# GENERAL NOTES

1. THE CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS AT THE SITE AND SHALL NOTIFY THE ENGINEER OF DISCREPANCIES BETWEEN THE ACTUAL CONDITIONS AND INFORMATION SHOWN ON THE DRAWINGS BEFORE PROCEEDING WITH THE WORK.

2. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE COMPLETE DESIGN OF THE STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKMEN, OR OTHER PERSONS DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE BUT NOT BE LIMITED TO BRACING, SHORING FOR EARTH BANKS, FORMS, SCAFFOLDING, PLANKING, SAFETY NETS, SUPPORT AND BRACING FOR CRANES AND GIN POLES.

3. THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTURAL MEMBERS AS REQUIRED FOR STRUCTURAL STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE STRUCTURAL ENGINEER OF ANY CONDITION WHICH, IN HIS OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS IN THE STRUCTURE.

4. CONSTRUCTION MATERIALS SHALL NOT BE STACKED ON ROOFS IN EXCESS OF THE POSTED ROOF LIVE LOAD. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO INSURE THAT THE SUBCONTRACTORS ARE INFORMED AND DO NOT VIOLATE THIS IMPORTANT REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON ROOFS.

5. PLANS, SECTIONS AND DETAILS ARE NOT TO BE SCALED FOR DETERMINATION OF QUANTITIES, LENGTHS, OR FIT OF MATERIALS.

6. SUBMIT WRITTEN REQUESTS TO THE ENGINEER FOR APPROVAL OF ANY PROPOSED CHANGE TO THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. SPLICING, CUTTING, NOTCHING OR OTHER ALTERATIONS TO STRUCTURAL MEMBERS ARE NOT PERMITTED WITHOUT WRITTEN AUTHORIZATION OF THE STRUCTURAL ENGINEER. ANY UNAUTHORIZED DEVIATION FROM THE CONTRACT DOCUMENTS, AND CORRECTION THEREOF, IS THE RESPONSIBILITY OF THE CONTRACTOR.

7. THE ENGINEER DOES NOT HAVE CONTROL OR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK FRO THE ATS OR OMISSIONS OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

8. PERIODIC SITE OBSERVATION BY FIELD REPRESENTATIVES OF NORTH FLORIDA PROFESSIONAL SERVICES, INC. IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK OF THE CONTRACTOR IS PROCEEDING IN ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THIS LIMITED SITE OBSERVATION SHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS TO CHECK THE QUALITY OR QUANTITY OF THE WORK. BUT RATHER PERIODIC IN AN EFFORT TO GUARD THE OWNER AGAINST DEFECTS OR DEFICIENCIES IN THE WORK OF THE CONTRACTOR.

# DESIGN CRITERIA

1. THE DESIGN IS BASED ON, AND ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE 2023 FLORIDA BUILDING CODE (FBC) WITH AMENDMENTS AND DESIGN CODES REFERENCED WITHIN THESE DOCUMENTS. USE THE REFERENCED EDITIONS FROM THE FBC CHAPTER 35 OR THE LATEST EDITIONS IF NOT REFERENCED:
   AMERICAN SOCIETY OF CIVIL ENGINEERS, ASCE 7-22
   "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES"
   STRUCTURAL CONCRETE:
   "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
   THE AMERICAN CONCRETE INSTITUTE (ACI 318-19 AND ACI 350-06)
   MASONRY:
   "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
   THE MASONRY SOCIETY (TMS 402/602-16)
   STRUCTURAL STEEL:
   STEEL CONSTRUCTION MANUAL - FIFTEENTH EDITION BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC 360-16)
   WOOD:
   "NATIONAL DESIGN SPECIFICATION" AND SUPPLEMENT (ANS/AWC NDS-18)

2. LIVE LOADS:
   ROOF — 20 PSF (REDUCIBLE BY CODE)

3. SUPERIMPOSED DEAD LOADS:
   ROOFING / CEILING / M.E.P. — 5 PSF

4. WIND LOAD DESIGN DATA:
   WIND LOADS SHALL BE IN ACCORDANCE WITH THE 2023 FLORIDA BUILDING CODE (REFERENCING ASCE 7-22).
   MAIN WIND FORCE RESISTING SYSTEM
   WIND DESIGN DATA:

| | | |
|---|---|---|
| a. | ULTIMATE DESIGN WIND SPEED, 3 SECOND GUSTS, VULT. | 120 MPH |
| b. | HURRICANE PRONE REGION | YES |
| c. | WINDBORNE DEBRIS REGION | NO |
| d. | BUILDING RISK CATEGORY | II |
| e. | WIND EXPOSURE CATEGORY | C |
| f. | WIND TOPOGRAPHIC FACTOR (KZT) | 1.0 |
| g. | ENCLOSURE CATEGORY | PARTIALLY OPEN |
| h. | INTERNAL PRESSURE COEFFICIENT | ±0.18 |
| i. | MEAN ROOF HEIGHT | 15 FEET |
| j. | WIND DIRECTIONALLY FACTOR, KD | 0.85 |
| k. | VELOCITY PRESSURE COEFFICIENT (KH) | 0.85 |
| l. | ULTIMATE VELOCITY PRESSURE (QH[ULTI]) | 31.3 PSF |
| m. | COMPONENT & CLADDING WIND PRESSURES | SEE TABLE THIS SHEET |
| n. | DIMENSION "a" | 3'-0" |
| o. | GROUND ELEVATION FACTOR, KE | 1.0 |

5. 60-MINUTE RAINFALL INTENSITY — 4.5 INCHES PER HOUR.

