DATE 10/16/2	Columbia County B This Permit Must Be Prominently Posted	uilding Permit	PERMIT 000027431
APPLICANT F	PATRICK WILSON	PHONE 352 206-5459	
	6800 SOUTHPOINT PARKWAY	LAKE CITY	FL 32216
	MARONDA HOMES	PHONE 904-296-1490	
SERVING MANAGEMENT	304 SW MULBERRY DRIVE	LAKE CITY	FL 32024
CONTRACTOR		PHONE 904 296-1490	
LOCATION OF	PROPERTY 90W, TL 247S, TR ON 252B, TL	TIMBER RIDGE, TL	
	MULBERRY, 7TH LOT ON RIG	HT	
TYPE DEVELO	PMENT SFD,UTILITY ES	TIMATED COST OF CONSTRUCTION	138550.00
HEATED FLOO	R AREA 2223.00 TOTAL AR	EA _2771.00 HEIGHT _2	25.00 STORIES 2
FOUNDATION	CONC WALLS FRAMED	ROOF PITCH 6/12 F	LOOR SLAB
LAND USE & Z	ONING RSF-2	MAX. HEIGHT	
Minimum Set Ba	ck Requirments: STREET-FRONT 25.00	REAR15.00	SIDE 10.00
NO. EX.D.U.	0 FLOOD ZONE X PP	DEVELOPMENT PERMIT NO.	
PARCEL ID	10-4S-16-02856-130 SUBDIVISIO	N TIMBERLANDS	
LOT 30	BLOCK PHASE UNIT	TOTAL ACRES 0	.50
000001681	CBC1256382	17/1/1/1/	//
Culvert Permit No		mber Applicant/Owne	r/Contractor
CULVERT	08-622 BK	HD	Y
Driveway Connec	ction Septic Tank Number LU & Zoni	ng checked by Approved for Issuan	ce New Resident
COMMENTS:	ELEVATION CONFIRMATION LETTER REQUIRES	O AT SLAB,	77
1			
		Check # or C	Cash 941618
	FOR BUILDING & ZONII	IO DEDADTMENT ONLY	
Temporary Powe	I ON DOILDING & ZOM	NG DEPARTMENT ONLY	(footer/Slab)
	w 1.0	Monolithic	(footer/Slab)
	w 1.0	5.5 W.W.	(footer/Slab) date/app. by
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a Tar	fr Foundation date/app. by Slab date/app. by	date/app. by Sheathing	date/app. by
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NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

The same of the sa	
For Office Use Only Application # 0809-51 Date	e Received 9/24 By W Permit # 1681/27431
Zoning Official BLK Date 26 09.08 Flood Zone	Land Use KES Low DE Zoning RSF-2
FEMA Map #	4
Comments Ecution Construction Letter Required	
	of Auth. from Contractor D F W Comp. letter
// //	Corr 4409-16 Road/Code 1 046.00 /210
School 500.00 = TOTAL	3,063,67
Septic Permit No	Fax (904) 332-10375
Name Authorized Person Signing Permit Theodore Brock	Patrick Wilson Phone (904) 2910-1490
Address 10800 Southpoint PKWy #300 Jackson	
Owners Name Maronda Homes Inc. of Florida	Phone (0104) 2911-1490
911 Address 304 SW Mulberry Drive	Lake City, FL 32055
Contractors Name Theodore C. Brock	Phone (904) 2910-1490
Address 6800 Southpoint PKM #300 BICKSONNI	6. FL 32210
Fee Simple Owner Name & Address NA	
Bonding Co. Name & Address NA	
Architect/Engineer Name & Address TOMS Ponce 400	5 Maronda Way Sanford, Pl 32771
Mortgage Lenders Name & Address Bank of Property 2	250 Park Ave. S. #400 Wirther Park, Ph. 32789
Circle the correct power company - FL Power & Light -(Clay Elec. – Suwannee Valley Elec. – Progress Energy
Property ID Number 0-45-10-02850-130	EstImated Cost of Construction \(\bigcirc \) \(\text{OO} \) \(\text{OS5} \).
Subdivision Name Timber lands	Lot <u>30</u> Block <u>/</u> Unit Phase
Driving Directions HW 90, Left on 241 South;	Right on 252B; left on Timber Ridge,
TIL ON SW Mulberry DRIVE, 7th LOT ON	Right.
, 1 	Number of Existing Dwellings on Property
Construction of Residential Single Family Dwelling	Total Acreage Lot Size
Do you need a - Culvert Permit or Culvert Waiver or Have	e an Existing Drive Total Building Height 25
Actual Distance of Structure from Property Lines - Front_50	0 Side 48.0 Side 48.0 Rear 61.0
Number of Stories 2 Heated Floor Area 2223	Total Floor Area 2771Roof Pitch _12/6
Application is hereby made to obtain a permit to do work a installation has commenced prior to the issuance of a perm of all laws regulating construction in this jurisdiction.	nit and that all work be performed to meet the standards
Page 1 of 2 (Both Pages must be submitted together.)	Revised 1-10-08

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION: An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)

The color C. Brock

Competency Card Number

Competency Card Number

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 28 day of August 2008

Personally known X or Produced Identification

State of Florida Notary Signature (For the Contractor)

MEISSEL MCKAGE

SEAL:

Notary Public State of Florida Melissa L McKague My Commission DD493647 Expires 11/22/2009 This Instrument Prepared by and Return to:

Amy Wesp SOUTHERN

HOLDING

COMPANY, LLC.

3943 BAY MEADOWS ROAD **JACKSONVILLE, Florida 32217**

as a necessary incident to the fulfillment of conditions contained in a title insurance commitment issued by it.

Property Appraisers Parcel LD. (Folio) Number(s): R02856-000

Grantee(s) LD..#(s): File No: JX0812085

Inst:200812010775 Date:6/6/2008 Time:1:05 PM

WARRANTY DEED (CORPORATION)

This Warranty Deed Made this 27th day of May, 2008, by RML HOLDINGS INC., A FLORIDA CORPORATION, and having its place of business at 703 NW BLACKBERRY CIRCLE, LAKE CITY, Florida 32055, hereinafter called the grantor,

to MARONDA HOMES, INC. OF FLORIDA, A FLORIDA CORPORATION, whose post office address is: 11200 ST. JOHNS INDUSTRIAL PARKWAY, JACKSONVILLE, FLORIDA 32246, hereinafter called the grantee,

\$899,000.00

WITNESSETH: That said grantor, for and in consideration of the sum of \$49.00 Dollars and other valuable considerations, receipt whereof is hereby acknowledged, by these presents grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land simute in Columbia County, Florida, viz: LOTS 1, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 24, 27, 28, 29(30/31, 32, 33, 34, 35, 36, 37, 38, 39, 40, AND 41, OF TIMBERLANDS, PHASE 1, ACCORDING TO PLAT THEREOF AS RECORDED IN PLAT BOOK 9, PAGE 26 AND 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

TOGETHER with all the tenements, heredituments and appurtenances thereto belonging or in anywise appertaining. To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except

taxes accruing subsequent to December 31, 2007, reservations, restrictions and easements of record, if any.

(Wherever used herein the terms "granter" and "grantee" included all the parties to this instrument, and the heirs, legal representatives and exigens of individuals, and the successors and assigns of corporation.)

in Witness Whereof, the Grantor has caused these presents to be executed in its name, and its corporate seal to be hereaften affixed, by its proper officers thereunto duly authorized, the day and year first above written.

Signed: seafed and delivered in our presence:

AIFESI: -		20			
*	Secretary	RML HOLDIN	GS INC.		
Witness Signature: New S	D. Marus	- 0		-0.	1
Printed Name: World D.		BY:	nt K	7	X
Witness Signature: Jody Printed Name:	n. Gable. AVP m. Gable	ROBERTAL LA	RDIZABAL,	PRESIDENT	
STATE OF FLORIDA COUNTY OF DUVAL				******	_
The foregoing instrument was a ROBERT R. LARDIZABAL as behalf of the corporation. He/she	PRESIDENT OF RIVIL	HOLDINGS INC., A	L'LUKUDA C	ORPORATIO	
My Commission Expires		0 7	$\overline{}$	Yn-	

WORTH D. MORRIS ry Public - State of Florida s Jun 18, 201

March N-1/Jain WOETH D MURRIS Printed Name: Notary Public

Serial Number



STATE OF FLORIDA DEPARTMENT OF HEALTH

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ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

PHASE 1" AS N PLAT BOOK RECORDS OF

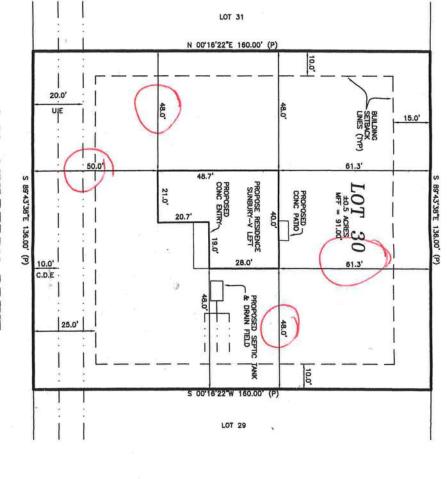


PROPOSEDBUILDING LAYOUT

30

IN SECTION 10, TOWNSHIP 4 SOUTH, RAN 16 EAST, COLUMBIA COUNTY, FLORIDA

TOWNSHIP 4 SOUTH, RANGE



Z = TELEPHONE PEDESTAL

0 = WOOD POWER POLE

MULBERRY DRIVE

FLOOD NOTE:

HEREON ARE
BUT ARE SUBJECT
ONSTRUCTION, THE
'SHOULD BE
BACK REQUIREMENTS.

N HAVE NOT BEEN

NOMPLETION (SEE

THE BOUNDARY
THE LOCATION OF
IN TO THE
CORNERS FOUND

NTS, OTHER THAN

IS, IF ANY, MAY NOT

UPON A BENCHMARK SET IN A WITH AN ELEVATION OF 98.76°, THIS S SURVEYOR BY BRITT SURVEYING TWN.

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE WATTOWAL FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, NIED 16-BB, THIS PROPERTY IS IN FLOOD ZONE, "Y," WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-TEAR FLOOD PLAN, AS SOLED FROM SUID MAP. INFORMATION FROM THE FEDERAL BURGESHOT MANAGEMENT ASPENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND MAENIMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

THE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. IT HIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAMS OF TITLE, EXEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

1" = 30' BI = FOUND 6" X 6" S.R.O. R/W MON. D = SET 4" X 4" CONC. MON. P.S.M. 5582 B = CATV RISER X = FOUND NAIL & DISK X = SET NAIL & DISK P.S.M. 5582 ■ = FOUND 4" X 4" CONC. MON NO IDENTIFICATION O = SET 1/2" REBAR & CAP LB. 6894 0 = FOUND 1/2" REBAR & CAP LB. 6894 FOUND 1/2" REBAR NO IDENTIFICATION a = FOUND 3/4" IRON PIPE LEGEND: 0 ABBREVATIONS: A/C = AIR CONDITIONER ASPH = ASPHAT ASPH = ASPHAT ASPH = ASPHAT ASPH = ASPHAT ASPH = ASPHAT ASPH = ASPHAT ASPHAT = ASPHAT ASPHAT = ASPHAT ASPHAT = ASPHAT ASP 30 NC = CONCRETE EC = ELECTRIC EY = ELEVATION EY = FOUND NC = FOUND N TRANSFORMER TYPICAL WATER METER WATER VALVE 60 90

THIS IS NOT A BOUNDARY SURVEY

CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF
A FLORIDA LICENSED SURVEYOR AND MAPPER, ADDITIONS OR DELETIONS
TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND I MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD-OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STANDEZ, AND CHATTER 53577-6, FLORIDA ADMINISTRATIVE CODE.



DATE

BRINKMAN, PSM - FLA CERT# 5582

108

BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609

PHONE: (352) 374-7707 FAX: (352) 374-8757

DATE: 8/14/08 SCALE: 1" = 30" THE BENCHMARK IN QUALITY SERVICE. DRAWN BY: ZL CHECKED BY: J.B.

... FIELDBOOK **, PAGE **

FIELD WORK COMPLETED ON

PREPARED FOR: MARONDA

DRAWING NUMBER 176-08

Project Name: SUNBURY GAINESVILLE

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Builder:

MARONDA HOMES

Address: 307 SW WINDOWN DYWE City, State: LAIL CHY, TO 132055 Owner: ELECTRIC Climate Zone: North		Permitting Office: Permit Number: 27 Jurisdiction Number:	(0/umbig 431 221000
 Single family or multi-family Number of units, if multi-family Number of Bedrooms Is this a worst case? 	1	 12. Cooling systems a. Central Unit b. N/A c. N/A 13. Heating systems a. Electric Heat Pump b. N/A c. N/A 14. Hot water systems a. Electric Resistance b. N/A c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating) 	Cap: 40.5 kBtu/hr SEER: 13.00
Glass/Floor Area: U 12		pints: 28722 pints: 30122 PASS	

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY:

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Milma MCKan

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: DATE:

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub:, Plat:,,,
PERMIT #:

	BASE		C-W-26-2-1	a a		AS-	BU	ILT				***************************************
GLASS TYPES .18 X Condition Floor A	ned X BS	PM = I	Points	Type/SC		erhang Len		Area X	SPI	их	SOF	= Points
.18 2223	3.0	18.59	7439.0	1.Single, Clear	E	1.0	6.0	30.0	47.9	92	0.97	1394.0
				2.Single, Clear	Ε	1.0	15.0	30.0	47.9		1.00	1432.0
				3.Single, Clear	Ε	1.0	4.0	9.0	47.9	92	0.91	393.0
				4.Single, Clear	E	1.0	4.0	6.0	47.9	92	0.91	262.0
				5.Single, Clear	Е	1.0	4.0	6.0	47.9		0.91	262.0
				6.Single, Clear	N	1.0	15.0	30.0	21.7		1.00	649.0
				7.Single, Clear	W	1.0	16.0	40.0	43.8		1.00	1748.0
	9			8.Single, Clear	W	1.0	15.0	30.0	43.8		1.00	1311.0
7,				9.Single, Clear	W	1.0	13.0	9.0	43.8		1.00	393.0
				10.Single, Clear	W	1.0	5.5	15.0	43.8		0.96	632.0
				11.Single, Clear	W	1.0	5.5	15.0	43.8		0.96	632.0
			100	12.Single, Clear 13.Single, Clear	W	1.0	5.5	30.0	43.8		0.96	1265.0
				14.Single, Clear	E	4.0	2.5 7.0	8.0 5.0	47.9		0.42	161.0
				14.Single, Clear	_	4.0	7.0	5.0	47.9	92	0.68	162.0
			36	As-Built Total:	Manager State Sta			263.0				10696.0
WALL TYPES	Area X	BSPM	= Points	Туре		R	-Value	e Area	X	SPM	=	Points
Adjacent	159.0	0.70	111.3	1. Concrete, Int Insul, Exterio	or		4.1	720.0		1.13		817.2
Exterior	1712.0	1.70	2910.4	2. Frame, Wood, Exterior			13.0	992.0		1.50		1488.0
				3. Frame, Steel, Adjacent			13.0	159.0		0.90		143.1
Base Total:	1871.0		3021.7	As-Built Total:		-		1871.0				2448.3
DOOR TYPES	Area X	BSPM	= Points	Туре				Area	X	SPM	1 =	Points
. Adjacent	17.0	2.40	40.8	1.Exterior Insulated				20.0		4.10		82.0
Exterior	20.0	6.10	122.0	2.Adjacent Wood				17.0		2.40		40.8
Base Total:	37.0		162.8	As-Built Total:			22	37.0				122.8
CEILING TYPE	S Area X	BSPM	= Points	Туре		R-Val	ue .	Area X S	SPM	X SC	:M =	Points
Under Attic	1120.0	1.73	1937.6	1. Under Attic			19.0	1120.0	2.34 2	X 1.00		2620.8
Base Total:	1120.0		1937.6	As-Built Total:				1120.0				2620.8
FLOOR TYPES	Area X	BSPM	= Points	Туре		R	-Value	e Area	X	SPN	=	Points
Slab Raised	136.0(p) 0.0	-37.0 0.00	-5032.0 0.0	1. Slab-On-Grade Edge Insu	lation		0.0	136.0(p		41.20		-5603.2
Base Total:			-5032.0	As-Built Total:		- FORTHOO		136.0				-5603.2

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub:, Plat:,,,
PERMIT #:

	BASE		AS-BUILT										
INFILTRATION	Area X BSPM =	Points					Area	X SPM	=	Points			
	2223.0 10.21	22696.8					2223.0	10.21		22696.8			
Summer Bas	e Points: 3022	5.9	Summer A	\s-Built	Points:				32	2981.5			
Total Summer > Points	~~ (a. c.) (~~ 5.00 ~ 5.00 ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	ooling oints	Total Component (System - Po	X Cap Ratio pints)	X Duct Multiplie (DM x DSM x	er Multipl		Credit Multiplier		Cooling Points			
30225.9	0.3250	9823.4	(sys 1: Central Uni 32982 32981.5	1.00 1.00	SEER/EFF(13.0 (1.08 x 1.147 1.07 2	x 0.86) 0.2	60	nt(AH),R6.0(II 0.950 0.950	- 1	8732.2 732.2			

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , , PERMIT #:

BASE			AS-	-BU	LT		CONTRACT.		
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area	Type/SC	Ove Ornt	erhang Len		Area X	WPM	X	WOF	= Points
.18 2223.0 20.17 8071.0	1.Single, Clear	E	1.0	6.0	30.0	26.41		1.02	804.0
	2.Single, Clear	E	1.0	15.0	30.0	26.41		1.00	795.0
4-	3.Single, Clear	E	1.0	4.0	9.0	26.41		1.04	246.0
	4.Single, Clear	E	1.0	4.0	6.0	26.41		1.04	164.0
,	5.Single, Clear	E	1.0	4.0	6.0	26.41		1.04	164.0
	6.Single, Clear	N	1.0	15.0	30.0	33.22		1.00	996.0
	7.Single, Clear	W	1.0	16.0	40.0	28.84		1.00	1154.0
*	8.Single, Clear	W	1.0	15.0	30.0	28.84		1.00	866.0
11	9.Single, Clear	W	1.0	13.0	9.0	28.84		1.00	259.0
12	10.Single, Clear	W	1.0	5.5	15.0	28.84		1.01	436.0
<u></u>	11.Single, Clear	W	1.0	5.5	15.0	28.84		1.01	436.0
*	12.Single, Clear	W	1.0	5.5	30.0	28.84		1.01	873.0
4	13.Single, Clear	E	4.0	2.5	8.0	26.41		1.41	297.0
*	14.Single, Clear	E	4.0	7.0	5.0	26.41		1.15	151.0
	As-Built Total:				263.0				7641.0
WALL TYPES Area X BWPM = Points	Туре		R-	-Value	e Area	X V	/PN	1 =	Points
Adjacent 159.0 3.60 572.4	1. Concrete, Int Insul, Exterio)r	All Orange	4.1	720.0	F	.42		4622.4
Exterior 1712.0 3.70 6334.4	2. Frame, Wood, Exterior			13.0	992.0		.40		3372.8
	3. Frame, Steel, Adjacent			13.0	159.0		.90		779.1
Base Total: 1871.0 6906.8	As-Built Total:				1871.0				8774.3
DOOR TYPES Area X BWPM = Points	Туре					x v	/PN	=	Points
Adjacent 17.0 11.50 195.5	1.Exterior Insulated				20.0	5	.40		168.0
Exterior 20.0 12.30 246.0	2.Adjacent Wood				17.0		.50		195.5
Base Total: 37.0 441.5	As-Built Total:				37.0				363.5
CEILING TYPES Area X BWPM = Points	Туре	R	-Value	- Δι	rea X W	PM Y	MC	:M =	Points
Under Attic 1120.0 2.05 2296.0	1. Under Attic		Value	19.0	1120.0	-			3024.0
Base Total: 1120.0 2296.0	As-Built Total:				1120.0				3024.0
FLOOR TYPES Area X BWPM = Points	Type	1	R.	-Value	The state of the s	x v	/PM	=	Points
			11	value	, Alea	, V	1 10	-	-
Slab 136.0(p) 8.9 1210.4 Raised 0.0 0.00 0.0	Slab-On-Grade Edge Insul	ation		0.0	136.0(p	. 18	.80		2556.8
Base Total: 1210.4	As-Built Total:				136.0				2556.8

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub:, Plat:,,,
PERMIT #:

BASE	AS-BUILT
INFILTRATION Area X BWPM = Poin	Area X WPM = Points
2223.0 -0.59 -131	.6 2223.0 -0.59 -1311.6
Winter Base Points: 17614	1 Winter As-Built Points: 21048.0
Total Winter X System = Heating Points Multiplier Points	Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)
	(sys 1: Electric Heat Pump 40500 btuh ,EFF(8.1) Ducts:Unc(S),Con(R),Int(AH),R6.0 21048.0 1.000 (1.060 x 1.169 x 0.88)0.421 0.950 9215.8
17614.1 0.5540 9758	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , , PERMIT #:

	E	BASE					983	Α	S-BUII	Т			
WATER HEA Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio		X Credi Multip	t =	Total
4		2635.00		10540.0	50.0	0.90	4		1.00	2693.56	1.00		10774.2
					As-Built To	otal:							10774.2

				CODE	C	OMPLI	ANCE	ST	TATUS	3			
		BAS	SE					VID		AS	-BUILT	C OGNIN	
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
9823		9758		10540		30122	8732		9216		10774		28722

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: ,	Plat:,,,	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	V
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	/
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	1
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	V
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	V
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	./
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	1

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	/
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	V
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	1
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	V

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.3

The higher the score, the more efficient the home.

