

RE: 2718981 - DETAILS

Site Information:

Customer Info: DETAILS Project Name: N/A Model: N/A Lot/Block: N/A Subdivision: N/A Address: N/A, N/A City: N/A State: N/A

Name Address and License # of Structural Engineer of Record, If there is one, for the building. Name: License #: Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014 Wind Code: ASCE 7-16 Roof Load: 37.0 psf

Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

This package includes 20 individual, General Truss Details and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Detail Name	Date	No.	Seal#	Detail Name	Date
1	T23399806	MII-REP10	4/2/21	15	T23399820	MII-VALLEY HIGH WIND2	4/2/21
2	T23399807	MII-T-BRACE 2	4/2/21	16	T23399821	MII-VALLEY SP	4/2/21
3	T23399808	MII-SCAB-BRACE	4/2/21	17	T23399822	MII-VALLEY SP	4/2/21
4	T23399809	MII-REP05	4/2/21	18	T23399823	MII-GE146-001	4/2/21
5	T23399810	MII-GE130-D-SP	4/2/21	19	T23399824	MII-REP13B	4/2/21
6	T23399811	MII-GE130-SP	4/2/21	20	T23399825	MII-STRGBCK	4/2/21
7	T23399812	MII-GE140-001	4/2/21				
8	T23399813	MII-GE170-D-SP	4/2/21				
9	T23399814	MII-GE180-D-SP	4/2/21				
10	T23399815	MII-GE180-D-SP	4/2/21				
11	T23399816	MII-PIGGY-ALT-7-16	4/2/21				
12	T23399817	MII-REP01A1	4/2/21				
13	T23399818	MII-TOENAIL_SP	4/2/21				
14	T23399819	MII-VALLEY HIGH WIND1	4/2/21				

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: ORegan, Philip

My license renewal date for the state of Florida is February 28, 2023.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



MiTek USA, Inc. 6904 Parke East Blvd. Tampa, FL 33610-4115



RE: \$JOBNAME - \$JOBDESC

MiTek USA, Inc. 6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info:\$SI_CUSTOMERProject Name:\$SI_JOBNAMEModel:\$SI_MODELLot/Block:\$SI_LOTNUMSubdivision:\$SI_SUBDIVAddress:\$SI_SITEADDRState:\$SI_SITESTATE



NOTES:

- 1. LOADING: TOP CHORD: (REFER TO THE MAIN TRUSS DESIGN FOR TOP CHORD LOADING). BOTTOM CHORD: LL = 0 PSF, DL = 10 PSF.
- 2. REFER TO THE MAIN TRUSS DESIGN FOR LUMBER AND PLATING REQUIREMENTS.
- 3. MAXIMUM BOTTOM CHORD PITCH = 6/12.
- 4. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
- 5. FALSE BOTTOM CHORD ONLY DESIGNED TO CARRY VERTICAL LOAD. NO LATERAL (SHEAR) LOAD ALLOWED.
- 6. FILLER MAY EXTEND FOR FULL LENGTH OF TRUSS.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021



AUGUST 1, 2016

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2 T23399807

Page 1 of 1

MiTek USA, Inc.



Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

lailing Pattern							
– – – – – – – – – – – – – – – – – – –							
Nail Size	Nail Spacing						
10d (0 131" X 3")	6" o.c.						
100 (0:101 7.0)	0 0.0.						
entire length of T-Br	ace / I-Brace						

(On Two-Ply's Nail to Both Plies)



	Brace Size for One-Ply Truss				
	Specified Continuous Rows of Lateral Bracing				
Web Size	1	2			
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace			
2x6	2x6 T-Brace	2x6 I-Brace			
2x8	2x8 T-Brace	2x8 I-Brace			

	Brace Size for Two-Ply Truss Specified Continuous Rows of Lateral Bracing				
Web Size	1	2			
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace			
2x6	2x6 T-Brace	2x6 I-Brace			
2x8	2x8 T-Brace	2x8 I-Brace			

T-Brace / I-Brace must be same species and grade (or better) as web member.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021



AUGUST 1, 2016

SCAB-BRACE DETAIL

MII-SCAB-BRACE T23399808



MiTek USA, Inc.

Page 1 of 1

Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical. Scab must cover full length of web +/- 6".

*** THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

2 ROWS OF 10d SCAB MUST BE	CAB TO ONE FAC (0.131" X 3") NAI THE SAME GRA ETTER) AS THE V	CE OF WEB WITH ILS SPACED 6" O.C. DE, SIZE AND WEB.
	M	
SCAB BRACE		MAXIMUM WEB AXIAL FORCE = 2500 lb MAXIMUM WEB LENGTH = 12'-0" 2x4 MINIMUM WEB SIZE MINIMUM WEB GRADE OF #3
		M
Nails	Scab-Bra	on Detail
	🦳 Web	

Scab-Brace must be same species grade (or better) as web member.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021

















6904 Parke East Blvd Tampa, FL 36610









January 8, 2019

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

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MII-PIGGY-7-16 T23399815

Page 1 of 1

MiTek USA, Inc.

D

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E





A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

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- A PIGGBACK TRUSS, REFER TO WITEK TRUSS DESIGN DRAWING.
 SHALL BE CONNECTED TO EACH PURLIN WITH (2) (0.131" X 3.5") TOE-NAILED.
 B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
 C PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
- UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0:131" X 3.5") NAILS EACH. 2 X __ X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIPECTIONS AND: D-2X DIRECTIONS AND:
- 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 180 MPH WITH A MAXIMUM
- PIGGYBACK SPAN OF 12 ft. FOR WIND SPEEDS BETWEEN 116 AND 180 MPH, ATTACH MITEK NP37 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" NAIL EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK NP37 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" NAIL EDGE DISTANCE.

