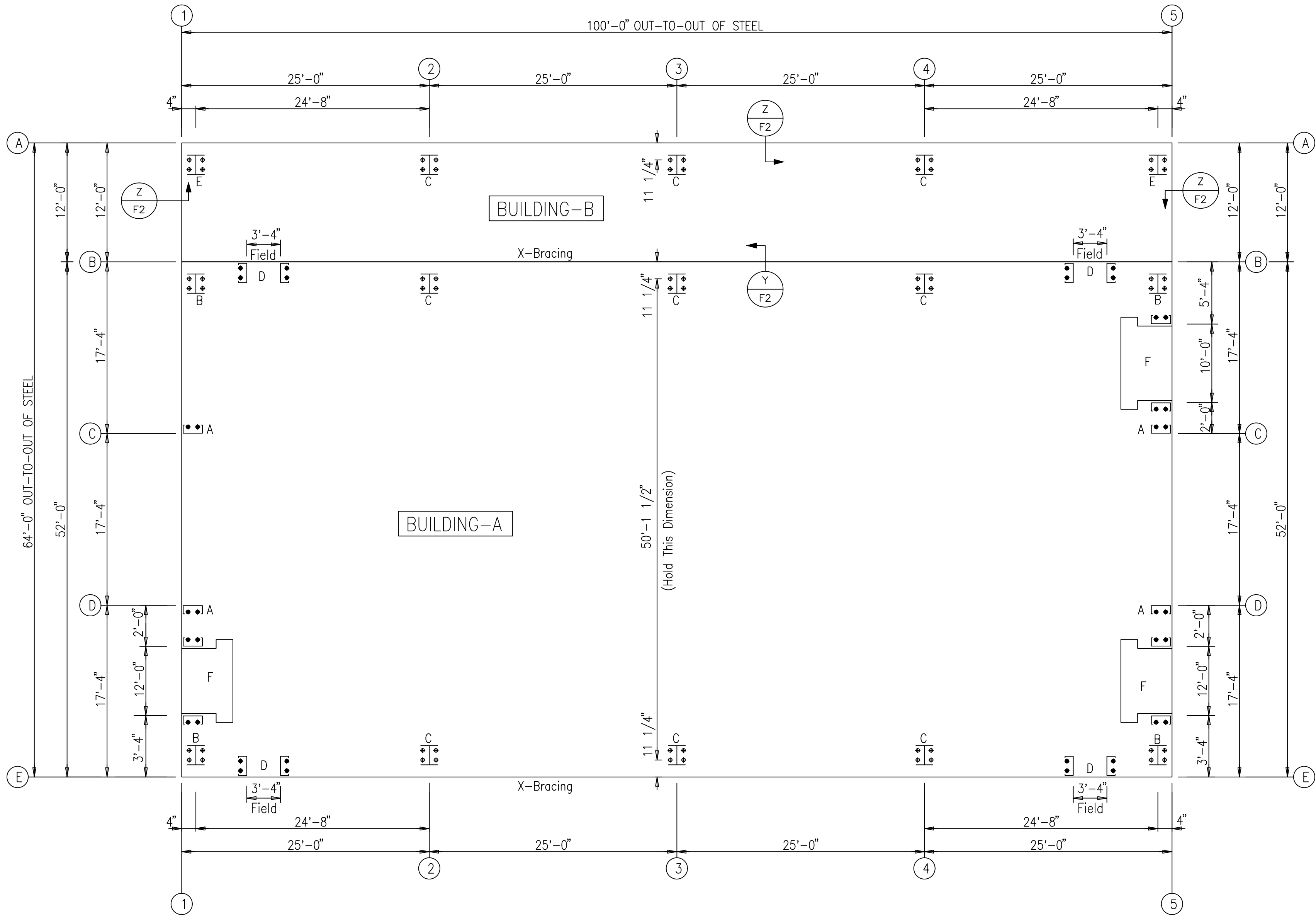
Digitally signed by
Jeremy Mathews
Location:
Date: 2024.12.23
15:32:22-06'00'

12/23/2024

JEREMY S. MATHEWS
LICENSE
No. 85468
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

● Dia= 5/8"

⊕ Dia= 3/4"



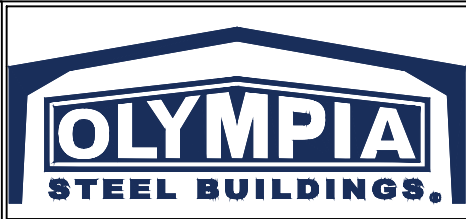
ANCHOR BOLT PLAN

NOTE: All Base Plates @ Finished Floor (U.N.)

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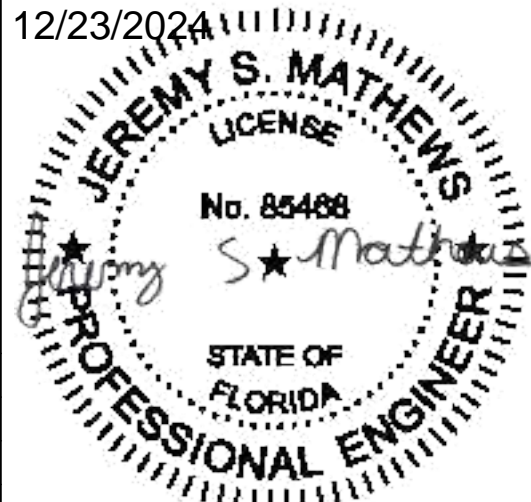
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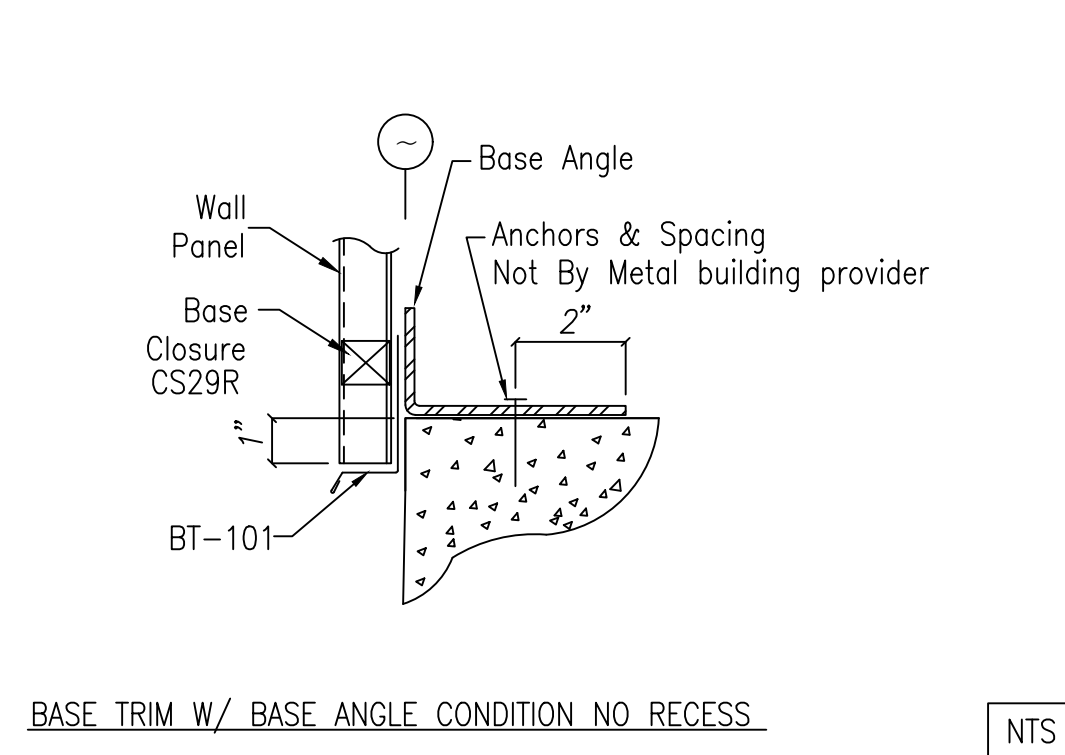
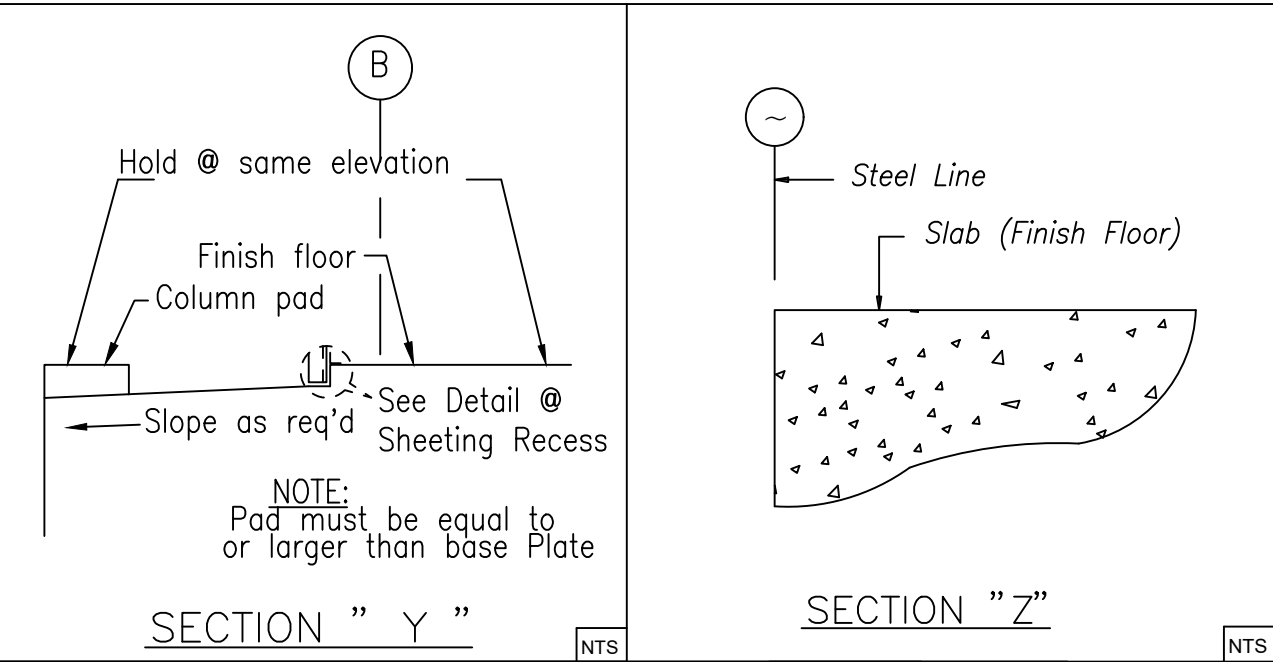
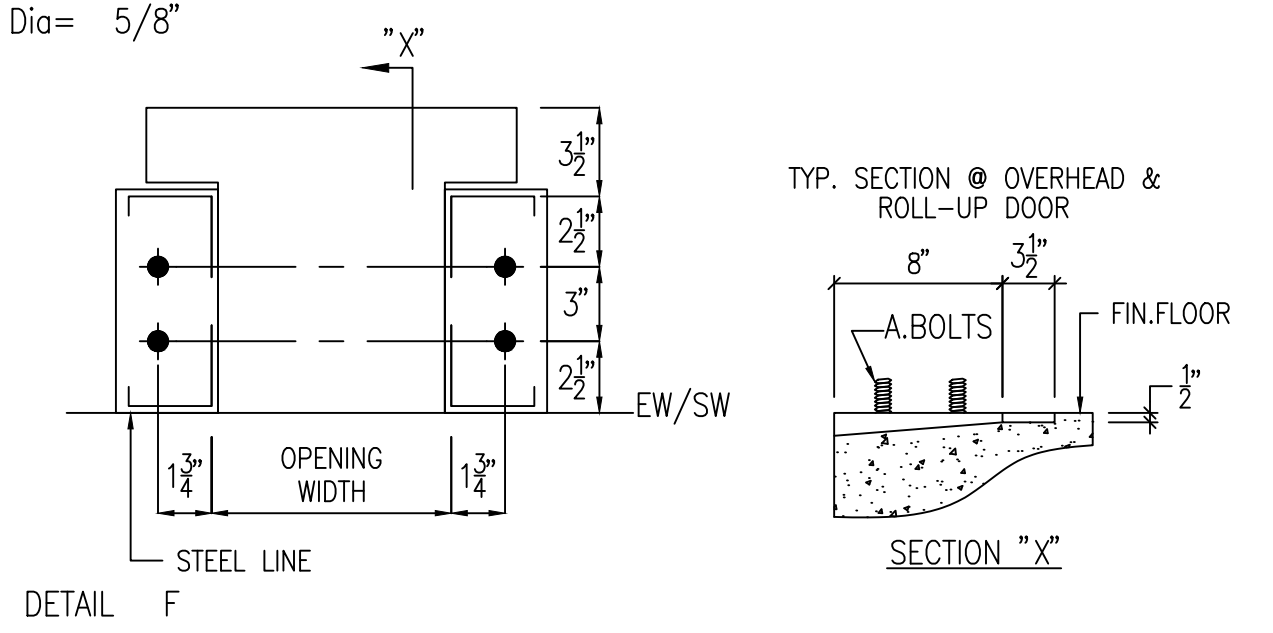
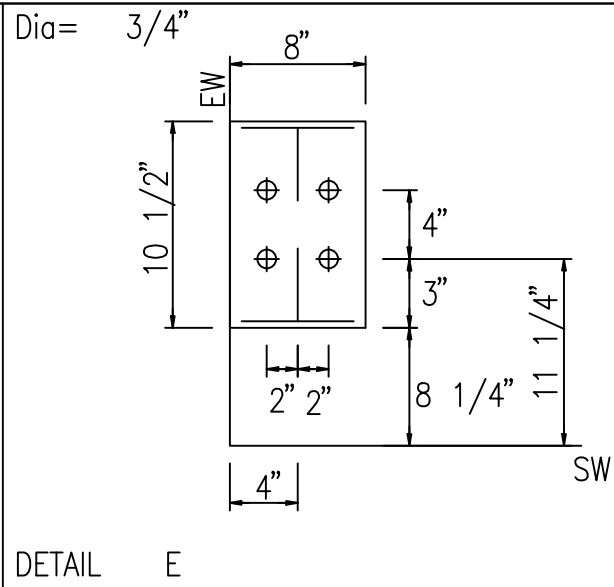
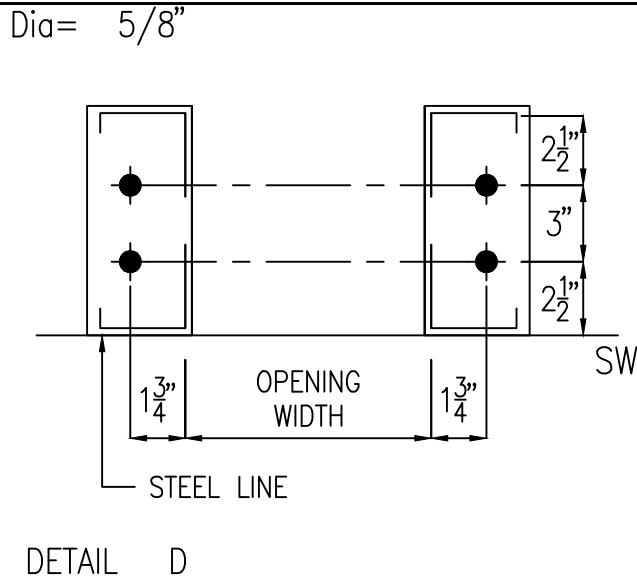
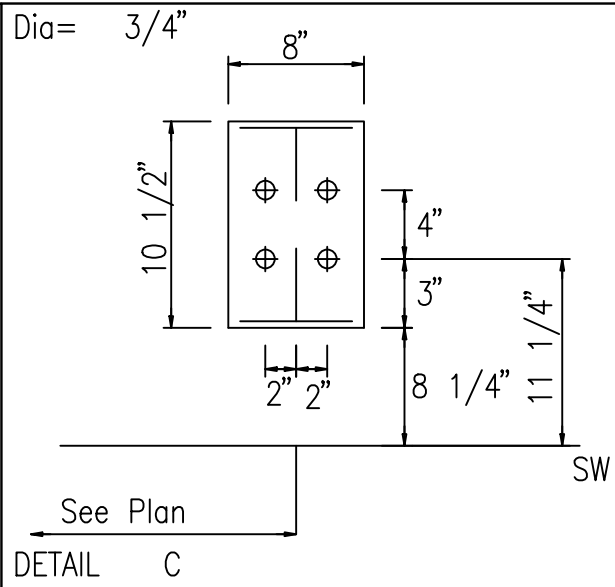
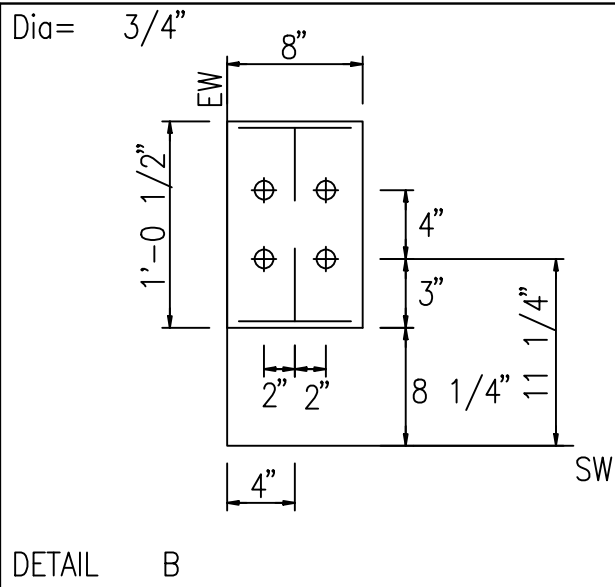
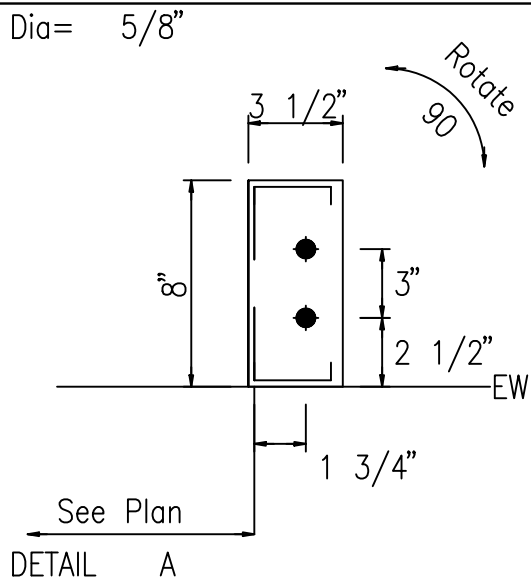
☒ FOR ERECTOR INSTALLATION:
Final drawings for construction.



| ISSUE | DATE | DESCRIPTION | BY | CHK | SHEET DESCRIPTION: | BLDG SIZE: |
|-------|----------|------------------------------|-----|-----|---|---|
| 0 | 08.23.23 | FOR ERECTOR INSTALLATION | PND | PNC | ANCHOR BOLT PLAN | VARIES |
| 1 | 12.20.24 | REV FOR ERECTOR INSTALLATION | PND | PNC | CUSTOMER: MARCO BUILDER INC. | CUSTOMER LOCATION: FORT WHITE, FL 32038 |
| | | | | | PROJECT REFERENCE: MARCO BUILDER INC. | |
| | | | | | JOB SITE LOCATION: FORT WHITE, FL 32038 | JOB SITE COUNTY: COLUMBIA |
| | | | | | DWN: PND | CHK: PNC |
| | | | | | DATE: 08.23.23 | ENG: MAH |
| | | | | | JOB NO: 11463-32450 | DWG NO: F1 |
| | | | | | ISSUE: 1 | |

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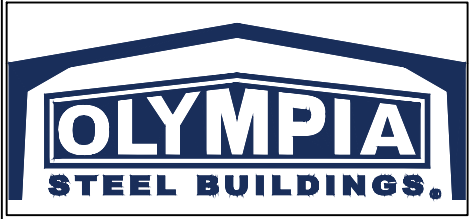




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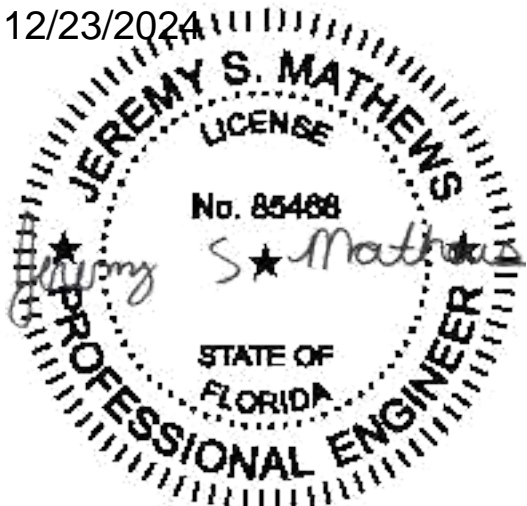
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Final drawings for construction.

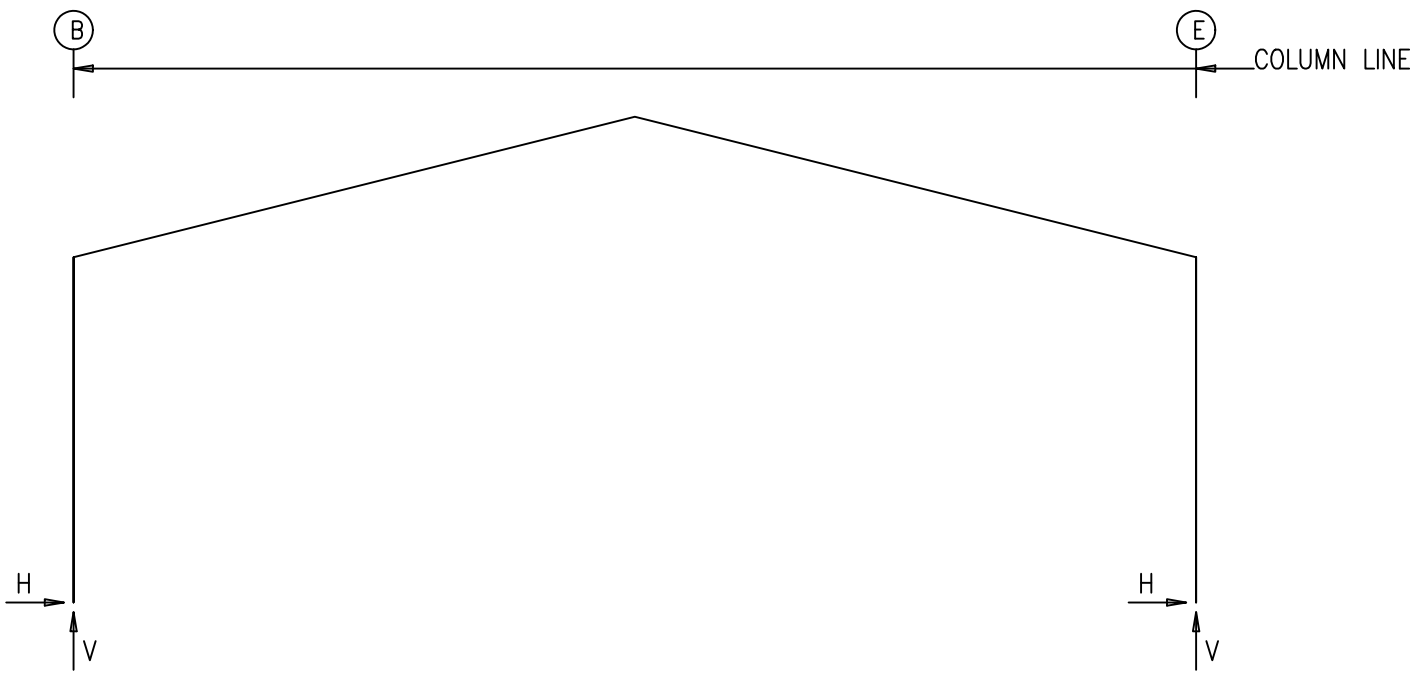


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| 1 | 12.20.24 | REV FOR ERECTOR INSTALLATION | PND | PNC | CUSTOMER: MARCO BUILDER INC. | CUSTOMER LOCATION: FORT WHITE, FL 32038 |
| | | | | | PROJECT REFERENCE: MARCO BUILDER INC. | |
| | | | | | JOB SITE LOCATION: FORT WHITE, FL 32038 | JOB SITE COUNTY: COLUMBIA |
| | | | | | DWN: PND | CHK: PNC |
| | | | | | DATE: 08.23.23 | ENG: MAH |
| | | | | | JOB NO: 11463-32450 | DWG NO: F2 |
| | | | | | ISSUE: 1 | |

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FRAME LINES: 1 2 3 4 5



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Column_Reactions(k) | | | | | | Bolt(in) | | Base_Plate(in) | | Thick | Elev. (in) |
|----------|------------------|----------------------|--------|--------|---------|--------|--------|----------|-------|----------------|--------|-------|------------|
| | | Load Id | Hmax H | V Vmax | Load Id | Hmin H | V Vmin | Qty | Dia | Width | Length | | |
| 1* | B | 2 | 2.4 | 3.4 | 5 | -3.0 | -3.3 | 4 | 0.750 | 8.000 | 12.50 | 0.375 | 0.0 |
| | | 1 | 2.1 | 7.3 | 3 | -2.1 | -4.9 | | | | | | |
| 1* | E | 4 | 3.2 | -4.6 | 1 | -2.1 | 5.5 | 4 | 0.750 | 8.000 | 12.50 | 0.375 | 0.0 |
| | | 1 | -2.1 | 5.5 | 4 | 3.2 | -4.6 | | | | | | |
| 1* | Frame lines: 1 5 | | | | | | | | | | | | |

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Column_Reactions(k) | | | | | | Bolt(in) | | Base_Plate(in) | | Thick | Elev. (in) |
|----------|--------------------|----------------------|--------|--------|---------|--------|--------|----------|-------|----------------|--------|-------|------------|
| | | Load Id | Hmax H | V Vmax | Load Id | Hmin H | V Vmin | Qty | Dia | Width | Length | | |
| 2* | B | 2 | 5.1 | 6.9 | 5 | -5.4 | -4.6 | 4 | 0.750 | 8.000 | 10.50 | 0.375 | 0.0 |
| | | 1 | 4.8 | 13.5 | 3 | -3.4 | -7.7 | | | | | | |
| 2* | E | 4 | 5.6 | -7.3 | 1 | -4.8 | 10.4 | 4 | 0.750 | 8.000 | 10.50 | 0.375 | 0.0 |
| | | 1 | -4.8 | 10.4 | 6 | 1.5 | -7.8 | | | | | | |
| 2* | Frame lines: 2 3 4 | | | | | | | | | | | | |

RIGID FRAME: BASIC COLUMN REACTIONS (k)

| Frame Line | Column Line | Dead | | Collateral | | Live | | Wind_Left1 | | Wind_Right1 | | Wind_Left2 | |
|------------|--------------------|-------------|------|------------|-------|------------|-------|--------------|-------|---------------|-------|--------------|-------|
| | | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert |
| 1* | B | 0.4 | 1.5 | 0.1 | 0.4 | 1.6 | 5.4 | -3.9 | -9.6 | 1.4 | -5.5 | -5.5 | -7.0 |
| 1* | E | -0.4 | 1.2 | -0.1 | 0.3 | -1.6 | 3.9 | 0.7 | -5.7 | 5.8 | -8.8 | -0.8 | -2.2 |
| 2* | B | 0.8 | 2.4 | 0.3 | 0.8 | 3.6 | 10.3 | -6.5 | -15.3 | 2.8 | -8.8 | -9.8 | -10.0 |
| 2* | E | -0.8 | 2.0 | -0.3 | 0.7 | -3.7 | 7.8 | 1.2 | -9.3 | 10.1 | -14.2 | -1.7 | -2.2 |
| Frame Line | Column Line | Wind_Right2 | | Wind_Long1 | | Wind_Long2 | | Seismic_Left | | Seismic_Right | | Seismic_Long | |
| | | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert |
| 1* | B | -0.1 | -2.9 | 1.2 | -7.6 | 0.3 | -6.5 | -0.1 | -0.1 | 0.1 | 0.1 | 0.0 | 0.0 |
| 1* | E | 4.3 | -5.3 | 2.7 | -6.8 | 1.8 | -7.9 | -0.1 | 0.1 | 0.1 | -0.1 | 0.0 | 0.0 |
| 2* | B | -0.4 | -3.6 | 2.4 | -14.8 | 1.3 | -13.3 | -0.1 | -0.1 | 0.1 | 0.1 | 0.0 | -0.5 |
| 2* | E | 7.3 | -7.1 | 4.5 | -13.5 | 3.3 | -15.0 | -0.1 | 0.1 | 0.1 | -0.1 | 0.0 | -0.3 |
| 1* | Frame lines: 1 5 | | | | | | | | | | | | |
| 2* | Frame lines: 2 3 4 | | | | | | | | | | | | |

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

| Frm Line | Col Line | Dead Vert | Wind Press Horiz | Wind Suct Horiz | Seis Long Vert |
|----------|----------|-----------|------------------|-----------------|----------------|
| | | | | | |
| 1 | C | 0.1 | -3.0 | 3.3 | 0.0 |
| 1 | D | 0.1 | -3.0 | 3.3 | 0.0 |
| 5 | D | 0.1 | -3.0 | 3.3 | 0.0 |
| 5 | C | 0.1 | -3.0 | 3.3 | 0.0 |

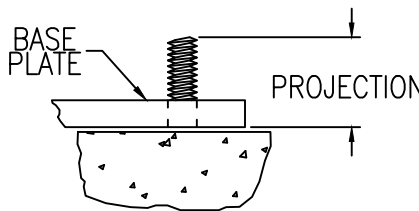
ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Column_Reactions(k) | | | | | | Bolt(in) | | Base_Plate(in) | | Thick | Elev. (in) |
|----------|----------|----------------------|--------|--------|---------|--------|--------|----------|-------|----------------|--------|-------|------------|
| | | Load Id | Hmax H | V Vmax | Load Id | Hmin H | V Vmin | Qty | Dia | Width | Length | | |
| 1 | C | 7 | 2.0 | 0.1 | 8 | -1.8 | 0.1 | 2 | 0.625 | 3.500 | 8.000 | 0.250 | 0.0 |
| | | 9 | 2.0 | 0.1 | | | | | | | | | |
| 1 | D | 7 | 2.0 | 0.1 | 8 | -1.8 | 0.1 | 2 | 0.625 | 3.500 | 8.000 | 0.250 | 0.0 |
| | | 9 | 2.0 | 0.1 | | | | | | | | | |
| 5 | D | 7 | 2.0 | 0.1 | 8 | -1.8 | 0.1 | 2 | 0.625 | 3.500 | 8.000 | 0.250 | 0.0 |
| | | 9 | 2.0 | 0.1 | | | | | | | | | |
| 5 | C | 7 | 2.0 | 0.1 | 8 | -1.8 | 0.1 | 2 | 0.625 | 3.500 | 8.000 | 0.250 | 0.0 |
| | | 9 | 2.0 | 0.1 | | | | | | | | | |

GENERAL NOTES

- All anchor bolts (by others) to have nuts and flat washers.
- All anchor bolts are designed to full S.A.E. diameters with cut threads. No substitutions are allowed.
- The Metal Building Provider is not responsible for the design, materials and workmanship of the foundation. Anchor bolt plans prepared by the Metal Building Provider are intended to show only location, diameter, and projection of anchor bolts required to attach the Metal Building System to the foundation. The Metal Building Provider is responsible for providing to the Builder the loads imposed by the Metal Building System on the foundation. It is the responsibility of the End Customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and/or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. This is typically the responsibility of the Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended. (2012 MBMA Metal Building Systems Manual, Section 3.2.2)
- The projection is based from the bottom of the base plate. Adjustments must be made for grout and/or leveling plates.

THREADED ANCHOR BOLT



NOTE: PROJECTION BASED FROM BOTTOM OF BASE PLATE. ADJUSTMENTS SHOULD BE MADE FOR GROUT AND/OR LEVELING PLATES.

