

COA #0 278
 Florida Certificate of Product Approval #FL1999
 12/01/2025

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This item has been digitally signed by Douglas Fleming on the date adjacent to the seal.

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Site Information:	Page 1:
Customer: Seminole Trusses, Inc.	Job Number: B62186a
Job Description: JACKSON RESIDENCE	
Address: NW LONA LOOP, Lake City, FL 32055	

Job Engineering Criteria:	
Design Code: FBC 8th Ed. 2023 Res.	IntelliVIEW Version: 24.02.00C JRef #: 1YJ8570001
Wind Standard: ASCE 7-22 Wind Speed (mph): 120	Design Loading (psf): 37
Building Type: Enclosed	

This package contains general notes pages, 20 truss drawing(s) and 4 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	335.25.0729.34720	A1 26'7" Common	2	335.25.0729.29420	A2-G 26'7" Gable
3	335.25.0729.27703	A3 26'7" Common	4	335.25.0729.23303	B1 23'11" Common
5	335.25.0729.19427	B1a 23'11" Common	6	335.25.0729.14777	B2-G 23'11" Gable
7	335.25.0729.13017	C1 23'11" Common	8	335.25.0729.11120	C1a 23'11" Common
9	335.25.0729.09320	C1b 23'11" Common	10	335.25.0729.07370	C2-G 23'11" Gable
11	335.25.0729.05457	D1 12'7"4 Common	12	335.25.0729.04013	D2-G 12'7"4 Gable
13	335.25.0729.02453	E1 7'11"8 Mono	14	335.25.0729.01230	E2-G 7'11"8 Gable
15	335.25.0729.00253	G1 5'11"8 Mono	16	335.25.0728.59127	G2 5'11"8 Mono
17	335.25.0728.57507	PB1 7'0"12 Common	18	335.25.0728.54083	PB2-G 7'0"12 Gable
19	335.25.0728.41073	V1 12'11"7 Valley	20	335.25.0728.39820	V2 6'1"2 Valley
21	REPCHRD1014		22	VAL180220723	
23	VALTN220723		24	GBLDIAG220923	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:

The bearing area factor, C_b , is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

General Notes (continued)

Coated Lumber:

Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C -TE = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-BF = Boraflame Fire Retardant Treated lumber

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-ON = OnWood Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

General Notes (continued)

Key to Terms (continued):

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

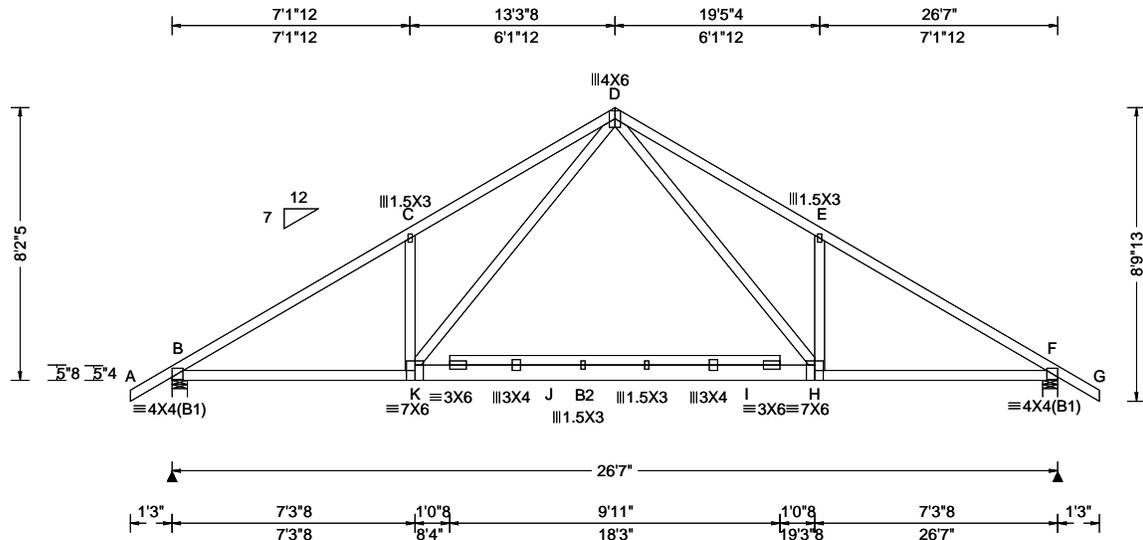
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com



Loading Criteria (psf) TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.317 J 993 360 VERT(CL): 0.380 J 829 240 HORZ(LL): 0.059 F - - HORZ(TL): 0.069 F - - Creep Factor: 2.0 Max TC CSI: 0.453 Max BC CSI: 0.571 Max Web CSI: 0.376 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1329 /- /- /523 /138 /115 F 1329 /- /- /523 /138 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.6 (Truss) F Brg Wid = 5.5 Min Req = 1.6 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 261 -2025 D - E 253 -1989 C - D 253 -1989 E - F 261 -2025
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Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;

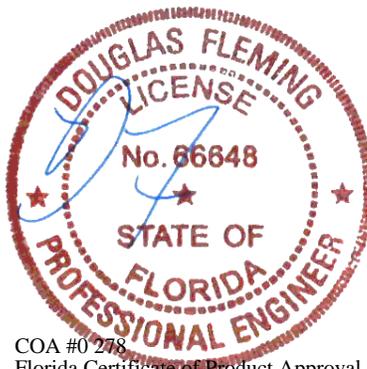
Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From 56 plf at -1.25 to 56 plf at 27.83
BC: From 5 plf at -1.25 to 5 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 7.29
BC: From 60 plf at 7.29 to 60 plf at 19.29
BC: From 20 plf at 19.29 to 20 plf at 26.58
BC: From 5 plf at 26.58 to 5 plf at 27.83

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

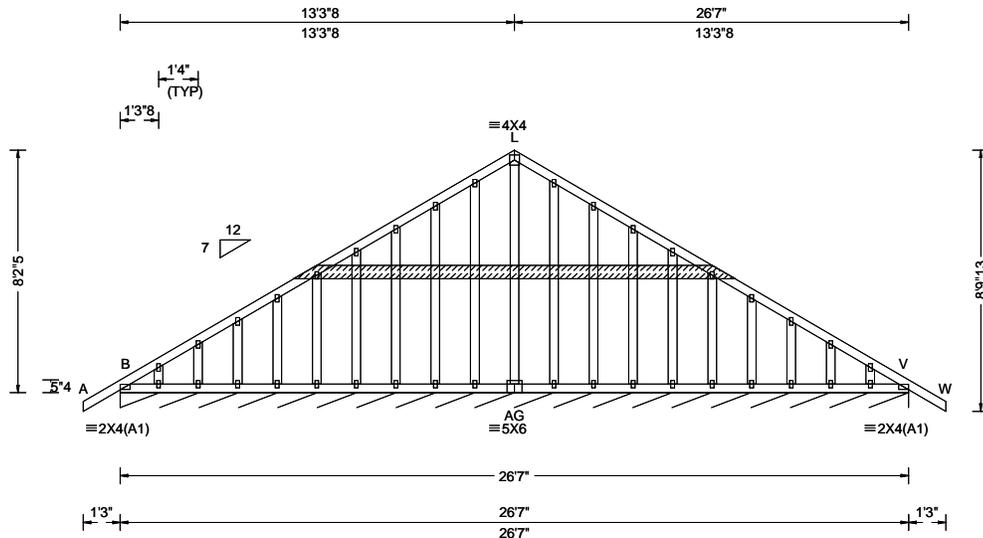


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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 75804 FROM: RJL	GABL Ply: 1 Qty: 2	Job Number: B62186a JACKSON RESIDENCE Truss Label: A2-G 26'7" Gable	Cust: R 857 JRef: 1YJ8570001 T2 DrwNo: 335.25.0729.29420 SSB / DF 12/01/2025
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Loading Criteria (psf) TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 K 999 360 VERT(CL): 0.002 K 999 240 HORZ(LL): 0.000 V - - HORZ(TL): 0.002 O - - Creep Factor: 2.0 Max TC CSI: 0.094 Max BC CSI: 0.020 Max Web CSI: 0.266 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL V* 101 /- /- /40 /2 /5 Wind reactions based on MWFRS V Brg Wid = 318 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

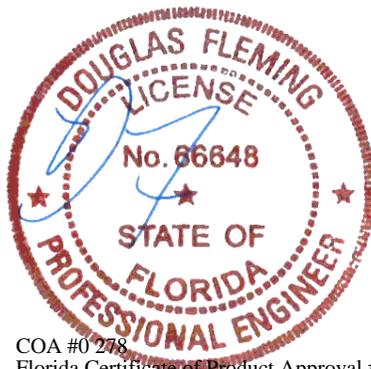
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

See DWGS GBLDIAG220325 and GBLLEDG220325 for gable ledger and diagonal reinforcement details.

