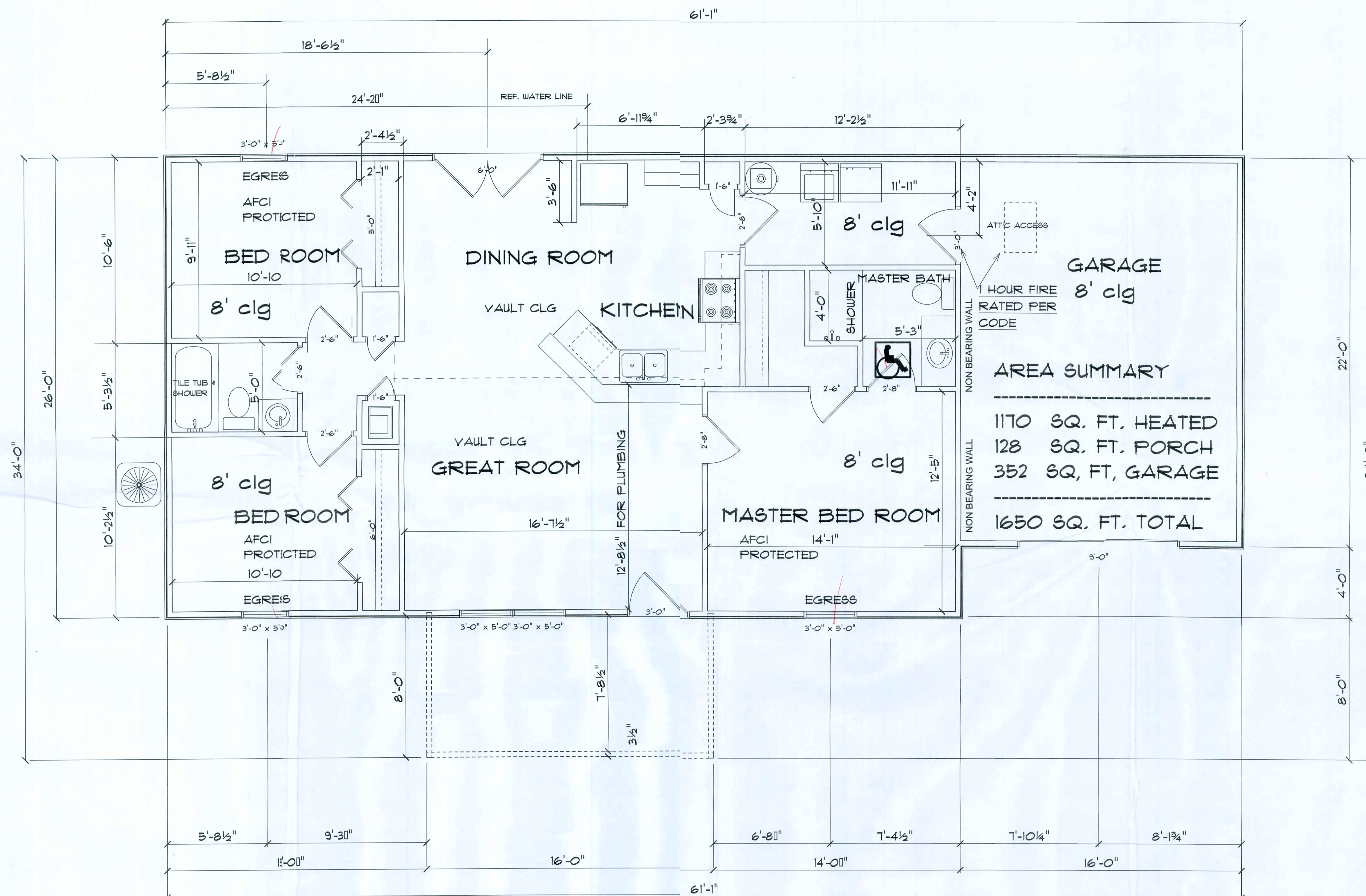


REVISIONS	
1ST DRAWING 01-17-08	JFB

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ARCHITECTURAL DESIGN SOFTWARE



AREA SUMMARY

1170 SQ. FT. HEATED
128 SQ. FT. PORCH
352 SQ. FT. GARAGE
1650 SQ. FT. TOTAL

HOMETOWN HOMES

ADDRESS:
Lot 1 Country Creek Est.
Lake City
COLUMBIA COUNTY, FLORIDA
ARC. DRAWN BY:
JFB

PRINTED DATE:
January 22, 2008

JOB NAME:

MOSS RES.
Keen Job

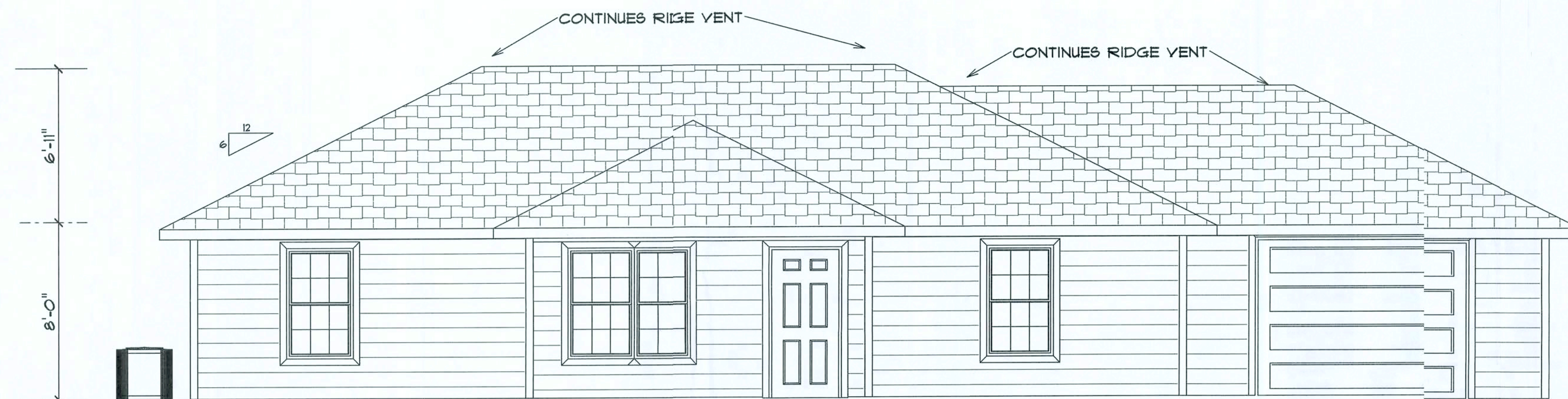
FINALS DATE:
06 / OCT / 04

DRAWING NUMBER

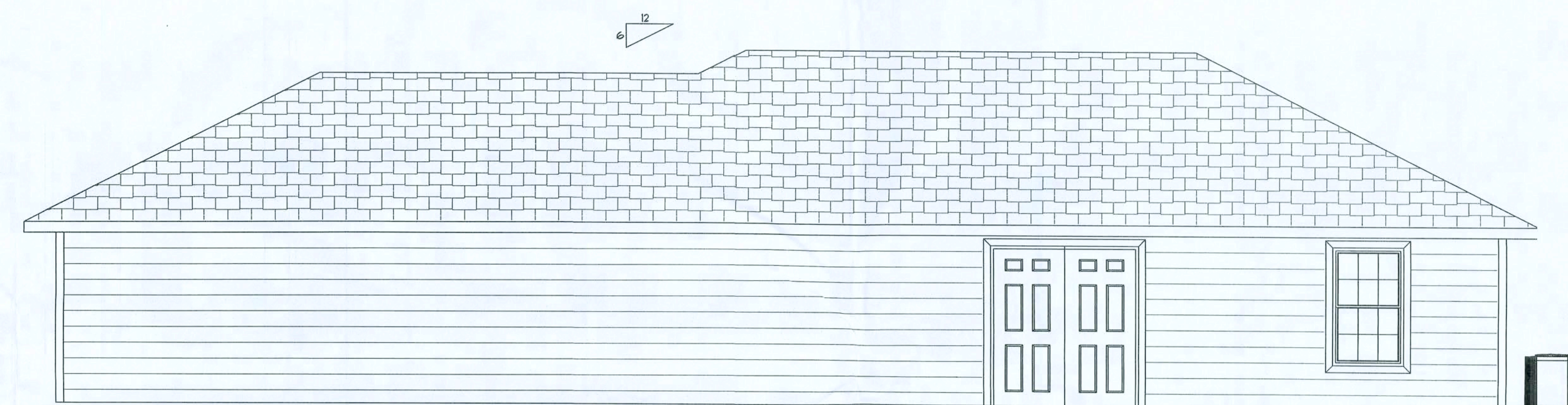
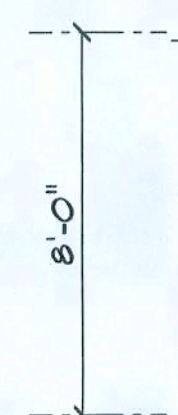
A1 FLOOR
1 OF 3 SHEETS

REVISIONS		
1ST DRAWING	01-17-08	JFB

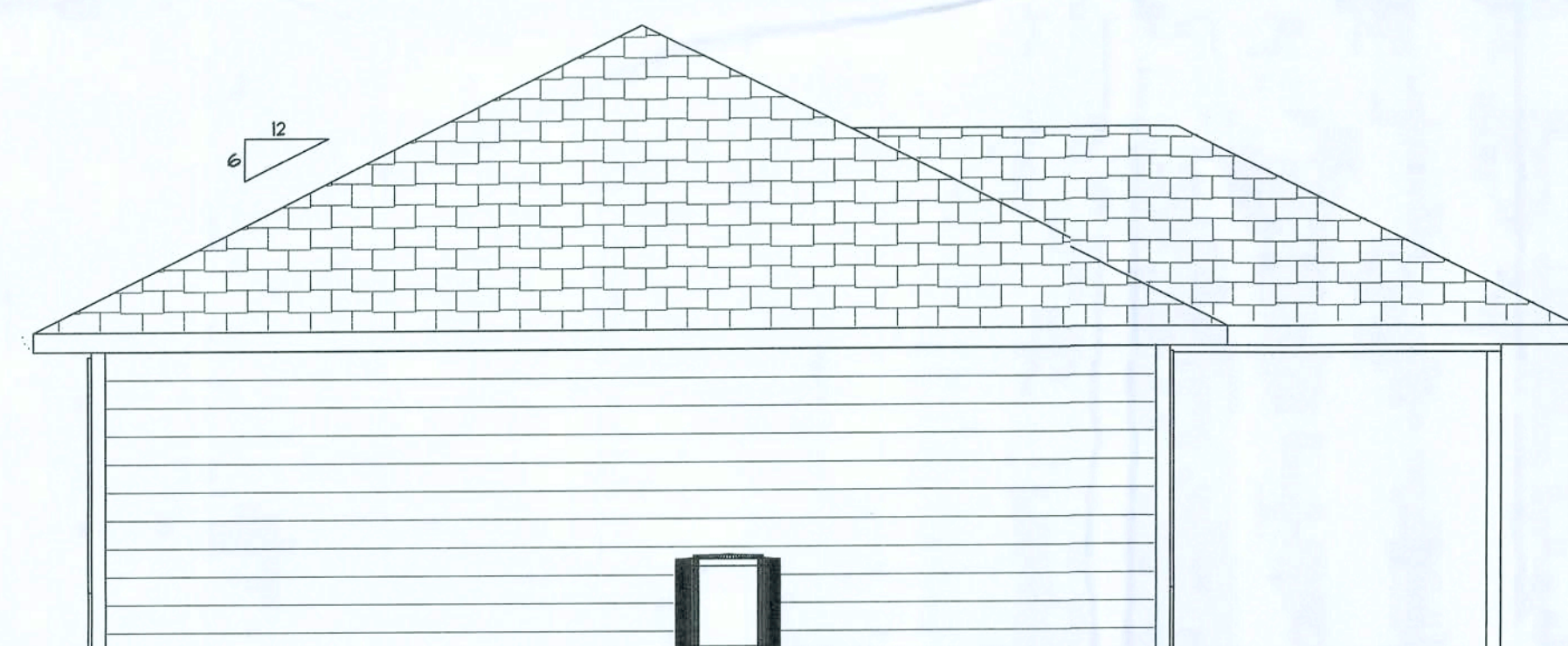
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



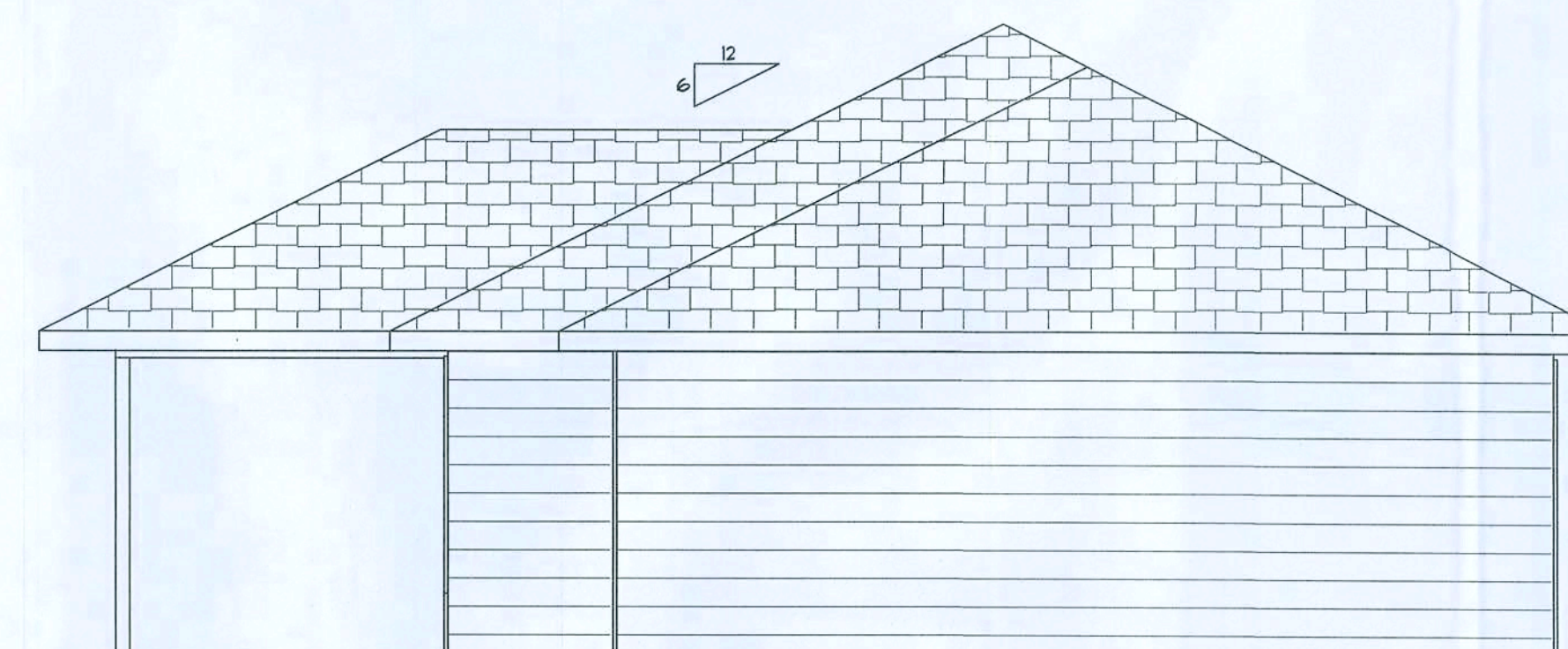
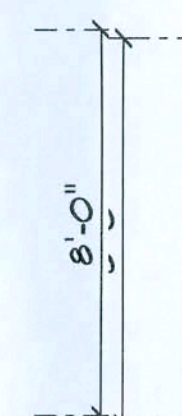
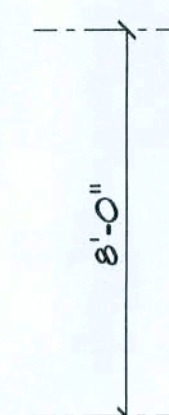
1ST FLOOR PLAN