6. DISTRIBUTE THE MAXIMUM LOAD HUNG FROM ANY STRUCTURAL MEMBERS FOR MEP DUCTWORK, PIPING ETC OVER THE MEMBER'S TRIBUTARY AREA IN A WAY THAT THE DESIGN SUPERIMPOSED DEAD LOADS LISTED IN CONTRACT DOCUMENTS ARE NOT EXCEEDED. THE CONTRACTOR SHALL COORDINATE THE LOADS OF ALL TRADES AND PROVIDE ADDITIONAL SUPPORT OR DISTRIBUTION FRAMING AS REQUIRED TO ACHIEVE THE ALLOWABLE LOAD DISTRIBUTION.

7. STRUCTURAL COMPONENTS ARE NOT DESIGNED FOR VIBRATING EQUIPMENT. MOUNT VIBRATING EQUIPMENT ON VIBRATION ISOLATORS.

# FOUNDATIONS

1. BEARING SOILS SHALL BE FREE OF ORGANIC MATERIAL AND MEET THE FBC REQUIREMENTS TO PROVIDE A MINIMUM OF 1,500 PSF SOIL BEARING DESIGN PRESSURES. PER TABLE R401.4.1 OF THE FLORIDA RESIDENTIAL BUILDING CODE. IT IS THE HOMEOWNERS RESPONSIBILITY TO VERIFY THAT THE SOIL CONDITIONS ARE SUITABLE FOR THESE ASSUMPTIONS. IT IS SUGGESTED THAT PRIOR TO CONSTRUCTION A GEOTECHNICAL INVESTIGATION BE MADE TO VERIFY THE BEARING PRESSURE AND SUBSURFACE CONDITIONS. STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT FROM THOSE ASSUMED FOR DESIGN.

# WOOD

1. STRUCTURAL FRAMING PLANS DEPICT THE PRIMARY STRUCTURAL FRAMING SYSTEM. CONTRACTOR SHALL PROVIDE SECONDARY AND MISCELLANEOUS FRAMING AS REQUIRED TO COMPLETE THE PROJECT.

2. DRESSED SEASONED LUMBER: S4S, 19% MAXIMUM MOISTURE CONTENT AT TIME OF DRESSING.
   2.1. COLUMNS AND STUD FRAMING: SOUTHERN PINE NO.2 OR STRONGER.
   2.2. LINTELS, FLOOR JOISTS AND BEAMS: SOUTHERN PINE, NO.2 GRADE
   2.3. WOOD IN CONTACT WITH CONCRETE OR MASONRY OR EXPOSED TO WEATHER: ABOVE GRADE PRESSURE-TREATED (AWPA-UC3A OR UC3B) OR GROUND CONTACT RATED PRESSURE TREATED (AWPA-UC4A). GROUND CONTACT RATED WOOD IS RECOMMENDED AT THE CRAWLSPACE AND DECK AREAS (IF PRESENT). USE HOT-DIP GALVANIZED NAILS IN PRESSURE TREATED WOOD.

3. STRUCTURAL PANELS
   3.1. WALL PANELS: 1/2" APA RATED SHEATHING.
   3.2. ROOF PANELS: 1/2" APA RATED SHEATHING.

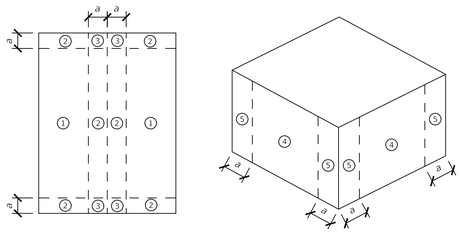
4. WOOD SHEAR WALLS
   4.1. PANELS SHALL BE ORIENTED WITH THE LONG DIMENSION IN THE VERTICAL DIRECTION.
   4.2. SOLID 2x BLOCKING SHALL BE PROVIDED AT UNSUPPORTED, HORIZONTAL PANEL EDGES.
   4.3. NAIL PANELS WITH 8d HOT-DIPPED GALVANIZED RINGSHANK NAILS SPACED AT 6" AT THE PERIMETER OF THE PANELS AND AT 12" AT INTERMEDIATE SUPPORTS, UON.
   4.4. DOUBLE 2x FRAMING STUDS SHALL BE USED AT THE ENDS OF EACH SHEAR WALL, UON.
   4.5. CONNECTIONS FOR STRUCTURAL TIMBER: GALVANIZED STRONG-TIE CONNECTORS BY THE SIMPSON STRONG TIE COMPANY OR APPROVED EQUAL.