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Loading conditions are:
 - Dead+Collateral+Live
 - Dead+Collateral+0.75Live+0.45Wind_Right1
 - 0.6Dead+0.6Wind_Left1
 - 0.6Dead+0.6Wind_Right1
 - 0.6Dead+0.6Wind_Left2
 - 0.6Dead+0.6Wind_Long2L
 - 0.6Dead+0.6Wind_Right2+0.6Wind_Suction
 - 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
 - Dead+0.6Wind_Right2+0.6Wind_Suction

BUILDING-A

| Zone | Width (ft) | Length (ft) | Components & Cladding | | | |
|------|------------|-------------|-----------------------|-------|----------------------|--------|
| | | | Pressure(psf) Member | Panel | Suction(psf) Member | Panel |
| 1 | | | 16.00 | 16.00 | -16.00 | -41.84 |
| 2 | | 5.20 | 16.00 | 16.00 | -28.96 | -61.09 |
| 3 | 5.20 | | 16.00 | 16.00 | -28.96 | -61.09 |
| 4 | 5.20 | 5.20 | 16.00 | 16.00 | -43.36 | -72.38 |
| 5 | | | 18.24 | 22.65 | -20.16 | -24.57 |
| 6 | 5.20 | | 18.24 | 22.65 | -21.57 | -30.22 |
| 7 | | | 18.20 | 22.60 | -20.20 | -24.60 |
| 8 | 5.20 | | 18.20 | 22.60 | -21.61 | -30.26 |

(+) wind towards surface
(-) wind away from surface

BUILDING BRACING REACTIONS

| Wall Loc | Col Line | ± Reactions(k) | | | | Panel_Shear (lb/ft) | | Note |
|----------|----------|-----------------|-----------|---------------|--------------|---------------------|------|------|
| | | Wind Horiz | Wind Vert | Seismic Horiz | Seismic Vert | Wind | Seis | |
| L_EW | 1 | | | | | | | (h) |
| F_SW | E | 2,3 | 4.1 | 2.3 | 0.5 | | | |
| R_EW | 5 | | | | | | | (h) |
| B_SW | B | 3,2 | 5.7 | 3.2 | 0.9 | | | |

(h)Rigid frame at endwall

Reactions for seismic represent shear force, Eh

ANCHOR BOLT SUMMARY (GRADE 36)

| Qty | Locate | Dia (in) | Type | Proj (in) |
|-----|---------|----------|-------|-----------|
| 28 | Jamb | 5/8" | F1554 | 2.50 |
| 8 | Endwall | 5/8" | F1554 | 2.50 |
| 40 | Frame | 3/4" | F1554 | 3.00 |

☐ FOR APPROVAL: These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered complete.

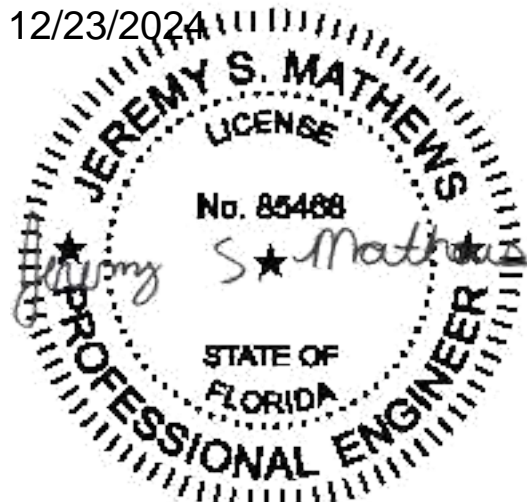
☐ FOR CONSTRUCTION PERMITS: These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

☒ FOR ERECTOR INSTALLATION: Final drawings for construction.

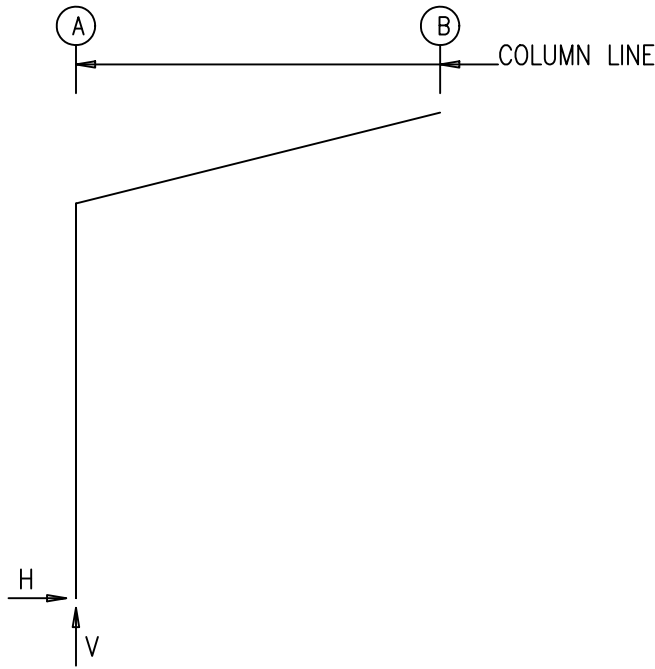


| ISSUE | DATE | DESCRIPTION | BY | CHK | PNC | SHEET DESCRIPTION: | BLDG SIZE: |
|-------|----------|------------------------------|------|-------------|---------|-----------------------|----------------------|
| 0 | 08.23.23 | FOR ERECTOR INSTALLATION | PND | | PNC | ANCHOR BOLT REACTIONS | VARIES |
| 1 | 12.20.24 | REV FOR ERECTOR INSTALLATION | PND | | PNC | | |
| | | | | | | CUSTOMER: | CUSTOMER LOCATION: |
| | | | | | | MARCO BUILDER INC. | FORT WHITE, FL 32038 |
| | | | | | | PROJECT REFERENCE: | |
| | | | | | | MARCO BUILDER INC. | |
| | | | | | | JOB SITE LOCATION: | JOB SITE COUNTY: |
| | | | | | | FORT WHITE, FL 32038 | COLUMBIA |
| DWN: | CHK: | DATE: | ENG: | JOB NO: | DWG NO: | ISSUE: | |
| PND | PNC | 08.23.23 | MAH | 11463-32450 | F3 | 1 | |

The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes part such as doors, windows, foundation design, and erection of the building.



FRAME LINES: 1 2 3 4 5



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Column_Reactions(k) | | | | Hmin H | Vmin V | Bolt(in) Qty | Dia | Base_Plate(in) | | Thick | Elev. (in) |
|----------|------------------|----------------------|------------|-------------|---------|-------------|-------------|--------------|-------|----------------|--------|-------|------------|
| | | Load Id | Hmax H | V Vmax | Load Id | | | | | Width | Length | | |
| 1* | A | 6 1 | 1.0 0.0 | -1.2 2.1 | 4 5 | -1.1 1.0 | 0.2 -1.6 | 4 | 0.750 | 8.000 | 10.50 | 0.375 | 0.0 |
| 1* | Frame lines: 1 5 | | | | | | | | | | | | |

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Column_Reactions(k) | | | | Hmin H | Vmin V | Bolt(in) Qty | Dia | Base_Plate(in) | | Thick | Elev. (in) |
|----------|--------------------|----------------------|------------|-------------|---------|-------------|-------------|--------------|-------|----------------|--------|-------|------------|
| | | Load Id | Hmax H | V Vmax | Load Id | | | | | Width | Length | | |
| 2* | A | 6 1 | 2.0 0.1 | -2.3 3.7 | 4 5 | -2.0 1.9 | 0.6 -3.0 | 4 | 0.750 | 8.000 | 10.50 | 0.375 | 0.0 |
| 2* | Frame lines: 2 3 4 | | | | | | | | | | | | |

RIGID FRAME: BASIC COLUMN REACTIONS (k)

| Frame Line | Column Line | Dead | | Collateral | | Live | | Wind_Left1 | | Wind_Right1 | | Wind_Left2 | |
|------------|-------------|-------|------|------------|------|-------|------|------------|------|-------------|------|------------|------|
| | | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert |
| 1* | A | 0.0 | 0.4 | 0.0 | 0.1 | 0.0 | 1.6 | -0.1 | -2.7 | 1.6 | -2.5 | -1.9 | 0.0 |
| 2* | A | 0.0 | 0.6 | 0.0 | 0.2 | 0.1 | 2.9 | 0.2 | -4.7 | 3.0 | -4.6 | -3.4 | 0.5 |

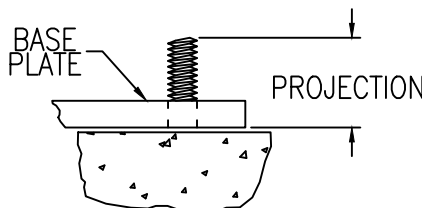
| Frame Line | Column Line | Wind_Right2 | | Wind_Long1 | | Wind_Long2 | |
|------------|-------------|-------------|------|------------|------|------------|------|
| | | Horiz | Vert | Horiz | Vert | Horiz | Vert |
| 1* | A | -0.2 | 0.1 | 1.6 | -3.1 | 1.7 | -2.4 |
| 2* | A | -0.6 | 0.6 | 3.2 | -5.6 | 3.2 | -4.5 |

| | | | |
|----|--------------|---|-----|
| 1* | Frame lines: | 1 | 5 |
| 2* | Frame lines: | 2 | 3 4 |

GENERAL NOTES

- All anchor bolts (by others) to have nuts and flat washers.
- All anchor bolts are designed to full S.A.E. diameters with cut threads. No substitutions are allowed.
- The Metal Building Provider is not responsible for the design, materials and workmanship of the foundation. Anchor bolt plans prepared by the Metal Building Provider are intended to show only location, diameter, and projection of anchor bolts required to attach the Metal Building System to the foundation. The Metal Building Provider is responsible for providing to the Builder the loads imposed by the Metal Building System on the foundation. It is the responsibility of the End Customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and/or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. This is typically the responsibility of the Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended. (2012 MBMA Metal Building Systems Manual, Section 3.2.2)
- The projection is based from the bottom of the base plate. Adjustments must be made for grout and/or leveling plates.

THREADED ANCHOR BOLT



NOTE: PROJECTION BASED FROM BOTTOM OF BASE PLATE. ADJUSTMENTS SHOULD BE MADE FOR GROUT AND/OR LEVELING PLATES.

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Loading conditions are:
 - Dead+Collateral+Live
 - Dead+Collateral+0.75Snow+0.45Wind_Long2L+0.75Slide_Snow
 - 0.6Dead+0.6Wind_Left1
 - 0.6Dead+0.6Wind_Left2
 - 0.6Dead+0.6Wind_Long1R
 - 0.6Dead+0.6Wind_Long2R

BUILDING BRACING REACTIONS

| Loc | Wall Line | Col Line | ± Reactions(k) | | | | Panel_Shear (lb/ft) | | Note |
|---|-----------|------------------------|-----------------|-----------|---------------|--------------|---------------------|------|------|
| | | | Wind Horiz | Wind Vert | Seismic Horiz | Seismic Vert | Wind | Seis | |
| L_EW | 1 | | | | | | | | (h) |
| F_SW | B | | | | | | | | (f) |
| R_EW | 5 | | | | | | | | (h) |
| B_SW | A | Torsional Bracing Used | | | | | | | |
| (f)Bracing loads are applied to adjacent building | | | | | | | | | |
| (h)Rigid frame at endwall | | | | | | | | | |
| Reactions for seismic represent shear force, Eh | | | | | | | | | |

| Zone | Width (ft) | Length (ft) | Components & Cladding | | | |
|-----------------------------|------------|-------------|-----------------------|---------|--------------------|---------|
| | | | Pressure(psf Member |) Panel | Suction(psf Member |) Panel |
| 1 | | | 16.00 | 16.97 | -29.47 | -33.04 |
| 2 | | | 16.00 | 16.97 | -31.24 | -38.33 |
| 3 | 3.00 | 3.00 | 16.00 | 16.97 | -31.24 | -38.33 |
| 4 | | | 23.58 | 27.68 | -25.37 | -29.47 |
| 5 | 3.00 | | 23.58 | 27.68 | -26.64 | -34.77 |
| 6 | | | 23.60 | 27.70 | -25.40 | -29.50 |
| 7 | 3.00 | | 23.60 | 27.70 | -26.67 | -34.81 |
| (+) wind towards surface | | | | | | |
| (-) wind away from surface | | | | | | |

ANCHOR BOLT SUMMARY (GRADE 36)

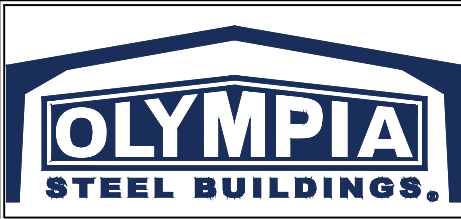
| Qty | Locate | Dia (in) | Type | Proj (in) |
|------|--------|----------|-------|-----------|
| ⊕ 20 | Frame | 3/4" | F1554 | 3.00 |

BUILDING-B

☐ FOR APPROVAL: These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered complete.

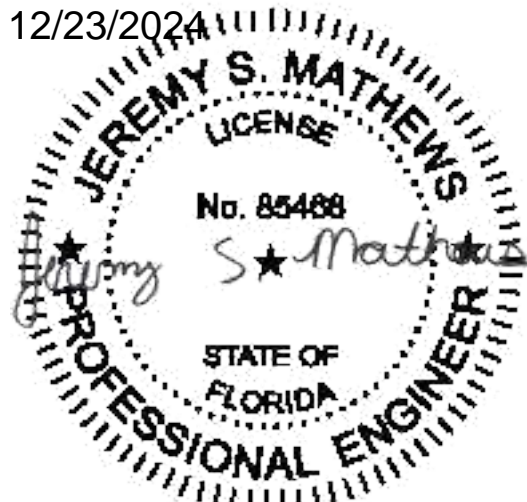
☐ FOR CONSTRUCTION PERMIT: These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

☒ FOR ERECTOR INSTALLATION: Final drawings for construction.



| ISSUE | DATE | DESCRIPTION | BY | CHK | SHEET DESCRIPTION: | BLDG SIZE: |
|-------|----------|------------------------------|-----|-----|---|---|
| 0 | 08.23.23 | FOR ERECTOR INSTALLATION | PND | PNC | ANCHOR BOLT REACTIONS | VARIES |
| 1 | 12.20.24 | REV FOR ERECTOR INSTALLATION | PND | PNC | CUSTOMER: MARCO BUILDER INC. | CUSTOMER LOCATION: FORT WHITE, FL 32038 |
| | | | | | PROJECT REFERENCE: MARCO BUILDER INC. | |
| | | | | | JOB SITE LOCATION: FORT WHITE, FL 32038 | JOB SITE COUNTY: COLUMBIA |
| | | | | | DWN: PND | CHK: PNC |
| | | | | | DATE: 08.23.23 | ENG: MAH |
| | | | | | JOB NO: 11463-32450 | DWG NO: F4 |
| | | | | | ISSUE: 1 | |

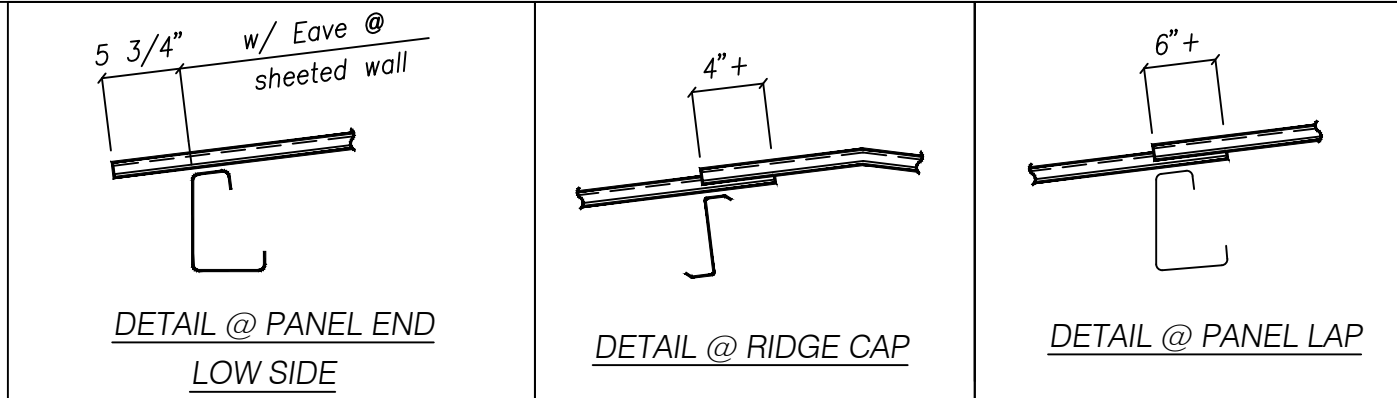
The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes part such as doors, windows, foundation design, and erection of the building.



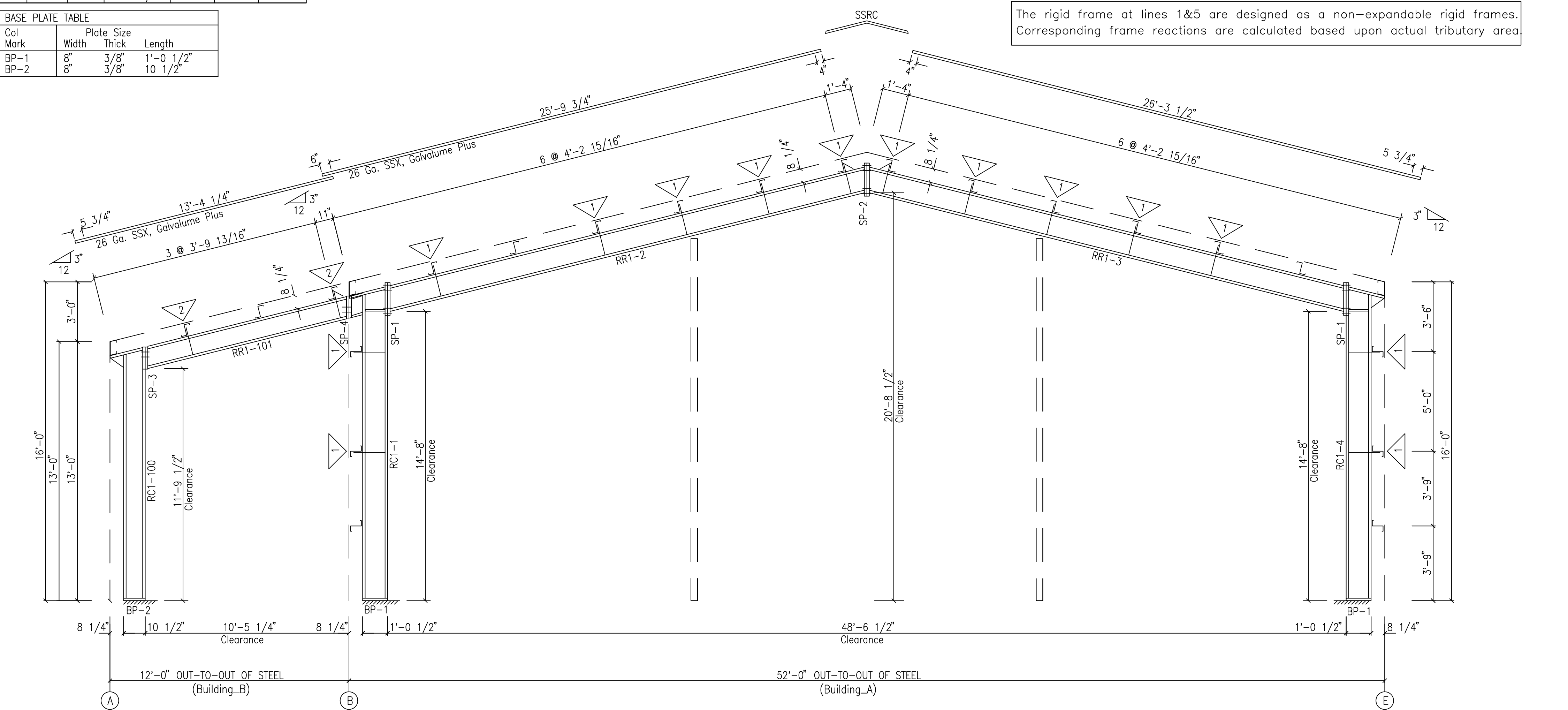
| SPlice PLATE & BOLT TABLE | | | | | | | | | |
|---------------------------|-----|-----|-----|-----|------|------|--------|-------|-------|
| Mark | Qty | Top | Bot | Int | Type | Dia | Length | Width | Thick |
| SP-1 | 4 | 4 | 0 | 0 | A325 | 3/4" | 2" | 6" | 1/2" |
| SP-2 | 4 | 4 | 0 | 0 | A325 | 3/4" | 1 3/4" | 6" | 3/8" |
| SP-3 | 4 | 0 | 0 | 0 | A325 | 3/4" | 1 3/4" | 6" | 3/8" |
| SP-4 | 4 | 0 | 0 | 0 | A325 | 3/4" | 1 3/4" | 6" | 3/8" |

| FLANGE BRACE TABLE | | | | | | |
|--|---|--------|-----------|--------|--------|------|
| A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16 | | | | | | |
| FRAME LINE: 1 5 | | | | | | |
| ▽ ID | # | MARK | LENGTH | OFFSET | DETAIL | CLIP |
| 1 | 1 | FB3A | 2'-6 1/2" | 2'-4" | — | — |
| 2 | 1 | FB100A | 2'-5 3/4" | 2'-4" | — | — |

| BASE PLATE TABLE | | | |
|------------------|------------|-------|-----------|
| Col Mark | Plate Size | Width | Thick |
| BP-1 | 8" | 3/8" | 1'-0 1/2" |
| BP-2 | 8" | 3/8" | 10 1/2" |



| MEMBER TABLE | | | | |
|--------------|-----------|-----------|----------------|---------------|
| Mark | Web Depth | Web Plate | Outside Flange | Inside Flange |
| RC1-1 | 12.0/12.0 | 0.135 | 6 x 1/4" | 6 x 1/4" |
| RR1-2 | 12.0/12.0 | 0.135 | 5 x 1/4" | 5 x 1/4" |
| RR1-3 | 12.0/12.0 | 0.135 | 5 x 1/4" | 5 x 1/4" |
| RC1-4 | 12.0/12.0 | 0.135 | 5 x 1/4" | 5 x 1/4" |
| RC1-100 | 10.0/10.0 | 0.135 | 5 x 1/4" | 5 x 1/4" |
| RR1-101 | 10.0/10.0 | 0.135 | 5 x 1/4" | 5 x 1/4" |



RIGID FRAME ELEVATION: FRAME LINE 1 5

BOLT TIGHTENING (Snug-Tight)

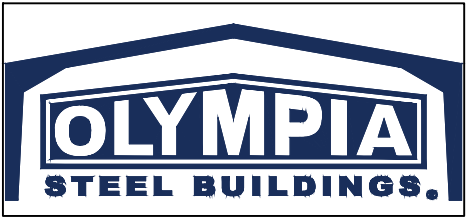
All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug-Tightened Joints in accordance with the Specification of Structural Joints Using High-Strength Bolts, June 11, 2020, installation as given in Section 7.1 Washers are not required for Snug-Tightened Joints using standard standard size holes per Section 6.1 of the Specification

Pretensioning methods, including Turn-of-Nut, calibrated wrench, twist-off tension control bolts or direct tension indicator are not required. Installation inspection requirements for Snug-Tight Bolt is found in Section 9.1 of the Specification.

☐ FOR APPROVAL: These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered complete.

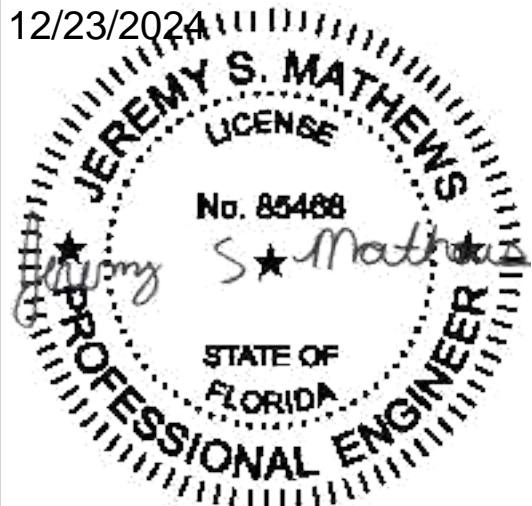
☐ FOR CONSTRUCTION PERMIT: These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

☒ FOR ERECTOR INSTALLATION: Final drawings for construction.



| ISSUE | DATE | DESCRIPTION | BY | CHK |
|-------|----------|-----------------------------|-----|-----|
| P1 | 08.23.23 | FOR CONSTRUCTION PERMIT | PND | PNC |
| 0 | 08.29.23 | FOR ERECTOR INSTALLATION | PND | PNC |
| P2 | 12.20.24 | REV FOR CONSTRUCTION PERMIT | PND | PNC |

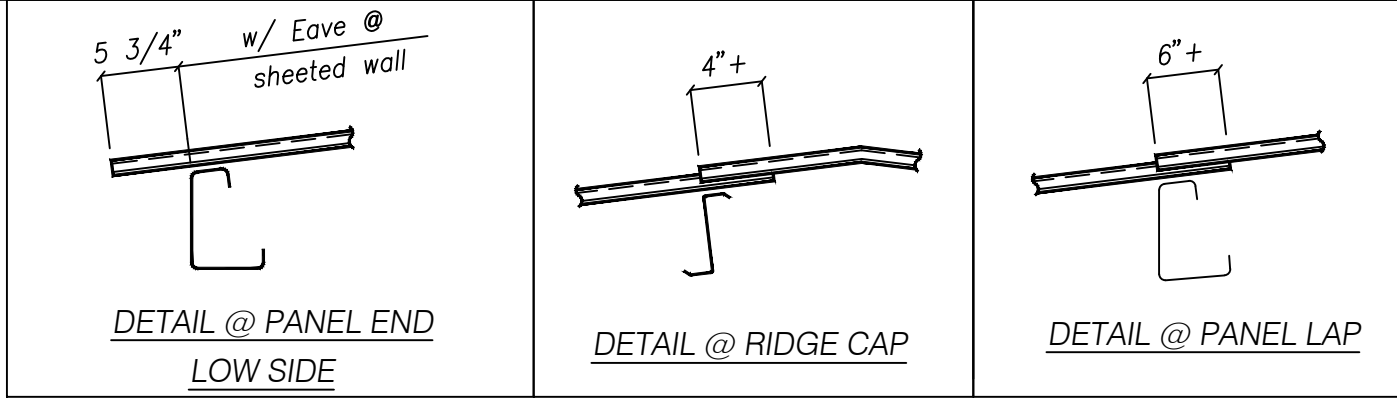
| | | | |
|-----------------------|------|----------------------|------|
| SHEET DESCRIPTION: | | BLDG SIZE: | |
| RIGID FRAME ELEVATION | | VARIES | |
| CUSTOMER: | | CUSTOMER LOCATION: | |
| MARCO BUILDER INC. | | FORT WHITE, FL 32038 | |
| PROJECT REFERENCE: | | MARCO BUILDER INC. | |
| JOB SITE LOCATION: | | JOB SITE COUNTY: | |
| FORT WHITE, FL 32038 | | COLUMBIA | |
| OWN: | CHK: | DATE: | ENG: |
| PND | PNC | 08.29.23 | MAH |
| JOB NO: | | DWG NO: | |
| 11463-32450 | | P1 | |
| ISSUE: | | P2 | |



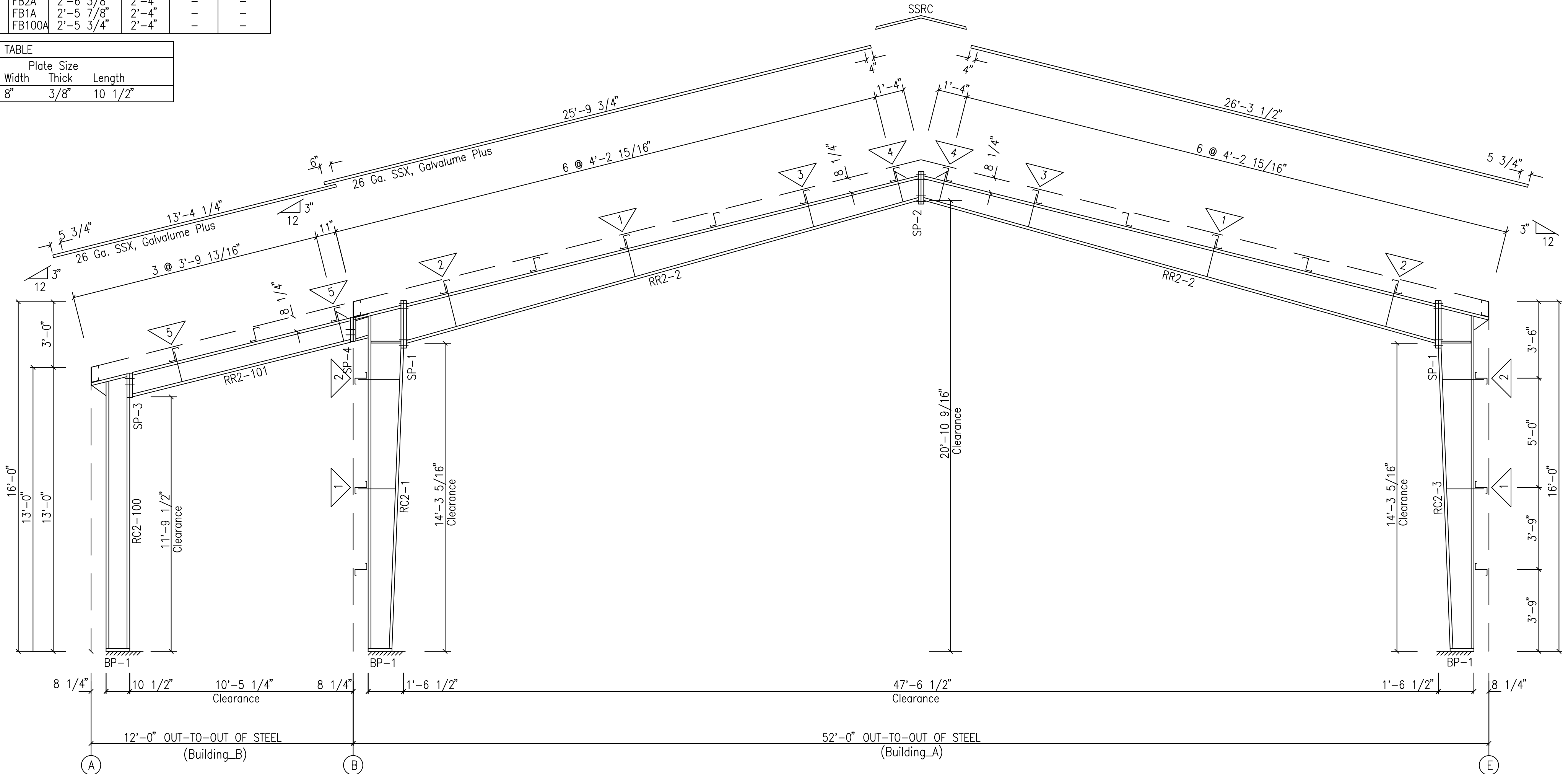
| SPlice PLATE & BOLT TABLE | | | | | | | | | |
|---------------------------|-----|-----|-----|-----|------|------|--------|-------|-------|
| Mark | Qty | | Bot | Int | Type | Dia | Length | Width | Thick |
| | Top | Bot | | | | | | | |
| SP-1 | 4 | 4 | 0 | | A325 | 3/4" | 2" | 6" | 1/2" |
| SP-2 | 4 | 4 | 0 | | A325 | 3/4" | 1 3/4" | 6" | 3/8" |
| SP-3 | 4 | 0 | 0 | | A325 | 3/4" | 1 3/4" | 6" | 3/8" |
| SP-4 | 4 | 0 | 0 | | A325 | 3/4" | 1 3/4" | 6" | 3/8" |

| FLANGE BRACE TABLE | | | | | | |
|--|---|--------|-----------|--------|--------|------|
| A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16 | | | | | | |
| FRAME LINE: 2 3 4 | | | | | | |
| ▽ ID | # | MARK | LENGTH | OFFSET | DETAIL | CLIP |
| 1 | 1 | FB4A | 2'-7 5/8" | 2'-4" | - | - |
| 2 | 1 | FB5A | 2'-9 1/8" | 2'-4" | - | - |
| 3 | 1 | FB2A | 2'-6 3/8" | 2'-4" | - | - |
| 4 | 1 | FB1A | 2'-5 7/8" | 2'-4" | - | - |
| 5 | 1 | FB100A | 2'-5 3/4" | 2'-4" | - | - |

| BASE PLATE TABLE | | | |
|------------------|------------|-------|---------|
| Col Mark | Plate Size | | |
| | Width | Thick | Length |
| BP-1 | 8" | 3/8" | 10 1/2" |



| MEMBER TABLE | | | | |
|--------------|-----------|-----------|-----------------|------------------------|
| Mark | Web Depth | | Web Plate Thick | Outside Flange W x Thk |
| | Start/End | End | | |
| RC2-1 | 10.0/15.8 | 15.8/18.0 | 0.135 | 6 x 1/4" |
| RR2-2 | 15.8/18.0 | 18.0/15.2 | 0.164 | 6 x 1/4" |
| RC2-3 | 18.0/15.8 | 15.2/10.0 | 0.135 | 5 x 1/4" |
| RC2-100 | 15.8/10.0 | 10.0/10.0 | 0.135 | 6 x 1/4" |
| RR2-101 | 10.0/10.0 | 10.0/10.0 | 0.135 | 5 x 1/4" |



RIGID FRAME ELEVATION: FRAME LINE 2 3 4

BOLT TIGHTENING (Snug-Tight)

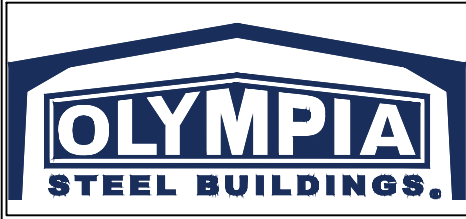
All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug-Tightened Joints in accordance with the Specification of Structural Joints Using High-Strength Bolts, June 11, 2020, installation as given in Section 7.1 Washers are not required for Snug-Tightened Joints using standard standard size holes per Section 6.1 of the Specification

Pretensioning methods, including Turn-of-Nut, calibrated wrench, twist-off tension control bolts or direct tension indicator are not required. Installation inspection requirements for Snug-Tight Bolt is found in Section 9.1 of the Specification.