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

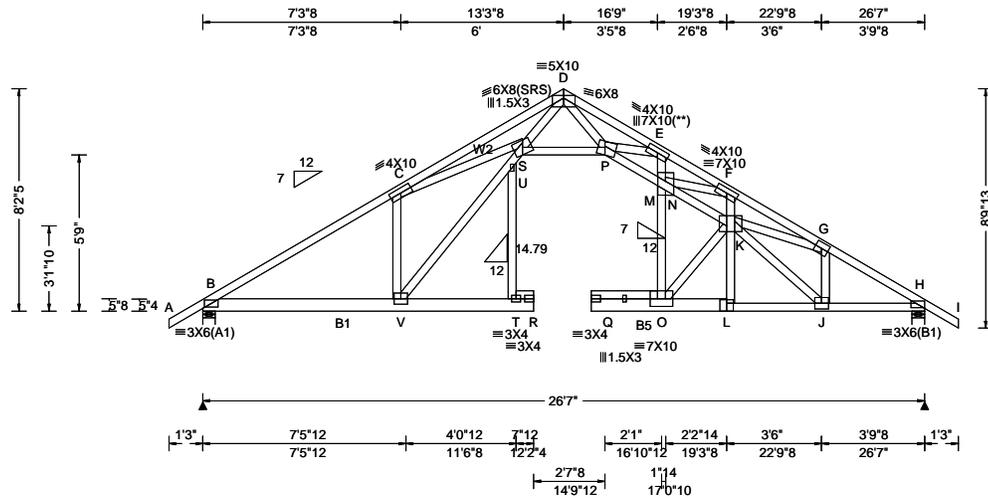


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2 Complete Trusses Required



Loading Criteria (psf) TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.592 P 532 360 VERT(CL): 0.958 P 329 240 HORZ(LL): 0.513 H - - HORZ(TL): 0.830 H - - Creep Factor: 2.0 Max TC CSI: 0.532 Max BC CSI: 0.745 Max Web CSI: 0.970 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1329 - / - / - /555 /138 /115 H 1329 - / - / - /563 /138 - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Truss) H Brg Wid = 5.5 Min Req = 1.5 (Truss) Bearings B & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 132 -1018 E - F 535 -3880 C - D 594 -4224 F - G 193 -1451 D - E 571 -4073 G - H 115 -1011 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - V 838 -105 O - L 613 -77 V - U 1327 -166 M - K 1440 -190 U - S 1375 -177 L - J 619 -78 S - P 2107 -298 J - H 839 -93 P - M 3861 -532 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - V 142 -915 O - K 124 -991 C - S 2922 -422 M - F 2092 -295 S - D 2545 -350 F - K 172 -1189 D - P 2399 -331 K - G 412 -73 O - N 932 -123
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Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B1,B5 2x6 SP #1;
Webs: 2x4 SP #3; W2 2x4 SP #1;

Nailnote

Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

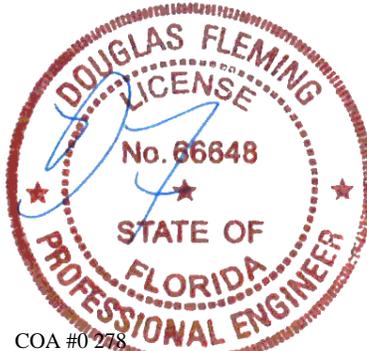
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BC: From 60 plf at 7.29 to 60 plf at 19.29
BC: From 20 plf at 19.29 to 20 plf at 26.58
BC: From 5 plf at 26.58 to 5 plf at 27.83

Plating Notes

All plates are 5X6 except as noted.
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

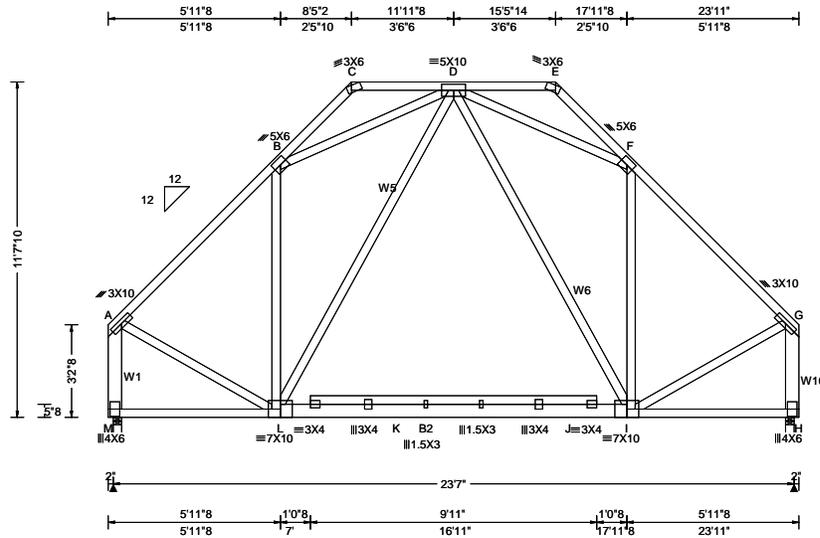
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 16.52 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.141 K 999 360 VERT(CL): 0.234 K 999 240 HORZ(LL): 0.014 H - - HORZ(TL): 0.024 H - - Creep Factor: 2.0 Max TC CSI: 0.322 Max BC CSI: 0.493 Max Web CSI: 0.535 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 1194 /- /- /475 /116 /229 H 1194 /- /- /475 /116 /- Wind reactions based on MWFRS M Brg Wid = 3.5 Min Req = 1.5 (Truss) H Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings M & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 153 - 1089 F - G 152 - 1089 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. L - K 607 -96 J - I 607 -96 K - L 1650 - 1110 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. A - M 151 - 1159 D - F 158 - 618 A - L 716 -93 I - G 716 -93 B - D 159 - 618 G - H 151 - 1159
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Lumber

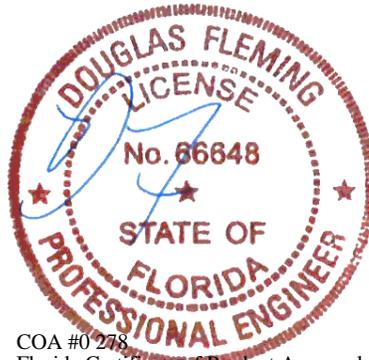
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3; W1,W10 2x6 SP #1; W5,
W6 2x4 SP #1;

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 60 plf at 0.00 to 60 plf at 23.92
BC: From 20 plf at 0.00 to 20 plf at 5.96
BC: From 60 plf at 5.96 to 60 plf at 17.96
BC: From 20 plf at 17.96 to 20 plf at 23.92

Wind

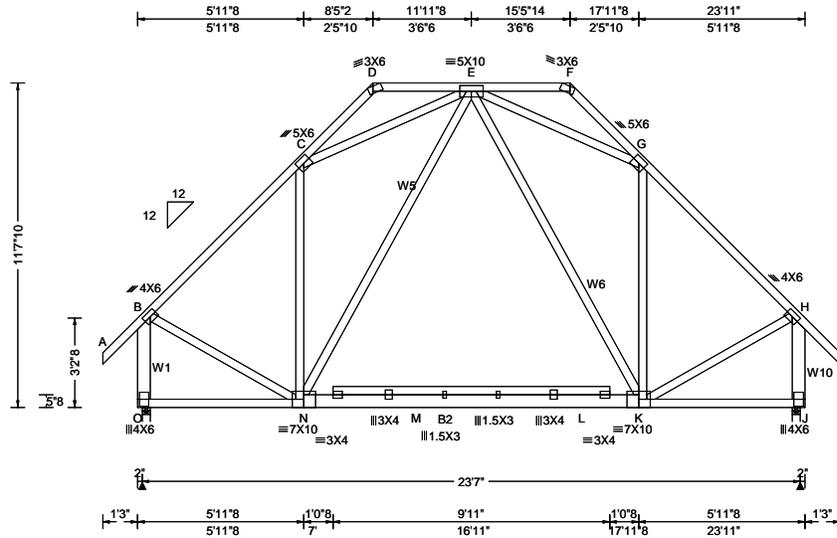
Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/360.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.



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Loc	Gravity			Non-Gravity																																			
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O	1276	-	-	/487	/125	/247																																	
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Chords	Tens.Comp.	Chords	Tens. Comp.																																				
B - C	165 - 1083	G - H	163 - 1083																																				

Lumber

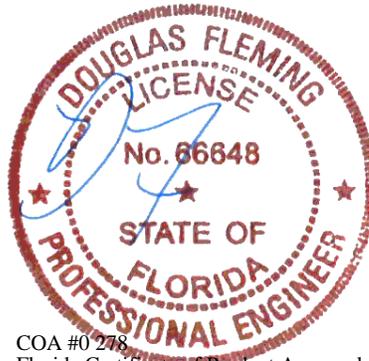
Top chord: 2x4 SP #1;
 Bot chord: 2x4 SP #1; B2 2x6 SP #1;
 Webs: 2x4 SP #3; W1,W10 2x6 SP #1; W5,
 W6 2x4 SP #1;

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 60 plf at -1.25 to 60 plf at 25.17
 BC: From 6 plf at -1.25 to 6 plf at 0.00
 BC: From 20 plf at 0.00 to 20 plf at 5.96
 BC: From 60 plf at 5.96 to 60 plf at 17.96
 BC: From 20 plf at 17.96 to 20 plf at 23.92
 BC: From 6 plf at 23.92 to 6 plf at 25.17

Wind

Wind loads based on MWFRS with additional C&C member design.
 End verticals exposed to wind pressure. Deflection meets L/360.
 Left and right cantilevers are exposed to wind
 Wind loading based on both gable and hip roof types.

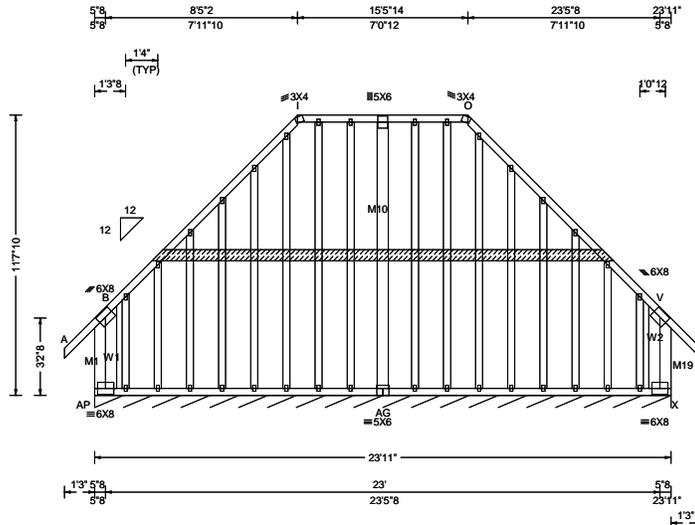


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SEQN: 75885 FROM: RJL	GABL Ply: 1 Qty: 1	Job Number: B62186a JACKSON RESIDENCE Truss Label: B2-G 23'11" Gable	Cust: R 857 JRRef: 1YfJ8570001 T8 DrwNo: 335.25.0729.14777 SSB / DF 12/01/2025
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Loading Criteria (psf) TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 15.89 ft TCCL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 6.06 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.002 I 999 360 VERT(CL): -0.016 B 999 240 HORZ(LL): -0.001 C - - HORZ(TL): 0.119 B - - Creep Factor: 2.0 Max TC CSI: 0.249 Max BC CSI: 0.348 Max Web CSI: 0.674 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th colspan="2">Gravity</th> <th colspan="4">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+ / R-</th> <th>/ Rh</th> <th>/ Rw</th> <th>/ U</th> <th>/ RL</th> </tr> </thead> <tbody> <tr> <td>X*</td> <td>146</td> <td>-</td> <td>-</td> <td>/49</td> <td>/4 /17</td> </tr> </tbody> </table> Wind reactions based on MWFRS X Brg Wid = 286 Min Req = - Bearing AP is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - I</td> <td>457 -347</td> <td>O - V</td> <td>457 -347</td> </tr> </tbody> </table> Maximum Web Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>AP- B</td> <td>347 -420</td> <td>V - X</td> <td>347 -420</td> </tr> </tbody> </table>	Gravity		Non-Gravity				Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL	X*	146	-	-	/49	/4 /17	Chords	Tens.Comp.	Chords	Tens. Comp.	B - I	457 -347	O - V	457 -347	Webs	Tens.Comp.	Webs	Tens. Comp.	AP- B	347 -420	V - X	347 -420
Gravity		Non-Gravity																																				
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Lumber
 Top chord: 2x4 SP #1;
 Bot chord: 2x4 SP #1;
 Webs: 2x4 SP #3; W1,W2,M1,M10,M19 2x6 SP #1;

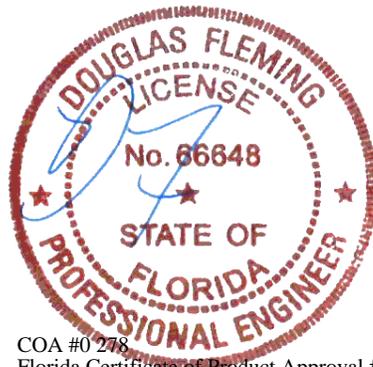
Bracing
 Fasten rated sheathing to one face of this frame.