5. LAMINATED VENEER LUMBER (LVL) SHALL BE WEYERHAUSER/TRUS JOIST MICROLLAM LVL (OR EQUAL) WITH f'b NOT LESS THAN 2,600 PSI AND MINIMUM 2.0E.

6. BOLTED CONNECTIONS SHALL CONSIST OF ASTM A307 BOLTS, FASTENED TO A SNUG-TIGHT CONDITION.

# WIND PRESSURE DIAGRAM

1. DESIGN WIND PRESSURES TO BE USED IN THE DESIGN OF ALL COMPONENTS AND CLADDING ELEMENTS. PRESSURES INDICATED IN TABLE ARE SERVICE LOADS. MULTIPLY TABULATED VALUES BY 0.6 FOR ALLOWABLE STRESS DESIGN (ASD) AND BY 1.0 FOR LOAD AND RESISTANCE FACTOR DESIGN (LRFD).
2. REFER TO WIND PRESSURE DIAGRAM FOR ZONE LOCATIONS AND EXTENTS.
3. POSITIVE PRESSURES ACT TOWARD COMPONENT SURFACES AND NEGATIVE PRESSURES ACT AWAY FROM COMPONENT SURFACES.




COMPONENTS AND CLADDING WIND PRESSURES ON ROOF AND WALLS (PSF)

| ZONE | 1, 2, 3 | 1 | 2 | 3 | 4 | | 5 | |
|---|---|---|---|---|---|---|---|---|
| TRIB AREA | (+) | (-) | (-) | (-) | (+) | (-) | (+) | (-) |
| 10 | 21 | -58 | -77 | -101 | 31 | -34 | 31 | -42 |
| 20 | 19 | -50 | -67 | -87 | 30 | -33 | 30 | -39 |
| 50 | 17 | -39 | -53 | -68 | 29 | -31 | 29 | -36 |
| 100 | 15 | -31 | -43 | -53 | 27 | -30 | 27 | -33 |
| 200 | 13 | -18 | -31 | -53 | 25 | -28 | 25 | -30 |
| 500 | 13 | -18 | -31 | -53 | 23 | -26 | 23 | -26 |

# WINDOWS, DOORS, AND ROOFING

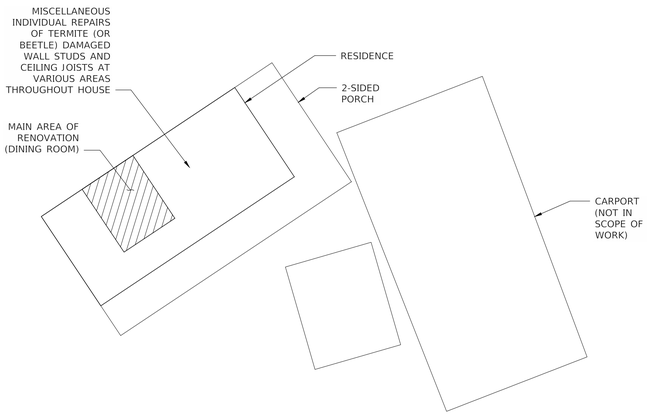
1. FOR THE SELECTION OF WINDOW, DOOR, AND ROOFING PRODUCTS, TABULATED VALUES ARE NORMALLY MULTIPLIED BY 0.6 PRIOR TO COMPARISON WITH THE POSITIVE AND NEGATIVE PRESSURE RATINGS PROVIDED IN EACH FLORIDA PRODUCT APPROVAL. IT IS RECOMMENDED THAT THE MANUFACTURER'S REPRESENTATIVE REVIEW THESE DRAWINGS FOR VERIFICATION. THE TRIBUTARY AREA FOR ROOFING PRODUCTS IS TYPICALLY BASED ON 10 SQUARE FEET, AND FOR DOORS AND WINDOWS IT IS BASED ON THE SURFACE AREA OF THE WALL OPENINGS.

# ROOF OVERHANG PRESSURES (WHERE NOT TABULATED ABOVE)

1. ROOF OVERHANG PRESSURES ARE DETERMINED BY SUMMING THE ABSOLUTE VALUE OF THE NEGATIVE ROOF ZONE (1, 2, OR 3) PRESSURE AND THE POSITIVE WALL ZONE (4 OR 5) PRESSURE, BASED ON THE APPLICABLE TRIBUTARY AREA. THE SUM IS THEN MULTIPLIED BY -1 TO GET THE CORRESPONDING ROOF OVERHANG UPLIFT PRESSURE.

