

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



SCALE : 1/4" = 1'0"



SCALE : 1/4" = 1'-0"



NOTE: SCREENED BACK PORCH
WITH 1 SCREEN DOOR

* REAR ELEVATION *

SCALE : 1/4" = 1'-0"



* RIGHT SIDE ELEVATION *

SCALE : 1/4" = 1'-0"



* LEFT SIDE ELEVATION *

SCALE : 1/4" = 1'-0"

LAKE CITY
FLORIDA

PRINTED DATE
June05, 2006

DRAWN BY: Teena M. Ruff	CHECKED BY:
----------------------------	-------------

DESIGNED BY:
Teena M. Ruffo

FINIALS DATE

JOB NUMBER:

DRAWING NUMBER

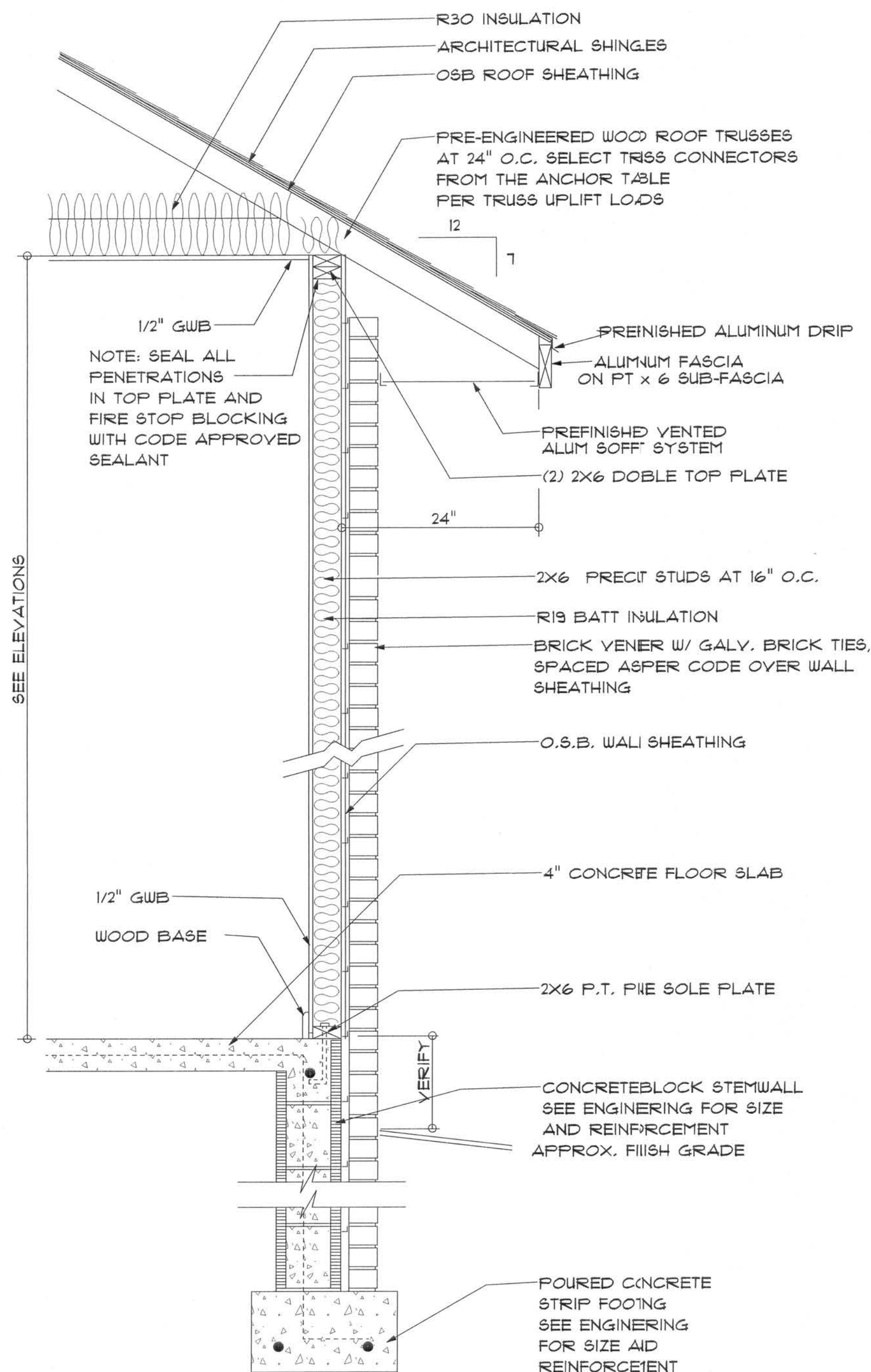
A-1

OF 2 SHEETS

OFFICE COPY

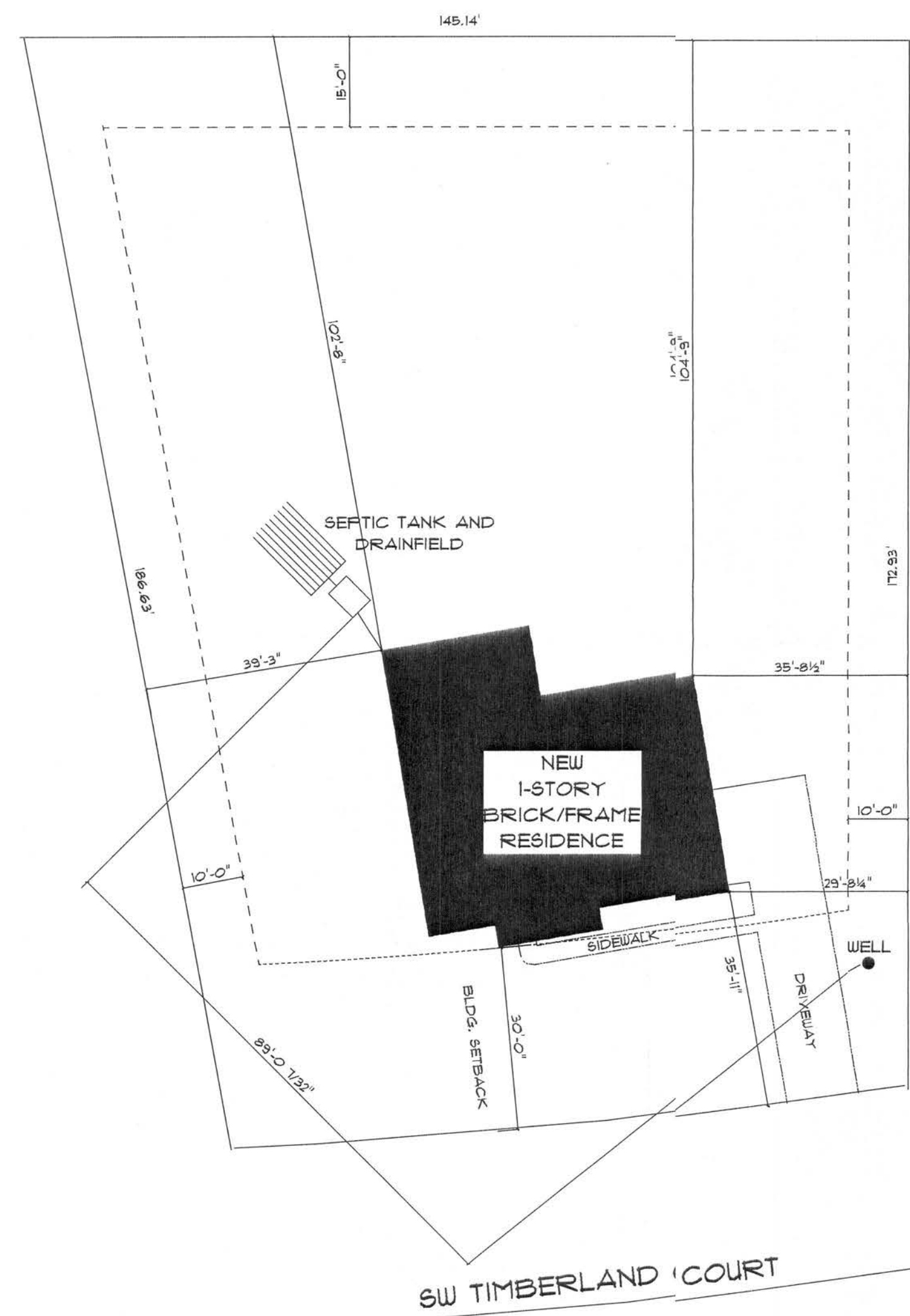
REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