ELECTRIC, Lot:, Sub:, Plat:,,,

1.	New construction or existing Ne	w _	12. Cooling systems	
2.	Single family or multi-family Single famil	у	a. Central Unit	Cap: 40.5 kBtu/hr
3.	Number of units, if multi-family	1		SEER: 13.00
4.	Number of Bedrooms	4 _	b. N/A	
5.	Is this a worst case?	es		
6.	Conditioned floor area (ft²) 2223 f	t ²	c. N/A	-
7.	Glass type 1 and area: (Label reqd. by 13-104.4.5 if not default))		
a.	U-factor: Description Area		13. Heating systems	
	(or Single or Double DEFAULT) 7a(Sngle Default) 263.0 ft ²		a. Electric Heat Pump	Cap: 40.5 kBtu/hr
b.	SHGC:		*	HSPF: 8.10
	(or Clear or Tint DEFAULT) 7b. (Clear) 263.0 ft ²		b. N/A	
8.	Floor types	_	31 1925	*
a.	Slab-On-Grade Edge Insulation R=0.0, 136.0(p)	ft	c. N/A	
	N/A	_	V. 1011	_
	N/A	-	14. Hot water systems	-
9.	Wall types	_	a. Electric Resistance	Cap: 50.0 gallons
a.	Concrete, Int Insul, Exterior R=4.1, 720.0 f	t ²	a. Diverse resistance	EF: 0.90
	Frame, Wood, Exterior R=13.0, 992.0 f		b. N/A	DI. 0.50
	Frame, Steel, Adjacent R=13.0, 159.0 f		W. 1911	
	N/A		c. Conservation credits	-
e.	N/A		(HR-Heat recovery, Solar	
10.	Ceiling types		DHP-Dedicated heat pump)	
	Under Attic R=19.0, 1120.0 f	12	15. HVAC credits	PT,
	N/A		(CF-Ceiling fan, CV-Cross ventilation,	11,
	N/A	100	HF-Whole house fan,	
	Ducts	_	PT-Programmable Thermostat,	
100000	Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 150.0	A	MZ-C-Multizone cooling,	
	N/A	_	MZ-H-Multizone heating)	
U.	17/11	_	wiz-ri-wuitzone neating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features

Builder Signature: (V

SN Mulberry Drivecity/FL zip: Lake CHY Address of New Home:

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTMdesignation). your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4 EnergyGauge® (Version: FLRCSB v4.5)

Duct System Summary

Entire House

Job: SUNBURY

Date:

By: G. CARMACK ...

MARONDA HOMES

4005 MARONDA WAY, SANFORD, FL 32771 Phone: (407) 321-0064

Project Information

For:

SUNBURY

	Не	eating		C	ooling
External static pressure		in H2O			in H2O
Pressure losses	0.00	in H2O		0.00	in H2O
Available static pressure	0.60	in H2O		0.60	in H2O
Supply / return available pressure	0.48 / 0.12	in H2O		0.48 / 0.12	in H2O
Lowest friction rate	1.935	in/100ft		1.935	in/100ft
Actual air flow	1240	cfm		1240	cfm
Total effective length (TEL)			31 ft		

Supply Branch Detail Table

Name		esign Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
KITCHEN	h	423 5	150	150	1.935	6	x00	VlFx	25.0	0.0	st1
FAMILY ROOM	h	424 1	140	140	1.935	6	\mathbf{x}	VIFx	25.0	0.0	ST2
DINING ROOM	h	399 1	150	150	1.935	6	\mathbf{x}	VIFx	25.0	0.0	st1
LIVING ROOM	c	3329	140	140	9.677	6	$\times 00$	VIFx	5.0	0.0	ST3
UTILITY ROOM	h	829	25	25	9.677	4	\mathbf{x}	VIFx	5.0	0.0	ST3
BATHROOM	c	143 2	25	25	9.677	4	\mathbf{x}	VIFx	5.0	0.0	ST3
MASTER BEDROOM	c	3303	140	140	3.226	6	\mathbf{x}	VIFx	15.0	0.0	ST2
MASTER BATHROOM	c	2516	100	100	3.226	5	\mathbf{x}	VIFx	15.0	0.0	st5
HALL BATHROOM	c	1109	25	25	4.839	4	\mathbf{x}	VIFx	10.0	0.0	ST2
TOILET	c	1023	25	25	3.226	4	x(0)	VlFx	15.0	0.0	st5
BEDROOM #3	h	4511	110	110	4.839	5	x00	VIFx	10.0	0.0	st1
BEDROOM #2	C	242 2	100	100	4.839	5	\mathbf{x}	VIFx	10.0	0.0	ST4
BEDROOM #4	c	268 1	110	110	4.839	5	x00	VlFx	10.0	0.0	st1

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	520	520	1.935	515	12	0 x 0	VinlFlx	
ST2	Peak AVF	620	620	1.935	613	12	0 x 0	VinlFlx	
ST3	Peak AVF	190	190	9.677	569	7	0 x 0	VinlFlx	st5
ST4	Peak AVF	100	100	4.839	510	5	0 x 0	VinlFlx	
st5	Peak AVF	315	315	3.226	632	10	0 x 0	VinlFlx	st2

Bold/Italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)		Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	1240	1240	6.0	1.935	568	18	x0	0		VlFx	



Project Summary Entire House **MARONDA HOMES**

Job: SUNBURY

G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: (407) 321-0064

Project Information

For:

SUNBURY

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Summer Design Conditions

Outside db Inside db Design TD	70 °	°F °F °F	Outside db Inside db Design TD Daily range Relative humidity	92 75 17 M 50	°F °F °F
			Moisture difference	52	gr/lb

Heating Summary

Sensible Cooling Equipment Load Sizing

Structure	39178	Btuh	Structure	19600	Btuh
Ducts	7466	Btuh	Ducts	9255	The state of the s
Central vent (100 cfm)	4048	Btuh	Central vent (100 cfm)	1860	Btuh
Humidification	0	Btuh	Blower	0	Btuh
Piping	0	Btuh			
Equipment load	50692	Btuh	Use manufacturer's data	n	
30 (200 (200 (200 (200 (200 (200 (200 (2			Rate/swing multiplier	0.97	1222001 107
Infiltration			Equipment sensible load	29794	Btuh

Method Simplified Construction quality Average

riiepiaces		U
Area (ft²)	Heating 2223	Cooling 2223
Volume (ft³)	17517	17517
Air changes/hour Equiv. AVF (cfm)	0.32 93	0.16 47

Latent Cooling Equipment Load Sizing

Structure Ducts Central vent (100 cfm) Equipment latent load	2842 2144 3516 8502	Btuh Btuh	
Equipment total load Reg. total capacity at 0.76 SHR	38295		

Heating Equipment Summary

Cooling Equipment Summary

Make Trade Model	TEMPSTAR HEAT PUMP N4H342AKA		
Tempe Actual : Air flow	g input g output rature rise air flow	0 0 1240 0.027	HSPF Btuh @ 47°F °F cfm cfm/Btuh in H2O

Make Trade Cond Coil	TEMPSTAR HEAT PUMP N4H342AKA FSU4X4200A		97
Efficiend Sensible	cy e cooling	30780	SEER Btuh
Latent cooling Total cooling Actual air flow		9720 40500 1240	Btuh Btuh cfm
Air flow	factor	0.043	cfm/Btuh
Load se	ressure nsible heat ratio	0.78	111120

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.



Building Analysis Entire House **MARONDA HOMES**

Job: SUNBURY

Date:

By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: (407) 321-0064

Project Information

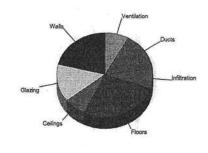
For:

SUNBURY

		Design Co	onditions		
Location: Gainesville, FL, US Elevation: 151 ft Latitude: 30°N Outdoor: Dry bulb (°F) Daily range (°F) Wet bulb (°F) Wind speed (mph)	Heating 33	Cooling 92 19 (M) 77 7.5	Indoor: Indoor temperature (°F) Design TD (°F) Relative humidity (%) Moisture difference (gr/lb) Infiltration: Method Construction quality Fireplaces	Heating 70 37 30 10.6 Simplified Average 0	Cooling 75 17 50 52.0

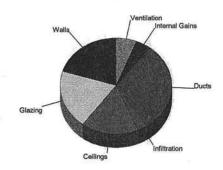
Heating

Component	Btuh/ft²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Piping Humidification Ventilation Adjustments	4.1 47.0 0.0 1.3 28.1 1.4	10723 7283 0 3724 13667 3782 7466 0 4048	21.2 14.4 0.0 7.3 27.0 7.5 14.7 0.0 0.0 8.0
Total	1.01	50692	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Ventilation Internal gains Blower Adjustments Total	2.4 38.9 0.0 1.8 0.0 0.3	6218 6036 0 5097 0 869 9255 1860 1380 0 0	20.2 19.7 0.0 16.6 0.0 2.8 30.1 6.1 4.5 0.0



Overall U-value = 0.160 Btuh/ft2-°F

Data entries checked.

Columbia County Building Department Culvert Permit

Culvert Permit No.

000001681

DATE $\frac{10/1}{}$	6/2008	PARCEL ID #	10-45-16-02856-130		
APPLICANT	PATRICK WILSO	N	РНО	NE 352 206	5-5459
ADDRESS 6	SOUTHPO	NT PARKWAY	JACKSONVI	LLE	FL 32216
OWNER M	ARONDA HOMES		PHO	NE 904 296	-1490
ADDRESS 30	04 SW MULBE	RRY DRIVE	LAKE CITY		FL 32055
CONTRACTO	R THEODORE B	ROCK	PHO	NE 904 296	6-1490
LOCATION O	F PROPERTY	90W, TL ON 247S, TR O	N 252B, TL TIMBER RID	GE, TL MULE	BERRY DR.,
7TH LOT ON RIG	НТ				
SUBDIVISION SIGNATURE	/LOT/BLOCK/F	HASE/UNIT TIMBE	RLANDS	30)
X	Culvert size wi	ION REQUIREMEN I be 18 inches in diame Both ends will be mite concrete slab.	eter with a total lenght	of 32 feet, le slope and p	eaving 24 feet of poured with a 4 inch
	 a) a majority b) the drivew Turnouts s concrete or 	N NOTE: Turnouts will of the current and exis ay to be served will be nall be concrete or pave paved driveway, which existing paved or con-	ting driveway turnouts paved or formed with ed a minimum of 12 f never is greater. The	are paved, concrete. eet wide or t	the width of the
	Culvert installa	ation shall conform to	the approved site plar	standards.	
	Department of	Transportation Permit	installation approved	standards.	
	Other			<u> </u>	
		3			
	-				

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



	J. J. J. J. J. J. J. J. J. J. J. J. J. J	and Page 1 of 1 B:1160 P:1552
NOTICE OF COMMENCEMENT		
Tax Parcel Identification Number 10-45-110-0285	2-130 County Clerk's Office Stam	p or Seal
THE UNDERSIGNED hereby gives notice that improvements will be a Florida Statutes, the following information is provided in this NOTICE	OF COAAR FURNISH PER IN.	The state of the s
1. Description of property (legal description): a) Street (job) Address: 2. General description of improvements:	Imbertands	
2. General description of improvements:	of a stragte farmly du	Jestina .
3. Owner Information a) Name and address: MIONA HOWES INCA P b) Name and address of fee simple titleholder (if other than or c) Interest in property 4. Contracts the feet of the simple titleholder (if other than or	LOOD Southpoint PKny #3	IN lay to 2004.
a) Name and address: MAYONGA HOMES INC & b) Telephone No. 100 2012 - 1490	FA USOD Southpoint Plany	#300. Jax 7232211
5. Surety information		
n) Name and address: b) Amount of Bond:		
b) Amount of Band: c) Telephone No.: 6. Lender	Fax No. (Opt.)	
a) Name and address:	and the same of th	•
b) Phone No. 7. Identity of person within the State of Florida designated by our arms.		
7. Identity of person within the State of Florida designated by owner upon a) Name and address: WATHEN THE HOWN b) Telephone No.: (1004) 739-2205	Fax No. (Opl.)	WE Rd Jax Fr 3221
8. In addition to himself, owner designates the following person to receive		
Pionoa Statutes:		
u) Name and address: b) Telephone No.:	Fee No. (O-4)	
	-	
Expiration date of Notice of Commencement (the expiration date is is specified):	ne year from the date of recording unless a d	ifferent date
WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWN COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENT STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR COMMENCEMENT MUST BE RECORDED AND POSTED ON TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN A YOUR NOTICE OF COMMENCEMENT.	S UNDER CHAPTER 713, PART I, SECTION IMPROVEMENTS TO YOUR PROPERTY HE JOR SITE REFORE THE FIRST INSPE	ON 713.13, FLORIDA
STATE OF FLORIDA	NCE	
COUNTY OF COLUMBIA 10	nature of Owner or Conor's Authorized Office/Dia	rector/Purtner/Manager
	Theodore C. Burk	· · · · · · · · · · · · · · · · · · ·
Pr	nt Name	
The foregoing instrument was acknowledged before me, a Plorida Notary. the The World C. BOCK as V.P. (A CON	truction	g. officer, trustice, attorney
Each for Morronda Homes inc of Plurida	(name of party on behalf of whom	
Personally Known OR Produced IdentificationType	Course of barry on benefit of whom	Notary Public State of Florida Melissa L McKague
Notary Signature MUWYA L. MCKaple	Notary Stamp or Scal:	My Commission DD493647 "; Expires 11/22/2009
11. Verification pursuant to Section 92.525, Florida Statutes. Under pursuant to Section 92.525, Florida Statutes. Under pursuant stated in it are true to the best of my knowledge and belief	No nalties of perjury, I declare that I have read th	e foregoing and that the

Signature of Natural Person Signing (in line #10 above.)

facts stated in it are true to the best of my knowledge and belief.





Engineering Consultants in Geotechnical • Environmental • Construction Materials Testing

#27431

FIELD DENSITY WORKSHEET

CLIENT MATOHDA H) my l=	2			DATE .		16	octo	8
			1 1		PROJE	CT N	10		
PROJECT NAME TIMBUR ZA	405	la	TH	30	PERMI	T NO		-	
EARTH CONTRACTOR	KL= C		7.5		TESTE	D BY		JIMI	
COMPACTION REQUIREMENT (%)	3540	□ Sta	andard F odified P					FIELD	
TOTAL ON-SITE TIME	1.1		1-		MILES	FRO	M OF	FICE	
□ Limerock □ Subgrade □ Pipe Backfill	Building	Pad /	Building	g Footing	Oth	ner _			
	LARPE	OCTOR				W	ET	DRY	
TEST LOCATION	DENS.	ОМС	TEST DEPTH	PROBE DEPTH	% MOIST.	100 march 200 M	ISITY CF)	DENSITY (PCF)	% COMP.
CARLUS PAP	1049	10.1	15/2		41.6	-		103.4	98,6
CTM. of E. Fth.))	1	1	5.8	-	_		97.7
CAR. US ILL. F. F.	1	1	5	4	5.3	2.2			98.0
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		Street, Square		180		_			
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	notice is I	required to	o be pos	tea. Any i ad-destrovi	ng organ	ism			
	chall noc	t notice	of said	treatment e attic or o	immedia	tely			
	other reac	dily access	ible area	of property	treated.	-			
		lot 3	80 4	mber	kno				
REMARKS					D C	Ct - 2		sity failed to mum projec	
	6	2		umbia Parl cksonville,				irement	
	Hon	neTea			904) 730-			st indicates	
	PES	T DEFENS	E*		888) 464-		dens	ity requiren	nent was
				Fax ((904) 730-	-3244		nt is aware	of
<u> </u>							unsa	tisfactory te	est results.

2603 NW 74TH PLACE • GAINESVILLE, FLORIDA 32033 • PHONE: (352) 3/2-1274 • FAX: (352) 372-2721



OCCUPANO

COLUMBIA COUNTY, FLORIDA

epartment of Building and Zoning I Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-4S-16-02856-130

Fire: 57.78

Building permit No. 000027431

Permit Holder THEODORE BROCK

Use Classification SFD,UTILITY

Owner of Building MARONDA HOMES

Waste: 150.75

208.53

Location: 304 SW MULBERRY DR., LAKE CITY, FL

Date: 01/13/2009

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)

Maronda Systems

Maronda Systems

4005 Maronda Way

Sanford FL 32771

MiTek software

(407) 321-0064

Fax (407) 321-3913

Engineer/Architect of Record: Design Criteria: TPI Desig

ecord: Tomas Ponce P.E.
Design: Matrix Analysis

367 Medallion PL.

Chuluota

PLAN JOB#	LOT	ADDRESS	DIV/SUB	MODEL
9TM03001	30-1	304 SW	JAX-9TM	SUNV3 LEFT

SUNBURY V

This structure was designed in accordance with, and meets the requirements of TPI standards and the FLORIDA 2004 BUILDING CODE for 125 M.P.H. Wind Zone. Truss loading is in accordance with ASCE 7-02. These trusses are designed for an enclosed building.

The Truss Engineering package for the above referenced site was generated by the Truss Designer/Architect/MiTek/Trenco.

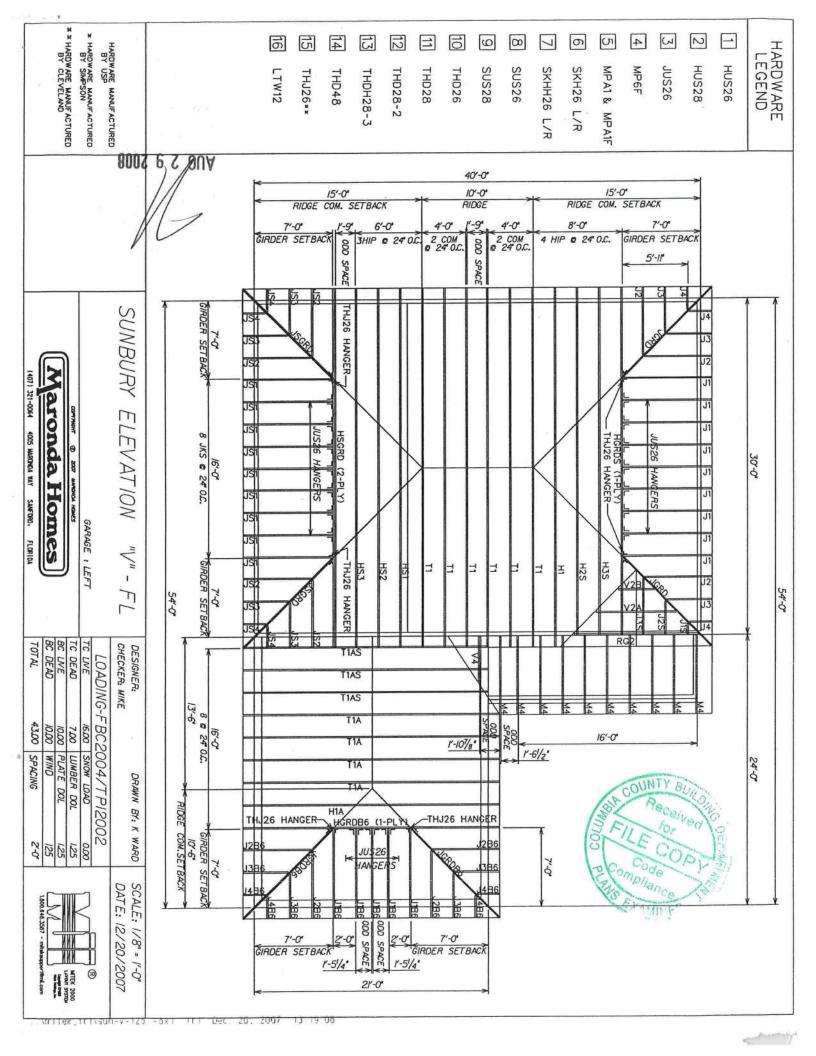
I, Tomas Ponce P.E.

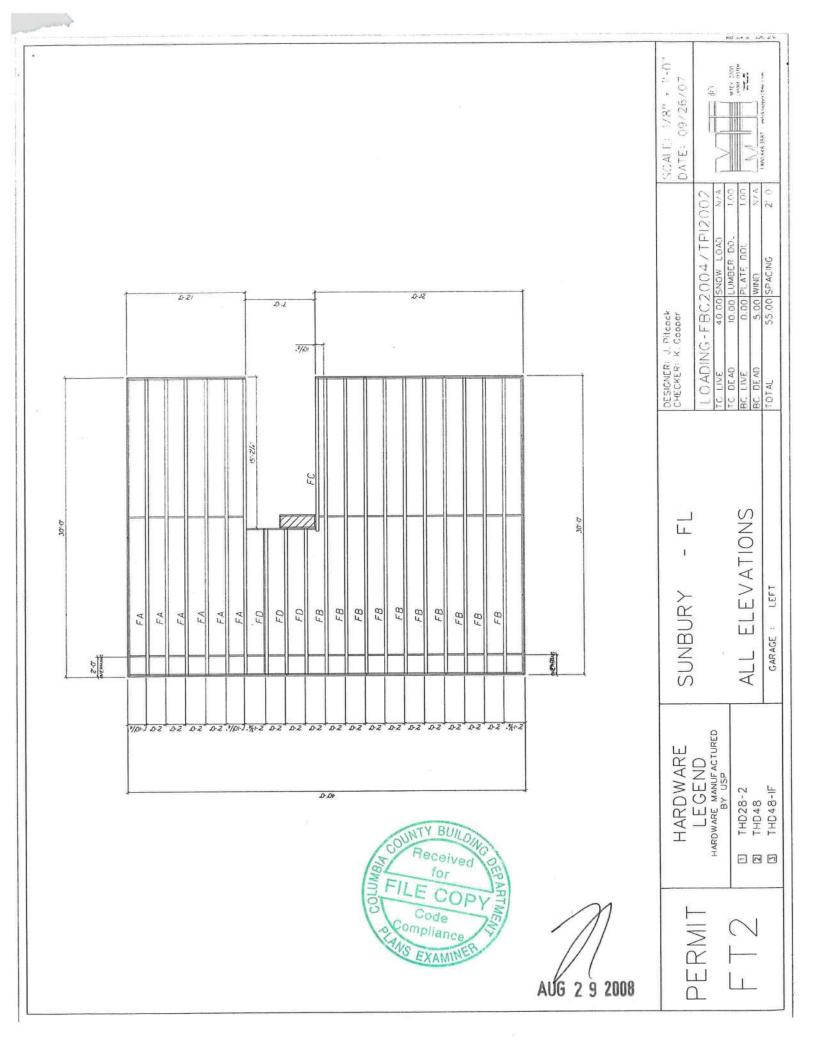
the Architect/Engineer of Record for the above referenced lot

Have reviewed the package and confirmed that it matches the physical and structural

Parameters found on the set of permit drawings.