# SYMBOLS AND ABBREV.

| | |
|---|---|
| ALT | ALTERNATE/ALTERNATIVE |
| ACI | AMERICAN CONCRETE INSTITUTE |
| AFF | ABOVE FINISHED FLOOR |
| AISC | AMERICAN INSTITUTE OF STEEL CONSTRUCTION |
| AISI | AMERICAN IRON AND STEEL INSTITUTE |
| ASTM | AMERICAN SOCIETY FOR TESTING AND MATERIALS |
| AWS | AMERICAN WELDING SOCIETY |
| AB | ANCHOR BOLTS |
| ARCH | ARCHITECTURE/ARCHITECTURAL |
| ASTM | AMERICAN SOCIETY OF TESTING MATERIALS |
| AWS | AMERICAN WELDING SOCIETY |
| BB | BOND BEAM |
| BFB | BOTTOM FLANGE BRACE |
| BP | BASE PLATE/BEARING PLATE |
| BRG | BEARING |
| BM | BEAM |
| BLK | BLOCK |
| B/ | BOTTOM OF |
| BLDG | BUILDING |
| CANT | CANTILEVER |
| CL | CENTERLINE |
| CLR | CLEAR/CLEARANCE |
| COL | COLUMN |
| CB | CONCRETE BEAM |
| CC | CONCRETE COLUMN |
| CMU | CONCRETE MASONRY UNIT |
| CONT | CONTINUOUS |
| CONNX | CONNECTION |
| CONST | CONSTRUCTION |
| CS J | CONSTRUCTION JOINT |
| C J | CONTRACTION JOINT / CONTROL JOINT |
| DET | DETAIL |
| DEPT | DEPARTMENT |
| DBA | DEFORMED BAR ANCHOR |
| DFT | DRY FILM THICKNESS |
| DIA | DIAMETER |
| DIM | DIMENSION |
| DIST | DISTANCE |
| DN | DOWN |
| DR | DRAIN |
| DWG | DRAWING |
| EA | EACH |
| EE | EACH END |
| EF | EACH FACE |
| EJ | EXPANSION JOINT |
| EL | ELEVATION |
| EMB | EMBEDMENT |
| ENGR | ENGINEER |
| EOS | EDGE OF SLAB |
| EQ | EQUAL |
| ES | EACH SIDE |
| EW | EACH WAY |
| EXIST | EXISTING |
| EXP | EXPANSION |
| EXT | EXTERIOR |
| FBC | FLORIDA BUILDING CODE |
| FL | FULL LENGTH WELD, WELD ENTIRE DIST. |
| F V | FIELD VERIFY |
| F F | FINISHED FLOOR |
| FLR | FLOOR |
| F D | FLOOR DRAIN |
| FTG | FOOTING |
| GA | GAGE/GAUGE |
| GALV | GALVANIZED |
| GC | GENERAL CONTRACTOR |
| GLU-LAM | GLUE LAMINATED |
| HAS | HEADED ANCHOR STUD |
| HC | HOLLOW CORE |
| HK | HOOK |
| HORIZ | HORIZONTAL |
| HP | HIGH POINT |
| HSS | HOLLOW STRUCTURAL SECTION |
| HT | HEIGHT |
| ID | INSIDE DIAMETER |
| IF | INSIDE FACE |
| INT | INTERIOR |
| JT | JOINT |
| JST | JOIST |
| K | KIP = 1000 LB |
| KO | KNOCK OUT |
| LGTH | LENTGH |
| LLH | LONG LEG HORIZONTAL |
| LLV | LONG LEG VERTICAL |
| LONG | LONGITUDINAL |
| MANUF | MANUFACTURE/MANUFACTURER |
| M B | MASONRY BEAM |
| MATL | MATERIAL |
| MAX | MAXIMUM |
| MECH | MECHANICAL |
| MEZZ | MEZZANINE |
| MIN | MINIMUM |
| MISC | MISCELLANEOUS |
| MO | MASONRY OPENING |
| MTL | METAL |
| N I C | NOT IN CONTRACT |
| NOM | NOMINAL |
| N T S | NOT TO SCALE |
| N W T | NORMAL WEIGH TOPPING |
| O C | ON CENTER |
| OPNG | OPENING |
| OPP | OPPOSITE |
| PAF | POWER ACTUATED FASTENER |
| PL | PLATE |
| PLY | PLYWOOD |
| PSF | POUNDS PER SQUARE FOOT |
| PSI | POUNDS PER SQUARE INCH |
| PC | PRECAST CONCRETE |
| PRE-ENG | PRE-ENGINEERED |
| PREFAB | PREFABRICATED |
| PROJ | PROJECTION |
| PT | PRESSURE TREATED |
| PW | PANEL WIDTH |
| REF | REFERENCE |
| REINF | REINFORCING |
| R C P | REINFORCED CONCRETE PIPE |
| REQD | REQUIRED |
| R W | RETAINING WALL |
| R D | ROOF DRAIN |
| SCHED | SCHEDULE |
| SIM | SIMILAR |
| SPC | SPACE/SPACES |
| SPECS | SPECIFICATIONS |
| SPF | SPRUCE PINE FUR |
| SQ | SQUARE |
| S A | STUD ANCHOR |
| S S | STAINLESS STEEL |
| STD | STANDARD |
| STL | STEEL |
| STRUC | STRUCTURAL |
| SYM | SYMMETRICAL |
| SF | STEPPED FOOTING |
| SYP | SOUTHERN YELLOW PINE |
| THK | THICK |
| THD | THREAD/THREADED |
| TB | TIE BEAM |
| T & B | TOP AND BOTTOM |
| T & G | TONGUE AND GROOVE |
| T.O.C. | TOP OF CONCRETE |
| T.O.S. | TOP OF STEEL |
| TRANS | TRANSVERSE |
| TYP | TYPICAL |
| T/ | TOP OF |
| UNO | UNLESS NOTED OTHERWISE |
| VERT | VERTICAL |
| VOL | VOLUME |
| WF | WALL FOOTING |
| WPF | WATERPROOF |
| WWF | WELDED WIRE FABRIC |
| WH | WEEP HOLE |
| WT | WEIGHT |
| WF | WIDE FLANGE |
| W / | WITH |
| W / O | WITHOUT |
| WD | WOOD |
| WP | WORKING POINT |