Plating Notes
 All plates are 1.5X3 except as noted.

Additional Notes
 See DWGS GBLDIAG220325 and GBLLEDG220325 for gable ledger and diagonal reinforcement details.
 Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Loading
 Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

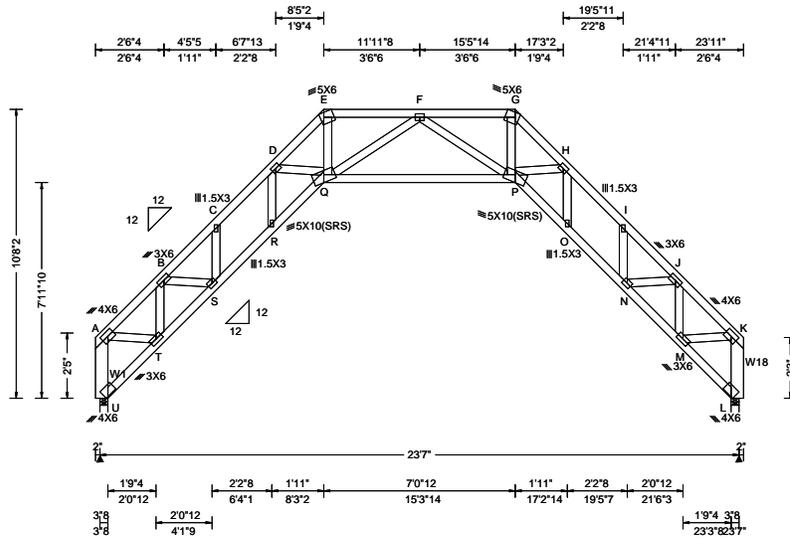
Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals exposed to wind pressure. Deflection meets L/360.
 Wind loading based on both gable and hip roof types.
 Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/394.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 16.52 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.246 F 999 360 VERT(CL): 0.508 F 543 240 HORZ(LL): 0.514 L - - HORZ(TL): 1.087 L - - Creep Factor: 2.0 Max TC CSI: 0.601 Max BC CSI: 0.868 Max Web CSI: 0.696 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL U 1011 - / - / - / 540 - / 204 L 1011 - / - / - / 538 - / - Wind reactions based on MWFRS U Brg Wid = 3.5 Min Req = 1.5 (Support) L Brg Wid = 3.5 Min Req = 1.5 (Support) Bearings U & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 154 -1043 F - G 230 -2196 B - C 260 -2240 G - H 299 -3020 C - D 322 -2546 H - I 290 -2546 D - E 319 -3020 I - J 224 -2240 E - F 242 -2196 J - K 102 -1043
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Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W1,W18 2x6 SP #1;

Plating Notes
All plates are 3X4 except as noted.

Wind
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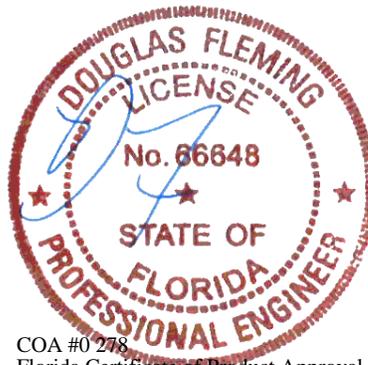
Additional Notes
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Shim all supports to solid bearing.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
U - T	173 -381	Q - P	2301 -196
T - S	1160 -234	P - O	2432 -167
S - R	2137 -218	O - N	2137 -180
R - Q	2432 -200	N - M	1160 -133

Maximum Web Forces Per Ply (lbs)

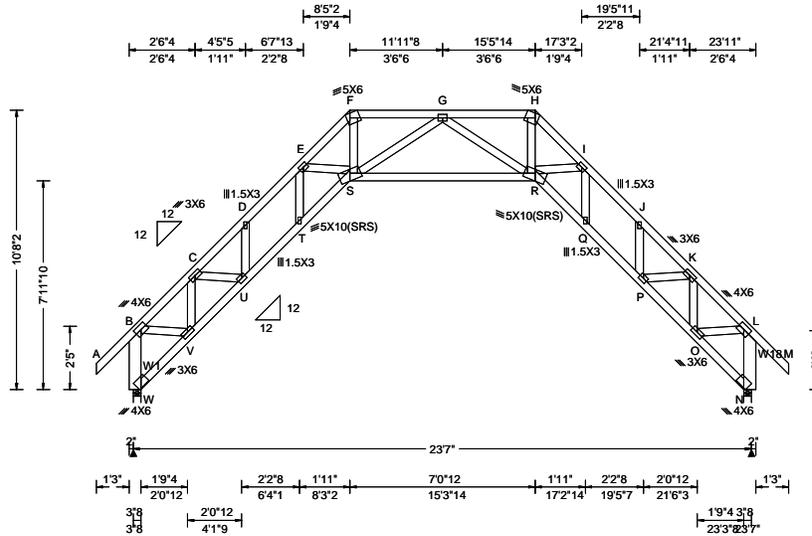
Webs	Tens.Comp.	Webs	Tens. Comp.
A - U	117 -794	G - P	1827 -134
A - T	865 -71	P - H	615 -59
T - B	104 -1028	O - H	14 -393
B - S	932 -24	N - I	69 -562
C - S	62 -562	N - J	932 -36
D - R	7 -393	J - M	116 -1028
D - Q	615 0	M - K	865 -80
E - Q	1827 -149	L - K	82 -794



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Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W1,W18 2x6 SP #1;

Plating Notes
All plates are 3X4 except as noted.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/360.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

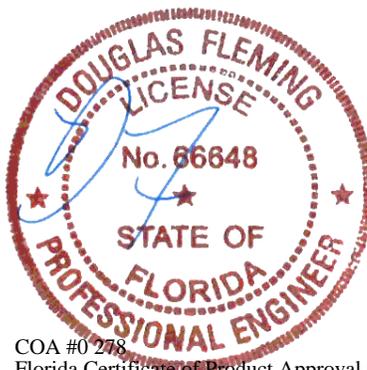
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Shim all supports to solid bearing.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
W - V	190 -404	S - R	2264 -71
V - U	1100 -237	R - Q	2379 -36
U - T	2084 -175	Q - P	2084 -65
T - S	2379 -131	P - O	1100 -59

Maximum Web Forces Per Ply (lbs)

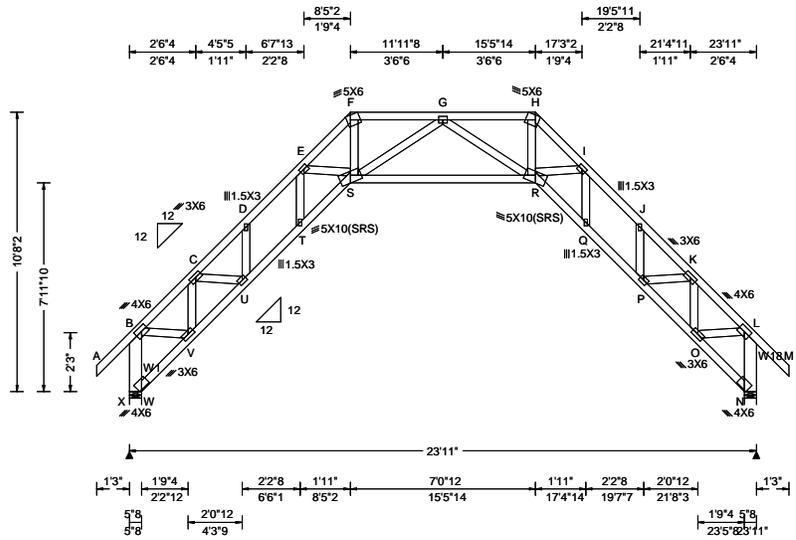
Webs	Tens.Comp.	Webs	Tens. Comp.
B - W	133 -856	H - R	1790 -48
B - V	847 -26	R - I	615 -62
V - C	46 -1009	Q - I	6 -393
C - U	936 0	P - J	45 -567
D - U	39 -567	P - K	936 0
E - T	0 -393	K - O	67 -1009
E - S	615 0	O - L	847 -43
F - S	1790 -66	N - L	101 -856



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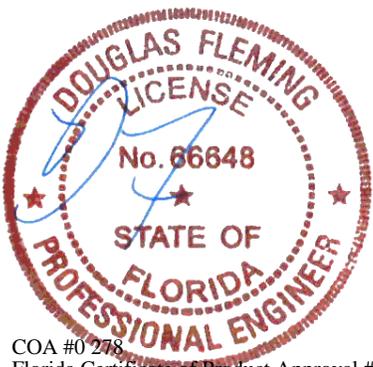
Loading Criteria (psf) TCCL: 20.00 TCDL: 7.00 BCDL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 15.89 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.248 G 999 360 VERT(CL): 0.501 G 550 240 HORZ(LL): 0.518 N - - HORZ(TL): 1.071 N - - Creep Factor: 2.0 Max TC CSI: 0.602 Max BC CSI: 0.858 Max Web CSI: 0.682 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL X 1093 -/- /- /554 -/- /223 N 1093 -/- /- /554 -/- /- Wind reactions based on MWFRS X Brg Wid = 5.5 Min Req = 1.5 (Support) N Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings X & N are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 148 -998 G - H 135 -2158 C - D 188 -2183 H - I 172 -2967 D - E 234 -2492 I - J 198 -2492 E - F 194 -2967 J - K 148 -2183 F - G 149 -2158 K - L 85 -998
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Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W1,W18 2x6 SP #1;

Plating Notes
All plates are 3X4 except as noted.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/360.
Wind loading based on both gable and hip roof types.