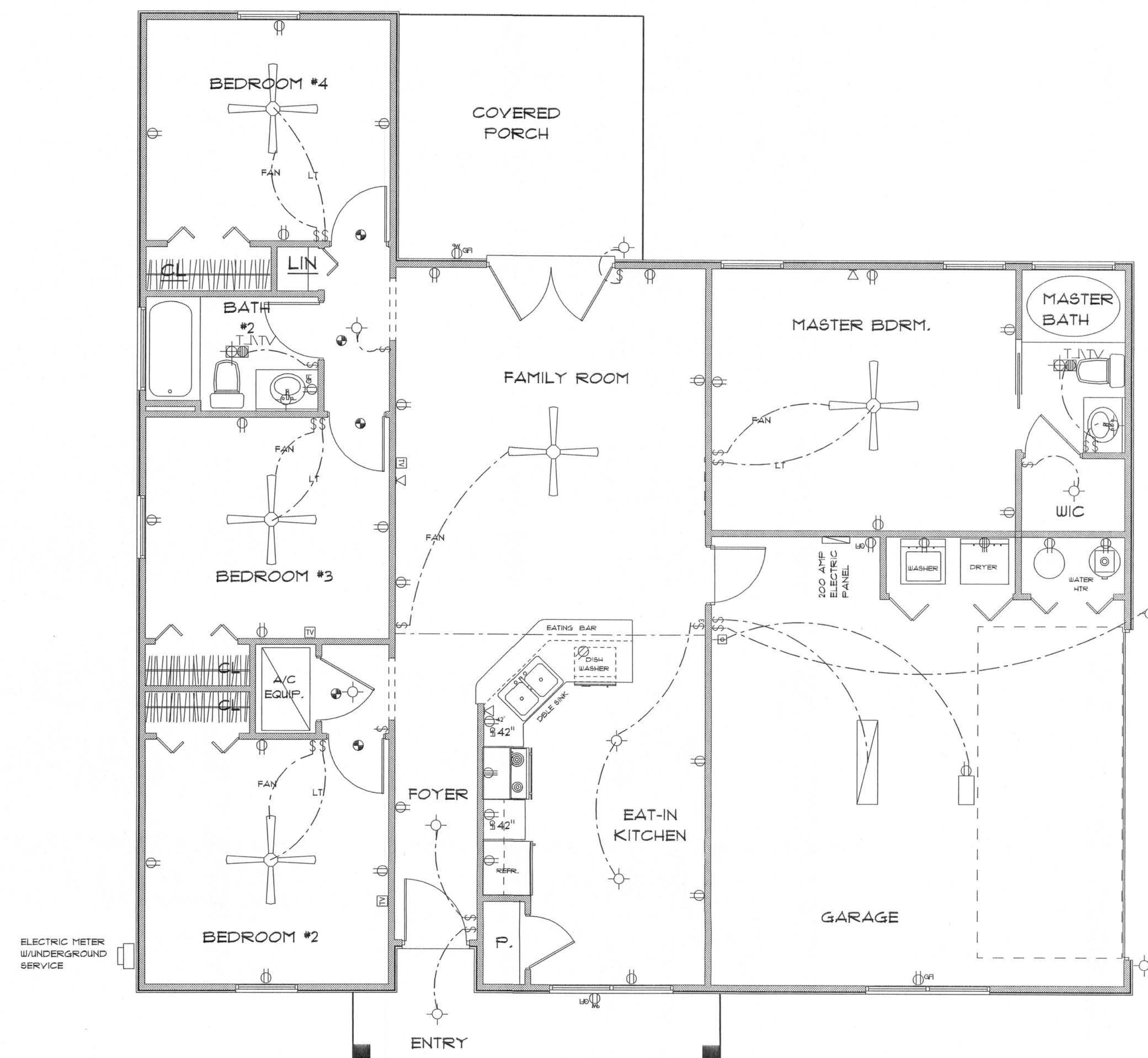


TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA
N.T.S.

LOT #74 of EMERALD COVE



* SITE PLAN *
SCALE : 1" = 20'-0"



* ELECTRICAL PLAN *

SCALE : 1/4" = 1'-0"

ELECTRICAL PLAN NOTES

- E-1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUFACTURER'S SPECIFICATIONS.
- E-2 CONSULT THE OWNER FOR THE NUMBER OF SEPARATE TELEPHONE LINES TO BE INSTALLED.
- E-3 ALL INSTALLATIONS SHALL BE PER NATIONAL ELECTRIC CODE.
- E-4 ALL SMOKE DETECTORS SHALL BE 120V W/BATTERY BACKUP OF THE PHOTOELECTRIC TYPE AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E-5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTION AND IN ACCORDANCE WITH APPLICABLE SECTIONS OF NATIONAL ELCT. CODE LATEST EDITION.
- E-6 ELECTRICAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E-7 ENTRY OF SERVICE UNDERGROUND OR OVERHEAD) IS TO BE DETERMINED BY THE POWER COMPANY.
- E-8 ALL BEDROOM RECEPTALS ARE TO BE AFCI (ARC FAULT CIRCUIT INTERRUPT).