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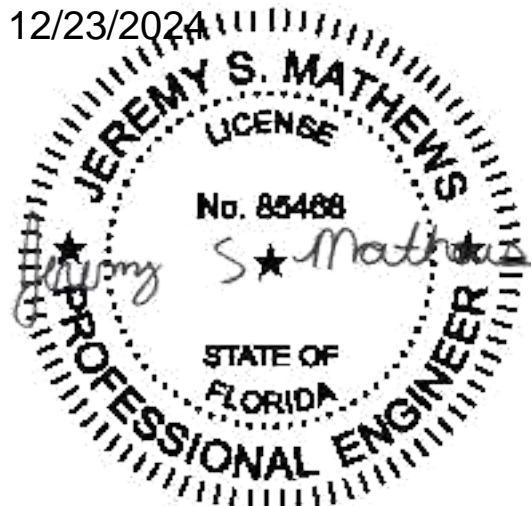
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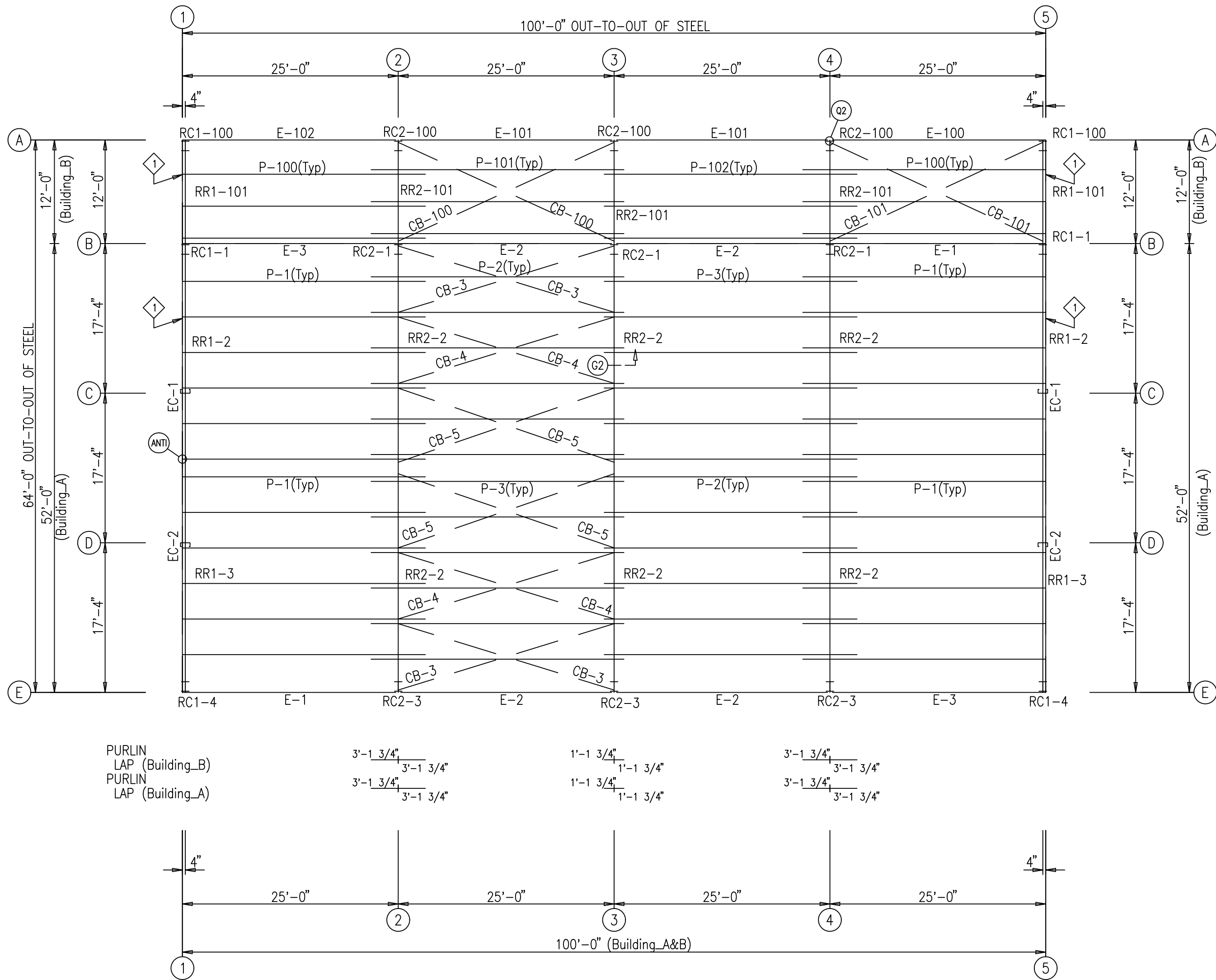
☒ FOR ERECTOR INSTALLATION: Final drawings for construction.



| ISSUE | DATE | DESCRIPTION | BY | CHK | SHEET DESCRIPTION: | BLDG SIZE: |
|-------|----------|-----------------------------|-----|-----|---------------------------------------|---|
| P1 | 08.23.23 | FOR CONSTRUCTION PERMIT | PND | PNC | RIGID FRAME ELEVATION | VARIES |
| 0 | 08.29.23 | FOR ERECTOR INSTALLATION | PND | PNC | CUSTOMER: MARCO BUILDER INC. | CUSTOMER LOCATION: FORT WHITE, FL 32038 |
| P2 | 12.20.24 | REV FOR CONSTRUCTION PERMIT | PND | PNC | PROJECT REFERENCE: MARCO BUILDER INC. | JOB SITE LOCATION: FORT WHITE, FL 32038 |
| | | | | | JOB SITE COUNTY: COLUMBIA | |
| | | | | | DWN: PND | CHK: PNC |
| | | | | | DATE: 08.29.23 | ENG: MAH |
| | | | | | JOB NO: 11463-32450 | DWG NO: P2 |
| | | | | | ISSUE: P2 | |

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ROOF FRAMING PLAN

| MEMBER TABLE | | |
|--------------|----------|--|
| ROOF PLAN | | |
| MARK | PART | |
| (Building_A) | | |
| P-1 | 8X25Z16 | |
| P-2 | 8X25Z16 | |
| P-3 | 8X25Z16 | |
| E-1 | 8ES143 | |
| E-2 | 8ES143 | |
| E-3 | 8ES143 | |
| CB-3 | 0.25_CBL | |
| CB-4 | 0.25_CBL | |
| CB-5 | 0.25_CBL | |
| (Building_B) | | |
| P-100 | 8X25Z16 | |
| P-101 | 8X25Z16 | |
| P-102 | 8X25Z16 | |
| E-100 | 8ES143 | |
| E-101 | 8ES143 | |
| E-102 | 8ES143 | |
| CB-100 | 0.25_CBL | |
| CB-101 | 0.25_CBL | |

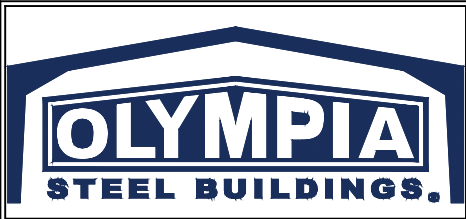
| ANGLE TABLE | | |
|-------------|--------|--------|
| ROOF PLAN | | |
| ID | MARK | LENGTH |
| 1 | RA2000 | 20'-0" |

UL580, CLASS 90 CONST. NUMBER 167

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☒ FOR ERECTOR INSTALLATION:
Final drawings for construction.

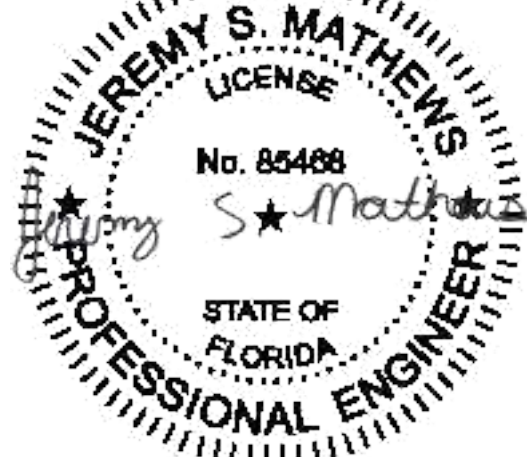


| ISSUE | DATE | DESCRIPTION | BY | CHK |
|-------|----------|-----------------------------|-----|-----|
| P1 | 08.23.23 | FOR CONSTRUCTION PERMIT | PND | PNC |
| 0 | 08.29.23 | FOR ERECTOR INSTALLATION | PND | PNC |
| P2 | 12.20.24 | REV FOR CONSTRUCTION PERMIT | PND | PNC |

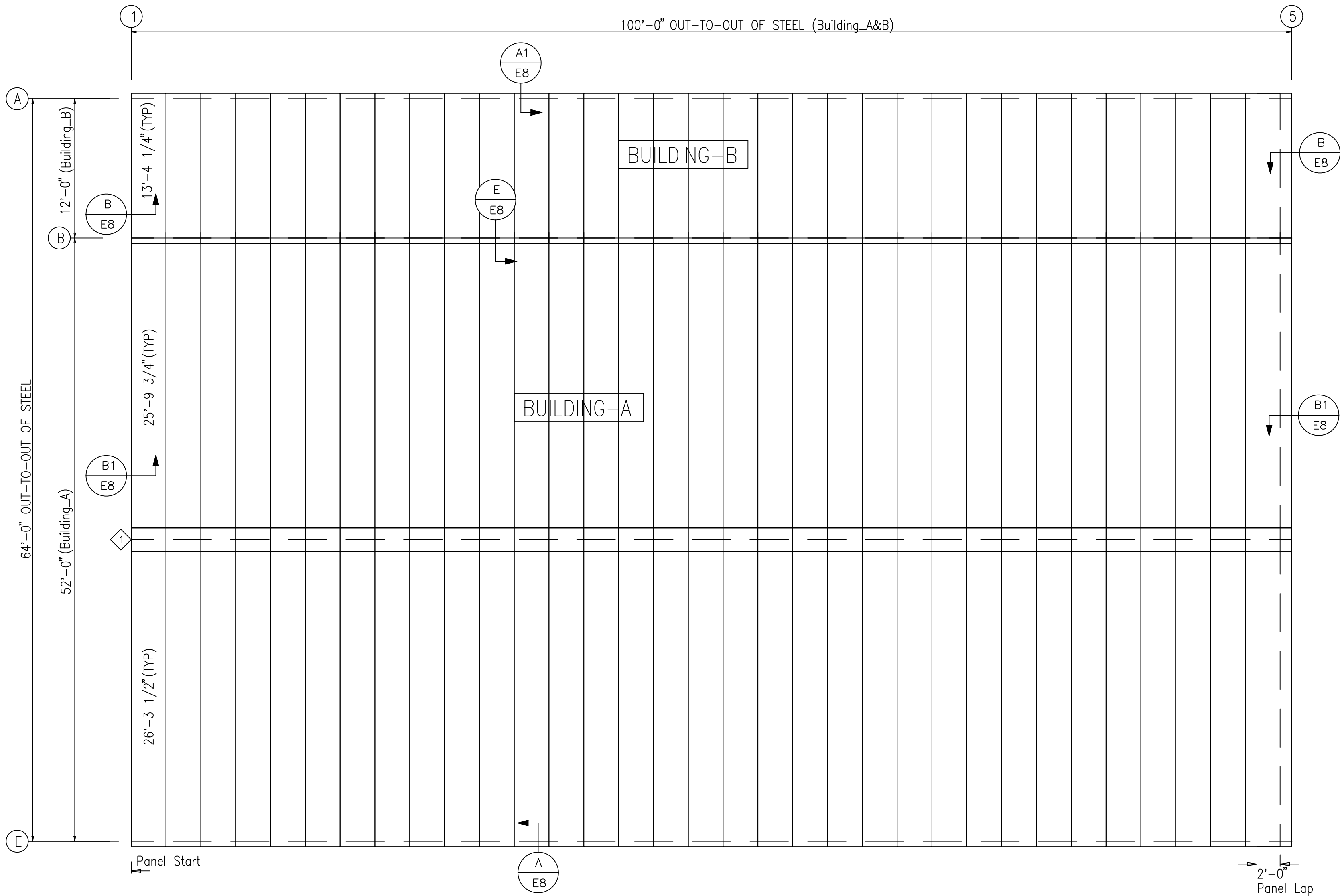
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| SHEET DESCRIPTION: | | BLDG SIZE: | |
|----------------------|------|----------------------|------|
| ROOF FRAMING PLAN | | VARIES | |
| CUSTOMER: | | CUSTOMER LOCATION: | |
| MARCO BUILDER INC. | | FORT WHITE, FL 32038 | |
| PROJECT REFERENCE: | | MARCO BUILDER INC. | |
| JOBSITE LOCATION: | | JOBSITE COUNTY: | |
| FORT WHITE, FL 32038 | | COLUMBIA | |
| DWN: | CHK: | DATE: | ENG: |
| PND | PNC | 08.29.23 | MAH |
| JOB NO: | | DWG NO: | |
| 11463-32450 | | E1 | |
| ISSUE: | | P2 | |

12/23/2024



| ROOF SHEETING TRIM TABLE | | |
|--------------------------|--------|--------|
| ◊ID | PART | LENGTH |
| 1 | SSRC30 | 3'-0" |



ROOF SHEETING PLAN
PANELS: 26 Ga. SSX – Galvalume Plus

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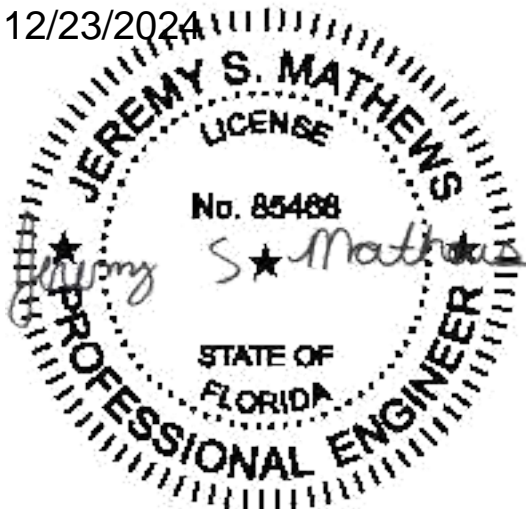
☒ FOR ERECTOR INSTALLATION:
Final drawings for construction.

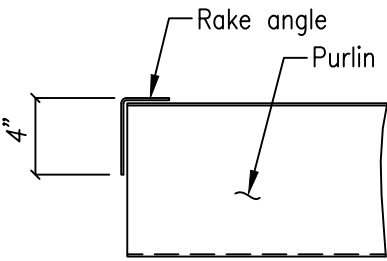


| ISSUE | DATE | DESCRIPTION | BY | CHK |
|-------|----------|-----------------------------|-----|-----|
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| 0 | 08.29.23 | FOR ERECTOR INSTALLATION | PND | PNC |
| P2 | 12.20.24 | REV FOR CONSTRUCTION PERMIT | PND | PNC |

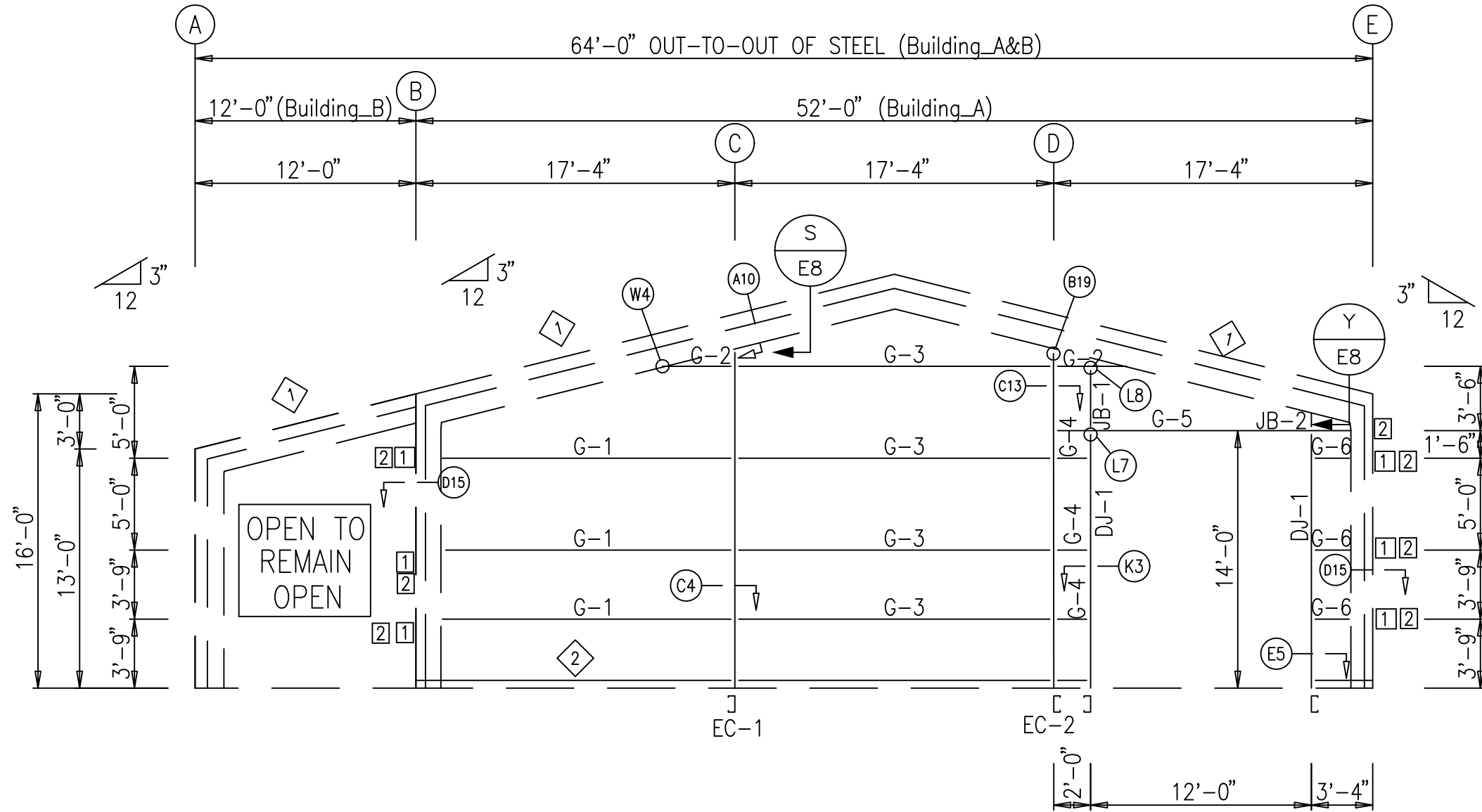
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|----------------------|------|----------------------|------|
| ROOF SHEETING PLAN | | VARIES | |
| CUSTOMER: | | CUSTOMER LOCATION: | |
| MARCO BUILDER INC. | | FORT WHITE, FL 32038 | |
| PROJECT REFERENCE: | | MARCO BUILDER INC. | |
| JOBSITE LOCATION: | | JOBSITE COUNTY: | |
| FORT WHITE, FL 32038 | | COLUMBIA | |
| DWN: | CHK: | DATE: | ENG: |
| PND | PNC | 08.29.23 | MAH |
| JOB NO: | | DWG NO: | |
| 11463-32450 | | E2 | |
| ISSUE: | | P2 | |

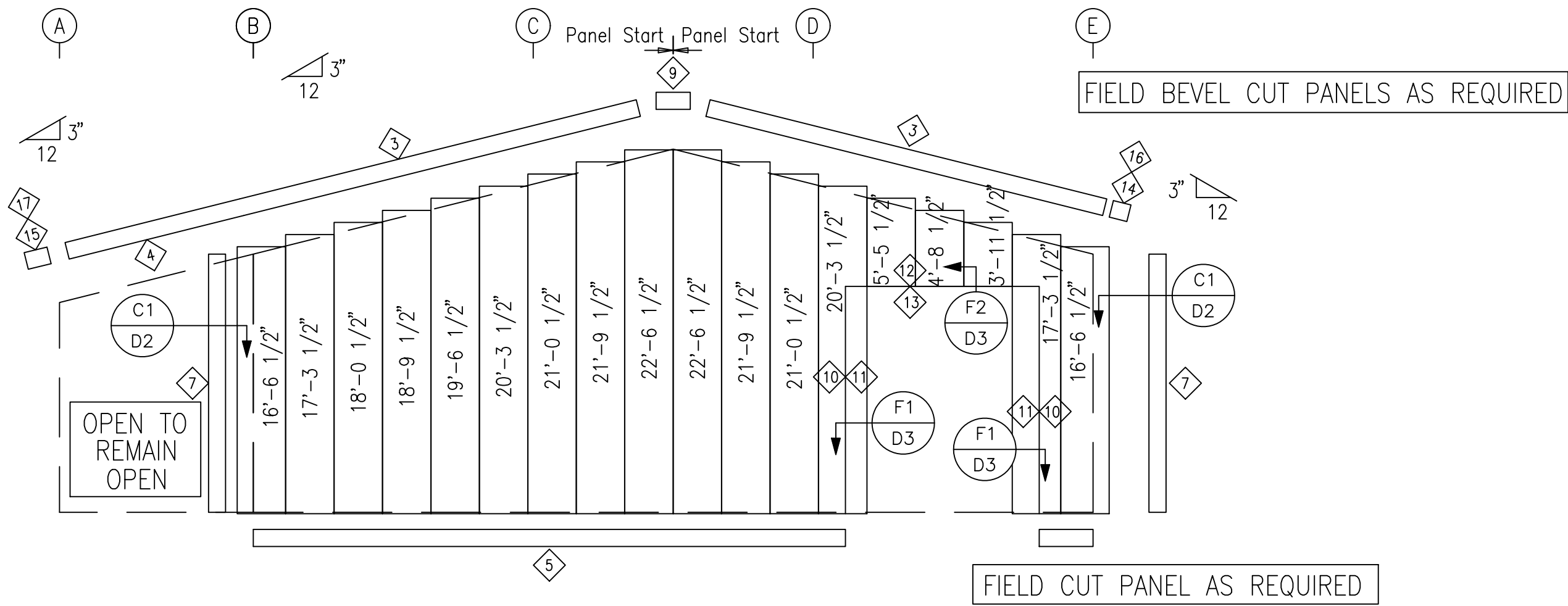




Detail at Rake Angle



ENDWALL FRAMING: FRAME LINE 1



GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout straps are located 6" from base and at every girt location.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self-tap girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
(2) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd
(2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
14. Sub-jams for overhead doors, if required, is not furnished by Metal Building Provider

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Final drawings for construction.



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| P1 | 08.23.23 | FOR CONSTRUCTION PERMIT | PND | PNC |
| 0 | 08.29.23 | FOR ERECTOR INSTALLATION | PND | PNC |
| P2 | 12.20.24 | REV FOR CONSTRUCTION PERMIT | PND | PNC |

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| SHEET DESCRIPTION: | | BLDG SIZE: | |
|------------------------------------|------|----------------------|--------|
| ENDWALL FRAME & SHEETING ELEVATION | | VARIES | |
| CUSTOMER: | | CUSTOMER LOCATION: | |
| MARCO BUILDER INC. | | FORT WHITE, FL 32038 | |
| PROJECT REFERENCE: | | MARCO BUILDER INC. | |
| JOB SITE LOCATION: | | JOB SITE COUNTY: | |
| FORT WHITE, FL 32038 | | COLUMBIA | |
| OWN: | CHK: | DATE: | ENG: |
| PND | PNC | 08.29.23 | MAH |
| JOB NO: | | DWG NO: | ISSUE: |
| 11463-32450 | | E3 | P2 |

| BOLT TABLE FRAME LINE 1 | | | | |
|----------------------------|------|------|------|--------|
| LOCATION | QUAN | TYPE | DIA | LENGTH |
| (Building_A) | | | | |
| Columns/Raf | 2 | A325 | 5/8" | 1 1/2" |
| Jamb/Raf | 2 | A325 | 5/8" | 1 1/2" |

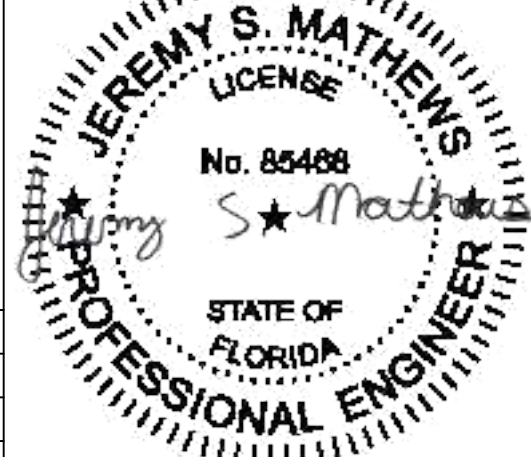
| TRIM TABLE FRAME LINE 1 | | |
|----------------------------|---------|--------|
| ID | MARK | LENGTH |
| 3 | RT-101 | 15'-3" |
| 4 | CF-102 | 15'-3" |
| 5 | BT-101 | 10'-3" |
| 7 | CT-102 | 16'-4" |
| 9 | SPB- | |
| 10 | MT-116B | 14'-4" |
| 11 | FL-22 | 14'-4" |
| 12 | MT-116B | 12'-4" |
| 13 | HT-101 | 12'-4" |
| 14 | SPCB-3R | |
| 15 | SPCB-3L | |
| 16 | SF-3R | |
| 17 | SF-3L | |

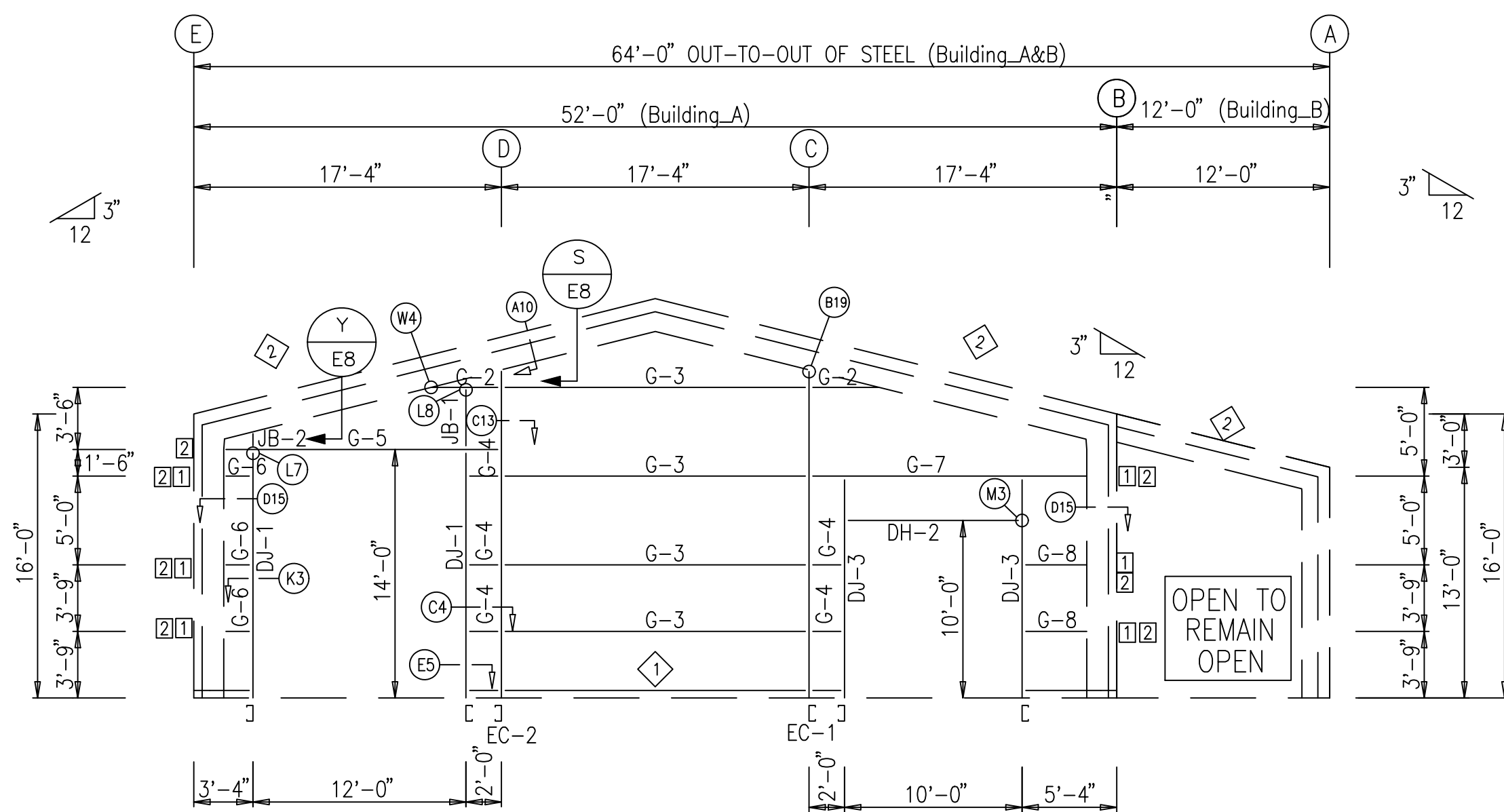
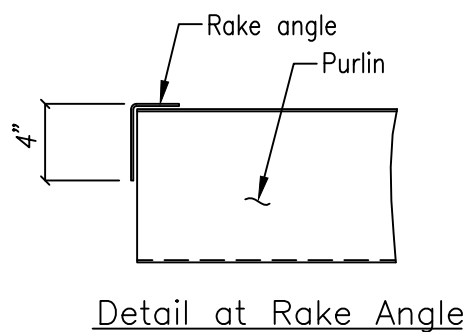
| MEMBER TABLE FRAME LINE 1 | |
|------------------------------|---------|
| MARK | PART |
| (Building_A) | |
| EC-1 | 8M35C12 |
| EC-2 | 8M35C12 |
| DJ-1 | 8M35C14 |
| JB-1 | 8M35C14 |
| JB-2 | 8M35C14 |
| G-1 | 8X25Z16 |
| G-2 | 8X25Z16 |
| G-3 | 8X25Z16 |
| G-4 | 8X25Z16 |
| G-5 | 8X25C14 |
| G-6 | 8X25Z16 |

| ANGLE TABLE FRAME LINE 1 | | |
|-----------------------------|---------|--------|
| ID | MARK | LENGTH |
| 1 | RA2000 | 20'-0" |
| 2 | BB2000G | 20'-0" |

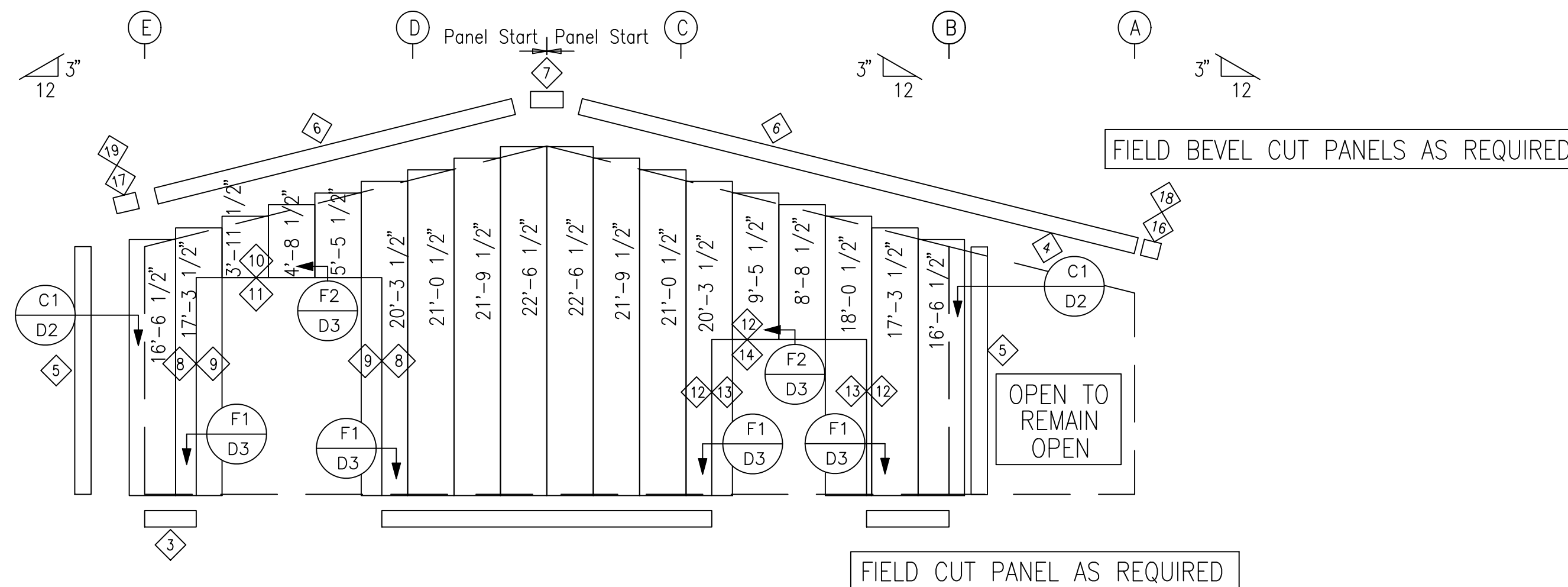
| CONNECTION PLATES FRAME LINE 1 | |
|-----------------------------------|-----------|
| ID | MARK/PART |
| 1 | SC-5 |
| 2 | Z-1 |

12/23/2024





ENDWALL FRAMING: FRAME LINE 5



ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 Ga. SSX -SMP Surfsand

GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stich screws are located at each member with two between members (20" max. spacing).
5. Wall stich screws are located at each member with one between members (20" max. spacing).
6. Skylight stich screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'-0" centers and caulk all laps.
12. Gutter support strapping: Super Span 3'-0", Super Seam 4'-0" Weather Lok-16 2"-8".
13. Corner and/or panel ends to be furnished with special rake or gutter profiles. Field miters as req'd.
14. Downspout straps are located 6" from base and at every girt location.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self-tap girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
 - (1) $5/8" \times 1\frac{1}{2}"$ A325 bolts if (1) AK400 req'd
 - (2) $5/8" \times 1\frac{3}{4}"$ A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill $9/16"$ diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
14. Sub-jambes for overhead doors, if required, is not furnished by Metal Building Provider

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☒ **FOR ERECTOR INSTALLATION:**
Final drawings for construction.



