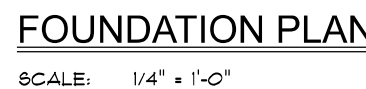


F1
S-2 MONOLITHIC FOOTING
(THICKEN EDGE)
SCALE: 1/2" = 1'-0"



FOUNDATION NOTES	
FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT TO BE REFERRED TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR DISOWAY INC. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION.
FN - 3	THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED W/ #4 WELDED WIRE MESH. MIN. 12" THICKNESS @ 1/2" THICK OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER W/ 6" LAPS SEALED W/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL. (ALSO SEE OTHER NOTES FOR TERMITE-TREATMENT METHOD CAN BE USED INSTEAD)



(TYP.) GABLE BRACING DETAIL
WOOD FRAME

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.

CONNECTOR TABLE				
Uplift SP	Uplift SPT	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600		-
1455	290	H3	4-8x8 1/2"	4-8x8 1/2"
1455	495	HTS2	5-8x8 1/2"	5-8x8 1/2"
1340	105	H10A	9-10x1 1/2"	9-10x1 1/2"
720	620	LTSL2-120	6-10x1 1/2"	6-10x1 1/2"
1000	860	M7SL32-30	7-10x1 1/2"	7-10x1 1/2"
1450	1245	HTS20-30	12-10x1 1/2"	12-10x1 1/2"
Uplift SP	Uplift SPT	Truss Ties	To Member	To Other Member
1235	1235	LSA21	8-10	8-10
1640	1455	MSTA24	9-10	9-10
Uplift SP	Uplift SPT	CSD20	To Stud	To Plate
1030	1030	CSD20	7-10	7-10
Uplift SP	Uplift SPT	Stud Plate Ties	To Stud	To Plate
685	685	SP1	4-10	4-10
1065	605	SP2	6-10	6-10
771	771	LSA224	10-10	wrap under or over plate
1235	1235	LSA224	14-10	wrap under or over plate
1825	1800	DT122	8-SDS 14"x1 1/2"	12"x12" Titen HD
1625	1800	DT122	8-SDS 14"x1 1/2"	12"x12" Titen HD
1425	1800	DT122	18-16x2 1/2"	12"x12" Titen HD
Uplift SP	Uplift SPT	Holdowns @ Mono	To Stud / Post	To Anchor
1825	1800	DT122	8-SDS 14"x1 1/2"	12"x12" Titen HD
1425	1800	DT122	12-SDS 14"x1 1/2"	12"x12" Titen HD
Uplift SP	Uplift SPT	Post Bases @ Stewmal	To Stud / Post	To Anchor
1800		ABU442	12-16d	5/8"x12" Drill & Epoxy
2475		ABU662	12-16d	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPT	Post Bases @ Mono	To Stud / Post	To Anchor
1800		ABU442	12-16d	5/8"x12" Drill & Epoxy
2475		ABU662	12-16d	5/8"x12" Drill & Epoxy

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR THE TRUSS DESIGNER TO VERIFY. TRUSSES SHALL BE DESIGNED FOR THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. THE TRUSS DESIGNER SHALL BE RESPONSIBLE FOR PROVIDING THE TRUSSER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 26X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCE REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCB, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOADS SUBMITTED BY TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER TO OBTAIN A COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL. THE TRUSS MANUFACTURER IS RESPONSIBLE FOR LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER IS RESPONSIBLE FOR THE TRUSS DESIGNER'S DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22
WIND LOADS	
BASIC WIND SPEED (ASCE 7-22, 38 GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C & C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)			
EFFECTIVE WIND AREA (FT ²)	ZONE 4 INTERIOR		ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vasd)	-27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)
0 - 20	+42.6(Vult)	-46.2(Vult)	+42.6(Vult) -57(Vult)

Bryan Zecher Homes, Inc

LACY PAVILION

PROJECT ADDRESS

Mark Disosway FL PE 53915
This item has been digitally signed and sealed by
Mark Disosway PE on digital signature date.
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DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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Lake City, Florida 32025
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JOB NUMBER:
240952

S-1
OF 1 SHEET