DAVID M. CRAPPS LICENSE No.60989 STATE OF FLORIDA PROFESSIONAL ENGINEER

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WWW.NFPS.NET
CA# 29011


JOB NUMBER: L241211KOT
EOR: DAVID M. CRAPPS
P.E. NO.: 60989

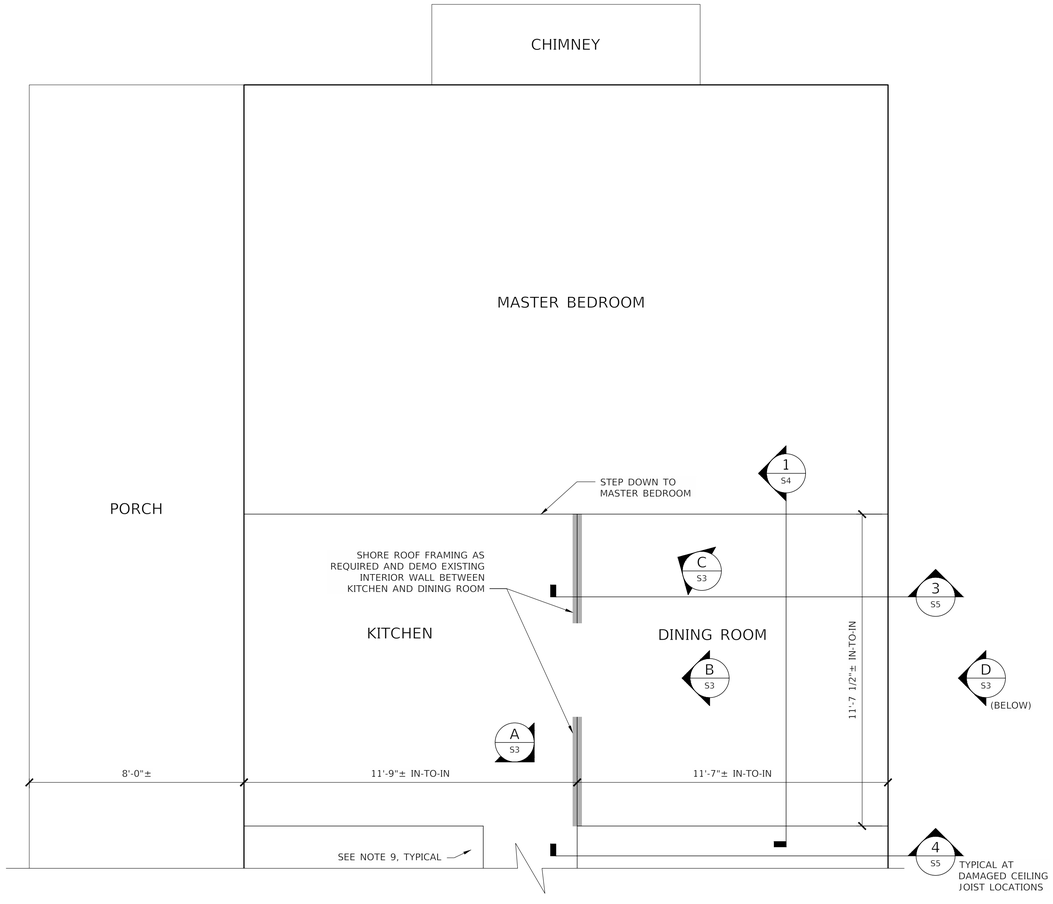
# STRUCTURAL GENERAL NOTES

KOTILA RESIDENCE - RENOVATION
660 NE HARRINGTON COURT
LAKE CITY, COLUMBIA COUNTY, FLORIDA

SHEET NO. S1

## PARTIAL FLOOR PLAN NOTES

1. LOCATE EXISTING UTILITIES PRIOR TO EXCAVATION FOR ANY NEW FOUNDATION PIER (SEE DETAIL 1/S4).
2. NEW FOUNDATIONS SHALL BE ALLOWED TO SETTLE PRIOR TO COMMENCEMENT OF WOOD-FRAMED CONSTRUCTION.
3. MAINTAIN POSITIVE SLOPE FOR FINISHED GRADE AWAY FROM NEW FOUNDATIONS PER CODE.
4. FIELD VERIFY DIMENSIONS AS REQUIRED.
5. PROVIDE TERMITE TREATMENT OF SUB-GRADE.
6. SPIKE MULTIPLE PLY BEAMS TOGETHER WITH 2 ROWS OF 10d GALV. COMMON NAILS @12"O.C. STAGGERED, PER PLY.
7. WALL SHEATHING SHALL BE NOT LESS THAN 1/2" APA RATED PLYWOOD OR OSB WITH 8d GALV x 2-1/2" LONG RINGSHANK NAILS @6" O.C. AT PANEL EDGES AND @12" O.C. IN FIELD OF PANEL. PROVIDE 2x SOLID BLOCKING AT HORIZONTAL PANEL EDGES.
8. AT STUD PACKS, SPIKE STUD PLIES TOGETHER WITH 10d GALV. COMMON NAILS @6" O.C. STAGGERED, PER PLY.