Truss ID	Run Date	Drawing Reviewed	Truss ID	Run Date	Drawing Reviewed	No. of Eng. Dwgs: 40
Layout	12/20/07		V2A	12/27/07		Roof Loads-
V	07/27/05		V2B	12/27/07		TC Live: 16.0 psf
HIP	11/02/06		V4	12/27/07		TC Dead: 7.0 psf
H1	12/27/07					BC Live: 10.0 psf
H1A	12/27/07					BC Dead: 10.0 psf
H2S	12/27/07		Floor Layout	09/26/07		Total 43.0 psf
H3S	12/27/07		FA	12/13/07		
HGRDS	12/27/07		FB	12/13/07		DurFac- Lbr: 1.25
HGRDB6	12/27/07		FC	12/13/07		DurFac- Plt: 1.25
HS1	12/27/07		FD	06/23/08		O.C. Spacing: 24.0"
HS2	12/27/07		FE	12/13/07		
HS3	12/27/07					
HSGRD	12/27/07					Floor Loads-
J1	12/27/07					TC Live: 40.0 psf
J1B6	12/27/07					TC Dead: 10.0 psf
J1S	12/27/07					BC Live: 0.0 psf
J2	12/27/07		-			BC Dead: 5.0 psf
J2B6	12/27/07					Total 55.0 psf
J2S	12/27/07					DurFac- Lbr: 1.00
J3	12/27/07					DurFac-Plt 601-601
J3B6	12/27/07					O.C Spacing: 24.0"
J3S	12/27/07					for PA
J4	12/27/07					FILE COPY R
J4B6	12/27/07					
JGRD	12/27/07					Q Code ₹
JGRDB6	12/27/07					Compliance
JS1	12/27/07					ANS FXAMINES
JS2	12/27/07		INV#	DESC	QNTY	
JS3	12/27/07		50060.0114	THD48		
JS4	12/27/07		50060.0047	THD28		
JSGRD	12/27/07		50060.0110	JUS26	17	1
M4	12/27/07		50060.0058	THJ26	6	
RG2	12/27/07		50060.0049	THD28-2		
T1	12/27/07					1 // /
T1A	12/27/07					
T1AS	12/27/07		SEAT PLAT	ES	37	lug (
13.			FLOOR SEAT	PLATES	29	DATE: AUG 2 9 2000





MAN WAR IN COL BCOLBIONMAKA SHEEL - GOTDELOK III

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the <u>BCST-B3</u> <u>Summary Sheet — Permanent Restraint/Bracing of Chords & Web Members for more information.</u> All other permanent bracing design is the responsibility of the Building Designer.

personal Injury or death.

The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious

El resultado de un manejo, levantamiento, instalación, restricción y arrisotre incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.

Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

HANDLING — MANEJO

Avoid lateral bending. — Evitze la flexión lateral.

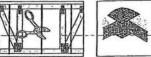
Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.

NOTAS GENERALES

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arriostre diagonal temporales. Use las recomendadones de manejo, instalación, restricción y arriostre temporal de los trusses. Vea el folleto <u>BCSI</u> Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Placas de Metal para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen BCSI-B3 — Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del Diseñador del Edificio.





HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES.

Warning! Don't overload the crane.

¡Advertencia! ¡No sobrecarga la grue!

Never use banding alone **to lift** a bundle.
Do not lift a group of individually banded bundles.
Nunca use sólo los empaques para levantar un paquete.
Ro levante un grupo de empaques individuales.

A single lift point may be **used** for bundles with trusses up to 45°. Two lift points may be used **for** bundles with

trusses up to 60'. Usé at least 3 lift points for bundles with trusses greater than 60'.

Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies. Puede usar dos puntos de levantar para

paquetes más de 60 pies. Use por lo menos tres puntos de levantar para paquetes más de 60 pies.



Warning! Do not over load supporting structure with truss bundle.

> ¡Advertendal No sobrecargue la estructura apoyada con el paquete de trusses.

Place truss bundles in stable position.

Puse paquetes de trusses en una posición estable.

INSTALLATION OF SINGLE TRUSSES BY HAND INSTALACIÓN POR LA MANO DE TRUSSES INDIVIDUALES

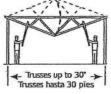
Trusses 20' or less, support at peak.

Levante del pico los trusses de 20 pies o menos.



Trusses 30° or less, support at quarter points.

Levante de los cuartos de tramo los trusses de 30 pies o menos.



HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUALI

Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fasteried to the bearing points.

Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalado y el truss está asegurado en los soportes.

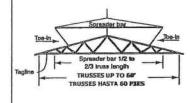
Warning! Using a single pick-point at the peak can damage the truss.

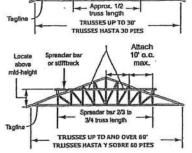
iAdvertencial El uso de un solo lugar para levantar en el pico puede hacer daño al truss.



HOISTING RECOMMENDATIONS FOR SINGLE

RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES

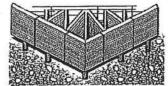




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The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra.



If trusses are to be stored horizontally, place blocking of sufficient height beneath the stack of trusses at 8' to 10' on center.

For trusses stored for more than one week, cover bundles to prevent moisture gain but allow for ventilation.

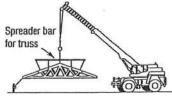
Refer to BCSI Guide to Good Practice for Handling. Installing. Restraining & Bracing. of Metal Plate Connected Wood Trusses for more detailed information pertaining to handling and jobsite storage of trusses.

Si los trusses estarán guardados horizontalmente, ponga bloqueando de altura suficiente detrás de la pila de los trusses.

Para trusses guardados por más de una sensana, cubra los paquetes para prevenir aumento de humedad pero permita venti-

Vea el folleto <u>BCSI Guía de Buena Práctica</u> nara el Maneia, Instalación, Restriccón y Arriostres de los Trusses de Madera Conectados con <u>Placas de Metal</u> para información más detallada sobre el manejo y almacenado de los trusses en área de trabajo. Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



Use proper rigglng and hoisting equipment.

Use equipo apropiado para levantar e improvisar.



Do not store unbraced bundles upright. No almacene verticalmente los trusses sueltos.



O Do not store on uneven ground.

No almacene en tierra desigual.



TEMPORARY RESTRAINT & BRACING RESTRICCIÓN Y ARRIOSTRE TEMPORAL

Refer to <u>BCSI-B2 Summary Sheet</u> – Truss. <u>Installation & Temporary Restraint/Bracing for</u> more information.

Vea el resumen <u>BCSI B2 — Restricción/</u>
<u>Arriostre Temporal y Instalación de los</u>
<u>Trusses</u> para más información.

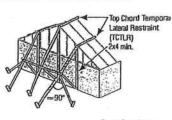
Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

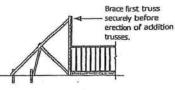
> Coloque los arriostres de tiema para el primer truss directamente en línea con Cada una de las filas de restricción lateral temporal de la cuerda superior (vea la table en la próxima columna).



Do not walk on unbraced trusses.

No camine en trusses sueltos.





ELLCANDLY ENGINEER OF THE WINDS AND THE PROPERTY OF THE PROPER

TEPS TO SETTING TRUSSES AS MEDIDAS DE LA INSTALLACIÓN DE LOS TRUSSES

- 1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). 7) Repeat process on groups of four trusses until all trusses are set.
- Instale los arriostres de tierra.
 Instale el primero truss y ate seguramente al arriostre de tierra.
 Instale los próximos cuatro trusses con restricción lateral temporal de miembro corto (vea abajo).
 Instale el arriostre diagonal de la cuerda superior (vea abajo).
 Instale arriostre diagonal para los planos de los miembros secundários para estable los primeros cinco trusses (vea abajo).
 Instale la restricción lateral temporal y arriostre diagonal para la cuerda inferior (vea abajo).
 Replta éste procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.
- A Refer to <u>BCSI-B2 Summary Sheet Truss Installation & Temporary Restraint/Bracing</u> for more information.

Vea el resúmen <u>BCSI-B2 - Instalación de Trusses y Arriostre Temporal</u> para mayor información.

ESTRAINT/BRACING FOR ALL PLANES OF TRUSSES RESTRICCIÓN/ARRIOSTRE EN TODOS PLANOS DE TRUSSES.

This restraint & bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.

Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas 3x2 y 4x2.

) TOP CHORD — CUERDA SUPERIOR

Truss Span	Top Chord Temporary Lateral Restraint (TCTLR) Spacing
Longitud de Tramo	Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30'	10' a.c. max,
Hasta 30 ples	10 ples máximo
30' to 45'	8° o.c. max.
30 a 45 pies	8 pies máximo
45' to 60'	6' α.c. max.
45 a 60 ples	6 pies máximo
60' to 80'*	4° o.c. max.
60 a 80 pies*	4 pies máximo

Consult a Professional Engineer for trusses longer than 60'.

Consulte a un Ingeriero para trusses de mas de 60 ples.

See BCSI-B2 for TCTLR options.

Vea el BCSI-B2 para las opciones de TCTLR.

Refer to BCSI-B3 Summary Sheet — Permanent.
Restraint/Bracing of Chords.

Restraint/Bracing of Chords

Web Members for Gable
End Frame restraint/bracing/
reinforcement information.

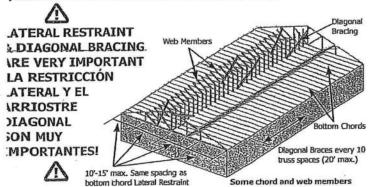
Para información sobre
restricción/arriostre/refuerzo
para armazón de bastial yea

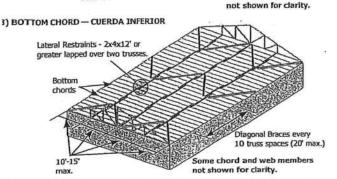
Para informacion sobre restricción/arriostre/refuerzo para armazón de hastial vea el resumen BCSI-B3 — Restricción/Arriostre. Permanente de Cuerdas y. Miembros Secundarios. Repeat diagonal braces for each set of 4 trusses.

Ground bracing not shown for clarity. Repita los arrisotres

Repita los arrisotres diagonales para cada grupo de 4 trusses.

) WEB MEMBER PLANE - PLANO DE LOS MIEMBROS SECUNDARIOS



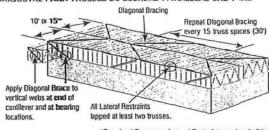


RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES
LA RESTRICCIÓN Y EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

Refer to BCSI-BZ
Summary Sheet
- Temporary & Permanent Restraint/
Bracing for Parallel
Chord Trusses for more Information.

more information.

Vea el resumen
8CSI-87 — Restricción y Arriostre
Temporal y.
Permanente de
Trusses de Cuerdas,
Paralelas para más
información.



*Top chord Temporary Lateral Restraint spacing shall be 10' o.c. max. for 3x2 chords and 15' o.c. for 4x2 chords.

INSTALLING — INSTALACION

Tolerances for Out-of-Plane.
Tolerancias para Fuera-de-Plano.



新聞	
D/50	D (ft.)
1/4"	ľ
1/2"	5.
3/4*	3.
1"	4'
1-1/4"	5
1-1/2*	6"
1-3/4*	7
2*	≥8,

	eo.	
	Max. Bow	Truss Length
H	3/4"	12.5'
1	7/8*	14.6"
1	1*	16.7
H	1-1/8"	18.8*
H	1-1/4"	20.8*
	1-3/8*	22.9'
H	1-1/2*	25.0
	1-3/4"	29.2
П	2-	≥33.3′

CONSTRUCTION LOADING - CARGA DE CONSTRUCCION

⚠ Do not proceed with construction until all lateral restraint and bracing is securely and properly in place.

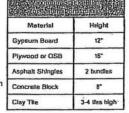
No proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma aproplada y segura.

O Do not exceed maximum stack heights. Refer to <u>BCSI-B4</u>.

<u>Summary Sheet - Construction Loading for more information.</u>

No exceda las máximas alturas recomendadas. Vea el resumen BCSI-B4 Carga de Construcción para mayor información.





O Do not overload small groups or single trusses.

No sobrecargue pequeños grupos o trusses individuales.

Never stack materials near a peak.

Nunca amontone material cerca del picco.

Place loads over as many trusses as possible.
Coloque las cargas sobre tantos trusses como sea posible.

Position loads over load bearing walls.

Coloque las cargas sobre las paredes soportantes.



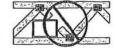
ALTERATIONS — ALTERACIONES

Refer to BCSI-B5 Summary Sheet - Truss Damage, Jobsite Modifications & Installation Errors.

Vea el resúmen BCSI-B5 Daños de trusses, Modificaciones en la Obra y Errores de Instalación,

O Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.

No corte, altere o perfore ningún miembro estructural de los trusses, a menos que esté especificamente permitido en el dibujo del diseño del truss.



Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin una autorización previa del Fabricante de Trusses, pueden reducir o eliminar la garantía del Fabricante de Trusses.

NOTE: The Thus Manufacturer and Truss Designer rely on the presumption that the Contractor and crane operator (If applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the Contractor believes it needs assistance in some paper of the construction project, it should seek assistance from a competent party. The methods and procedures outlined in this document are thinsided to resume that the meanal construction individues employed will put the trusses into plane SAFELY. These recommendations for handling, installing, restraining and bracing trusses are based upon the collective experience of leading personnel involved with trust seleging, manufacture and installation, but must, due to the nature of responsibilities involved, be presented only as a GuIDE for use by a qualified Building Designer or Contractor. It is not introded that these recommendations be interpreted as superior to the Building Designer's deskin presidention for leanning, installing, in serialing, in bracing manufactures and it does not predute the use of other equivalent methods for restraining/bracing and providing stability for the valls, columns, floors, roofs and all the intervalual structural building components as determined by the Contractors. Thus, WTCA and TPI appraisally dischain any responsibility for densages arising from the use, application, or reliance on the recommendations and information contained herein.



6300 Enterprise Lane - Madison, WI 53719



TRUSS PLATE INSTITUTE
218 N. Lee St., Ste. 312 • Alexandira, VA 22314
703/683-1010 • www.tplnst.org

B1WARN11x17 2006113



MARONDA SYSTEMS

4005 Maronda Way

Sanford, FL 32771

(407) 321-0064

Fax (407) 321-3913

Date: November 1, 2006

To:

Building Department

From: Maronda Systems

Tomas Ponce

Professional Engineer State of Florida #0050068

Subject: Valley Trusses

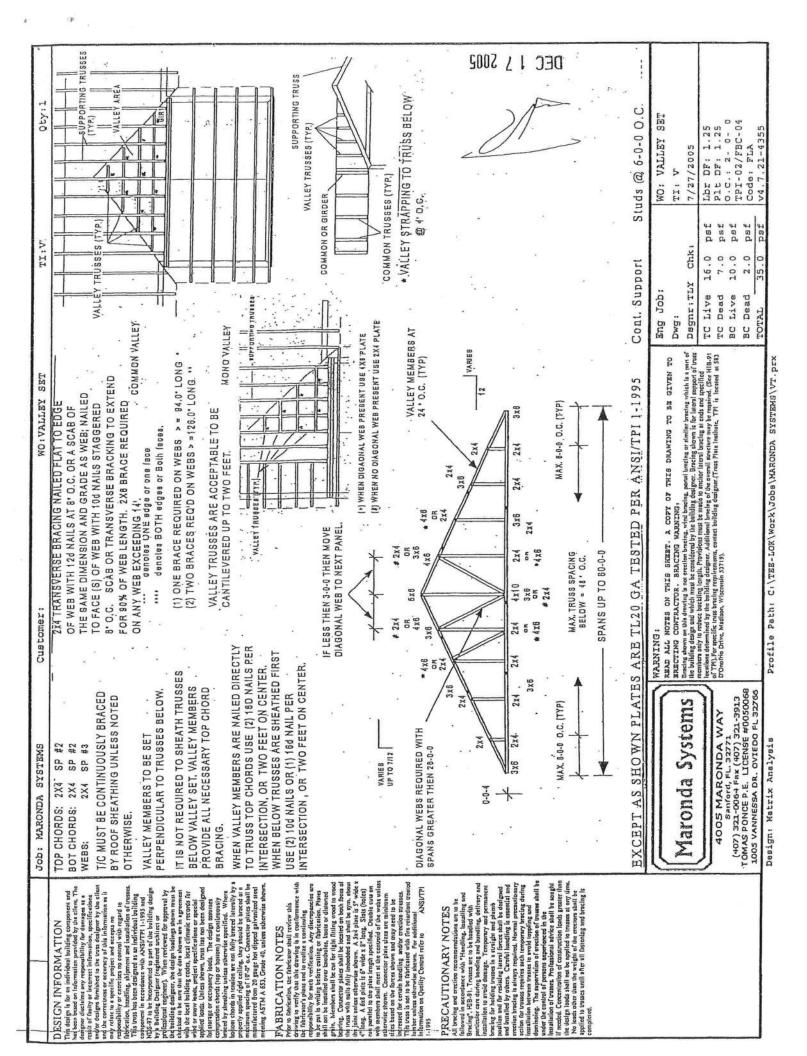
All valley trusses labeled V-1 through 100 are covered under the general valley sheet provided in the truss package signed and sealed by the engineer of record. The connections are noted on the structural info sheet of the plans. All criteria of the valley trusses are noted on the general sheet.

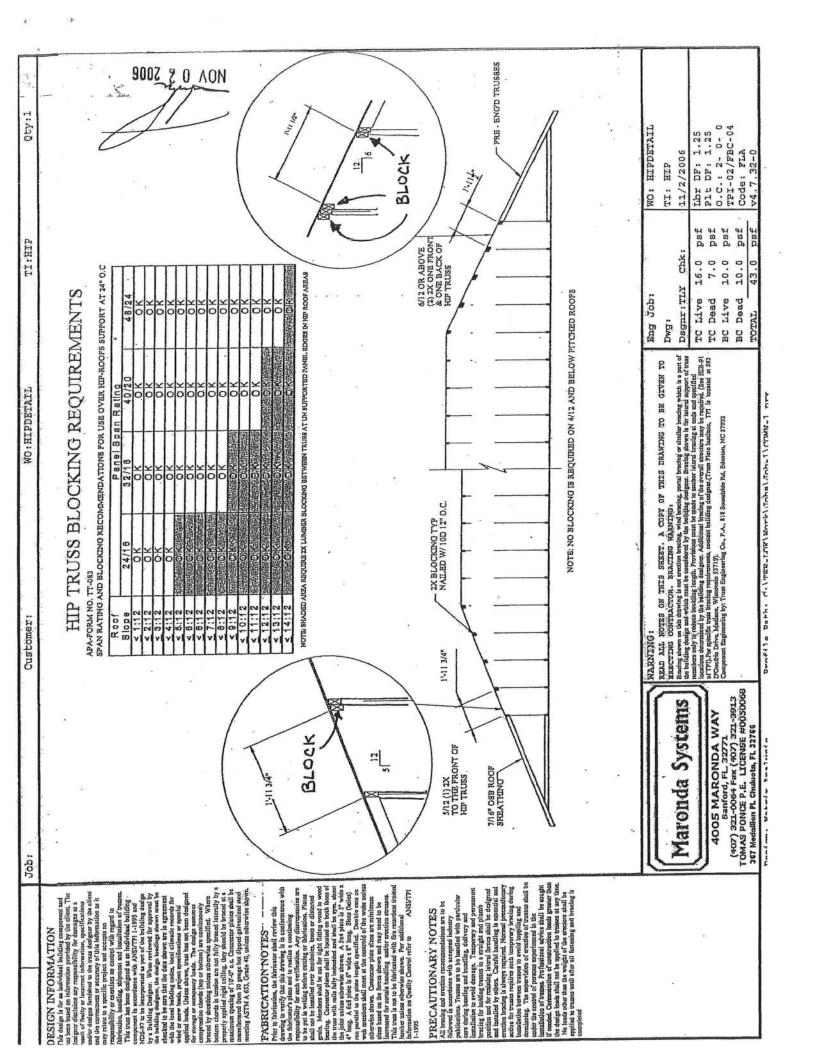
If you have any questions please feel free to call at 407-321-0064.

Sincerely,

Tomas Ponce, P.E.

Date: 11/1/06







Trenco

818 Soundside Rd Edenton, NC 27932

Re: SUNBURY

SUNBURY_FLORIDA_125

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Maronda Homes-Pittsburgh, PA.

Pages or sheets covered by this seal: E4585782

thru E4585895

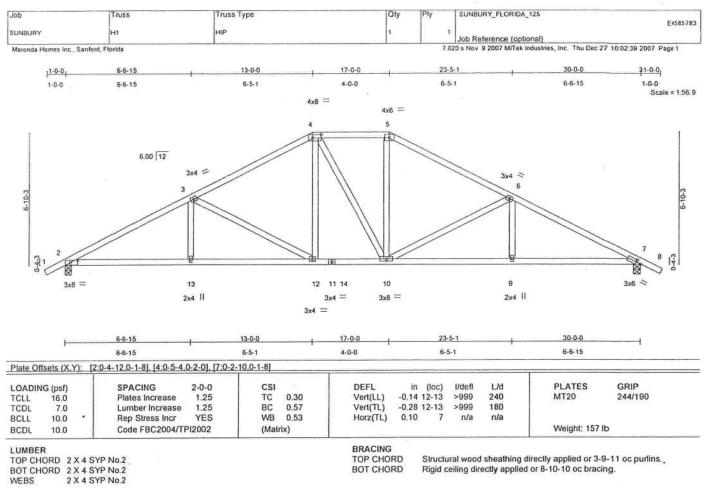
My license renewal date for the state of Florida is February 28, 2009.



December 27,2007

Strzyzewski, Marvin

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2. Engineering services provided by Truss Engineering Company.