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| P1 | 08.23.23 | FOR CONSTRUCTION PERMIT | PND | PND |
| 0 | 08.29.23 | FOR ERECTOR INSTALLATION | PND | PND |
| P2 | 12.20.24 | REV FOR CONSTRUCTION PERMIT | PND | PND |
| | | | | |
| | | | | |
| | | | | |

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| | | | | | | | |
|--|------|----------|------|--|---------|-----------------------------|--|
| SHEET DESCRIPTION: ENDWALL FRAME & SHEETING ELEVATION | | | | BLDG SIZE: VARIES | | | |
| CUSTOMER: MARCO BUILDER INC. | | | | CUSTOMER LOCATION: FORT WHITE, FL 32038 | | | |
| PROJECT REFERENCE: MARCO BUILDER INC. | | | | | | | |
| JOBSITE LOCATION: FORT WHITE, FL 32038 | | | | | | JOBSITE COUNTY: COLUMBIA | |
| DWN: | CHK: | DATE: | ENG: | JOB NO: | DWG NO: | ISSUE: | |
| PND | PNC | 08.29.23 | MAH | 11463-32450 | E4 | P2 | |

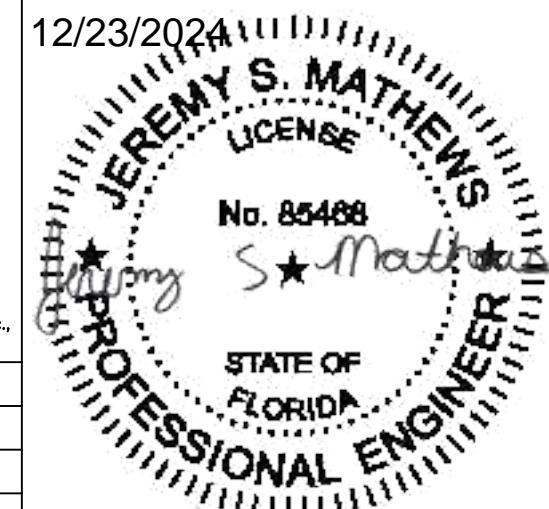
| | | | | |
|--------------|------|------|------|--------|
| BOLT TABLE | | | | |
| FRAME LINE 5 | | | | |
| LOCATION | QUAN | TYPE | DIA | LENGTH |
| (Building_A) | | | | |
| Columns/Raf | 2 | A325 | 5/8" | 1 1/2" |
| Jamb/Raf | 2 | A325 | 5/8" | 1 1/2" |

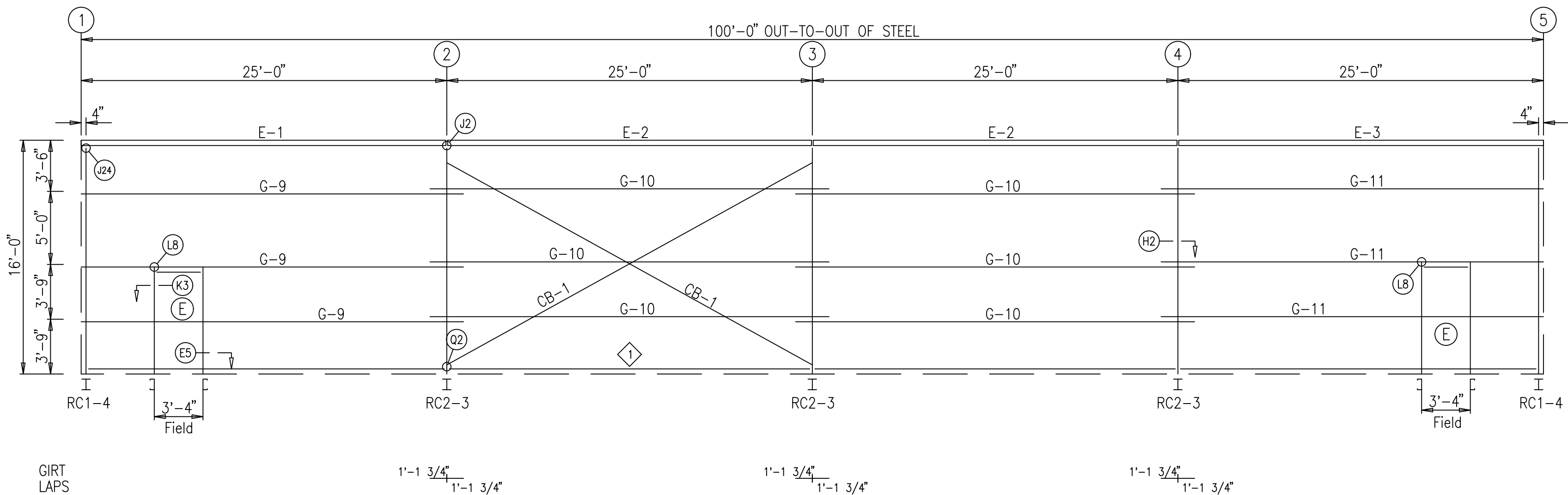
| TRIM TABLE FRAME LINE 5 | | |
|----------------------------|---------|--------|
| ◇ID | MARK | LENGTH |
| 3 | BT-101 | 10'-3" |
| 4 | CF-102 | 15'-3" |
| 5 | CT-102 | 16'-4" |
| 6 | RT-101 | 15'-3" |
| 7 | SPB_ | |
| 8 | MT-116B | 14'-4" |
| 9 | FL-22 | 14'-4" |
| 10 | MT-116B | 12'-4" |
| 11 | HT-101 | 12'-4" |
| 12 | MT-116B | 10'-4" |
| 13 | FL-22 | 10'-4" |
| 14 | HT-101 | 10'-4" |
| 16 | SPCB-3R | |
| 17 | SPCB-3L | |
| 18 | SF-3R | |
| 19 | SF-3L | |

| MEMBER TABLE FRAME LINE 5 | |
|------------------------------|---------|
| MARK | PART |
| (Building A) | |
| EC-1 | 8M35C12 |
| EC-2 | 8M35C12 |
| DJ-1 | 8M35C14 |
| DJ-3 | 8M35C14 |
| DH-2 | 8M25C14 |
| G-2 | 8X25Z16 |
| G-3 | 8X25Z16 |
| G-4 | 8X25Z16 |
| G-5 | 8X25C14 |
| G-6 | 8X25Z16 |
| G-7 | 8X25Z14 |
| G-8 | 8X25Z16 |
| JB-1 | 8M35C14 |
| JB-2 | 8M35C14 |

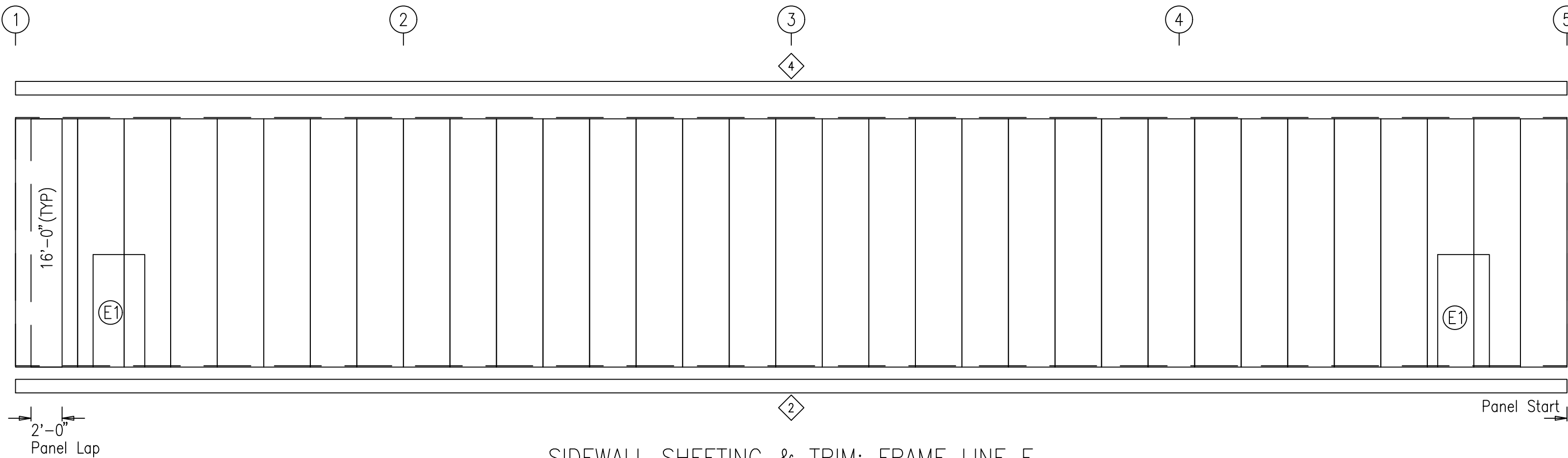
| ANGLE TABLE | | |
|--------------|---------|--------|
| FRAME LINE 5 | | |
| ◇ID | MARK | LENGTH |
| 1 | BB2000G | 20'-0" |
| 2 | RA2000 | 20'-0" |

| CONNECTION PLATES FRAME LINE 5 | |
|-----------------------------------|-----------|
| □ ID | MARK/PART |
| 1 | SC-5 |
| 2 | Z-1 |





SIDEWALL FRAMING: FRAME LINE E



SIDEWALL SHEETING & TRIM: FRAME LINE E

PANELS: 26 Ga. SSX -SMP Surfsand

GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout straps are located 6" from base and at every girt location.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

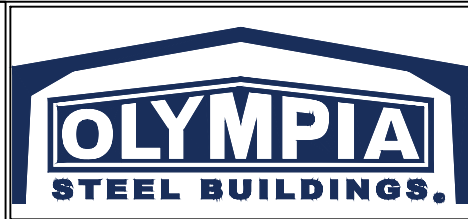
GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self-top girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
(2) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd
(2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
14. Sub-jams for overhead doors, if required, is not furnished by Metal Building Provider

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☒ FOR ERECTOR INSTALLATION:
Final drawings for construction.

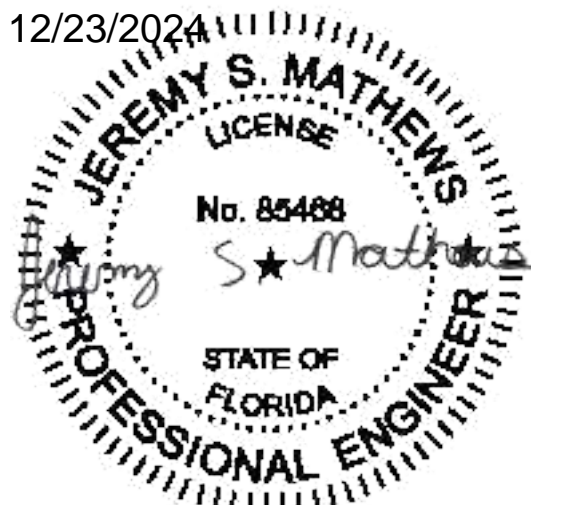
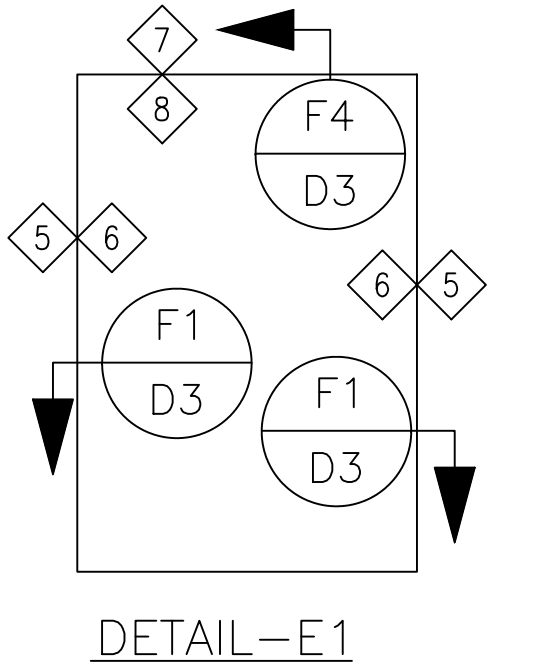
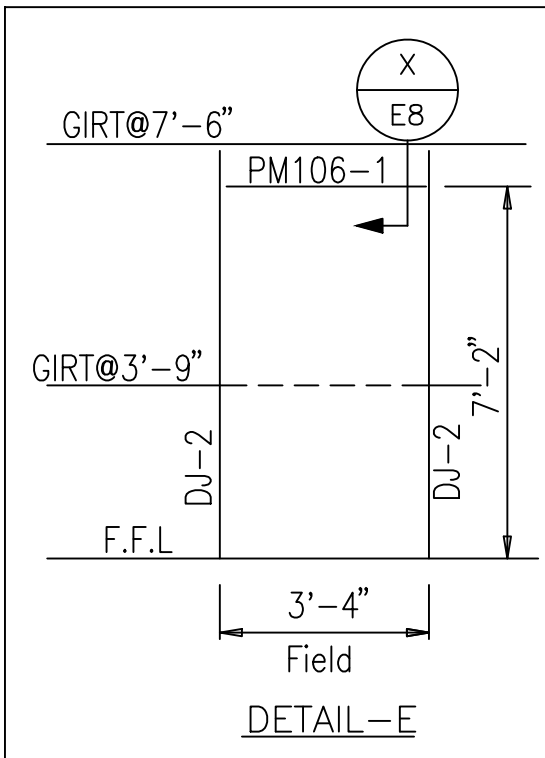


| ISSUE | DATE | DESCRIPTION | BY | CHK | SHEET DESCRIPTION: | BLDG SIZE: |
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| P1 | 08.23.23 | FOR CONSTRUCTION PERMIT | PND | PNC | SIDEWALL FRAME & SHEETING ELEVATION | VARIES |
| 0 | 08.29.23 | FOR ERECTOR INSTALLATION | PND | PNC | CUSTOMER: | CUSTOMER LOCATION: |
| P2 | 12.20.24 | REV FOR CONSTRUCTION PERMIT | PND | PNC | MARCO BUILDER INC. | FORT WHITE, FL 32038 |
| | | | | | PROJECT REFERENCE: | |
| | | | | | MARCO BUILDER INC. | |
| | | | | | JOB SITE LOCATION: | JOB SITE COUNTY: |
| | | | | | FORT WHITE, FL 32038 | COLUMBIA |
| | | | | | DWN: | CHK: |
| | | | | | PND | PNC |
| | | | | | DATE: | ENG: |
| | | | | | 08.29.23 | MAH |
| | | | | | JOB NO: | DWG NO: |
| | | | | | 11463-32450 | E5 |
| | | | | | ISSUE: | P2 |

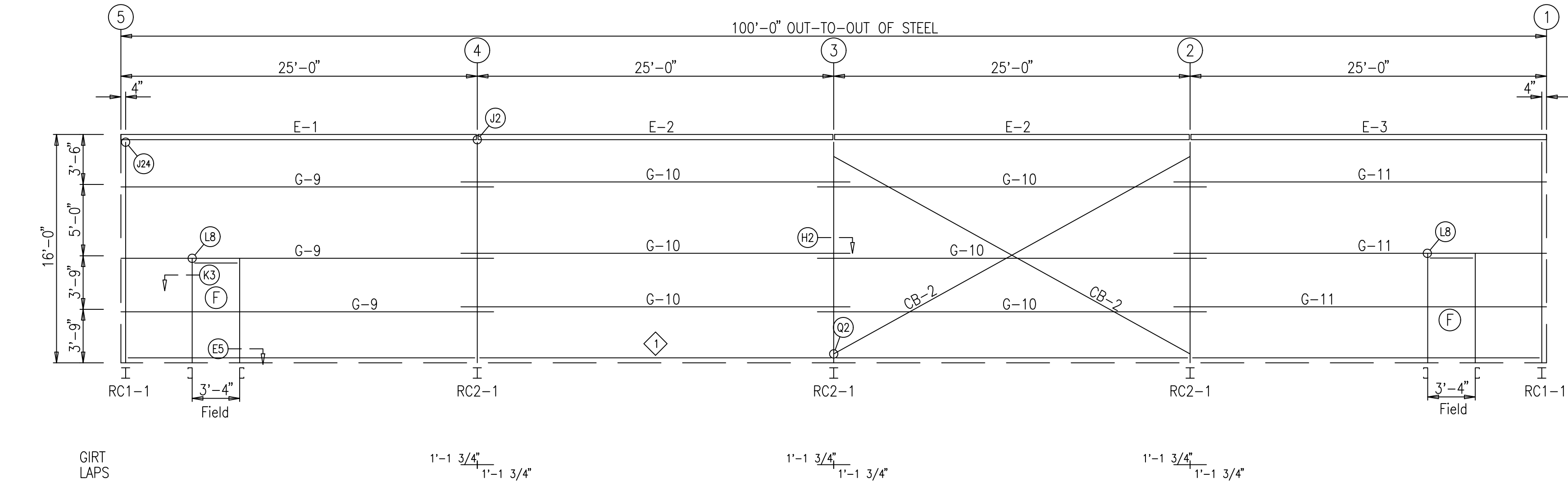
| TRIM TABLE | | |
|--------------|---------|--------|
| FRAME LINE E | | |
| ◇ ID | MARK | LENGTH |
| 2 | BT-101 | 10'-3" |
| 4 | ET-103 | 15'-3" |
| 5 | MT-116B | 7'-6" |
| 6 | FL-22 | 7'-6" |
| 7 | MT-116B | 3'-8" |
| 8 | HT-101 | 3'-8" |

| MEMBER TABLE | | |
|--------------|----------|--|
| FRAME LINE E | | |
| MARK | PART | |
| DJ-2 | 8M25C14 | |
| PM106-1 | PM106 | |
| E-1 | 8ES143 | |
| E-2 | 8ES143 | |
| E-3 | 8ES143 | |
| G-9 | 8X25Z16 | |
| G-10 | 8X25Z16 | |
| G-11 | 8X25Z16 | |
| CB-1 | 0.25_CBL | |

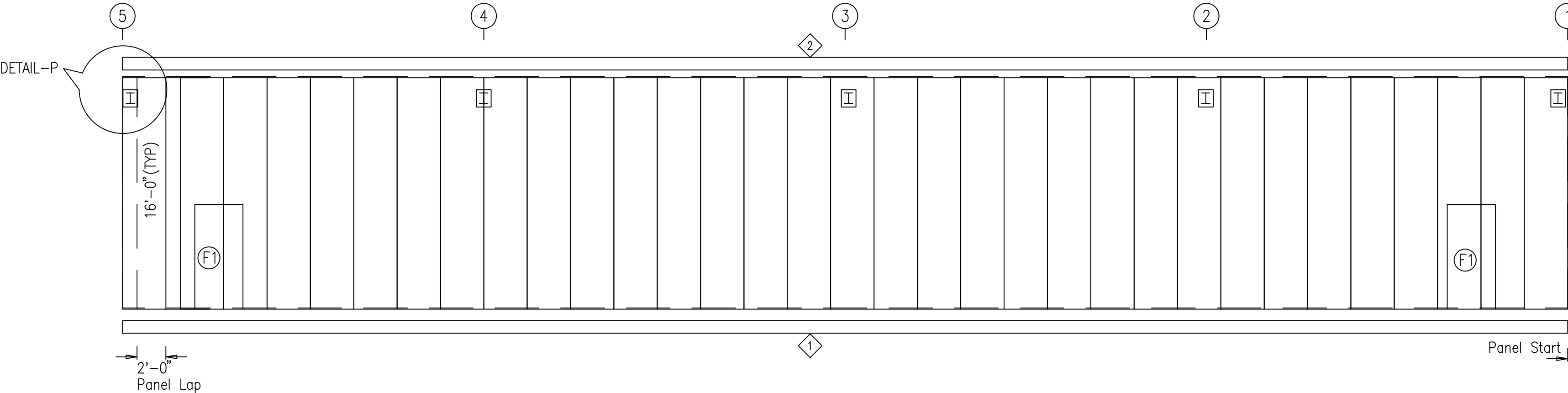
| ANGLE TABLE | | |
|--------------|---------|--------|
| FRAME LINE E | | |
| ◇ ID | MARK | LENGTH |
| 1 | BB2000G | 20'-0" |



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SIDEWALL FRAMING: FRAME LINE B



SIDEWALL SHEETING & TRIM: FRAME LINE B

PANELS: 26 Ga. SSX -SMP Surfsand

GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout straps are located 6" from base and at every girt location.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self-top girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
(2) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd
(2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
14. Sub-jams for overhead doors, if required, is not furnished by Metal Building Provider

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☒ FOR ERECTOR INSTALLATION:
Final drawings for construction.

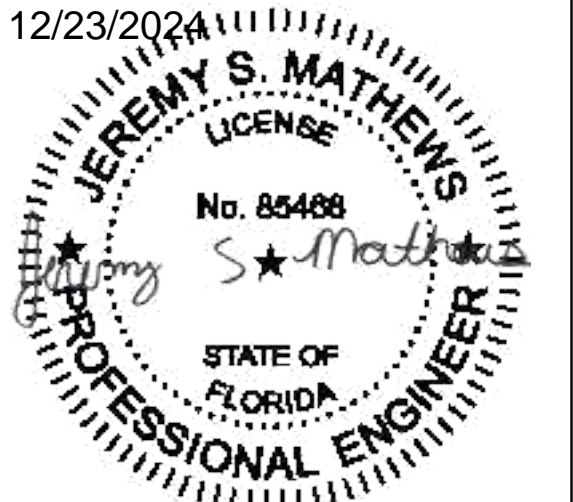
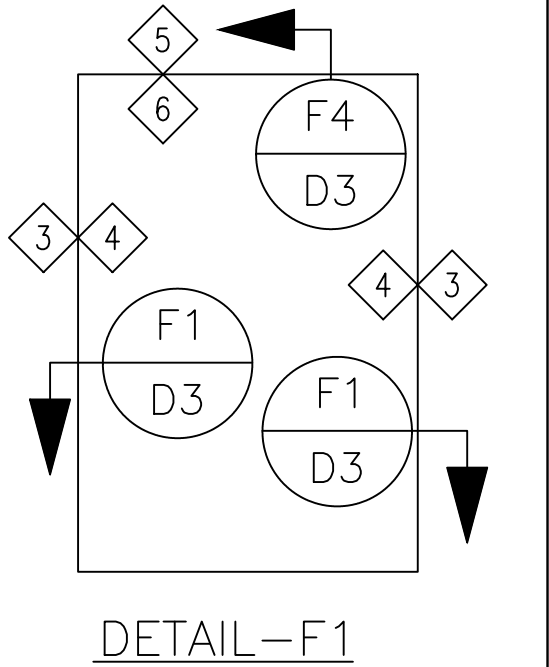
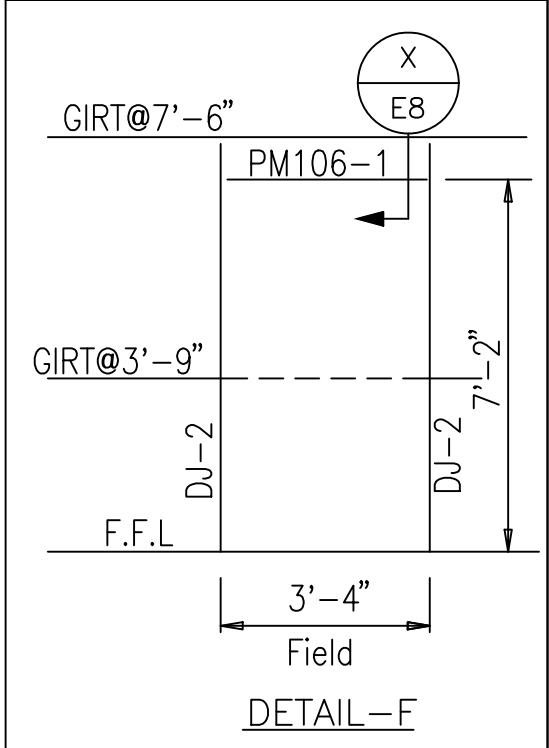


| ISSUE | DATE | DESCRIPTION | BY | CHK | SHEET DESCRIPTION: | BLDG SIZE: |
|-------|----------|-----------------------------|-----|-----|---|---|
| P1 | 08.23.23 | FOR CONSTRUCTION PERMIT | PND | PNC | SIDEWALL FRAME & SHEETING ELEVATION | VARIES |
| 0 | 08.29.23 | FOR ERECTOR INSTALLATION | PND | PNC | CUSTOMER: MARCO BUILDER INC. | CUSTOMER LOCATION: FORT WHITE, FL 32038 |
| P2 | 12.20.24 | REV FOR CONSTRUCTION PERMIT | PND | PNC | PROJECT REFERENCE: MARCO BUILDER INC. | |
| | | | | | JOB SITE LOCATION: FORT WHITE, FL 32038 | JOB SITE COUNTY: COLUMBIA |
| | | | | | DWN: PND | CHK: PNC |
| | | | | | DATE: 08.29.23 | ENG: MAH |
| | | | | | JOB NO: 11463-32450 | DWG NO: E6 |
| | | | | | | ISSUE: P2 |

| TRIM TABLE | | |
|--------------|---------|--------|
| FRAME LINE B | | |
| ID | MARK | LENGTH |
| 1 | BT-101 | 10'-3" |
| 2 | ET-803 | 15'-3" |
| 3 | MT-116B | 7'-6" |
| 4 | FL-22 | 7'-6" |
| 5 | MT-116B | 3'-8" |
| 6 | HT-101 | 3'-8" |

| MEMBER TABLE | |
|--------------|----------|
| FRAME LINE B | |
| MARK | PART |
| DJ-2 | 8M25C14 |
| PM106-1 | PM106 |
| E-1 | 8ES143 |
| E-2 | 8ES143 |
| E-3 | 8ES143 |
| G-9 | 8X25Z16 |
| G-10 | 8X25Z16 |
| G-11 | 8X25Z16 |
| CB-2 | 0.31-CBL |

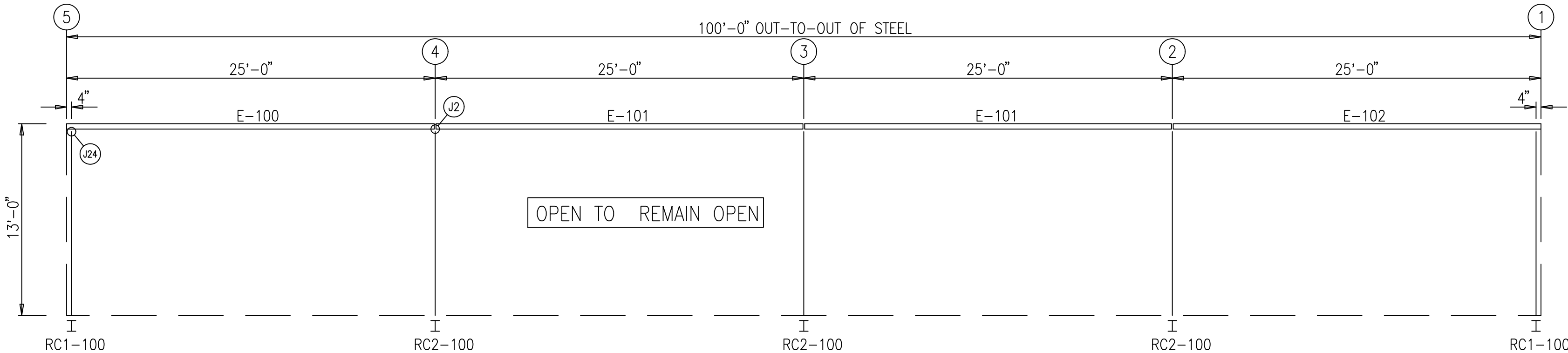
| ANGLE TABLE | | |
|--------------|---------|--------|
| FRAME LINE B | | |
| ID | MARK | LENGTH |
| 1 | BB2000G | 20'-0" |



