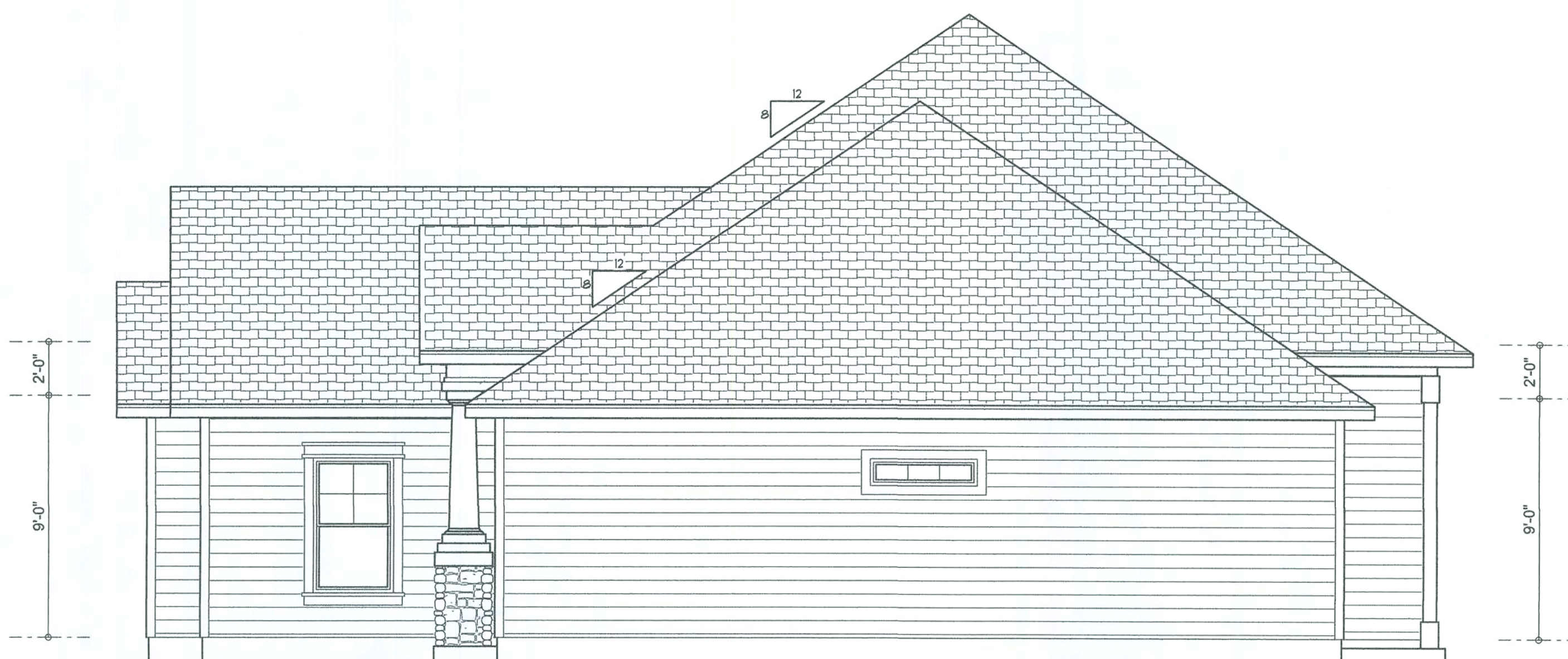




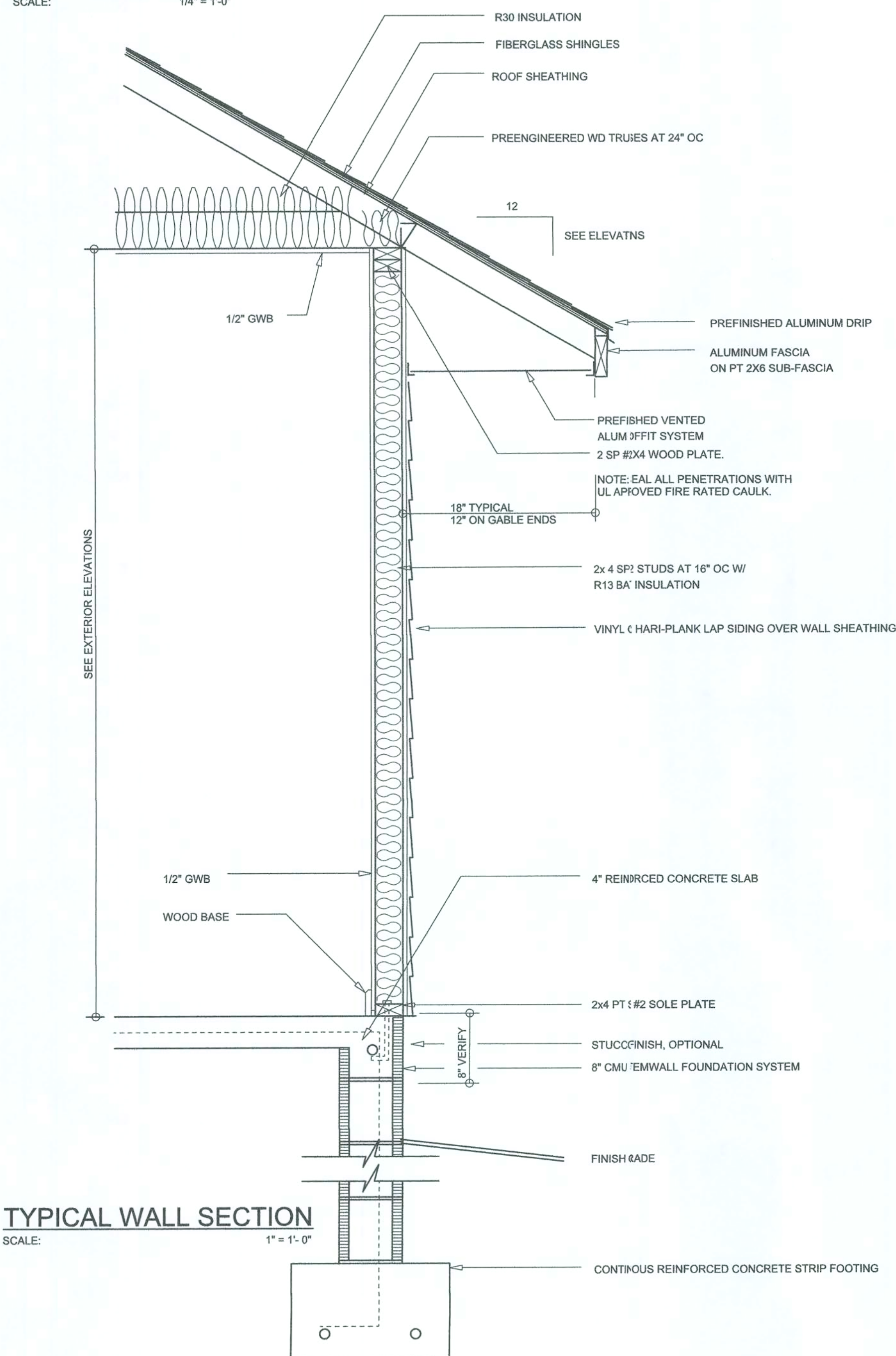
LEFT ELEVATION

SCALE: 1/4" = 1'-0"



RIGHT ELEVATION

SCALE: 1/4" = 1'-0"