REACTIONS (lb/size) 2=1364/0-4-0, 7=1360/0-4-0

Max Horz 2=123(LC 6)

Max Uplift2=-300(LC 6), 7=-300(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-2387/712, 3-4=-1747/608, 4-5=-1502/605, 5-6=-1738/608, 6-7=-2377/712, 7-8=0/21 TOP CHORD

2-13=-494/2080, 12-13=-494/2080, 11-12=-247/1510, 11-14=-247/1510, 10-14=-247/1510, 9-10=-494/2071,

BOT CHORD 7-9=-494/2071

3-13=0/295, 3-12=-654/279, 4-12=-67/538, 4-10=-140/107, 5-10=-67/517, 6-10=-653/279, 6-9=0/295

WEBS NOTES

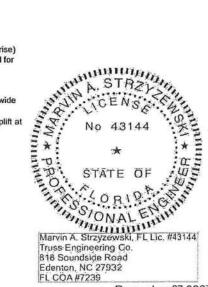
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5)* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 300 lb uplift at joint 2 and 300 lb uplift at joint 7.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permonent bracing of the overall structure is the responsibility of the didding designer. For general guidance regarding tobication, qualify control, storage, delivery, erection and bracing, consult. ANSI/TPI1 Quality Criteria, DS8-89 and &CS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Truss Type Otv SUNBURY_FLORIDA_125 loh Truss F4585784 HIP SUNBURY HIA Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:02:40 2007 Page 1 Maronda Homes Inc., Sanford, Florida 21-0-0 12-0-0 22-0-0 -1-0-0 4-5-10 9-0-0 4-5-6 3-0-0 4-5-6 4-6-10 1-0-0 4-6-10 Scale = 1:39.4 4x6 = 4x8 = 5 6.00 12 2x4 > 2x4 = 10 3x8 = 3x8 = 5x8 = 3x4 = 9-0-0 12-0-0 21-0-0 9-0-0 9-0-0 3-0-0 Plate Offsets (X,Y); [2:0-8-0.0-0-10], [5:0-5-4.0-2-0], [7:0-8-0.0-0-10], [10:0-2-12.0-3-0] DEFL L/d **PLATES** 2-0-0 CSI (loc) I/defi LOADING (psf) SPACING TC -0.15 7-9 240 MT20 244/190 1.25 0.32 Vert(LL) >999 TCLL 16.0 Plates Increase BC 0.54 -0.37 >665 180 1.25 Vert(TL) 7-9 TCDL 7.0 Lumber Increase WB 0.10 0.04 Rep Stress Incr YES Horz(TL) n/a BCLL 10.0 Weight: 104 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0 BRACING LUMBER TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 4-10-7 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.2

REACTIONS (lb/size) 2=939/0-8-0, 7=939/0-8-0

Max Horz 2=93(LC 6)

Max Uplift2=-237(LC 6), 7=-237(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-1394/484, 3-4=-1172/383, 4-5=-1000/389, 5-6=-1163/381, 6-7=-1392/485, 7-8=0/21 TOP CHORD 2-10=-304/1215, 9-10=-124/1002, 7-9=-304/1214 **BOT CHORD**

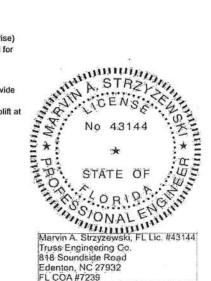
3-10=-243/198, 4-10=-17/345, 5-10=-91/82, 5-9=-11/343, 6-9=-250/199 WEBS

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 2 and 237 lb uplift at joint 7.

LOAD CASE(S) Standard



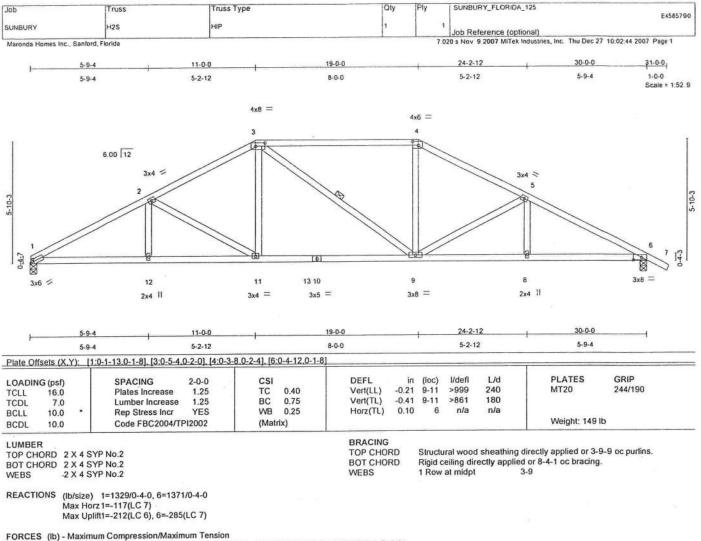
December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the undiding designer. For general guidance regarding distriction, quality control, storage, delivery, erection and bracing, consult.

ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.



Edenton, NC 27932



TOP CHORD 1-2=-2446/759, 2-3=-1998/679, 3-4=-1724/664, 4-5=-1958/675, 5-6=-2423/737, 6-7=0/21

BOT CHORD 1-12=-543/2128, 11-12=-543/2128, 11-13=-355/1761, 10-13=-355/1761, 9-10=-355/1761, 8-9=-521/2099,

6-8=-521/2099 WEBS 2-12=0/212, 2-

2-12=0/212, 2-11=-437/214, 3-11=-17/578, 3-9=-161/77, 4-9=-11/532, 5-9=-445/193, 5-8=0/213

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 212 lb uplift at joint 1 and 285 lb uplift at joint 6.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE U.S.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not fuss designer, Bracking shown is for lateral support of individual web members only. Additional temporary bracking to insure stability during construction is the responsibility of the erector. Additional permanent bracking of the overall structure is the responsibility of the building designer, for general guidance regarding tobication, qualify control storage, delivery, erection and bracking. AMSI/TRI Quality Citieria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



818 Soundside Road Edenton, NC 27932

SUNBURY FLORIDA_125 Truss Type Qty Job E4585793 SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:02;46 2007 Page 1 Maronda Homes Inc., Sanford, Florida 31-0-0 25-5-6 21-0-0 9-0-0 15-0-0 4-6-10 1-0-0 6-0-0 6-0-0 4-5-6 4-6-10 4-5-6 Scale = 1:53.0 4x6 = 4x6 = 3x8 = 5 3 6.00 12 2x4 = 0-4-7 3x8 = 10 12 11 3x8 = 3x8 = 3x4 2x4 || 21-0-0 30-0-0 15-0-0 9-0-0 9-0-0 6-0-0 9-0-0 6-0-0 Plate Offsets (X.Y): [1:0-8-0,0-0-8], [7:0-8-4,0-0-10] PLATES GRIP DEFL l/defl L/d LOADING (psf) SPACING 2-0-0 CSI (loc) 1-12 240 MT20 244/190 TC Vert(LL) -0.20 >999 0.38 16.0 Plates Increase 1.25 TCLL -0.45 >789 180 BC Vert(TL) 1-12 TCDL 7.0 Lumber Increase 1.25 0.81 0.10 n/a WB 0.36 Horz(TL) YES BCLL 10.0 Rep Stress Incr Weight: 149 lb (Matrix) Code FBC2004/TPI2002 BCDL 10.0 BRACING LUMBER Structural wood sheathing directly applied or 3-8-9 oc purlins. TOP CHORD 2 X 4 SYP No.2 TOP CHORD BOT CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 7-11-15 oc bracing. WEBS

2 X 4 SYP No.2

REACTIONS (lb/size) 1=1273/0-4-0, 7=1332/0-4-0 Max Horz 1=-102(LC 7)

Max Uplift1=-194(LC 6), 7=-267(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-2272/828, 2-3=-2057/720, 3-4=-1818/698, 4-5=-1820/690, 5-6=-2058/711, 6-7=-2276/809, 7-8=0/21

BOT CHORD

1-12=-617/1996, 11-12=-564/2107, 10-11=-564/2107, 9-10=-564/2107, 7-9=-594/2001

WEBS

2-12=-225/206, 3-12=-94/658, 4-12=-445/176, 4-10=0/168, 4-9=-441/178, 5-9=-86/657, 6-9=-228/192

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 194 lb uplift at joint 1 and 267 lb uplift at joint 7.

LOAD CASE(S) Standard



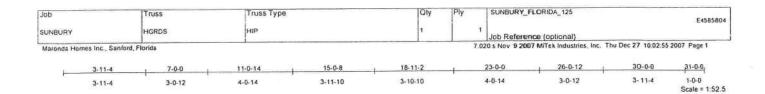
December 27,2007

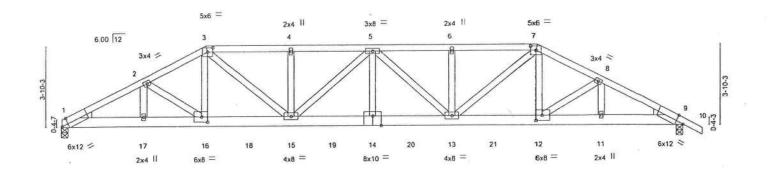
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer. For general guidance regarding tobarication, quality control stronger, delivery, erection and bracing, consult

ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onafro Drive, Madison, WI 53719.







3-11-4	3-0-12	4-0-14	3-11-10	3-10-10			4-0-14		3-0-12	3-11-4
Plate Offsets (X,Y): [1:0-3-13.Edge], [3:0-3-0.	0-2-7]. [7:0-3-0.	0-2-7], [9:0-4-6.Edge], [12:0-3-8,0-3-0].	[14:0-5-	0.Edge	e]. [16:0-3	3-8,0-3-0]		
LOADING (psf) TCLL 16.0 TCDL 7.0 BCLL 10.0	SPACING Plates Increase Lumber Increase Rep Stress Incr Code FBC2004/TF	2-0-0 1.25 1.25 NO Pl2002	CSI TC 0.58 BC 0.91 WB 0.38 (Matrix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in 0.40 -0.71 0.16	(loc) 14 14 9	l/defl >891 >502 n/a	L/d 240 180 n/a	PLA MT2	

BRACING

TOP CHORD

BOT CHORD

15-0-8

18-11-2

23-0-0

26-0-12

Structural wood sheathing directly applied or 2-1-14 oc purlins.

Rigid ceiling directly applied or 4-8-1 oc bracing.

30-0-0

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.1D 2 X 4 SYP No.2

WERS

WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

REACTIONS (lb/size) 1=2911/0-4-0, 9=2962/0-4-0

Max Horz 1=-88(LC 6)

Max Uplift1=-1220(LC 4), 9=-1245(LC 6)

7-0-0

FORCES (Ib) - Maximum Compression/Maximum Tension

 $1-2 = -5851/2534, \ 2-3 = -5830/2665, \ 3-4 = -6577/3043, \ 4-5 = -6577/3043, \ 5-6 = -6585/3048, \ 6-7 = -6585/3048, \ 7-8 = -5849/2674, \ 7-8$ TOP CHORD

8-9=-5874/2546, 9-10=0/23

1-17=-2252/5158, 16-17=-2252/5158, 16-18=-2325/5167, 15-18=-2325/5167, 15-19=-3196/7077, 14-19=-3196/7077, **BOT CHORD** 14-20=-3196/7077, 13-20=-3196/7077, 13-21=-2284/5183, 12-21=-2284/5183, 11-12=-2218/5189, 9-11=-2218/5189

2-17=-81/143, 2-16=-187/173, 3-16=-544/1179, 3-15=-913/1905, 4-15=-165/126, 5-15=-700/342, 5-14=-283/678,

5-13=-696/341, 6-13=-162/124, 7-13=-909/1893, 7-12=-545/1194, 8-12=-187/149, 8-11=-90/153

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

11-0-14

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1220 lb uplift at joint 1 and 1245 lb uplift
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 784 lb down and 461 lb up at 7-0-0, 243 lb down and 155 lb up at 9-0-12, 243 lb down and 155 lb up at 11-0-12, 243 lb down and 155 lb up at 13-0-12, 243 lb down and 155 lb up at 15-0-0, 243 lb down and 155 lb up at 16-11-4, 243 lb down and 155 lb up at 18-11-4, and 243 lb down and 155 lb up at 20-11-4, and 784 lb down and 461 lb up at 22-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2



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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the certain parameters and proper designer to the certain support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the exector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fobrication, quality control storage, delivery, erection and bracing, consult. ANSI/IPII Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, S83 D'Onatrio Drive, Modison, WI 53719.



Edenton, NC 27932

Job	Truss	Truss Type	Oty	Ply	SUNBURY_FLORIDA_125	
SUNBURY	HGRDS	HIP	1	1	E4	4585804
					Job Reference (optional)	

Maronda Homes Inc., Sanford, Florida

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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-46, 3-7=-46, 7-10=-46, 1-9=-40

Concentrated Loads (lb)

Vert: 16=-784(F) 15=-243(F) 14=-243(F) 13=-243(F) 12=-784(F) 18=-243(F) 19=-243(F) 20=-243(F) 21=-243(F)



Job Truss Truss Type Qty SUNBURY FLORIDA 125 SUNBURY HGRDB6 HIP Job Reference (optional) Maronda Homes Inc., Sanford, Florida 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:02:54 2007 Page 1 1-1-0-0 3-9-8 7-0-0 10-6-0 14-0-0 21-0-0 22-0-0 1-0-0 3-9-8 3-2-8 3-6-0 3-6-0 3-0-12 3-11-4 Scale = 1:39.0 4x8 = 2x4 || 4x8 = 5 6 6.00 12 3x4 > 13 12 17 11 10 3x10 = 3x10 = 2x4 II 4x6 = 6x8 = 3x8 = 6x8 = 2x4 ||

		3-9-8	3-2-8		3-6-0	0		3-6-0			3-0-12	3-11-4		
sets (X.Y.): [2:0	0-5-0.0-1-7]. [4:0-5-4.0-	-2-0], [6:0-5-4	.0-2-0]. [8:0	-5-0,0-	1-7). [1	;0-3-8,0-3-0], [13:0-3-8	3.0-3-0]			_		
G (psf)		SPACING	2-0-0	CSI	0.20		DEFL ·	in	(loc)	I/defl	L/d	PLATES	GRIP	
7.0		Lumber Increase	1.25	ВС	0.61		Vert(TL)	-0.24	12	>999	180	M120	244/190	
	•			WB (Mate		*	Horz(TL)	0.07	8	n/a	n/a	Weight: 128 lb		
	G (psf) 16.0	G (psf) 16.0 7.0 10.0	Seets (X,Y): [2:0-5-0.0-1-7], [4:0-5-4.0] G (psf) SPACING 16.0 Plates Increase 7.0 Lumber Increase 10.0 Rep Stress Incr	Sets (X.Y): [2:0-5-0.0-1-7], [4:0-5-4.0-2-0], [6:0-5-4] G (psf) 16.0 Plates Increase 1.25 7.0 Lumber Increase 1.25 Rep Stress Incr	Sets (X,Y): [2:0-5-0.0-1-7] [4:0-5-4.0-2-0] [6:0-5-4.0-2-0] [8:0-5-4.0-2-0]	Sets (X.Y): [2:0-5-0.0-1-7]. [4:0-5-4.0-2-0]. [6:0-5-4.0-2-0]. [8:0-5-0.0- G (psf) SPACING 2-0-0 CSI 16.0 Plates Increase 1.25 TC 0.29 7.0 Lumber Increase 1.25 BC 0.61 10.0 Rep Stress Incr NO WB 0.20	Sets (X,Y): [2:0-5-0.0-1-7]. [4:0-5-4.0-2-0]. [6:0-5-4.0-2-0]. [8:0-5-0.0-1-7]. [11 G (psf) SPACING 2-0-0 CSI 16.0 Plates Increase 1.25 TC 0.29 7.0 Lumber Increase 1.25 BC 0.61 10.0 Rep Stress Incr NO WB 0.20	Sets (X,Y): [2:0-5-0.0-1-7], [4:0-5-4.0-2-0], [6:0-5-4.0-2-0], [8:0-5-0.0-1-7], [11:0-3-8.0-3-0],	Sets (X,Y): [2:0-5-0.0-1-7], [4:0-5-4.0-2-0], [6:0-5-4.0-2-0], [8:0-5-0.0-1-7], [11:0-3-8.0-3-0], [13:0-3-8], [1	Sets (X.Y): [2:0-5-0.0-1-7]. [4:0-5-4.0-2-0]. [6:0-5-4.0-2-0]. [8:0-5-0.0-1-7]. [11:0-3-8.0-3-0]. [13:0-3-8.0-3-0]. [13:0-3-8.0-3-0]. [13:0-3-8.0-3-0]. [13:0-3-8.0-3-0]. [13:0-3-8.0-3-0]. [13:0-	Sets (X,Y): [2:0-5-0.0-1-7], [4:0-5-4.0-2-0], [6:0-5-4.0-2-0], [8:0-5-0.0-1-7], [11:0-3-8.0-3-0], [13:0-3-8.0-3-0] G (psf)	Sets (X,Y): [2:0-5-0.0-1-7], [4:0-5-4.0-2-0], [6:0-5-4.0-2-0], [8:0-5-0.0-1-7], [11:0-3-8.0-3-0], [13:0-3-8.0-3-0]	Sets (X,Y): [2:0-5-0.0-1-7], [4:0-5-4.0-2-0], [6:0-5-4.0-2-0], [8:0-5-0.0-1-7], [11:0-3-8.0-3-0], [13:0-3-8.0-3-0]	Sets (X,Y): [2:0-5-0.0-1-7], [4:0-5-4.0-2-0], [6:0-5-4.0-2-0], [8:0-5-0.0-1-7], [11:0-3-8.0-3-0], [13:0-3-8.0-3-0]

BRACING

TOP CHORD

BOT CHORD

14-0-0

17-0-12

Structural wood sheathing directly applied or 3-1-4 oc purlins.

Rigid ceiling directly applied or 6-6-8 oc bracing.

21-0-0

10-6-0

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.2

2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 2=2027/0-8-0, 8=2027/0-8-0

Max Horz 2=79(LC 5)

Max Uplift2=-876(LC 5), 8=-876(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/23, 2-3=-3764/1545, 3-4=-3652/1636, 4-5=-3680/1681, 5-6=-3680/1681, 6-7=-3649/1637, 7-8=-3770/1552

8-9=0/23

2-15=-1365/3281, 14-15=-1365/3281, 13-14=-1365/3281, 13-16=-1437/3291, 12-16=-1437/3291, 12-17=-1388/3290, **BOT CHORD**

11-17=-1388/3290, 10-11=-1321/3287, 8-10=-1321/3287

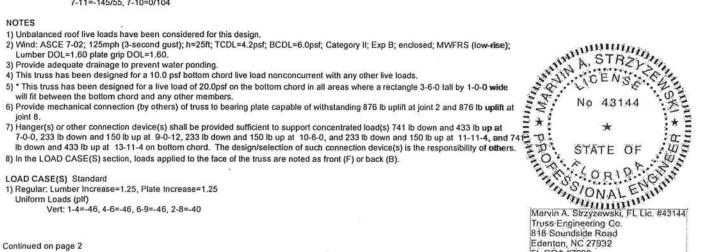
WEBS 3-15=0/107, 3-13=-149/64, 4-13=-473/1055, 4-12=-320/602, 5-12=-127/103, 6-12=-319/603, 6-11=-477/1058, 7-11=-145/55, 7-10=0/104

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise);

Continued on page 2



FL COA #7239

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Miles connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shows is for lateral support of individual web members only. Additional temporary bracing to insure stability of unique construction is the responsibility of the erector. Additional permanent bracing of the overal structure is the responsibility of the building designer, for general guidance regarding lobrication, quality control, storage, delivery, erection and bracing, consult. ANSI/ITI (quality Criteria, DSS-89 and 8CSI) Building Component Safety Information available from truss Plate Institute, 583 D'Onatrio Drive, Madison, WI 53719.



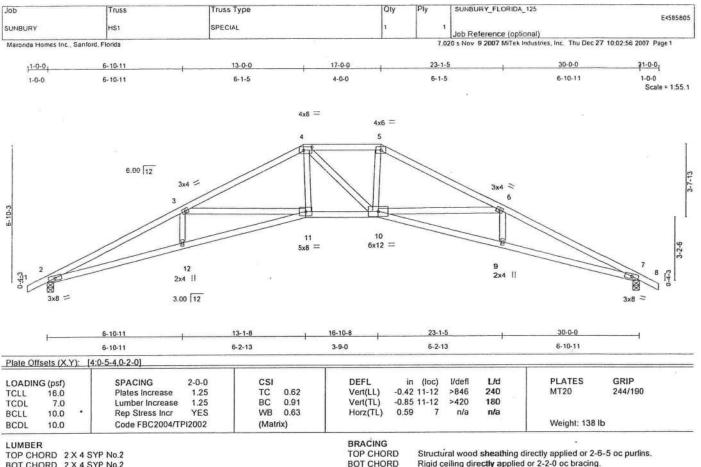
Job	Truss	Truss Type	Qty	Ply	SUNBURY_FLORIDA_125	
	. Icanana				. E450	85802
SUNBURY	HGRDB6	HIP	1		Job Reference (optional)	

Maronda Homes Inc., Sanford, Florida

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LOAD CASE(S) Standard Concentrated Loads (ib) Vert: 13=-741(B) 12=-233(B) 11=-741(B) 16=-233(B) 17=-233(B)





BOT CHORD 2 X 4 SYP No.2

WEBS REACTIONS 2 X 4 SYP No.2

(lb/size) 2=1329/0-4-0, 7=1329/0-4-0 Max Horz 2=-121(LC 7)

Max Uplift2=-296(LC 6), 7=-296(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-4252/1190, 3-4=-3118/864, 4-5=-2821/846, 5-6=-3172/875, 6-7=-4249/1191, 7-8=0/18

2-12=-955/3873, 11-12=-954/3859, 10-11=-493/2824, 9-10=-956/3862, 7-9=-955/3871 **BOT CHORD**

3-12=0/309, 3-11=-1005/441, 4-10=-165/156, 6-10=-959/432, 6-9=0/285, 4-11=-188/1150, 5-10=-194/1165 WEBS

NOTES

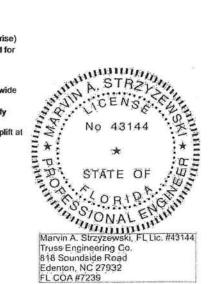
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 2, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 2 and 296 lb uplift at joint 7.

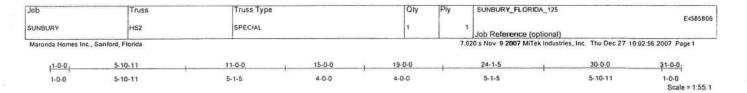
LOAD CASE(S) Standard

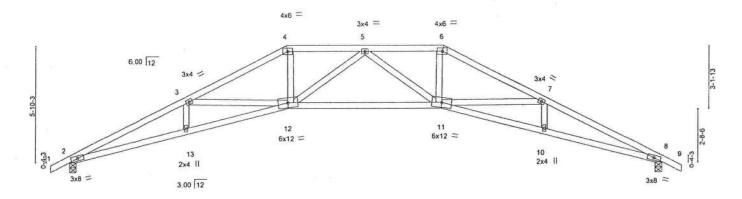


December 27,2007

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	1	5-1	0-11	11-1-8			18-10-8		24-1-	5	30-0-0	
		5-1	0-11	5-2-13			7-9-0		5-2-1		5-10-11	
LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in (loc	I/defl	L/d	PLATES	GRIP
TCLL	16.0		Plates Increase	1.25	TC	0.46	Vert(LL)	-0.47 11-12	>755	240	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.95	Vert(TL)	-0.97 11-12	>368	180		
BCLL	10.0	•	Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.59	n/a	n/a		
BCDL	10.0		Code FBC2004/TF	P12002	(Mati	rix)	1				Weight: 138 li	b

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.2

BRACING TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 2-8-8 oc purlins. Rigid ceiling directly applied or 2-2-0 oc bracing, Except: 7-3-3 oc bracing: 11-12.

