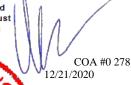
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Alpine, an ITW Company 6750 Forum Drive, Suite 305 Orlando, FL 32821 Phone: (800)755-6001 www.alpineitw.com





Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 20-4950
Job Description: Jenkins Res	
Address:	

Job Engineering Criteria:				
Design Code: FBC 2017 RES	IntelliVIEW Version: 20.01.01A			
	JRef #: 1X1b2150007			
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 37.00			
Building Type: Closed				

This package contains general notes pages, 25 truss drawing(s) and 1 detail(s).

Item	Drawing Number	Truss
1	356.20.0843.03650	A01
3	356.20.0842.46393	A03
5	356.20.0842.31823	A05
7	356.20.0842.28730	A07
9	356.20.0842.24963	A09
11	356.20.0842.13823	A11
13	356.20.0842.08703	A13
15	356.20.0842.05287	A15
17	356.20.0842.01417	B01
19	356.20.0841.58047	B03
21	356.20.0841.52333	EJ7
23	356.20.0841.53647	CJ5
25	356.20.0841.56723	CJ1

Item	Drawing Number	Truss
2	356.20.0842.59237	A02
4	356.20.0842.34157	A04
6	356.20.0842.30263	A06
8	356.20.0842.27140	A08
10	356.20.0842.22520	A10
12	356.20.0842.10477	A12
14	356.20.0842.07083	A14
16	356.20.0842.03317	A16
18	356.20.0841.59433	B02
20	356.20.0841.49337	HJ7
22	356.20.0841.50740	EJ7A
24	356.20.0841.54950	CJ3
26	BRCLBSUB0119	

## **General Notes**

## Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

## **Temporary Lateral Restraint and Bracing:**

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

## Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

## **Connector Plate Information:**

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

## Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

## **General Notes** (continued)

## **Key to Terms:**

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment. W = Width of non-hanger bearing, in inches.

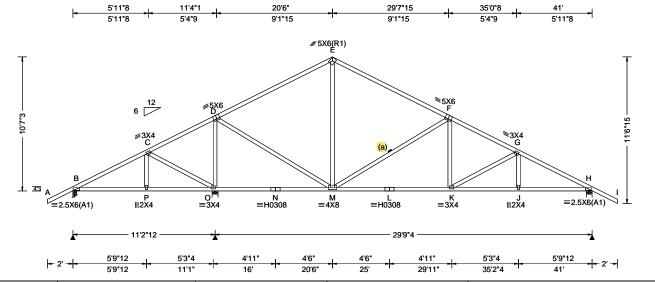
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

## References:

- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; www.iccsafe.org.
- 3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; <a href="https://www.alpineitw.com">www.alpineitw.com</a>.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.

SEQN: 3953 COMN Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 FROM: SDY Qty: 2 DrwNo: 356.20.0843.03650 Jenkins Res Truss Label: A01 / WHK 12/21/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	4
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 20.00 Soffit: 2.00	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.07 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.10 ft Loc. from endwall: not in 13.00 ft	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes	Defi/CSI Criteria	
	GCpi: 0.18 Wind Duration: 1.60	Plate Type(s): WAVE. HS	VIEW Ver: 20.01.01A.0724.12	
Lumber	1	IWAVE, 113		ב בנ

		G	ravity	-	Non-Gravity		
,	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
)	В	390	/-	/-	/217	/41	/330
	0	2018	/-	/-	/1019	/75	/-
	Н	1258	/-	/-	/750	/85	/-
	Wir	nd read	tions bas	sed on M	WFRS		
	В	Brg V	/idth = 3	.5	Min Re	q = 1.5	
	0	Brg V	/idth = 5	.5	Min Re	q = 1.5	
	Н	Brg V	/idth = 5	.5	Min Re	q = 1.5	
	Bea	ırings l	3, O, & F	ł are a rię	gid surfa	ce.	
	Mei	mbers	not listed	have fo	rces less	than 3	75#
_	Maximum Top Chord Forces Per Ply (lbs)						
	Cho	ords T	ens.Con	np. C	hords	Tens.	Comp.

▲ Maximum Reactions (lbs)

## C-D 433 - 1669 533 - 56 F-G D-E 338 - 853 G-H 446 - 2000 E-F 338 - 854

# **Bracing**

Top chord: 2x4 SP #2;

Bot chord: 2x4 SP M-31; Webs: 2x4 SP #3;

(a) Continuous lateral restraint equally spaced on member.

## Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

## Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		Chords	Tens. Comp.	
M - L	1431	- 191	K-J	1721	- 293
L-K	1431	- 191	J - H	1723	- 291

## Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.	
C-O	137 - 429	E - M	430 - 16	;
O - D	467 - 1559	M - F	305 - 901	
D - M	1155 - 190	F-K	644 0	ì

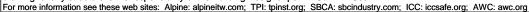


12/21/2020

\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!

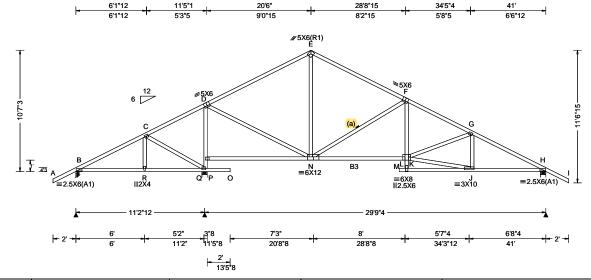
\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.





SEQN: 3995 COMN Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 FROM: SDY DrwNo: 356.20.0842.59237 Qty: 6 Jenkins Res Truss Label: A02 / WHK 12/21/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00		Pf: NA Ce: NA	VERT(LL): 0.091 K 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.170 K 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.020 E
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.040 E
NCBCLL: 20.00	Mean Height: 15.07 ft TCDL: 4.2 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.800
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.991
Spacing: 24.0 "	C&C Dist a: 4.10 ft	Rep Fac: Yes	Max Web CSI: 0.676
' '	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)	
		Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12

# Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; B3 2x4 SP M-31; Webs: 2x4 SP #3;

## **Bracing**

(a) Continuous lateral restraint equally spaced on

## **Plating Notes**

All plates are 3X4 except as noted.

## Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

	▲ Maximum Reactions (lbs)						
		G	ravity		No	n-Grav	/ity
,	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
,	В	500	/-	/-	/213	/105	/330
	Q	1691	/-	/-	/1027	/46	/-
	Н	1224	/-	/-	/801	/109	/-
	Win	d read	tions ba	ased on	MWFRS		
	В	Brg W	/idth =	3.5	Min Red	q = 1.5	;
	Q	Brg W	/idth =	5.5	Min Red	q = 1.6	i
	Н	Brg W	/idth =	5.5	Min Red	q = 1.5	;
	Bearings B, Q, & H are a rigid surface.						
	Members not listed have forces less than 375#						375#
	Maximum Top Chord Forces Per Ply (lbs)						
	Cho	rds T	ens.Co	mp.	Chords	Tens.	Ćomp.

B - C	239	- 403	E-F	410	- 933
C - D	386	- 224	F-G	560	- 1800
D-E	417	- 946	G-H	521	- 1928

## Maximum Bot Chord Forces Per Ply (lbs)

Cnoras	oras rens.Comp. Choras		is lens.	rens. Comp.		
N - K	1555 - 2	291 J-H	1676	- 354		

## Maximum Web Forces Per Ply (lbs)

vvebs	rens.comp.	vvebs	rens. C	.omp.
C-Q	123 - 461	E - N	601	- 65
Q-P	324 - 1362	N - F	341	- 950
P - D	340 - 1318	F-K	764	- 43
D - N	1009 - 50	K - J	1690	- 344



12/21/2020

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\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

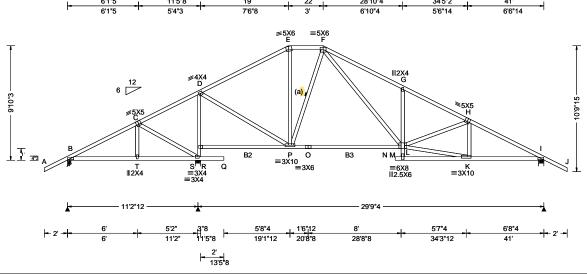
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 3992 HIPS Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 T25 FROM: SDY DrwNo: 356.20.0842.46393 Qty: 1 Jenkins Res Truss Label: A03 / WHK 12/21/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.104 G 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.193 G 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.019 E
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.038 E
NCBCLL: 20.00	Mean Height: 15.00 ft TCDL: 4.2 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.670
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.847
Spacing: 24.0 "	C&C Dist a: 4.10 ft	Rep Fac: Yes	Max Web CSI: 0.899
' '	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12

Lumbe

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; B2,B3 2x4 SP M-31; Webs: 2x4 SP #3;

## **Bracing**

(a) Continuous lateral restraint equally spaced on

## **Plating Notes**

All plates are 2.5X6(A1) except as noted.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

CSI Criteria ▲ Maximum Reactions (Ibs)								
eflection in loc L/defl L/#		Gravity				Non-Gravity		
(LL): 0.104 G 999 24		Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C(CL): 0.193 G 999 18	80	В	503	/-	/-	/232	/96	/310
Z(LL): 0.019 E -	-	S	1673	/-	/-	/1004	/47	/-
Z(TL): 0.038 E	-	I	1225	/-	/-	/801	/110	/-
Factor: 2.0		Wind reactions based on MWFRS						
TC CSI: 0.670		В	Brg V	Vidth =	3.5	Min Red	q = 1.5	
		S Brg Width = 5.5 Min Reg = 1.6						
BC CSI: 0.847		1	Bra V	Vidth =	5.5	Min Red	i = 1.5	
Veb CSI: 0.899		Bearings B, S, & I are a rigid surface.						
		Members not listed have forces less than 375#						
		Maximum Top Chord Forces Per Ply (lbs)						
Ver: 20.01.01A.0724.12		Chords Tens.Comp. Chords Tens.Comp.						
Ver: 20.01.01A.0724.12								

B-C	215	- 409	F-G	766	- 1841
D-E	441	- 897	G-H	619	- 1808
E-F	448	- 736	H-I	569	- 1930

## Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.C	omp.	Chords	Tens. Comp.		
P-0	835	- 95	K-I	1686	- 396	
0 - L	838	- 95				

## Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. (	Comp.
C-S	133 - 464	P-F	192	- 386
S - R	374 - 1350	F-L	1182	- 390
R - D	381 - 1323	L-G	268	- 375
D-P	1011 - 105	L-K	1739	- 380



12/21/2020

\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!