9. CONTRACTOR TO FIELD VERIFY STUDS, CEILING JOISTS, AND RAFTERS IN THE HOUSE FOR ANY SIGNS OF TERMITE / BEETLE DAMAGE. REPAIR DAMAGED MATERIALS PER DETAIL 3/S5 SIMILAR FOR RAFTERS AND 4/S5 FOR CEILING JOISTS. AT DAMAGED STUDS, SISTER NEW 2x4 STUD WITH 10d HOT DIPPED GALVANIZED COMMON NAILS @8" O.C. STAGGERED, AND WITH SIMPSON A34Z AT EACH END.




1 PARTIAL FLOOR PLAN
SCALE: 3/16" = 1'-0"

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**NORTH FLORIDA PROFESSIONAL SERVICES, INC.**
P.O. BOX 3823, LAKE CITY, FL 32056, PH. 386-752-4675, LIC NO. LB8356
2551 BLAIRSTONE PINES DR., TALLAHASSEE, FL 32301, WWW.NFPS.NET, CA# 29011

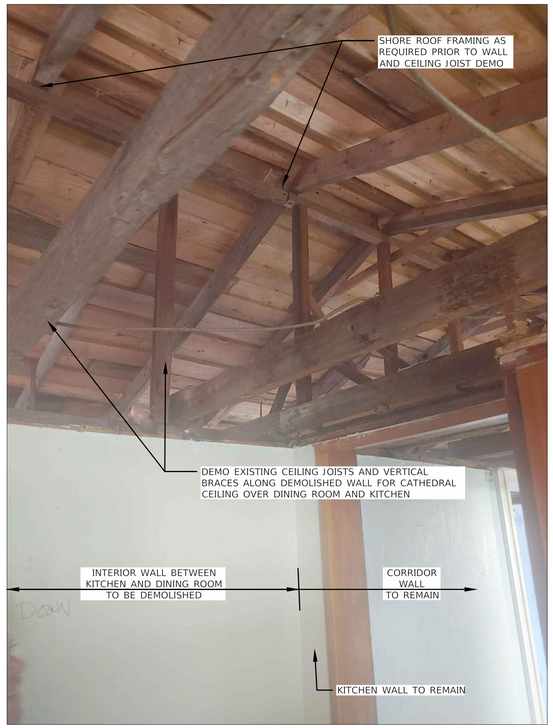

**JOB NUMBER:** L241211KOT
**EOR:** DAVID M. CRAPPS
**P.E. NO.:** 60989

# PARTIAL FLOOR PLAN

**KOTILA RESIDENCE - RENOVATION**
**660 NE HARRINGTON COURT**
**LAKE CITY, COLUMBIA COUNTY, FLORIDA**

**SHEET NO.** S2

*Joshua Galler* *3/20/2025 1:37:12 PM* *L241211K0T Kotila Residence.dwg S2*




A KITCHEN AREA REPAIR PHOTO DETAIL
SCALE: N.T.S.


B DINING ROOM AREA REPAIR PHOTO DETAIL
SCALE: N.T.S.


C DINING ROOM AREA REPAIR PHOTO DETAIL
SCALE: N.T.S.




D CRAWLSPACE REPAIR PHOTO DETAIL
SCALE: N.T.S.