REAR ELEVATION



LEFT ELEVATION



RIGHT ELEVATION

**HOMETOWN
HOMES**

ADDRESS:
Lot 9 Country Creek Est.
Lake City
COLUMBIA COUNTY, FLORIDA
ARC. DRAWN BY:
JFB

PRINTED DATE:
January 22, 2008

JOB NAME:

FINALES DATE:
06 / OCT / 04

DRAWING NUMBER

A2 ELEV.
2 OF 3 SHEETS

REVISIONS	
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SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

HOMETOWN HOMES

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Lake City
COLUMBIA COUNTY, FLORIDA
ARC. DRAWN BY:
JFB

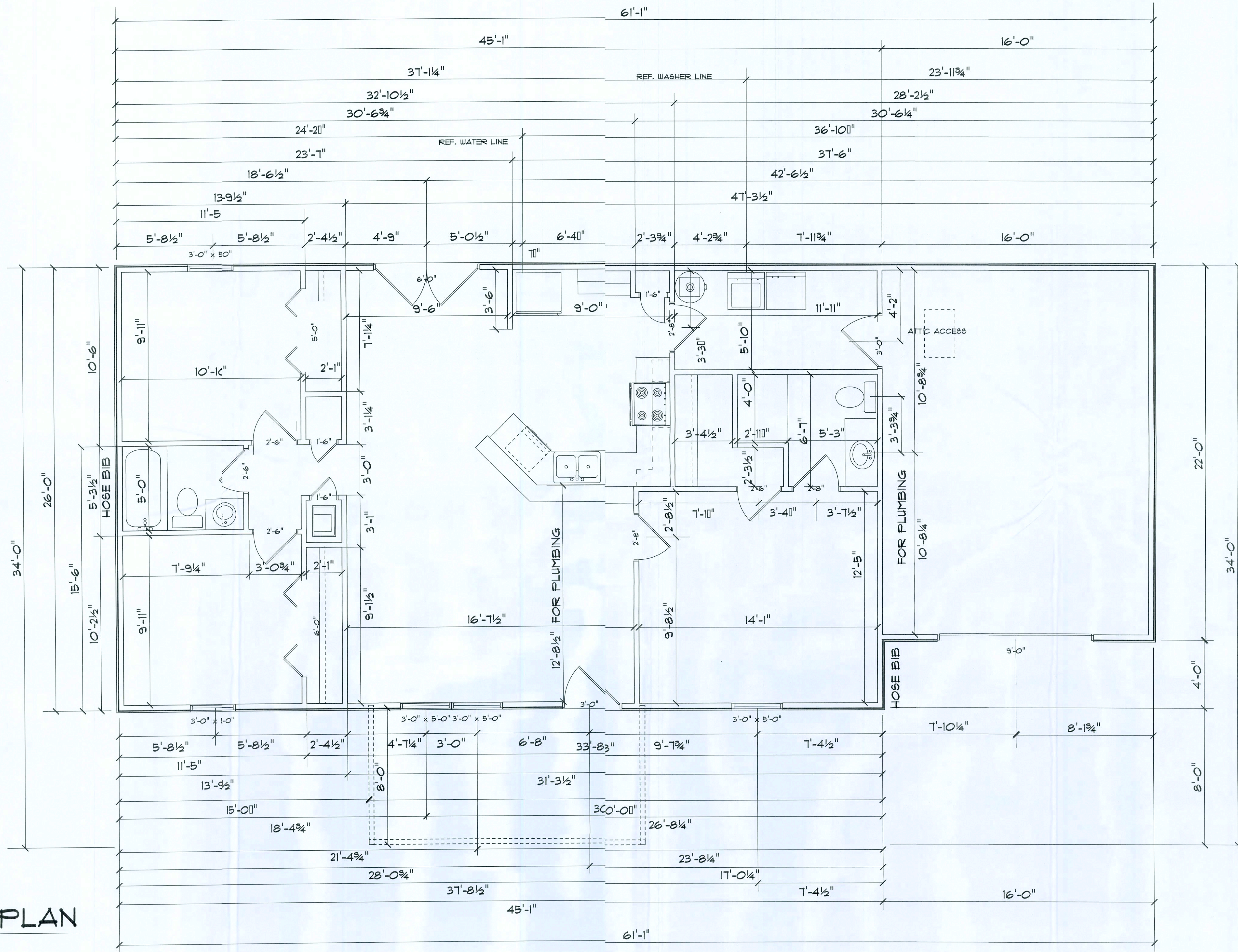
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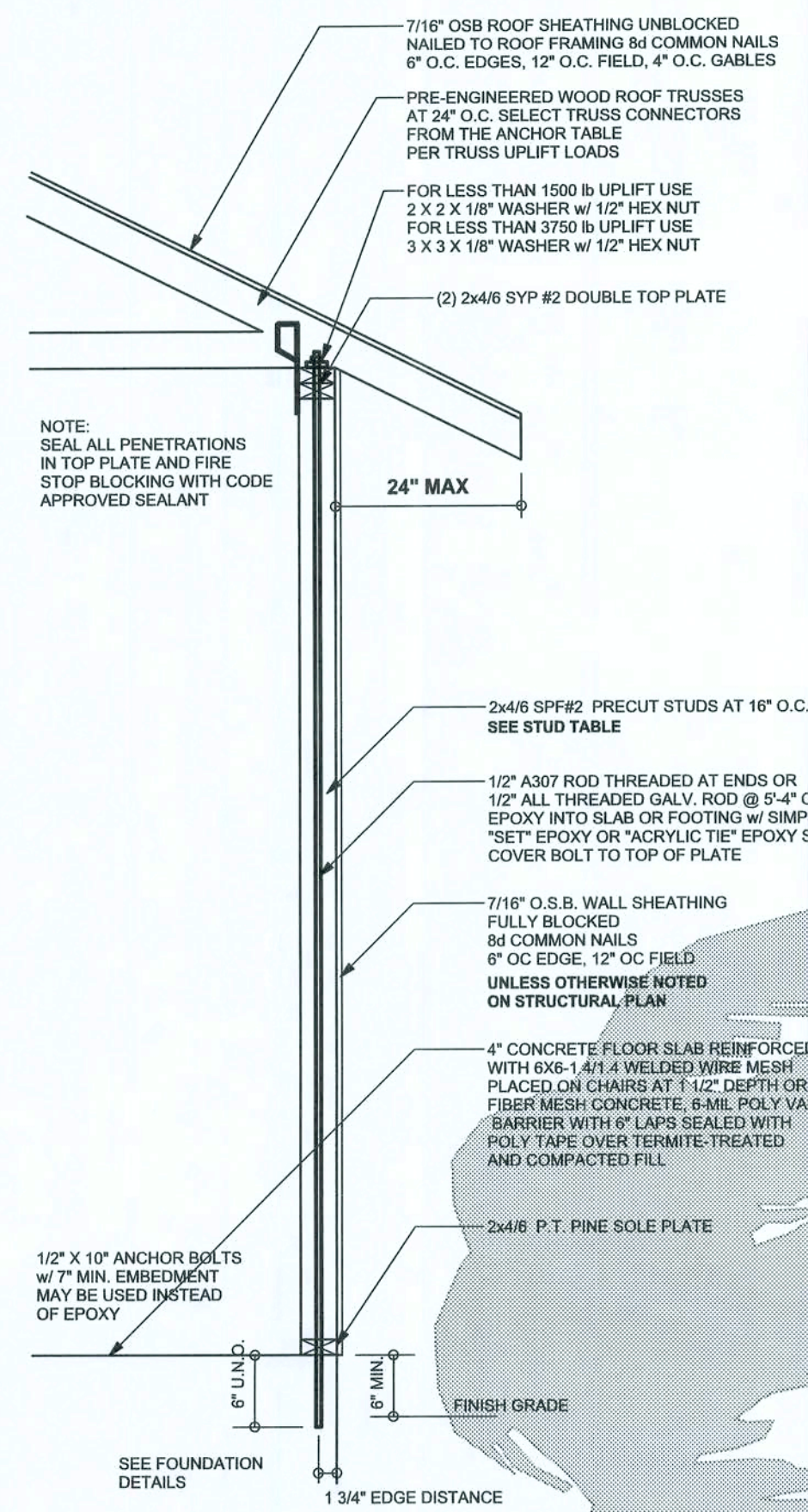
FINALES DATE:

DRAWING NUMBER

DIM. PLAN FOR FEILD

FEILD PLAN



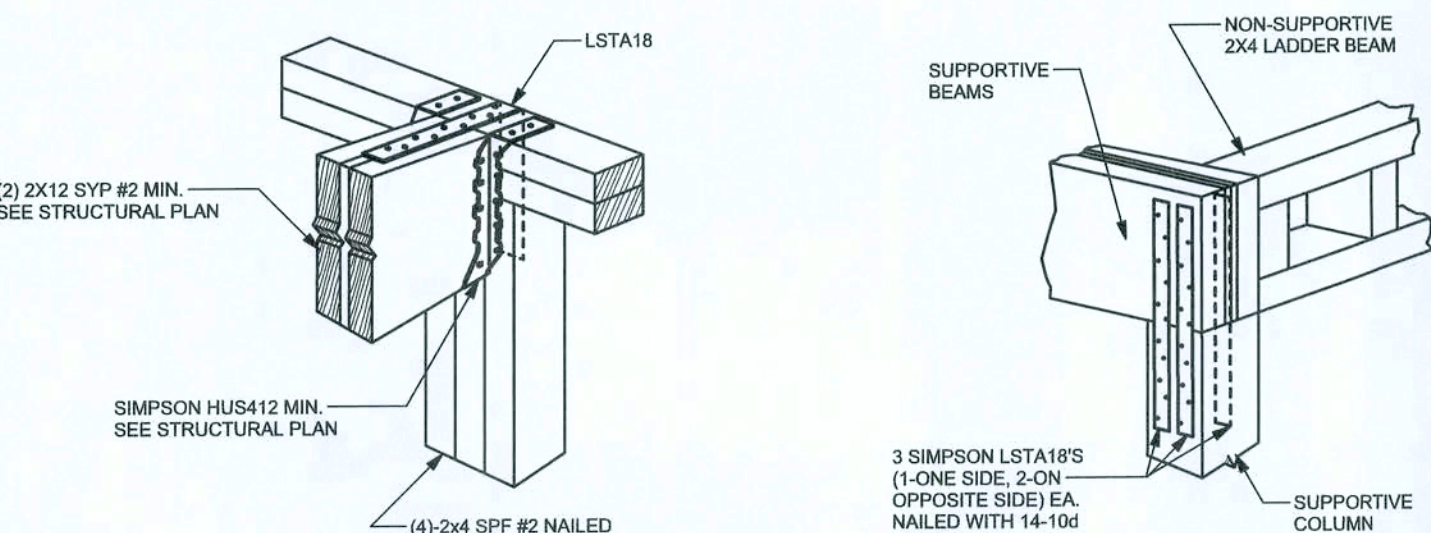


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

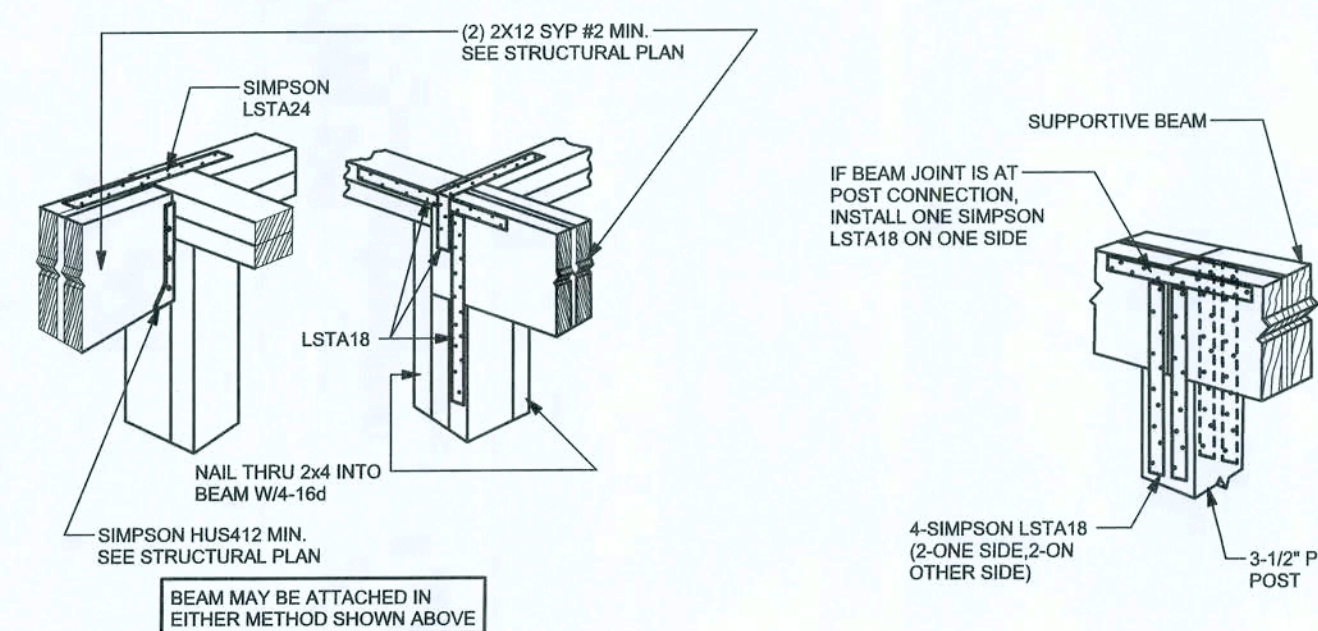
(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.