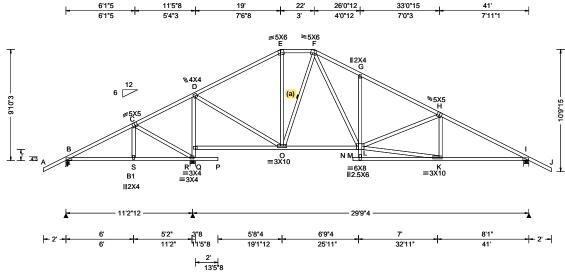
\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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SEQN: 3949 HIPS Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 T23 FROM: SDY Qty: 1 DrwNo: 356.20.0842.34157 Jenkins Res Truss Label: A04 / WHK 12/21/2020



Loading	Criteria (psf)	Wind Criteria	Snow Cri	<b>teria</b> (Pg	Pf in PSF)	Defl/CSI Cri	teria		
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection	n in loc	L/defl	L/#
TCDL:		Speed: 130 mph	Pf: NA		Ce: NA	VERT(LL):	0.089 G	999	240
BCLL:	0.00		Lu: NA	Cs: NA		VERT(CL):	0.161 G	999	180
BCDL:	10.00	Risk Category: II	Snow Dur	ation: NA		HORZ(LL):	0.018 E	-	-
Des Ld:	37 00	EXP: C Kzt: NA				HORZ(TL):	0.035 E	-	-
NCBCLL:	20.00	Mean Height: 15.00 ft TCDL: 4.2 psf	Building C	ode:		Creep Facto	r: 2.0		
Soffit:	2.00	BCDL: 5.0 psf	FBC 2017	RES		Max TC CSI	: 0.65	7	
Load Dura		MWFRS Parallel Dist: h to 2h	TPI Std:	2014		Max BC CSI	: 0.46	3	
Spacing:	24.0 "	C&C Dist a: 4.10 ft	Rep Fac:	Yes		Max Web C	SI: 0.75	3	
' '		Loc. from endwall: not in 13.00 ft	FT/RT:20	(0)/10(0)					
			Plate Type						
		Wind Duration: 1.60	WAVE			VIEW Ver: 2	0.01.01	.0724.	12

6'1"5

11'5"8

Top chord: 2x4 SP #2; Bot chord: 2x4 SP M-31; B1 2x4 SP #2; Webs: 2x4 SP #3;

## **Bracing**

(a) Continuous lateral restraint equally spaced on

## **Plating Notes**

All plates are 2.5X6(A1) except as noted.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

F)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)					
٧A	PP Deflection in loc L/defl L/#	Gravity Non-Gravity					
4	VERT(LL): 0.089 G 999 240	Loc R+ /R- /Rh /Rw /U /RL					
	VERT(CL): 0.161 G 999 180						
	HORZ(LL): 0.018 E	R 1739 /- /- /1003 /46 /-					
	HORZ(TL): 0.035 E	I 1277 /- /- /803 /112 /-					
	Creep Factor: 2.0	Wind reactions based on MWFRS					
	Max TC CSI: 0.657	B Brg Width = 3.5 Min Req = 1.5					
	Max BC CSI: 0.468	R Brg Width = 5.5 Min Req = 1.7					
	Max Web CSI: 0.756	I Brg Width = 5.5 Min Req = 1.5					
	Max Web CSI: 0.756	Bearings B, R, & I are a rigid surface.					
		Members not listed have forces less than 375#					
		Maximum Top Chord Forces Per Ply (lbs)					
	VIEW Ver: 20.01.01A.0724.12	Chords Tens.Comp. Chords Tens.Comp.					

33'0"15

B - C	223	- 410	F-G	692	- 1607
D-E	450	- 995	G-H	562	- 1628
E-F	456	- 805	H - I	573	- 2000

# Maximum Bot Chord Forces Per Ply (lbs)

Cilolus	16113.0	onip.	Cilolus	rens. Comp.		
0 - L	924	- 86	K-I	1709	- 388	

## Maximum Web Forces Per Ply (lbs)

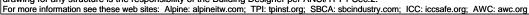
Webs	Tens.Comp.	Webs	Tens. (	Comp.
C-R	134 - 463	0 - F	138	- 376
R - Q	369 - 1428	F-L	1041	- 358
Q - D	380 - 1405	L-K	1704	- 380
D - O	1044 - 106	I - H	161	- 437



\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!

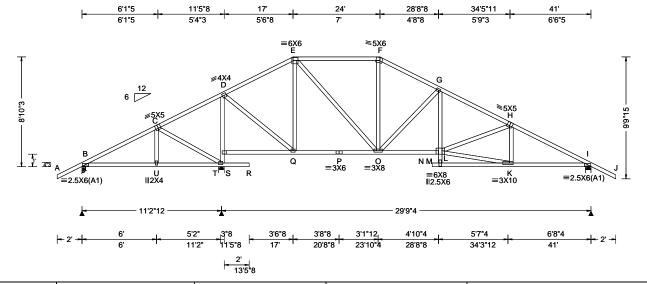
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SEQN: 3989 HIPS Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 FROM: SDY Qty: 1 DrwNo: 356.20.0842.31823 Jenkins Res Truss Label: A05 / WHK 12/21/2020



Loading Criteria (psf) Wind Criteria		Snow Criteria (Pg,Pf in PSF)		DefI/CSI Criteria					
TCLL: 2	20.00	Wind Std: ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection	n in loc L	/defl	L/#
TCDL:		Speed: 130 mph	Pf: NA		Ce: NA	VERT(LL):	0.091 L	999	240
BCLL: (	0.00	Enclosure: Closed	Lu: NA	Cs: NA		VERT(CL):	0.169 L	999	180
BCDL: 1		Risk Category: II	Snow Dur	ation: NA		HORZ(LL):	0.018 K	-	-
Des Ld: 3	27 00	EXP: C Kzt: NA				HORZ(TL):	0.034 K	-	-
NCBCLL: 2	20.00	Mean Height: 15.00 ft TCDL: 4.2 psf	Building C	ode:		Creep Factor	r: 2.0		
Soffit: 2		BCDL: 5.0 psf	FBC 2017	RES		Max TC CSI:	0.490		
Load Durati		MWFRS Parallel Dist: h to 2h	TPI Std:	2014		Max BC CSI:	0.874		
Spacing: 24	4.0 "	C&C Dist a: 4.10 ft	Rep Fac: '	Yes		Max Web CS	SI: 0.744		
		Loc. from endwall: not in 13.00 ft	FT/RT:20(	(0)/10(0)					
		GCpi: 0.18	Plate Type	e(s):					
		Wind Duration: 1.60	WAVE			VIEW Ver: 20	0.01.01A.	0724.	12

## Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

## **Plating Notes**

All plates are 3X4 except as noted.

## Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Defl/CSI Criteria				laximu	ım Rea	actions (II	bs)		
PP Deflection in loc L	/defl	L/#		G	ravity		No	on-Grav	∕ity
VERT(LL): 0.091 L	999	240	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RI
VERT(CL): 0.169 L	999	180	В	508	/-	/-	/250	/93	/283
HORZ(LL): 0.018 K	-	-	Т	1662	/-	/-	/978	/48	/-
HORZ(TL): 0.034 K	-	-	1	1230	/-	/-	/800	/116	/-
Creep Factor: 2.0			Wir	nd reac	tions b	ased on N	<b>MWFRS</b>		
Max TC CSI: 0.490			В	Brg W	/idth =	3.5	Min Re	q = 1.5	;
Max BC CSI: 0.874			Т		/idth =		Min Re		
			1	Brg W	/idth =	5.5	Min Re	q = 1.5	;
Max Web CSI: 0.744			Bea	ırings E	3, T, &	I are a rig	jid surfac	e.	
			Mei	mbers	not list	ed have fo	orces les	s than 3	375#

Chords	Tens.Comp.	Chords	Tens. Comp.		
B-C	230 - 420	F-G	610 - 1208		
ь г	470 770	C 11	744 4700		

Maximum Top Chord Forces Per Ply (lbs)

/RL

/283

D - E	4/8 - //3	G-H	711 - 1799
E-F	603 - 1034	H-I	654 - 1946

Maximum Bot Chord Forces Per Ply (lbs)										
Chords Tens.Comp.			Chords	Tens. Comp						
Q-P	635	- 98	0 - L	1544	- 419					

Q - P P - O	 	O-L K-I	1544 1693	- 419 - 474

Maximum Web Forces Per Ply (lbs)									
Webs	Tens.Comp.	Webs	Tens. 0	Comp.					
C-T	156 - 471	E-O	615	- 195					
T - S	425 - 1318	0 - F	419	-6					
S - D	426 - 1301	O - G	277	- 742					
D - Q	973 - 203	G-L	695	- 88					
E - Q	197 - 493	L-K	1674	- 469					