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK

FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS 1) MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL
- 2) ATTACH 2 x _ x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
- VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM 3) CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
 CONCENTRATED LOAD MUST BE APPLIED TO BOTH

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

EXPOSURE B or C ENCLOSED BUILDING LOADING = 5 PSF TCDL ASCE 7-10, ASCE 7-16 **DURATION OF LOAD INCREASE : 1.60** DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED. Е

MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING



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This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date



JANUARY 8, 2019

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT-7-16 T23399816 Page 1 of 1 MiTek USA, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E

MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING

LOADING = 5 PSF TCDL MINIMUM ASCE 7-10, ASCE 7-16

DURATION OF LOAD INCREASE : 1.60 DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

EXPOSURE B or C ENCLOSED BUILDING

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- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- BIGBACK TRUSS, REFER TO WITEK TRUSS DESIGN DRAWING.
 SHALL BE CONNECTED TO EACH PURLIN
 WITH (2) 0(0.131" X 3.5") TOE-NAILED.
 BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
 PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
 UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. C
- ONLESS SPECIFIED CLOSER ON MITTER TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH. 2 X __ X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" 0.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING _____ D - 2 X IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
- 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 180 MPH WITH A MAXIMUM
- PIGGYBACK SPAN OF 12 ft. E FOR WIND SPEED IN THE RANGE 116 MPH 180 MPH ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL
- ATTACH 2 x ___ x 4-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH ATTACH 2 x 2) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- THIS CONNECTION IS ONLY VALID FOR A MAXIMUM 3) CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS. 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,
- NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH E PIGGYBACK AND THE BASE TRUSS DESIGN

🗼 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-7473 rev. 5/19/2/02/ BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date



AUGUST 1, 2016

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1 T23399817

MiTek USA, Inc. Page 1 of 1



NOTES 1.

- THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
- ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR
- AND HELD IN PLACE DURING APPLICATION OF REPAIR
- THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD. 3
- WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2X_ ORIENTATION ONLY.
- 5
- 6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date

April 2,2021



MAY 7, 2019

LATERAL TOE-NAIL DETAIL

MII-TOENAIL SP T23399818

Tampa, FL 36610

Page 1 of 1

MiTek USA. Inc.



- NOTES:
- 1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
- 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- 3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

	TOE-NAIL SINGLE SHEAR VALUES PER NDS 2018 (lb/nail)								
	DIAM.	SP	DF	HF	SPF	SPF-S			
G	.131	88.0	80.6	69.9	68.4	59.7			
LONG	.135	93.5	85.6	74.2	72.6	63.4			
.5" L	.162	108.8	99.6	86.4	84.5	73.8			
ы.									
ġ	.128	74.2	67.9	58.9	57.6	50.3			
LONG	.131	75.9	69.5	60.3	59.0	51.1			
3.25"	.148	81.4	74.5	64.6	63.2	52.5			
ŝ									

VALUES SHOWN ARE CAPACITY PER TOE-NAIL

APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

EXAMPLE:

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity















6904 Parke East Blvd Tampa, FL 36610



6904 Parke East Blvd Tampa, FL 36610



- 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- 3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf. Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021



Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:











Roof Load: 37.0 psf

RE: 2623364 - HOUSECRAFT - LAW RES. MiTek USA, Inc. 6904 Parke East Blvd. Site Information: Tampa, FL 33610-4115 Customer Info: Housecraft Homes Project Name: Law Res. Model: Custom Subdivision: N/A Lot/Block: N/A Address: PARCEL RO 4572-001, n/a City: Columbia Cty State: FL Name Address and License # of Structural Engineer of Record, If there is one, for the building. Name: License #: Address: City: State: General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions): Design Code: FBC2020/TPI2014 Design Program: MiTek 20/20 8.4 Wind Code: ASCE 7-16 Wind Speed: 130 mph

This package includes 21 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

Floor Load: N/A psf

No. 1 2 3 4 5 6 7 8 9 10 11 12 3 4	Seal# T24379585 T24379586 T24379587 T24379588 T24379589 T24379590 T24379591 T24379593 T24379593 T24379595 T24379595 T24379595 T24379596 T24379597 T24379597	Truss Name CJ01 CJ03 CJ03A CJ05 CJ05A EJ01 EJ02 HJ10 HJ10A T01 T02 T03 T04 T05	Date 6/18/21 6/18/21 6/18/21 6/18/21 6/18/21 6/18/21 6/18/21 6/18/21 6/18/21 6/18/21 6/18/21 6/18/21 6/18/21 6/18/21	No. 15 16 17 18 19 20 21	Seal# T24379599 T24379600 T24379601 T24379602 T24379603 T24379604 T24379605	Truss Name T06 T07 T08 T09 T10 T11 T11G	Date 6/18/21 6/18/21 6/18/21 6/18/21 6/18/21 6/18/21
13 14	124379597 T24379598	104 T05	6/18/21 6/18/21				

This item has been electronically signed and sealed by Velez, Joaquin, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Lake City, FL.