ELECTRICAL	COUNT	SYMBOL
200 amp service panel	1	
Light	2	
automatic garage dr opener	1	
cable tv outlet	3	
ceiling fan	5	
electric meter	1	
fluorescent lt fixture	1	
garage dr opener	1	
gfi waterproof outlet	2	
light	10	
outlet	31	
outlet 220v	2	
outlet gfi	5	
smoke detector	5	
switch	20	
switch 3 way	1	
telephone	3	
vent light combo	2	

THE VICTORIA
LOT #74
EMERALD COVE

LAKE CITY
FLORIDA

Tsena M. Ruffo
251 NW Hall of Fame Dr.
Lake City, Florida 32055
Phone: (386) 755-5983
Cell: (386) 867-1191
Email: rarchdesigns@lani.net

PRINTED DATE:
June 05, 2006

DRAWN BY: Tsena M. Ruffo
CHECKED BY:

DESIGNED BY:

Tsena M. Ruffo

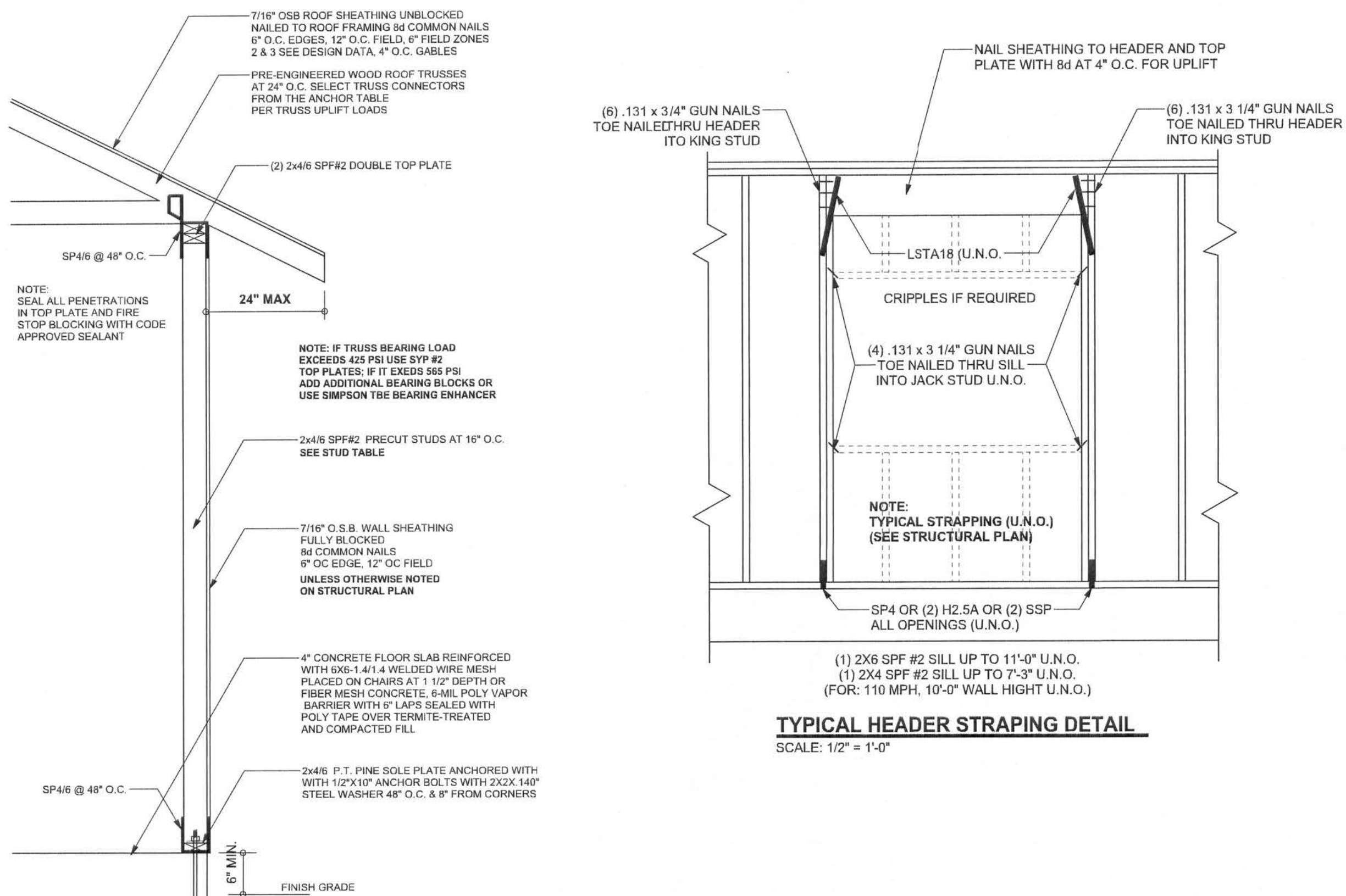
FINALS DATE:

JCB NUMBER:

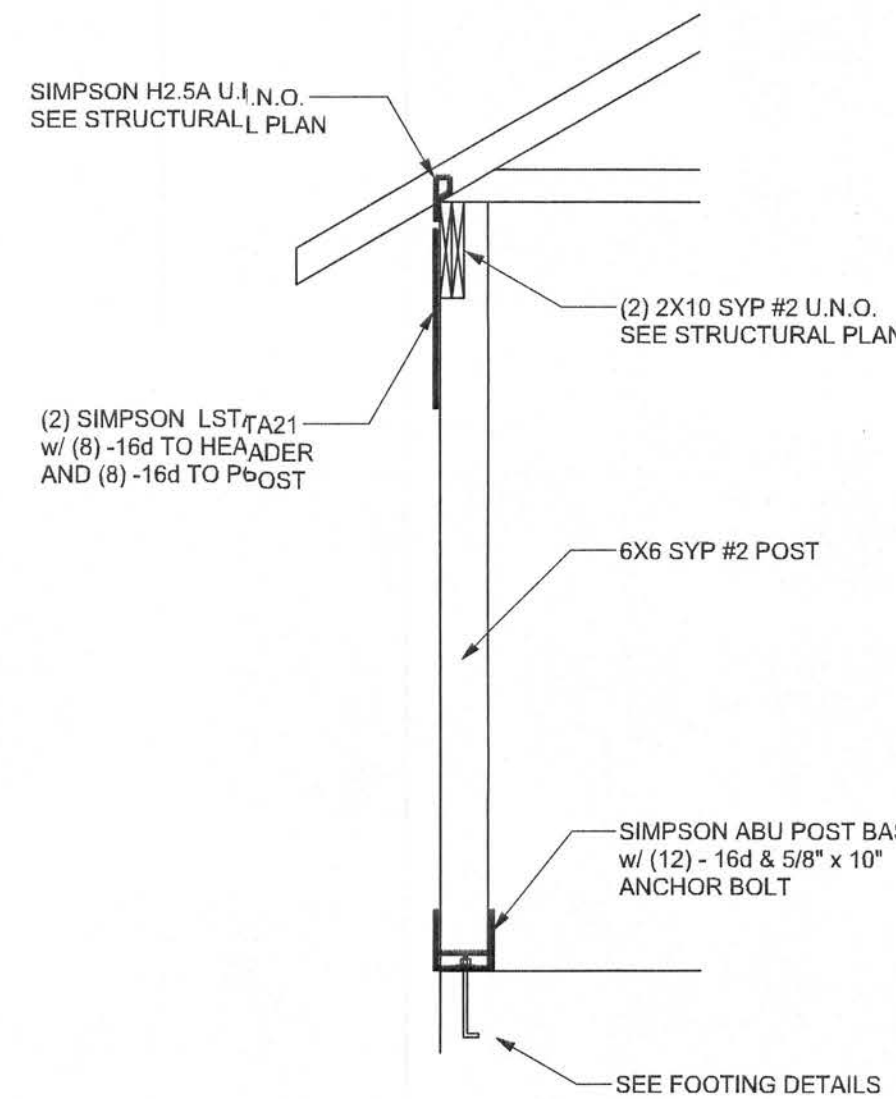
DRAWING NUMBER

A-2

OF 2 SHEETS

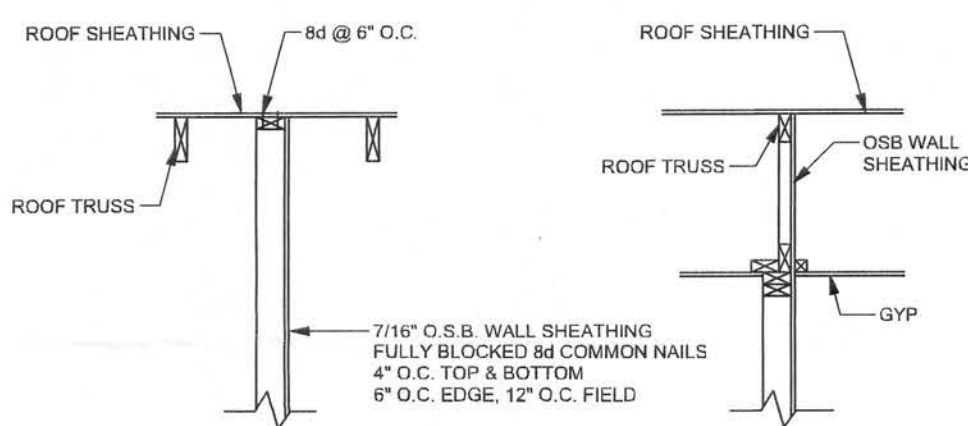