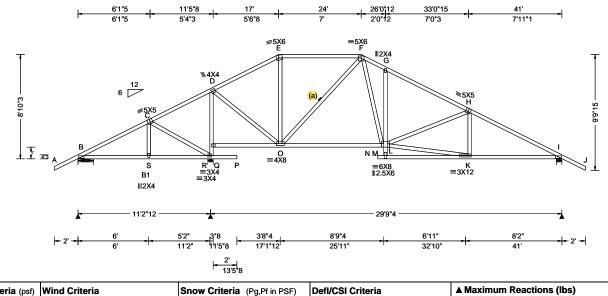
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SEQN: 3942 HIPS Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 T17 FROM: SDY Qty: 1 DrwNo: 356.20.0842.30263 Jenkins Res Truss Label: A06 / WHK 12/21/2020



Loading Criteria (psf) Wind Criteria		Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.087 G 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.156 G 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.015 E
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.029 E
NCBCLL: 20.00	Mean Height: 15.00 ft TCDL: 4.2 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.536
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.475
Spacing: 24.0 "	C&C Dist a: 4.10 ft	Rep Fac: Yes	Max Web CSI: 0.845
' "	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12

ט	320	/-	/-	/202	/03	/203	- 1		
R	1728	/-	/-	/964	/50	/-	-		
1	1295	/-	/-	/801	/116	/-	-		
Wi	Wind reactions based on MWFRS								
В	B Brg Width = 16.0 Min Req = 1.5								
R	Brg Width = 5.5 Min Reg = 1.7					-			
1	Brg Width = 5.5 Min Req = 1.5								
Bea	Bearings B, R, & I are a rigid surface.								
Members not listed have forces less than 375#									
Maximum Top Chord Forces Per Ply (lbs)									

/Rh

/\_

Non-Gravity

/RL

/283

/Rw /U

Gravity

1\_

Loc R+

R

520

Tens. Comp. Chords Tens.Comp. Chords B - C 740 - 1612 230 - 391 D-E 476 G-H 645 - 1673 - 904

## E-F 650 - 2040 - 753 472 H - I

(a) Continuous lateral restraint equally spaced on

## **Plating Notes**

All plates are 2.5X6(A1) except as noted.

Top chord: 2x4 SP #2; Bot chord: 2x4 SP M-31; B1 2x4 SP #2; Webs: 2x4 SP #3;

Lumber

**Bracing** 

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

# Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		Chords	Tens. Comp.		
B-S	577	- 189	K - I	1745	- 457	
0 - L	1165	- 241				

## Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.		
C-R	158 - 433	0 - F	220	-617	
R-Q	426 - 1416	F-L	949	- 270	
Q - D	422 - 1430	L-K	1770	- 441	
D - O	1086 - 201	L-H	155	- 429	



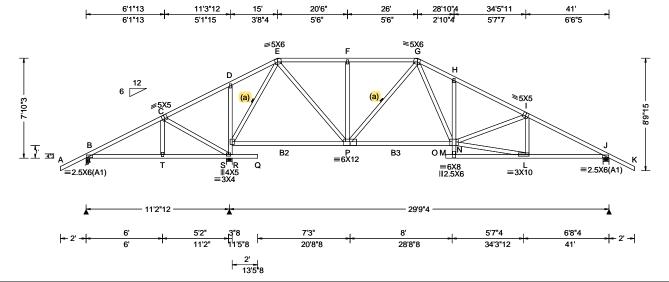
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SEQN: 3983 HIPS Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 T16 FROM: SDY DrwNo: 356.20.0842.28730 Qty: 1 Jenkins Res Truss Label: A07 / WHK 12/21/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.091 H 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.170 H 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.015 S
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.029 L
NCBCLL: 20.00	Mean Height: 15.00 ft TCDL: 4.2 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.370
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.847
Spacing: 24.0 "	C&C Dist a: 4.10 ft	Rep Fac: Yes	Max Web CSI: 0.665
' '	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12

Lumbe

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; B2,B3 2x4 SP M-31; Webs: 2x4 SP #3;

## **Bracing**

(a) Continuous lateral restraint equally spaced on

## **Plating Notes**

All plates are 2X4 except as noted.

## Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

<b>▲</b> N	/laximu	ım Rea	ctions (I	bs)		
	G	ravity		No	on-Grav	vity
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
В	511	/-	/-	/267	/90	/256
S	1658	/-	/-	/947	/45	/-
J	1235	/-	/-	/793	/119	/-
Wi	nd reac	tions b	ased on I	MWFRS		
В	Brg V	Vidth =	3.5	Min Re	q = 1.5	;
S	Brg V	Vidth =	5.5	Min Re	q = 1.6	;
J	Brg V	Vidth =	5.5	Min Re	q = 1.5	j
Be	arings l	B, S, &	J are a ri	igid surfac	ce.	
Me	Members not listed have forces less than 375#					375#
Ma	ximum	Top C	hord Fo	rces Per	Ply (lb	s)
Ch	ords T	ens.Co	omp.	Chords	Tens.	Ćomp.

B-C	246 - 427	G-H	872	- 1790
E-F	651 - 1071	H - I	771	- 1818
F-G	651 - 1071	I - J	706	- 1959

## Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. (	Comp.
R - P P - M	483 - 113 1226 - 348		1696	- 520

## Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. (	Comp.
C-S	155 - 469	E-P	1036	- 307
S - R	455 - 1293	G - M	913	- 300
R-E	337 - 1169	M - L	1716	- 508



12/21/2020

\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!

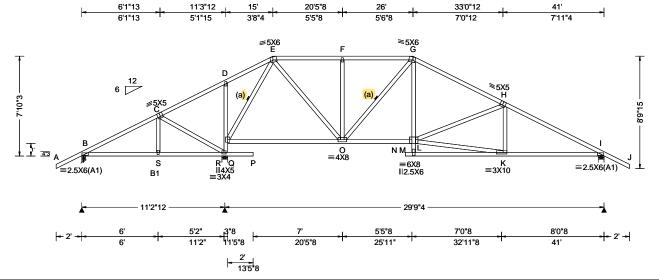
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SEQN: 3938 HIPS Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 T22 FROM: SDY DrwNo: 356.20.0842.27140 Qty: 1 Jenkins Res Truss Label: A08 / WHK 12/21/2020



Loading (	Criteria (psf)	Wind Criteria	Snow Cri	teria (Pg	Pf in PSF)	Defl/CSI Crit	eria		
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection	n in loc L	_/defl	L/#
TCDL:		Speed: 130 mph	Pf: NA		Ce: NA	VERT(LL): (	0.085 L	999	240
BCLL:	0.00		Lu: NA	Cs: NA		VERT(CL): (	0.150 L	999	180
BCDL:	10.00	Risk Category: II	Snow Dur	ation: NA		HORZ(LL): (	0.016 E	-	-
Des Ld:	37 00	EXP: C Kzt: NA				HORZ(TL): (	0.030 E	-	-
NCBCLL:	20.00	Mean Height: 15.00 ft TCDL: 4.2 psf	Building C	ode:		Creep Factor	: 2.0		
Soffit:	2.00	BCDL: 5.0 psf	FBC 2017	RES		Max TC CSI:	0.531		
Load Dura		MWFRS Parallel Dist: h to 2h	TPI Std:	2014		Max BC CSI:	0.503		
Spacing: 2	24.0 "	C&C Dist a: 4.10 ft	Rep Fac:	Yes		Max Web CS	I: 0.670		
-		Loc. from endwall: not in 13.00 ft	FT/RT:20	(0)/10(0)					
		GCpi: 0.18	Plate Type	e(s):					
		Wind Duration: 1.60	WAVE			VIEW Ver: 20	).01.01A.	0724.	12

## Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP M-31; B1 2x4 SP #2; Webs: 2x4 SP #3;

## **Bracing**

(a) Continuous lateral restraint equally spaced on

## **Plating Notes**

All plates are 2X4 except as noted.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

	▲ Max	imum R	eactions (	lbs)		
L/defl L/#		Gravity	/	No	n-Grav	/ity
999 240	Loc F	R+ /R-	/ Rh	/ Rw	/ U	/ RL
999 180	B 49	7 /-	/-	/268	/91	/255
		06 /-	/-	/946	,	/-
	I 12	90 /-	/-	/794	/118	/-
	Wind r	eactions	based on	MWFRS		
1	B Bi	g Width	= 3.5	Min Re	q = 1.5	
3	R Bi	g Width	= 5.5	Min Re	g = 1.8	
-	I Br	g Width	= 5.5	Min Re		
0	Bearin	gs B, R,	& I are a ri	gid surfac	ė.	
	Memb	ers not li	sted have f	orces less	than 3	375#
	Maxim	num Top	Chord Fo	rces Per	Ply (lbs	s)
A.0724.12	Chord:	s Tens.	Comp.	Chords	Tens.	Ćomp.