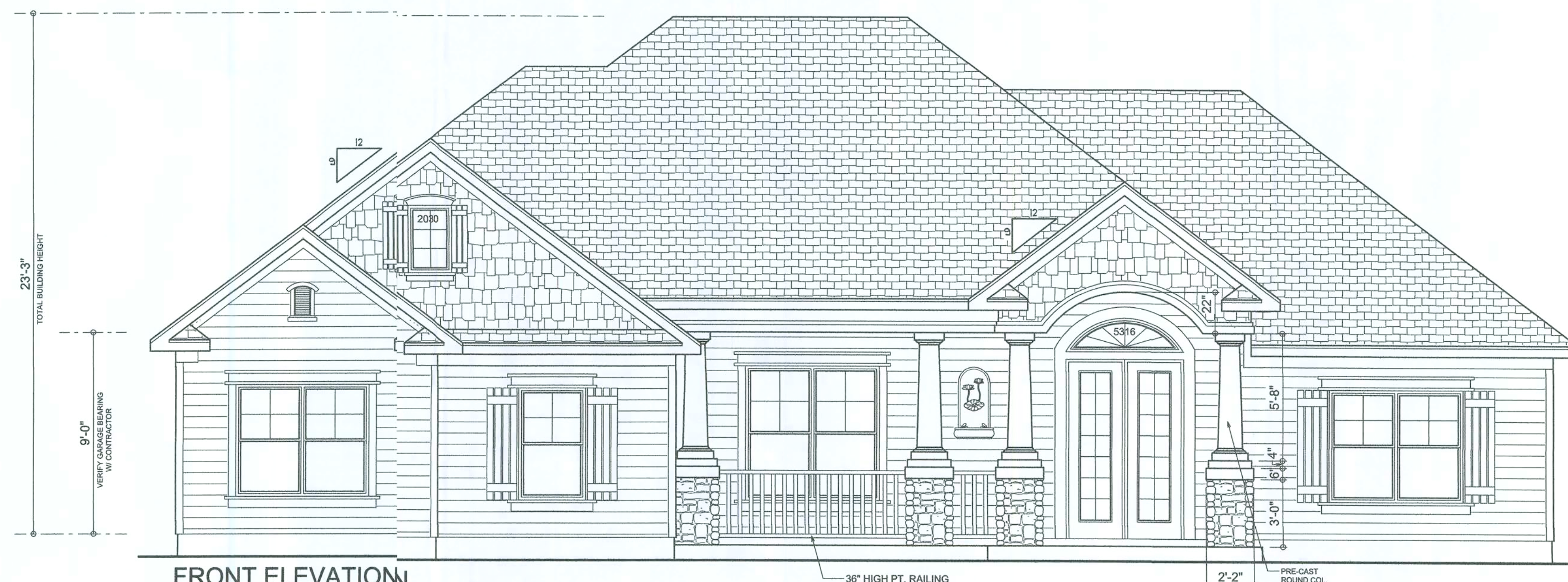
TYPICAL WALL SECTION

SCALE: 1" = 1'-0"



REAR ELEVATION

SCALE: 1/4" = 1'-0"



FRONT ELEVATION

SCALE: 1/4" = 1'-0"

REVISIONS
September 11, 2008

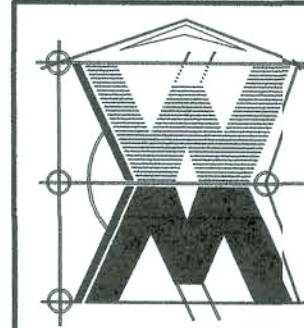
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

EXTERIOR ELEVATIONS
SCALE: 1/4" = 1'-0"

TYPICAL WALL SECTION
SCALE: 1" = 1'-0"

A CUSTOM RESIDENCE FOR:
MIKE & DAISY NICKELSON
PROJECT ADDRESS: Parcel 01-55-16-03390-021, COLUMBIA COUNTY, FL

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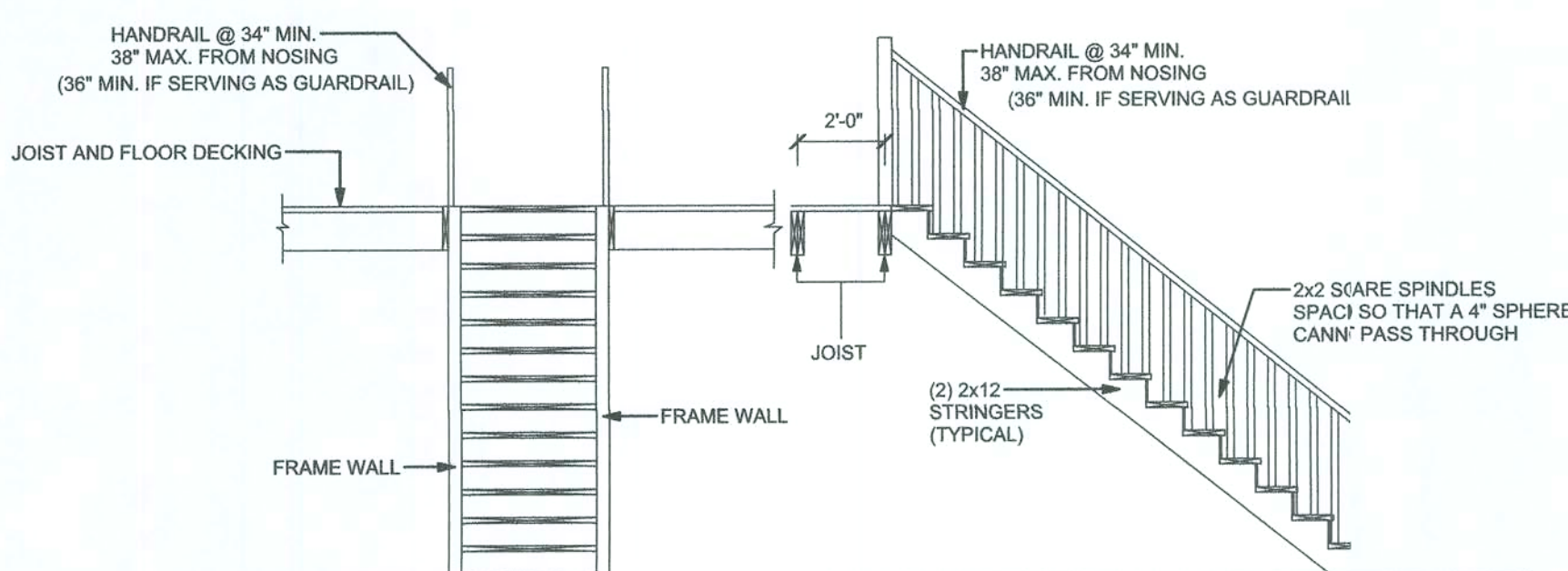