Truss Design Engineer's Name: Velez, Joaquin

My license renewal date for the state of Florida is February 28, 2023.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



June 18,2021

Velez, Joaquin



1-0-0	
1-0-0	

Plate Offsets (X,Y)	[2:0-0-3,0-0-5]			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.25	Vert(LL) 0.00 7 >999 240	MT20 244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.06	Vert(CT) 0.00 7 >999 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 2 n/a n/a	
BCDL 10.0	Code FBC2020/TPI2014	Matrix-MP		Weight: 7 lb FT = 20%

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

H

Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

-

REACTIONS. (size) 3=Mechanical, 2=0-8-0, 4=Mechanical Max Horz 2=46(LC 12) Max Uplift 3=-27(LC 1), 2=-102(LC 12), 4=-46(LC 1) Max Grav 3=16(LC 16), 2=254(LC 1), 4=29(LC 16)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3, 102 lb uplift at joint 2 and 46 lb uplift at joint 4.

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June 18,2021





			3-0-0	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES	CSI. TC 0.25 BC 0.07 WB 0.00	DEFL. in (loc) l/defl L/d PLATES GRIP Vert(LL) -0.00 4-7 >999 240 MT20 244/190 Vert(CT) -0.01 4-7 >999 180 Horz(CT) 0.00 3 n/a n/a	
BCDL 10.0	Code FBC2020/TPI2014	Matrix-MP	Weight: 13 lb FT = 2	20%
LUMBER-			BRACING-	

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-8-0, 4=Mechanical

Max Horz 2=80(LC 12)

Max Uplift 3=-31(LC 12), 2=-76(LC 12) Max Grav 3=52(LC 1), 2=253(LC 1), 4=48(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 2-11-4 zone;C-C for

members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 4) will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 3 and 76 lb uplift at joint 2.

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> > June 18,2021





					2-8-0			3-0-0			
					2-8-0			0-4-0			
Plate Offsets (X,Y) [3:0-7-8,0-3-3]											
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plate Grip DOL	1.25	TC 0.25	Vert(LL)	-0.00	6	>999	240	MT20	244/190	
TCDL 7.0	Lumber DOL	1.25	BC 0.09	Vert(CT)	-0.01	6	>999	180			

11	JM	R	=P_	

BCLL

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 *Except* 3-6: 2x4 SP No.3

0.0

10.0

BRACING-TOP CHORD BOT CHORD

Horz(CT)

0.01

5

n/a

n/a

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

Weight: 15 lb

FT = 20%

REACTIONS. (size) 4=Mechanical, 2=0-8-0, 5=Mechanical Max Horz 2=80(LC 12) Max Uplift 4=-17(LC 12), 2=-76(LC 12), 5=-6(LC 12) Max Grav 4=42(LC 1), 2=255(LC 1), 5=45(LC 3)

Rep Stress Incr

Code FBC2020/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 2-11-4 zone;C-C for

members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

WB

Matrix-MR

0.00

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

YES

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 4, 76 lb uplift at joint 2 and 6 lb uplift at joint 5.

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	5-0-0											
LOADING	i (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	тс	0.26	Vert(LL)	0.03	4-7	>999	240	MT20	244/190
CDL	7.0	Lumber DOL	1.25	BC	0.23	Vert(CT)	-0.05	4-7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0 Code FBC2020/TPI2014		Matri	x-MP						Weight: 19 lb	FT = 20%	

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-8-0, 4=Mechanical

Max Horz 2=114(LC 12)

Max Uplift 3=-64(LC 12), 2=-80(LC 12) Max Grav 3=108(LC 1), 2=313(LC 1), 4=87(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 4-11-4 zone; C-C for

members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 3 and 80 lb uplift at joint 2.

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Plate Offse	ets (X,Y)	[3:0-4-0,0-1-7]				2-8-0			2-4-0			
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL TCDL	20.0 7.0	Plate Grip DOL Lumber DOL	1.25 1.25	TC BC	0.35 0.24	Vert(LL) Vert(CT)	0.05 -0.08	6 6	>999 >763	240 180	MT20	244/190
BCLL BCDL	0.0 * 10.0	Rep Stress Incr Code FBC2020/T	YES PI2014	WB Matri	0.00 x-MR	Horz(CT)	0.04	5	n/a	n/a	Weight: 21 lb	FT = 20%

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 *Except* 3-6: 2x4 SP No.3 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (size) 4=Mechanical, 2=0-8-0, 5=Mechanical Max Horz 2=114(LC 12) Max Uplift 4=-50(LC 12), 2=-79(LC 12), 5=-10(LC 12) Max Grav 4=97(LC 1), 2=316(LC 1), 5=82(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 4-11-4 zone;C-C for

members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 4, 79 lb uplift at joint 2 and 10 lb uplift at joint 5.

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Plate Offsets (X,Y)	[2:0-1-13,0-1-8]		1							1	
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC	0.60	Vert(LL)	0.10	4-7	>876	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC	0.51	Vert(CT)	-0.21	4-7	>393	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.01	2	n/a	n/a		
BCDL 10.0	Code FBC2020/T	PI2014	Matri	x-MS	. ,					Weight: 26 lb	FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-8-0, 4=Mechanical Max Horz 2=144(LC 12)

Max Holz 2=144(LC 12) Max Uplift 3=-84(LC 12), 2=-90(LC 12)

Max Grav 3=160(LC 1), 2=380(LC 1), 4=125(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 6-11-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 3 and 90 lb uplift at joint 2.