B - C	250 - 395	G-H	700 - 1650
E-F	650 - 1191	H - I	703 - 2032
E C	GEO 1101		

## Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.C	Comp.	Chords	Tens. (	Comp.	
Q - O O - L			K - I	1738	- 506	

## Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. (	Comp.
C-R	155 - 469	G-L	723	- 106
R-Q	451 - 1477	L-K	1705	- 501
Q-E	336 - 1299	L-H	158	- 445
E - O	1038 - 302			



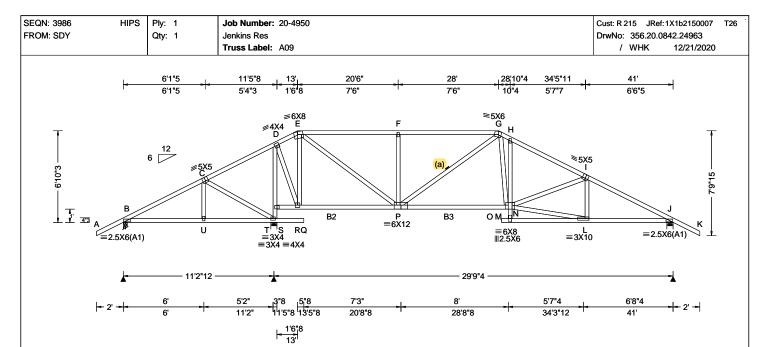
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.096 H 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.178 H 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.016 L
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.031 L
NCBCLL: 20.00	Mean Height: 15.00 ft TCDL: 4.2 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.592
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.846
Spacing: 24.0 "	C&C Dist a: 4.10 ft	Rep Fac: Yes	Max Web CSI: 0.772
' "	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12
Lumber			

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/Rw	/ U	/ RL
В	505	/-	/-	/277	/77	/228
т -	1663	/-	/-	/924	/111	/-
J	1230	/-	/-	/778	/134	/-
Wind reactions based on MWFRS						
В	Brg W	idth = 3.	5	Min Re	q = 1.5	
T Brg Width = 5.5 Min Req = 1.6						
J	Brg W	idth = 5.	5	Min Re	q = 1.5	
Bearings B, T, & J are a rigid surface.						
Members not listed have forces less than 375#						
Maximum Top Chord Forces Per Ply (lbs)						
Cho	rds T	ens.Com	np. Ch	nords	Tens.	Comp.

▲ Maximum Reactions (lbs)

## B - C 846 - 1718 220 - 413 G - H E-F 718 - 1255 H - I 767 - 1805 710 - 1946 F-G 718 - 1255

## **Bracing**

(a) Continuous lateral restraint equally spaced on

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; B2,B3 2x4 SP M-31; Webs: 2x4 SP #3;

## **Plating Notes**

All plates are 2X4 except as noted.

## Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

## Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. (	Comp.
P - M	1454 - 422	L-J	1693	- 524

## Maximum Web Forces Per Ply (lbs)

vvebs	rens.Comp.	webs	rens. (	Jomp.
 С-Т	173 - 471	E-P	1290	- 480
T - S	484 - 1318	F-P	375	- 453
S - D	457 - 1328	G - M	810	- 252
D-R	1065 - 336	M - L	1721	- 511
E-R	371 - 936			



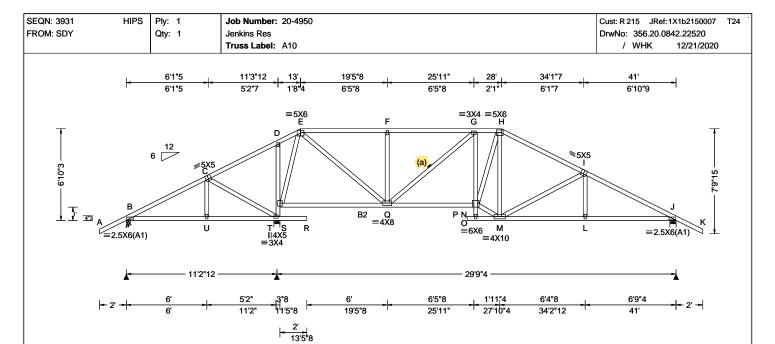
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.094 P 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.171 P 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.019 Q
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.035 Q
NCBCLL: 20.00	Mean Height: 15.00 ft TCDL: 4.2 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.443
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max BC CSI: 0.882
Spacing: 24.0 "	C&C Dist a: 4.10 ft	Rep Fac: Yes	Max Web CSI: 0.756
	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12
Lumber			•

1 1733	/- /-	1922 1210 1-				
J 1236	/- /-	/780 /299 /-				
Wind read	ctions based o	n MWFRS				
B Brg V	Vidth = 3.5	Min Req = 1.5				
T Brg V	Vidth = 5.5	Min Req = 1.7				
J Brg V	Vidth = 5.5	Min Req = 1.5				
Bearings	Bearings B, T, & J are a rigid surface.					
Members not listed have forces less than 375#						
Maximum Top Chord Forces Per Ply (lbs)						
Chords 1	Tens.Comp.	Chords Tens. Comp.				

Non-Gravity

/175

/RL

/228

/Rw / U

/022

▲ Maximum Reactions (lbs) Gravity

/Rh

1\_

Loc R+

1735 1\_

В

## B - C - 1495 241 - 387 G - H 756 E-F 698 - 1180 H - I 663 - 1438 F-G - 1951 698 - 1179 720

## **Bracing**

(a) Continuous lateral restraint equally spaced on

## **Plating Notes**

All plates are 2X4 except as noted.

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; B2 2x4 SP M-31; Webs: 2x4 SP #3;

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

# Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. C	omp.
Q - N	1499 - 456	L - J	1683	- 531
M - L	1674 - 533			

## Maximum Web Forces Per Ply (lbs)

vvebs	rens.Comp.	webs	rens. (	Jomp.
C - T	169 - 469	N - M	1423	- 401
T - S	470 - 1365	N - H	846	- 370
S-E	396 - 1200	M - H	166	- 403
E - Q	1293 - 446	M - I	208	- 543
F-Q	341 - 394	I-L	399	0
Q-G	117 - 425			



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FROM: SDY Qty: 1 DrwNo: 356.20.0842.13823 Jenkins Res Truss Label: A11 / WHK 12/21/2020 5'11" 17'6"5 23'7"7 30' 35'2"12 11 5'11' 5'1" 6'6"5 6'1"1 6'4"9 5'2"12 5'9"4 =6X8 D ≡5X6 ∥2<u>X</u>4 ≡5X6 G **∌**3X4 C 5'10"3 15 6'9". ≡3X8 ≡3X6 O N ≡4X5 В2 Q |||2X4 \_\_\_\_K =3X4  $\equiv 2.5 \times 6(A1)$ €2.5X6(A1) =3X5 =3X6 11'2"12 29'9"4 1'4<u>"</u>9 25 5'9"4 5'2"12 5' 1'2|"13 6'4"9 5' 11' 5'9"4 11' 16' 17'2"13 23'7"7 30 41

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00		Pf: NA Ce: NA	VERT(LL): 0.072 F 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.135 F 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.014 K
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.026 K
NCBCLL: 20.00	Mean Height: 15.00 ft TCDL: 4.2 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.580
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.646
Spacing: 24.0 "	C&C Dist a: 4.10 ft	Rep Fac: Yes	Max Web CSI: 0.989
' '	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)	
		Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12
Lumber			•

Job Number: 20-4950

	Gravity			No.	n-Grav	/ity
Lo	c R+	/ R-	/ Rh	/ Rw	/ U	/ RL
В	363	/-72	/-	/200	/52	/201
Р	1998	/-	/-	/1008	/194	/-
1	1151	/-	/-	/709	/110	/-
Wind reactions based on MWFRS						
В	Brg V	Vidth =	3.5	Min Re	q = 1.5	;
Р	Brg V	Vidth =	5.5	Min Re	q = 1.5	;
1	Brg V	Vidth =	5.5	Min Re	q = 1.5	,
Bearings B, P, & I are a rigid surface.						
Members not listed have forces less than 375#						
Maximum Top Chord Forces Per Ply (lbs)						

▲ Maximum Reactions (lbs)

Chords Tens.Comp.

Cust: R 215 JRef: 1X1b2150007

## F-G B - C 465 - 182 593 - 1154 C-D 797 - 237 G-H 567 - 1440 649 - 1743 D-E 348 - 551 H - I F-F 592 - 1153

Chords

Tens. Comp.

## **Bracing**

Top chord: 2x4 SP #2;

SEQN: 3969

HIPS

Ply: 1

(a) Continuous lateral restraint equally spaced on

Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

## Wind

Wind loads based on MWFRS with additional C&C

Wind loading based on both gable and hip roof types.

## Maximum Bot Chord Forces Per Ply (lbs)

Choras	rens.c	omp.	Choras	rens. v	Jomp.
B - Q Q - P		- 394 - 397	N - M M - I	598 1265	- 112 - 281
P-O O-N	423	- 668 - 668	L-K K-I	1265 1265 1504	- 281 - 473
0-11	423	- 000	IX = 1	1304	-4/3

# Maximum Web Forces Per Ply (lbs)

webs	Tens.Comp.	webs	Tens. (	Jomp.
C - P	226 - 495	N - E	511	- 912
D - P	724 - 1631	E - M	770	- 336
D - N	1563 - 662	G - K	664	- 4



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SEQN: 3927 HIPS Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 FROM: SDY DrwNo: 356.20.0842.10477 Qty: 1 Jenkins Res Truss Label: A12 / WHK 12/21/2020 5'11' 11' 18'7"4 25'11" 35'2"12 41 30' 5'11' 5'1" 7'7"4 7'3"12 4'1" 5'2"12 5'9"4 ≡6X8 D ≡5X6 E 5'10"3 6.9 =4X10 S ∥2X4 ≡6X8 ∥2X4 K ≡4X10 =2.5X6(A1) =2.5X6(A1) 11'2"12 29'9"4 11'1"12 5'9"4 5'2"12 7'3"12 3'11"4 - 2' <del>--</del> 5'9"4 **Դ**Կ։3"8 18'7"4 25'11" 29'10"4 41' 2'2" 13'5"8

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA	PP Deflection in loc L/defl L/# VERT(LL): 0.108 N 999 240 VERT(CL): 0.201 N 999 180 HORZ(LL): -0.024 O -	
Des Ld: 37.00 NCBCLL: 20.00 Soffit: 2.00	EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 4.10 ft	Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0)	HORZ(TL): 0.044 O Creep Factor: 2.0  Max TC CSI: 0.703  Max BC CSI: 0.650  Max Web CSI: 0.981	
Lumber	Loc. from endwall: not in 6.50 ft GCpi: 0.18 Wind Duration: 1.60	Plate Type(s):	VIEW Ver: 20.01.01A.0724.12	

▲ M	▲ Maximum Reactions (lbs)					
	Gravity Non-Gravity					
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
В	505	/-	/-	/288	/168	/201
R	1667	/-	/-	/888	/280	/-
1	1236	/-	/-	/764	/299	/-
Wind reactions based on MWFRS						
В	Brg W	/idth = 3	.5	Min Re	q = 1.5	;
R	Brg V	/idth = 5	.5	Min Re	q = 1.5	;
1	Brg V	/idth = 5	.5	Min Re	q = 1.5	i
Bearings B, R, & I are a rigid surface.						
Members not listed have forces less than 375#						
Maximum Top Chord Forces Per Ply (lbs)						
Cho	rds T	ens.Cor	np.	Chords	Tens.	Ćomp.