BEAM MID-WALL CONNECTION DETAIL

SCALE: N.T.S.



BEAM CORNER CONNECTION DETAIL

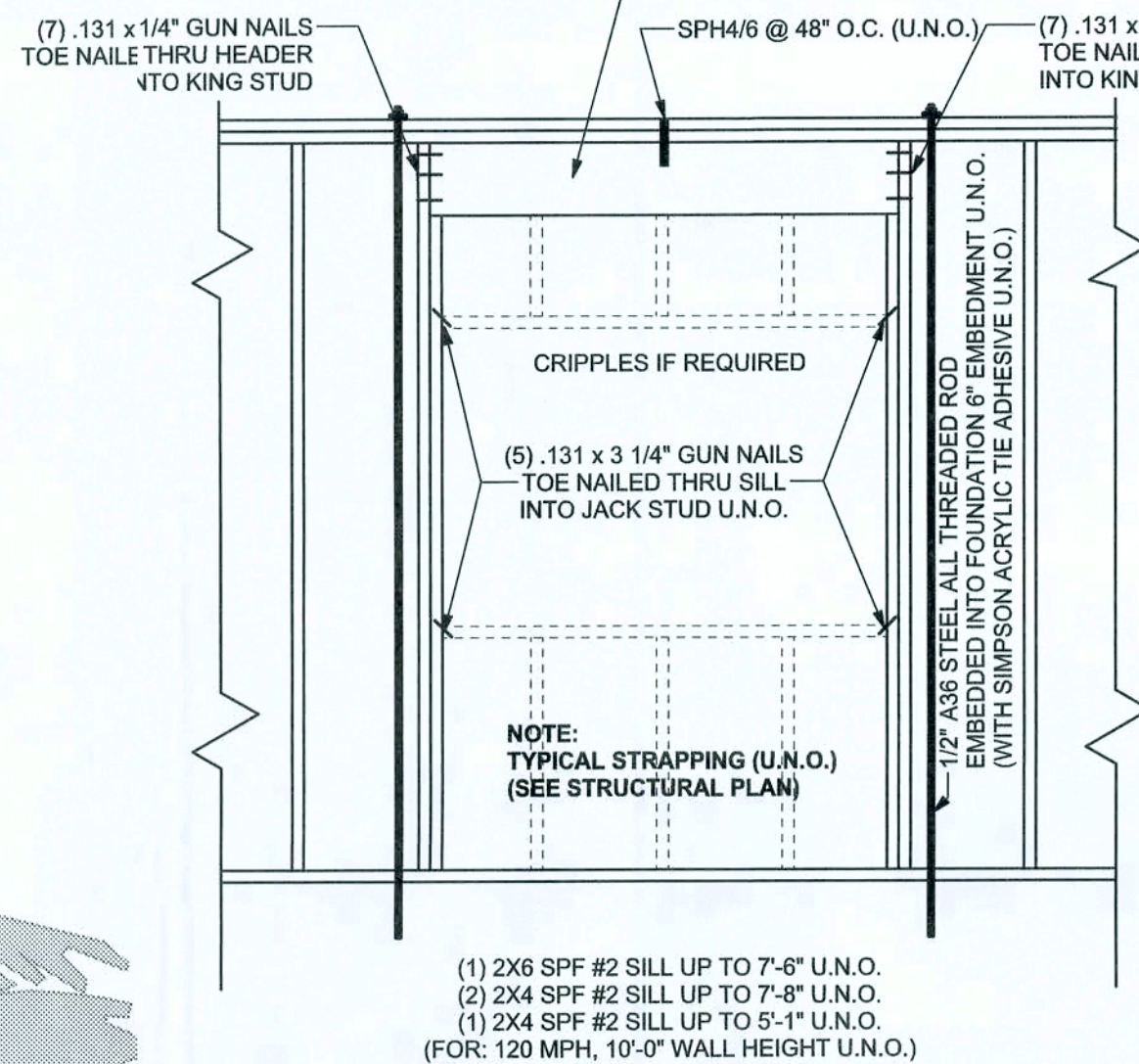
SCALE: N.T.S.

SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.

NOTE:
IF TRUSS 1 WALL STRAPS ARE NAILED TO THE HDER THE SPH4/6 @ 48" O.C. ARE NOT REQUIRED

(7) .131 x 1/4" GUN NAILS TOE NAIL THRU HEADER INTO KING STUD



TYPICAL 1 STORY HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

FOR LESS THAN 1500 LB UPLIFT USE 2 X 2 X 1/8" WASHER
FOR LESS THAN 3750 LB UPLIFT USE 3 X 3 X 1/8" WASHER

SPH4/6 @ 48" O.C. (U.N.O.)

NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 3" O.C. FOR UPLIFT

(7) .131 x 1/4" GUN NAILS TOE NAIL THRU HEADER INTO KING STUD

(5) .131 x 3/4" GUN NAILS TOE NAIL THRU SILL INTO JACK STUD U.N.O.

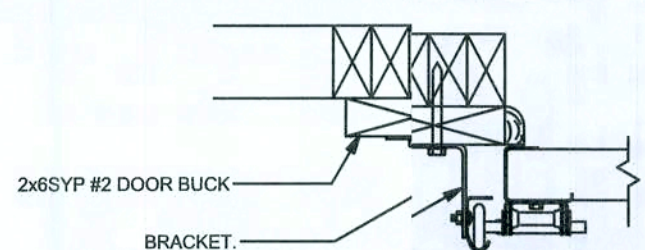
NOTE:
TYPICAL STRAPPING (U.N.O.) (SEE STRUCTURAL PLAN)

(1) 2x6 SPF #2 SILL UP TO 7'-6" U.N.O.
(2) 2x4 SPF #2 SILL UP TO 7'-8" U.N.O.
(1) 2x4 SPF #2 SILL UP TO 5'-1" U.N.O.
(FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8" x 4" LAG SCREWS W/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3/4" GN PER TABLE BELOW:

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



GARAGE DOOR BUCK INSTALLATION DETAIL

SCALE: N.T.S.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	HSA	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 585	H2.5A	5-8d	5-8d	
< 950	< 825	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LSG2	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3340	MG1		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6465	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*					TO STUDS
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HQ2A	2-5/8" BOLT		5/8" AB
< 4175	< 3965	JTT16	15-16d		5/8" AB
< 1400	< 1400	HPANQ2	16-16d		
< 3335	< 3335	HPANQ22	16-16d		
< 2220	< 2220	ABU44	12-16d		1/2" AB
< 2390	< 2390	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2x6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2x6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F'_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, FB = 8KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 308. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, $F_y = 60$ KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, $F_b = 2484$, $E = 1800$ ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIGNED CALCULATIONS.