TYPICAL HEADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"



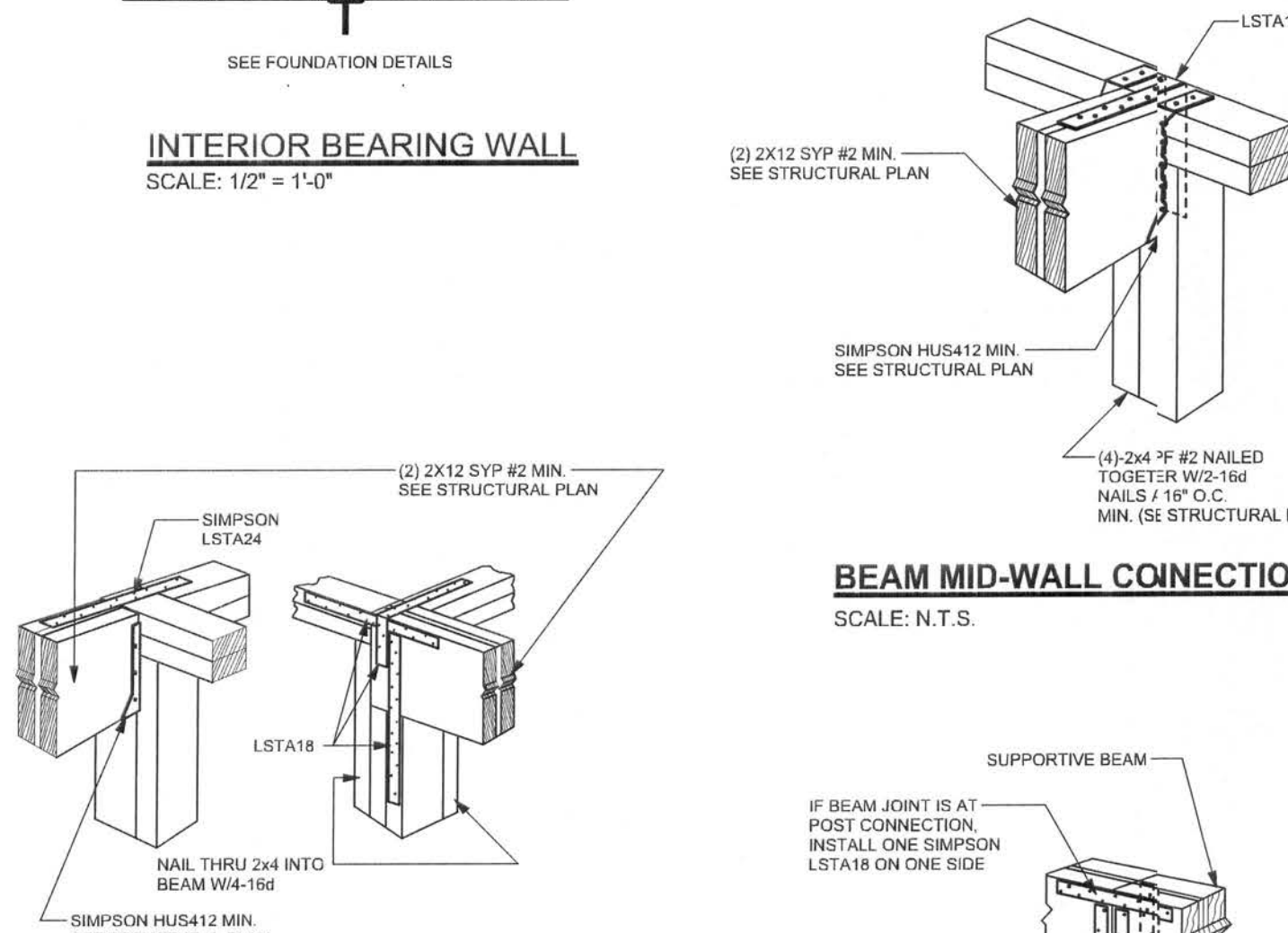
TYPICAL PORCH POST DETAIL
SCALE: 1/2" = 1'-0"

ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



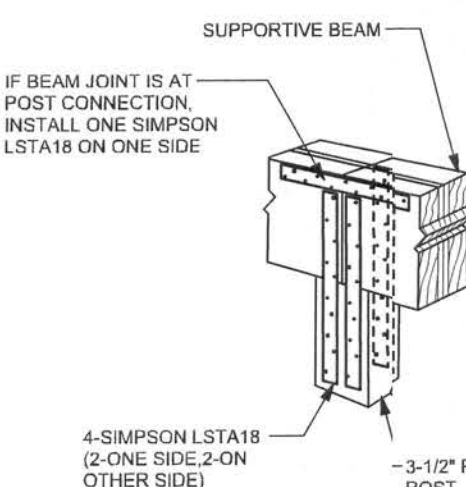
INTERIOR SHEAR WALL DETAIL
SCALE: 1/2" = 1'-0"

INTERIOR BEARING WALL
SCALE: 1/2" = 1'-0"



BEAM CORNER CONNECTION DETAIL
SCALE: N.T.S.

BEAM MID-WALL CONNECTION DETAIL
SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL
SCALE: N.T.S.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR	TO PLATES	TO RAFTERS/TRUSS	TO STUDS
< 420	< 245	HSA	3-8d	3-8d	
< 455	< 265	HS	4-8d	4-8d	
< 390	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 780	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H15-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H15-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2-HTS24			
< 2050	< 1785	LGT2	14-16d	14-16d	
HEAVY GIRDER TIED DOWN			TO FOUNDATION		
< 3965	< 3330	MGT		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTION			TO STUDS		
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 800	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LST18	14-10d		
< 1235	< 1235	LST21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHOR			TO FOUNDATION		
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	H20A	2-5/8" BOLTS		5/8" AB
< 4175	< 3995	HTT16	18-16d		5/8" AB
< 1400	< 1400	PH402	16-16d		
< 3335	< 3335	HP4022	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 100 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F'_c = 3000$ PSI

WELDED WIRE REINFORCED SLAB: 8" x 8" W1.4 x W1.4 FB #8@5d, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, $F_y = 60$ KSI. ALL LAP SPICES 41" DB (30" FOR #6 BARS). UNO ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-06, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNLOCKED. APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131, 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO)

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 6" IN GROUTED CMU

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 7" x 5/8", WITH 5/8" BOLTS TO BE 3" x 3" x 5/8", WITH 3/4" BOLTS TO BE 3" x 3" x 5/8", WITH 7/8" BOLTS TO BE 3" x 3" x 5/8", UNO