B - C	239 - 430	F-G	916	- 1827
D-E	769 - 1347	G-H	695	- 1610
E-F	769 - 1346	H - I	784	- 1931

## Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

K-I

1672

1498

689

- 595

- 385

1835 - 650

503 - 1272

1772

O - L

Maximum Web Forces Per Ply (lbs)					
Webs	Tens.Comp.	Webs	Tens. Comp.		
C-R	219 - 484	E-O	405 - 481	Ī	
P - O	/Q7 <sub>-</sub> 1200	O - F	212 - 579	2	

# **Bracing**

(a) Continuous lateral restraint equally spaced on

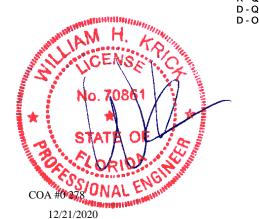
## **Plating Notes**

Top chord: 2x4 SP #2; Bot chord: 2x4 SP M-31; Webs: 2x4 SP #3;

All plates are 3X4 except as noted.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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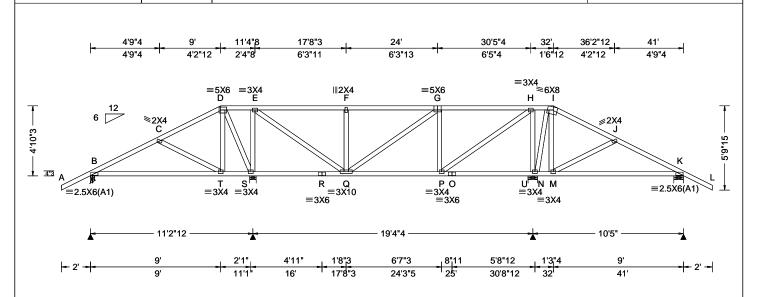
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 3964 HIPS Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 T10 FROM: SDY DrwNo: 356.20.0842.08703 Qty: 1 Jenkins Res Truss Label: A13 / WHK 12/21/2020



Loading Criteria (psf) V	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 V	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity
TCDL: 7.00 S	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.022 F 999 240	Loc R+ /R- /Rh /
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.042 F 999 180	B 452 /- /- /2
BCDL: 10.00 F	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.011 M	S 1343 /- /- /7
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.022 M	U 1289 /- /- /6
NCBCLL 20 00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	K 426 /- /- /2
0-#:4	TCDL: 4.2 psr	FBC 2017 RES	Max TC CSI: 0.465	Wind reactions based on MWF
	BCDL: 5.0 psr	TPI Std: 2014	Max BC CSI: 0.943	B Brg Width = 3.5 Min
	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.406	S Brg Width = 5.5 Min
	COC DISCA. 4. TO IL	·	Wax Web CSI. 0.406	U Brg Width = 8.0 Mi
	Loc. from endwall: not in 13.00 ft			K Brg Width = 8.0 Mi
	GCpi: 0.18	Plate Type(s):		Bearings B, S, U, & K are a rigi
V	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12	Members not listed have forces

Glavity				Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
В	452	/-	/-	/280	/17	/174
S	1343	/-	/-	/709	/159	/-
U	1289	/-	/-	/652	/151	/-
K	426	/-	/-	/299	/50	/-
Wii	nd reac	tions b	ased on I	MWFRS		
В	Brg V	Vidth =	3.5	Min Re	q = 1.5	;
s	Brg V			Min Re	q = 1.5	;
U	Brg V	Vidth =	8.0	Min Re	q = 1.5	;
K	Brg V	Vidth =	8.0	Min Re	q = 1.5	;
Bearings B, S, U, & K are a rigid surface.						
Maximum Top Chord Forces Per Ply (lbs)						
Ch	ords T	ens.Co	omp.	Chords	Tens.	Comp.
U Brg Width = 8.0 Min Req = 1.5 K Brg Width = 8.0 Min Req = 1.5 Bearings B, S, U, & K are a rigid surface. Members not listed have forces less than 375#						

G-H

Non-Gravity

248

- 506

Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

## Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

## Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp.

Q-P 512

E - F

F-G

## Maximum Web Forces Per Ply (lbs)

234 - 510

234 - 510

16115.0	onip.	MEDS	i elis. v	Jonnp.
515	- 30	P-H	839	- 326
156	- 576	H - N	494	- 824
482	- 824	N - I	96	- 586
894	- 349	I - M	554	- 19
	515 156 482	515 - 30 156 - 576 482 - 824 894 - 349	515 - 30 P - H 156 - 576 H - N 482 - 824 N - I	515 -30 P-H 839 156 -576 H-N 494 482 -824 N-I 96



12/21/2020

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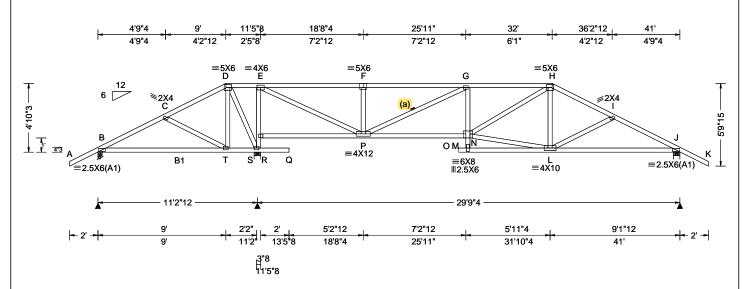
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SEQN: 3921 HIPS Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 T5 FROM: SDY DrwNo: 356.20.0842.07083 Qty: 1 Jenkins Res Truss Label: A14 / WHK 12/21/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.139 O 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.260 O 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.029 H
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.055 H
NCBCLL: 20.00	Mean Height: 15.00 ft TCDL: 4.2 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.609
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max BC CSI: 0.885
Spacing: 24.0 "	C&C Dist a: 4.10 ft	Rep Fac: Yes	Max Web CSI: 0.924
' "	Loc. from endwall: not in 6.50 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12

<b>▲</b> N	▲ Maximum Reactions (lbs)						
	G	ravity		No	on-Grav	/ity	
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
В	488	/-	/-	/285	/136	/174	
S	1722	/-	/-	/877	/329	/-	
J	1208	/-	/-	/729	/287	/-	
Wi	Wind reactions based on MWFRS						
B Brg Width = 3.5 Min Req =			q = 1.5	;			
S Brg Width = 5.5 Min Reg = 1.7					•		
J	Brg V	/idth =	5.5	Min Re	q = 1.5	i	
Be	Bearings B, S, & J are a rigid surface.						
Members not listed have forces less than 375#							
Maximum Top Chord Forces Per Ply (lbs)							
Ch	Chords Tens.Comp. Chords Tens. Comp.						

## Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP M-31; B1 2x4 SP #2; Webs: 2x4 SP #3;

## **Bracing**

(a) Continuous lateral restraint equally spaced on

## **Plating Notes**

All plates are 3X4 except as noted.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

B-C	196 - 385	G-H	1079	- 2248
E-F	766 - 1559	H-I		- 1696
F-G	766 - 1559	I-J	760	- 1910

## Maximum Bot Chord Forces Per Ply (lbs)

Cnoras	rens.Comp.	Choras	rens. (	Jomp.
P - M	2267 - 853	L - J	1655	- 568

## Maximum Web Forces Per Ply (lbs)

webs	rens.Comp.	vvebs	rens. (	.omp
D - T	527 - 36	F-P	311	- 410
D-S	74 - 596	P-G	366	-812
S - R	637 - 1285	M - L	1494	- 421
R-E	637 - 1238	M - H	915	- 462
E-P	2027 - 731			