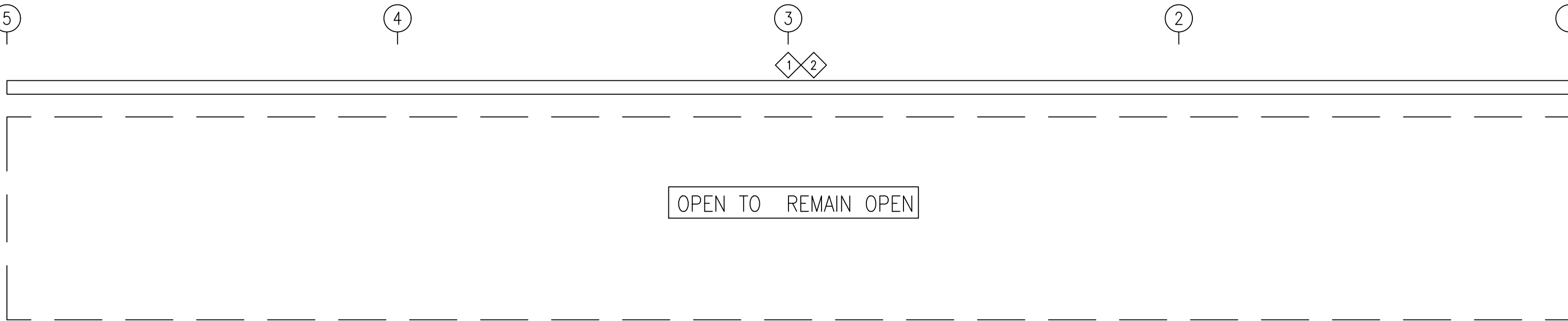
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| MEMBER TABLE | |
|--------------|--------|
| FRAME LINE A | |
| MARK | PART |
| E-100 | 8ES143 |
| E-101 | 8ES143 |
| E-102 | 8ES143 |

| TRIM TABLE | | |
|--------------|--------|--------|
| FRAME LINE A | | |
| ◇ID | MARK | LENGTH |
| 1 | ET-103 | 15'-3" |
| 2 | CF-102 | 15'-3" |



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL TRIM: FRAME LINE A

GENERAL SHEETING & TRIM NOTES

- Refer to erection drawings for rake angle locations.
- Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
- Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
- Roof stitch screws are located at each member with two between members (20" max. spacing).
- Wall stitch screws are located at each member with one between members (20" max. spacing).
- Skylight stitch screws are at 6" o.c.
- Start endwall panels at centerline of bldg. unless noted.
- Gutter, rake, & eave trim lap 2". All other trims lap 1".
- Field cut or lap panels as required to fit.
- Field cut panels for all openings.
- Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
- Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8".
- Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
- Downspout straps are located 6" from base and at every girt location.
- Hot-rolled or built-up members must be pre-drilled before attaching members screws.
- Metal shavings must be swept from the roof each day to avoid surface rusting.
- Windows and louvers must be installed before sheeting the walls.
- For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

GENERAL FRAMING NOTES

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- Flange braces are marked by their length in decimal inches.
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- Field slot girts for brace rods or cables.
- Field locate windows and walk doors.
- Field weld all splices at 14 gauge valley gutters.
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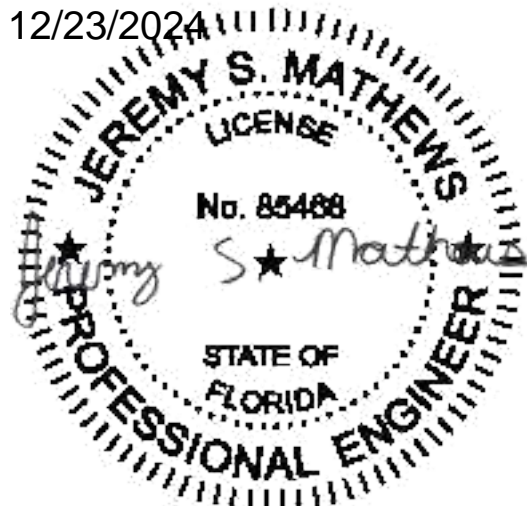
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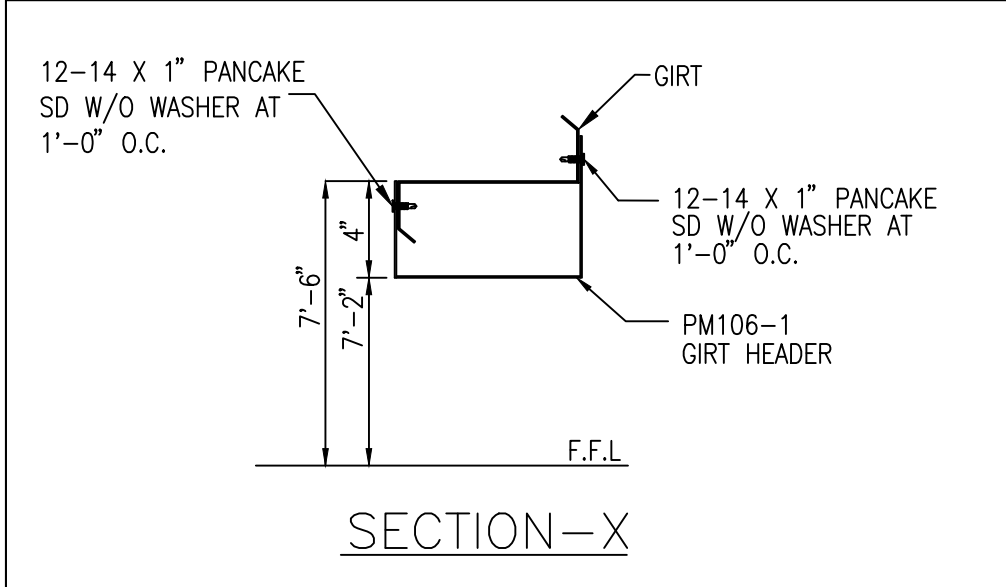
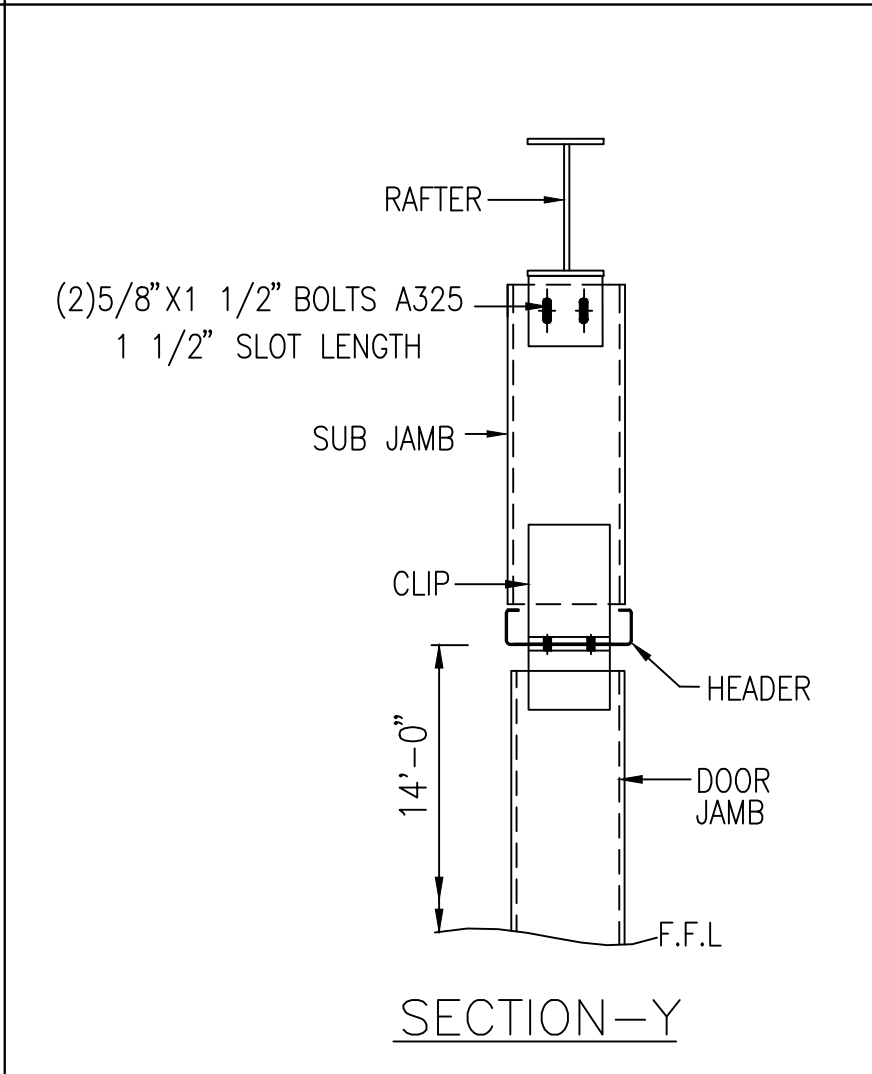
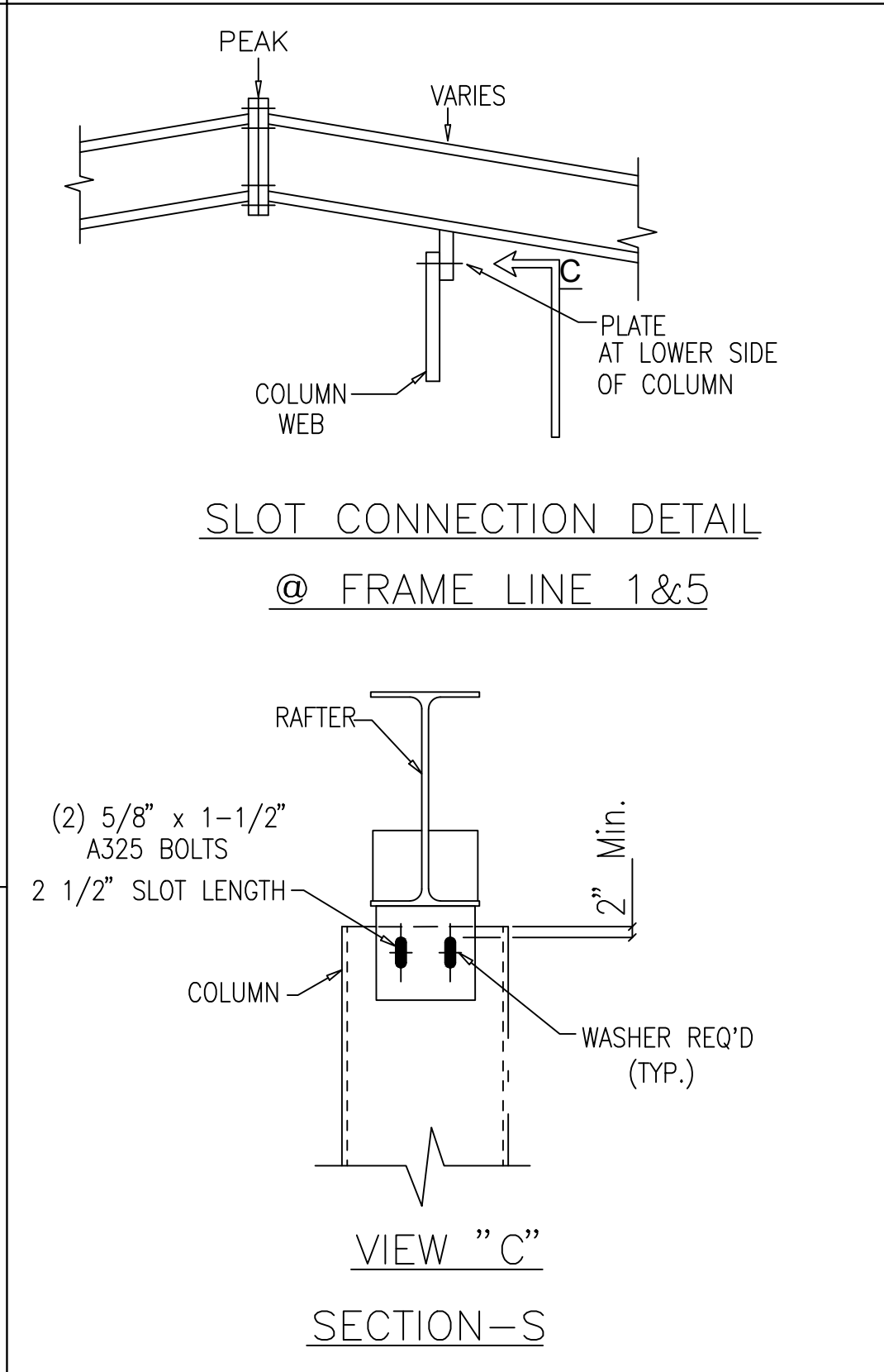
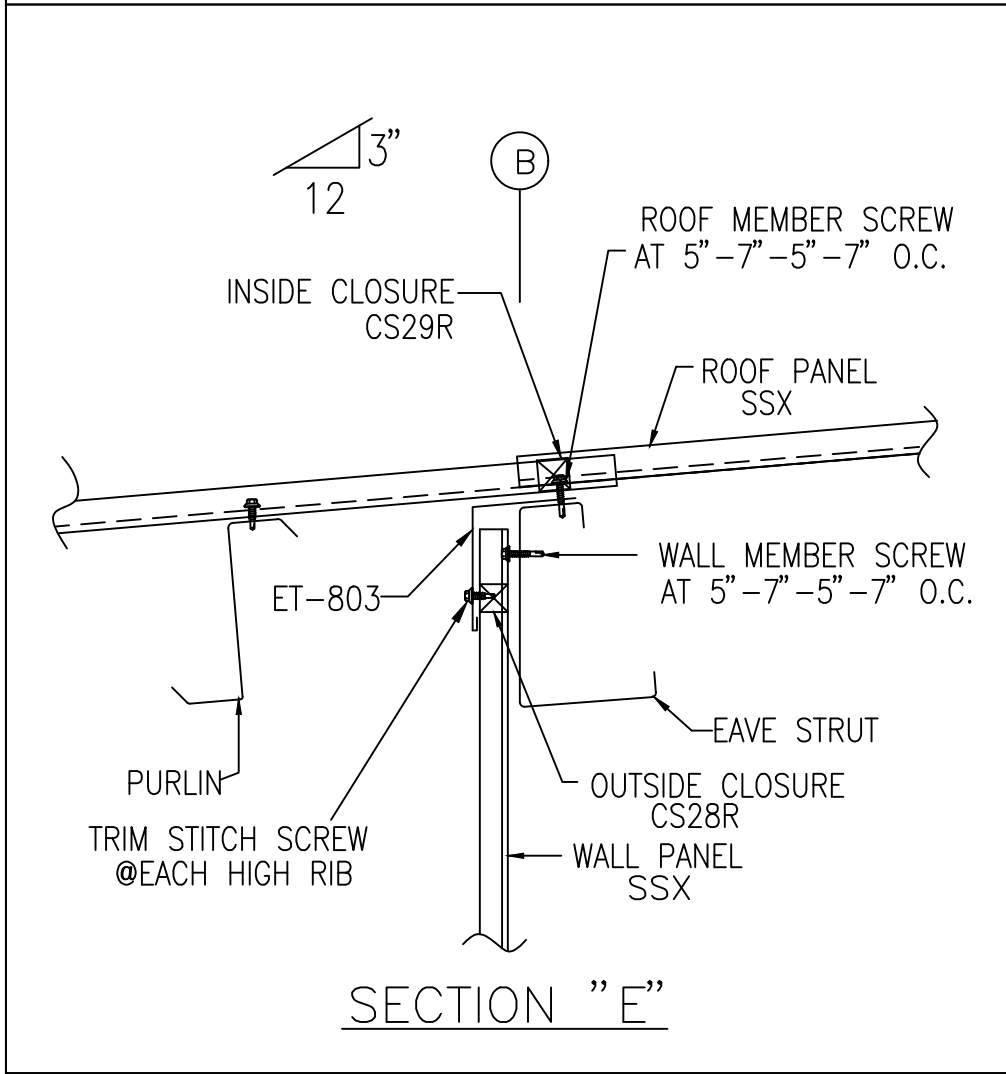
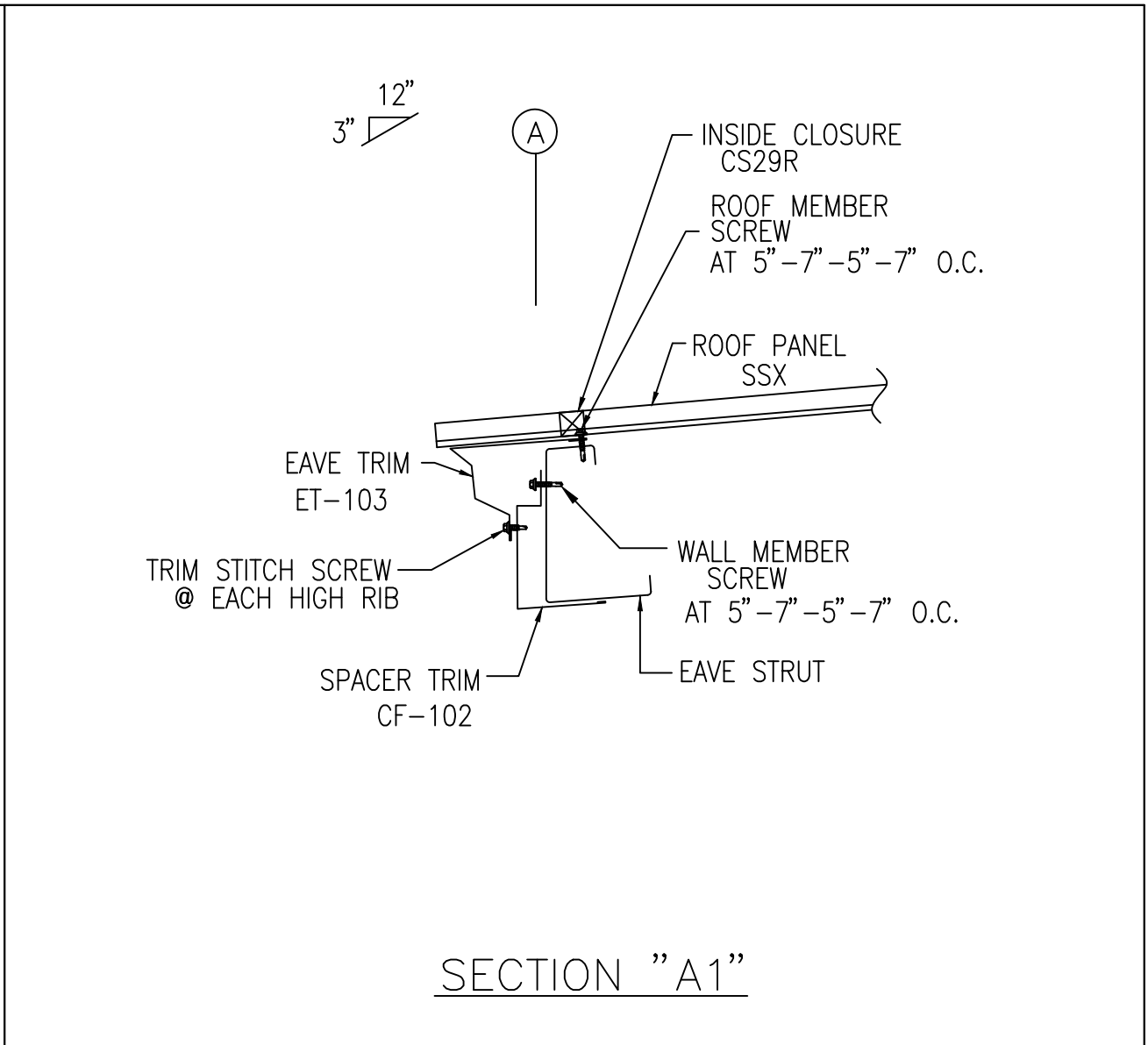
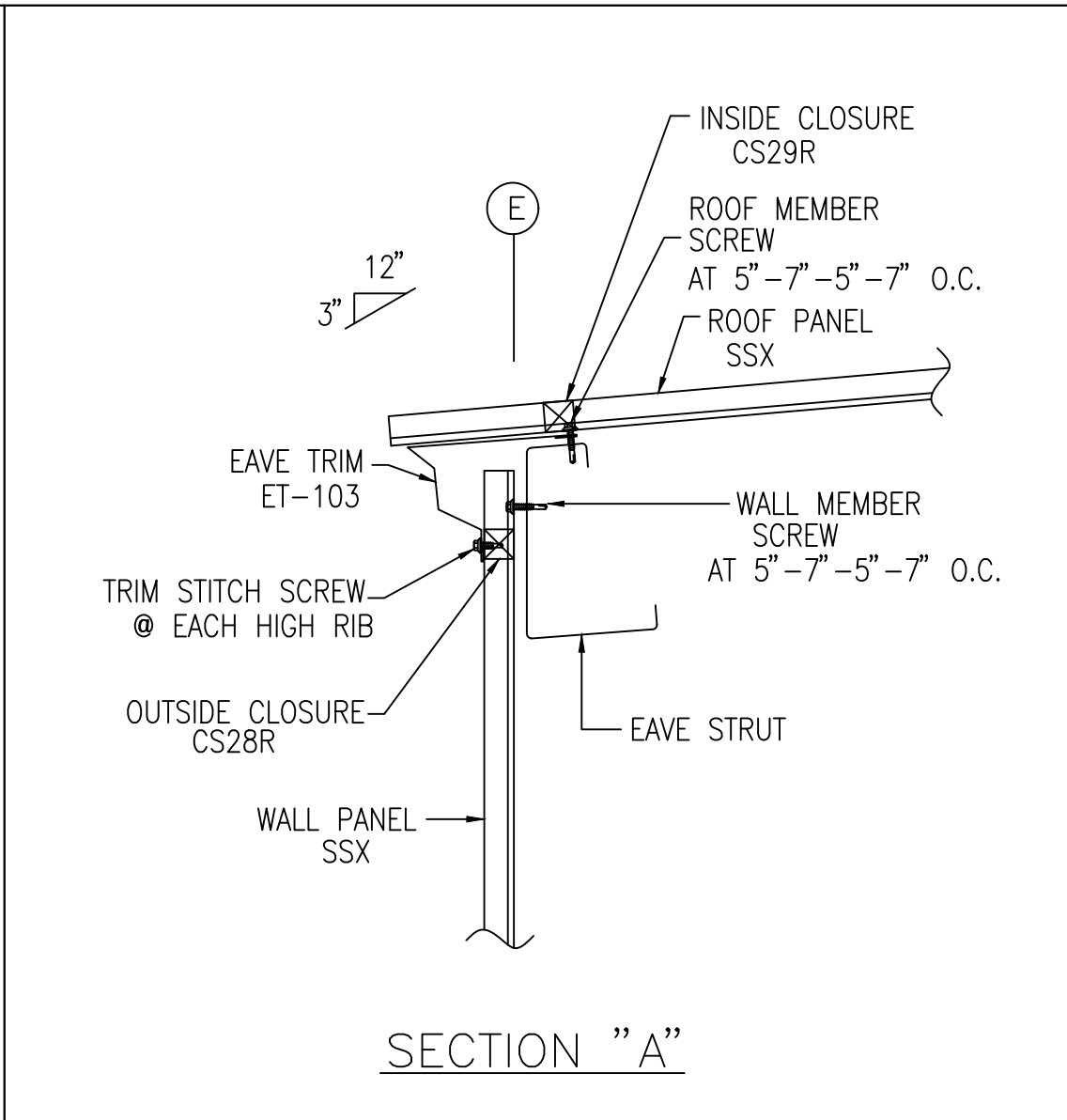
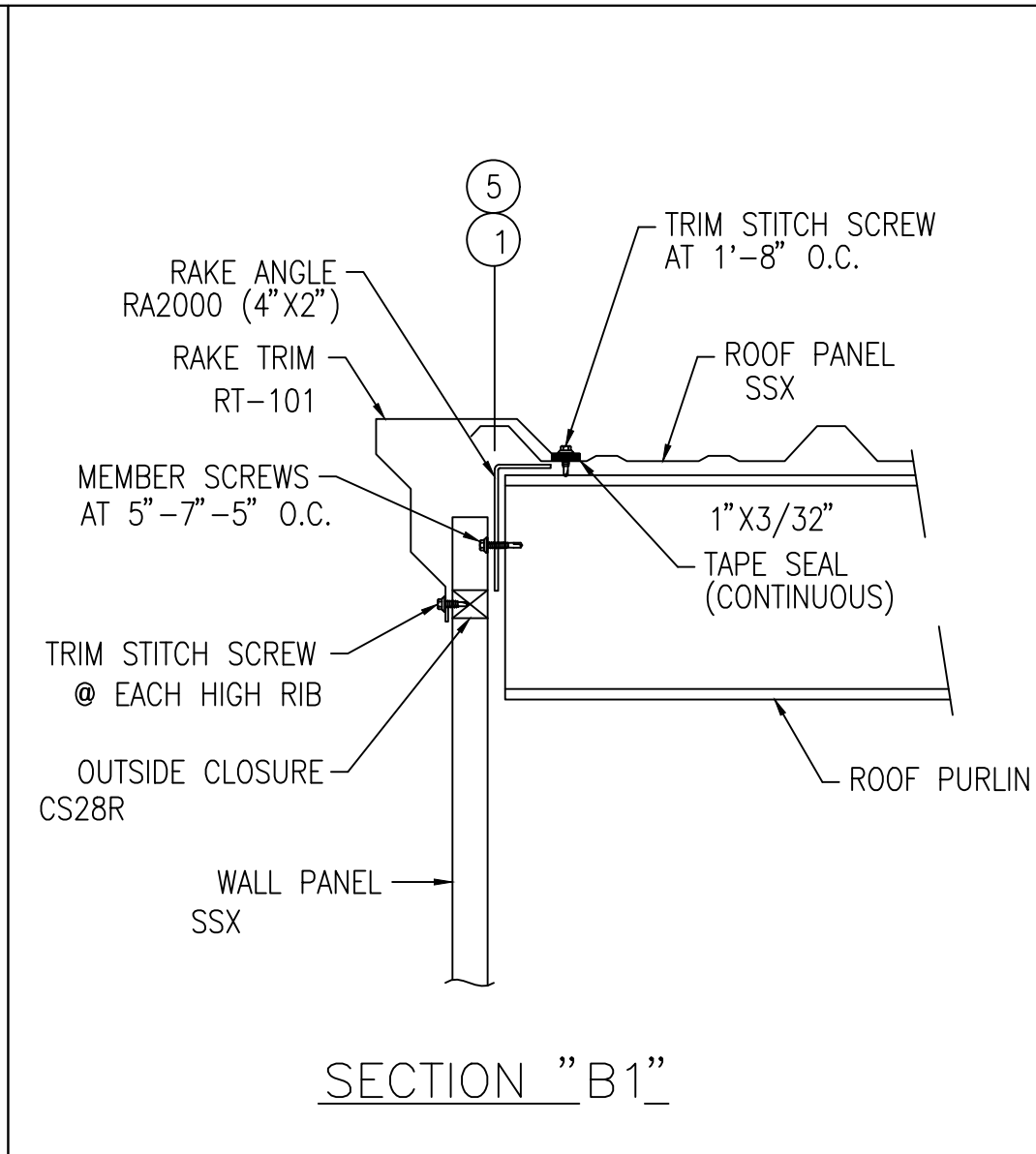
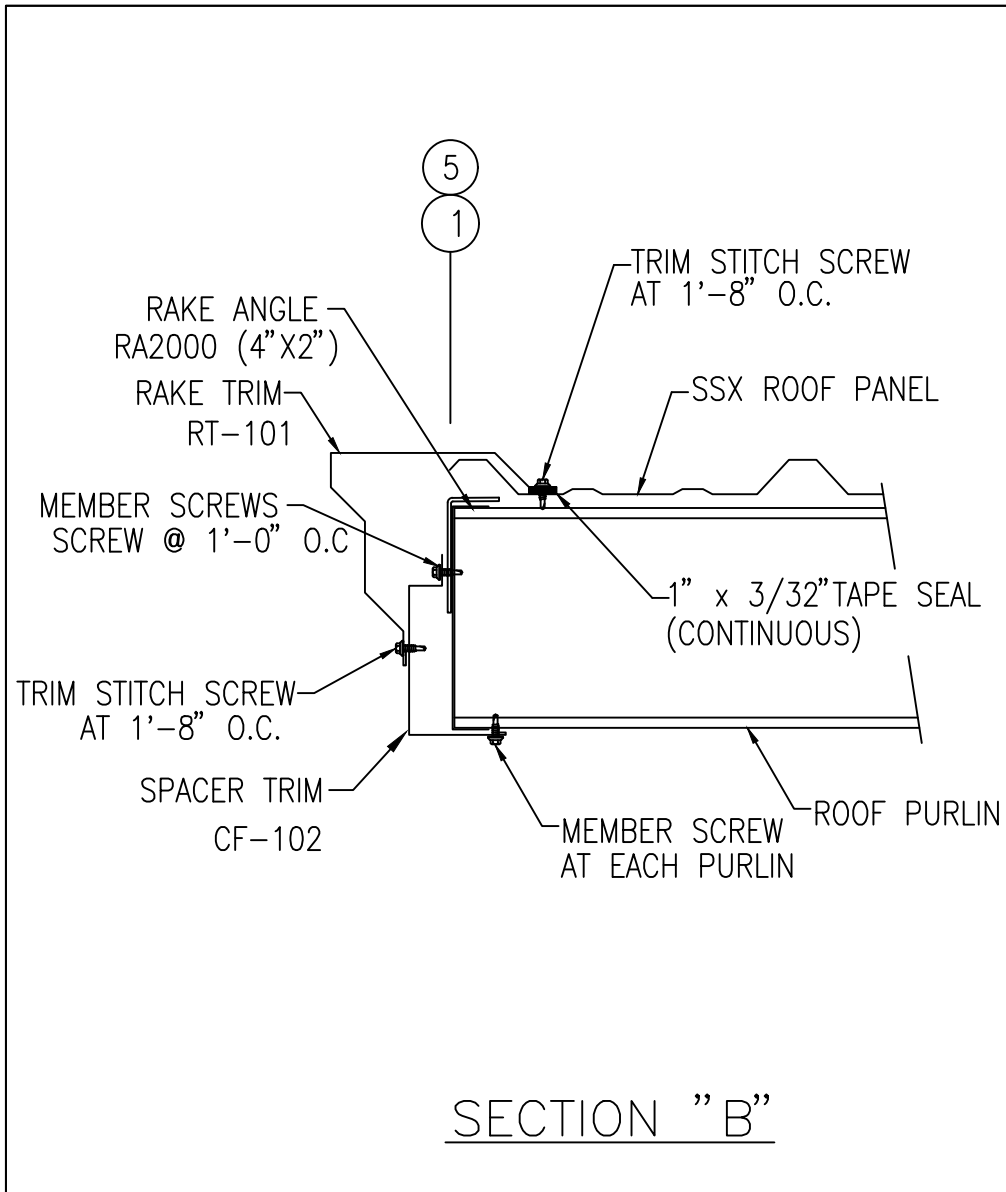
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Final drawings for construction.



| ISSUE | DATE | DESCRIPTION | BY | CHK | SHEET DESCRIPTION: | BLDG SIZE: |
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| P1 | 08.23.23 | FOR CONSTRUCTION PERMIT | PND | PNC | SIDEWALL FRAME & SHEETING ELEVATION | VARIES |
| 0 | 08.29.23 | FOR ERECTOR INSTALLATION | PND | PNC | CUSTOMER: | CUSTOMER LOCATION: |
| P2 | 12.20.24 | REV FOR CONSTRUCTION PERMIT | PND | PNC | MARCO BUILDER INC. | FORT WHITE, FL 32038 |
| | | | | | PROJECT REFERENCE: | |
| | | | | | MARCO BUILDER INC. | |
| | | | | | JOB SITE LOCATION: | JOB SITE COUNTY: |
| | | | | | FORT WHITE, FL 32038 | COLUMBIA |
| | | | | | DWN: | ENG: |
| | | | | | PND | MAH |
| | | | | | CHK: | JOB NO: |
| | | | | | PNC | 11463-32450 |
| | | | | | DATE: | DWG NO: |
| | | | | | 08.29.23 | E7 |
| | | | | | ISSUE: | P2 |

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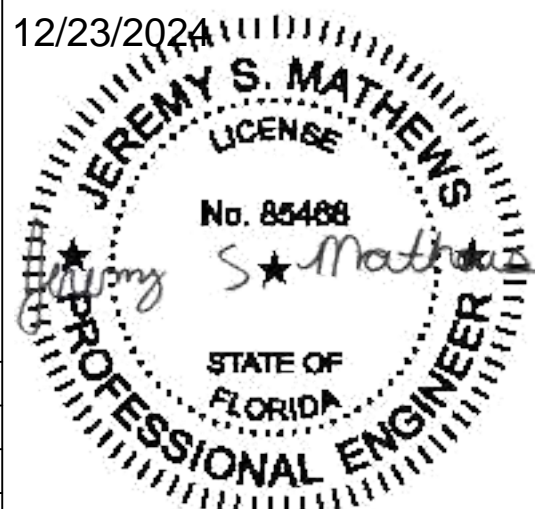
☒ FOR ERECTOR INSTALLATION:
Final drawings for construction.