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			2-8-0	7-0-0	
			2-8-0	4-4-0	
Plate Offsets (X,Y)	[2:0-1-13,0-1-8], [3:0-9-2	2,0-2-4]			
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	

TCDL BCLL	20.0 7.0 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code EPC2020//PI2014	CSI. TC 0.65 BC 0.55 WB 0.00 Matrix MB	DEFL. in (loc) l/defl L/d Vert(LL) 0.15 6 >539 240 Vert(CT) -0.26 6 >322 180 Horz(CT) 0.13 5 n/a n/a	PLATES GRIP MT20 244/190 Weight: 28 lb ET = 20%
BCDL	10.0	Code FBC2020/TPI2014	Matrix-MR		Weight: 28 lb FT = 20%

LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2 *Except*

 3-6: 2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (size) 4=Mechanical, 2=0-8-0, 5=Mechanical Max Horz 2=144(LC 12)

Max Uplift 4=-70(LC 12), 2=-89(LC 12), 5=-10(LC 12) Max Grav 4=149(LC 1), 2=384(LC 1), 5=119(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 6-11-4 zone;C-C for

members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 4, 89 lb uplift at joint 2 and 10 lb uplift at joint 5.

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				1	4-6-0		1			5-3-5		0-0-12
	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.59	Vert(LL)	-0.05	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.61	Vert(CT)	-0.12	6-7	>967	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.35	Horz(CT)	0.01	5	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MS						Weight: 44 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

```
LUMBER-
```

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

REACTIONS. (size) 4=Mechanical, 2=0-10-15, 5=Mechanical Max Horz 2=160(LC 4) Max Uplift 4=-79(LC 4), 2=-168(LC 4), 5=-43(LC 8) Max Grav 4=150(LC 1), 2=463(LC 1), 5=266(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-672/142

BOT CHORD 2-7=-180/581, 6-7=-180/581

WEBS 3-6=-611/190

NOTES-

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 4, 168 lb uplift at joint 2 and 43 lb uplift at joint 5.

7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 56 lb down and 103 lb up at 1-6-1, 56 lb down and 103 lb up at 1-6-1, 52 lb down and 33 lb up at 4-4-0, 62 lb down and 33 lb up at 4-4-0, and 41 lb down and 75 lb up at 7-1-15, and 41 lb down and 75 lb up at 7-1-15 on top chord, and 21 lb down and 74 lb up at 1-6-1, 21 lb down and 74 lb up at 1-6-1, 24 lb down and 2 lb up at 4-4-0, 24 lb down and 2 lb up at 4-4-0, and 42 lb down at 7-1-15, and 42 lb down at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-4=-54, 5-8=-20

Concentrated Loads (lb)

Vert: 7=5(F=2, B=2) 11=50(F=25, B=25) 12=-64(F=-32, B=-32) 14=70(F=35, B=35) 15=-49(F=-24, B=-24)

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Plate Offsets (X,Y)	3:0-11-0,0-2-13]		3-8-8		1-9	-			4-3-5		0-0-12
	SPACING-	2.0.0	CSI.		DEFL.		(10.0)	l/defl	i /d	PLATES	GRIP
LOADING (psf) TCLL 20.0 TCDL 7.0	Plate Grip DOL Lumber DOL	2-0-0 1.25 1.25	TC BC	0.64 0.67	Vert(LL) Vert(CT)	in -0.20 -0.33	(loc) 9 9		L/d 240 180	MT20	244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr Code FBC2020/T	NO PI2014	WB Matriz	0.43 x-MS	Horz(CT)	0.12	6	n/a	n/a	Weight: 52 lb	FT = 20%

LUMBER-

 TOP CHORD
 2x4 SP M 31

 BOT CHORD
 2x4 SP No.2 *Except*

 3-9: 2x4 SP No.3, 3-6: 2x6 SP No.2

 WEBS
 2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (size) 5=Mechanical, 2=0-10-15, 6=Mechanical Max Horz 2=160(LC 4) Max Uplift 5=-49(LC 4), 2=-187(LC 4), 6=-97(LC 8) Max Grav 5=110(LC 1), 2=486(LC 1), 6=312(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 3-4=-1162/373

BOT CHORD 3-8=-425/1105, 7-8=-427/1112

WEBS 4-8=-68/353, 4-7=-1141/438

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl.,
- GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 5, 187 lb uplift at joint 2 and 97 lb uplift at joint 6.

7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 56 lb down and 103 lb up at 1-6-1, 56 lb down and 103 lb up at 1-6-1, 56 lb down and 103 lb up at 1-6-1, 66 lb down and 18 lb up at 4-4-0, 66 lb down and 18 lb up at 4-4-0, and 42 lb down and 60 lb up at 7-1-15, and 42 lb down and 60 lb up at 7-1-15 on top chord, and 21 lb down and 74 lb up at 1-6-1, 21 lb down and 74 lb up at 1-6-1, 25 lb down and 15 lb up at 4-4-0, 25 lb down and 15 lb up at 4-4-0, and 42 lb down and 26 lb up at 7-1-15, and 42 lb down and 26 lb up at 7-1-15, and 42 lb down and 51 lb up at 4-4-0, 25 lb down and 51 lb up at 4-4-0, and 42 lb down and 26 lb up at 7-1-15, and 42 lb down and 51 lb up at 7-1-15, and 42 lb down and 51 lb up at 7-1-15, and 42 lb down and 26 lb up at 7-1-15, and 42 lb down and 51 lb up at 7-1-15, and 42 lb down and 26 lb up at 7-1-15, and 42 lb down and 51 lb up at 7-1-15, and 42 lb down and 26 lb up at 7-1-15, and 42 lb down and 51 lb up at 7-1-15, and 42 lb down and 26 lb up at 7-1-15, and 42 lb down and 51 lb up at 7-1-15, and 42 lb down and 26 lb up at 7-1-15, and 42 lb down and 51 lb up at 7-1-15, and 42 lb down and 26 lb up at 7-1-15, and 42 lb down and 51 lb up at 7-1-15, and 42 lb down and 26 lb up at 7-1-15, and 42 lb down and 51 lb up at 7-1-15, and 42 lb down and 51 lb up at 7-1-15, and 42 lb down and 51 lb up at 7-1-15, and 42 lb down and 51 lb up at 7-1-15, and 42 lb down and 50 lb up at 7-1-15, and 42 lb down and 50 lb up at 7-1-15, and 42 lb down and 50 lb up at 7-1-15, and 42 lb down and 50 lb up at 7-1-15, and 42 lb down and 50 lb up at 7-1-15, and 42 lb down and 50 lb up at 7-1-15, and 42 lb down and 50 lb up at 7-1-15, and 42 lb down and 50 lb up at 7-1-15, and 42 lb down and 50 lb up at 7-1-15, and 42 lb down and 50 lb up at 7-1-15, and 42 lb down and 50 lb up at 7-1-15, and 42 lb down and 50 lb up at 7-1-15, and 42 lb down and 50 lb up at 7-1-15, and 42 lb down and 50 lb up at 7-1-15, and 42 lb down and 50 lb up at 7-1-15, and 42 lb down