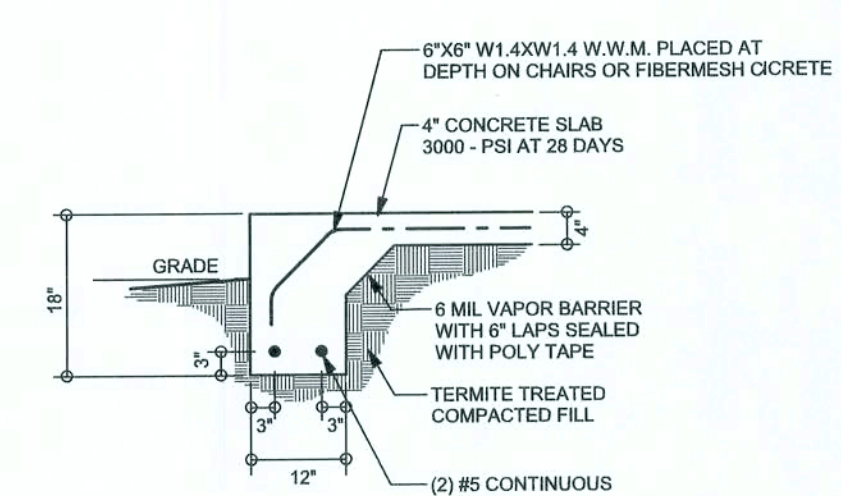
ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY. 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE. NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

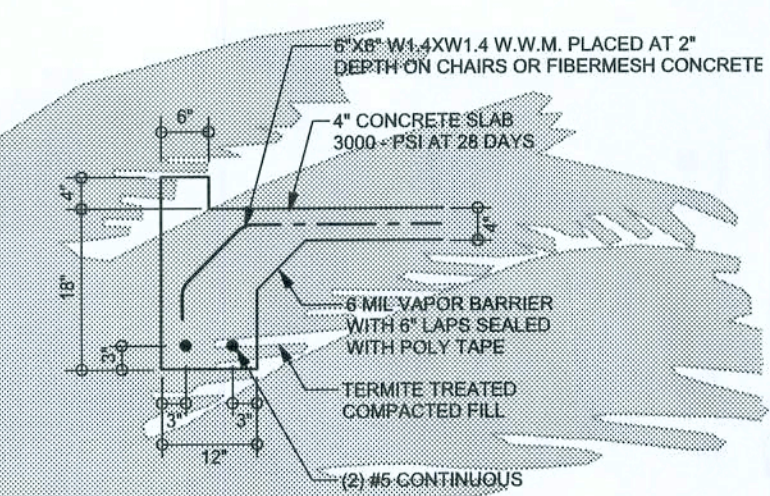
ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 3/8"; WITH 3/8" BOLTS TO BE 3" x 3" x 3/8"; WITH 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 5/8" BOLTS TO BE 3" x 3" x 3/8"; WITH 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 7/8" BOLTS TO BE 3" x 3" x 3/8"; WITH 1" BOLTS TO BE 3" x 3" x 3/8"; WITH 1 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 1 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 1 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 2" BOLTS TO BE 3" x 3" x 3/8"; WITH 2 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 2 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 2 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 3" BOLTS TO BE 3" x 3" x 3/8"; WITH 3 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 3 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 3 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 4" BOLTS TO BE 3" x 3" x 3/8"; WITH 4 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 4 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 4 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 5" BOLTS TO BE 3" x 3" x 3/8"; WITH 5 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 5 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 5 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 6" BOLTS TO BE 3" x 3" x 3/8"; WITH 6 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 6 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 6 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 7" BOLTS TO BE 3" x 3" x 3/8"; WITH 7 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 7 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 7 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 8" BOLTS TO BE 3" x 3" x 3/8"; WITH 8 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 8 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 8 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 9" BOLTS TO BE 3" x 3" x 3/8"; WITH 9 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 9 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 9 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 10" BOLTS TO BE 3" x 3" x 3/8"; WITH 10 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 10 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 10 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 11" BOLTS TO BE 3" x 3" x 3/8"; WITH 11 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 11 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 11 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 12" BOLTS TO BE 3" x 3" x 3/8"; WITH 12 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 12 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 12 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 13" BOLTS TO BE 3" x 3" x 3/8"; WITH 13 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 13 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 13 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 14" BOLTS TO BE 3" x 3" x 3/8"; WITH 14 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 14 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 14 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 15" BOLTS TO BE 3" x 3" x 3/8"; WITH 15 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 15 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 15 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 16" BOLTS TO BE 3" x 3" x 3/8"; WITH 16 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 16 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 16 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 17" BOLTS TO BE 3" x 3" x 3/8"; WITH 17 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 17 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 17 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 18" BOLTS TO BE 3" x 3" x 3/8"; WITH 18 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 18 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 18 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 19" BOLTS TO BE 3" x 3" x 3/8"; WITH 19 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 19 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 19 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 20" BOLTS TO BE 3" x 3" x 3/8"; WITH 20 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 20 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 20 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 21" BOLTS TO BE 3" x 3" x 3/8"; WITH 21 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 21 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 21 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 22" BOLTS TO BE 3" x 3" x 3/8"; WITH 22 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 22 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 22 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 23" BOLTS TO BE 3" x 3" x 3/8"; WITH 23 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 23 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 23 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 24" BOLTS TO BE 3" x 3" x 3/8"; WITH 24 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 24 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 24 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 25" BOLTS TO BE 3" x 3" x 3/8"; WITH 25 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 25 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 25 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 26" BOLTS TO BE 3" x 3" x 3/8"; WITH 26 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 26 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 26 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 27" BOLTS TO BE 3" x 3" x 3/8"; WITH 27 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 27 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 27 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 28" BOLTS TO BE 3" x 3" x 3/8"; WITH 28 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 28 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 28 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 29" BOLTS TO BE 3" x 3" x 3/8"; WITH 29 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 29 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 29 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 30" BOLTS TO BE 3" x 3" x 3/8"; WITH 30 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 30 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 30 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 31" BOLTS TO BE 3" x 3" x 3/8"; WITH 31 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 31 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 31 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 32" BOLTS TO BE 3" x 3" x 3/8"; WITH 32 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 32 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 32 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 33" BOLTS TO BE 3" x 3" x 3/8"; WITH 33 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 33 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 33 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 34" BOLTS TO BE 3" x 3" x 3/8"; WITH 34 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 34 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 34 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 35" BOLTS TO BE 3" x 3" x 3/8"; WITH 35 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 35 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 35 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 36" BOLTS TO BE 3" x 3" x 3/8"; WITH 36 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 36 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 36 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 37" BOLTS TO BE 3" x 3" x 3/8"; WITH 37 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 37 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 37 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 38" BOLTS TO BE 3" x 3" x 3/8"; WITH 38 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 38 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 38 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 39" BOLTS TO BE 3" x 3" x 3/8"; WITH 39 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 39 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 39 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 40" BOLTS TO BE 3" x 3" x 3/8"; WITH 40 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 40 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 40 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 41" BOLTS TO BE 3" x 3" x 3/8"; WITH 41 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 41 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 41 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 42" BOLTS TO BE 3" x 3" x 3/8"; WITH 42 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 42 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 42 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 43" BOLTS TO BE 3" x 3" x 3/8"; WITH 43 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 43 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 43 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 44" BOLTS TO BE 3" x 3" x 3/8"; WITH 44 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 44 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 44 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 45" BOLTS TO BE 3" x 3" x 3/8"; WITH 45 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 45 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 45 3/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 46" BOLTS TO BE 3" x 3" x 3/8"; WITH 46 1/4" BOLTS TO BE 3" x 3" x 3/8"; WITH 46 1/2" BOLTS TO BE 3" x 3" x 3/8"; WITH 46 3/4" BOLTS TO BE 3" x 3" x 3/8";