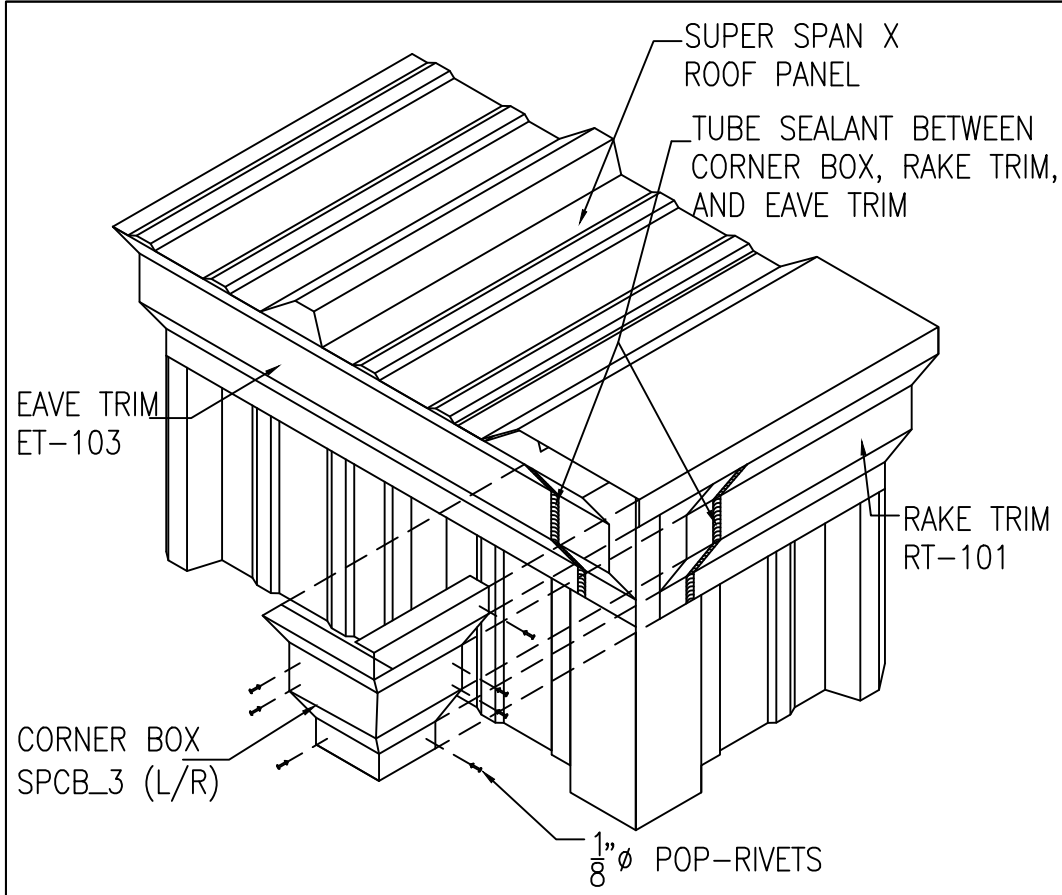


| ISSUE | DATE | DESCRIPTION | BY | CHK |
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| P1 | 08.23.23 | FOR CONSTRUCTION PERMIT | PND | PNC |
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| P2 | 12.20.24 | REV FOR CONSTRUCTION PERMIT | PND | PNC |

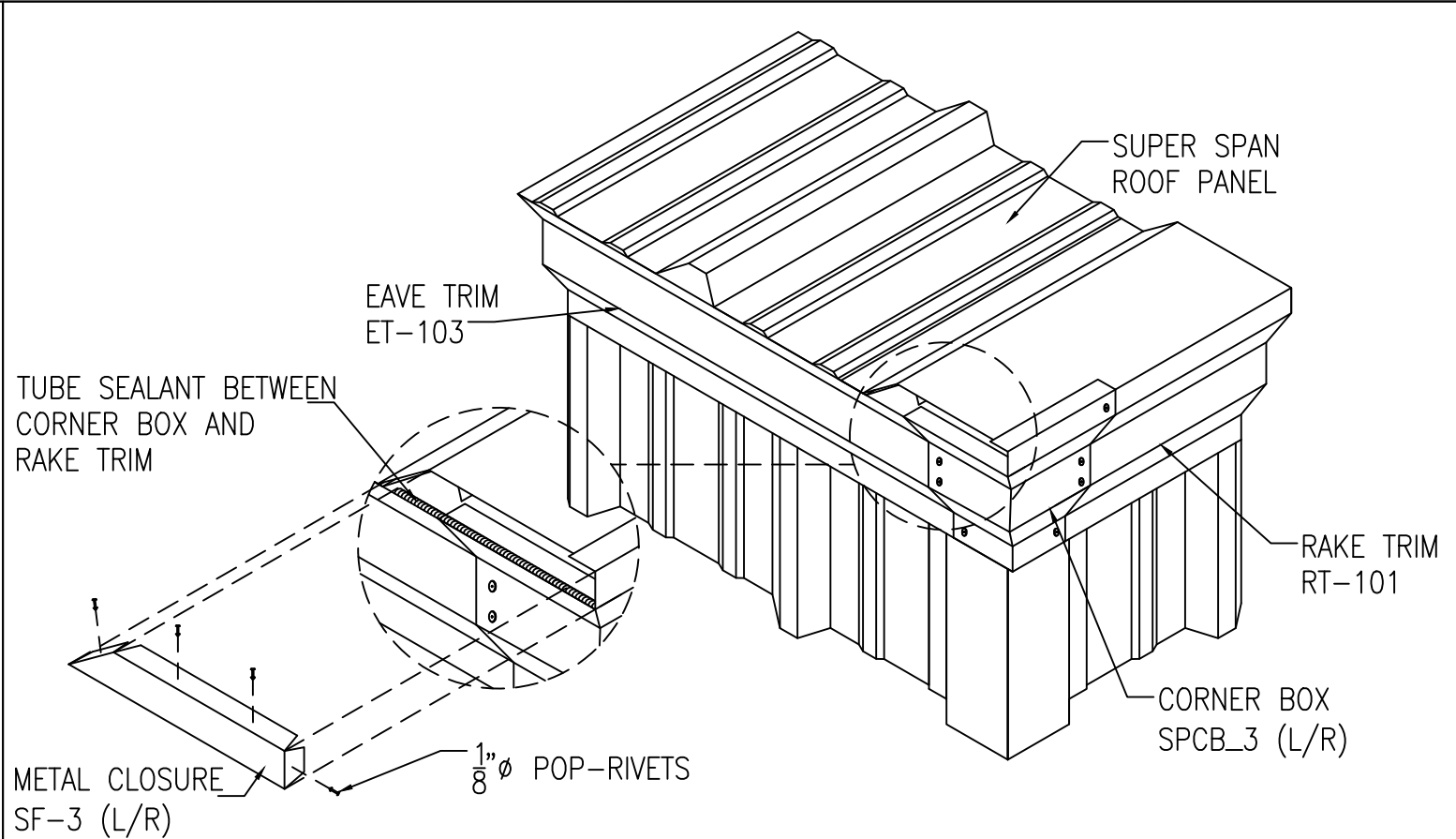
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| | | | |
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| SHEET DESCRIPTION: | | BLDG SIZE: | |
| BUILDING SECTIONS | | VARIES | |
| CUSTOMER: | | CUSTOMER LOCATION: | |
| MARCO BUILDER INC. | | FORT WHITE, FL 32038 | |
| PROJECT REFERENCE: | | MARCO BUILDER INC. | |
| JOBSITE LOCATION: | | JOBSITE COUNTY: | |
| FORT WHITE, FL 32038 | | COLUMBIA | |
| DWN: | CHK: | DATE: | ENG: |
| PND | PNC | 08.29.23 | MAH |
| JOB NO: | DWG NO: | ISSUE: | |
| 11463-32450 | E8 | P2 | |

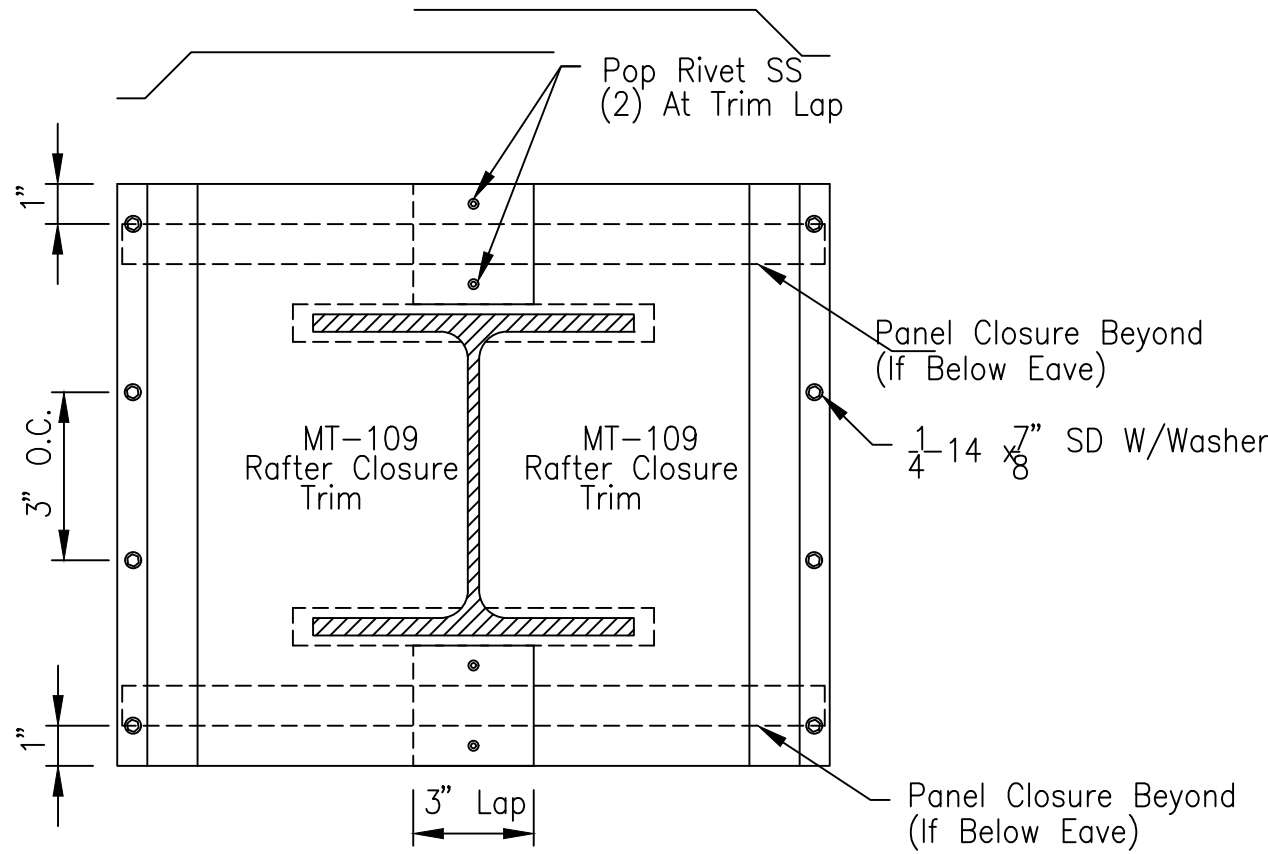




EAVE TRIM CORNER BOX TRIM INSTALLATION



EAVE TRIM CORNER BOX TRIM INSTALLATION

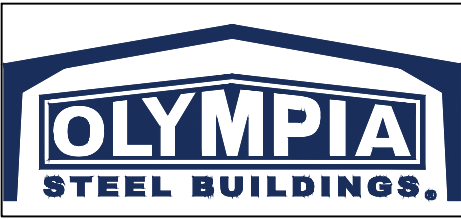


DETAIL-P

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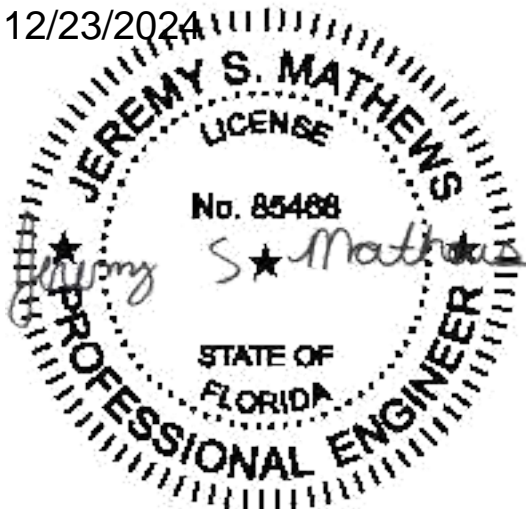
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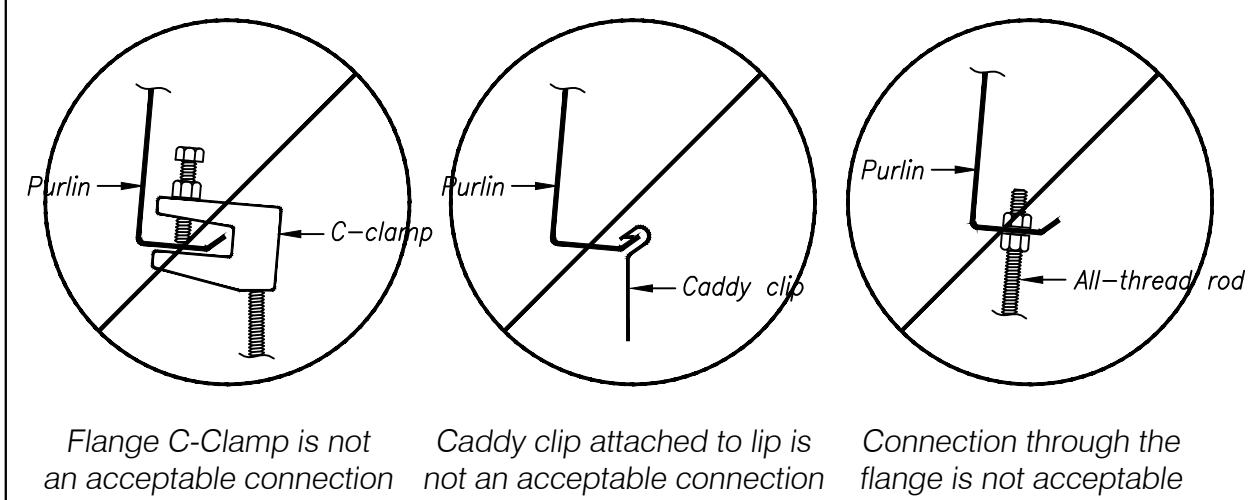
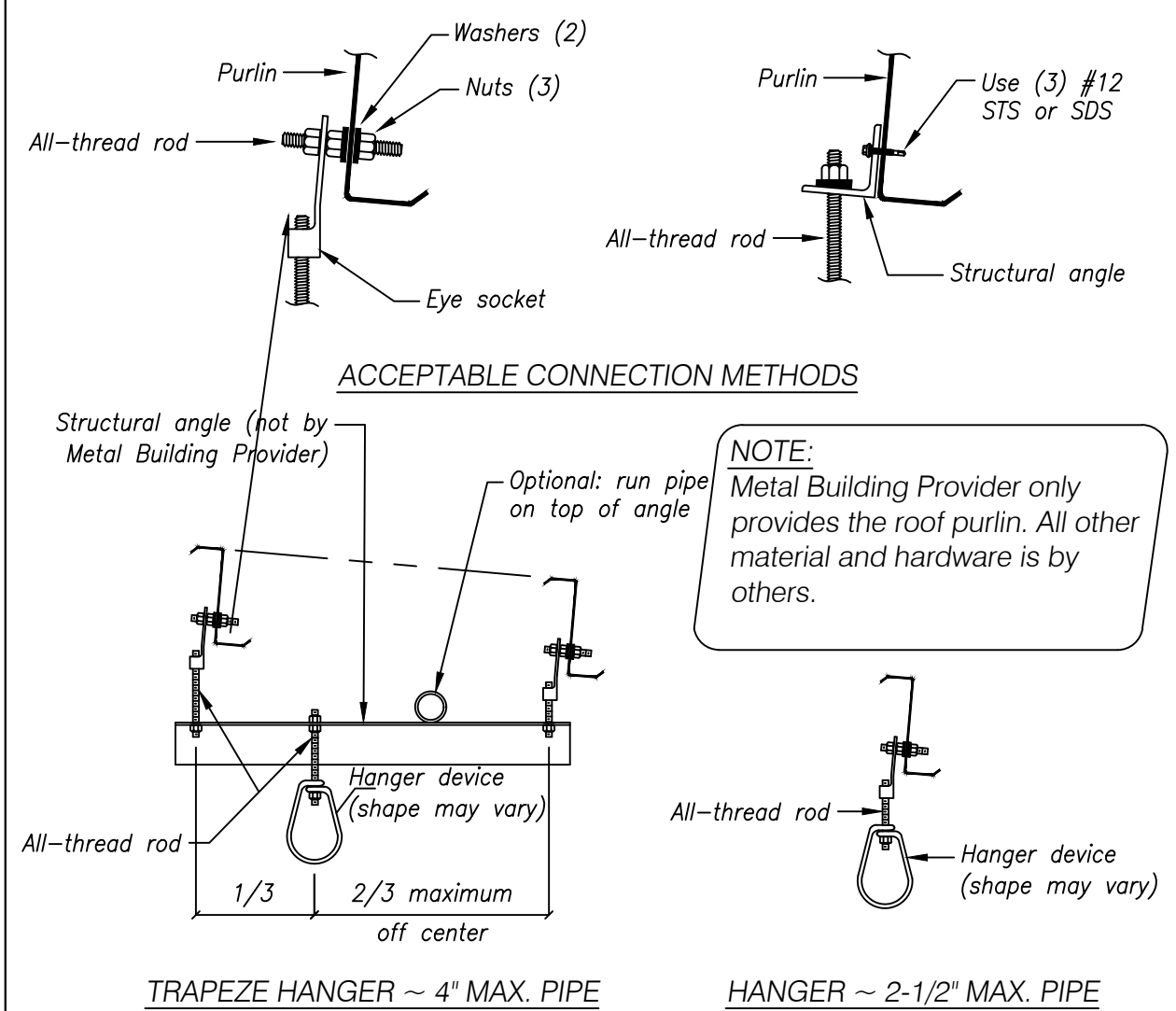
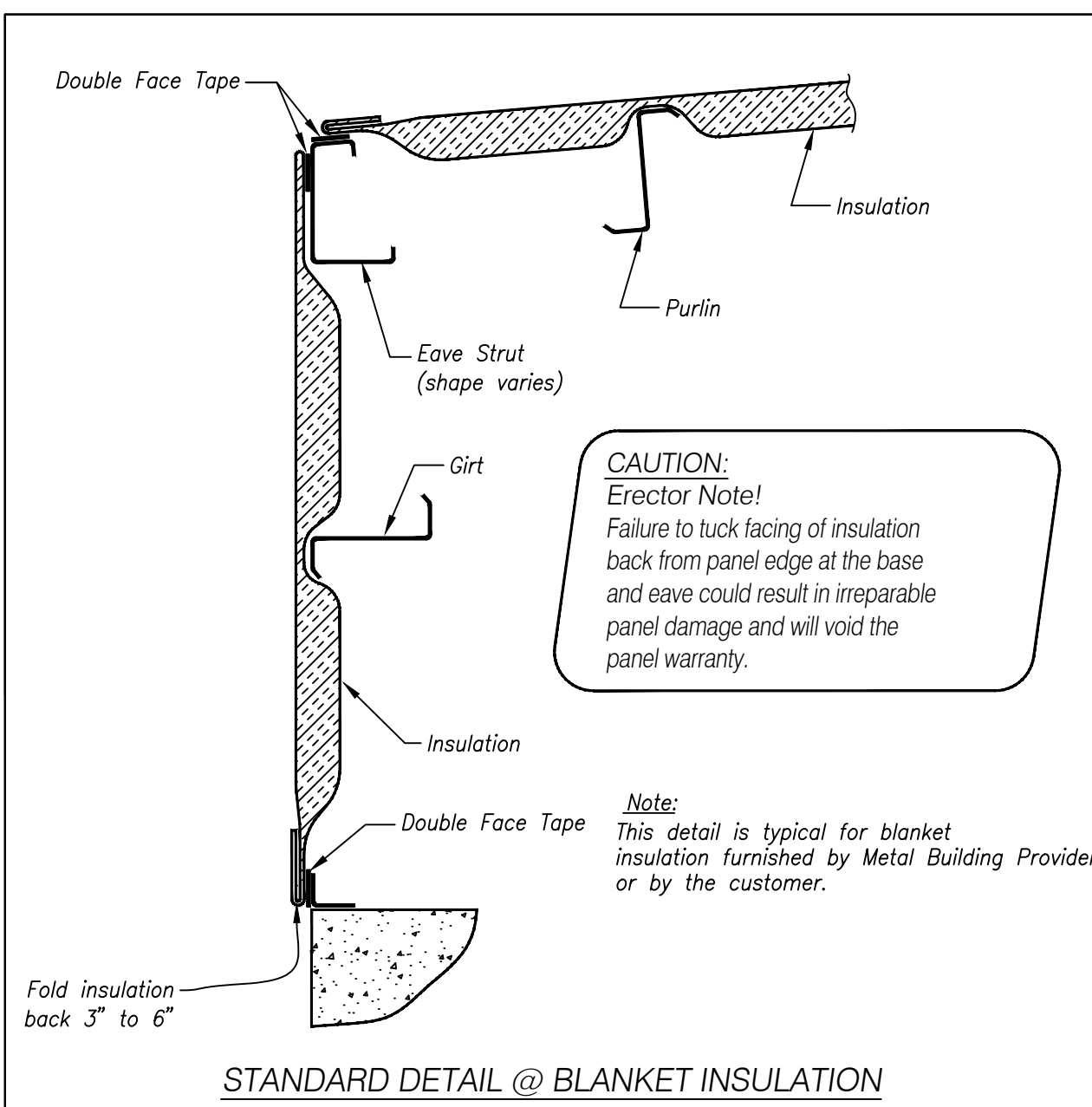
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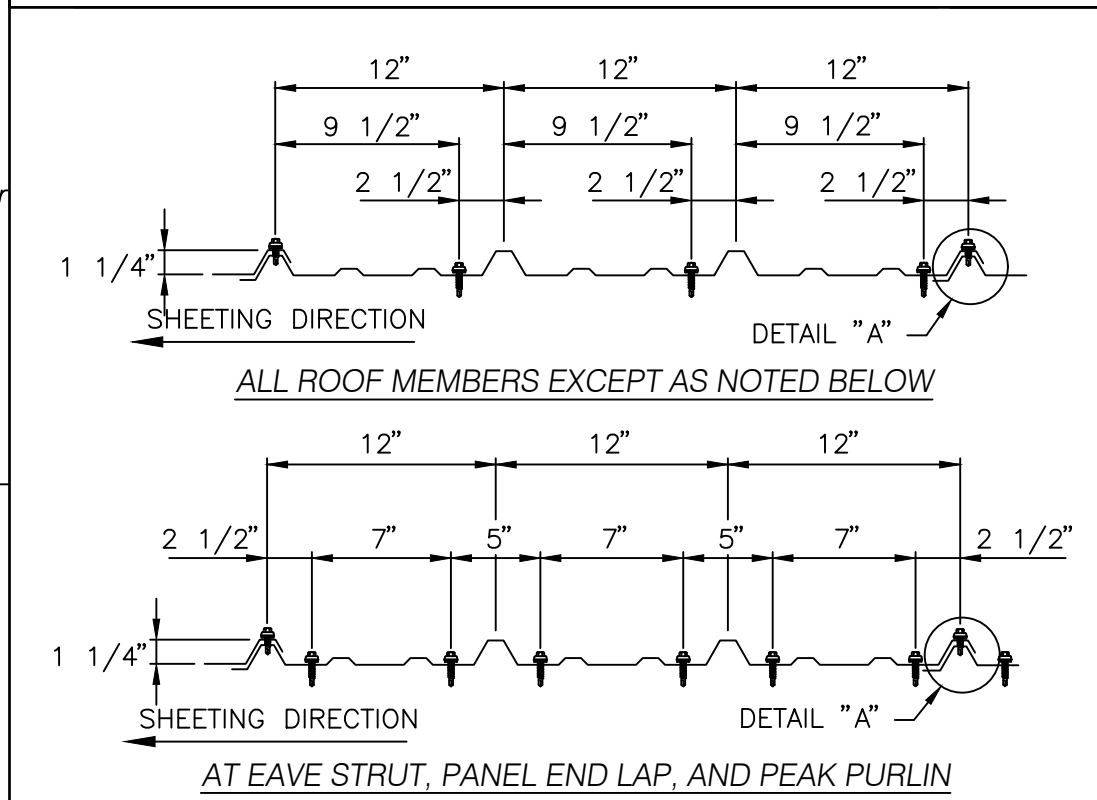
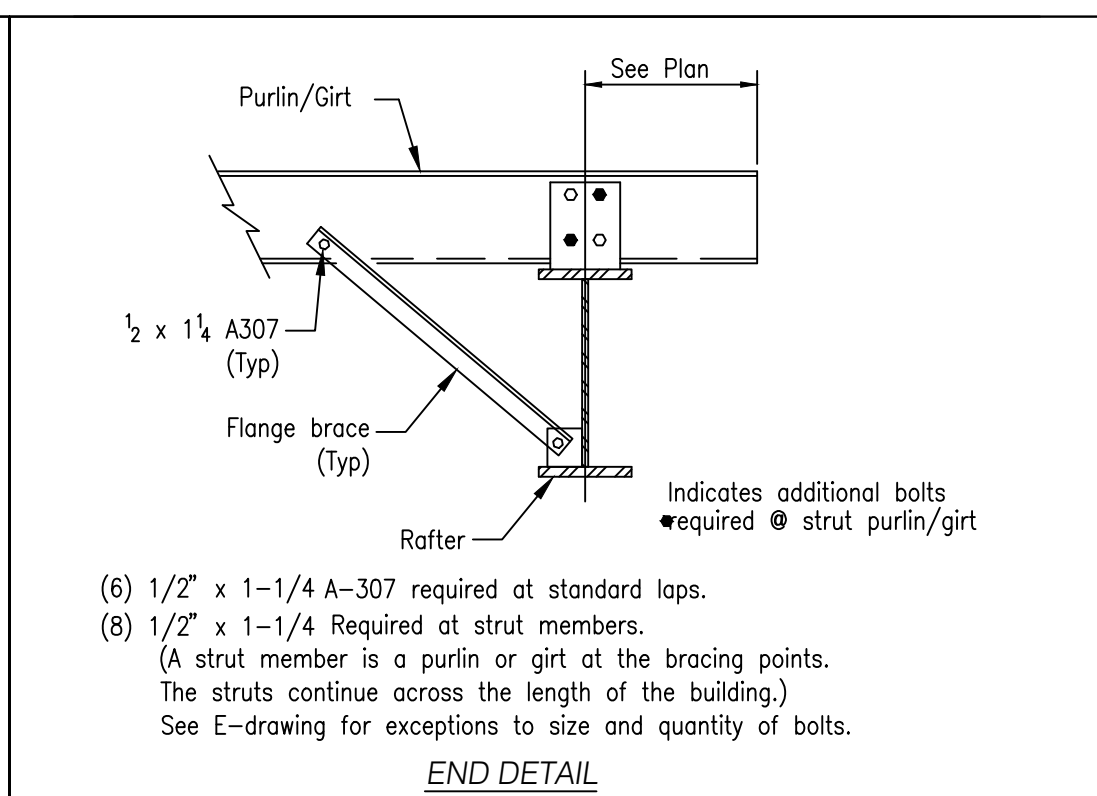
| ISSUE | DATE | DESCRIPTION | BY | CHK | SHEET DESCRIPTION: | BLDG SIZE: |
|-------|----------|-----------------------------|-----|-----|---|---|
| P1 | 08.23.23 | FOR CONSTRUCTION PERMIT | PND | PNC | BUILDING SECTIONS | VARIES |
| 0 | 08.29.23 | FOR ERECTOR INSTALLATION | PND | PNC | CUSTOMER: MARCO BUILDER INC. | CUSTOMER LOCATION: FORT WHITE, FL 32038 |
| P2 | 12.20.24 | REV FOR CONSTRUCTION PERMIT | PND | PNC | PROJECT REFERENCE: MARCO BUILDER INC. | |
| | | | | | JOB SITE LOCATION: FORT WHITE, FL 32038 | JOB SITE COUNTY: COLUMBIA |
| | | | | | DWN: PND | CHK: PNC |
| | | | | | DATE: 08.29.23 | ENG: MAH |
| | | | | | JOB NO: 11463-32450 | DWG NO: E9 |
| | | | | | | ISSUE: P2 |

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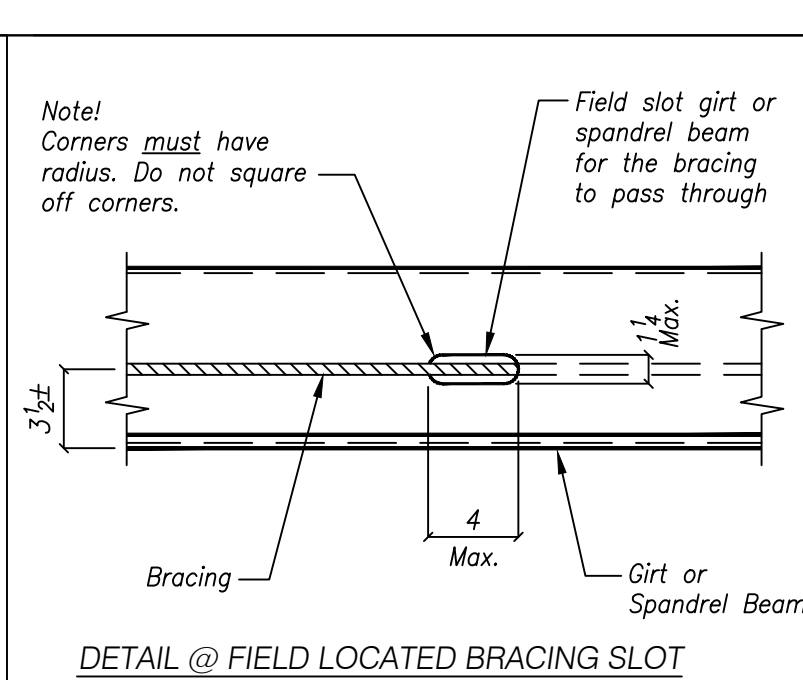
ACCEPTABLE CONNECTIONS FOR ALL COLLATERAL LOADS FOR HANGER ATTACHMENT