In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 9-10=-20, 3-6=-20

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See **MSIVTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 This item has been electronically signed and sealed by Velez, Joaquin, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies. Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 18,2021



Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - LAW RES.
0000004				· .	T24379593
2623364	HJ10A	Diagonal Hip Girder	2	1	
					Job Reference (optional)
Builders FirstSource (Lake City,FL),		Lake City, FL - 32055,		8.430 s Jur	n 2 2021 MiTek Industries, Inc. Thu Jun 17 06:22:52 2021 Page 2
			ID:6T5EAYZIt	2BwQ1Mw	cHt2ZyzpbUe-cr5IG5o?ft7Yu6wIydjKCpArA9o8VwbkGhFd5?z5Sxn

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 13=50(F=25, B=25) 14=-3(F=-1, B=-1) 15=-41(F=-21, B=-21) 17=70(F=35, B=35) 18=-21(F=-11, B=-11) 19=-77(F=-39, B=-39)





7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 589 lb uplift at joint 10 and 591 lb uplift at joint 2.

Continued on page 2

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-74/3 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component</u> Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 signed and sealed and the signature must be verified on any electronic copies. Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 18,2021



Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - LAW RES.
					T24379594
2623364	T01	Hip Girder	1	2	
				_	Job Reference (optional)
Builders FirstSource (Lake	City,FL), Lake City, FL - 3	2055,	5	3.430 s Jur	2 2021 MiTek Industries, Inc. Thu Jun 17 06:22:56 2021 Page 2

ID:6T5EAYZIt2BwQ1MwcHt2ZyzpbUe-UcLp5SrWj6d_NjE3BToGMfLY0mAeRfGKAJDrEmz5Sxj

NOTES-

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 125 lb down and 88 lb up at 7-0-0, 106 lb down and 88 lb up at 9-0-12, 106 lb down and 88 lb up at 11-0-12, 106 lb down and 88 lb up at 13-0-12, 106 lb down and 88 lb up at 15-0-12, 106 lb down and 88 lb up at 15-0-12, 106 lb down and 88 lb up at 15-4-4, 106 lb down and 88 lb up at 17-4-4, and 113 lb down and 74 lb up at 23-5-0 on top chord, and 294 lb down and 70 lb up at 7-0-0, 85 lb down at 9-0-12, 85 lb down at 13-0-12, 85 lb down at 13-0-12, 85 lb down at 15-0-12, 85 lb down at 15-0

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-4=-54, 4-8=-54, 8-9=-54, 9-20=-54, 2-14=-20, 9-13=-20, 10-11=-20

Concentrated Loads (lb)

Vert: 4=-106(F) 8=-95(F) 14=-61(F) 7=-106(F) 17=-284(F) 12=-358(F) 24=-106(F) 25=-106(F) 26=-106(F) 27=-213(F) 28=-106(F) 29=-106(F) 30=-61(F) 31=-61(F) 32=-61(F) 33=-122(F) 34=-61(F) 35=-61(F) 35





 	9-0-0 9-0-0	<u>15-2-8</u> 6-2-8	21-		21-9-0 0-4-0	27-9-0 6-0-0		<u>30-5-0</u> 2-8-0
Plate Offsets (X,Y)	[2:0-1-15,0-1-8], [4:0-5-4,0-2-0], [6:0-3-0			0	0.10			200
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0 *	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2020/TPI2014	CSI. TC 0.57 BC 0.76 WB 0.69 Matrix-MS	DEFL. i Vert(LL) -0.2 Vert(CT) -0.4 Horz(CT) 0.2	7 8-11	l/defl L/d >999 240 >777 180 n/a n/a	N	PLATES /IT20 Veight: 170 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SI 69:20 BOT CHORD 2x4 SI 11-12: WEBS 2x4 SI	2x6 SP No.2	BRACING- TOP CHORD BOT CHORD	Rigid c 9-1-3 c	ural wood sheathin eiling directly appl oc bracing: 2-16 oc bracing: 11-12.				
Max H Max L	te) 9=0-8-0, 2=0-8-0 Horz 2=98(LC 12) Jplift 9=-234(LC 13), 2=-285(LC 12) Brav 9=1124(LC 1), 2=1225(LC 1)							