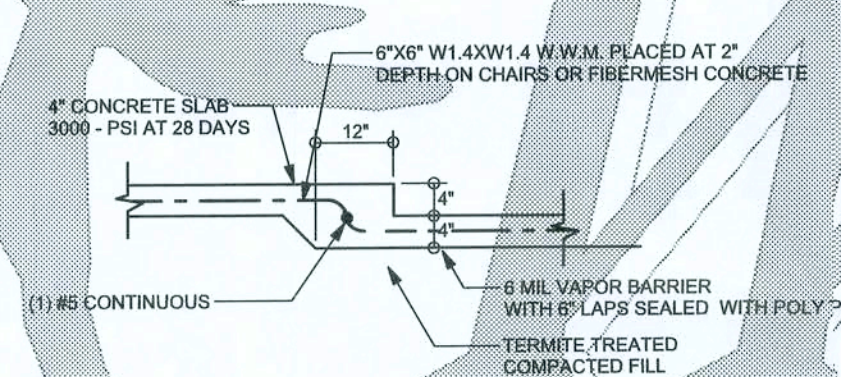
REVISIONS	



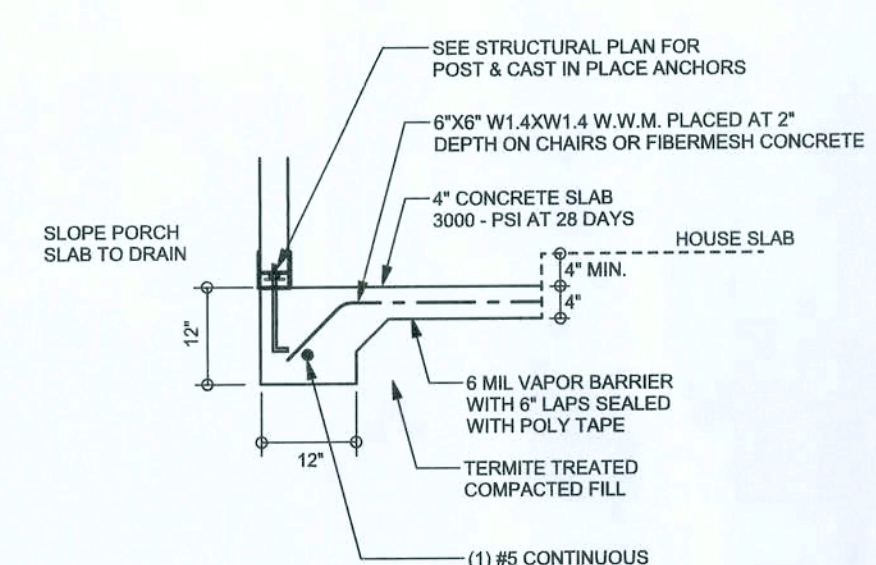
F1 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



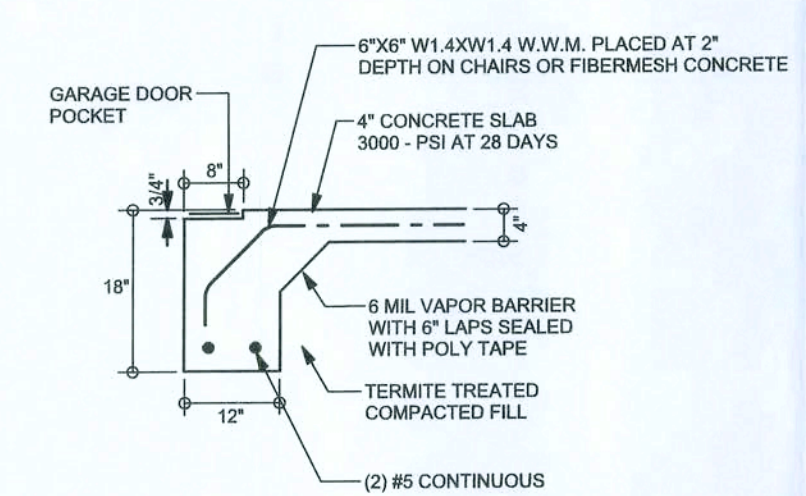
F8 GARAGE CURB FOOTING
SCALE: 1/2" = 1'-0"



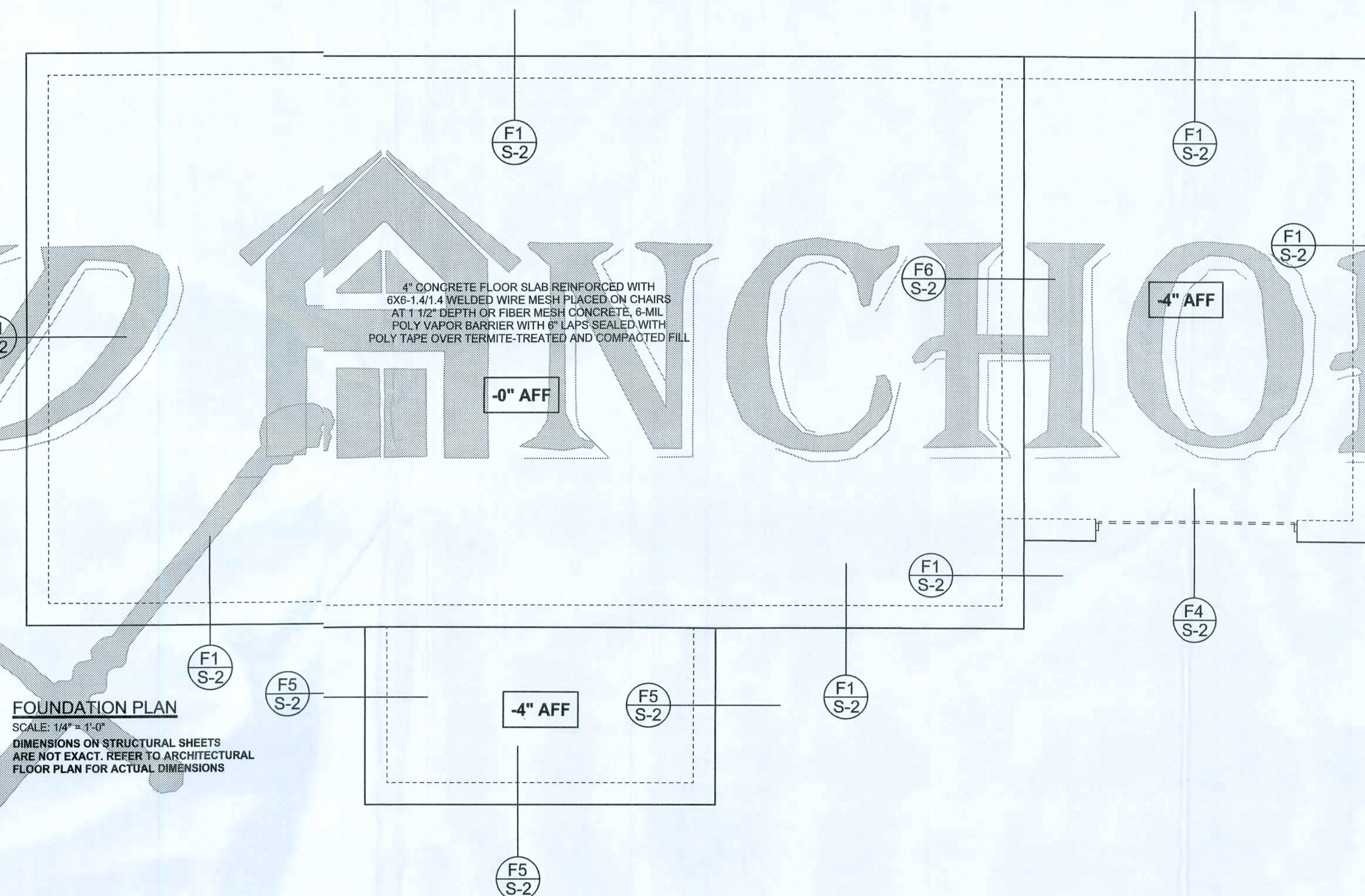
F6 TYPICAL NON - BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING
SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