JOB NUMBER
080805

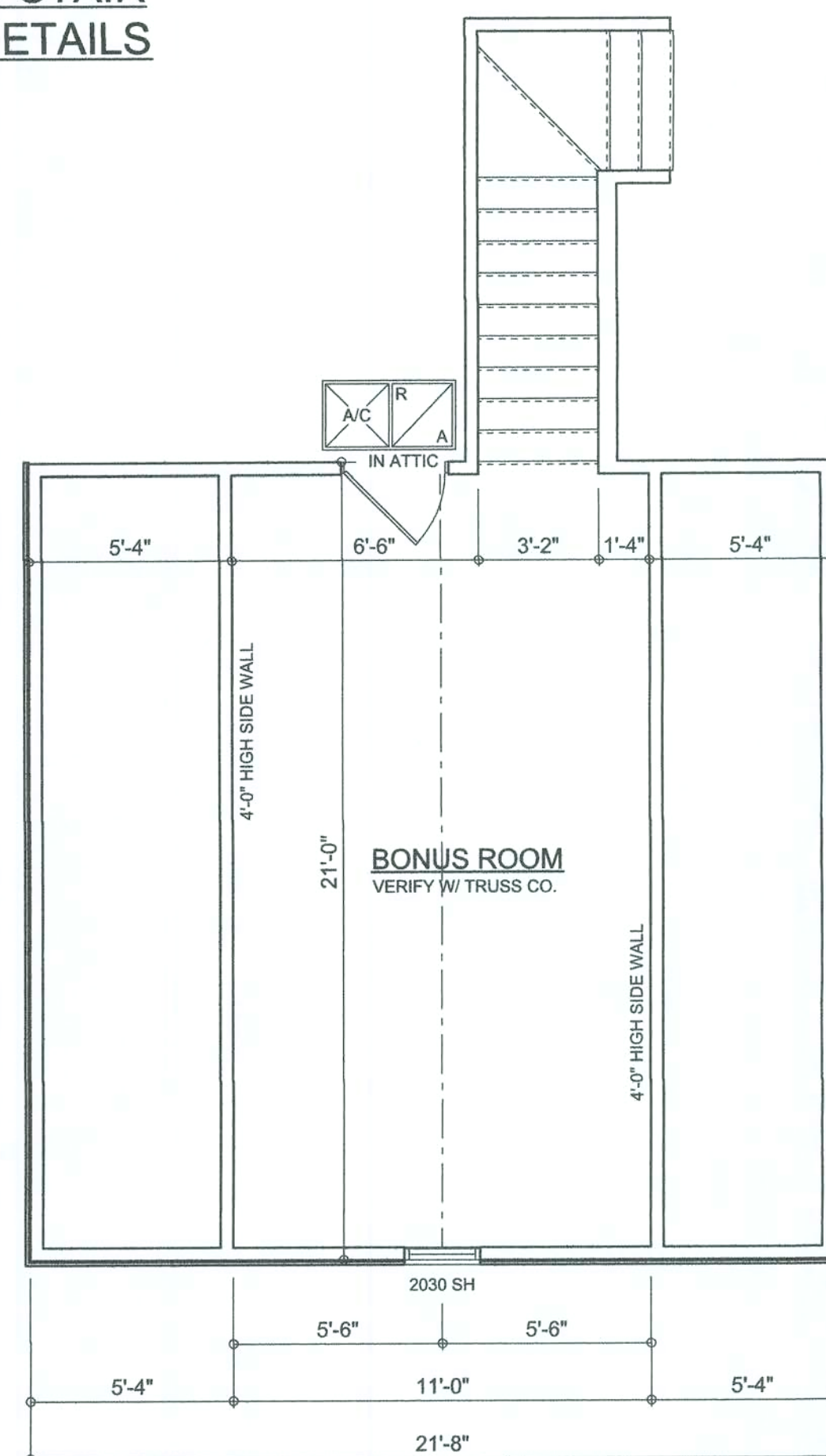
SHEET NUMBER
A.1
OF 3 SHEETS



Wm Myers



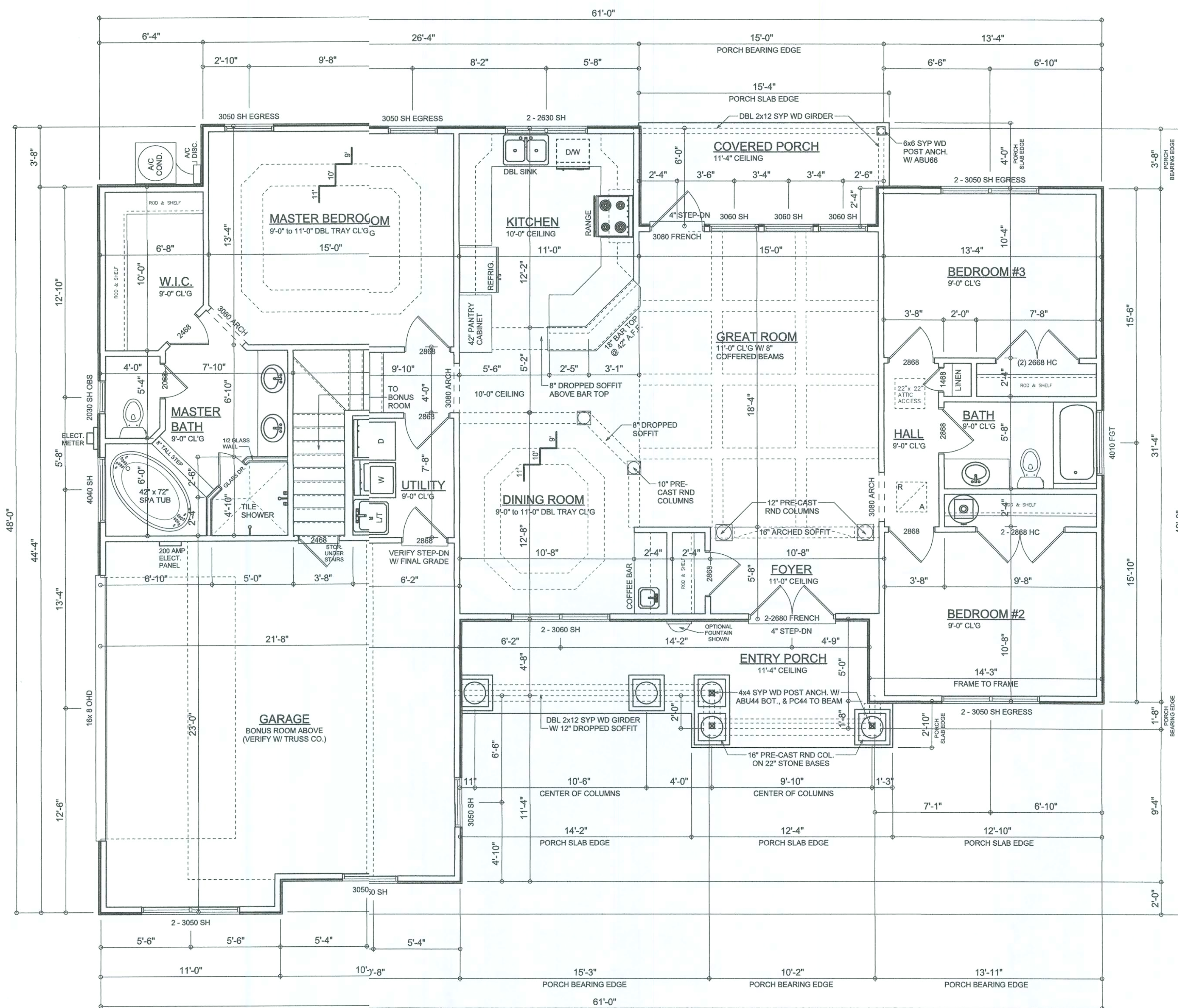
SCALE: NTS



SCALE: $1/4" = 1'-0"$

1. The private garage shall be separated from the dwelling unit and attic area by means of a minimum ½-inch (12.7 mm) gypsum board applied to the garagside. Garages beneath habitable rooms shall be separated from all habitable rooms above it not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core self doors not less than 13/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.

2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.
3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.
4. When installing an attic access and/or pull-down stair unit in the garage, devise shall have a minimum 20 min. fire rating.



SCALE: 1/4" = 1'-0"

LIVING AREA	1628	S.F.
BONUS ROOM AREA	227	S.F.
GARAGE AREA	477	S.F.
ENTRY PORCH AREA	145	S.F.
COVERED PORCH AREA	89	S.F.
TOTAL AREA	2566	S.F.

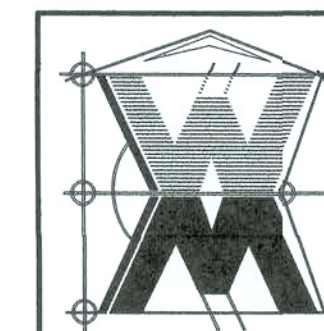
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September 22, 2008

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

FLOOR PLAN
SCALE: 1/4" = 1'-0"

MIKE & DAISY NICKELSON

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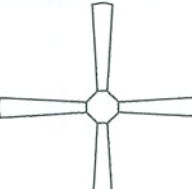
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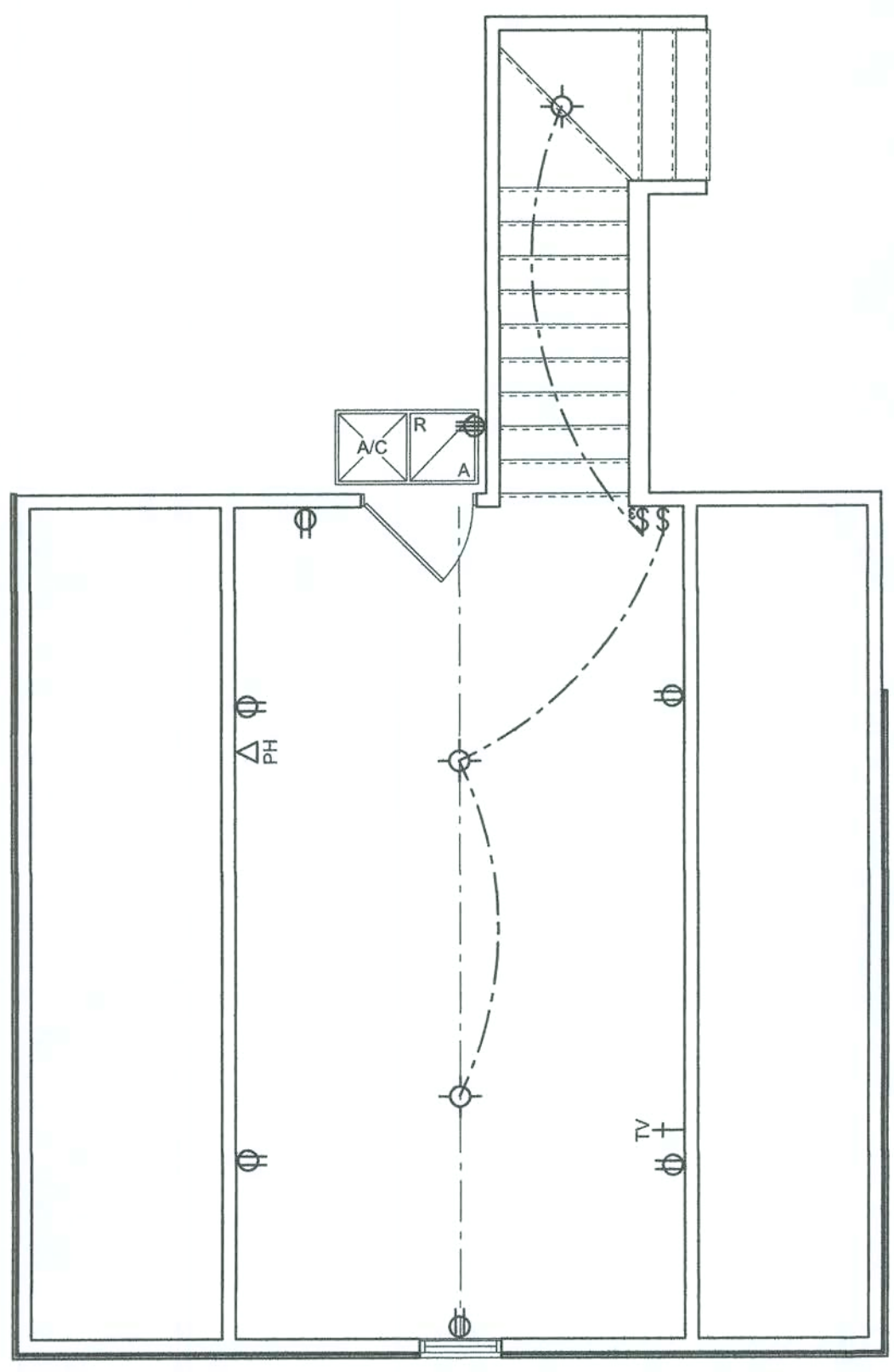
SHEET NUMBER