- TOP CHORD 2-3=-2046/450, 3-4=-1787/377, 4-5=-1865/434, 5-6=-1912/432, 6-7=-2313/480, 7-8=-2880/599, 8-19=-557/135
- BOT CHORD
 2-16=-417/1790, 14-16=-281/1557, 11-12=-411/0, 8-11=-507/2746

 WEBS
 3-16=-276/154, 4-16=-24/382, 4-14=-159/476, 5-14=-431/174, 11-13=0/579,
 - :BS 3-16=-276/154, 4-16=-24/382, 4-14=-159/476, 5-14= 6-11=-139/892, 11-14=-338/1744, 7-11=-897/256

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-8, Interior(1) 1-0-8 to 9-0-0, Exterior(2R) 9-0-0 to 13-3-10, Interior(1) 13-3-10 to 21-5-0, Exterior(2R) 21-5-0 to 25-8-10, Interior(1) 25-8-10 to 30-1-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 234 lb uplift at joint 9 and 285 lb uplift at joint 2.

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June 18,2021





REACTIONS. (size) 2=0-8-0, 8=0-8-0 Max Horz 2=97(LC 12) Max Uplift 2=-284(LC 12), 8=-284(LC 13) Max Grav 2=1307(LC 2), 8=1307(LC 2)

7-11=-461/189

- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-2243/426, 3-4=-1807/361, 4-5=-1579/355, 5-6=-1579/355, 6-7=-1807/361,
- 7-8=-2243/426
- BOT CHORD 2-14=-389/1964, 13-14=-389/1964, 11-13=-216/1648, 10-11=-297/1964, 8-10=-297/1964
- WEBS 3-13=-461/188, 4-13=-78/589, 5-13=-252/112, 5-11=-252/112, 6-11=-77/589,

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-8, Interior(1) 1-0-8 to 11-0-0, Exterior(2R) 11-0-0 to 15-2-8, Interior(1) 15-2-8 to 19-5-0, Exterior(2R) 19-5-0 to 23-8-10, Interior(1) 23-8-10 to 32-5-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 284 lb uplift at joint 2 and 284 lb uplift at joint 8.

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L	6-10-2	13-0-0	17-5-0	23-6-14	30-5-0			
	6-10-2	6-1-14	4-5-0	6-1-14	6-10-2			
Plate Offsets (X,Y)	[3:0-3-0,0-3-0], [4:0-5-4,0-2-0], [6:0-3-0,0	0-3-0], [7:0-2-15,Edge]						
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES	CSI. TC 0.42 BC 0.57 WB 0.56	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/ -0.10 12 >999 24 -0.20 12-13 >999 18 0.08 7 n/a n/	0 MT20 0	GRIP 244/190		
BCDL 10.0	Code FBC2020/TPI2014	Matrix-MS			Weight: 162 lb	FT = 20%		
TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 3-9-2 oc purlins. BOT CHORD 2x4 SP No.2 BOT CHORD BOT CHORD Rigid ceiling directly applied or 9-6-4 oc bracing. WEBS 2x4 SP No.3 Reactions. (size) 2=0-8-0, 7=0-8-0 Max Horz Max Horz Max Uplitt 2=-281(LC 12) Max Grav 2=1233(LC 1), 7=1233(LC 1) Here Structural wood sheathing directly applied or 9-6-4 oc bracing.								
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2039/408, 3-4=-1502/352, 4-5=-1276/350, 5-6=-1503/352, 6-7=-2039/409 BOT CHORD 2-13=-378/1762, 12-13=-377/1763, 10-12=-181/1275, 9-10=-283/1763, 7-9=-283/1761 WEBS 3-13=0/278, 3-12=-569/226, 4-12=-77/401, 5-10=-70/401, 6-10=-568/226, 6-9=0/278								
 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-8, Interior(1) 1-0-8 to 13-0-0, Exterior(2E) 13-0-0 to 17-5-0, Exterior(2R) 17-5-0 to 21-8-10, Interior(1) 21-8-10 to 32-5-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 								

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 281 lb uplift at joint 2 and 281 lb uplift at joint 7.

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 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 278 lb uplift at joint 2 and 233 lb uplift at joint 6.

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June 18,2021





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1	Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - LAW RES.
						T24379599
	2623364	T06	Hip Girder	1	2	
					-	Job Reference (optional)
	Builders FirstSource (Lake C	City EL) Lake City EL - 3	2055	1	8 430 s Jur	2 2021 MiTek Industries Inc. Thu Jun 17 06:23:04 2021 Page 2

ID:6T5EAYZIt2BwQ1MwcHt2ZyzpbUe-F8qrnBxXrZdrLyrbf8x8hLgvz?vCJHzV0YAGWJz5Sxb

NOTES-

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 125 lb down and 88 lb up at 7-0-0, 106 lb down and 88 lb up at 9-0-12, 106 lb down and 88 lb up at 11-0-12, 106 lb down and 88 lb up at 13-0-12, 106 lb down and 82 lb up at 15-0-12, 95 lb down and 68 lb up at 15-4-4, 95 lb down and 74 lb up at 17-4-4, 95 lb down and 74 lb up at 17-4-4, 95 lb down and 74 lb up at 17-4-4, 95 lb down and 74 lb up at 17-4-4, 95 lb down and 74 lb up at 17-4-4, 95 lb down and 74 lb up at 17-4-4, 95 lb down and 74 lb up at 19-4-4, and 95 lb down and 74 lb up at 12-4-4, and 113 lb down and 74 lb up at 23-5-0 on top chord, and 294 lb down and 70 lb up at 7-0-0, 85 lb down at 9-0-12, 85 lb down at 13-0-12, 85 lb down at 14-11-12, 79 lb down and 30 lb up at 15-4-4, 79 lb down and 30 lb up at 17-4-4, 79 lb down and 30 lb up at 17-4-4, and 178 lb down and 30 lb up at 21-4-4, and 358 lb down and 156 lb up at 23-4-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-4=-54, 4-9=-54, 9-10=-54, 10-21=-54, 2-16=-20, 10-15=-20, 11-12=-20