WINDLOAD ENGINEER: Mark Disoway
PE No.53015, FCB 04, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions are scaled dimensions. Refer all questions to Mark Disoway, P.E. or resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R601.2.1, Florida building code residential 2004 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53015
[Signature]
SEAL

Richard Keen
SpecHouse
Lot 9
County Creek
Estates S/D

ADDRESS:
Lot 9 Country Creek Estates S/D
(Columbia County)

Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
January 22, 2008

DRAWN BY: CHECKED BY:

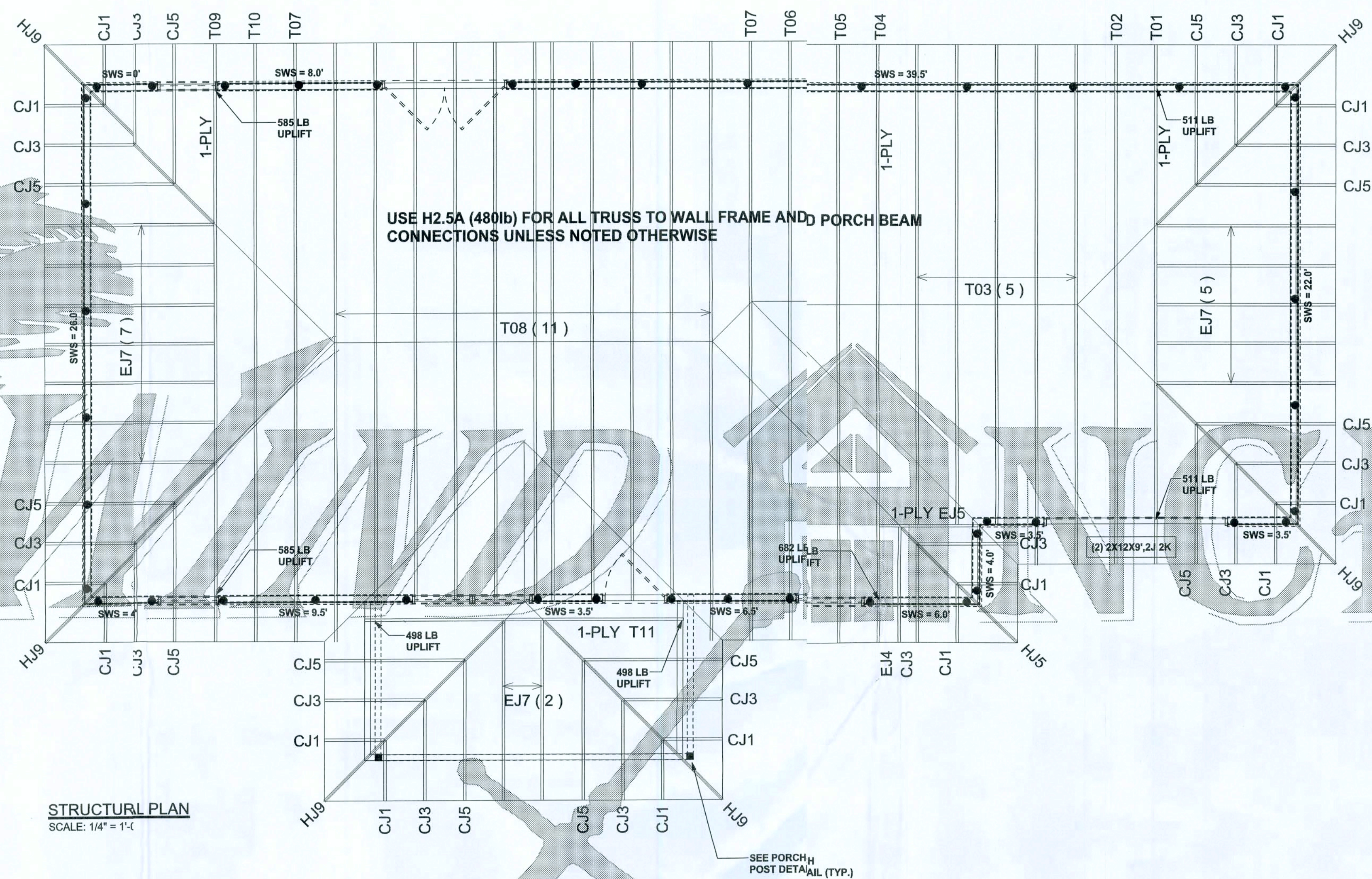
FINALS DATE:
22 / Jan / 08

JOB NUMBER:
801221

DRAWING NUMBER
S-2
OF 3 SHEETS

REVISIONS

SOFTPLAN
 ARCHITECTURAL ENGINEERING



STRUCTURAL PLAN
 SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
 SN-2 ALL LAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
 SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
 SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FINISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

THREADED ROD LEGEND

- INDICATES LOCATION OF:
 1ST FLOOR 1/2" A307 ALL THREADED ROD
 INDICATES LOCATION OF:
 2ND FLOOR 1/2" A307 ALL THREADED ROD

HEADER LEGEND

- (2) 2X12X9', 1J 1K
 NUMBER OF KING STUDS (FULL LENGTH)
 NUMBER OF JACK STUDS (UNDER HEADER)
 SPAN OF HEADER
 SIZE OF HEADER MATERIAL
 NUMBER OF PLIES IN HEADER

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALL
IBW	2ND FLOOR INTERIOR BEARING WALL

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	36.5'	52.0'
LONGITUDINAL	28.5'	88.0'

ALTERNATE WALL TIE CONNECTION WHERE
 THREADED ROD CANNOT BE PLACED IN WALL.
 SCALE: 1/2" = 1'-0"

WINDLOAD ENGINEER: Mark Discoway,
 PE No. 33915, P.O. Box 868 Lake City, FL
 32056, 386-754-5419

DIMENSIONS:
 Stated dimensions supersede scaled
 dimensions. Refer all questions to
 Mark Discoway, P.E. for resolution.
 Do not proceed without clarification.

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 these instruments of service. This document is
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 form or manner without the express written
 permission and consent of Mark Discoway.

CERTIFICATION: I hereby certify that I have
 examined this plan, and that the applicable
 portions of the plan, relating to wind engineering
 comply with section F301, L-1, Florida building
 code residential 2004, to the best of my
 knowledge.

LIMITATION: This design is valid for one
 building, at specified location.

MARK DISCOWAY
 P.E. 3815

22 JAN 08

58L

Richard Keen

Spec House
 Lot 9
 Country Creek
 Estates S/D

ADDRESS:
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 (Columbia County)

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PRINTED DATE:

January 2, 2008

DRAWN BY: CHECKED BY:

FINALS DATE:
 22 / Jan / 08

JOB NUMBER:
 801221

DRAWING NUMBER

S-3

OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED
 ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
 FURNISHED BY BUILDER. BUILDERS FIRST SOURCE
 JOB #L266128