FASTENER LOCATION FOR "SUPER SPAN X" ROOF PANEL

BUILT-UP SECTION LEGEND

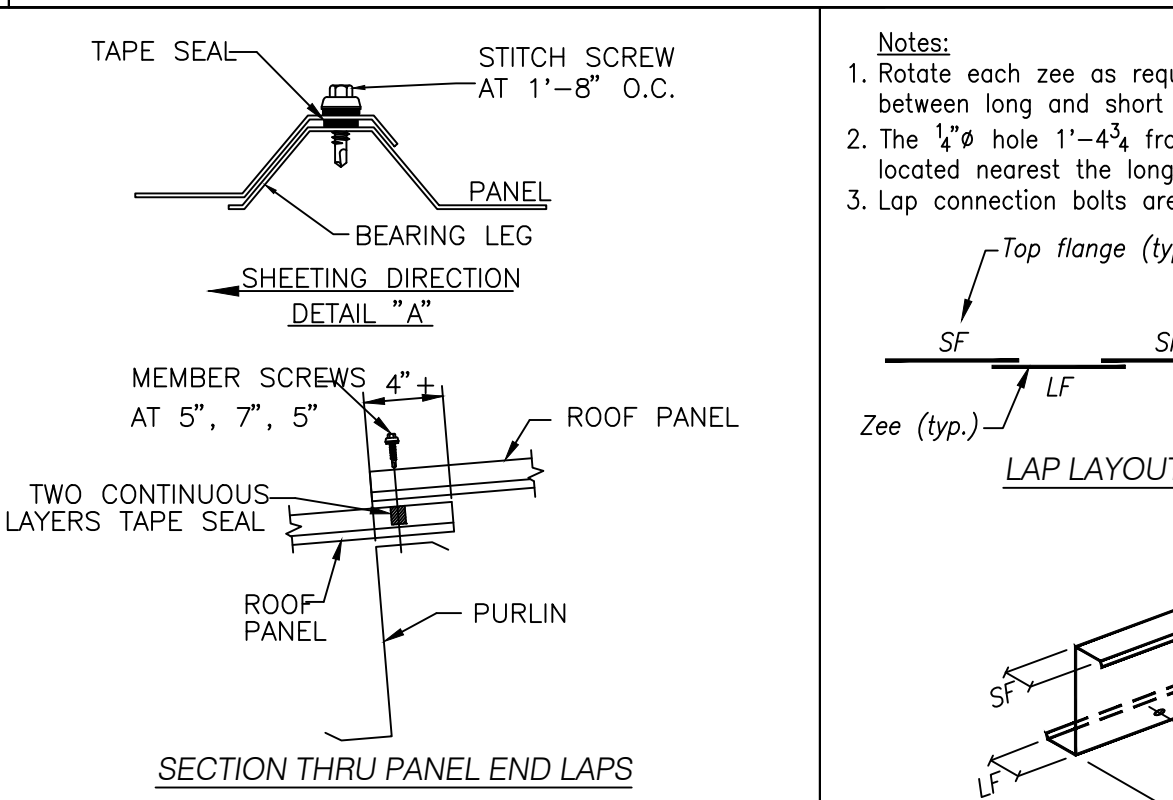
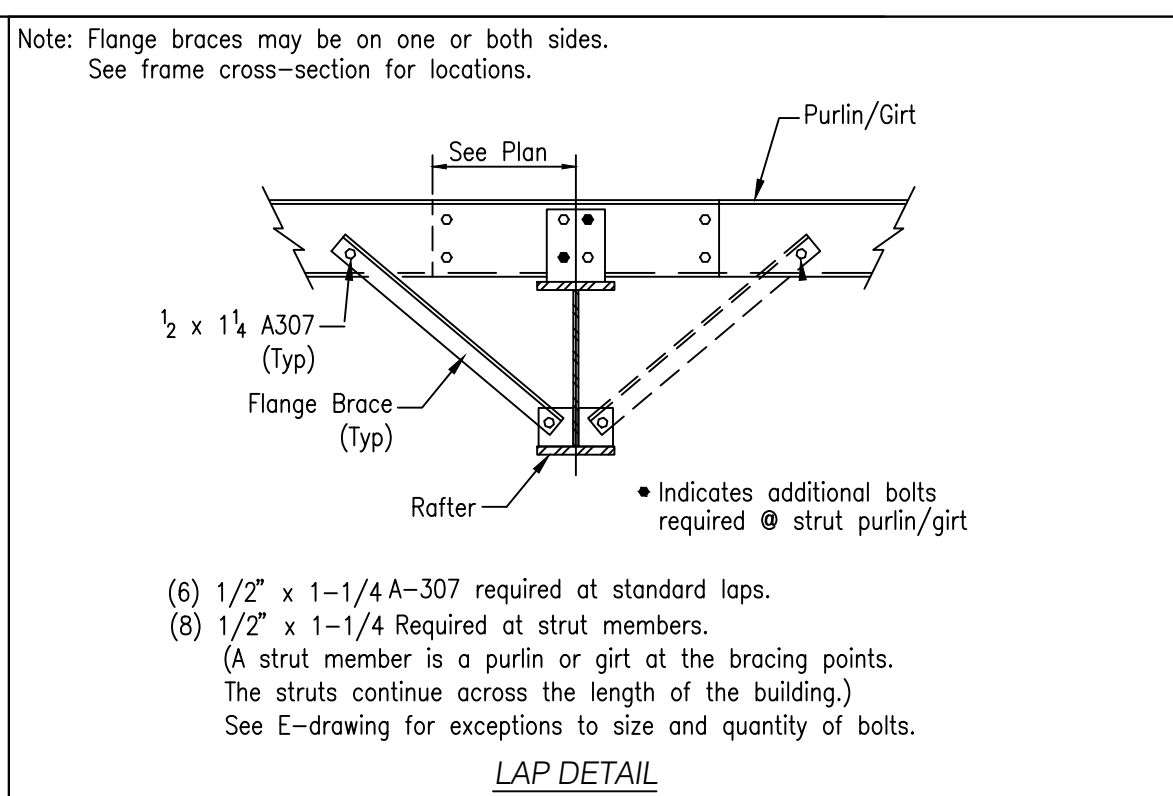
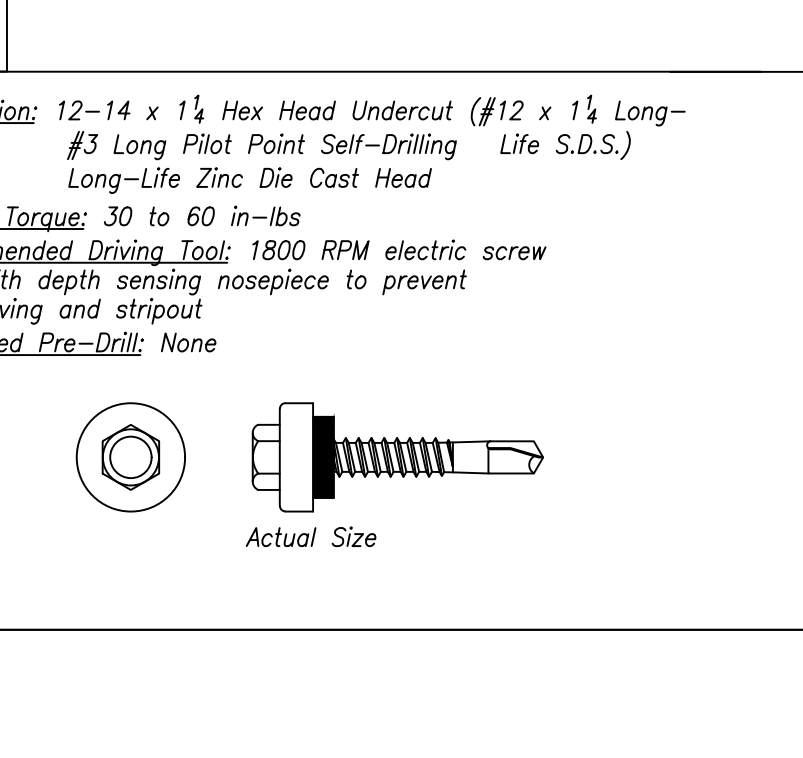
| Flange Width (in inches) | Flange Thickness (in inches) | Web Thickness (in inches) |
|--------------------------|------------------------------|---------------------------|
| 5 = 5 | 3 = 3/8 | 8 = 1/2 |
| 6 = 6 | 4 = 1/4 | 0 = 5/8 |
| 8 = 8 | 5 = 5/8 | 2 = 3/4 |
| 0 = 10 | 6 = 3/8 | 1 = 1 |
| 2 = 12 | | |



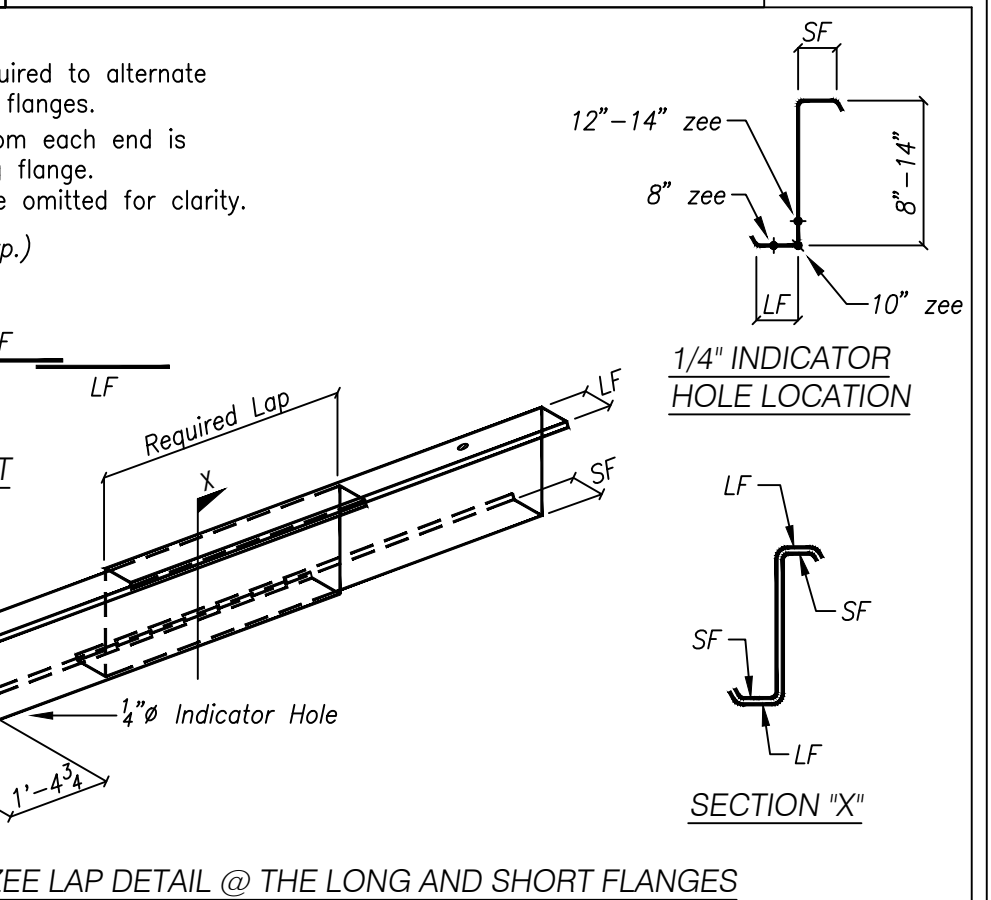
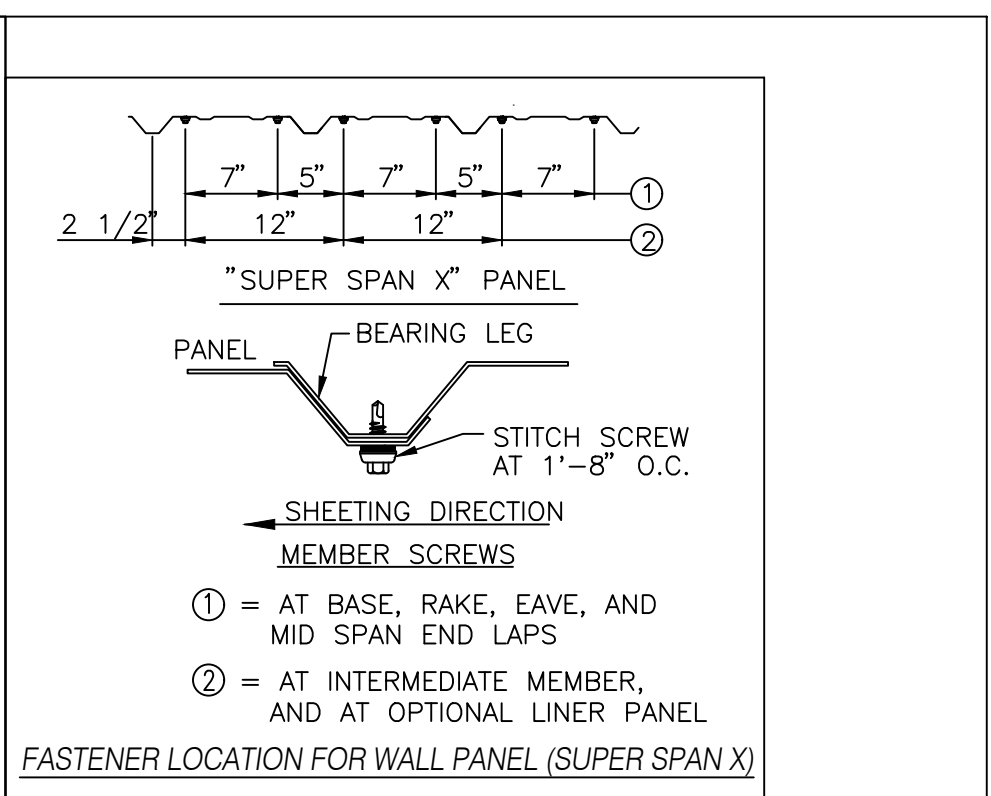
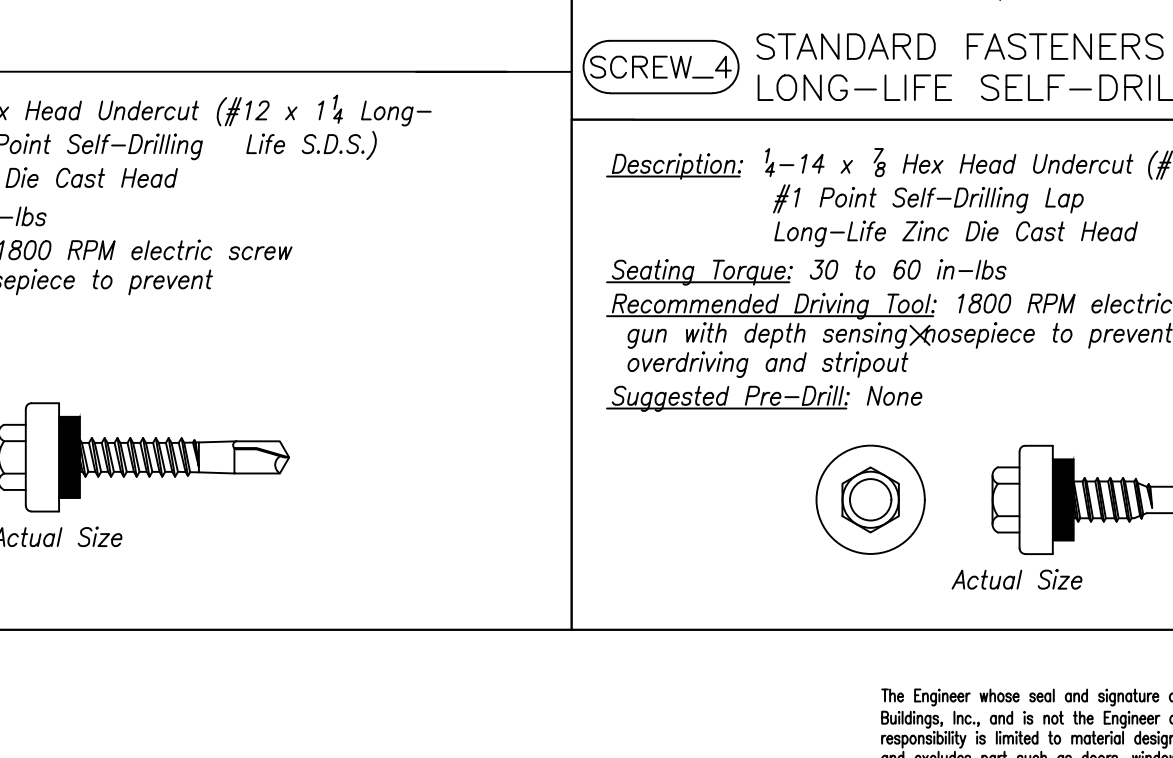
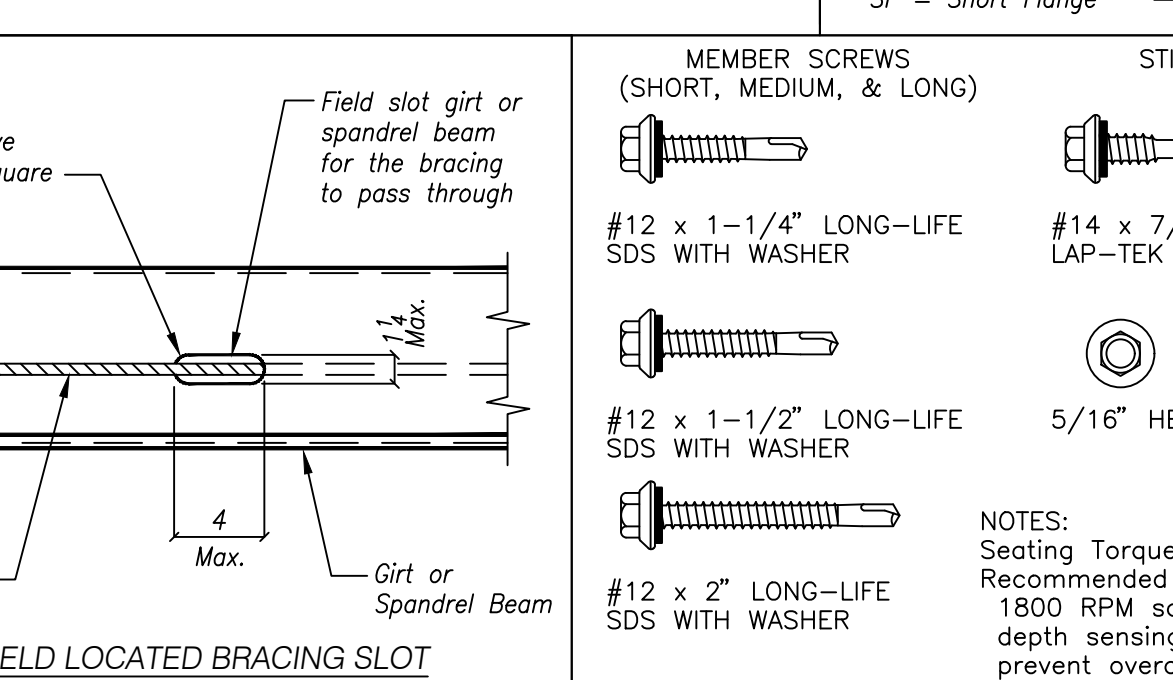
STANDARD FASTENERS MISCELLANEOUS

| Fastener Type | Dimensions |
|---|------------|
| 1/4-14 x 1-1/4" HWH TWP2 5/16" HEAD SELF-DRILLER NO SEALING WASHER - ZINC-PLATED | |
| 1/4-14 x 1-1/4" HWH SHOULDERED TOP3 5/16" HEAD SELF-DRILLER - NO SEALING WASHER - ZINC-PLATED | |
| #12 x 1" PANCAKE HEAD SDS QUADREX DRIVE, ZINC-PLATED | |

NOTES:
Seating Torque: 30 - 60 in-lbs
Recommended Driving Tool: 1800 RPM screw gun with depth sensing nosepiece to prevent overdriving and stripout



FASTENER LOCATION FOR "SUPER SPAN X" ROOF PANEL



STANDARD FASTENERS LONG-LIFE SELF-DRILLING

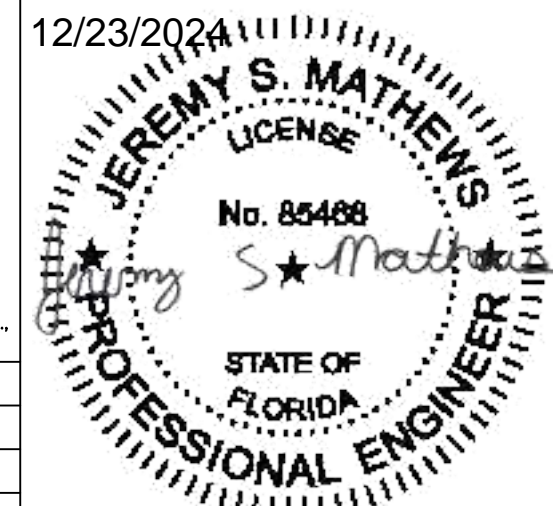
SCREW_4

Description: 1/4-14 x 1 1/4 Hex Head Undercut (#12 x 1 1/4 Long-Life Pilot Point Self-Drilling Life S.D.S.) Long-Life Zinc Die Cast Head

Seating Torque: 30 to 60 in-lbs
Recommended Driving Tool: 1800 RPM electric screw gun with depth sensing nosepiece to prevent overdriving and stripout
Suggested Pre-Drill: None

Description: 1/4-14 x 7/8 Hex Head Undercut (#14 x 7/8 Long-Life Point Self-Drilling Lap Lap-Tek S.D.S.) Long-Life Zinc Die Cast Head

Seating Torque: 30 to 60 in-lbs
Recommended Driving Tool: 1800 RPM electric screw gun with depth sensing nosepiece to prevent overdriving and stripout
Suggested Pre-Drill: None



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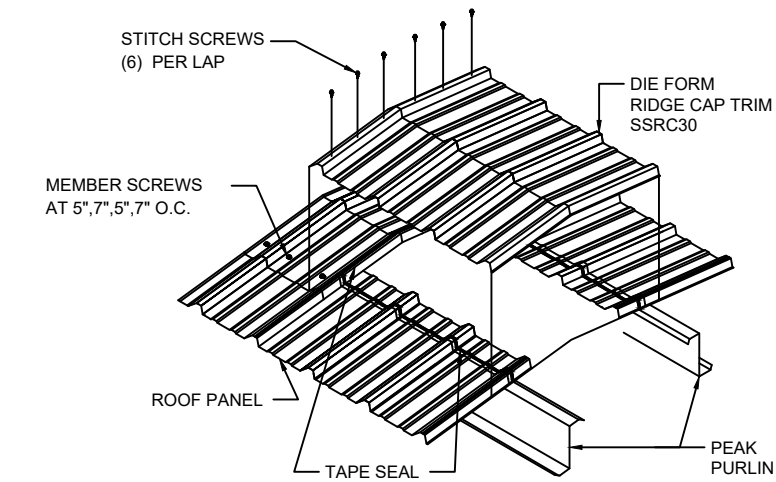
☒ **FOR ERECTOR INSTALLATION:**
Final drawings for construction.



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| P1 | 08.23.23 | FOR CONSTRUCTION PERMIT | PND | PNC |
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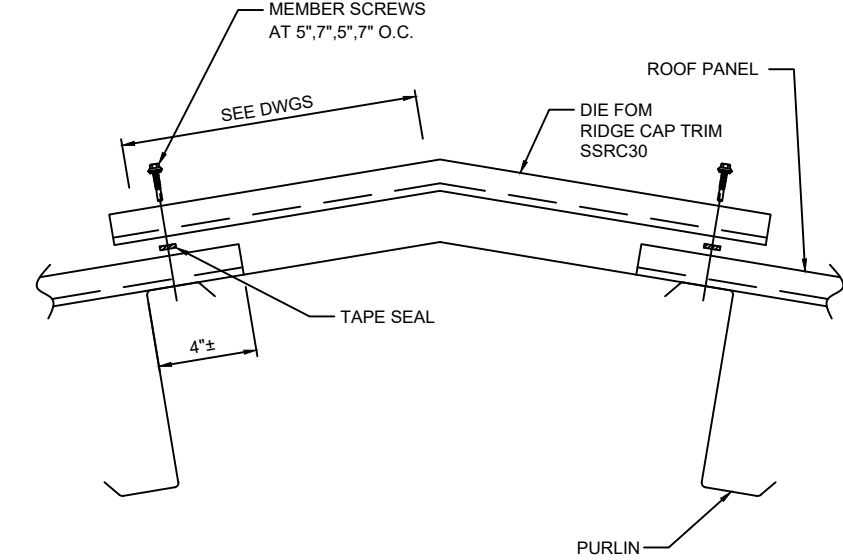
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| PROJECT REFERENCE: MARCO BUILDER INC. | | JOB SITE LOCATION: FORT WHITE, FL 32038 | |
| JOB SITE COUNTY: COLUMBIA | | JOB NO: 11463-32450 | |
| DWN: PND | CHK: PNC | DATE: 08.29.23 | ENG: MAH |
| DWG NO: D1 | | ISSUE: P2 | |

INSULATION NOT BY
METAL BUILDING PROVIDER

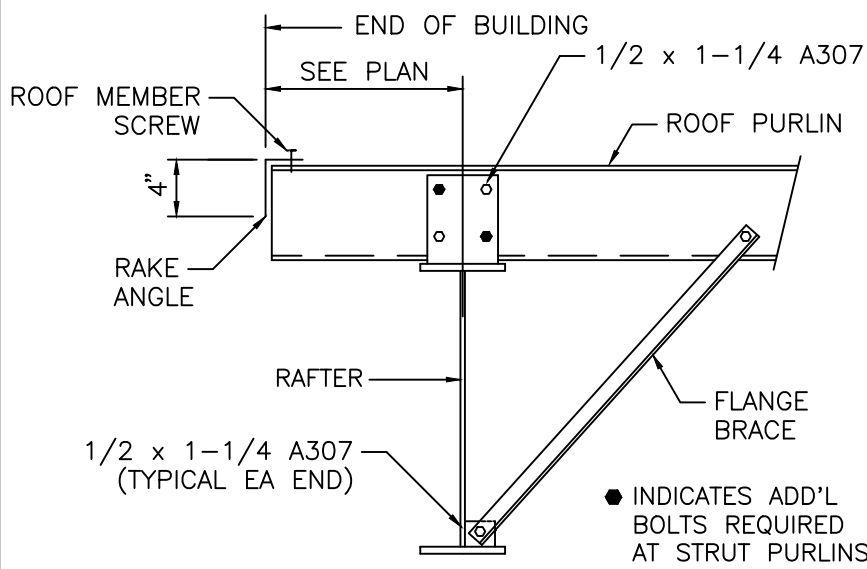


TRIM_54 DIE FORMED RIDGE CAP
INSTALLATION (SUPER SPAN X)

INSULATION NOT BY
METAL BUILDING PROVIDER

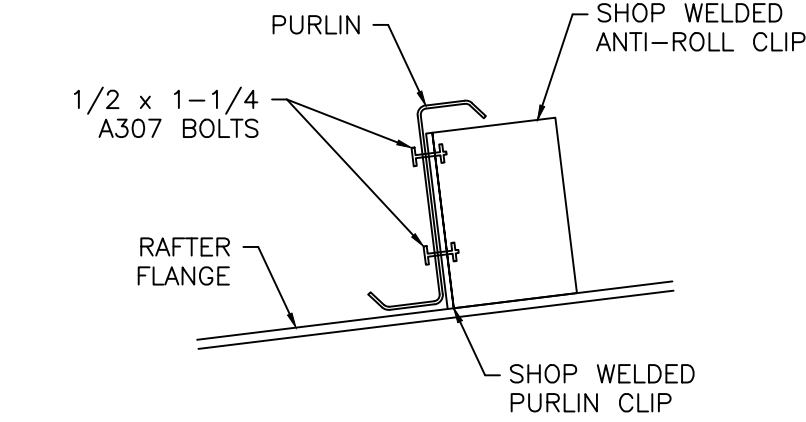


TRIM_55 DIE FORM RIDGE CAP
INSTALLATION (SUPER SPAN X)



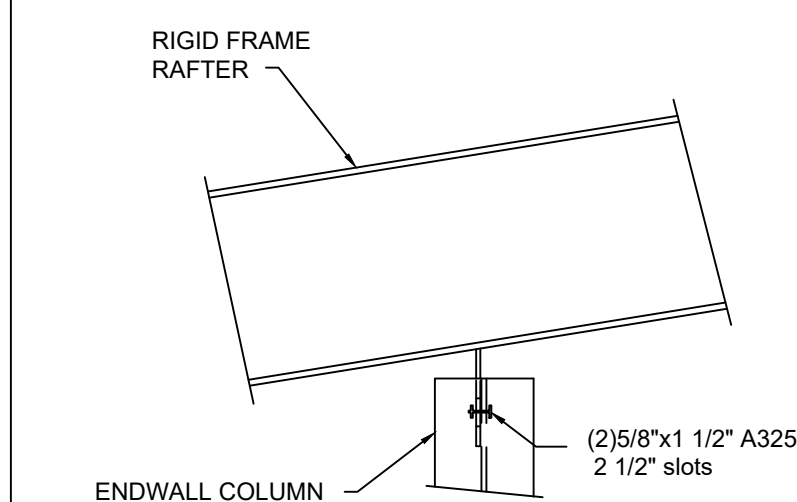
(2) 1/2 x 1-1/4 A307 REQUIRED AT STANDARD LAPS
(4) 1/2 x 1-1/4 A307 REQUIRED AT STRUT MEMBERS
A STRUT PURLIN IS A PURLIN LOCATED AT THE BRACE POINTS.
SEE PLANS FOR EXCEPTION TO SIZE & QTY OF BOLTS.

A10 ROOF PURLIN CONNECTION
AT MAIN FRAME ENDWALL

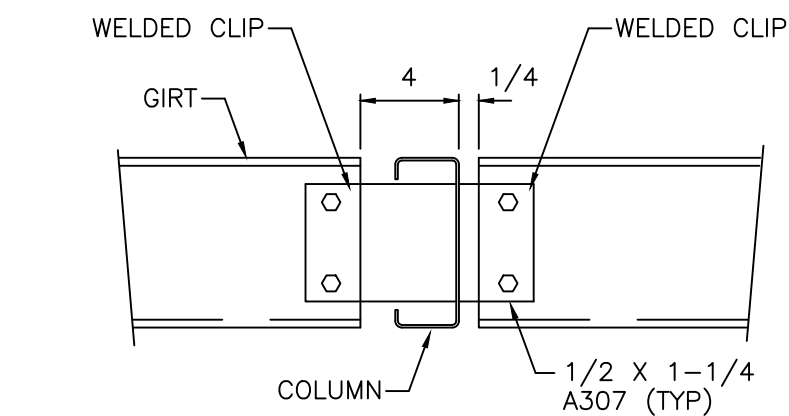


SOME PURLIN CLIPS WILL HAVE AN ADDITIONAL ANTI-ROLL
CLIP ATTACHED. THE QUANTITY AND SPACING OF THESE CLIPS
IS DETERMINED BY THE DESIGN FOR EACH SPECIFIC BUILDING.