A.2

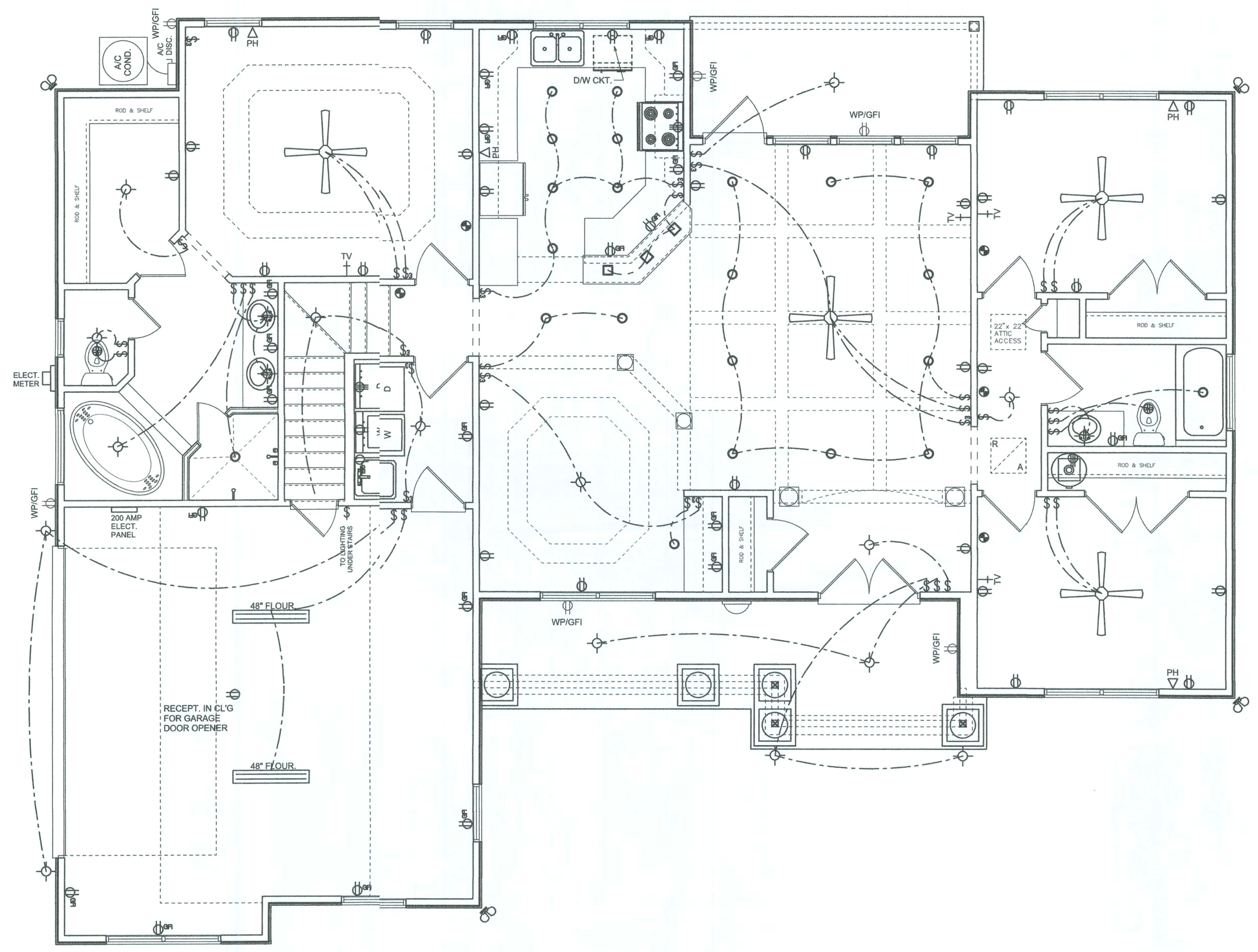
OF 3 SHEETS

Wall C-Mg

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	TELEVISION JACK
	TELEPHONE JACK
	SMOKE DETECTOR (see note below)
	WALL SWITCH
	3 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	2 OR 4 TUB FLUORESCENT FIXTURE



BONUS ELECTRICAL
SCALE: 1/4" = 1'-0"



ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

NOTE:
ALL BEDROOM RECEPTACLES SHALL BE AFCI
(ARC FAULT CIRCUIT INTERRUPT)

ALL SMOKE DETECTORS SHALL HAVE BATTERY BACKUP POWER
AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY
ALL ACTIVATE.

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE
INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS.
CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB
PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR
SHALL BE USED AS AN EQUIPMENT GROUND.