DAVID M. CRAPPS LICENSE No.60989 STATE OF FLORIDA PROFESSIONAL ENGINEER

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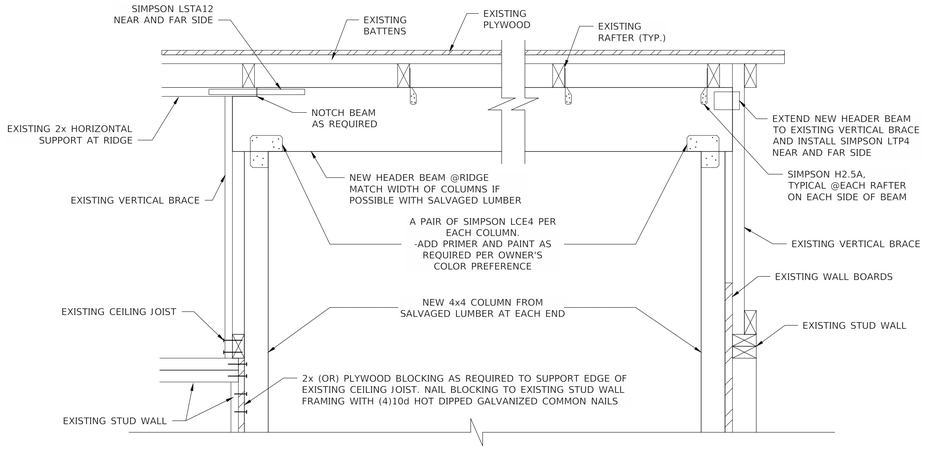
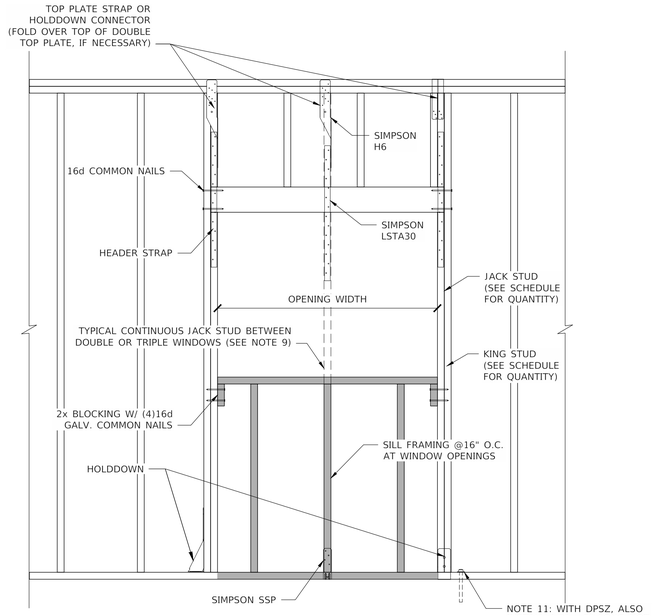
# REPAIR PHOTO DETAILS

KOTILA RESIDENCE - RENOVATION
660 NE HARRINGTON COURT
LAKE CITY, COLUMBIA COUNTY, FLORIDA

SHEET NO. S3




1 ELEVATION AT NEW DINING ROOM/KITCHEN WALL OPENING
SCALE: N.T.S.




HEADER SCHEDULE

| MAXIMUM OPENING WIDTH | HEADER SIZE | JACK STUDS | KING STUDS | SIMPSON STRONG-TIE CONNECTORS | | |
|---|---|---|---|---|---|---|
| | | | | TOP PLATE | HEADER | HOLDDOWN |
| SINGLE 3'-0" | (2)2x6 | (1)2x4 | (1)2x4 | H6 | LSTA30 | DSPZ (SEE NOTE 11) |
| TRIPLE 3'-0" | (2)2x10 | (1)2x4 | (2)2x4 | H6 | LSTA30 | DSPZ (SEE NOTES 9 & 11) |

## NOTES

1. ALL LUMBER SHALL BE SOUTHERN YELLOW PINE NO.2 OR BETTER.
2. USE PLYWOOD SPACERS BETWEEN HEADER PLIES AS REQUIRED TO MATCH STUD DEPTH.
3. NAIL STUD PACKS TOGETHER WITH 10d GALVANIZED NAILS @6" O.C. STAGGERED
4. INTERIOR NON-LOAD BEARING WALL HEADERS SHALL NOT BE LESS THAN CODE MINIMUMS.
5. USE 8d COMMON GALVANIZED NAILS ON SIMPSON H6, FULLY NAILED.
6. USE 10d COMMON GALVANIZED NAILS ON SIMPSON LSTA30, FULLY NAILED.
7. SIMPSON CS16 MAY BE SUBSTITUTED FOR LSTA30.
8. INSTALL HEADER STRAPS FROM THE INSIDE FACE OF WALL. IF THE TABLE CALLS FOR 2 HEADER STRAPS AT ONE JACK STUD, INSTALL 1 STRAP ON THE INSIDE FACE OF WALL AND 1 ON THE EXTERIOR FACE OF WALL. STAGGER NAIL LOCATIONS AS REQUIRED.
9. WHERE ADJACENT WINDOWS ARE SEPARATED BY A JAMB STUD, ATTACH TOP OF STUD TO CONTINUOUS HEADER WITH SIMPSON LSTA30. ATTACH BOTTOM OF STUD TO BOTTOM PLATE WITH SIMPSON SSPZ.
10. IF THE FOUNDATION PLAN CALLS FOR A HOLDDOWN AT A WALL OR DOOR JAMB, THE HOLDDOWN ON PLAN GOVERNS OVER THE HOLDDOWN SHOWN IN THE HEADER SCHEDULE