REACTIONS (lb/size) 2=1329/0-4-0, 8=1329/0-4-0

Max Horz 2=-105(LC 7)

Max Uplift2=-281(LC 6), 8=-281(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-4269/1246, 3-4=-3541/1012, 4-5=-3156/962, 5-6=-3156/962, 6-7=-3541/1012, 7-8=-4269/1246, 8-9=0/18

TOP CHORD **BOT CHORD** 2-13=-1015/3872, 12-13=-1013/3881, 11-12=-753/3255, 10-11=-1013/3881, 8-10=-1015/3872

3-13=0/195, 3-12=-633/327, 5-12=-252/187, 5-11=-252/186, 7-11=-633/327, 7-10=0/195, 6-11=-254/1328, WEBS

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 281 lb uplift at joint 2 and 281 lb uplift at joint 8.

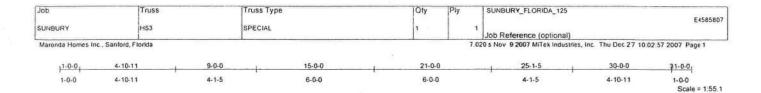
LOAD CASE(S) Standard

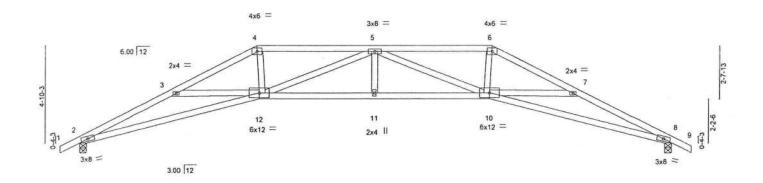


December 27,2007

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1	9-1-8	15-0-0	20-10-8	30-0-0			
	9-1-8	5-10-8	5-10-8	9-1-8			
OADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) I	/defi L/d PLATES	GRIP		
TCLL 16.0	Plates Increase 1.25	TC 0.49	Vert(LL) -0.44 11 >	802 240 MT20	244/190		
TCDL 7.0	Lumber Increase 1.25	BC 0.93	Vert(TL) -0.88 11 >	406 180			
BCLL 10.0 *	Rep Stress Incr YES	WB 0.41	Horz(TL) 0.58 8	n/a n/ a			
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)	**************************************	Weight: 13	17 lb		

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

10-12 2 X 4 SYP No.1D

WEBS

2 X 4 SYP No.2

REACTIONS (lb/size) 2=1329/0-4-0, 8=1329/0-4-0

Max Horz 2=-90(LC 7) Max Uplift2=-263(LC 6), 8=-263(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-4104/1344, 3-4=-3868/1137, 4-5=-3511/1084, 5-6=-3511/1084, 6-7=-3868/1137, 7-8=-4104/1344, TOP CHORD 8-9=0/18

2-12=-1111/3750, 11-12=-1086/4058, 10-11=-1086/4058, 8-10=-1111/3750 **BOT CHORD**

3-12=-177/287, 5-12=-716/290, 5-11=0/170, 5-10=-716/289, 7-10=-177/297, 4-12=-276/1461, 6-10=-276/1461 WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 263 lb uplift at joint 2 and 263 lb uplift at joint 8.

LOAD CASE(S) Standard



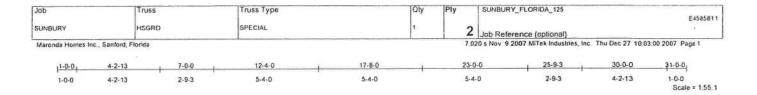
Structural wood sheathing directly applied or 2-7-14 oc purlins.

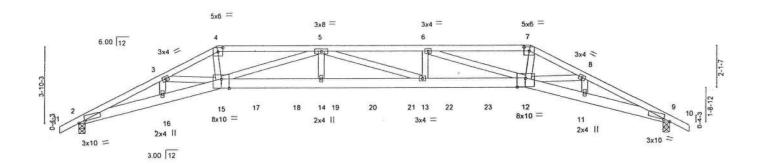
Rigid ceiling directly applied or 2-2-0 oc bracing.

December 27,2007

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		4-2-13	3-0-3		5-1-0		5-4-0	5-1	1-0	3-0	3 4-2-1	3
late Off	sets (X.)): [2:0	-3-14,0-0-6], [4:0-3-0,	0-2-7], [7:0-	3-0.0-2-7], [9:	0-3-14,0-0	6], [12:0-4-12,0-5-	4]. [15:0-4-12.	dge]			
LOADING			SPACING	2-0-0	CSI		DEFL	in (loc)	I/defl	L∕d	PLATES	GRIP
CLL	16.0		Plates Increase	1.25	TC	0.80	Vert(LL)	0.74 13-14	>481	240	MT20	244/190
TCDL	7.0	- 1	Lumber Increase	1.25	BC	0.94	Vert(TL)	-1.31 13-14	>272	180		
BCLL	10.0		Rep Stress Incr	NO	WB	0.44	Horz(TL)	0.65 9	n/a	n/a		
BCDL	10.0		Code FBC2004/TF	212002	(Mati	ix)				40000.00	Weight: 327 I	b

BRACING

TOP CHORD

BOT CHORD

17-8-0

22-9-0

25-9-3

Structural wood sheathing directly applied or 2-11-4 oc purlins.

Rigid ceiling directly applied or 7-1-14 oc bracing.

30-0-0

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D WEBS

4-2-13

2 X 4 SYP No.2

REACTIONS (lb/size) 2=2963/0-4-0, 9=2963/0-4-0

Max Horz 2=78(LC 5)

Max Uplift2=-1259(LC 5), 9=-1259(LC 6)

7-3-0

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/22, 2-3=-10338/4556, 3-4=-11145/5131, 4-5=-10559/4887, 5-6=-13646/6285, 6-7=-10595/4855, 7-8=-11184/5097

8-9=-10327/4507, 9-10=0/22

2-16=-4140/9363, 15-16=-4197/9483, 15-17=-6336/13789, 17-18=-6336/13789, 14-18=-6336/13789, **BOT CHORD**

12-4-0

14-19=-6336/13789, 19-20=-6336/13789, 20-21=-6336/13789, 13-21=-6336/13789, 13-22=-6218/13646,

22-23=-6218/13646, 12-23=-6218/13646, 11-12=-4102/9478, 9-11=-4043/9353

3-16=-387/280, 3-15=-633/1001, 5-15=-3492/1705, 5-14=-378/904, 5-13=-225/124, 6-13=-357/848, 6-12=-3305/1615, WEBS

8-12=-663/1047, 8-11=-410/290, 4-15=-2160/4753, 7-12=-2138/4755

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.

5) Provide adequate drainage to prevent water ponding.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Bearing at joint(s) 2, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1259 lb uplift at joint 2 and 1259 lb uplift at joint 9.

Continued on page 2



December 27,2007

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ANSI/TP11 Quality Criteria, DS8-89 and BCS11 Building Component Safety Information. available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Oty	Ply	SUNBURY_FLORIDA_125	E4585811
SUNBURY	HSGRD	SPECIAL	1	2	Job Reference (optional)	24303011

Maronda Homes Inc., Sanford, Florida

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10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 784 lb down and 467 lb up at 7-3-0, 243 lb down and 157 lb up at 11-0-12, 243 lb down and 157 lb up at 15-0-0, 243 lb down and 157 lb up at 15-0-0, 243 lb down and 157 lb up at 18-11-4, and 243 lb down and 157 lb up at 20-11-4, and 784 lb down and 467 lb up at 22-9-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-4=-46, 4-7=-46, 7-10=-46, 2-15=-40, 12-15=-40, 9-12=-40

Concentrated Loads (lb)

Vert: 15=-784(B) 12=-784(B) 17=-243(B) 18=-243(B) 19=-243(B) 20=-243(B) 21=-243(B) 22=-243(B) 23=-243(B)



Qty SUNBURY_FLORIDA_125 Truss Truss Type E4585813 14 JACK SUNBURY Job Reference (optional) Maronda Homes Inc., Sanford, Florida 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:02 2007 Page 1 -1-0-0 4-4-12 7-0-0 1-0-0 4-4-12 2-7-4 Scale: 1/2"=1" 2x4 || 3 6.00 12 2×4 = 2x4 || LOADING (psf) **PLATES** GRIP SPACING CSI L/d 2-0-0 (loc) I/defl 244/190 TC Vert(LL) -0.10 2-4 >765 240 MT20 Plates Increase 1.25 0.44 TCLL 16.0 0.42 -0.26 >306 180 1.25 BC Vert(TL) TCDI 7.0 Lumber Increase 0.03 0.00 n/a n/a YES WB Horz(TL) BCLL 10.0 Rep Stress Incr Code FBC2004/TPI2002 Weight: 29 lb BCDL 10.0

BRACING

TOP CHORD BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.2

REACTIONS (lb/size) 2=349/0-4-0, 4=282/Mechanical

Max Horz 2=176(LC 6)

Max Uplift2=-111(LC 6), 4=-100(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-105/54

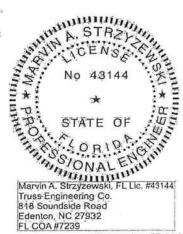
BOT CHORD 2-4=0/0

WEBS 3-4=-149/173

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 2 and 100 lb uplift at joint 4.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

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818 Soundside Roa Edenton, NC 27932

SUNBURY_FLORIDA_125 Job Truss Truss Type E4585815 14 SUNBURY J186 Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10.03:03 2007 Page 1 Maronda Homes Inc., Sanford, Florida 7-0-0 -1-0-0 1-0-0 4-4-12 2-7-4 Scale: 1/2"=1" 3 6.00 12 2x4 || 2x4 = **PLATES** GRIP CSI DEFL SPACING L/d LOADING (psf) (loc) I/defl 2-0-0 244/190 Vert(LL) -0.09 2-4 >825 240 MT20 1 25 TC 0.40 TCLL 16.0 Plates Increase BC -0.24 >330 180 1.25 0.40 Vert(TL) TCDL 70 Lumber Increase 0.03 0.00 n/a n/a WB Horz(TL) BCLL 10.0 Rep Stress Incr YES Code FBC2004/TPI2002 Weight: 29 lb (Matrix) 10.0 BCDL

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 2=352/0-8-0, 4=273/Mechanical Max Horz 2=176(LC 6)

Max Uplift2=-120(LC 6), 4=-97(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=-104/53

BOT CHORD 2-4=0/0

WEBS 3-4=-143/167

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 120 lb uplift at joint 2 and 97 lb uplift at joint 4.

6)

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

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Job		Truss	Truss Type	Oty	Ply	SUNBURY_FLORIDA_125	4585820
SUNB	URY	J1S	JACK	1	1	Job Reference (optional)	

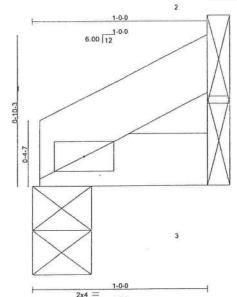
Maronda Homes Inc., Sanford, Florida

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Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Scale = 1:6:2



GRIP **PLATES** DEFL in -0.00 LOADING (psf) SPACING 2-0-0 CSI (loc) I/defl 1/d 240 MT20 244/190 >999 16.0 Plates Increase 1.25 TC 0.01 Vert(LL) TCLL -0.00 >999 180 Lumber Increase 1.25 BC 0.01 Vert(TL) TCDL 7.0 -0.00 n/a 10.0 Rep Stress Incr YES WB 0.00 Horz(TL) n/a BCLL Weight: 3 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=39/0-4-0, 3=18/Mechanical, 2=21/Mechanical

Max Horz 1=22(LC 6)

Max Uplift1=-3(LC 6), 2=-22(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-17/8

BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections,
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3 lb uplift at joint 1 and 22 lb uplift at joint
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 2 and 3.

LOAD CASE(S) Standard



December 27,2007

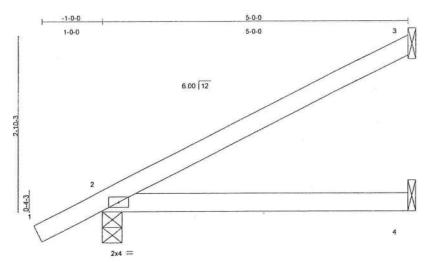
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SUNBURY FLORIDA 125 Qty Joh Truss Truss Type E4585821 SUNBURY J2 JACK Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:05 2007 Page 1

Maronda Homes Inc., Sanford, Florida

Scale = 1:17.8



5-0-0 **PLATES** GRIP LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl 1.25 TC 0.21 Vert(LL) -0.03 2-4 >999 240 MT20 244/190 TCLL 16.0 Plates Increase 1.25 BC 0.22 Vert(TL) -0.072-4 >842 180 TCDL 7.0 Lumber Increase Rep Stress Incr YES WB 0.00 Horz(TL) -0.003 n/a n/a BCLL 10.0 Weight: 18 lb 10.0 Code FBC2004/TPI2002 (Matrix) BCDI

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD **BOT CHORD**

5-0-0

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=269/0-4-0, 4=95/Mechanical

Max Horz 2=136(LC 6)

Max Uplift3=-95(LC 6), 2=-104(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-74/37

BOT CHORD

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 95 lb uplift at joint 3 and 104 lb uplift at ioint 2.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

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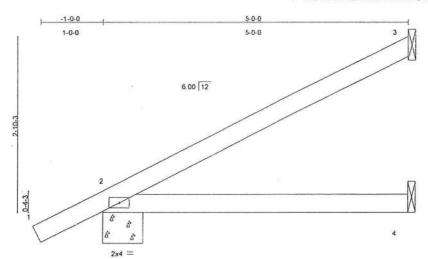


Job	Truss	Truss Type	Qty	Ply	SUNBURY_FLORIDA_125	E4666000
SUNBURY	J2B6	JACK	7	1	Job Reference (optional)	E4585823

Maronda Homes Inc., Sanford, Florida

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Scale = 1:17.8



5-0-0 CSI DEFL **PLATES** GRIP (loc) I/defl L/d TC Vert(LL) -0.02 2-4 >999 240 MT20 244/190

Lumber Increase 1.25 BC 0.20 Vert(TL) -0.06 2-4 >937 180 TCDL 7.0 BCLL 10.0 Rep Stress Incr YES WB 0.00 Horz(TL) -0.003 n/a n/a Code FBC2004/TPI2002 BCDL 10.0 (Matrix)

Weight: 18 lb

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER

TCLL

LOADING (psf)

TOP CHORD 2 X 4 SYP No.2

16.0

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=96/Mechanical, 2=272/0-8-0, 4=92/Mechanical Max Horz 2=136(LC 6)

2-0-0

1.25

Max Uplift3=-91(LC 6), 2=-114(LC 6)

SPACING

Plates Increase

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-72/36

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; ·Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 3 and 114 lb uplift at ioint 2.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

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SUNBURY_FLORIDA_125 Job Truss Truss Type F4585828 JACK SUNBURY J25 Job Reference (optional)

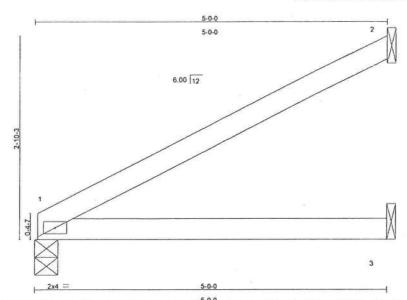
Maronda Homes Inc., Sanford, Florida

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Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Scale = 1.15.4



CSI DEFL **PLATES** GRIP LOADING (psf) SPACING 2-0-0 (loc) I/defl I /d -0.03 Vert(LL) TC. 1-3 >999 240 MT20 244/190 TCLL 16.0 Plates Increase 1.25 0.23 -0.07 >865 180 BC 0.21 1-3 TCDL 7.0 Lumber Increase 1.25 Vert(TL) WB 0.00 Horz(TL) -0.00 2 n/a BCLL 10.0 Rep Stress Incr YES n/a Weight: 16 lb Code FBC2004/TPI2002 (Matrix) 10.0 BCDL

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=203/0-4-0, 2=109/Mechanical, 3=95/Mechanical

Max Horz 1=104(LC 6)

Max Uplift1=-22(LC 6), 2=-106(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-81/41

BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 1 and 106 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 2 and 3.

LOAD CASE(S) Standard



December 27,2007

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SUNBURY FLORIDA 125 Truss Type Qty Job E4585829 JACK SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:06 2007 Page Maronda Homes Inc., Sanford, Florida 1-0-0 3-0-0 3-0-0 1-0-0 Scale = 1:10.8 6.00 12 0-4-3 3-0-0 3-0-0 GRIP PLATES LOADING (psf) SPACING DEFL (loc) 2-0-0 1.25 TC 0.12 Vert(LL) -0.00 2-4 >999 240 MT20 244/190 Plates Increase TCLL 16.0

LUMBER

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

7.0

10.0

BRACING

Vert(TL)

Horz(TL)

-0.01

-0.00

2-4 >999

> 3 n/a

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 11 lb

180

n/a

REACTIONS (lb/size) 3=51/Mechanical, 2=188/0-4-0, 4=55/Mechanical Max Horz 2=95(LC 6) Max Uplift3=-44(LC 6), 2=-101(LC 6)

Lumber Increase

Code FBC2004/TPI2002

Rep Stress Incr

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=-36/18

BOT CHORD 2-4=0/0

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

BC 0.07

WB 0.00

(Matrix)

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1.25

YES

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 3 and 101 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

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ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Salety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Modison, WI 53719.



SUNBURY_FLORIDA_125 Truss Truss Type Job E4585831 JACK J3B6 SUNBURY Job Reference (optional)
7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:09 2007 Page 1 Maronda Homes Inc., Sanford, Florida 1-0-0 3-0-0 3-0-0 1-0-0 Scale = 1:10.8 6.00 12 0-4-3 Ġ Δ° а Ó° 3-0-0 3-0-0

BCDL LUMBER

TCLL

TCDL

BCLL

LOADING (psf)

TOP CHORD 2 X 4 SYP No.2

16.0

7.0

100

10.0

BOT CHORD 2 X 4 SYP No.2

BRACING

DEFL

Vert(LL)

Vert(TL)

Horz(TL)

in

-0.00

-0.01

-0.00

(loc)

2-4 >999

2-4

3 n/a

I/defl

>999

L/d

240

180

n/a

TOP CHORD **BOT CHORD** Structural wood **sheathing** directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

PLATES

Weight: 11 lb

MT20

REACTIONS (lb/size) 3=43/Mechanical, 2=194/0-8-0, 4=52/Mechanical Max Horz 2=95(LC 6) Max Uplift3=-37(LC 6), 2=-113(LC 6)

SPACING

Plates Increase

Rep Stress Incr

Lumber Increase

Code FBC2004/TPI2002

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=-39/15

2-4=0/0 **BOT CHORD**

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

CSI

TC 0.15

BC

WB 0.00

(Matrix)

0.06

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

2-0-0

1.25

1.25

YES

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 3 and 113 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



GRIP

244/190

December 27,2007

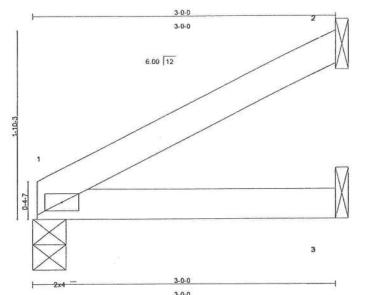
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not thus designer. Bracing shown is for falteral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permonent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding flobrication, quality control, storage, delivery, erection and bracing, consult. ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, \$83 D'Onofrio Drive, Madison, WI \$3719.



Job	Truss	Truss Type	Oty	Ply	SUNBURY_FLORIDA_125	E4585836
SUNBURY	J3S	JACK	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, Florida

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LOADIN TCLL TCDL	16.0 7.0		SPACING Plates Increase Lumber Increase	2-0-0 1.25 1.25	CSI TC BC	0.08 0.07	DEFL Vert(LL) Vert(TL)	in -0.00 -0.01	(loc) 1-3 1-3	l/defl >999 >999	L/d 240 180	PLATES MT20	GRIP 244/190
BCLL BCDL	10.0	•	Rep Stress Incr Code FBC2004/TF	YES	WB (Mati	0.00	Horz(TL)	-0.00	2	n/a	n/a	Weight: 10 lb	0

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=117/0-4-0, 2=63/Mechanical, 3=55/Mechanical Max Horz 1=63(LC 6) Max Uplift1=-11(LC 6), 2=-62(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-49/24 **BOT CHORD** 1-3=0/0

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 11 lb uplift at joint 1 and 62 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 2 and 3.