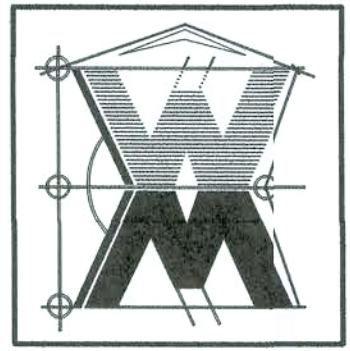
REVISIONS
September 08, 2008



ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

A CUSTOM RESIDENCE FOR:
MIKE & DAISY NICKELSON
PROJECT ADDRESS: Parcel 01-55-16-0338L-01 COI LMBIA COUNTY FL

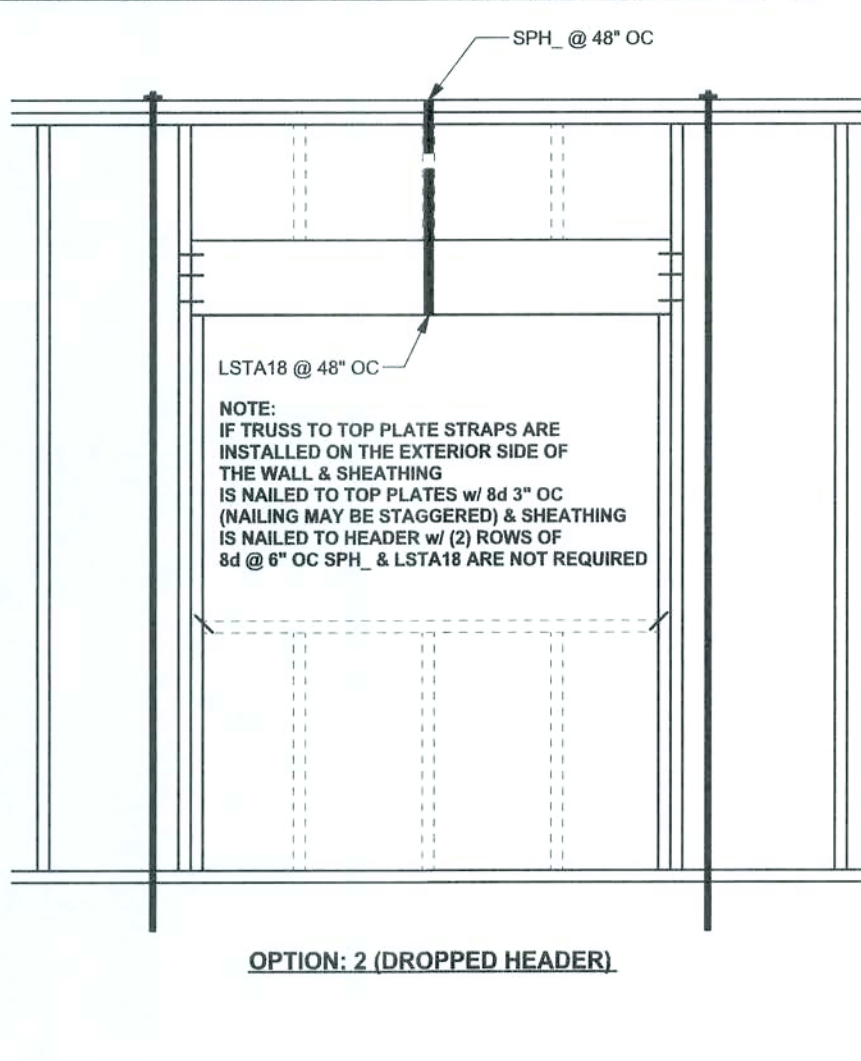
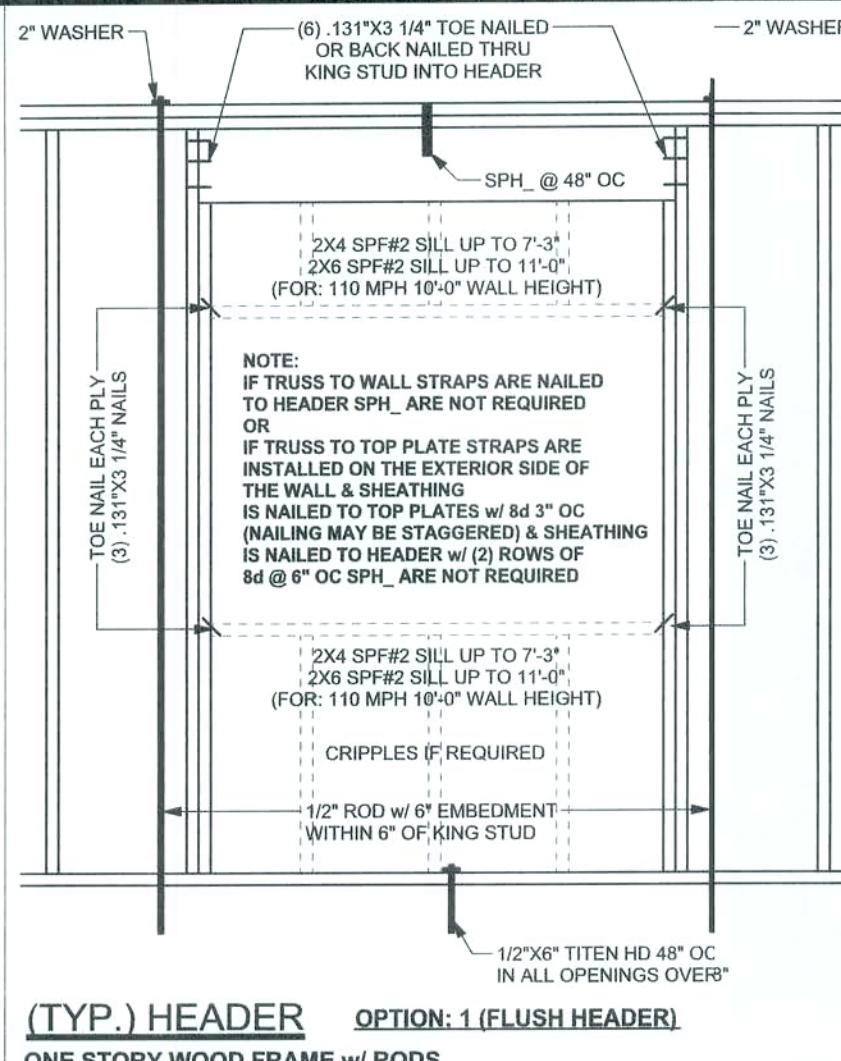
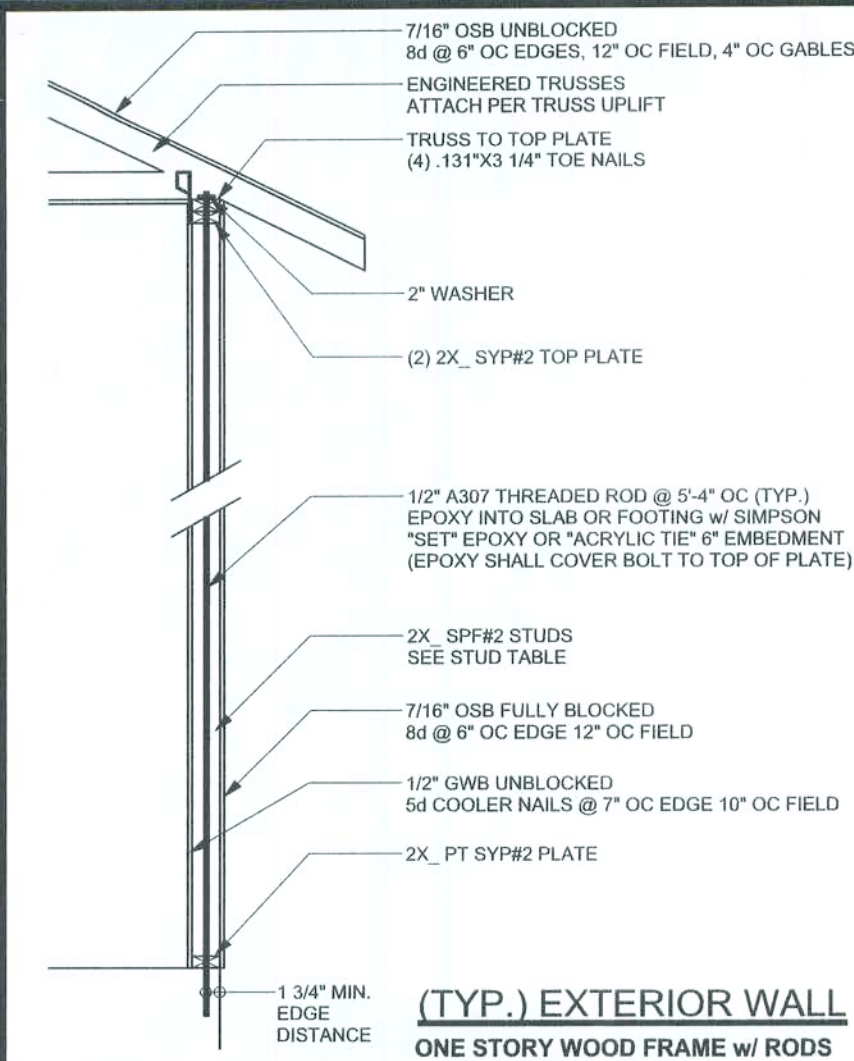
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JOB NUMBER
080805

SHEET NUMBER
A.3
OF 3 SHEETS

Wm Myers



ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SPF	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES
H5	455	285	11 ¹⁶ / ₃₂	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"
H3	415	290	12 ²⁵ / ₃₂	180	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"
H2.5	415	385	12 ⁵⁰ / ₃₂	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"
H2.5A	480	480	11 ¹¹ / ₃₂	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"
H6	950	820					8-8d	
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"
H14-1	1465	1050	5 ¹¹ / ₁₆	265	480	245	12-8d x 1 1/2"	13-8d
H14-2	1465	1050	5 ¹¹ / ₁₆	265	480	245	12-8d x 1 1/2"	15-8d
H10	990	850	5 ⁵⁸ / ₆₄	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"
H10-2	760	655	4 ⁵⁵ / ₆₄	395	390	340	6-10d	6-10d
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
LTS12 - LTS20	1000	620					6-10d x 1 1/2"	6-10d x 1 1/2"
MTS12 - MTS30	1000	860					7-10d x 1 1/2"	7-10d x 1 1/2"
HTS16 - HTS30	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"
HEAVY GIRDER TIEDOWNS								
LGT2	2050	1785	7 ¹⁰ / ₆₄	170	700	170	14-16d	14-16d
LGT3-SDS2.5	3685	2655	7 ⁷⁹ / ₆₄	410	795	410	12-SDS 1/4" x 2 1/2"	26-16dS
LGT4-SDS3	4060	3860	2 ¹⁰⁰ / ₆₄	675	2000	675	12-SDS 1/4" x 3"	36-16dS
MGT	3965	3330					22-10d	5/8" ANCHOR
HGT-2	10980	6485					16-10d	2-5/8" ANCHOR
HGT-3	10530	9035					16-10d	2-5/8" ANCHOR
HGT-4	9250	9250					16-10d	2-5/8" ANCHOR
STUD STRAP CONNECTOR								
SSP DOUBLE TOP PLATE	435	435					3-10d	4-10d
SSP SINGLE SILL PLATE	455	420					1-10d	4-10d
DSP DOUBLE TOP PLATE	825	825					6-10d	8-10d
DSP SINGLE SILL PLATE	825	600					2-10d	8-10d
SP1	585	535					4-10d	6-10d
SP2	1065	605					6-10d	6-10d
SP4	885	760					6-10d x 1 1/2"	
SP14	1240	1065					10-10d x 1 1/2"	
SP16	885	760					6-10d x 1 1/2"	
SP16	1240	1065					10-10d x 1 1/2"	
LSTA18	1235	1110					14-10d	
LSTA21	1235	1235					14-10d	
CS20	1030	1030					22-10d	
CS16	1705	1705						
STUD ANCHORS								
LTT19	1350	1305					8-16d	1/2" ANCHOR
LTT31	2310	2310					16-10d x 1 1/2"	5/8" ANCHOR
HD2A	2775	2570					2-5/8" BOLTS	5/8" ANCHOR
HTT16	4175	3695					16-16d	5/8" ANCHOR
HTT22	5260	5250					32-16d	5/8" ANCHOR
ABU44	2200	2200					12-16d	5/8" ANCHOR
ABU66	2300	2300					12-16d	5/8" ANCHOR
ABU88	2320	2320					16-16d	2-5/8" ANCHOR