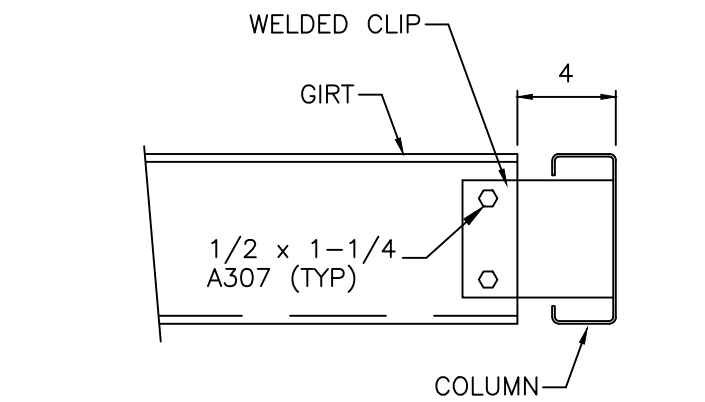
ANTI DETAIL AT ANTI-ROLL CLIP



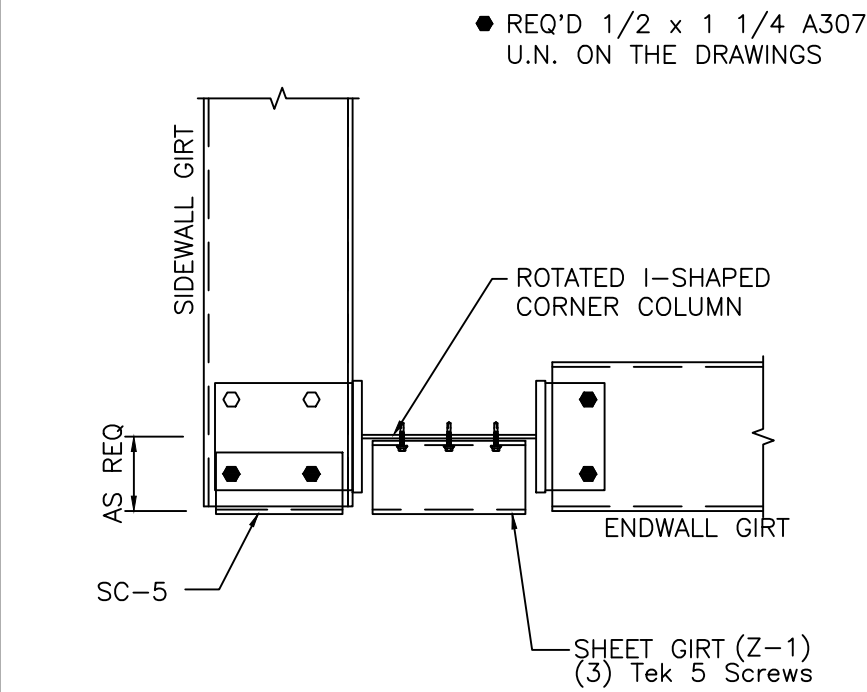
B19 COLUMN TO RIGID FRAME RAFTER



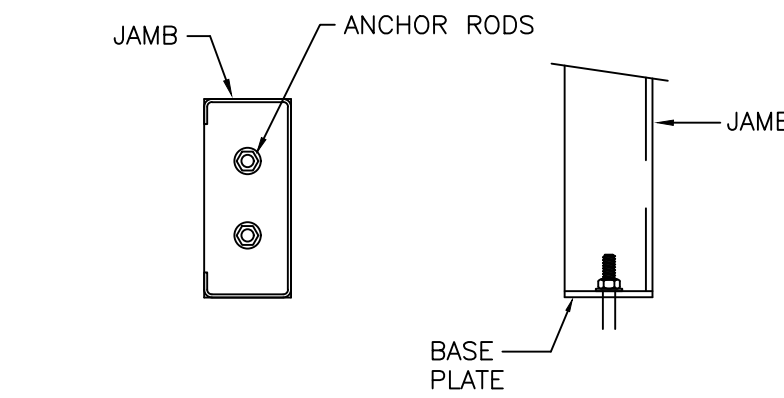
C4 GIRT TO COLUMN



C13 GIRT/HEADER TO CEE COLUMN

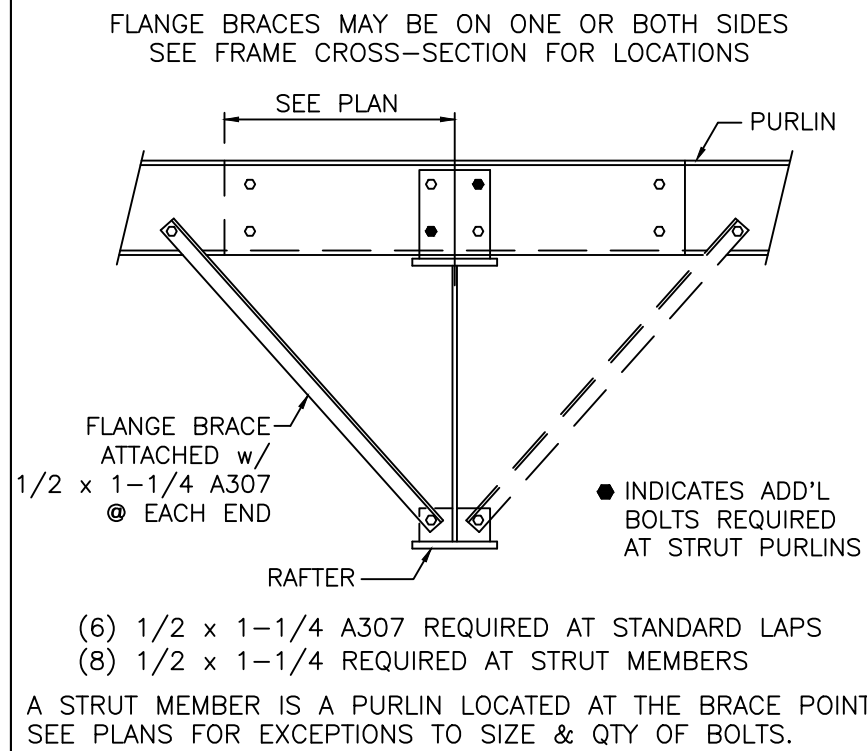


D15 CORNER COLUMN TO WALL GIRT

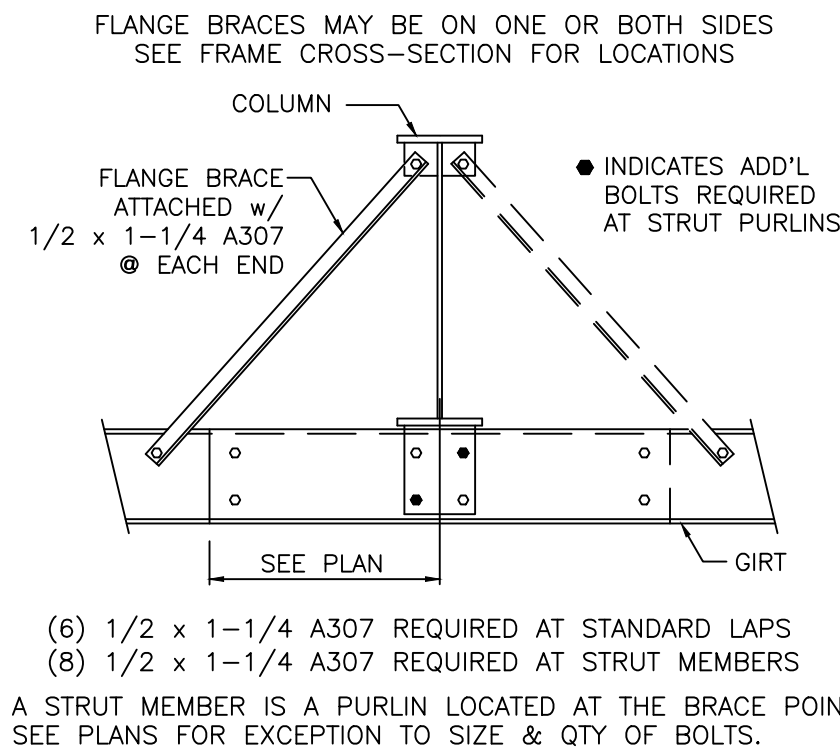


- ALL ANCHOR RODS (BY OTHERS) TO HAVE NUTS
AND FLAT WASHERS.

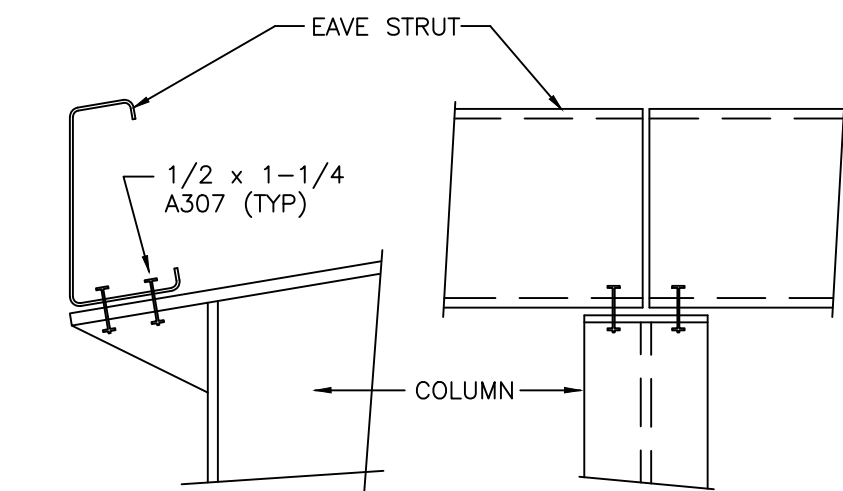
E5 BASE PLATE FOR DOOR JAMB



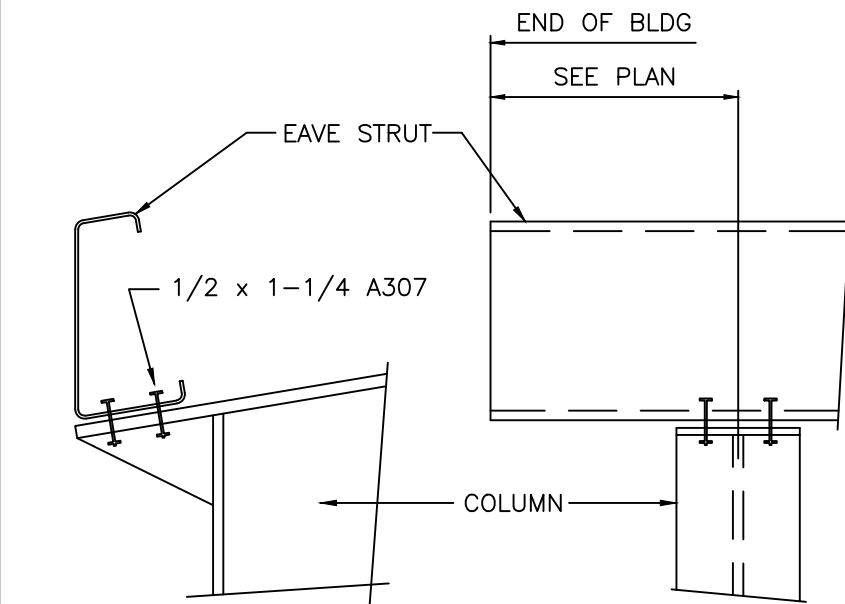
G2 ROOF PURLIN TO INTERIOR FRAME
RAFTER



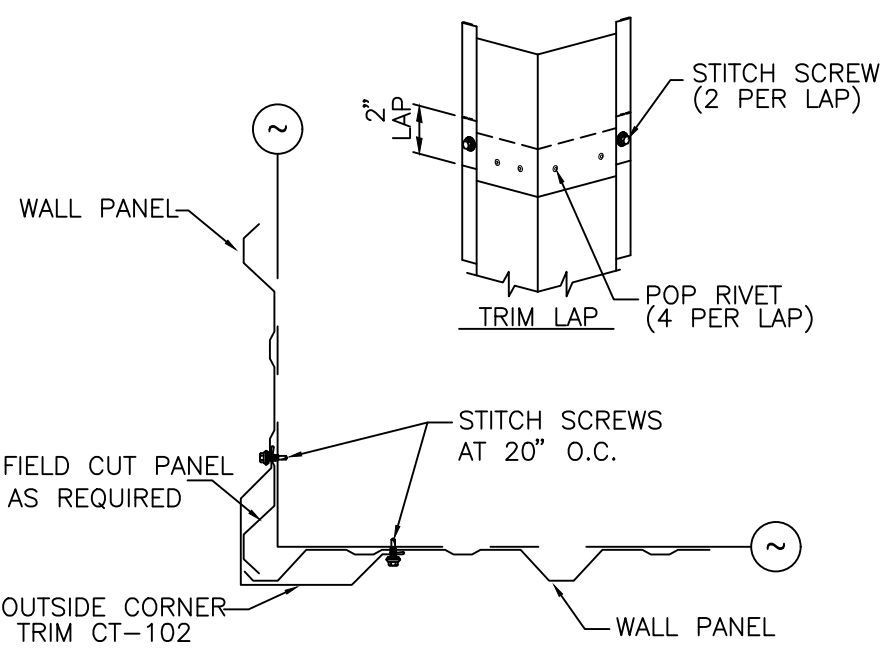
H2 WALL GIRT TO FRAME COLUMN



J2 EAVE STRUT TO RIGID FRAME

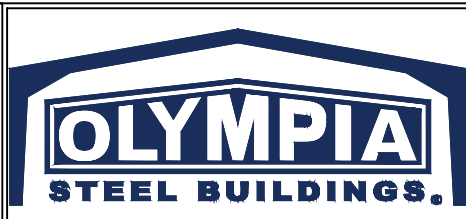


J24 EAVE STRUT TO RIGID FRAME



C1 CORNER TRIM INSTALLATION
(SUPER SPAN X)

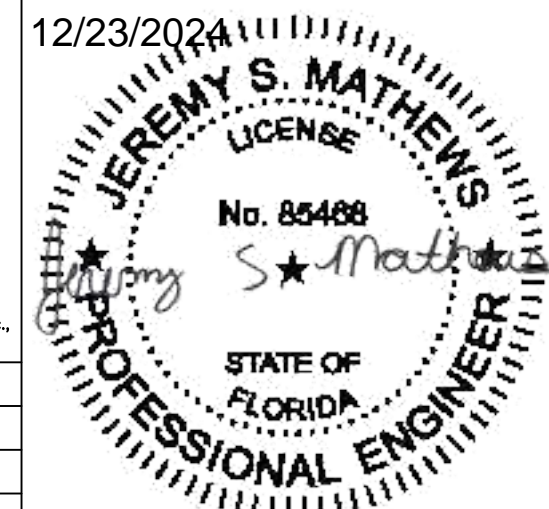
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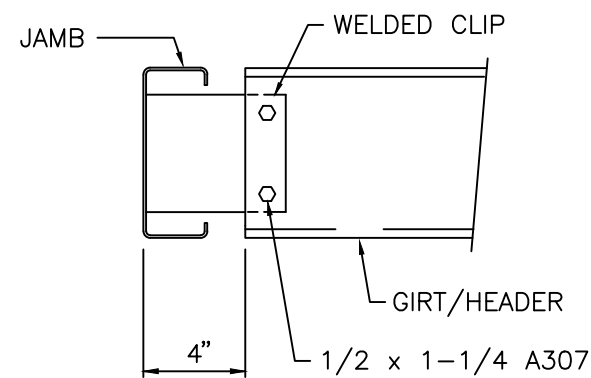


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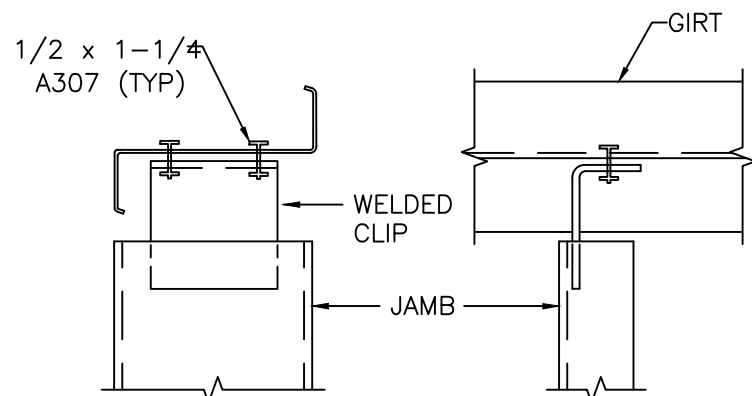
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| MARCO BUILDER INC. | | FORT WHITE, FL 32038 | |
| PROJECT REFERENCE: | | | |
| MARCO BUILDER INC. | | | |
| JOBSITE LOCATION: | | JOBSITE COUNTY: | |
| FORT WHITE, FL 32038 | | COLUMBIA | |
| DWN: | CHK: | DATE: | ENG: |
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| JOB NO: | | DWG NO: | |
| 11463-32450 | | D2 | |
| ISSUE: | | | |
| P2 | | | |

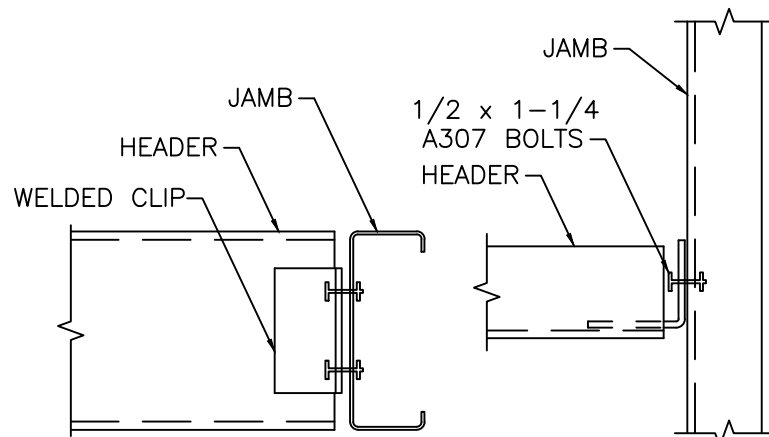




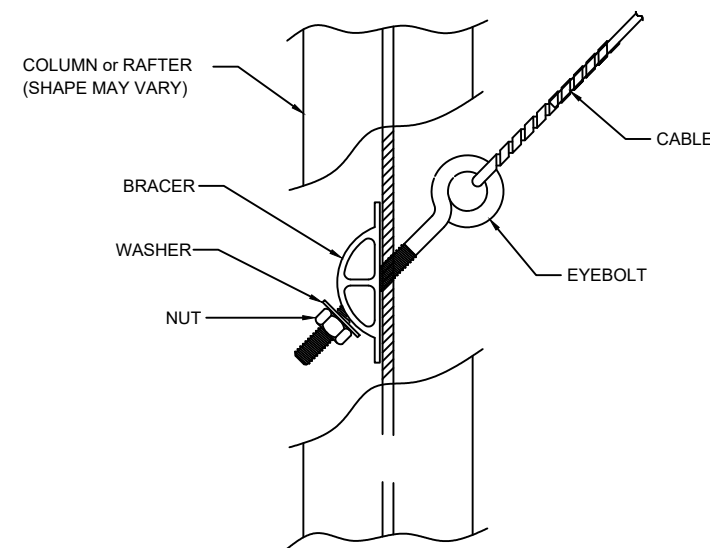
K3 WALL GIRT TO DOOR JAMB



L8 DOOR JAMB TO WALL GIRT

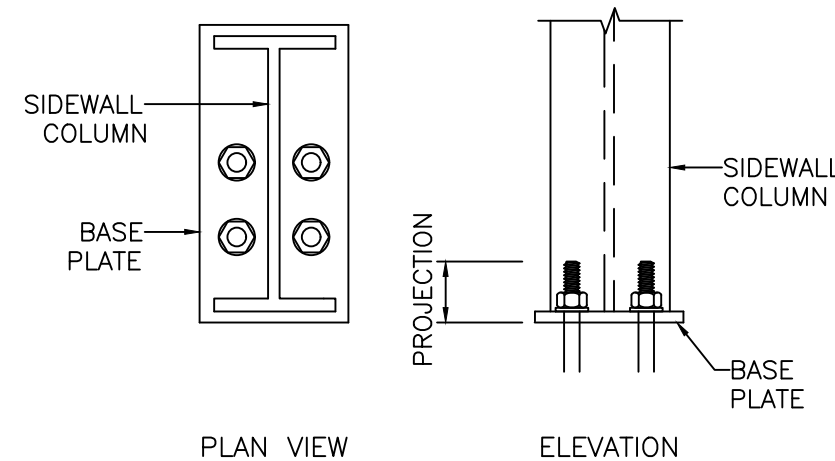


M3 HEADER TO CEE JAMB



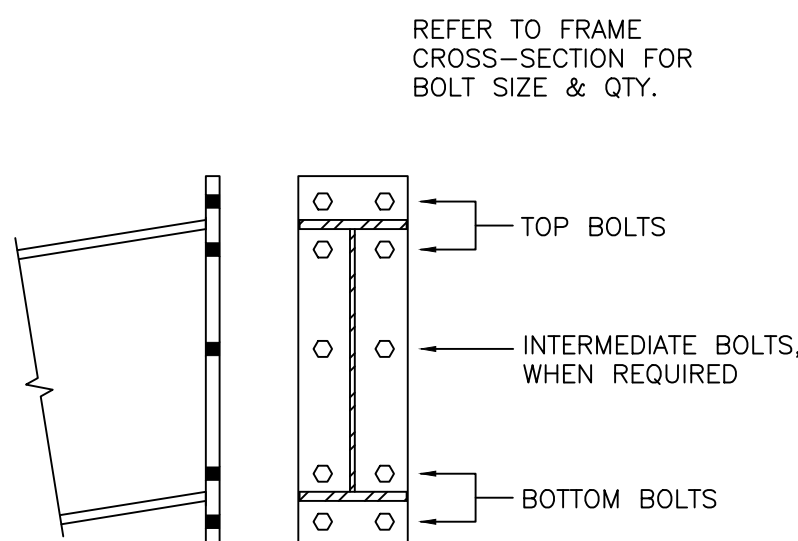
| CABLE SIZE | BRACER | WASHER | NUT |
|------------|-----------|-----------|-----------|
| 1/4" | BRACER #1 | F844 1/2" | A563 1/2" |
| 5/16" | BRACER #1 | F844 5/8" | A563 5/8" |
| 3/8" | BRACER #2 | F844 3/4" | A563 3/4" |
| 1/2" | BRACER #2 | F844 7/8" | A563 7/8" |

Q2 DIAGONAL CABLE BRACING INSTALLATION

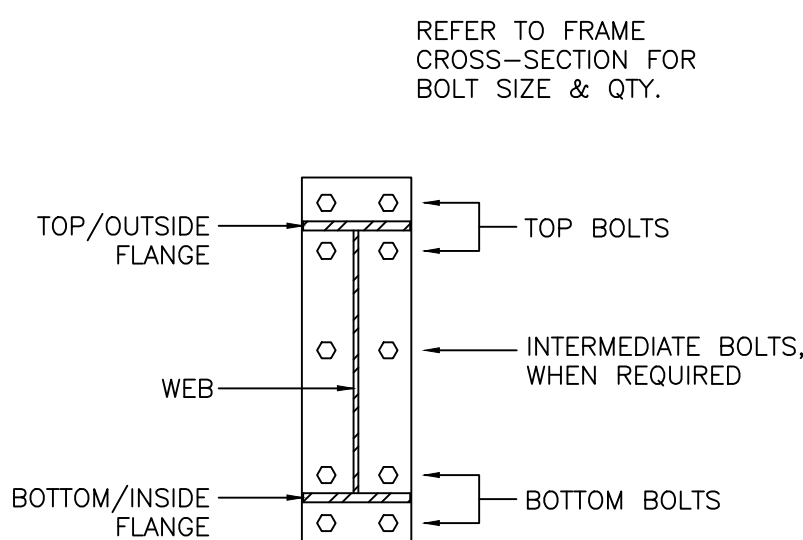


- ALL ANCHOR RODS (BY OTHERS) TO HAVE NUTS AND FLAT WASHERS.
- SEE BOLT SETTING PLAN FOR ACTUAL BOLT QTY.

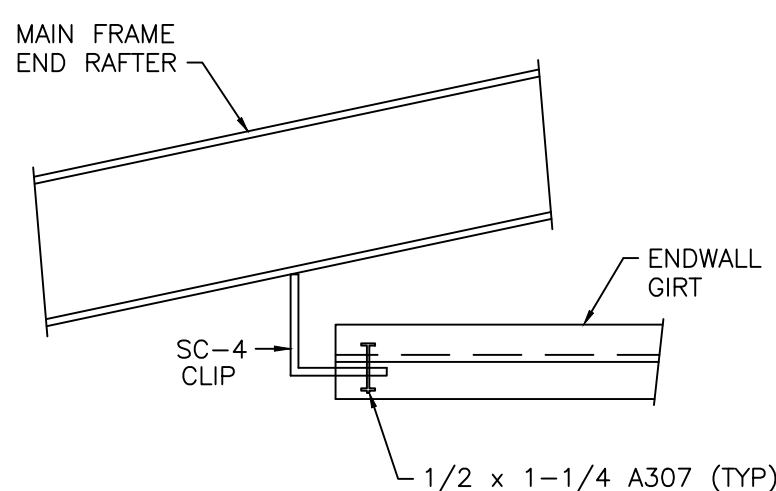
R2 ANCHOR RODS AT SIDEWALL COLUMN



U2 BOLTED END PLATE CONNECTION AT BUILDING PEAK

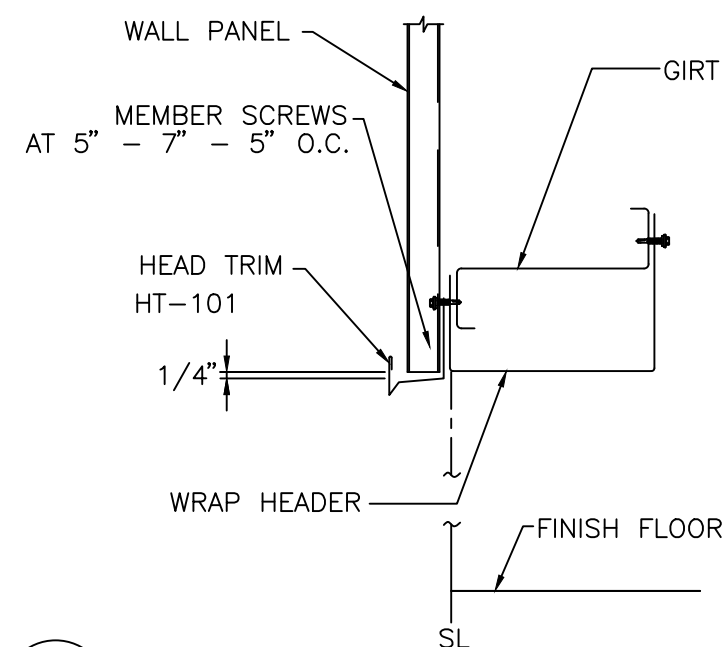


U3 BOLTS FOR RAFTER TO COLUMN CONNECTION

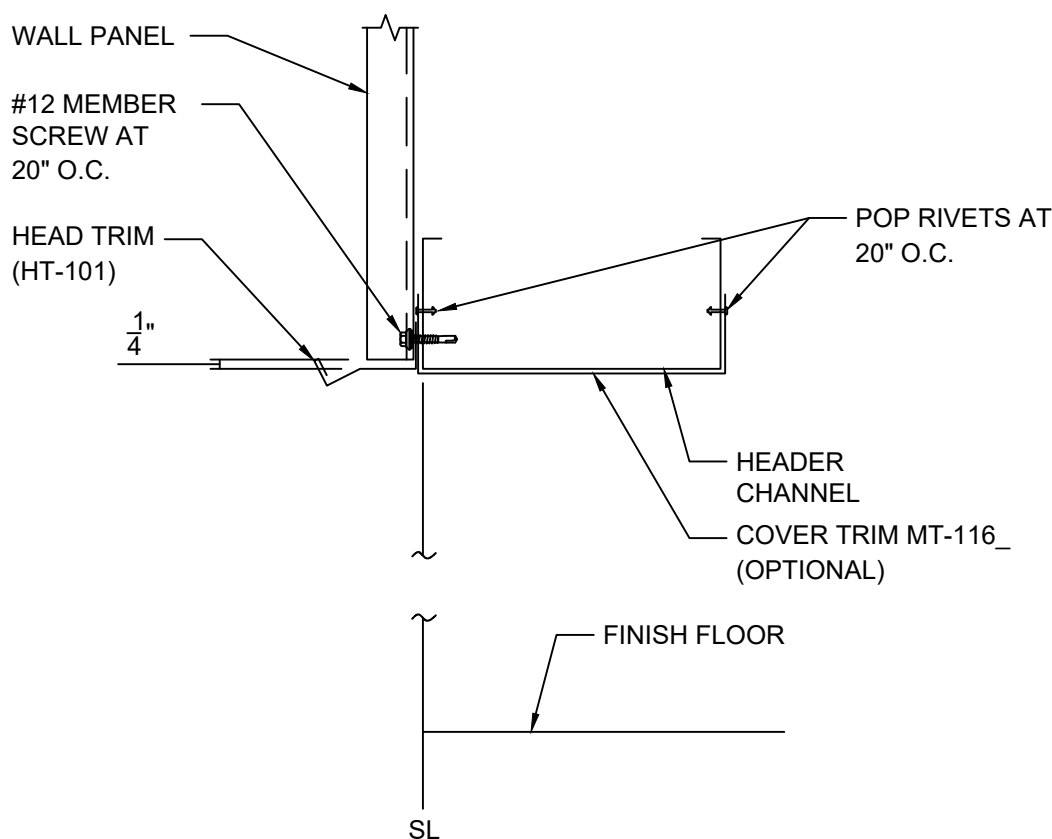


NOTE! ATTACH CLIP TO ENDWALL GIRT BEFORE ATTACHING CLIP TO MAIN FRAME RAFTER

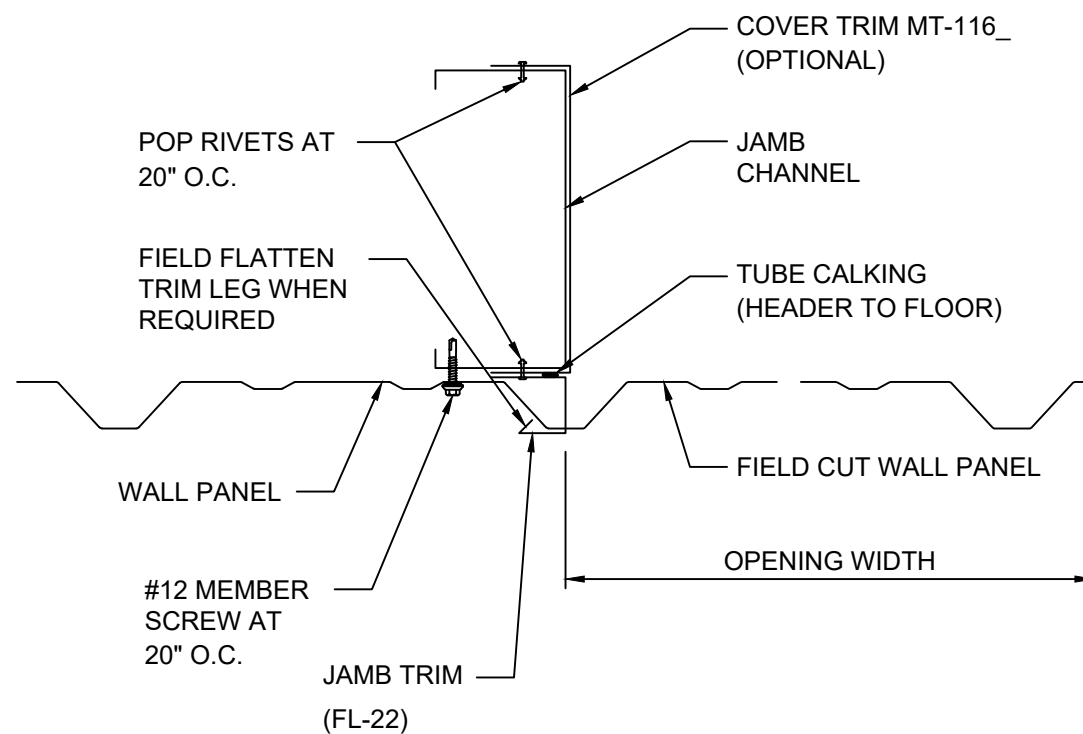
W4 ENDWALL GIRT ATTACHMENT TO MAIN FRAME ENDWALL RAFTER



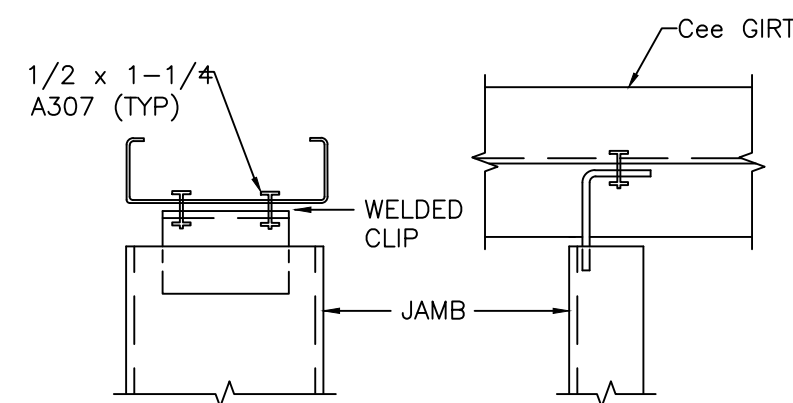
F4 HEAD TRIM INSTALLATION (SUPER SPAN X PANEL)



F2 HEAD TRIM DETAIL (SUPER SPAN X)



F1 JAMB TRIM DETAIL (SUPER SPAN X)



L7 DOOR JAMB TO WALL GIRT

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| JOB NO: 11463-32450 | DWG NO: 03 | ISSUE: P2 | |