Concentrated Loads (lb)

Vert: 4=-106(B) 9=-95(B) 16=-61(B) 6=-106(B) 18=-284(B) 17=-61(B) 5=-106(B) 8=-95(B) 14=-75(B) 13=-358(B) 25=-106(B) 26=-106(B) 27=-95(B) 28=-95(B) 29=-95(B) 30=-61(B) 31=-61(B) 32=-75(B) 33=-75(B) 34=-75(B) 34=-75(B





L	9-0-0	15-2-8	21-5-0	27-9-0	30-5-0
ļ.	9-0-0	6-2-8	6-2-8	6-4-0	2-8-0
Plate Offsets (X,Y)	[2:0-1-15,0-1-8], [4:0-5-4,0-2-0	[6:0-6-0,0-2-8], [8:0-1-8,0-0-0]			
LOADING (psf)	SPACING- 2-0	0 CSI.	DEFL. in (loc) I/def	I L/d PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.	5 TC 0.60	Vert(LL) -0.21 8-11 >999	9 240 MT20	244/190
TCDL 7.0	Lumber DOL 1.	5 BC 0.75	Vert(CT) -0.39 8-11 >932	2 180	
BCLL 0.0 *	Rep Stress Incr Y	S WB 0.52	Horz(CT) 0.23 9 n/a	a n/a	
BCDL 10.0	Code FBC2020/TPI201	Matrix-MS		Weight: 1	77 lb FT = 20%
	P No.2 *Except* 6 SP M 26			od sheathing directly applied or 3 irectly applied or 9-1-10 oc braci	•

BOT CHORD	2x4 SP No.2 *Except* 5-13: 2x4 SP No.3, 8-12: 2x6 SP M 26, 8-10: 2x8 SP 2400F 2.0E
WEBS	2x4 SP No.3
REACTIONS.	(size) 9=0-8-0, 2=0-8-0

Max Horz 2=98(LC 12) Max Uplift 9=-236(LC 13), 2=-284(LC 12) Max Grav 9=1119(LC 1), 2=1225(LC 1)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-2047/447, 3-4=-1793/378, 4-5=-2383/523, 5-6=-2398/527, 6-7=-2367/484, 7-8=-2898/603, 8-17=-636/152

 BOT CHORD
 2-14=-415/1790, 5-12=-379/184, 11-12=-319/2066, 8-11=-501/2715 3-14=-272/155, 12-14=-252/1375, 4-12=-247/989, 6-12=-174/502, 6-11=-103/700, 7-11=-762/235

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-8, Interior(1) 1-0-8 to 9-0-0, Exterior(2R) 9-0-0 to 13-3-10, Interior(1) 13-3-10 to 21-5-0, Exterior(2R) 21-5-0 to 25-8-10, Interior(1) 25-8-10 to 30-1-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 236 lb uplift at joint 9 and 284 lb uplift at joint 2.

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	5-7-15	<u>11-0-0</u> 5-4-1	15-2-8 4-2-8	19-5-0 4-2-8	27-9-0	30-5-0		
Plate Offsets (X,Y) [4:0-5-4,0-2-0], [6:0-6-0,0-2-8], [8:0-1-8,0-0-0]								
LOADING (psf TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	O Plate Grip DOL 0 Lumber DOL 0 Rep Stress Incr	1.25 BC YES W	0.60 0.51	Vert(LL) -0.23 8	loc) I/defl L/d 3-11 >999 240 3-11 >803 180 9 n/a n/a	PLATES GRIP MT20 244/190 Weight: 186 lb FT = 20%		
LUMBER- TOP CHORD 2x4 SP No.2 *Except* BRACING- 6-9: 2x6 SP M 26 TOP CHORD Structural wood sheathing directly applied or 3-11-14 oc purlins.								

- BOT CHORD
 2x4 SP No.2 *Except*

 5-13: 2x4 SP No.3, 8-12: 2x6 SP M 26, 8-10: 2x8 SP 2400F 2.0E

 WEBS
 2x4 SP No.3
- REACTIONS. (size) 9=0-8-0, 2=0-8-0 Max Horz 2=113(LC 12) Max Uplift 9=-234(LC 13), 2=-282(LC 12) Max Grav 9=1119(LC 1), 2=1225(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-2062/420, 3-4=-1628/370, 4-5=-1838/437, 5-6=-1842/439, 6-7=-2076/437, 7-8=-2671/561, 8-18=-636/151
- BOT CHORD 2-15=-400/1791, 14-15=-400/1791, 5-12=-254/124, 11-12=-253/1787, 8-11=-457/2487
- WEBS 3-14=-456/184, 12-14=-225/1291, 4-12=-136/692, 6-11=-111/703, 7-11=-800/278

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-8, Interior(1) 1-0-8 to 11-0-0, Exterior(2R) 11-0-0 to 15-0-12, Interior(1) 15-0-12 to 19-5-0, Exterior(2R) 19-5-0 to 23-10-4, Interior(1) 23-10-4 to 30-1-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 234 lb uplift at joint 9 and 282 lb uplift at joint 2.