(1) w/ INSTALLATION OF 4-16dS OPTIONAL NAIL L HOLES
(2) FOR SYP GIRDER & SPF STUDS

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.65 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.65 = 13'-6" O.C.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBOR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER IS FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X8 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'_c = 3000 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W1.4, FB = 8KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 12 INCH TO 2 INCHES, DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH/WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W.W.M. OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI, ALL LAP SPLICES 40" DB (20" FOR #5 BARS). UNCL. ANCHOR REINFORCEMENT SHALL BE DETAILED AND LOCATED IN ACCORDANCE WITH ACI 315-86, UNCL.

GLULAM BEAMS: GLB, 24F-V3SP, F_b = 2.4ksi, E = 1800ksi. UNCL. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (13d), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC, UNCL.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE. NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 16" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 3" x 3" x 5/16", UNCL.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK:

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBOR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBOR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBOR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE ROOF ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD HAVE THE ROOF CHECKED BY THE ROOF DESIGNER BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

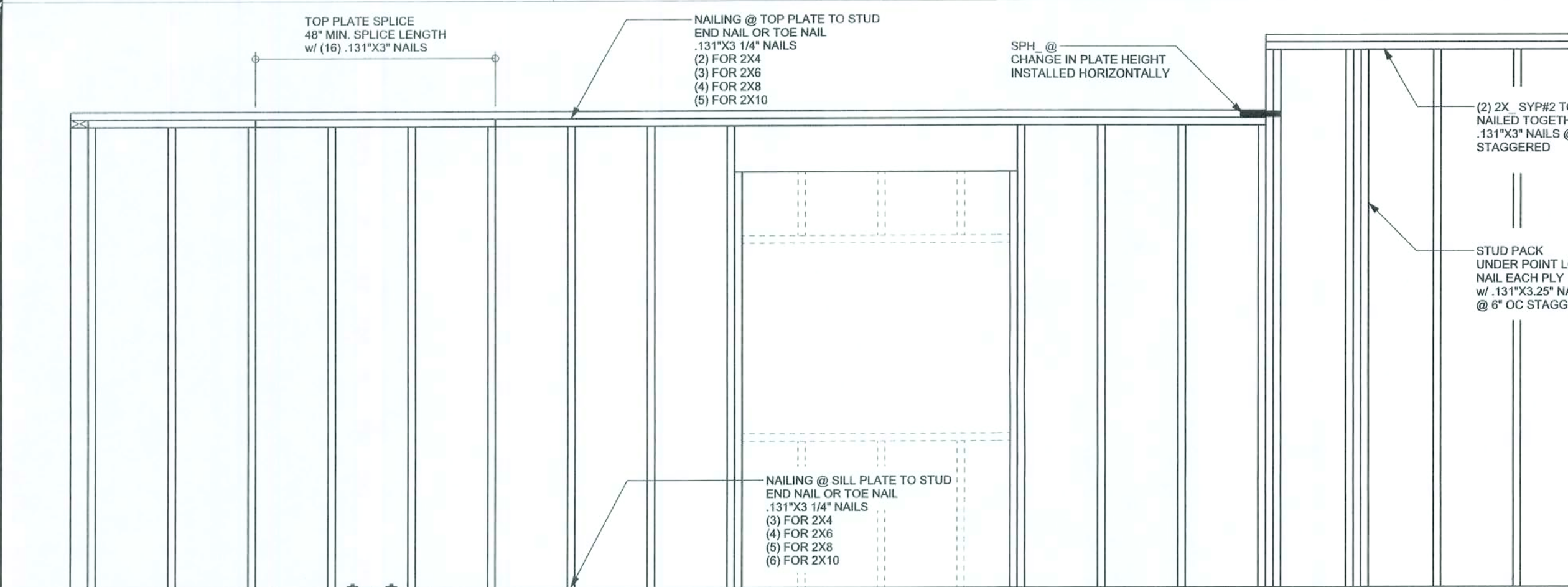
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C, AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

- BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
- BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION
- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = NA (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

