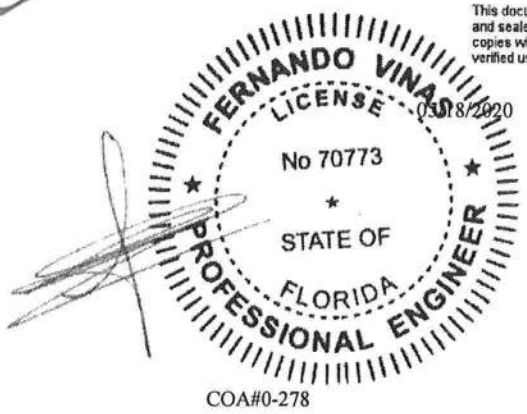
(73)



05/18/2020

COA#0-278

This document has been electronically signed and sealed using a Digital Signature. Printed copies without an original signature may not be verified using the original electronic version.

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Orlando, FL 32821
Phone: (800)755-6001
www.alpineitw.com

Site Information:	Page 1:
Customer: Riverside Roof Truss, Inc.	*Job Number:* 17556
Job Description: :THE HART RESIDENCE	
Address: S.W. MAPLETON STREET, HIGH SPRINGS, FL	

Job Engineering Criteria:			
Design Code: FBC 2017 RES			*IntelliVIEW Version:* 18.02.01B *JRef #:* 1WVd9170001
Wind Standard: ASCE 7-10	*Wind Speed (mph):* 130		*Roof Load (psf):* 20.00-10.00- 0.00-10.00
Building Type: Closed			*Floor Load (psf):* None

This package contains general notes pages, 12 truss drawing(s) and 3 detail(s).

Item	Drawing Number	Truss
1	139.20.0509.34266	A1
3	139.20.0509.34468	A3
5	139.20.0509.34251	A5
7	139.20.0509.34250	A7
9	139.20.0509.34235	B2
11	139.20.0509.34484	C1
13	A14015ENC101014	
15	GBLLETIN0118	

Item	Drawing Number	Truss
2	139.20.0509.34282	A2
4	139.20.0509.34313	A4
6	139.20.0509.34499	A6
8	139.20.0732.47150	B1
10	139.20.0509.34312	B3
12	139.20.0509.34283	C2
14	BRCLBSUB0119	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

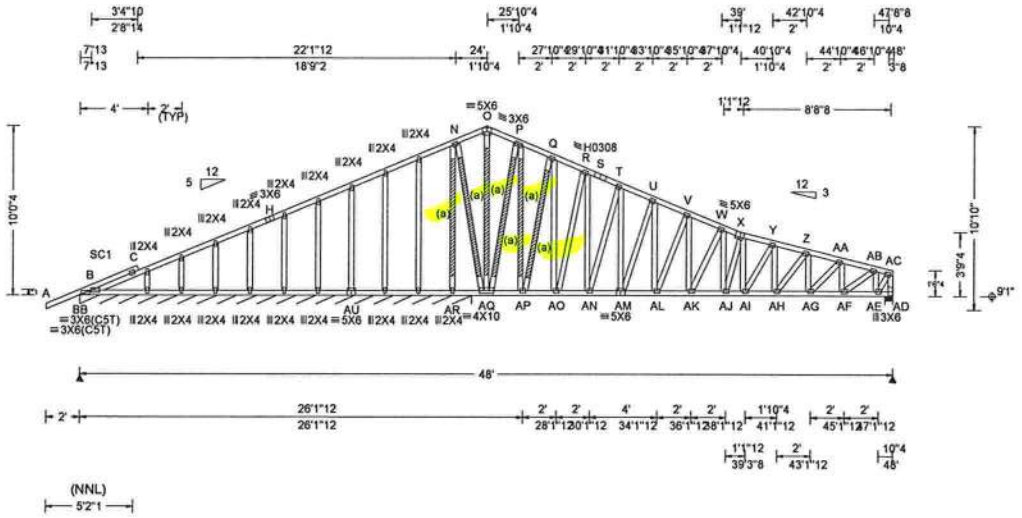
BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 13723 Riverport Drive, Suite 200, Maryland Heights, MO 63043; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.