Additional Notes
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Shim all supports to solid bearing.



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Maximum Bot Chord Forces Per Ply (lbs)

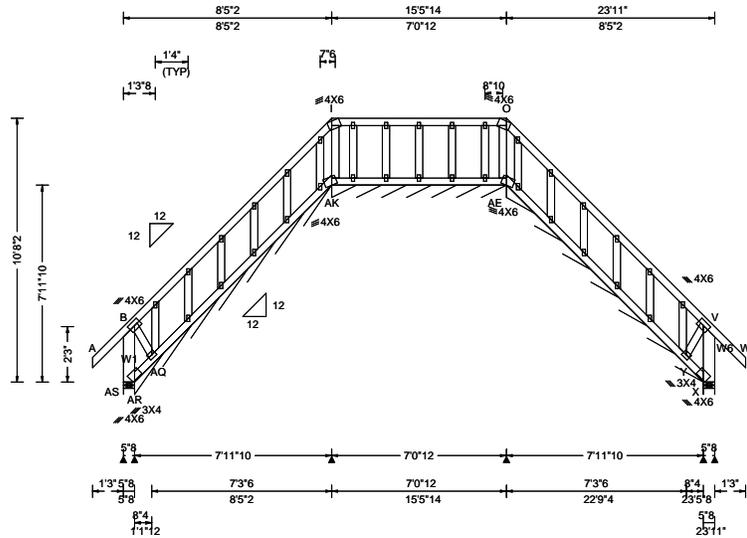
Chords	Tens.Comp.	Chords	Tens. Comp.
W - V	191 -406	S - R	2264 -77
V - U	1100 -239	R - Q	2379 -41
U - T	2084 -178	Q - P	2084 -70
T - S	2379 -134	P - O	1100 -63

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - W	134 -856	H - R	1790 -52
B - V	847 -27	R - I	615 -64
V - C	48 -1009	Q - I	7 -393
C - U	936 0	P - J	46 -567
D - U	40 -567	P - K	936 0
E - T	0 -393	K - O	68 -1009
E - S	615 0	O - L	847 -44
F - S	1790 -69	N - L	102 -856

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 15.89 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 10.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 B 999 360 VERT(CL): 0.001 B 999 240 HORZ(LL): -0.001 X - - HORZ(TL): 0.011 T - - Creep Factor: 2.0 Max TC CSI: 0.106 Max BC CSI: 0.062 Max Web CSI: 0.107 VIEW Ver: 24.02.00C.1213.15	Gravity Loc R+ / R- / Rh / Rw / U / RL AS 297 /- /- /734 /606 /307 AR*76 /- /- /139 /74 /- AK*89 /- /- /57 /9 /- AE*71 /- /- /86 /39 /- X 297 /- /- /149 /23 /- Non-Gravity Wind reactions based on MWFRS AS Brg Wid = 5.5 Min Req = 1.5 (Support) AR Brg Wid = 95.6 Min Req = - AK Brg Wid = 84.8 Min Req = - AE Brg Wid = 95.6 Min Req = - X Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings AS, AR, AK, AE, & X are a rigid surface. Members not listed have forces less than 375# Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. AR-AQ 500 -543

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W1,W6 2x6 SP #1;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/360.

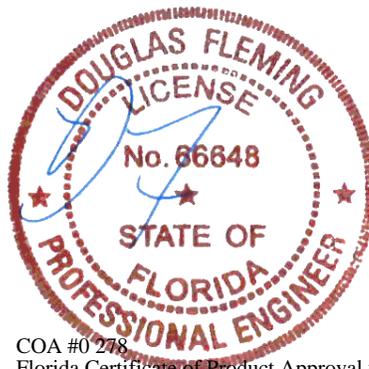
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

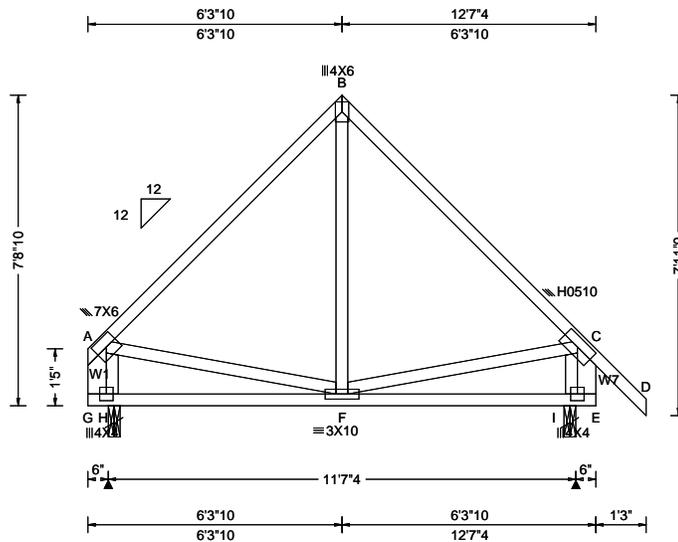
Shim all supports to solid bearing.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.020 B 999 360 VERT(CL): 0.039 B 999 240 HORZ(LL): 0.005 B - - HORZ(TL): 0.009 B - - Creep Factor: 2.0 Max TC CSI: 0.374 Max BC CSI: 0.515 Max Web CSI: 0.094 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 500 /- /0 /271 /- /127 I 595 /- /0 /287 /- /0 Wind reactions based on MWFRS H Brg Wid = 3.5 Min Req = 1.5 (Truss) I Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings H & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 104 -396 B - C 109 -391
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Lumber

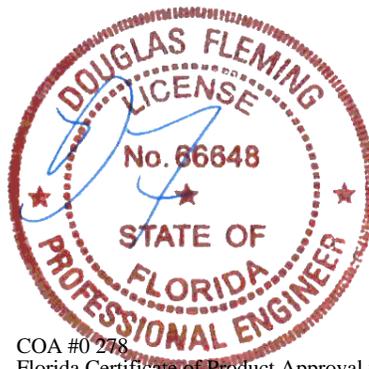
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W1,W7 2x6 SP #1;

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

Maximum Web Forces Per Ply (lbs)

Webs		Tens.Comp.	
A - G	105	-394	C - E 154 -481

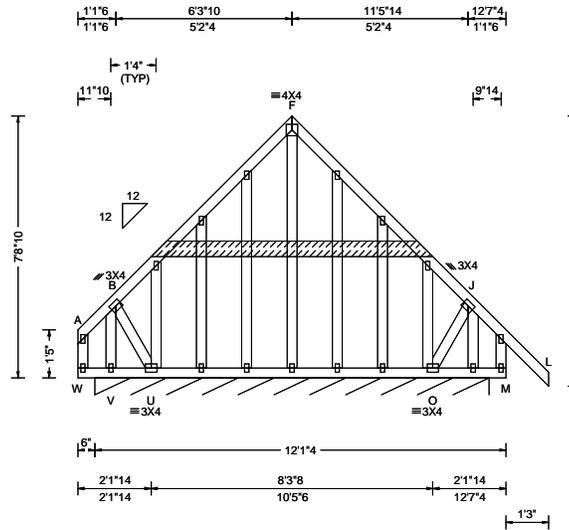


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SEQN: 75813 FROM: RJL	GABL Ply: 1 Qty: 1	Job Number: B62186a JACKSON RESIDENCE Truss Label: D2-G 127"4 Gable	Cust: R 857 JRef: 1YJ8570001 T3 DrwNo: 335.25.0729.04013 SSB / DF 12/01/2025
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Loading Criteria (psf) TCCL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 F 999 360 VERT(CL): 0.002 F 999 240 HORZ(LL): 0.001 I - - HORZ(TL): 0.005 G - - Creep Factor: 2.0 Max TC CSI: 0.159 Max BC CSI: 0.155 Max Web CSI: 0.337 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th colspan="2">Gravity</th> <th colspan="4">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+ / R-</th> <th>/ Rh</th> <th>/ Rw</th> <th>/ U</th> <th>/ RL</th> </tr> </thead> <tbody> <tr> <td>V*</td> <td>135</td> <td>-</td> <td>-</td> <td>/56</td> <td>/8 /15</td> </tr> </tbody> </table> Wind reactions based on MWFRS V Brg Wid = 139 Min Req = - Bearing V is a rigid surface. Members not listed have forces less than 375#	Gravity		Non-Gravity				Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL	V*	135	-	-	/56	/8 /15
Gravity		Non-Gravity																				
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL																	
V*	135	-	-	/56	/8 /15																	

Lumber

Top chord: 2x4 SP #1;
 Bot chord: 2x4 SP #1;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

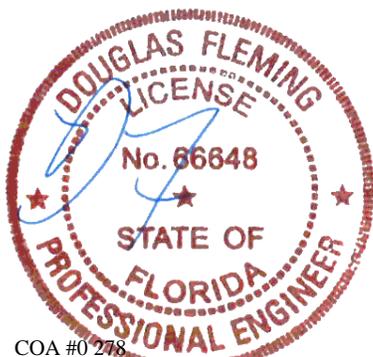
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Left and right cantilevers are exposed to wind
 Wind loading based on both gable and hip roof types.
 Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

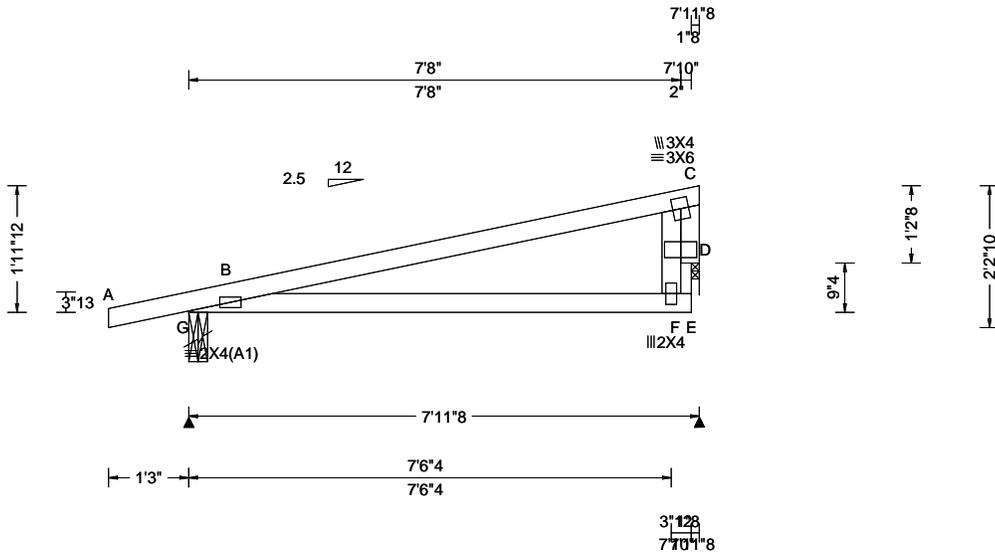
See DWGS GBLDIAG220325 and GBLLEDG220325 for gable ledger and diagonal reinforcement details.
 Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.