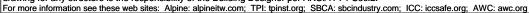


12/21/2020

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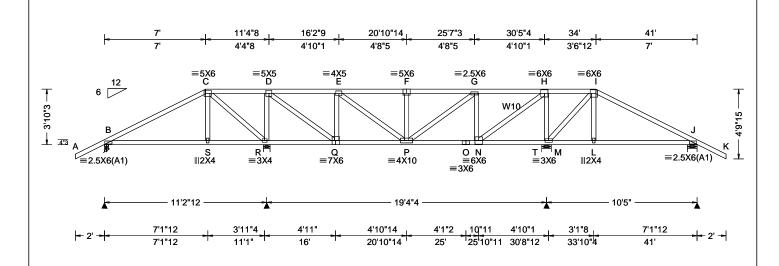
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SEQN: 3972 HIPS Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 T11 FROM: SDY Qty: 1 DrwNo: 356.20.0842.05287 Jenkins Res Truss Label: A15 / WHK 12/21/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.081 F 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.150 F 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.018 L
Des Ld: 37.00	EXP: C Kzt: NA Mean Height: 15.00 ft		HORZ(TL): 0.035 L
NCBCLL: 20.00	TCDL: 4.2 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.922
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.913
Spacing: 24.0 "	C&C Dist a: 4.10 ft	Rep Fac: Varies by Ld Case	Max Web CSI: 0.814
	Loc. from endwall: not in 13.00 ft		
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12

ı	umbo	

Top chord: 2x4 SP #2:

Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; W10 2x4 SP #2;

## **Special Loads**

(Lumber	Dur.Fac.=1	.25 / Plate [	Our.Fac.=1.2	25)
TC: From	56 plf at	-2.00 to	56 plf at	20.91
TC: From	28 plf at	20.91 to	28 plf at	34.00
TC: From	56 plf at	34.00 to	56 plf at	43.00
BC: From	4 plf at	-2.00 to	4 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	21.06
BC: From	10 plf at	21.06 to	10 plf at	33.97
BC: From	20 plf at	33.97 to	20 plf at	41.00
BC: From	4 plf at	41.00 to	4 plf at	43.00
TC: 164 lb	Conc. Load	at 21.94,23	3.94,25.94,2	7.94
29.94,31.94				
TC: 399 lb	Conc. Load	at 33.97		

955 lb Conc. Load at 21.06 186 lb Conc. Load at 21.94,23.94,25.94,27.94 BC: BC:

29.94,31.94

BC: 499 lb Conc. Load at 33.97

## Wind

Wind loads and reactions based on MWFRS

Wind loading based on both gable and hip roof types.

Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria				
Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#				
Pf: NA Ce: NA	VERT(LL): 0.081 F 999 240				
Lu: NA Cs: NA	VERT(CL): 0.150 F 999 180				
Snow Duration: NA	HORZ(LL): -0.018 L				
	HORZ(TL): 0.035 L				
Building Code:	Creep Factor: 2.0				
FBC 2017 RES	Max TC CSI: 0.922				
TPI Std: 2014	Max BC CSI: 0.913				
Rep Fac: Varies by Ld Case	Max Web CSI: 0.814				
FT/RT:20(0)/10(0)					
Plate Type(s):					
I	VIEW Ver 20 04 04 A 0724 42				

-	Т	4018	/-	/-	/-	/892	/-
	J	394	/-	7-	<i>J</i> -	/98	/-
	Wir	nd reac	tions	based on	MWFRS		
	В	Brg W	/idth =	= 3.5		q = 1.5	
	R	Brg W	/idth =	= 5.5	Min Re	q = 2.2	<u>'</u>
	Т	Brg W			Min Re	q = 4.7	•
	J	Brg W	/idth =	= 8.0	Min Re	q = 1.5	,
	Bea	arings (	3, R, <sup>-</sup>	Γ, & J are	a rigid su	rface.	
2	Mei	mbers	not lis	ted have	forces les	s than 3	375#
	Ma	ximum	Тор	Chord F	orces Per	Ply (lb:	s)
	Cho	ords T	ens.C	Comp.	Chords	Tens.	Com

479 - 2102

▲ Maximum Reactions (lbs) Gravity

/Rh

Loc R+

В 340

R 2223

ens. Comp. 479 - 2102 B - C 581 - 146 F-G 249 C - D 1009 - 235 G-H - 1134 - 735 D - E 158 H - I 1339 - 301

Non-Gravity

/70 /-

/497 /-

/RL

/Rw /U

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords B - S 101 1232 - 286 S - R 114 - 493 O - N 1232 - 286 R - Q 201 - 888 N - M 245 - 1153 Q-P 835 - 185

## Manual Ma Maximum Web Forces Per Ply (lbs) Tens. Comp. Tens.Comp. Webs Webs C-S P-G 383 0 1095 - 242 G-N C - R 175 - 883 459 - 1213 425 - 1567 2887 R-D N - H - 627 D - Q 2061 - 455 H - M 673 - 2299 Q-E 340 - 1223 M - I 368 - 1820 E-P 1595 - 368 L-I 669

E-F

COA #0278 ONAL

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SEQN: 3917 HIPS Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 T2 FROM: SDY DrwNo: 356.20.0842.03317 Qty: 1 Jenkins Res Truss Label: A16 / WHK 12/21/2020 11'7" 18'8"4 25'11" 29'9"4 34 41 7'1"4 7'2"12 3'10"4 4'2"12 **≡4X8** ≡5X6 C ≡5X10 D ≡5X6 E =4<u>X</u>6 **#7**X6 H T2 T3 G WQ 3'10"3 -W4 W3 =8x14 В1 Т Κ =4X8(B3) ≡SS1017 €2.5X6(A1) ≡H1021 =4X6 11'2"12 29'9"4 7'1"12 7'2"12 3'10"12 7'1"12 4'0"4 3"8 5'2"12 4'0"8 7'1"12 11'2" 11'5"8 18'8"4 25'11" 29'11"8 33'10"4 41 13'5"8

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.405 O 878 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.771 O 462 180
BCDL: 10.00	Risk Category: II EXP: C Kzt: NA	Snow Duration: NA	HORZ(LL): -0.060 H
Des Ld: 37.00	Mean Height: 15.00 ft		HORZ(TL): 0.113 H
NCBCLL: 0.00	TCDL: 4.2 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.735
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.854
Spacing: 24.0 "	C&C Dist a: 4.10 ft	Rep Fac: No	Max Web CSI: 0.995
	Loc. from endwall: NA	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE, 18SS, HS	VIEW Ver: 20.01.01A.0724.12
Lumber			

	▲ Ma	aximu	ım Rea	ctions (	lbs)			
		G	ravity		No	on-Grav	∕ity	
0	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
0	в	435	1-	/-	/-	/122	/-	
		3795	<b>/</b> -	/-	/-	/750		
	Ι :	2175	/-	/-	/-	/466	/-	
	Win	d reac	tions b	ased on	MWFRS			
			Vidth =		Min Req = 1.5			
	S Brg Width = 5.5				Min Req = 4.1			
	1	Brg V	Vidth =	5.5	Min Reg = 1.8			
	Bea	rings l	B, S, &	l are a ri	igid surfac	ė.		
	Members not listed have forces less than 375#							
_	Max	imum	Top C	hord Fo	orces Per	Ply (lb	s)	
					Chords			

## C-D F-G 1336 - 6613 976 - 191 D-E 842 - 4318 G-H 846 - 4306 E-F - 3934 842 - 4318 H - I 799

## **Bracing**

(a) Continuous lateral restraint equally spaced on member

Top chord: 2x4 SP #2; T2,T3 2x4 SP M-31; Bot chord: 2x4 SP M-31; B1 2x4 SP #2; Webs: 2x4 SP #3; W3,W4 2x4 SP M-31; W8,

## **Plating Notes**

W9 2x4 SP #2;

All plates are 2X4 except as noted.

## Loading

#1 hip supports 7-0-0 jacks with no webs.

Left side jacks have 7-0-0 setback with 0-0-0 cant and 2-0-0 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 2-0-0 overhang. Right side jacks have 7-0-0 setback with 0-0-0 cant and 2-0-0 overhang.

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

## Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. R-P

## 181 - 988 3460 - 678 I-K P - M 6674 - 1353 K - I 3437 -678

## Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - T 651 - 26 P - F 540 - 2495 C-S 255 - 1406 M - G 2643 - 557 S - R 705 - 2794 M - L 4407 -861 R - D 711 - 2648 G-L 478 - 1804

L-H

K - H

1125

564

- 223

0

5627 - 1084

397 - 837



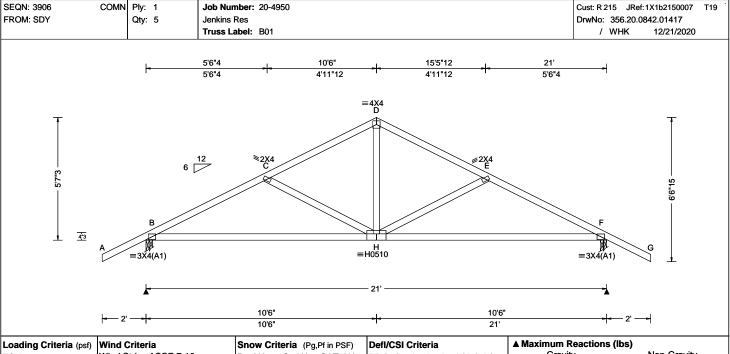
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	14
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 20.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):	PP Deflection in loc L/defl L/# VERT(LL): 0.039 H 999 240 VERT(CL): 0.071 H 999 180 HORZ(LL): 0.014 H HORZ(TL): 0.025 H Creep Factor: 2.0 Max TC CSI: 0.334 Max BC CSI: 0.630 Max Web CSI: 0.315	
Lumber	TVIIIG Baradon. 1.00	WAVE, HS	VIL. VOI. 20.01.01A.0724.12	٦ (