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H	<u>6-10-2</u> 6-10-2	13-0-0 6-1-14	15-2-8 17-5-0 2-2-8 2-2-8	22-7-0 5-2-0		7-9-0 5-2-0	30-5-0 2-8-0	
Plate Offsets (X,Y)	[5:0-5-4,0-2-0], [7:0-6-0,0-2-8], [9:0-2-7,0	0-0-0], [14:0-2-4,0-2-8]						
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2020/TPI2014	CSI. TC 0.60 BC 0.58 WB 0.73 Matrix-MS	Vert(LL) -0.21	9-12 >946	L/d 240 180 n/a	PLATES MT20 Weight: 197 lb	GRIP 244/190 FT = 20%	
7-10: 2 BOT CHORD 2x4 SF	² No.2 *Except* 2x6 SP M 26 ² No.2 *Except* 2x4 SP No.3, 9-14: 2x6 SP M 26, 9-11: 2; ² No.3	8 SP 2400F 2.0E	BRACING- TOP CHORD BOT CHORD JOINTS	Structural wood sh Rigid ceiling directl 1 Brace at Jt(s): 12	applied or 6-0		c purlins.	
Max H Max L	e) 10=0-8-0, 2=0-8-0 łorz 2=128(LC 12) Jplift 10=-231(LC 13), 2=-279(LC 12) Grav 10=1119(LC 1), 2=1225(LC 1)							
TOP CHORD 2-3= 8-9= BOT CHORD 2-17 WEBS 3-17	Comp./Max. Ten All forces 250 (lb) or -2019/411, 3-5=-1485/358, 5-6=-1496/38 -2688/535, 9-20=-636/149 =-388/1743, 16-17=-388/1743, 13-14=-19 =0/275, 3-16=-566/225, 5-16=-282/44, 14 =-105/541, 8-13=-1096/323, 8-12=-29/43	6, 6-7=-1501/387, 7-8=- 92/1475, 12-13=-418/242 I-16=-198/1331, 5-14=-1	1738/398, 29, 9-12=-418/2429					
 2) Wind: ASCE 7-16; V GCpi=0.18; MWFR to 17-5-0, Exterior(2 shown; Lumber DO 3) Building Designer / to the use of this true 	e loads have been considered for this de: /ult=130mph (3-second gust) Vasd=101r S (envelope) gable end zone and C-C Ex 2R) 17-5-0 to 21-8-10, Interior(1) 21-8-10 L=1.60 plate grip DOL=1.60 Project engineer responsible for verifying iss component.	nph; TCDL=4.2psf; BCD terior(2E) -2-0-0 to 1-0-8 to 30-1-0 zone;C-C for r	3, Interior(1) 1-0-8 to 13-0- nembers and forces & MW	0, Exterior(2E) 13-0 /FRS for reactions			as been Ily signed and /elez_loaquin	DE

- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 10 and 279 lb uplift at joint 2.

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Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 277 lb uplift at joint 2 and 233 lb uplift at joint 10.

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Plate Offsets (X,Y)	10-0- 10-0- [2:0-4-0,0-2-1], [8:0-4-0,0-2-1], [10:0-4-0	0	[20:0-1-15 0-1-0]			20-0-0 10-0-0		—
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2020/TPI2014	CSI. TC 0.87 BC 0.82 WB 0.39 Matrix-MS	DEFL. Vert(LL) 0. Vert(CT) -0.	in (loc) .33 10-27 .29 10-27 .03 8	l/defl >718 >812 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 117 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP OTHERS 2x4 SP	P No.2 P No.3		BRACING- TOP CHORD BOT CHORD				rectly applied or 4-0-12 or 4-6-12 oc bracing.	oc purlins.
Max H	e) 2=845/0-3-8 (min. 0-1-8), 8=845/0-3 lorz 2=84(LC 12) plift 2=-210(LC 9), 8=-210(LC 8)	-8 (min. 0-1-8)						
TOP CHORD 2-3=- 7-8=- BOT CHORD 2-31=	Comp./Max. Ten All forces 250 (lb) or l -1252/1335, 3-4=-1235/1341, 4-5=-935/1 -1252/1335 =-1163/1127, 10-31=-1163/1127, 10-32=- =-397/420, 5-10=-908/598, 6-10=-397/419	113, 5-6=-935/1113, 6-7= 1170/1127, 8-32=-1170/1	,					
 Wind: ASCE 7-16; V GCpi=0.18; MWFRS 10-0-0 to 13-0-0, Ex reactions shown; Lu Truss designed for 	e loads have been considered for this des /ult=130mph (3-second gust) Vasd=101m S (envelope) gable end zone and C-C Co terior(2N) 13-0-0 to 22-0-0 zone; porch le mber DOL=1.60 plate grip DOL=1.60 wind loads in the plane of the truss only.	ph; TCDL=4.2psf; BCDL ner(3E) -2-0-0 to 1-1-11, ft and right exposed;C-C For studs exposed to wir	Exterior(2N) 1-1-11 to for members and force nd (normal to the face)	10-0-0, Co es & MWFR	rner(3R) S for	try		
 4) Building Designer / I the use of this truss 5) All plates are 2x4 M 6) Gable studs spaced 7) This truss has been 8) * This truss has bee will fit between the b 	T20 unless otherwise indicated.	applied roof live load sho load nonconcurrent with e bottom chord in all area	wn covers rain loading any other live loads. as where a rectangle 3	8-6-0 tall by 2	2-0-0 wid	e	using a Digit Printed copie	v signed and elez, Joaquin, PE al Signature.
joint 8.		, ,	5 <u></u>		Pm		signed and s	sealed and the

LOAD CASE(S) Standard

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