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

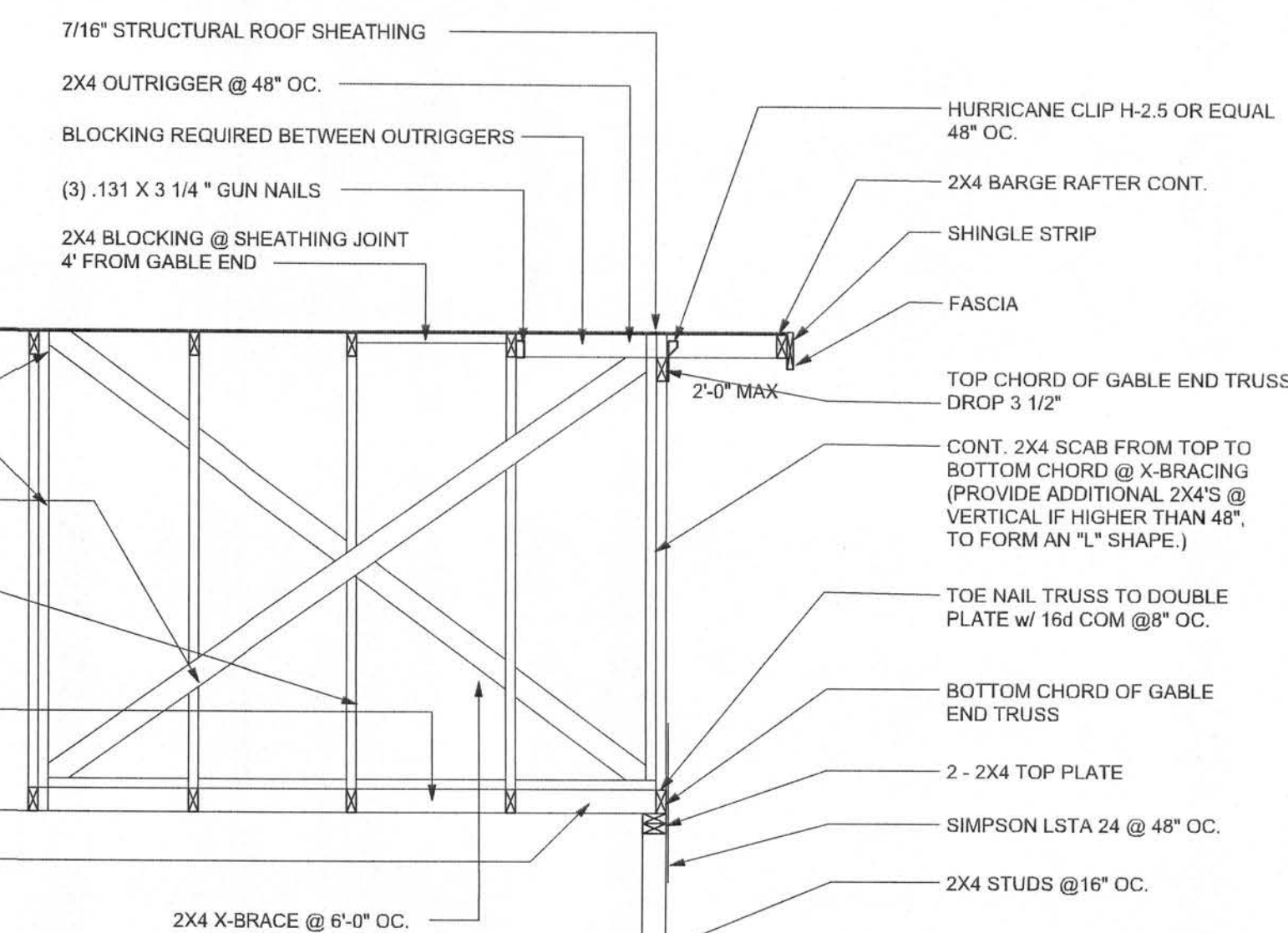
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R302.1.2 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACINGS. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24FV3 S	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.2.0B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMINGS LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 80 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 80 FT IN EXP. B, 30 FT IN EXP. C AND 10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- 1.) BASIC WIND SPEED = 110 MPH
- 2.) WIND EXPOSURE = B
- 3.) WIND IMPORTANCE FACTOR = 1.0
- 4.) BUILDING CATEGORY = II
- 5.) ROOF ANGLE = 10-45 DEGREES
- 6.) MEAN ROOF HEIGHT = < 30 FT
- 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)	10	100
1	19.9 - 21.8	18.1	-18.1
2	19.9 - 25.5	18.1	-21.8
2 O'ng	-40.6	-40.6	
3	19.9 - 25.5	18.1	-21.8
3 O'ng	-68.3	-42.4	
4	21.8 - 23.8	18.5	-20.4
5	21.8 - 28.1	18.5	-22.6
Doors & Windows Worst Case (Zone 5, 10, 102)		21.8	-29.1
8x7 Garage Door	19.5	-22.9	
16x7 Garage Door	18.5	-21.0	

DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

REVISIONS

NO.	DESCRIPTION	DATE

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, P.O. Box 888, Lake City, FL 32063, 386-754-5419

DIMENSIONS: Stud dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS: Mark Disoway, P.E. hereby expressly reserves all copyright and property rights in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disoway.

CERTIFICATION: I, hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY

P.E. 53915

SEAL

Cornerstone Development

Zecher Bryan

The Victoria Model

ADDRESS:

Lot #74 Emerald Cove S/D

Lake City, Florida

Mark Disoway P.E.

P.O. Box 888

Lake City, Florida 32056

Phone: (386) 754 - 5419

Fax: (386) 269 - 4871

PRINTED DATE:

June 05, 2006

STRUCTURAL BY:

EVAN BANSLEY

FINALS DATE:

June 5, 2006

JOB NUMBER:

605162

DRAWING NUMBER

S-1

OF 2 SHEETS