2 TYPICAL LOAD BEARING HEADER DETAIL
SCALE: N.T.S.

DAVID M. CRAPPS LICENSE No.60989 STATE OF FLORIDA PROFESSIONAL ENGINEER

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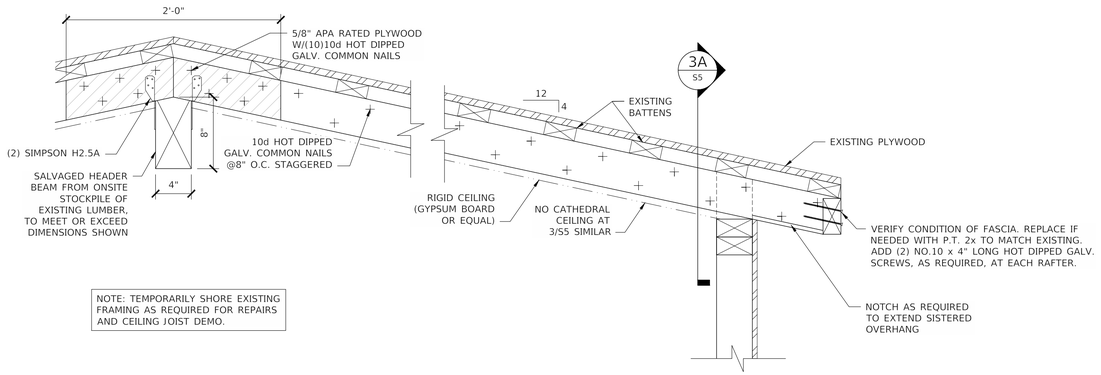
# STRUCT. SECTIONS & DETAILS

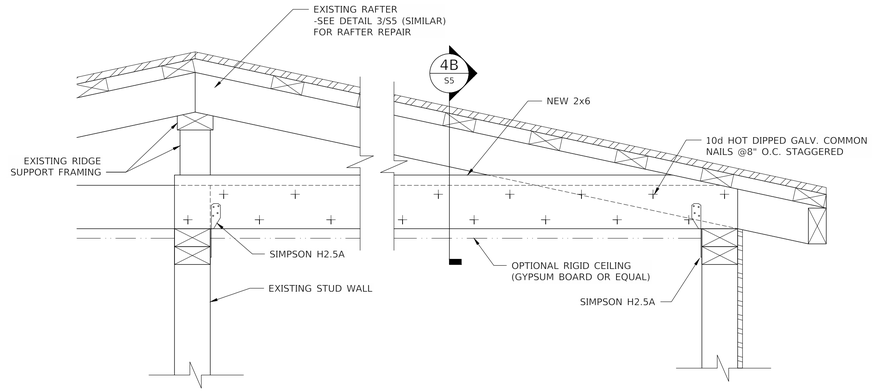
KOTILA RESIDENCE - RENOVATION
660 NE HARRINGTON COURT
LAKE CITY, COLUMBIA COUNTY, FLORIDA

SHEET NO. S4



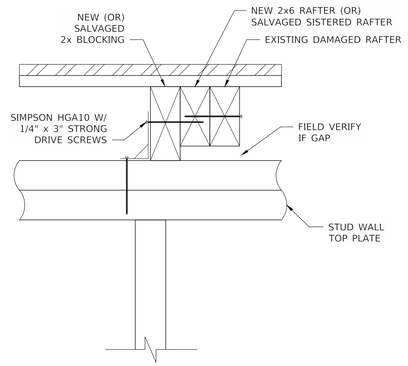

## 3 ELEVATION AT NEW DINING ROOM/KITCHEN CATHEDRAL CEILING

SCALE: N.T.S.



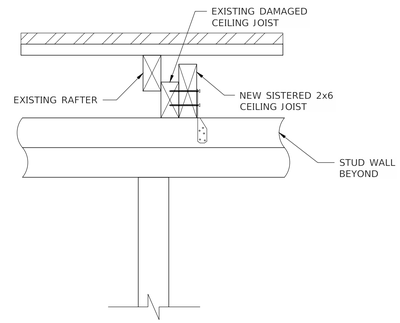

## 4 TYPICAL CEILING JOIST REPAIR DETAIL

SCALE: N.T.S.




## 3A SECTION AT DINING ROOM/KITCHEN CATHEDRAL CEILING

SCALE: N.T.S.




## 4B TYPICAL SECTION AT CEILING JOIST REPAIR

SCALE: N.T.S.

DAVID M. CRAPPS LICENSE No.60989 STATE OF FLORIDA PROFESSIONAL ENGINEER

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Digitally signed by David M Crapps
DN: C=US, E=david.crapps@nfps.net, CN=David M Crapps, O=North Florida Professional Services, Inc., OU=Engineering
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# STRUCT. SECTIONS & DETAILS

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**SHEET NO.** S5