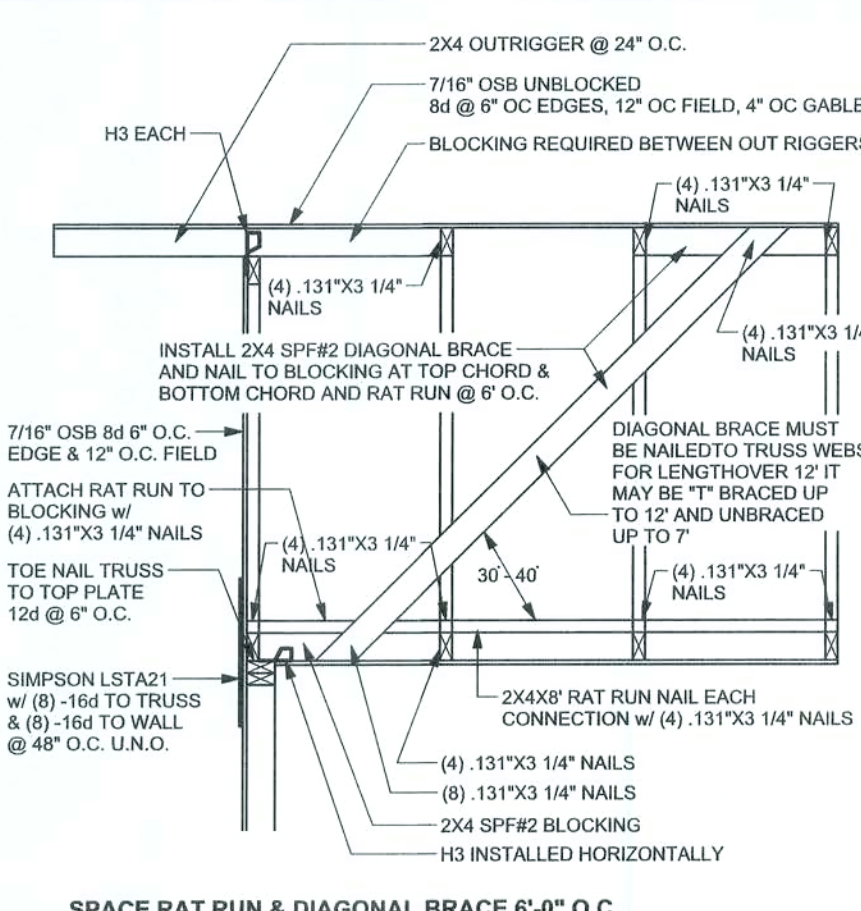
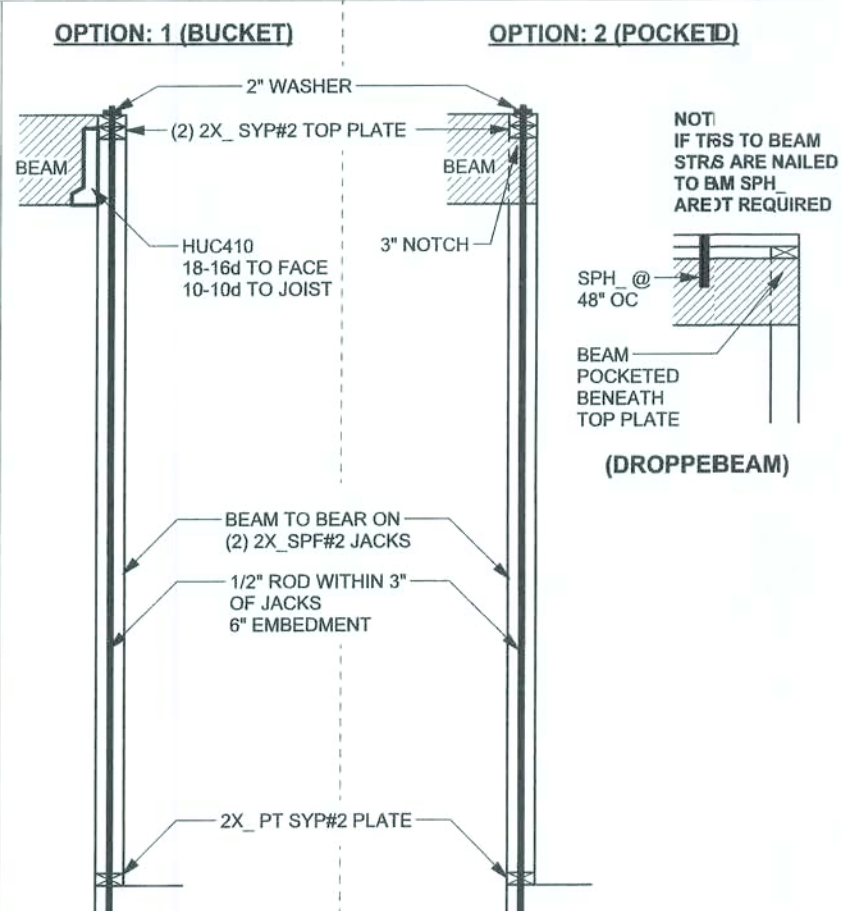
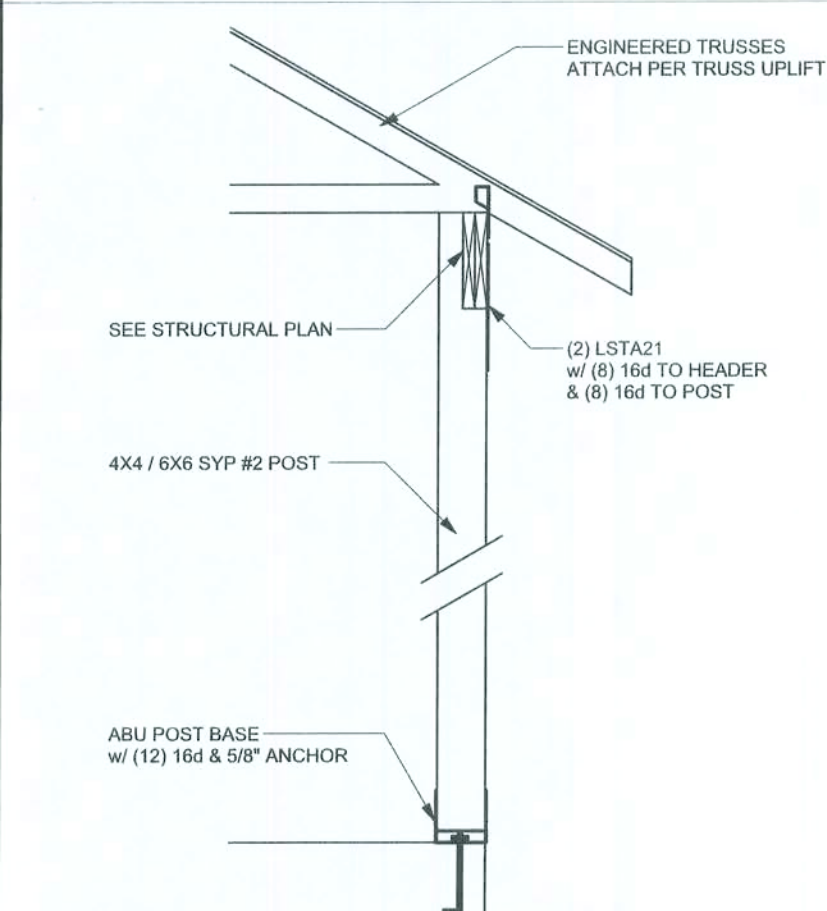
Zone	Effective Wind Area (ft ²)	10'	15'	20'	25'	30'	35'	40'	45'	50'	55'	60'	65'	70'	75'	80'	85'	90'	95'	100'
1	19.9	21.8	18.1	-18.1																
2	19.9	25.5	18.1	-21.8																
2.0th		-40.6		-40.6																
3	19.9	25.5	18.1	-21.8																
3.0th		-48.3		-42.4																
4	21.8	-23.6	18.5	-20.4																
5	21.8	-29.1	18.5	-22.6																
Doors & Windows Worst Case (Zone 5, 10 ft ²)																				
8x7 Garage Door																				
16x7 Garage Door																				

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3'12")
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

(TYP.) INTERIOR BEARING WALL ONE STORY WOOD FRAME w/ RODS



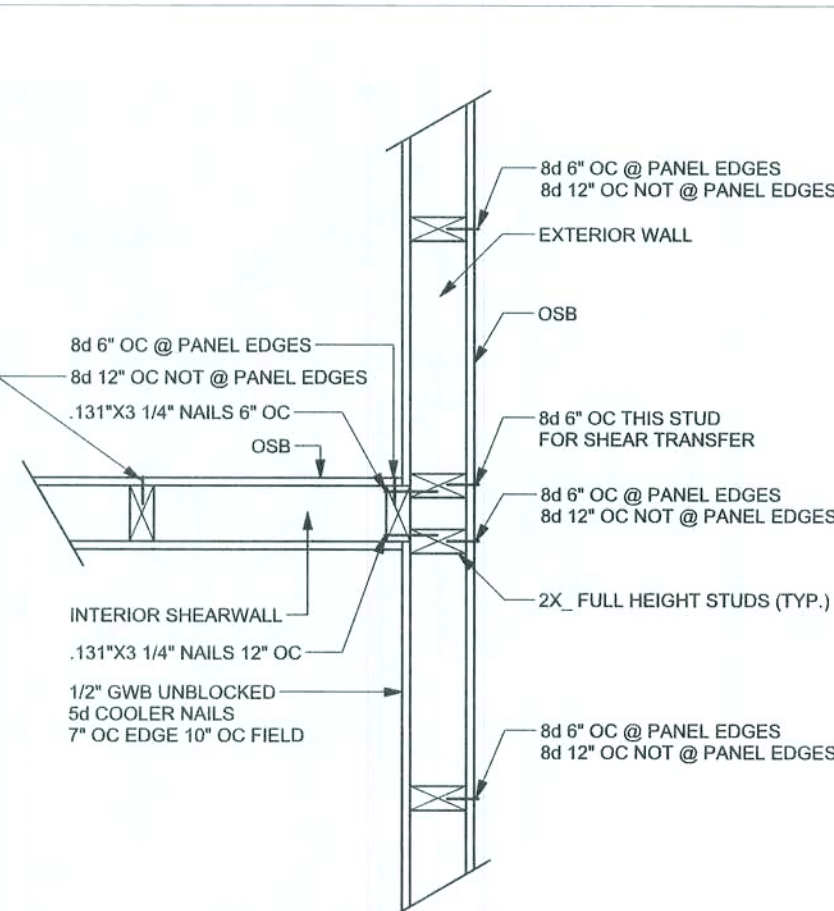
(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME



ALTERNATE CONNECTION WHERE ROD CANNOT BE PLACED IN WALL ONE STORY WOOD FRAME w/ RODS



(TYP.) CORNER FRAMING WOOD FRAME

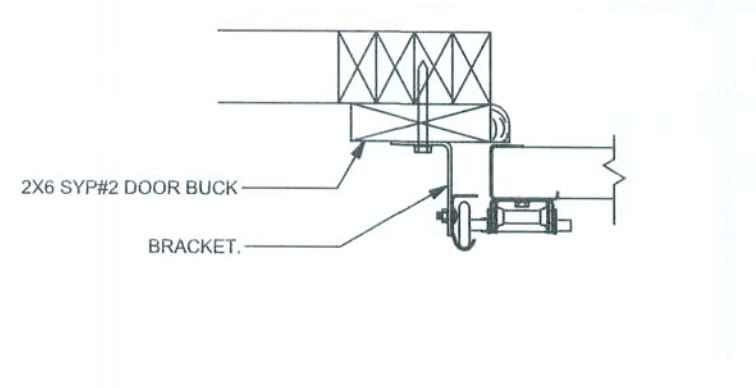


(TYP.) INTERSECTING WALL FRAMING WOOD FRAME



2X6 SYP#2 GARAGE DOOR BUCK ATTACHMENT

DOOR WIDTH	3/8\"/>
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(TYP.) GARAGE DOOR BUCK INSTALLATION WOOD FRAME



REVISIONS

NO.	DESCRIPTION	DATE
1	ISSUED FOR PERMIT	08/16/2008

SOFTWARE
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER
Mark Disoway, P.E.
No. 53915, P.O.B. 868, Lake City, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for solution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915