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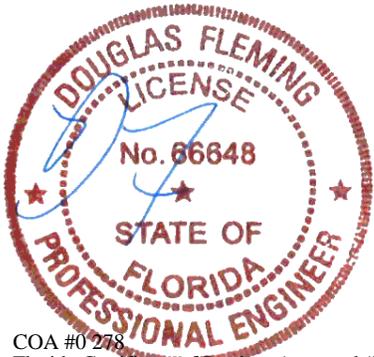




Loading Criteria (psf) TCCL: 20.00 TCDL: 7.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.055 B 999 360 VERT(CL): 0.099 B 937 240 HORZ(LL): 0.013 B - - HORZ(TL): 0.024 B - - Creep Factor: 2.0 Max TC CSI: 0.469 Max BC CSI: 0.414 Max Web CSI: 0.452 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL
				G 379 /- /- /155 /25 /39 D 283 /- /- /135 /9 /- Wind reactions based on MWFRS G Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = 1.5 (Support) Bearings G & D are a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. C - D 942 - 1086

Lumber
 Top chord: 2x4 SP #1;
 Bot chord: 2x4 SP #1;
 Webs: 2x4 SP #3;
 Rt Bearing Leg: 2x4 SP #3;

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

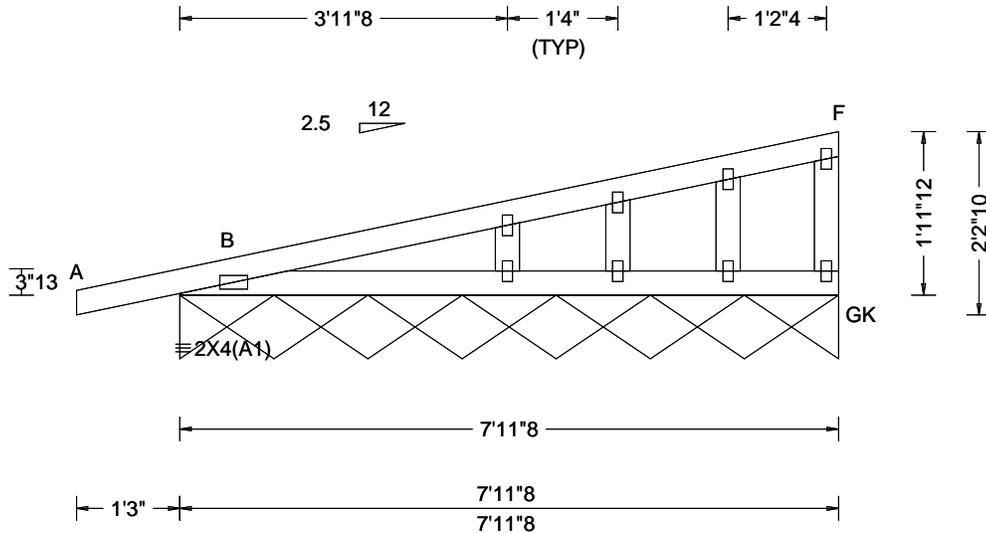


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SEQN: 75808 FROM: RJL	GABL Ply: 1 Qty: 2	Job Number: B62186a JACKSON RESIDENCE Truss Label: E2-G 7'11"8 Gable	Cust: R 857 JRef: 1YfJ8570001 T4 DrwNo: 335.25.0729.01230 SSB / DF 12/01/2025
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Loading Criteria (psf) TCCL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.005 B 999 360 VERT(CL): 0.008 B 999 240 HORZ(LL): 0.001 B - - HORZ(TL): 0.002 B - - Creep Factor: 2.0 Max TC CSI: 0.091 Max BC CSI: 0.078 Max Web CSI: 0.040 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL K* 87 /- /- /39 /23 /9 Wind reactions based on MWFRS K Brg Wid = 95.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

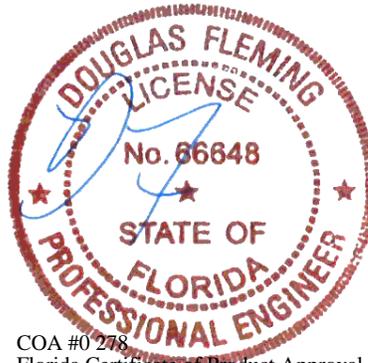
Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

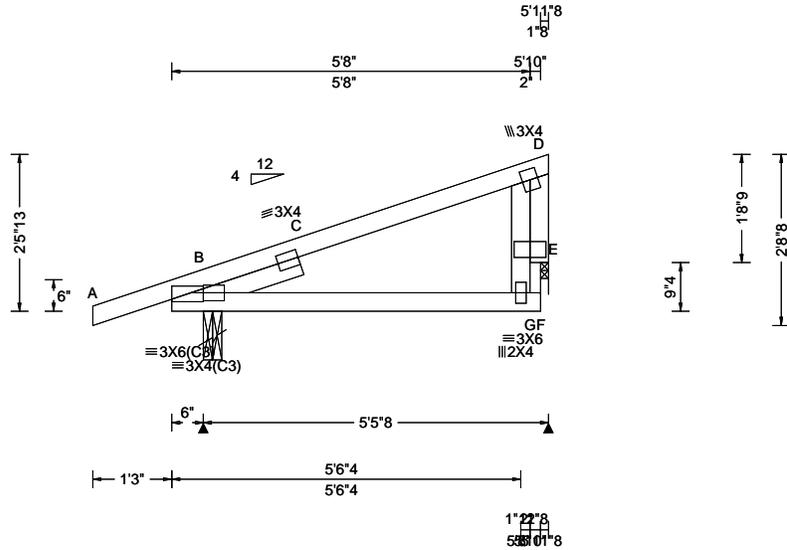


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12/01/2025

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SEQN: 75815 FROM: RJL	MONO Ply: 1 Qty: 5	Job Number: B62186a JACKSON RESIDENCE Truss Label: G1 5'11"8 Mono	Cust: R 857 JRef: 1YfJ8570001 T9 DrwNo: 335.25.0729.00253 SSB / DF 12/01/2025
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Loading Criteria (psf) TCCL: 20.00 TCDL: 7.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.020 C 999 360 VERT(CL): 0.041 C 999 240 HORZ(LL): 0.010 C - - HORZ(TL): 0.022 C - - Creep Factor: 2.0 Max TC CSI: 0.198 Max BC CSI: 0.181 Max Web CSI: 0.228 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>339</td> <td>-</td> <td>-</td> <td>/132</td> <td>/4</td> <td>/47</td> </tr> <tr> <td>E</td> <td>191</td> <td>-</td> <td>-</td> <td>/99</td> <td>/11</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	339	-	-	/132	/4	/47	E	191	-	-	/99	/11	-
				Loc		Gravity			Non-Gravity																						
R+	/R-	/Rh	/Rw		/U	/RL																									
B	339	-	-	/132	/4	/47																									
E	191	-	-	/99	/11	-																									
Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 816 -908																															

Lumber

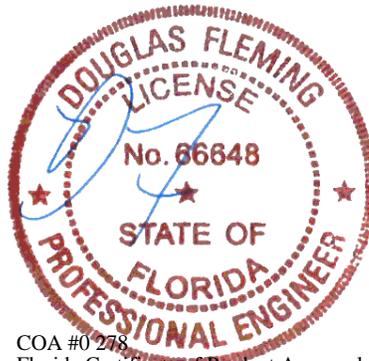
Top chord: 2x4 SP #1;
 Bot chord: 2x4 SP #1;
 Webs: 2x4 SP #3;
 Lt Slider: 2x4 SP #3; block length = 1.792'
 Rt Bearing Leg: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Maximum Web Forces Per Ply (lbs)

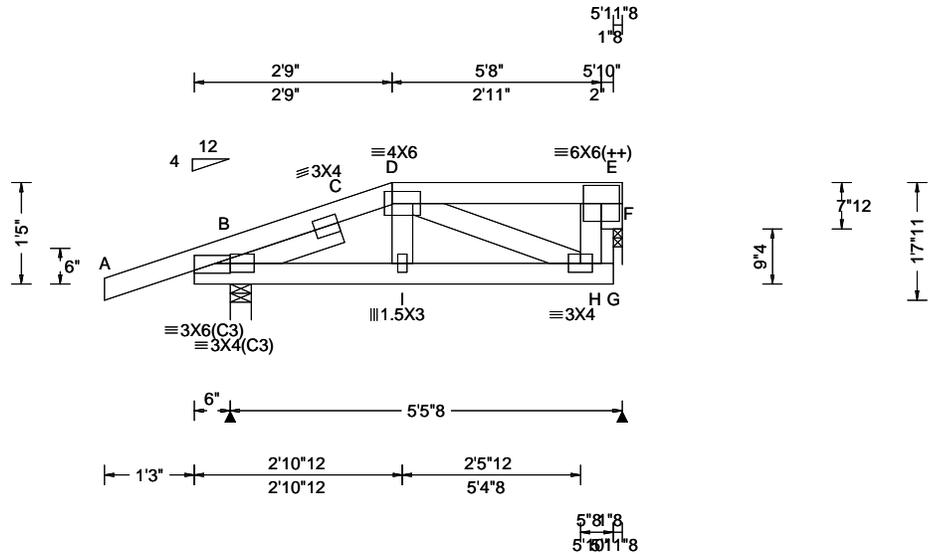
Webs	Tens.Comp.
D - E	618 -512



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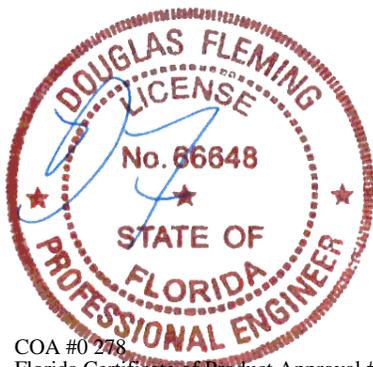