LOAD CASE(S) Standard



December 27,2007

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Job Truss Truss Type Oty SUNBURY FLORIDA 125 E4585837 JACK Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:11 2007 Page 1 Marenda Homes Inc., Sanford, Florida 3 1-0-0 1-0-0 1-0-0 1-0-0 Scale = 1:6.2 6.00 12 2 1-0-0

DEFL

Vert(LL)

Vert(TL)

Horz(TL)

BRACING

TOP CHORD

BOT CHORD

1-0-0

2 >999

3 n/a

in (loc)

-0.00

-0.00

0.00

I/defl

>999

L/d

240

180

n/a

PLATES

Weight: 5 lb

MT20

Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

GRIP

244/190

BCDL LUMBER

TCLL

TCDL

BCLL

LOADING (psf)

16.0

10.0

10.0

7.0

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

4 SYP No.2

2-0-0

1.25

1.25

YES

REACTIONS (Ib/size) 2=121/0-4-0, 4=19/Mechanical, 3=-7/Mechanical Max Horz 2=54(LC 6)

Max Uplift2=-111(LC 6), 3=-7(LC 1)

SPACING

Plates Increase

Lumber Increase Rep Stress Incr

Code FBC2004/TPI2002

Max Grav2=121(LC 1), 4=19(LC 1), 3=22(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-25/22

BOT CHORD 2-4=0/0

NOTES (6)

Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

CSI

TC 0.10

BC 0.01

WB 0.00

(Matrix)

- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 2 and 7 lb uplift at joint 3.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

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818 Soundside Rose Edenton, NC 27932

SUNBURY_FLORIDA_125 Job Truss Type Truss F4585839 SUNBURY J486 JACK Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:12 2007 Page 1 Maronda Homes Inc., Sanford, Florida 3 1-0-0 1-0-0 1-0-0 Scale = 1:6.2 6.00 12 ٨° 1-0-0 2x4 = DEFL **PLATES** GRIP LOADING (psf) CSI SPACING 2-0-0 in (loc) I/defl L/d Vert(LL) -0.00 >999 240 MT20 244/190 TC. 0.11 TCLL 16.0 Plates Increase 1.25 180 BC -0.00 >999 TCDL 7.0 Lumber Increase 1.25 0.01 Vert(TL) WB 0.00 0.00 n/a n/a BCLL 10.0 Rep Stress Incr YES Horz(TL) Weight: 5 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=124/0-8-0, 4=18/Mechanical, 3=-11/Mechanical

Max Horz 2=54(LC 6)

Max Uplift2=-116(LC 6), 3=-11(LC 1)

Max Grav2=124(LC 1), 4=18(LC 1), 3=27(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=-27/26

BOT CHORD 2-4=0/0

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 116 lb uplift at joint 2 and 11 lb uplift at joint 3.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

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Joh Truss Truss Type Qty SUNBURY FLORIDA 125 E4585847 JGRD MONO TRUSS SUNBURY Job Reference (optional)
7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:15 2007 Page 1 Maronda Homes Inc., Sanford, Florida -1-5-0 5-1-15 9-10-1 1-5-0 5-1-15 4-8-1 3x4 H 4.24 12 3x4 = 12 5 6 2x4 || 3x5 = 9-10-1 5-1-15 4-8-1 Plate Offsets (X.Y): [2:0-1-15.0-0-7] **PLATES** GRIP LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defi L/d TC 0.24 -0.03 5-6 >999 240 MT20 244/190 Vert(LL) TCLL 16.0 Plates Increase 1.25 BC -0.06 >999 180 1.25 0.33 Vert(TL) 5-6 TCDL 7.0 Lumber Increase NO WB 0.29 0.01 Horz(TL) n/a 10.0 Rep Stress Incr BCLL Code FBC2004/TPI2002 (Matrix) Weight: 46 lb BCDL 10.0

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 5=576/Mechanical, 2=536/0-5-11 Max Horz 2=174(LC 3) Max Uplift5=-237(LC 3), 2=-169(LC 3)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/21, 2-7=-905/228, 7-8=-870/250, 3-8=-830/229, 3-9=-123/50, 4-9=-55/0, 4-5=-129/142 2-10=-302/815, 10-11=-302/815, 6-11=-302/815, 6-12=-302/815, 5-12=-302/815 3-6=0/293, 3-5=-812/290

BOT CHORD WEBS

NOTES 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 5 and 169 lb uplift at

Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 5 lb down and 33 lb up at 4-4-0, 5 lb down and 33 lb up at 4-4-0, 56 lb down and 100 lb up at 7-1-15, 56 lb down and 100 lb up at 7-1-15, and 38 lb down at 1-6-1, and 38 lb down at 1-6-1 on top chord, and 21 lb up at 1-6-1, 21 lb up at 1-6-1, 15 lb down at 4-4-0, 15 lb down at 4-4-0, and 55 lb down at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

In the LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase

Uniform Locate 1-12. 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 5 lb down and 33 lb up at 4-4-0,

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-46, 2-5=-40

Vert: 8=-10(F=-5, B=-5) 9=-113(F=-56, B=-56) 10=42(F=21, B=21) 11=-31(F=-15, B=-15) 12=-111(F=-55, B=-55)

No No STATE OF WE STONAL ENGINEERING CO.

Marvin A. Strzyzewski, FL Lic. #43144

Truss Engineering Co.

Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

December 27,2007

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Truss Type City SUNBURY_FLORIDA_125 Job Truss F4585851 MONO TRUSS SUNBURY JGRD86 Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:17 2007 Page 1 Maronda Homes Inc., Sanford, Florida 9-10-1 5-1-15 -1-5-0 4-B-1 1-5-0 5-1-15 3x4 || Scale = 1:23.6 4.24 12 3x4 = 0.3-14 13 6 14 5 2x4 || 3x5 2x4 = 9-10-1 5-1-15 5-1-15 Plate Offsets (X,Y): [2:0-3-3,0-0-15] **PLATES** GRIP LOADING (psf) SPACING 2-0-0 CSI DEFL 1.25 TC 0.23 Vert(LL) -0.03 5-6 >999 240 MT20 244/190 TCLL 16.0 Plates Increase TCDL 7.0 Lumber Increase 1.25 BC 0.32 Vert(TL) -0.06 5-6 >999 180 BCLL 10.0 Rep Stress Incr NO WB 0.29 Horz(TL) 0.01 n/a n/a Code FBC2004/TPI2002 Weight: 46 lb BCDL 10.0 (Matrix)

BRACING

TOP CHORD

BOT CHORD

end verticals

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.2

WEBS 2 X 4 317 No.2

REACTIONS (lb/size) 5=565/Mechanical, 2=495/1-0-1

Max Horz 2=173(LC 3)

Max Uplift5=-223(LC 3), 2=-137(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-7=-883/183, 7-8=-881/187, 8-9=-852/209, 3-9=-817/196, 3-10=-118/46, 4-10=-53/0, 4-5=-125/140

BOT CHORD 2-11=-265/802, 11-12=-265/802, 12-13=-265/802, 6-13=-265/802, 6-14=-265/802, 5-14=-265/802

WEBS 3-6=0/284, 3-5=-801/251

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 223 lb uplift at joint 5 and 137 lb uplift at joint 2.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 24 lb up at 4-4-12, 24 lb up at 4-4-12, 50 lb down and 94 lb up at 7-2-11, 50 lb down and 94 lb up at 7-2-11, and 40 lb down at 1-6-12, and 40 lb down at 1-6-12, and 40 lb down at 1-6-12, and 40 lb down at 1-6-12, 22 lb up at 1-6-12, 12 lb down at 4-4-12, 12 lb down at 4-4-12, and 52 lb down at 7-2-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-2=-46, 2-7=-14, 4-7=-46, 2-11=-20, 5-11=-40

Concentrated Loads (lb)

Vert: 9=7(F=3, B=3) 10=-100(F=-50, B=-50) 12=43(F=22, B=22) 13=-24(F=-12, B=-12) 14=-104(F=-52, B=-52)

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component, Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for toleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding labrication, quality control storage, delivery, erection and bracing, consult. AMSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onafrio Drive, Madison, WI 53719.



818 Soundside Road Edenton, NC 27932

SUNBURY_FLORIDA_125 Job Truss Truss Type Qty E4585855 MONO SCISSOR Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:18 2007 Page 1 Maronda Homes Inc., Sanford, Florida -1-0-0 7-0-0 7-0-0 1-0-0 Scale: 1/2"=1" 2x4 11 6.00 12 2x4 = 3.00 12 3x4 = 7-0-0 7-0-0 **PLATES** GRIP LOADING (psf) SPACING 2-0-0 CSI DEFL L/d 1.25 TC 0.44 Vert(LL) -0.11 2-4 >742 240 MT20 244/190 TCLL 16.0 Plates Increase 1.25 BC 0.42 Vert(TL) -0.27 2-4 >297 180 TCDL 7.0 Lumber Increase Rep Stress Incr YES WB 0.03 Horz(TL) 0.00 n/a n/a BCLL 10.0 Weight: 27 lb

BRACING

TOP CHORD

BOT CHORD

LUMBER

BCDL

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

10.0

2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 2=346/0-4-0, 4=283/Mechanical

Max Horz 2=173(LC 6)

Max Uplift2=-105(LC 6), 4=-102(LC 6)

Code FBC2004/TPI2002

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-3=-108/55

BOT CHORD 2-4=-26/26

WEBS 3-4=-149/176

NOTES

NOTES
1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

(Matrix)

- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 105 lb uplift at joint 2 and 102 lb uplift at joint 4.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

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SUNBURY_FLORIDA_125 Truss Truss Type Job F4585856 MONO SCISSOR SUNBURY JS2 Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:19 2007 Page 1 Maronda Homes Inc., Sanford, Florida 5-0-0 1-0-0 3 1-0-0 5-0-0 Scale = 1:16.3 6.00 12 1-1-10 0-4-3 3.00 12 5-0-0

BCLL 100 BCDL 100

LOADING (psf)

SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002

CSI TC 0.21 BC 0.22 WB 0.00 (Matrix)

DEFL (loc) I/defl L/d Vert(LL) -0.03 2-4 >999 240 180 -0.07 >817 Vert(TL) 2-4 -0.00 3 Horz(TL) n/a n/a

5-0-0

BRACING

TOP CHORD

BOT CHORD

PLATES MT20

GRIP 244/190

Weight: 18 lb

Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER

TCLL

TCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

7.0

REACTIONS (lb/size) 3=103/Mechanical, 2=265/0-4-0, 4=95/Mechanical Max Horz 2=133(LC 6) Max Uplift3=-98(LC 6), 2=-98(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-3=-76/38

BOT CHORD 2-4=-19/19

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads,
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 lb uplift at joint 3 and 98 lb uplift at
- Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

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818 Soundside Road Edenton, NC 27932

SUNBURY_FLORIDA_125 Qty Job Truss Truss Type E4565857 JS3 MONO SCISSOR SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:19 2007 Page 1 Maronda Homes Inc., Sanford, Florida 3-0-0 -1-0-0 1-0-0 Scale = 1:10.8 6.00 12 0-7-10 0-4-3 3.00 12 3-0-0 3-0-0 PLATES GRIP LOADING (psf) SPACING CSI DEFL l/defl L/d 2-0-0 244/190 1.25 TC Vert(LL) -0.00 2-4 >999 240 MT20 Plates Increase 0.11 TCLL 16.0 1.25 BC 0.07 Vert(TL) -0.01 2-4 >999 180 Lumber Increase 7.0 TCDL Rep Stress Incr YES WB 0.00 Horz(TL) -0:00 3 n/a n/a BCLL 10.0 Weight: 12 lb Code FBC2004/TPI2002 (Matrix) BCDL

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=52/Mechanical, 2=184/0-4-0, 4=55/Mechanical

Max Horz 2=92(LC 6)

Max Uplift3=-48(LC 6), 2=-94(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

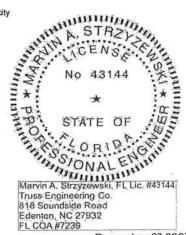
TOP CHORD 1-2=0/18, 2-3=-38/19

BOT CHORD 2-4=-11/11

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 3 and 94 lb uplift at
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

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SUNBURY_FLORIDA_125 Truss Type linh Truss E4585858 MONO SCISSOR SUNBURY JS4 Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:19 2007 Page 1 Maronda Homes Inc., Sanford, Florida 3 1-0-0 -1-0-0 1-0-0 Scale = 1:6.2 6.00 12 3x4 = 3.00 12 4 1-0-0 PLATES GRIP DEFL CSI LOADING (psf) (loc) 2 L/d SPACING 2-0-0 in I/defl -0.00 >999 240 MT20 244/190 Vert(LL) TCLL 16.0 Plates Increase 1.25 TC 0.09 -0.00 >999 180 TCDL 7.0 Lumber Increase 1 25 BC 0.01 Vert(TL) 0.00 3 0.00 n/a n/a BCLL 10.0 Rep Stress Incr YES WB Horz(TL) Weight: 5 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0 BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

Max Horz 2=51(LC 6) Max Uplift2=-98(LC 6), 3=-7(LC 5)

Max Grav2=114(LC 1), 4=19(LC 1), 3=14(LC 4)

REACTIONS (lb/size) 2=114/0-4-0, 4=19/Mechanical, 3=-4/Mechanical

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/17, 2-3=-25/17

BOT CHORD 2-4=-4/4

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 lb uplift at joint 2 and 7 lb uplift at joint
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

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ANSI/TPIT Quality Criteria, DSB-89 and BCSIT Building Component Safety Information.



SUNBURY_FLORIDA_125 Truss Type Joh Truss F4585864 SUNBURY **JSGRD** MONO SCISSOR Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:22 2007 Page 1 Maronda Homes Inc., Sanford, Florida 9-10-1 -1-5-0 5-4-0 4-6-1 1-5-0 Scale = 1:23.4 3×4 4.24 12 4x6 = 5x6 = -7-14 12 0-3-19 2x4 | 10 2.12 12 3x4 = 9-10-1 **PLATES** GRIP LOADING (psf) CSI DEFL SPACING 2-0-0 (loc) 1/defl L/d 244/190 TCLL 16.0 1.25 TC 0.30 Vert(LL) -0.06 2-6 >999 240 MT20 TCDL 7.0 Lumber Increase 1.25 BC. 0.46 Vert(TL) -0.112-6 >999 180 BCLL 100 Rep Stress Incr NO WB 0.36 Horz(TL) 0.02 n/a n/a Weight: 42 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0 LUMBER BRACING

TOP CHORD

BOT CHORD

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2

WEBS

REACTIONS (lb/size) 2=536/0-5-11, 5=580/Mechanical

Max Horz 2=173(LC 3)

Max Uplift2=-180(LC 3), 5=-248(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/19, 2-7=-1496/472, 7-8=-1456/490, 3-8=-1423/473, 3-9=-210/95, 4-9=-143/35, 4-5=-110/132 2-10=-543/1378, 10-11=-534/1381, 6-11=-533/1403, 6-12=-547/1371, 5-12=-539/1407 TOP CHORD

BOT CHORD

WEBS 3-6=0/273, 3-5=-1212/455

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 180 lb uplift at joint 2 and 248 lb uplift at
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 6 lb down and 38 lb up at 4-4-12.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

A STRZY ICENS will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Rearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 180 lb uplift at joint 2 and 248 lb uplift at joint 5.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 6 lb down and 38 lb up at 4-4-12, 6 lb down and 38 lb up at 4-4-12, 57 lb down and 103 lb up at 7-2-11, 57 lb down and 103 lb up at 7-2-11, and 35 lb down at 1-6-12, and 35 lb down at 1-6-12 on top chord, and 21 lb up at 1-6-12, 21 lb up at 1-6-12, 15 lb down at 4-4-12, 15 lb down at 4-4-12, and 55 lb down at 7-2-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) Standard
7) Regular: Lumber Increase=1.25, Plate Increase=1.25
7) Uniform Loads (pil)
7) Vert: 1-4-46, 2-5-40
7) Concentrated Loads (lb)
7) Vert: 1-4-46, 2-5-40
7) Concentrated Loads (lb)
7) Vert: 8-13(F=-6, B=-6) 9-114(F=-57, B=-57) 10-42(F=21, B=21) 11-31(F=-15, B=-15) 12-111(F=-55, B=-55)
7) Marvin A. Strzyzewski, FLLic. #43144
7) Truss Engineering Co. Truss Engineering Co. 818 Soundside Road

Structural wood sheathing directly applied or 5-1-0 oc purlins, except

Rigid ceiling directly applied or 8-5-5 oc bracing.

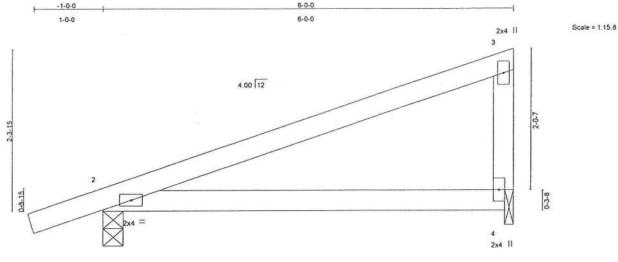
Edenton, NC 27932 FL COA #7239

December 27,2007

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Job Truss Truss Type Oty SUNBURY_FLORIDA_125 E4585866 MONO TRUSS 10 Job Reference (optional)
7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:23 2007 Page 1 Maronda Homes Inc., Sanford, Florida -1-0-0 6-0-0



6-0-0

BRACING

TOP CHORD

BOT CHORD

							6-0-0						
LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	în	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	16.0	- 1	Plates Increase	1.25	TC	0.23	Vert(LL)	0.22	2-4	>305	240	MT20	244/190
TCDL	7.0	- 1	Lumber Increase	1.25	BC	0.41	Vert(TL)	0.17	2-4	>393	180		
BCLL	10.0		Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00		n/a	n/a		
BCDL	10.0		Code FBC2004/TF	PI2002	(Mati	ix)						Weight: 22 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.1D

BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.2

REACTIONS (lb/size) 2=306/0-3-8, 4=240/0-1-8

Max Horz 2=103(LC 4) Max Uplift2=-214(LC 4), 4=-159(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/14, 2-3=-57/33, 3-4=-126/126

BOT CHORD 2-4=0/0

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 214 lb uplift at joint 2 and 159 lb uplift at ioint 4.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 6-0-0 oc purlins, except

Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

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ANSI/TP11 Quality Criteria, DS8-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Modison, WI 53719.



Truss Type SUNBURY_FLORIDA_125 .lob Truss F4585871 SUNBURY RG2 GABLE Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:25 2007 Page 1 Maronda Humes Inc., Sanford, Florida 11-0-13 5-6-7 Scale = 1:20.0 4x6 = 6.00 12 2x4 || 2x4 || 11-0-13 11-0-13 LOADING (psf) **PLATES** GRIP CSI DEFL SPACING 2-0-0 I/defl L/d 244/190 TCLL Plates Increase 1.25 TC 0.11 Vert(LL) n/a n/a 999 MT20 TCDL 7.0 Lumber Increase 1.25 BC 0.07 Vert(TL) n/a n/a 999 BCLL 10.0 Rep Stress Incr NO WB 0.03 Horz(TL) 0.00 5 n/a n/a Weight: 40 lb Code FBC2004/TPI2002 (Matrix) BCDL 100 LUMBER BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

OTHERS 2 X 4 SYP No.2

REACTIONS (lb/size) 1=106/11-1-13, 5=103/11-1-13, 7=105/11-1-13, 8=268/11-1-13, 6=264/11-1-13

Max Horz 1=-46(LC 4)

Max Uplift1=-35(LC 7), 5=-38(LC 7), 8=-167(LC 6), 6=-165(LC 7)

Max Grav1=106(LC 1), 5=103(LC 1), 7=105(LC 1), 8=269(LC 10), 6=264(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-34/34, 2-3=-34/117, 3-4=-33/114, 4-5=-25/30

BOT CHORD

1-8=0/54, 7-8=0/54, 6-7=0/55, 5-6=0/55

WEBS

3-7=-56/7, 2-8=-141/234, 4-6=-139/233

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 1, 38 lb uplift at joint 5. 167 lb uplift at joint 8 and 165 lb uplift at joint 6.
- Non Standard bearing condition. Review required.

LOAD CASE(S) Standard

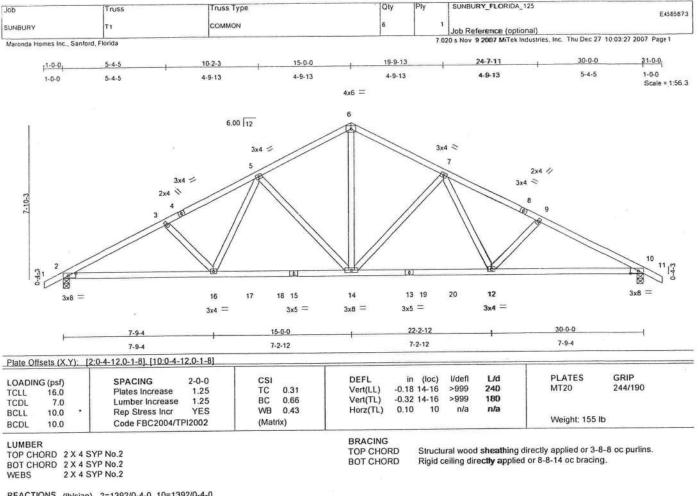


December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shows is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the distingting designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult.

ANSI/TPIT Quality Citleria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.





REACTIONS (Ib/size) 2=1392/0-4-0, 10=1392/0-4-0 Max Horz 2=-137(LC 7)

Max Uplift2=-309(LC 6), 10=-309(LC 7)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/20, 2-3=-2441/724, 3-4=-2287/666, 4-5=-2233/682, 5-6=-1589/568, 6-7=-1589/568, 7-8=-2233/682,

8-9=-2287/666, 9-10=-2441/724, 10-11=0/20

BOT CHORD 2-16=-519/2136, 16-17=-342/1760, 17-18=-342/1760, 15-18=-342/1760, 14-15=-342/1760, 13-14=-342/1760,

13-19=-342/1760, 19-20=-342/1760, 12-20=-342/1760, 10-12=-519/2136

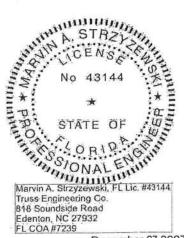
WEBS 3-16=-205/215, 5-16=-70/561, 5-14=-568/272, 6-14=-314/1144, 7-14=-568/272, 7-12=-70/561, 9-12=-205/215

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 309 lb uplift at joint 2 and 309 lb uplift at joint 10.

LOAD CASE(S) Standard

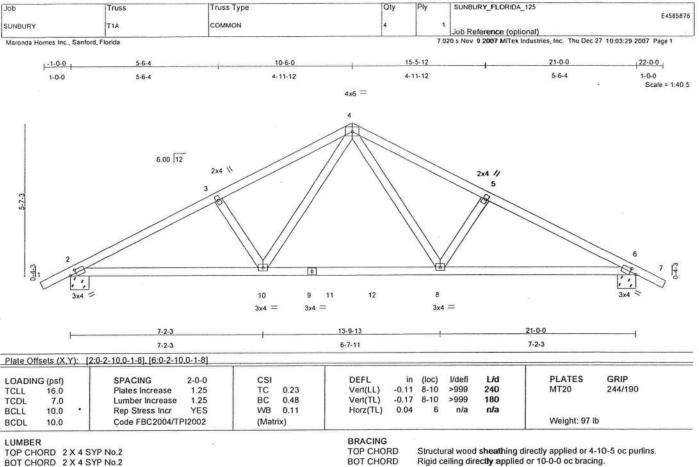


December 27,2007

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Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of besign parameters and proper incorporation of component is responsibility of building designer - not thus designer. Bracing shown is for toleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flabrication, qualify control storage, delivery, erection and bracing, consult. ANSI/IPI (Quality Criteria, DSB-87 and BCS11 Building Component Satety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





BOT CHORD

TOP CHORD 2 X 4 SYP No.2 2 X 4 SYP No.2

BOT CHORD

2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 2=971/0-8-0, 6=971/0-8-0 Max Horz 2=104(LC 6)

Max Uplift2=-247(LC 6), 6=-247(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-1498/434, 3-4=-1382/436, 4-5=-1382/436, 5-6=-1498/434, 6-7=0/21

BOT CHORD 2-10=-253/1288, 9-10=-85/886, 9-11=-85/886, 11-12=-85/886, 8-12=-85/886, 6-8=-253/1288 WEBS 3-10=-202/207, 4-10=-108/583, 4-8=-108/583, 5-8=-202/207

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 247 lb uplift at joint 2 and 247 lb uplift at joint 6.

LOAD CASE(S) Standard



December 27,2007

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ANSI/IP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onafrio Drive, Madison, WI 53719.