Adam's Framing
& Constuction

Mike & Daisy
Nickelson Residence

ADDRESS:
Parcel ID: 01-5S-6-03390-021
Columbia County, Florida

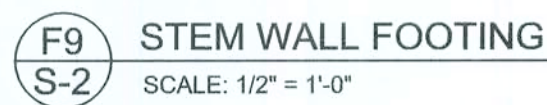
Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386)754 - 5419
Fax: (386) 289 - 4871

PRINTED DATE:
September 16, 2008

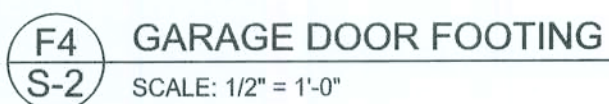
DRAWN BY: STRUCTURAL BY:
David Disoway

FINALS DATE:
16Sep08

JOB NUMBER:
809831
DRAWING NUMBER
S-1
OF 3 SHEETS

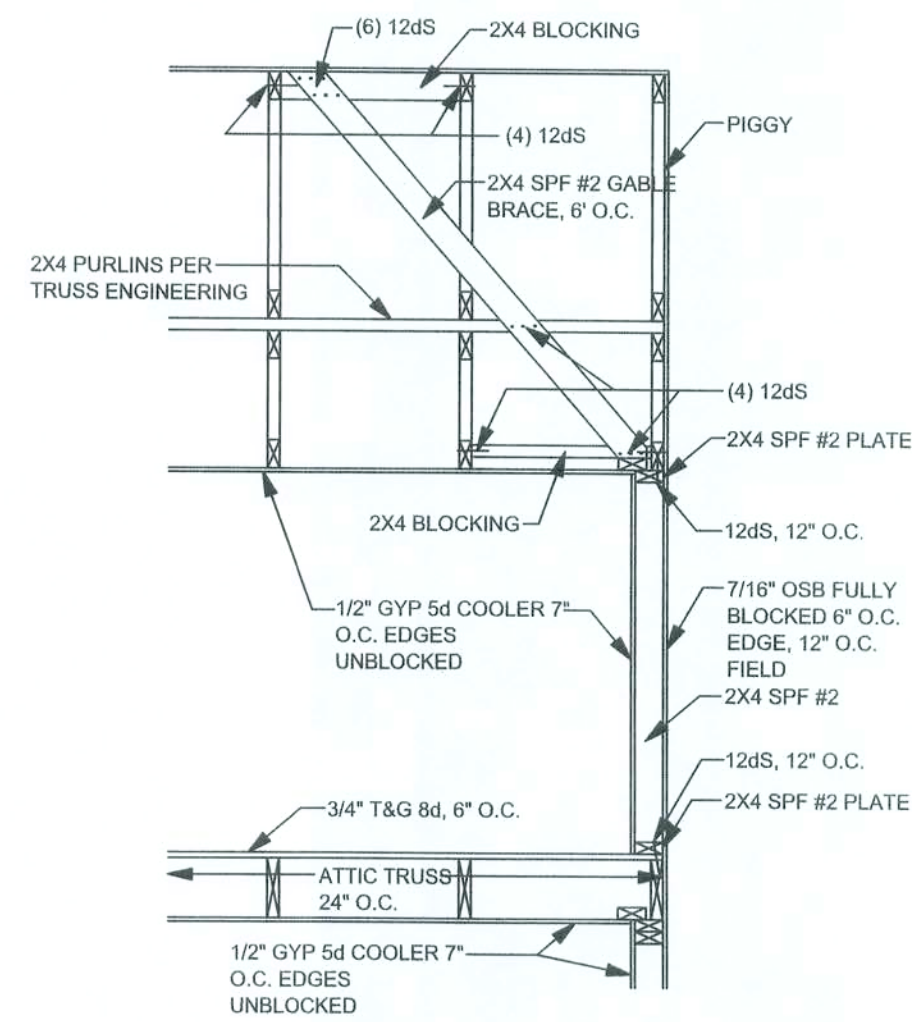
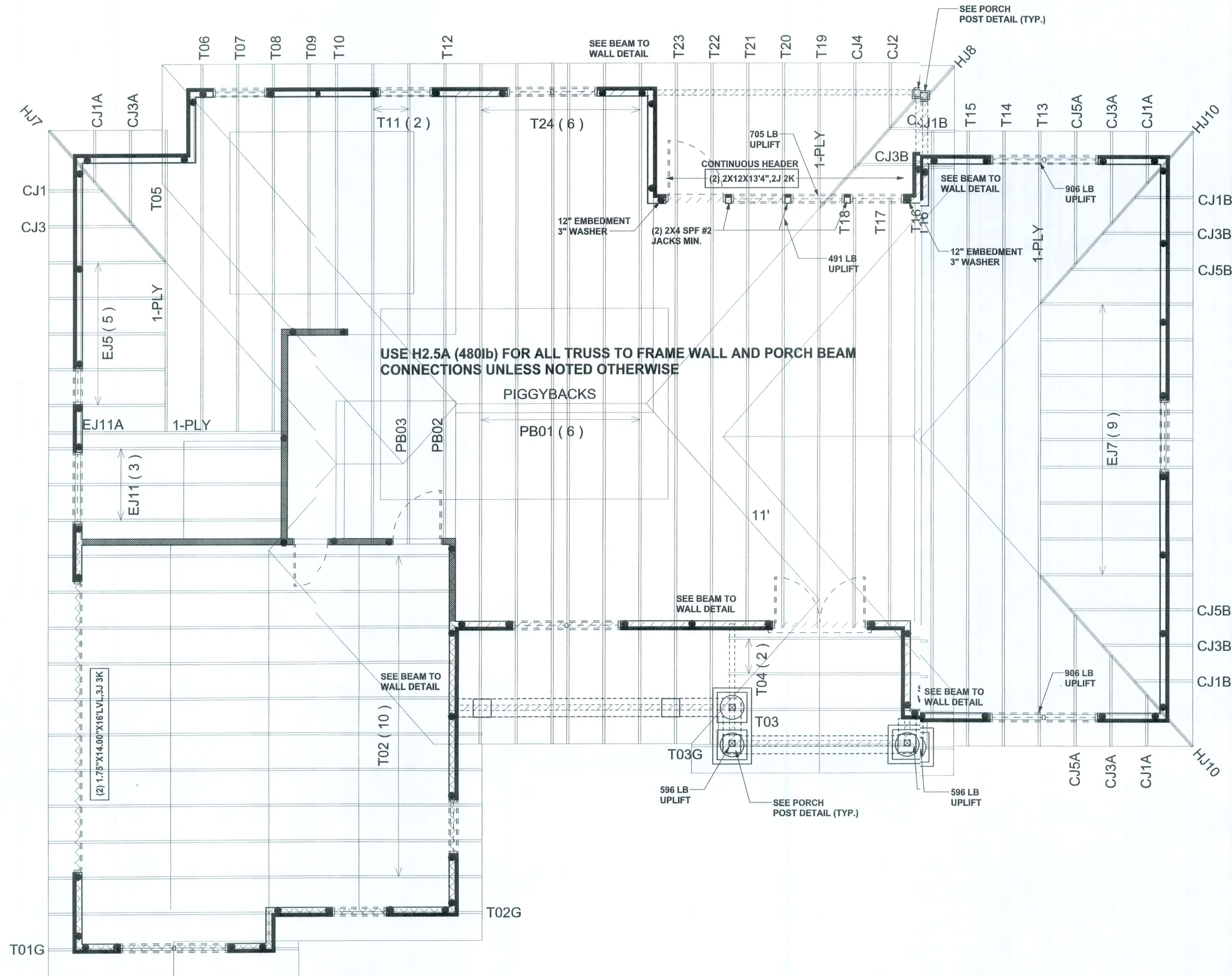


STEWALL HEIGHT (FEET)	UNBALANCE BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEWWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEWWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

OF 3 SHEETS



W67 - BONUS ROOM / GABLE END BRACING
SCALE: 1/2"=1'-0"

STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BC1-03, BC31-01, BC31-02, & BC31-03. BC31-02, & BC31-03 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

THREADED ROD LEGEND

- INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD
- INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

HEADER LEGEND

- HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH) (1)
- NUMBER OF JACK STUDS (UNDER HEADS) (ER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

TOTAL SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	33.6'	50.0'
LONGITUDINAL	23.9'	87.5'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB # L288684

REVISIONS



WINDLOAD ENGINEER:
Mark Disosway, P.E.
No. 53915, P.O. Box 868, Lake City, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions supersede noted dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section RS01.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

Mark Disosway
16 SEP 08
SEAL

Adam's Framing
& Construction

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PRINTED DATE
September 16, 2008

DRAWN BY: STRUCTURAL BY:
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FINALS DATE:
16Sep08

JOB NUMBER:
809081

DRAWING NUMBER

S-3

OF 3 SHEETS