	▲ Ma	ximu	ım Rea	ctions	(lbs)			_		
		G	ravity		N	Ion-Grav	vity			
0	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL			
0	В 9	15	/-	/-	/530	/204	/186	_		
	F 9	15	/-	/-	/530	/204	/-			
	Wind reactions based on MWFRS									
	ВЕ	3rg V	Vidth =	3.5	Min R	Min Req = 1.5				
	F E	3rg V	Vidth =	3.5	Min R	Min Req = 1.5				
	Beari	ngs l	B&Fa	re a rig	id surface.					
	Meml	oers	not liste	ed have	forces les	ss than 3	375#			
	Maxi	mun	Top C	hord F	orces Pe	r Ply (lb	s)			
	Chord	ds 1	Tens.Co	mp.	Chords	Tens.	Ćomp.	_		
	B-C		646 -	1257	D-E	509	- 1004			
	C-D			1004		647	- 1257			

## Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP M-31; Webs: 2x4 SP #3;

## Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

## Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

1073 - 459 1073

## Maximum Web Forces Per Ply (lbs) Tens.Comp. Webs

D-H 828 - 166



12/21/2020

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Ply: 1 FROM: SDY DrwNo: 356.20.0841.59433 Qty: 3 Jenkins Res Truss Label: B02 / WHK 12/21/2020 5'6"4 10'6" 15'5"12 21' 5'6"4 4'11"12 4'11"12 5'6"4 ≡4X4 <sup>≥</sup>2X4 C 4\*3 G H ≡5X6 =2X4(A1) 11'2"12 9'9"4 10'6" 10'6" 10'6' 21

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 20.00 Soffit: 2.00 Load Duration: 1.25	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.026 G 999 240	Gravity Loc R+ /R- /Rh /
Lumbor				

Job Number: 20-4950

SEQN: 3910

COMN

Top chord: 2x4 SP #2; Bot chord: 2x4 SP M-31; Webs: 2x4 SP #3;

## Hangers / Ties

(J) Hanger Support Required, by others

## Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

flectio	n in loc L	/defl	L/#		G	avity		No	on-Grav	<b>∕ity</b>
	0.026 G			Lo	c R+	/ R-	/ Rh	/ Rw	/ U	/RL
	0.050 G		180	В	735	/-	/-	/432	/174	/167
(LL):	0.014 G	-	-	Н	436	/-	/-	/209	/72	/-
TL):	0.028 G	-	-	F	572	/-	/-	/324	/124	/-
Facto	r: 2.0			Wi	nd rea	ctions b	ased on I	MWFRS		
c csi	: 0.434			В		Nidth =		Min Re	q = 1.5	;
C CS				Н		Nidth =		Min Re		,
	. 0.0.0				Dra V	1/idth -		Min Do	~ _	

B - C

Brg Width = -Min Req = -Bearings B & H are a rigid surface.

Cust: R 215 JRef: 1X1b2150007

Members not listed have forces less than 375# **Maximum Top Chord Forces Per Ply (lbs)** Chords Tens.Comp. Chords Tens. Comp.

222 - 544 C-D E-F 352 - 859

## Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

B - G 720 - 246 G-F 1447 - 486

D-E

- 542

232

## Maximum Web Forces Per Ply (lbs) Webs Tens.Comp.

344 - 857

D - G 423



12/21/2020

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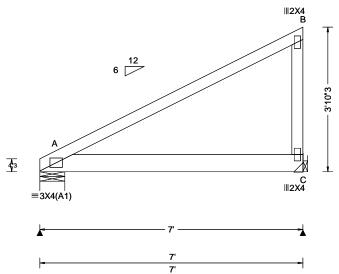
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SEQN: 3913 MONO Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 T20 FROM: SDY DrwNo: 356.20.0841.58047 Qty: 1 Jenkins Res Truss Label: B03 / WHK 12/21/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.035 C
Doc 1 d 27 00	EXP: C Kzt: NA		HORZ(TL): 0.063 C
MODOLL, 20 00	Mean Height: 15.00 ft TCDL: 4.2 psf	Building Code:	Creep Factor: 2.0
0.40	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.863
	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.933
1	C&C Dist a: 3.00 ft	Rep Fac: Varies by Ld Case	Max Web CSI: 0.300
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12

## ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL 1221 /-/291 955 /-/-/233 Wind reactions based on MWFRS Brg Width = 8.0 Min Reg = 1.5Brg Width = -Min Req = -Bearing A is a rigid surface. Members not listed have forces less than 375#

## Lumber

Top chord: 2x4 SP #2; Bot chord: 2x6 SP 2400f-2.0E; Webs: 2x4 SP #3;

## **Special Loads**

--(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 56 plf at 0.00 to 56 plf a BC: From 10 plf at 0.00 to 10 plf a BC: 572 lb Conc. Load at 1.06, 3.06, 5.06 56 plf at 10 plf at

## Hangers / Ties

(J) Hanger Support Required, by others

Wind loads and reactions based on MWFRS. Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!

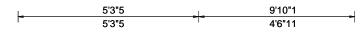
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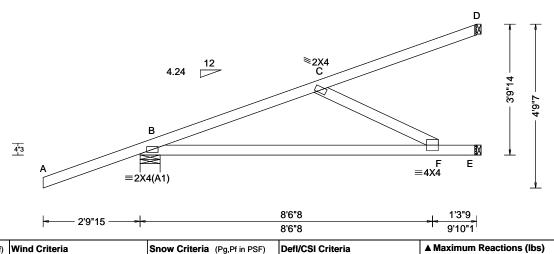
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SEQN: 3902 HIP\_ Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 T21 FROM: SDY DrwNo: 356.20.0841.49337 Qty: 3 Jenkins Res Truss Label: HJ7 / WHK 12/21/2020





Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	▲ Maximum Reactions (Ibs	3)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity	No
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.130 F 892 240	Loc R+ /R- /Rh	/ Rw
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.237 F 490 180	B 489 /- /-	/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.029 C	E 313 /- /-	/-
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.053 C	D 235 /- /-	/-
NCBCLL: 0.00	Mean Height: 15.00 ft TCDL: 4.2 psf	Building Code:	Creep Factor: 2.0	Wind reactions based on MV	_
Soffit: 2.00	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.820	3	Min Rec
l <b>.</b>	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.923	9	Min Rec
	C&C Dist a: 3.00 ft	Rep Fac: No	Max Web CSI: 0.388	D Brg Width = 1.5 Bearing B is a rigid surface.	Min Red
	Loc. from endwall: NA	FT/RT:20(0)/10(0)		Members not listed have for	ces less
	GCpi: 0.18	Plate Type(s):		Maximum Top Chord Forc	es Per I
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12	Chords Tens.Comp.	

	W" 1D " 400	Plate Type(s): WAVE	VIEW Ver: 20.01.01A.0724.12	Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp.
mber				D 0 404 405

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

## Loading

Lum

Hipjack supports 6-11-8 setback jacks with no webs.

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

## **Additional Notes**

Provide (2) 16d common(0.162"x3.5") toe-nails at top chord. Provide (3) 16d common(0.162"x3.5") toe-nails at bottom chord.

# - 465 181 Maximum Bot Chord Forces Per Ply (lbs)

Members not listed have forces less than 375#

Non-Gravity

/132 /-

/105

/RL

/Rw /U

Min Req = 1.5

Min Req = -

Min Req = -

Chords Tens.Comp. 422 - 156

Maximum Web Forces Per Ply (lbs) Webs Tens.Comp.

C-F 182 - 469



\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!

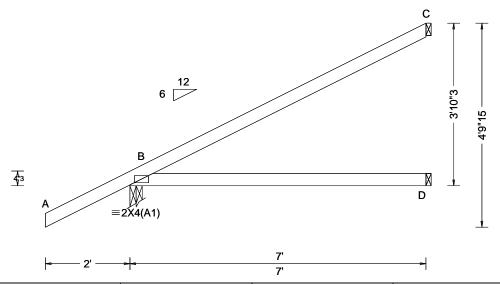
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SEQN: 3896 **EJAC** Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 T15 FROM: SDY DrwNo: 356.20.0841.52333 Qty: 16 Jenkins Res Truss Label: EJ7 / WHK 12/21/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
Coading Criteria (psf)	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0)	DefI/CSI Criteria
	GCpi: 0.18 Wind Duration: 1.60	Plate Type(s): WAVE	VIEW Ver: 20.01.01A.0724.12
Laurabana			

▲ Maximum Reactions (lbs)						
G	ravity		No	Non-Gravity		
: R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
411	/-	/-	/274	/69	/151	
186	/-	/-	/67	/-	/-	
164	/-	/-	/95	/97	/-	
nd read	ctions b	ased on I	MWFRS			
Brg V	Vidth =	3.5	Min Req = 1.5			
Brg V	Vidth =	1.5	Min Re	q = -		
Brg V	Vidth =	1.5	Min Re	q = -		
Bearing B is a rigid surface.						
Members not listed have forces less than 375#						
	411 186 164 nd read Brg V Brg V aring B	Gravity R+ / R-  411 /- 186 /- 164 /- nd reactions b Brg Width = Brg Width = Brg Width = Brg Width =	Gravity  R+ /R- /Rh  411 /- /- 186 /- /- 164 /- /- dreactions based on I Brg Width = 3.5 Brg Width = 1.5 Brg Width = 1.5 aring B is a rigid surface	Gravity No. R+ /R- /Rh /RW  411 /- /- /274 186 /- /- /67 164 /- /- /95 nd reactions based on MWFRS Brg Width = 3.5 Min Re Brg Width = 1.5 Min Re	Gravity Non-Gravity Non-Gravity R+ /R- /Rh /Rw /U  411 /- /- /274 /69  186 /- /- /67 /- 164 /- /- /95 /97  nd reactions based on MWFRS  Brg Width = 3.5 Min Req = 1.8  Brg Width = 1.5 Min Req = -  Brg Width = 1.5 Min Req = -  Brg Width = 1.5 Min Req = -  Brg Width = 1.5 Min Req = -	

## Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

## **Additional Notes**

Provide (2) 16d common(0.162"x3.5") toe-nails at top chord. Provide (2) 16d common(0.162"x3.5") toe-nails at bottom chord.