Loading Criteria (psf) TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.003 I 999 360 VERT(CL): 0.006 I 999 240 HORZ(LL): 0.001 H - - HORZ(TL): 0.001 H - - Creep Factor: 2.0 Max TC CSI: 0.104 Max BC CSI: 0.099 Max Web CSI: 0.161 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 339 /- /- /132 /13 /25 F 191 /- /- /91 /5 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 1.5 Min Req = 1.5 (Support) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 560 -633
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Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.792'
Rt Bearing Leg: 2x4 SP #3;

Plating Notes
(++) - This plate works for both joints covered.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

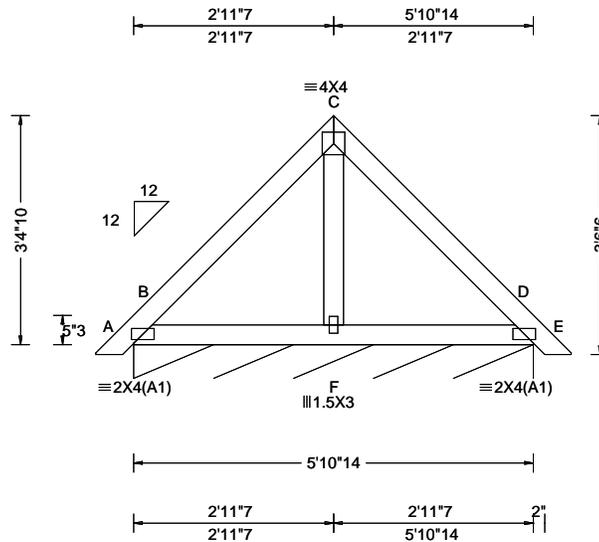


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SEQN: 75913 FROM: RJL	COMN Ply: 1 Qty: 15	Job Number: B62186a JACKSON RESIDENCE Truss Label: PB1 7'0"12 Common	Cust: R 857 JRRef: 1YfJ8570001 T20 DrwNo: 335.25.0728.57507 SSB / DF 12/01/2025
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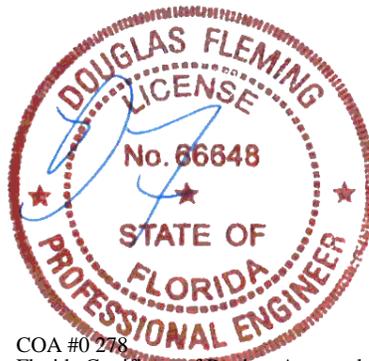
Loading Criteria (psf) TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 22.51 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 360 VERT(CL): 0.001 D 999 240 HORZ(LL): -0.001 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.125 Max BC CSI: 0.070 Max Web CSI: 0.026 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B* 88 /- /- /42 /6 /10 Wind reactions based on MWFRS B Brg Wid = 70.9 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.
Refer to DWG PB160220723 for piggyback details.

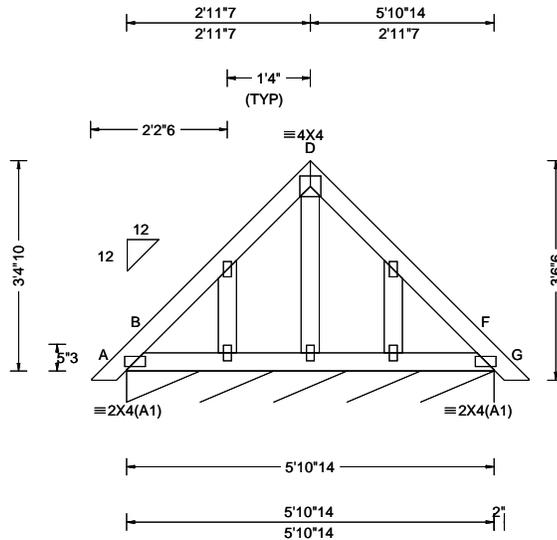


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SEQN: 75916 FROM: RJL	GABL Ply: 1 Qty: 2	Job Number: B62186a JACKSON RESIDENCE Truss Label: PB2-G 70"12 Gable	Cust: R 857 JRef: 1YfJ8570001 T13 DrwNo: 335.25.0728.54083 SSB / DF 12/01/2025
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Loading Criteria (psf) TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 22.51 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 6.06 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.000 D 999 360 VERT(CL): 0.000 D 999 240 HORZ(LL): -0.000 B - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.039 Max BC CSI: 0.022 Max Web CSI: 0.066 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B* 114 /- /- /51 /47 /13 Wind reactions based on MWFRS B Brg Wid = 70.9 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

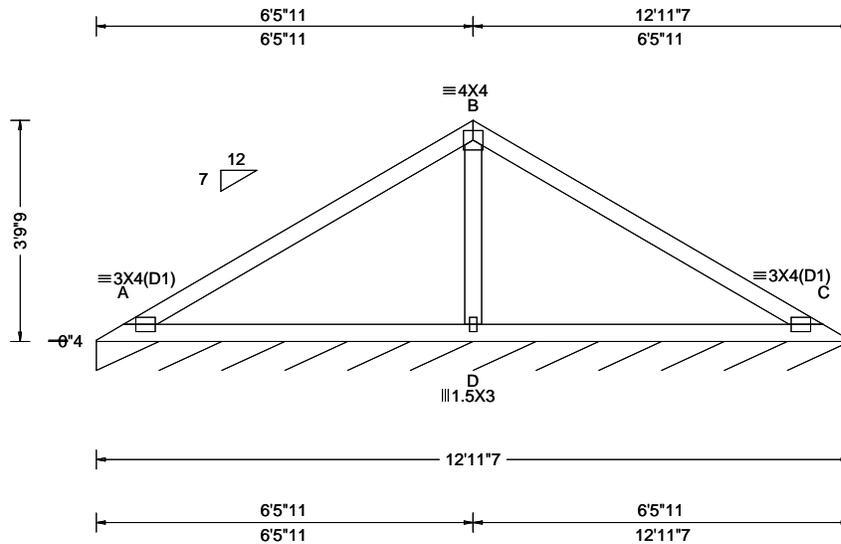
Refer to DWG PB160220723 for piggyback details.



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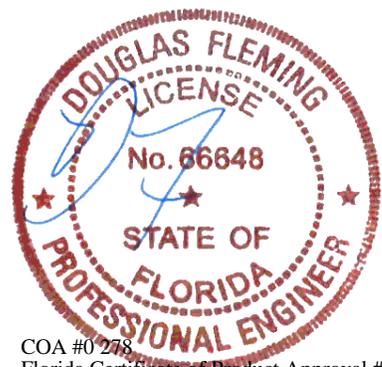


Loading Criteria (psf) TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 15.53 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.025 A 999 360 VERT(CL): 0.048 A 999 240 HORZ(LL): -0.011 C - - HORZ(TL): 0.022 C - - Creep Factor: 2.0 Max TC CSI: 0.453 Max BC CSI: 0.380 Max Web CSI: 0.202 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>C*</td> <td>76</td> <td>/-</td> <td>/-</td> <td>/36</td> <td>/-</td> <td>/4</td> </tr> </tbody> </table> Wind reactions based on MWFRS C Brg Wid = 155 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>509</td> <td>B - C</td> <td>509</td> </tr> </tbody> </table>						Gravity			Non-Gravity			Loc	R+	/R-	/Rh	/Rw	/U	/RL	C*	76	/-	/-	/36	/-	/4	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	509	B - C	509
				Gravity			Non-Gravity																														
Loc	R+	/R-	/Rh	/Rw	/U	/RL																															
C*	76	/-	/-	/36	/-	/4																															
Chords	Tens.Comp.	Chords	Tens. Comp.																																		
A - B	509	B - C	509																																		
Maximum Bot Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - D</td> <td>129</td> <td>D - C</td> <td>129</td> </tr> </tbody> </table>						Chords	Tens.Comp.	Chords	Tens. Comp.	A - D	129	D - C	129																								
Chords	Tens.Comp.	Chords	Tens. Comp.																																		
A - D	129	D - C	129																																		

Lumber
 Top chord: 2x4 SP #1;
 Bot chord: 2x4 SP #1;
 Webs: 2x4 SP #3;

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
 See DWGS VALTN220723 and VAL180220723 for valley details.