SUNBURY_FLORIDA_125 Truss Type Truss Job E4585877 TIAS SUNBURY COMMON Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:29 2007 Page 1 Maronda Hornes Inc., Sanford, Florida 21-0-0 -1-0-0 15-5-12 10-6-0 5-6-4 4-11-12 5-6-4 5-6-4 4-11-12 1-0-0 Scale = 1:39.7 4x6 = 6.00 12 2x4 1 2x4 // 14.9 6 9 10 8 11 3x4 = 3x4 = 3x4 13-9-13 21-0-0 7-2-3 7-2-3 7-2-3 Plate Offsets (X,Y): [2:0-2-10.0-1-8], [6:0-2-10.0-1-8] I/defl **PLATES** GRIP SPACING 2-0-0 CSI DEFL LOADING (psf) 244/190 0.25 Vert(LL) -0.10 7-9 >999 240 MT20 Plates Increase 1.25 TC 16.0 TCIL Lumber Increase 1.25 BC 0.49 Vert(TL) -0.176-7 >999 180 TCDL 7.0 10.0 Rep Stress Incr YES WB 0.11 Horz(TL) 0.04 6 n/a n/a BCLL Weight: 95 lb Code FBC2004/TPI2002 (Matrix) 10.0 BCDL BRACING LUMBER TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.2

REACTIONS (lb/size) 6=905/0-8-0, 2=973/0-8-0 Max Horz 2=113(LC 6) Max Uplift6=-164(LC 7), 2=-247(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-1502/444, 3-4=-1386/446, 4-5=-1395/466, 5-6=-1511/465 TOP CHORD

2-9=-315/1292, 9-10=-147/891, 8-10=-147/891, 8-11=-147/891, 7-11=-147/891, 6-7=-339/1302 **BOT CHORD**

3-9=-202/208, 4-9=-105/583, 4-7=-137/595, 5-7=-209/222 WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 164 lb uplift at joint 6 and 247 lb uplift at joint 2.

LOAD CASE(S) Standard



December 27,2007

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ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Salety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison. Wi 53719.



SUNBURY_FLORIDA_125 Truss Truss Type Oty E4585885 SUNBURY V2A VALLEY Job Reference (optional)
7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:34 2007 Page 1 Maronda Homes Inc., Sanford, Florida 3-6-14 7-1-13 3-6-15 3-6-14 Scale = 1:14.4 3x4 = 6.00 12 2x4 > 2x4 = 7-1-13 7-1-13 Plate Offsets (X,Y): [2:0-2-0,Edge] PLATES GRIP DEFL 1./d LOADING (psf) SPACING 2-0-0 CSI in 1/defl 244/190 999 MT20 0.10 Vert(LL) TCLL 16.0 Plates Increase 1.25 TC n/a n/a 999 BC n/a n/a TCDL 7.0 Lumber Increase 1.25 0.36 Vert(TL) 0.00 0.00 n/a n/a YES WB Horz(TL) BCLL 100 Rep Stress Incr Weight: 20 lb Code FBC2004/TPI2002 BCDL 10.0 (Matrix) BRACING LUMBER TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=254/7-1-13, 3=254/7-1-13 Max Horz 1=-24(LC 4)

Max Uplift1=-48(LC 6), 3=-48(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-176/143, 2-3=-176/143 TOP CHORD

1-3=-87/136 **BOT CHORD**

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 1 and 48 lb uplift at joint 3.

LOAD CASE(S) Standard



December 27,2007

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SUNBURY FLORIDA 125 Truss Type Qty Job E4585886 VALLEY SUNBURY V28 Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:34 2007 Page 1 Maronda Homes Inc., Sanford, Florida 2 3-1-13 1-6-15 1-6-14 6.00 12 Scale = 1:5 9 0-0-4 2x4 > 2x4 = 3-1-13 Plate Offsets (X.Y): [2:0-2-0.Edge] DEFL l/defi PLATES GRIP SPACING 2-0-0 CSI in (loc) Lid LOADING (psf) Vert(LL) 999 MT20 244/190 TC BC 0.02 n/a n/a TCLL 16.0 Plates Increase 1.25 n/a 999 n/a TCDL 7.0 Lumber Increase 1.25 0.04 Vert(TL) 0.00 n/a WB 0.00 Horz(TL) BCLL 10.0 Rep Stress Incr YES Weight: 8 lb BCDL Code FBC2004/TPI2002 (Matrix) BRACING LUMBER TOP CHORD Structural wood sheathing directly applied or 3-1-13 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=82/3-1-13, 3=82/3-1-13 Max Horz 1=-8(LC 4)

Max Uplift1=-16(LC 6), 3=-16(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-57/48, 2-3=-57/48

BOT CHORD 1-3=-29/44

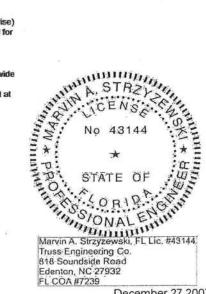
Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 1 and 16 lb uplift at

LOAD CASE(S) Standard



December 27,2007

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SUNBURY_FLORIDA_125 Truss Type Job Truss E4585889 VALLEY SUNBURY V4 Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:35 2007 Page 1 Maronda Homes Inc., Sanford, Florida 4-0-0 4-0-0 Scale = 1:8.4 4.00 12 4-0-0 PLATES DEFL LOADING (psf) SPACING 2-0-0 CSI in (loc) I/defl L/d 244/190 MT20 999 Plates Increase 1.25 TC 0.09 Vert(LL) n/a n/a TCLL 16.0 999 7.0 Lumber Increase 1.25 BC 0.08 Vert(TL) n/a n/a TCDL 0.00 n/a n/a BCLL 10.0 Rep Stress Incr YES WB 0.00 Horz(TL) Weight: 12 lb Code FBC2004/TPI2002 BCDL 10.0 (Matrix) BRACING LUMBER Structural wood sheathing directly applied or 4-0-0 oc purlins, except TOP CHORD TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 1=127/4-0-0, 3=127/4-0-0 Max Horz 1=41(LC 4)

Max Uplift1=-24(LC 4), 3=-38(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

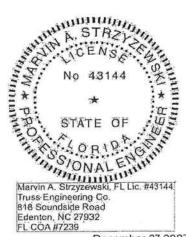
TOP CHORD 1-2=-30/17, 2-3=-68/72

BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 1 and 38 lb uplift at joint 3.

LOAD CASE(S) Standard



December 27,2007

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RE: ELEV F - SUNBURY FL

Trenco

818 Soundside Rd Edenton, NC 27932

Site Information:

Project Customer: Maronda Homes Project Name: Sunbury FL

Lot/Block: Sanford

Subdivision: Sanford

Address: Sanford

City: Sanford

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002

Design Program: MiTek 20/20 7.0

Wind Code: N/A

Wind Speed: N/A mph

Floor Load: 55.0 psf

Roof Load: N/A psf

This package includes 5 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules. This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

No.	Seal#	Truss Name	Date .
1	E4617771	FA	1/15/08
2	E4617772	FB	1/15/08
3	E4617773	FC	1/15/08
4	E4617774	FD	1/15/08
5	E4617775	FE	1/15/08

The truss drawing(s) referenced above have been prepared by TRENCO under my direct supervision based on the parameters provided by Maronda Homes-Pittsburgh, PA.

Truss Design Engineer's Name: Strzyzewski, Marvin My license renewal date for the state of is February 28, 2009.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



Truss-Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

January 15,2008

Truss Type Job Truss F461777 1 SUNBURY FL ALL ELEVATIONS Job Reference (optional) ELEV_F FLOOR 7.030 s Jan 3 2008 MiTek Industries, Inc. Tue Jan 15 12:03:14 2008 Page 1 Maronda Homes, Inc., Pittsburgh, PA 0-10-4 9-7-8 9-7-89-1-8 Scale = 1:52.2 H 9-10-8 1-0-0 9-10-2 1-9-12 9-11-19 9-14/ 11-0-14 TRUSS IS DESIGNED TO SUPPORT A SMALL CONCENTRATED LOAD AT ITS CANTILEVERED END(S). 1.5x4 II 11 3x6 FP = 1.5x4 1.5x4 4x6 = 3x6 = 3x6 3x8 3x4 3x6 3x4 3x8 = 4x6 = 3x4 | 14x8 = = 3x6 4x8 1.5x4 3x4 3x4 4x6 5 10 11 12 13 15 19 20 21 22 27 26 3130 29 25 24 46 45 44 43 42 41 40 39 38 37 36 35 34 33 32 50 48 11 5x6 5x6 4x8 П 3x6 FP = 4x6 = 4x8= 3x6 = 5x8 3x4 4x6 1.5x4 || 3x6 = 1.5x4 II 4x6 = 3x6 11 3x8 3x6 = 1.5x4 || NOTE: DUE TO THE OVERALL CANTILEVER LENGTH THE FLOOR MAY EXHIBIT OBJECTIONABLE VIBRATION AND/OR DEFLECTION. BUILDING DESIGNER TO CONSIDER PROVIDING MEANS TO DAMPEN POSSIBLE FLOOR VIBRATION. 22-0-10 9-4-14 29-9-0 21-2-6 21-7-8 27-9-0 27-10-8 13-10-8 7-3-14 5-8-6 0-1-81-10-8 0-10-14 4-5-10 7-7-2 0-10-14 Plate Offsets (X,Y): [1:Edge,0-1-8], [3:0-2-0,Edge], [5:0-1-8,Edge], [6:0-1-8,Edge], [8:0-1-8,Edge], [12:0-3-0,Edge], [16:0-1-8,Edge], [18:0-2-12,Edge], [23:0-3-0,Edge], [26:0-2-4, Edge], [32:0-1-8,Edge], [42:0-1-8,Edge], [43:0-1-8,Edge], [45:0-1-8,Edge], [47:0-3-0,Edge], [48:0-1-8,Edge], [50:Edge,0-1-8] DEFL in (loc) V/defl 1 /d PLATES GRIP LOADING (psf) SPACING 2-0-0 -0.27 44-45 244/190 Plates Increase 1.00 TC 1.00 Vert(LL) >616 360 MT20 TCLL TCDL Lumber Increase 1.00 BC 0.71 Vert(TL) -0.39 44-45 >424 240 WB 0.77 0.03 39 n/a BCLL 0.0 Rep Stress Incr NO Horz(TL) n/a Weight: 161 lb Code FBC2004/TPI2002 (Matrix) BCDL BRACING LUMBER TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except TOP CHORD 4 X 2 SYP No.2 *Except* 13-23 4 X 2 SYP No.1D Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 4 X 2 SYP No.2 *Except* **BOT CHORD BOT CHORD** 10-0-0 oc bracing: 49-50,48-49,47-48,46-47,45-46. 41-50 4 X 2 SYP No.1D, 41-48 4 X 2 SYP No.1D WEBS 4 X 2 SYP No.3 REACTIONS (lb/size) 50=2156/0-6-8, 39=1576/0-3-8, 26=2595/0-8-0 Max Grav50=2222(LC 4), 39=1795(LC 2), 26=2654(LC 3) FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 50-51=-2249/0, 1-51=-2245/0, 24-52=0/6, 23-52=0/6, 1-2=-1421/0, 2-3=-1421/0, 3-4=-3096/0, 4-5=-3766/0, 5-6=-3969/0 , 6-7=.3375/274, 7-8=-3375/274, 8-9=-2177/949, 9-10=-142/1901, 10-11=-142/1901, 11-12=0/3301, 12-13=-231/1277, 13-14=-231/1277, 14-15=-1561/1325, 15-16=-2271/1671, 16-17=-2356/1983, 17-18=-2356/1983, 18-19=-1487/2899, 19-20=0/3875, 20-21=0/4396, 21-22=0/4393, 22-23=0/2157 49-50=0/428, 48-49=0/2299, 47-48=0/2302, 46-47=0/3615, 45-46=0/3969, 44-45=-274/3375, 43-44=-274/3375, **BOT CHORD** 42-43=-949/2177, 41-42=-1272/1229, 40-41=-1272/1225, 39-40=-3301/0, 38-39=-1912/0, 37-38=-1912/0, 36-37=-1241/1034, 35-36=-1448/2085, 34-35=-1448/2085, 33-34=-1983/2356, 32-33=-1983/2356, 31-32=-2514/2031, 30-31=-2514/2031, 29-30=-2514/2031, 28-29=-3317/904, 27-28=-3317/904, 26-27=-3875/0, 25-26=-2157/0, 24-25=-1/0 2-49=-96/0, 3-49=-1012/0, 3-48=-103/0, 3-47=0/909, 4-47=-602/21, 4-46=-99/175, 5-46=-231/91, 5-45=-279/0, WEBS 6-45=0/1260, 11-40=0/1687, 10-40=-93/15, 9-40=-1364/0, 9-42=0/1158, 8-42=-804/0, 8-43=0/1911, 12-39=-1581/0, 12-38=-25/3, 12-37=0/1223, 14-37=-1128/32, 14-36=-101/729, 15-36=-725/144, 15-35=-27/19, 6-44=-552/0, 7-43=-386/0, 11-39=-965/0, 16-33=-278/117, 17-32=-243/1, 21-26=-197/0, 15-34=-261/416, 16-34=-441/497, 20-26=-1584/0, 20-27=0/715, 19-27=-1491/0, 19-28=0/18, 19-29=0/1026, 18-29=-960/0, 18-31=-85/44, 18-32=-175/1044, 22-25=0/1553, 22-26=-2766/0, 23-25=-2681/0, 1-49=0/1163

1) Unbalanced floor live loads have been considered for this design.

2) Posi-Strut webs to be applied to both sides of truss unless otherwise noted. FF=Front Face, BF=Back Face

3) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

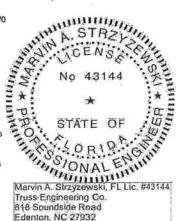
4) CAUTION, Do not erect truss backwards

5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1554 lb down at 0-3-0, and 1554 Ib down at 29-6-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2



January 15,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the exercity. Additional permanent bracing of the overall structure is the responsibility of the ubilding designer and in the responsibility of the design of the control permanent bracing of the overall structure is the responsibility of the ubilding designer. For general guidance regarding fobrication, quality control, storage, defivery, erection and bracing, consult.

ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Modison, WI 53719.



818 Soundside Road Edenton, NC 27932

FL COA #7239

Job	Truss	Truss Type	Qty	Ply	SUNBURY FL	E4617771
ELEV_F	FA	FLOOR	6		SUNBURY FL ALL ELEVATIONS Job Reference (optional)	

Maronda Homes, Inc., Pittsburgh, PA

7.030 s Jan 3 2008 MiTek Industries, Inc. Tue Jan 15 12:03:14 2008 Page 2

LOAD CASE(S) Standard

1) Floor: Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 24-50=-10, 1-23=-100
Concentrated Loads (lb)
Vert: 23=-1554(F) 1=-1554(F)



SUNBURY FL Job Truss Type F4617772 10 SUNBURY FL ALL ELEVATIONS FLOOR ELEV_F Job Reference (optional) 7 030 s Jan 3 2008 MiTek Industries, Inc. Tue Jan 15 12:03:16 2008 Page 1 Maronda Homes, Inc. Pittsburgh, PA 0-10-4 9-7-8 9-7-89-1-8 Scale = 1:52.2 H 9-10-8 1-0-0 9-10-2 1-9-12 A-11-19 9-9-14 19-0-14 NOTE: DUE TO THE OVERALL CANTILEVER LENGTH THE FLOOR MAY EXHIBIT OBJECTIONABLE VIBRATION AND/OR DEFLECTION. BUILDING DESIGNER TO CONSIDER PROVIDING MEANS TO DAMPEN POSSIBLE FLOOR VIBRATION. 1.5x4 II 4x6 = 1.5x4 II 1.5x4 II 3x6 FP = 4x6 3x6 3x4 = 3x6 = 3x8 = 4x6 = 3x4 | 14x8 = 3x4 = 3x6 = 4v6 = 3v6 = 4x8 = 3x8 = 11 16 18 19 20 21 22 23 q 10 11 12 13 14 15 17 36 33 32 3130 29 28 27 26 25 24 45 30 38 37 35 50 4x6 || 5x6 = 11 4x6 = 4x8 3x6 = 5x6 4x8 = 3x6 = 3x6 = 1.5x4 3x6 FP 5x8 1.5x4 5x6 = 3x6 = 1.5x4 1.5x4 3x4 = 3x6 FP = 3x6 H 22-0-10 29-9-0 9-4-14 21-7-8 27-9-0 7-7-2 B-6-0 13-10-8 0-1-8 1-10-8 7-3-14 0-5-2 5-8-6 0-10-14 4-5-10 0-10-14 0-5-2 Plate Offsets (X,Y): [1:Edge,0-1-8], [3:0-2-0,Edge], [5:0-1-8,Edge], [6:0-1-8,Edge], [8:0-1-8,Edge], [12:0-3-0,Edge], [16:0-1-8,Edge], [18:0-2-12,Edge], [18:0-2-12,Edge], [23:0-3-0,Edge], [26:0-2-4, Edge], [23:0-1-8,Edge], [23:0-1-8,Edge], [42:0-1-8,Edge], [43:0-1-8,Edge], [45:0-1-8,Edge], [47:0-3-0,Edge], [48:0-1-8,Edge], [50:Edge,0-1-8] DEFL in (loc) I/def GRIP CSI SPACING 2-0-0 LOADING (psf) -0.27 44-45 >616 360 MT20 244/190 TC 1.00 Vert(LL) 40.0 Plates Increase 1.00 TCLL 0.71 -0.39 44-45 >424 240 BC Vert(TL) TCDL 10.0 Lumber Increase 1.00 39 Rep Stress Incr 0.03 n/a n/a WB 0.77 Horz(TL) BCLL NO Weight: 161 lb Code FBC2004/TPI2002 (Matrix) BCDL 50 BRACING LUMBER TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except TOP CHORD 4 X 2 SYP No.2 *Except* 13-23 4 X 2 SYP No.1D Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 4 X 2 SYP No.2 *Except* **BOT CHORD BOT CHORD** 10-0-0 oc bracing: 49-50,48-49,47-48,46-47,45-46. 41-50 4 X 2 SYP No.1D, 41-48 4 X 2 SYP No.1D WEBS 4 X 2 SYP No.3 REACTIONS (lb/size) 50=2156/0-3-8, 39=1576/0-3-8, 26=2595/0-8-0 Max Grav50=2222(LC 4), 39=1795(LC 2), 26=2654(LC 3) FORCES (Ib) - Maximum Compression/Maximum Tension 50-51=-2249/0, 1-51=-2245/0, 24-52=0/6, 23-52=0/6, 1-2=-1421/0, 2-3=-1421/0, 3-4=-3096/0, 4-5=-3766/0, 5-6=-3969/0, 6-7=-3375/274, 7-8=-3375/274, 8-9=-2177/949, 9-10=-142/1901, 10-11=-142/1901, 11-12=0/3301, 12-13=-231/1277, TOP CHORD 13-14=-231/1277, 14-15=-1561/1325, 15-16=-2271/1671, 16-17=-2356/1983, 17-18=-2356/1983, 18-19=-1487/2899, 19-20=0/3875, 20-21=0/4396, 21-22=0/4393, 22-23=0/2157 49-50=0/428, 48-49=0/2299, 47-48=0/2302, 46-47=0/3615, 45-46=0/3969, 44-45=-274/3375, 43-44=-274/3375, BOT CHORD 42-43=-949/2177, 41-42=-1272/1229, 40-41=-1272/1225, 39-40=-3301/0, 38-39=-1912/0, 37-38=-1912/0, 36-37=-1241/1034, 35-36=-1448/2085, 34-35=-1448/2085, 33-34=-1983/2356, 32-33=-1983/2356, 31-32=-2514/2031, 30-31=-2514/2031, 29-30=-2514/2031, 28-29=-3317/904, 27-28=-3317/904, 26-27=-3875/0, 25-26=-2157/0, 24-25=-1/0 WEBS

2-49=-96/0, 3-49=-1012/0, 3-48=-103/0, 3-47=0/909, 4-47=-602/21, 4-46=-99/175, 5-46=-231/91, 5-45=-279/0, 6-45=0/1260, 11-40=0/1687, 10-40=-93/15, 9-40=-1364/0, 9-42=0/1158, 8-42=-804/0, 8-43=0/1911, 12-39=-1581/0, 12-38=-25/3, 12-37=0/1223, 14-37=-1128/32, 14-36=-101/729, 15-36=-725/144, 15-35=-27/19, 6-44=-552/0,

7-43=-386/0, 11-39=-965/0, 16-33=-278/117, 17-32=-243/1, 21-26=-197/0, 15-34=-261/416, 16-34=-441/497, 20-26=-1584/0, 20-27=0/715, 19-27=-1491/0, 19-28=0/18, 19-29=0/1026, 18-29=-960/0, 18-31=-85/44,

18-32=-175/1044, 22-25=0/1553, 22-26=-2766/0, 23-25=-2681/0, 1-49=0/1163

NOTES

Unbalanced floor live loads have been considered for this design.

Posi-Strut webs to be applied to both sides of truss unless otherwise noted. FF=Front Face, BF=Back Face

3) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

CAUTION, Do not erect truss backwards.

5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1554 lb down at 0-3-0, and 1554 Ib down at 29-6-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2

No 431 Truss Engineering Co.

818 Soundside Road Edenton, NC 27932 FL COA #7239

January 15,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer. Bracing shown is for toteral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding debrication, quality control storage, delivery, erection and bracing, consult.

ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information.