\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!

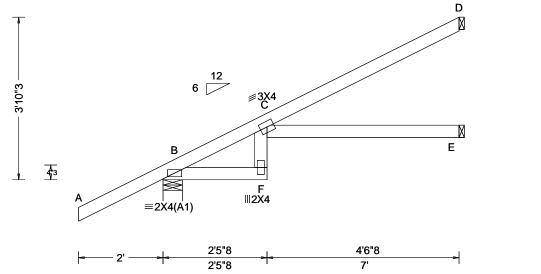
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org

6750 Forum Drive Suite 305 Orlando FL, 32821 SEQN: 3899 **EJAC** Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 FROM: SDY DrwNo: 356.20.0841.50740 Qty: 6 Jenkins Res Truss Label: EJ7A / WHK 12/21/2020



Coading Criteria (psf)   TCLL: 20.00   TCDL: 7.00   BCLL: 0.00   BCDL: 10.00   Des Ld: 37.00   NCBCLL: 20.00	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code:	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.174 F 471 240 VERT(CL): 0.321 F 256 180 HORZ(LL): 0.102 E - HORZ(TL): 0.189 E - Creep Factor: 2.0	L E E
Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Max TC CSI: 0.800 Max BC CSI: 0.421 Max Web CSI: 0.057 VIEW Ver: 20.01.01A.0724.12	E C E N

▲ Maximum Reactions (lbs)						
Gravity			No	on-Gra	vity	
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
В	411	/-	/-	/274	/30	/109
Е	147	/-	/-	/53	/-	/-
D	180	/-	/-	/108	/60	/-
Wir	nd read	ctions b	ased on I	MWFRS		
В	Brg V	Vidth =	Min Re	q = 1.5	5	
Ε	Brg V	Vidth =	1.5	Min Re	q = -	
D	Brg V	Vidth =	1.5	Min Re	q = -	
Bearing B is a rigid surface.						
Mei	mbers	not list	ed have f	orces les	s than	375#

## Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

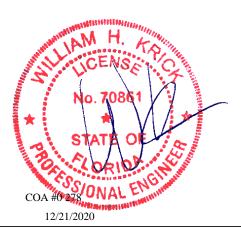
## Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

## **Additional Notes**

Provide (2) 16d common(0.162"x3.5") toe-nails at top chord. Provide (2) 16d common(0.162"x3.5") toe-nails at bottom chord.



\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!

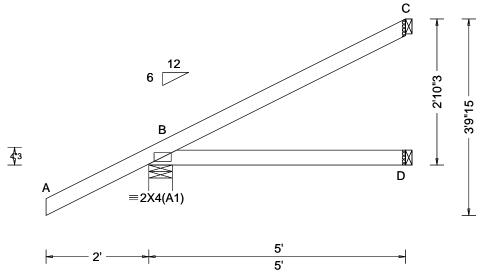
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6750 Forum Drive Suite 305 Orlando FL, 32821

SEQN: 3893 **JACK** Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 T12 FROM: SDY DrwNo: 356.20.0841.53647 Qty: 6 Jenkins Res Truss Label: CJ5 / WHK 12/21/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.004 D
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.009 D
NCBCLL: 20.00	Mean Height: 15.00 ft TCDL: 4.2 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.379
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.344
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.000
'	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12
Lumber	•	•	•

	G	ravity		No	on-Gra	vity
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
В	343	/-	/-	/236	/66	/116
D	129	/-	/-	/46	/-	/-
С	108	/-	/-	/60	/66	/-
Win	d read	ctions b	ased on I	MWFRS		
В	Brg V	Vidth =	5.5	Min Re	q = 1.5	5
D	Brg V	Vidth =	1.5	Min Re	q = -	
С	Brg V	Vidth =	1.5	Min Re	q = -	
Bearing B is a rigid surface.						
Members not listed have forces less than 375#						

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

## **Additional Notes**

Provide (2) 16d common(0.162"x3.5") toe-nails at top chord. Provide (2) 16d common(0.162"x3.5") toe-nails at bottom chord.



\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!

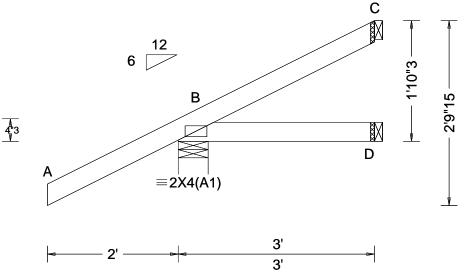
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6750 Forum Drive Suite 305 Orlando FL, 32821

SEQN: 3890 **JACK** Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 T13 FROM: SDY DrwNo: 356.20.0841.54950 Qty: 6 Jenkins Res Truss Label: CJ3 / WHK 12/21/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.001 D
Des Ld: 37.00	EXP: C Kzt: NA Mean Height: 15.00 ft		HORZ(TL): 0.002 D
NCBCLL: 20.00	TCDL: 4.2 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.542
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.104
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.000
	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 20.01.01A.0724.12

	G	ravity		No	on-Gra	vity
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
В	288	/-	/-	/208	/66	/81
D	71	/-	/-	/25	/1	/-
С	44	/-	/-	/24	/31	/-
Win	d read	ctions b	ased on I	MWFRS		
B Brg Width = 5.5			Min Req = 1.5			
D	Brg V	Vidth =	1.5	Min Re	q = -	
С	Brg V	Vidth =	1.5	Min Re	q = -	
Bearing B is a rigid surface.						
Members not listed have forces less than 375#					375#	

## Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

## **Additional Notes**

Provide (2) 16d common(0.162"x3.5") toe-nails at top chord. Provide (2) 16d common(0.162"x3.5") toe-nails at bottom chord.



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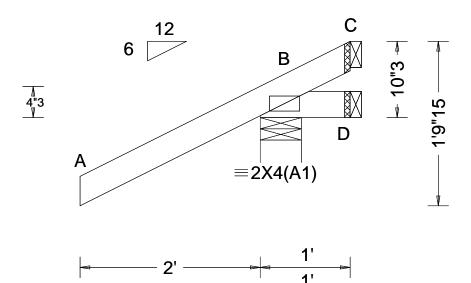
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org





SEQN: 3887 **JACK** Ply: 1 Job Number: 20-4950 Cust: R 215 JRef: 1X1b2150007 T14 FROM: SDY DrwNo: 356.20.0841.56723 Qty: 6 Jenkins Res Truss Label: CJ1 / WHK 12/21/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 20.00 Soffit: 2.00	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0)	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.001 D HORZ(TL): 0.001 D Creep Factor: 2.0 Max TC CSI: 0.542 Max BC CSI: 0.072 Max Web CSI: 0.000
Lumber	GCpi: 0.18 Wind Duration: 1.60	Plate Type(s): WAVE	VIEW Ver: 20.01.01A.0724.12

▲ Maximum Reactions (Ibs)							
	Gravity				Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
В	326	/-	/-	/260	/123	/45	
D	7	/-34	/-	/26	/30	/-	
С	-	/-96	/-	/60	/88	/-	
Wir	nd rea	ctions b	ased on I	<b>MWFRS</b>			
В	Brg \	Vidth =	5.5	Min Req = 1.5			
D	Brg \	Vidth =	1.5	Min Re	q = -		
С	Brg \	Vidth =	1.5	Min Re	q = -		
Bearing B is a rigid surface.							
Mei	mbers	not liste	ed have fo	orces les	than 3	375#	

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

## **Additional Notes**

Provide (2) 16d common(0.162"x3.5") toe-nails at top chord. Provide (2) 16d common(0.162"x3.5") toe-nails at bottom chord.



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# CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

## Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

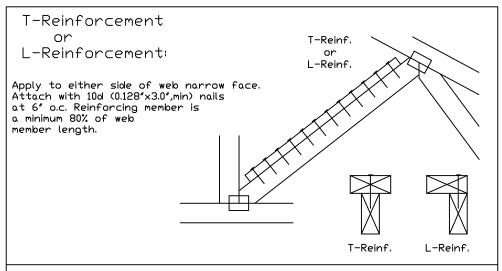
Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member	Specified CLR	Alternative Reir	
Size	Restraint	T- or L- Reinf.	
2x3 or 2x4	1 row	2×4	1-2×4
2x3 or 2x4	2 rows	2×6	2-2×4
2×6	1 row	2×4	1-2×6
2×6	2 rows	2×6	2-2×4( <b>%</b> )
2×8	1 row	2x6	1-2×8
	2 rows	2x6	2-2×6(*)

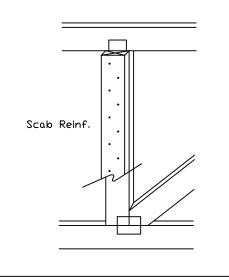
T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(\*\*) Center scab on wide face of web. Apply (1) scab to each face of web.



## Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nalls at 6" o.c. Reinforcing member is a minimum 80% of web member length.



AM H. TOWN

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Trusses require extreme care in fabricating, handling, shipping, installing and macing. Refer to and follow the latest edition of BCSI (Buldling Component Safety Information, by FPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bot on choice shall have a properly attached rigid celling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each of truss and position as shown above and on the Joint Details, unless noted otherwise.

Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation of this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation to bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinstorg; SBCA: www.sbcindustry.org; ICC: www.iccsafe.org

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