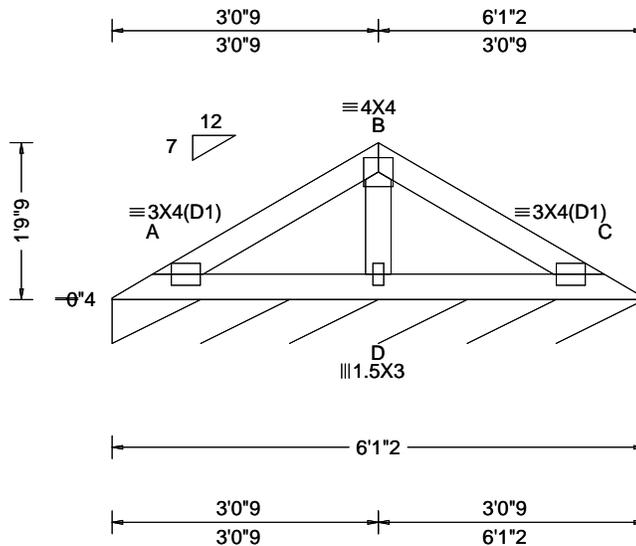


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 12/01/2025

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SEQN: 75836 FROM: RJL	VAL Ply: 1 Qty: 2	Job Number: B62186a JACKSON RESIDENCE Truss Label: V2 6'1"2 Valley	Cust: R 857 JRef: 1YfJ8570001 T14 DrwNo: 335.25.0728.39820 SSB / DF 12/01/2025
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Loading Criteria (psf) TCCL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Enclosed Risk Category: II EXP: B Kzt: NA Mean Height: 16.53 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.002 A 999 360 VERT(CL): 0.005 A 999 240 HORZ(LL): -0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.075 Max BC CSI: 0.071 Max Web CSI: 0.040 VIEW Ver: 24.02.00C.1213.15	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 76 /- /- /34 /- /4 Wind reactions based on MWFRS C Brg Wid = 73.1 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#
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Lumber

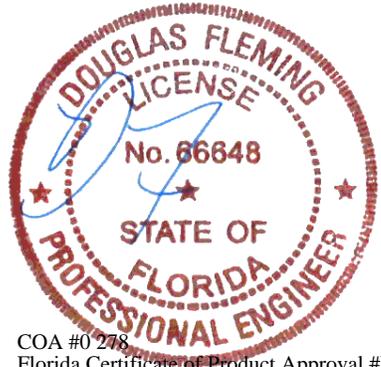
Top chord: 2x4 SP #1;
 Bot chord: 2x4 SP #1;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



COA #0 278
 Florida Certificate of Product Approval #FL1999
 12/01/2025

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Cracked or Broken Member Repair Detail

This drawing specifies repairs for a truss with broken chord or web member.

Load Duration = 0%
Member forces may be increased for Duration of Load

This design is valid only for single ply trusses with 2x4 or 2x6 broken members. No more than one break per chord panel and no more than two breaks per truss are allowed. Contact the truss manufacturer for any repairs that do not comply with this detail.

(B) = Damaged area, 12" max length of damaged section
(L) = Minimum nailing distance on each side of damaged area (B)
(S) = Two 2x4 or two 2x6 side members, same size, grade, and species as damaged member. Apply one scab per face. Minimum side member length(s) = (2)(L) + (B)

Scab member length (S) must be within the broken panel.

Nail into 2x4 members using two (2) rows at 4" o.c., rows staggered.
Nail into 2x6 members using three (3) rows at 4" o.c., rows staggered.

Nail using 10d box or gun nails (0.128"x3", min) into each side member.

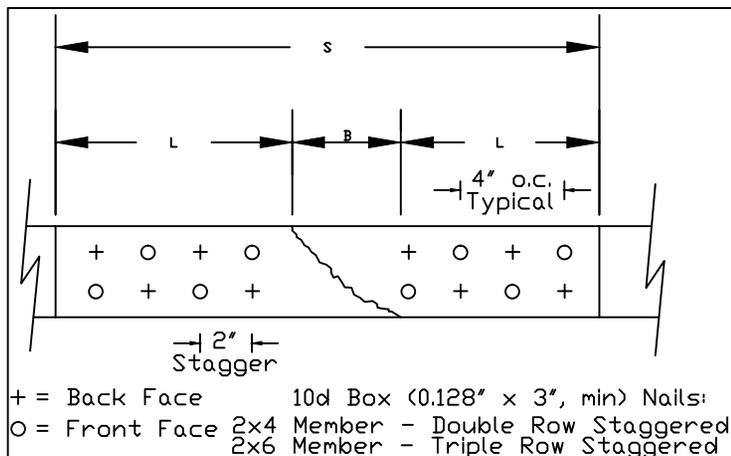
The maximum permitted lumber grade for use with this detail is limited to Visual grade #1 and MSR grade 1650f.

This repair detail may be used for broken connector plate at mid-panel splices.

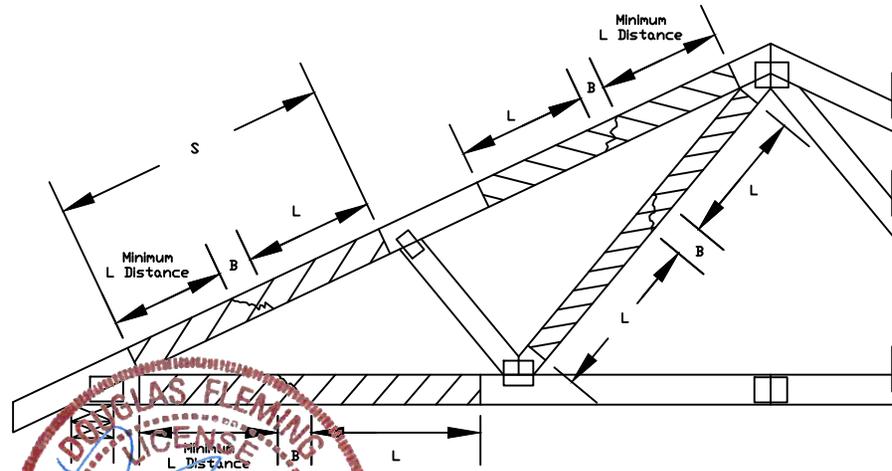
This repair detail may not be used for damaged chord or web sections occurring within the connector plate area.

Broken chord may not support any tie-in loads.

Member	Size	L	Maximum Member Axial Force			
			SPF-C	HF	DF-L	SYP
Web Only	2x4	12"	620#	635#	730#	800#
Web Only	2x4	18"	975#	1055#	1295#	1415#
Web or Chord	2x4	24"	975#	1055#	1495#	1745#
Web or Chord	2x6		1465#	1585#	2245#	2620#
Web or Chord	2x4	30"	1910#	1960#	2315#	2555#
Web or Chord	2x6		2230#	2365#	3125#	3575#
Web or Chord	2x4	36"	2470#	2530#	2930#	3210#
Web or Chord	2x6		3535#	3635#	4295#	4745#
Web or Chord	2x4	42"	2975#	3045#	3505#	3835#
Web or Chord	2x6		4395#	4500#	5225#	5725#
Web or Chord	2x4	48"	3460#	3540#	4070#	4445#
Web or Chord	2x6		5165#	5280#	6095#	6660#



Nail Spacing Detail



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REF	MEMBER REPAIR
DATE	10/01/14
DRWG	REPCHRD1014
SPACING	24.0" MAX

Valley Detail - ASCE 7-22: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 535# connection or with (1) Simpson H2.5A or equivalent connector for
 ASCE 7-22 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
 Or
 ASCE 7-22 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

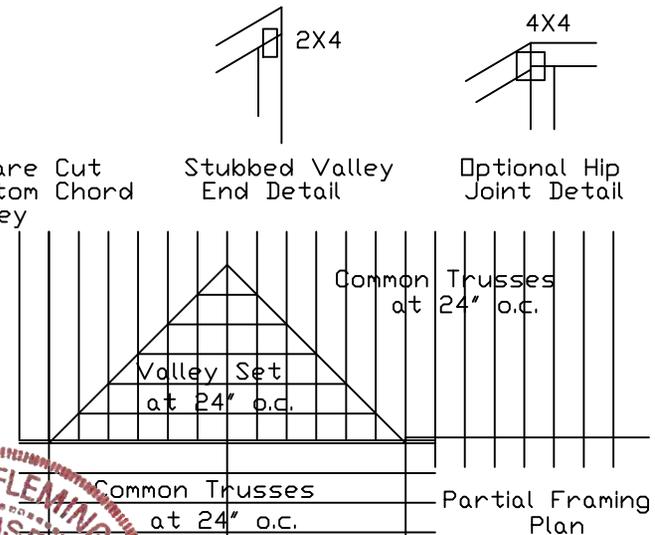
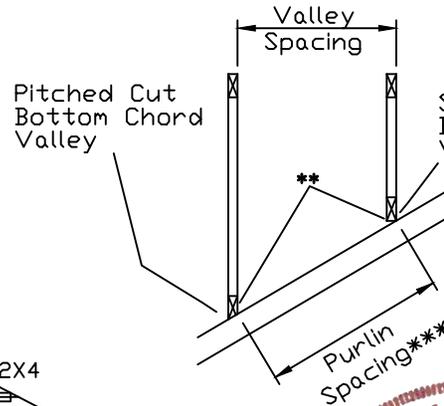
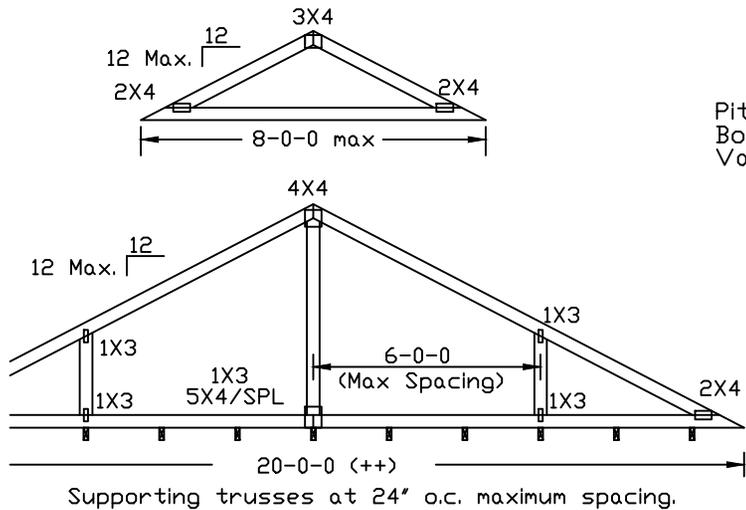
Top chord of truss beneath valley set must be braced with:
 properly attached, rated sheathing applied prior to valley truss installation.

Or
 Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

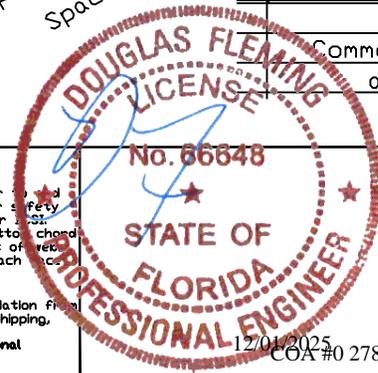
Or
 By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



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TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7PSF	DATE	07/03/2023
BC DL	10	10	10 PSF	DRWG	VAL180220723
BC LL	0	0	0PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.	1.25/1.33	1.15	1.15		
SPACING			24.0"		

Valley Detail - ASCE 7-22: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 (2) 16d box (0.135" x 3.5") nails toe-nailed for
 ASCE 7-22, 30' Mean Height, Enclosed Building, Exp. C,
 Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
 supporting truss material at connection location:
 140 mph for SP (G = 0.55, min.),
 125 mph for DF-L (G = 0.50, min.), or
 105 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses
 below valley trusses.