Job	Truss	Truss Type	Qty	Ply	SUNBURY FL	E4617772
ELEV_F	FB	FLOOR	10		SUNBURY FL ALL ELEVATIONS Job Reference (optional)	25077772

Maronda Homes, Inc. Pittsburgh, PA

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LOAD CASE(S) Standard
1) Floor: Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 24-50=-10, 1-23=-100
Concentrated Loads (lb)
Vert: 23=-1554(F) 1=-1554(F)



818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Oly	Ply	SUNBURY FL	E4617773
ELEV_F	FC	FLOOR	1	.1	SUNBURY FL ALL ELEVATIONS Job Reference (optional)	

Maronda Homes, Inc., Pittsburgh, PA

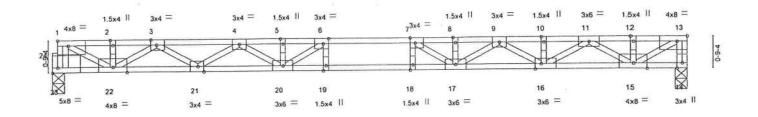
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0-1-8

H 1-0-0

| 0-11-2 | 2-0-0 | 0-11-2 |

Scale = 1:26.3



	6-8-2		1 7-8	3-2 8-8-2			15-4	-4	
	6-8-2		1-0	0-0 1-0-0	6-8-2				
ate Offsets (X.Y):	[1:Edge,0-1-8], [3:0-1-12.Ed	ige], [6:0-1-8,	Edge], [7:0-1-8.Edge]	[13:0-3-0.Edge]	.[16:0-1-12.Edg	je]. [21:0	-1-12,Edge].	[23:Edge,0-1-8]	
OADING (psf) CLL 40.0 CDL 10.0		2-0-0 1.00 1.00	CSI TC 0.61 BC 0.97	DEFL Vert(LL) Vert(TL)	in (loc) -0.37 18-19 -0.58 18-19	l/defl >496 >313	L/d 360 240	PLATES MT20	GRIP 244/190
3CLL 0.0 3CDL 5.0	Rep Stress Incr Code FBC2004/TPI2	NO 2002	WB 0.64 (Matrix)	Horz(TL)	0.07 14	n/a	n/a	Weight: 74 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.2 BOT CHORD 4 X 2 SYP No.1D

WEBS 4 X 2 SYP No.3

BRACING

Structural wood sheathing directly applied or 5-7-12 oc purlins, except TOP CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing. **BOT CHORD**

15-4-4

REACTIONS (lb/size) 23=1239/0-3-8, 14=834/0-3-8 Max Grav23=2248(LC 2), 14=834(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

23-24=-2276/0, 1-24=-2272/0, 13-14=-823/0, 1-2=-1606/0, 2-3=-1606/0, 3-4=-3362/0, 4-5=-4469/0, 5-6=-4469/0, 6-7=-4793/0, 7-8=-4450/0, 8-9=-4450/0, 9-10=-3317/0, 10-11=-3317/0, 11-12=-1412/0, 12-13=-1412/0

 $22-23=0/433,\ 21-22=0/2645,\ 20-21=0/4045,\ 19-20=0/4793,\ 18-19=0/4793,\ 17-18=0/4793,\ 16-17=0/3986,\ 15-16=0/2455,\ 18-19=0/4793,\ 18-19$ **BOT CHORD**

14-15=0/0

6-19=-126/139, 7-18=-107/157, 1-22=0/1558, 2-22=-99/0, 3-22=-1216/0, 3-21=0/854, 4-21=-813/0, 4-20=0/495, 5-20=-131/103, 13-15=0/1607, 12-15=-121/0, 11-15=-1220/0, 11-16=0/1010, 10-16=-101/0, 9-16=-782/0, 9-17=0/543, 8-17=-144/113, 7-17=-835/65, 6-20=-768/125

NOTES

WEBS

Unbalanced floor live loads have been considered for this design.

- 2) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 3) CAUTION, Do not erect truss backwards.
- 4) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1554 lb down at 0-3-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Floor: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 14-23=-10, 1-13=-100

Concentrated Loads (lb) Vert: 1=-424(F)

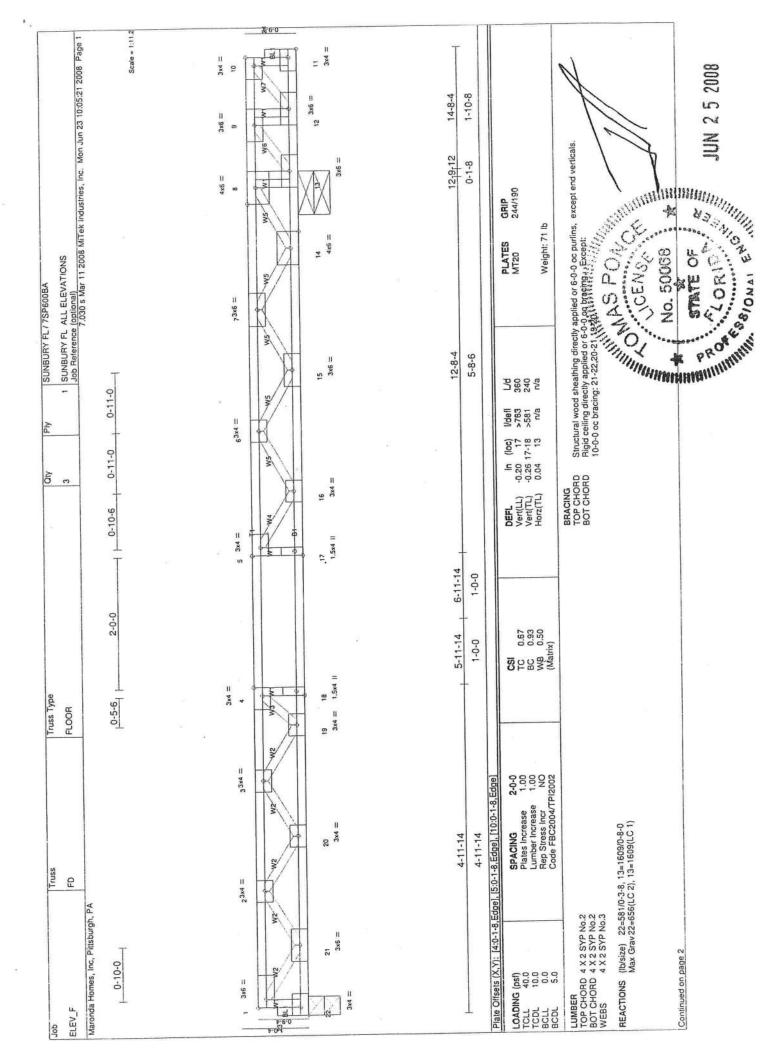
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Edenton, NC 27932 FL COA #7239

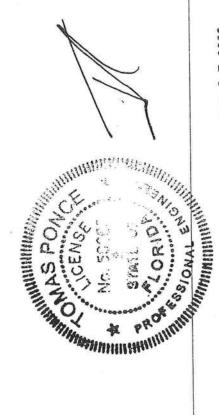
January 15,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for verify assign parameters and READ MOLES ON THE NAM INCLUDED BILLER REPRESENCE PLACE BILLS THE BEFORE DESIGN VALID FOR VIEW OF VIEW OF THE BEFORE DESIGN VALID FOR VIEW OF VIE





dol.	Truss	Truss Type	άσ	Ply SUNBURY FL / 7SP600BA
ELEV_F	ē	FLOOR	e .	1 SUNBURY FL ALL ELEVATIONS Job Reference (optional)
Maronda Homes, Inc. Pittsburgh, PA	burgh, PA			7,030 s Mar 11 2008 MiTek Industries, Inc. Man Jun 23 10:05:21 2008 Page 2
FORCES (lb) - Maximum Comp TOP CHORD 22-23=-6500, 1 BOT CHORD 21-22=-063, 20 WEBS 4-18=-304/206, 4-19=-534/316	FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 22-23=-650/0, 1-23=-648/0, 11-24=0/7, 10-24=0/7 BOT CHORD 21-22=0/63, 20-21=0/1592, 19-20=0/2512, 18-19- WEBS 4-18=-304/206, 8-13=-850/0, 5-17=-112/228, 1-21 4-19=-534/316	7, 1-2=-863/0, 2-3=-2064/0, 3-4=-2809/0, 4-5=-3010/43, 5-6=-2752/ 9=-43/3010, 17-18=-43/3010, 16-17=-43/3010, 15-16=-536/2458, 14 31=0/986, 2-21=-923/0, 2-20=0/597, 10-12=-1093/0, 9-12=0/637, 9-1	y385, 6-7=-1907/892, 4-15=-1191/1330, 13- 13=-1298/0, 3-20=-5E	Maximum Compression/Maximum Tension 22-23=-863/0, 2-3=-2064/0, 3-4=-2809/0, 4-5=-3010/43, 5-6=-2752/385, 6-7=-1907/892, 7-8=-482/1546, 8-9=0/1941, 9-10=0/873 25-25=-6480, 11-24=07, 10-24=077, 10-24=
NOTES 1) Unbalanced floor live los 2; Attach ribbon block to tr 3; Attach ribbon block to tr 4; Attach ribbon block to to 2; Attach ribbon block to tr 4; CAUTION. Do not erect 5; Hanger(s) or other conn 6; In the LOAD CASE(S) s	NOTES 1) Unbalanced floor live loads have been considered for this design. 2) Attach ribbon block to truss with 3-16d nails applied to flat face. 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d ni 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss backwards. 4) CAUTION. Do not erect truss backwards. 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 61 (6) in the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).	NOTES 1) Unbalanced floor live loads have been considered for this design. 2) Attach ribbon block to truss with 3-10d nails applied to flat face. 3) Recommend 2x8 strongbacks, on edge, spaced at 10-0.0 or and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means. 3) Recommend 2x8 strongbacks, on edge, spaced at 10-0.0 or and fastened to each truss with 3-10d nails and a spaced at 10-0.0 or and fastened to each truss backwards. 4) CAUTION. Do not erect truss backwards. 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 615 lb down at 14-6-0 on top chord. The design/selection of such connection device(s) shall be provided sufficient to support concentrated load(s) 615.	tohed to walls at their or thord. The design/si	NOTES 1) Unbalanced floor live loads have been considered for this design. 2) Attach ribbon block to truss with 3-10d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means. 3) Recommend 2x8 design edge, spaced at 10-0.0 or and fastened to each truss with 3-16d nails. Strongbacks to the action of some spaced at 10-0.0 or and fastened to each truss with 3-16d nails. Strongbacks to each spaced at 10-0.0 or and fastened to each truss with 3-16d nails. Strongbacks to each spaced at 10-0.0 or and fastened to each truss and fastened to each truss are notentially of others. 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 615 ib down at 14-6-0 on top chord. The design/selection of such connection device(s) shall be provided sufficient to support concentrated load(s) 615 ib down at 14-6-0 on top chord. The design/selection of such connection device(s) shall be provided sufficient to support concentrated load(s) 615 ib down at 14-6-0 on top chord. The design/selection of such connection device(s) shall be provided sufficient to support concentrated load(s) 615 ib down at 14-6-0 on top chord. The design/selection of such connection device(s) the face of the truss are noted as front (F) or back (B).
LOAD CASE(S) Standard 1) Floor: Lumber Increase=1.00, Plate Uniform Loads (plf) Vert: 11-22=-10, 1-10=-100 Concentrated Loads (lb) Vert: 10=-615(F)	LOAD CASE(S) Standard 1) Floor: Lumber Increase=1.00 Uniform Loads (9/1) Concentrated Loads (1b) Vert: 11-22=-10, 1-10=-100 Vert: 10=-615(F)			
		×*3		



Job	Truss	Truss Type	Qty	Ply	SUNBURY FL	E4617775
ELEV_F	FE	FLOOR	4	1	SUNBURY FL ALL ELEVATIONS Job Reference (optional)	

Marenda Homes, Inc. Pittsburgh, PA

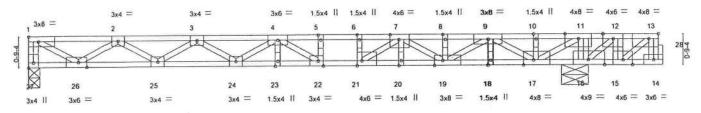
7.030 s Jan 3 2008 MiTek Industries, Inc. Tue Jan 15 12:03:18 2008 Page 1

1-0-0

0-11-6 0-10-4 0-11-6

0-7-0 0-8-0 0-1-8

Scale = 1:27.5



NOTE: DUE TO THE OVERALL CANTILEVER LENGTH THE FLOOR MAY EXHIBIT OBJECTIONABLE VIBRATION AND/OR DEFLECTION. BUILDING DESIGNER TO CONSIDER PROVIDING MEANS TO DAMPEN POSSIBLE FLOOR VIBRATION.

8-3-10 7-10-8 16-0-0 7-5-6 14-0-0 14,1,8 1-10-8 0-5-2 7-5-6 0-5-2

OADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL	40.0	Plates Increase	1.00	TC	0.95	Vert(LL)	-0.25 22	2-23	>658	360	MT20	244/190
CDL	10.0	Lumber Increase	1.00	BC	0.81	Vert(TL)	-0.32 22	2-23	>522	240		
CLL	0.0	Rep Stress Incr	NO	WB	0.69	Horz(TL)	0.04	16	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Matr	ix)	JCC-1999-1999-1993-1					Weight: 82 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.1D BOT CHORD 4 X 2 SYP No.1D

WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing, Except:

10-0-0 oc bracing: 26-27.

REACTIONS (lb/size) 27=562/0-3-8, 16=2658/0-8-0

Max Grav27=708(LC 2), 16=2658(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

1-27=-699/0, 14-28=0/6, 13-28=0/6, 1-2=-1062/24, 2-3=-2548/191, 3-4=-3309/553, 4-5=-3329/1301, 5-6=-3329/1301, TOP CHORD

6-7=-3329/1301, 7-8=-2158/2382, 8-9=-2158/2382, 9-10=-200/3578, 10-11=-200/3578, 11-12=0/4198, 12-13=0/2181

26-27=-0/0, 25-26=-71/1984, 24-25=-347/3079, 23-24=-774/3540, 22-23=-774/3540, 21-22=-1301/3329, **BOT CHORD**

20-21=-1929/2780, 19-20=-1929/2780, 18-19=-2943/1292, 17-18=-2943/1292, 16-17=-4198/0, 15-16=-2181/0,

14-15=-1/0

5-22-61/204, 6-21--354/0, 11-16=-1007/0, 1-26=-28/1225, 2-26=-1098/56, 2-25=-143/671, 3-25=-633/185, 3-24=-246/273, 4-24=-270/259, 4-23=0/135, 4-22=-952/158, 11-17=0/1722, 10-17=-101/9, 9-17=-1564/0, 9-18=**0/1**9,

9-19=0/1293, 8-19=-130/0, 7-19=-975/0, 7-20=-117/15, 7-21=0/1281, 13-15=-2667/0, 12-15=0/1493, 12-16=-2559/0

NOTES

WEBS

1) Unbalanced floor live loads have been considered for this design.

Posi-Strut webs to be applied to both sides of truss unless otherwise noted. FF=Front Face, BF=Back Face

3) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

CAUTION, Do not erect truss backwards.

5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1500 lb down at 15-9-0 on top chord. The design/selection of such connection device(s) is the responsibility of others

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Floor: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 14-27=-10, 1-13=-100

Concentrated Loads (lb) Vert: 13=-1500(F)



818 Soundside Road Edenton, NC 27932 FL COA #7239

January 15,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is lossed only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stobility during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tobiocation, quality control, storage, delivery, erection and bracing, consult. ANSI/ITII Quality Criteria, DS8-89 and 8CSI1 Building Component Safety Information.

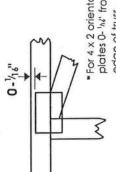


Symbols

PLATE LOCATION AND ORIENTATION



Apply plates to both sides of truss Dimensions are in ft-in-sixteenths. Center plate on joint unless x, y and fully embed teeth. offsets are indicated.



For 4 x 2 orientation, locate plates 0- 1/4" from outside edge of truss. *This symbol indicates the required direction of slots in

connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

width measured perpendicular to slots. Second dimension is the length parallel to slots. The first dimension is the plate

LATERAL BRACING LOCATION



by text in the bracing section of the output. Use T, I or Eliminator bracing Indicated by symbol shown and/or if indicated

BEARING



ndicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

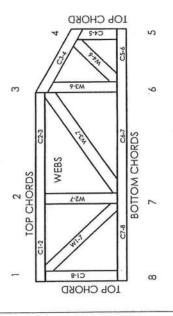
National Design Specification for Metal ANSI/IPII:

DSB-89: BCSI1:

Plate Connected Wood Truss Construction. Design Standard for Bracing. Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System





JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

NER-487, NER-561 95110, 84-32, 96-67, ER-3907, 9432A ESR-1311, ESR-1352, ER-5243, 9604B 9730, 95-43, 96-31, 9667A

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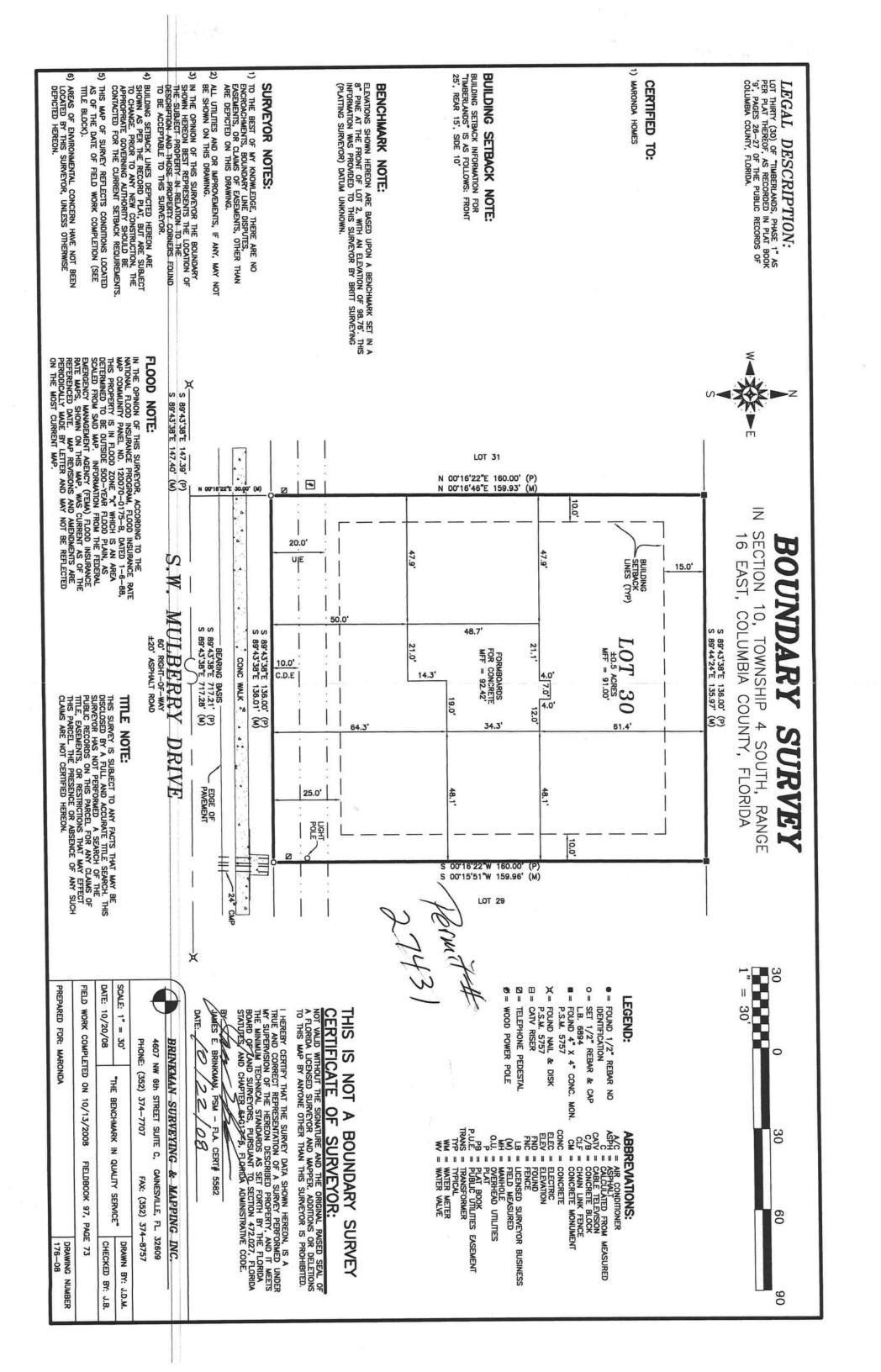


MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSII.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, 1, or Eliminator bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses. က်
- designer, erection supervisor, property owner and all other interested parties. Provide copies of this truss design to the building
- Cut members to bear tightly against each other.
- joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1. Place plates on each face of truss at each
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements
- Lumber used shall be of the species and size, and in all respects, equal to ar better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design
- Bottom chords require lateral bracing at 10 ft. spacing. or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- Do not cut or after truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or freated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use
- 19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI I Quality Criteria.



LEGAL DESCRIPTION:

LOT THIRTY (30) OF "TIMBERLANDS, PHASE 1" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGES 26-27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

1) MARONDA HOMES

CERTIFIED TO:



ROPOSED BU LAYO

IN SECTION 10, 16 EAST, COLUMBIA COUNTY, FLORIDA TOWNSHIP 4 SOUTH, RANGE



LOT 31 N 00°16'22"E 160.00' (P) 10.0 20.0 48.0 48.0 UE BUILDING - SETBACK LINES (TYP) 15.0 61.3 48.7 S 89'43'38"E 136.00' CONC ENTRY PROPOSE RESIDENCE SUNBURY-V LEFT LOT 30 ±0.5 ACRES MFF = 91.00' 21.0 89"43"38"E 136.00" (P) PROPOSED CONC PATIO 20.7 40.0 T 10.0 28.0' 61.3 C.D.E & DRAIN FIELD 25.0 48.0 S 00'16'22"W 160.00' (P) LOT 29

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

BUILDING SETBACK NOTE:

MULBERRY 60' RIGHT-OF-WAY DRIVE

FLOOD NOTE:

3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.

1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.

SURVEYOR NOTES:

ELEVATIONS SHOWN HEREON ARE BASED UPON A BENCHMARK SET IN A 8" PINE AT THE FRONT OF LOT 2, WITH AN ELEVATION OF 98.76'. THIS INFORMATION WAS PROVIDED TO THIS SURVEYOR BY BRITT SURVEYING

BENCHMARK NOTE:

ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-8B, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE

AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

BUILDING SETBACK LINES DEPICTED HEREON ARE SUBJECT SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.

TILE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

1" = 30" 30 0

FOUND 1/2" REBAR NO IDENTIFICATION FOUND 1/2" REBAR & CAP LB. 6894 SET 1/2" REBAR & CAP LB.

THIS IS NOT A BOUNDARY SURVEY CERTIFICATE OF SURVEYOR: NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED. I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STADDITES, AND CHAPTER 61877—6, FLORIDA ADMINISTRATIVE CODE. FLA. CERT# 5582

PREPARED FOR: MARONDA	FIELD WORK COMPLETED ON ****	DATE: 8/14/08	SCALE: 1" = 30'	PHONE: (35	4607 NW 6	BRINKM
	ON **** FIELDBOOK **, PAGE **	THE DESCRIPTION OF SECULATION OF SECULATION OF SECULATION OF SECULATION OF SECULATION OF SECULATION OF SECULATION OF SECURITIES SECU	THE BENCHMARK IN OHALTY SERVICE.	PHONE: (352) 374-7707 FAX: (352) 374-8757	4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609	BRINKMAN SURVEYING & MAPPING INC.
DRAWING NUMBER		CHECKED BY: J.B.	DRAWN BY: ZL	4-8757	32609	INC.