Bottom chord of valley trusses may be square or
 pitched cut as shown.

Valleys short enough to be cut as solid triangular
 members from a single 2x6, or larger as required,
 shall be permitted in lieu of fabricating from
 separate 2x4 members.

All plates shown are Alpine Wave Plates.

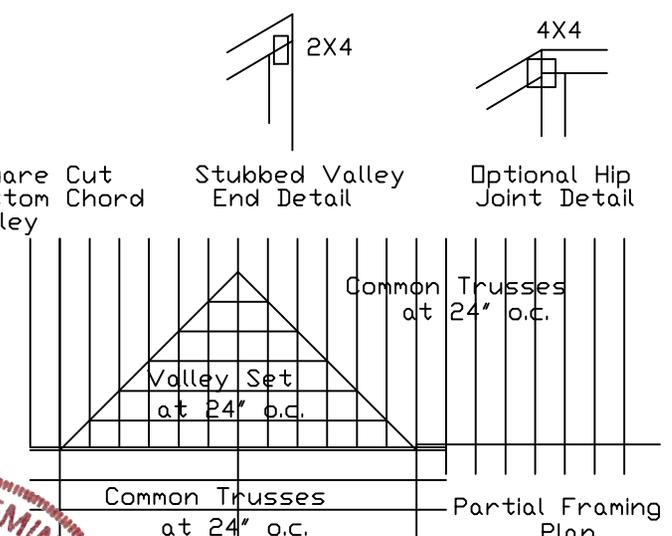
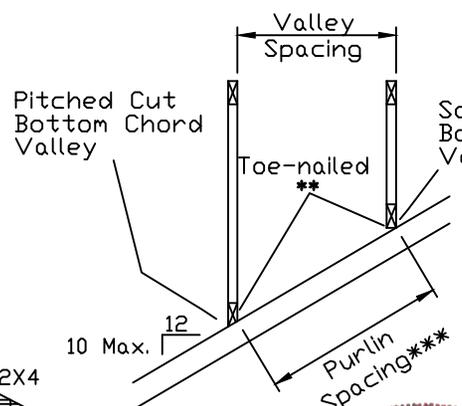
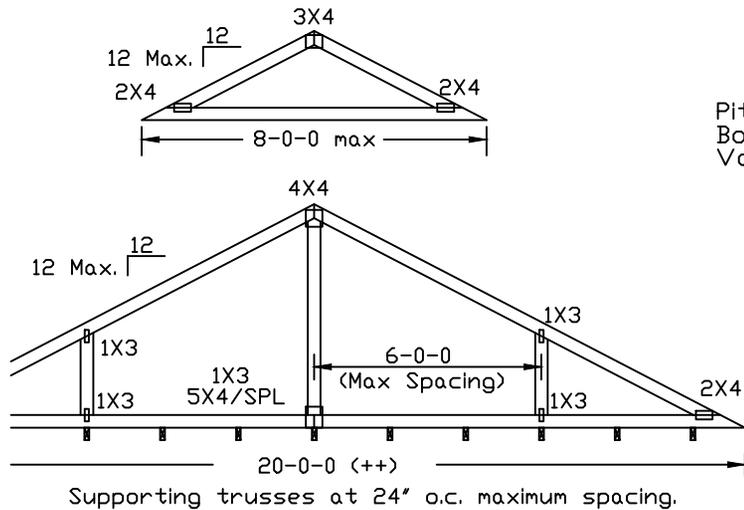
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 (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous
 Lateral Restraint applied at mid-length of web is permitted with diagonal
 bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:
 properly attached, rated sheathing applied prior to valley truss
 installation.

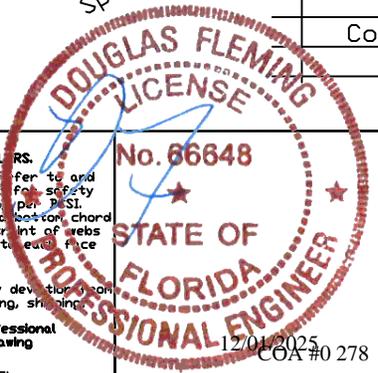
- Or
- Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
- Or
- By valley trusses used in lieu of purlin spacing as specified on
 Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss
 beneath the valley is measured along the slope of the top chord.

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 not exceed 14'-0".



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TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7 PSF	DATE	07/03/2023
BC DL	10	10	10 PSF	DRWG	VALTN220723
BC LL	0	0	0 PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.	1.25/1.33	1.15	1.15		
SPACING	24.0"				

Gable End Wind Bracing Details - Stiffback w/ Diagonal Bracing

Apply single or double stiffback as per Engineer's sealed truss design referencing this detail.

Refer to Engineer's sealed truss design for additional information not provided on this detail.

The required locations for lateral restraint or bracing depicted on this detail are for the permanent lateral transfer and support to transfer load and reduce buckling lengths. Details shall be specified by the Building Designer or other Registered Design Professional. This Detail does not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Refer to Building Designer for conditions not addressed by this detail.

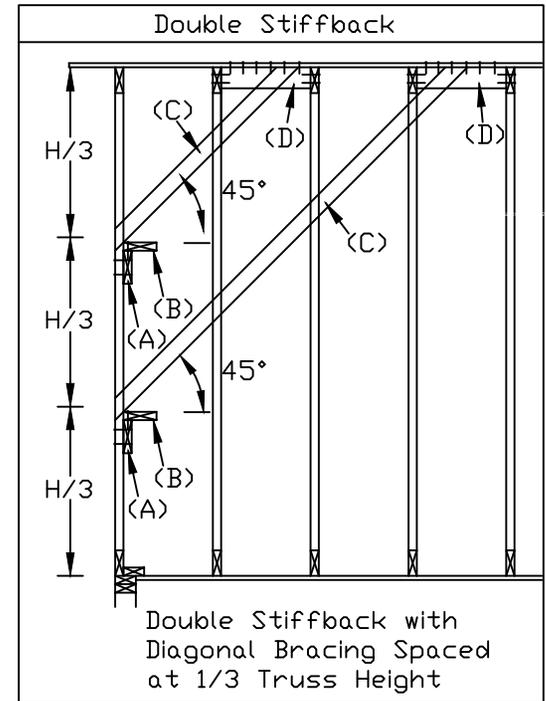
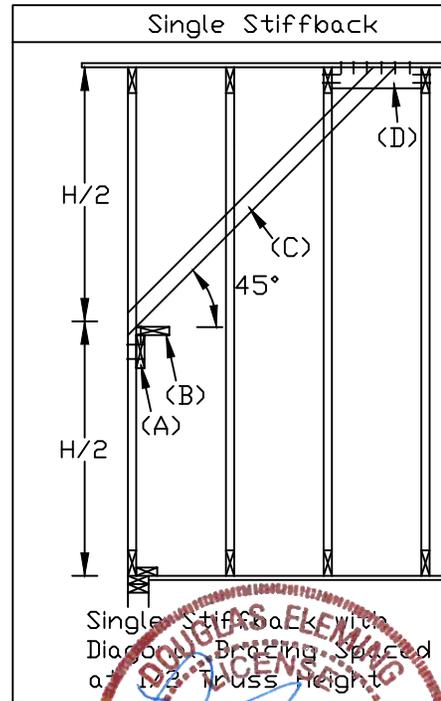
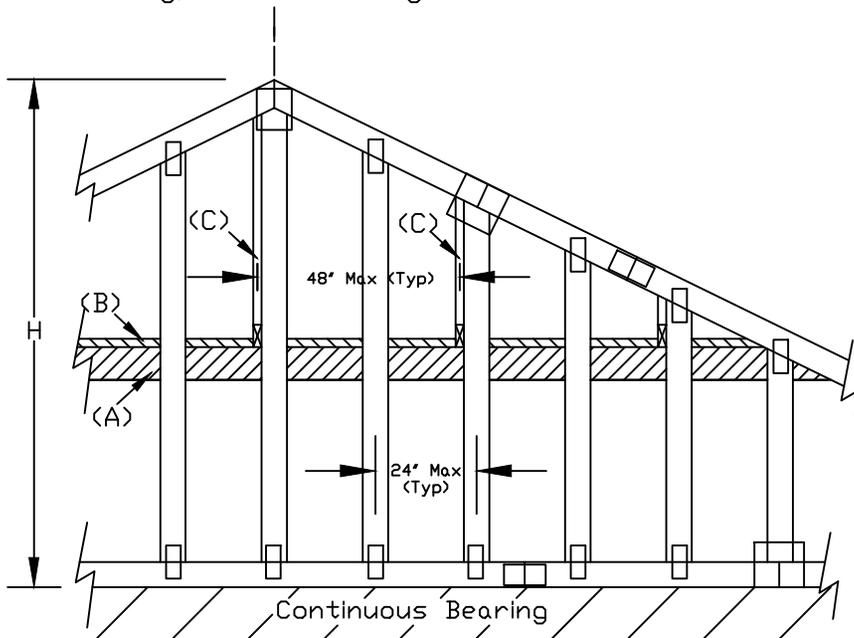
Gable Lateral Bracing Components

(A) Stiffback. Provide connection to each intersecting stud and chord.

(B) L-reinforcement. Provide connection to narrow edge of stiffback.

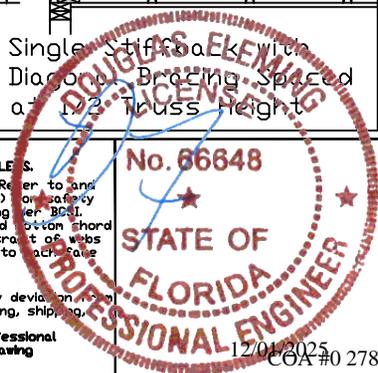
(C) Diagonal brace. Provide connection to gable stud at bottom end and to blocking at top end.

(D) Blocking, cut to fit tight between trusses. Attach blocking to trusses at each end and to roof sheathing.



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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REF	GE STIFFBACK
DATE	09/27/2023
DRWG	GBLDIAG220923
MAX. TOT. LD.	
MAX. SPACING	