SEQN: 380552 / GABL FROM: Page 1 of 2	Ply: 1 Qty: 1	**Job Number:** 17556 :THE HART RESIDENCE **Truss Label:** A1	Cust: R 917 JRef: 1WVd9170001 T10 DrwNo: 139.20.0509.34266 JB / DF 05/18/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 4.80 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.25

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Code / Misc Criteria
Bldg Code: FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.075	AK	999	360
VERT(CL): 0.150	AK	999	240
HORZ(LL): 0.018	O	-	-
HORZ(TL): 0.037	O	-	-

Creep Factor: 2.0
Max TC CSI: 0.268
Max BC CSI: 0.239
Max Web CSI: 0.848

VIEW Ver: 18.02.01B.0321.11

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BB*	135	/-	/-	/77	/30	/11
AD	901	/-	/-	/493	/211	/-

Wind reactions based on MWFRS
BB Brg Width = 277 Min Req = -
AD Brg Width = 5.5 Min Req = 1.5
Bearings BB & AE are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	467	-440	V - W	839	-1190
C - H	417	-387	W - X	951	-1410
H - N	425	-235	X - Y	956	-1459
R - S	480	-480	Y - Z	976	-1516
S - T	476	-508	Z -AA	942	-1459
T - U	601	-731	AA-AB	784	-1203
U - V	725	-958	AB-AC	348	-531

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B -AU	514	-364	AK-AJ	1274	-716
AU-AR	394	-367	AJ-AI	1390	-792
AR-AQ	778	-713	AI-AH	1457	-838
AO-AN	424	-99	AH-AG	1420	-844
AN-AM	630	-223	AG-AF	1206	-754
AM-AL	839	-384	AF-AE	629	-410
AL-AK	1053	-555			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
N -AQ	817	-397	AK- W	354	-481
AQ- P	670	-989	AJ- X	304	-383
AP- Q	537	-814	AA-AF	268	-413
AO- R	523	-773	AF-AB	692	-410
AN- T	495	-691	AB-AE	485	-719
AM- U	479	-618	AE-AC	882	-569
AL- V	440	-547			

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.	Gables	Tens.	Comp.
AR- N	535	-1019	U -AL	553	-414
P -AP	900	-556	V -AK	480	-325
Q -AO	779	-504	W -AJ	411	-304

Lumber

Top chord: 2x4 SP M-30;
Bot chord: 2x4 SP M-30;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP M-30;

Bracing

(a) #3 or better scab reinforcement. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Plating Notes

All plates are 3X4 except as noted.

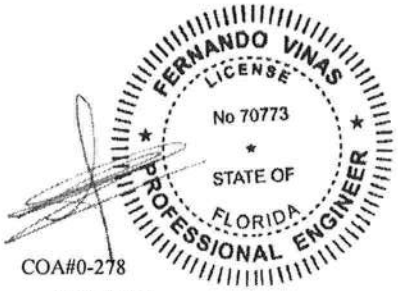
Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

See DWGS A14015ENC101014 & GBLLETIN0118 for more requirements.



COA#0-278

05/18/2020

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing,any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses**A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.**

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org

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SEQN: 380552 / GABL FROM: Page 2 of 2	Ply: 1 Qty: 1	**Job Number:** 17556 :THE HART RESIDENCE **Truss Label:** A1	Cust: R 917 JRef: 1WVd9170001 T10 DrwNo: 139.20.0509.34266 JB / DF 05/18/2020

Additional Notes

Refer to General Notes for additional information

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Top Chord overhang(s) may be field trimmed.

R -AN	708	-480	AC-AD	577	-881
T -AM	630	-459			

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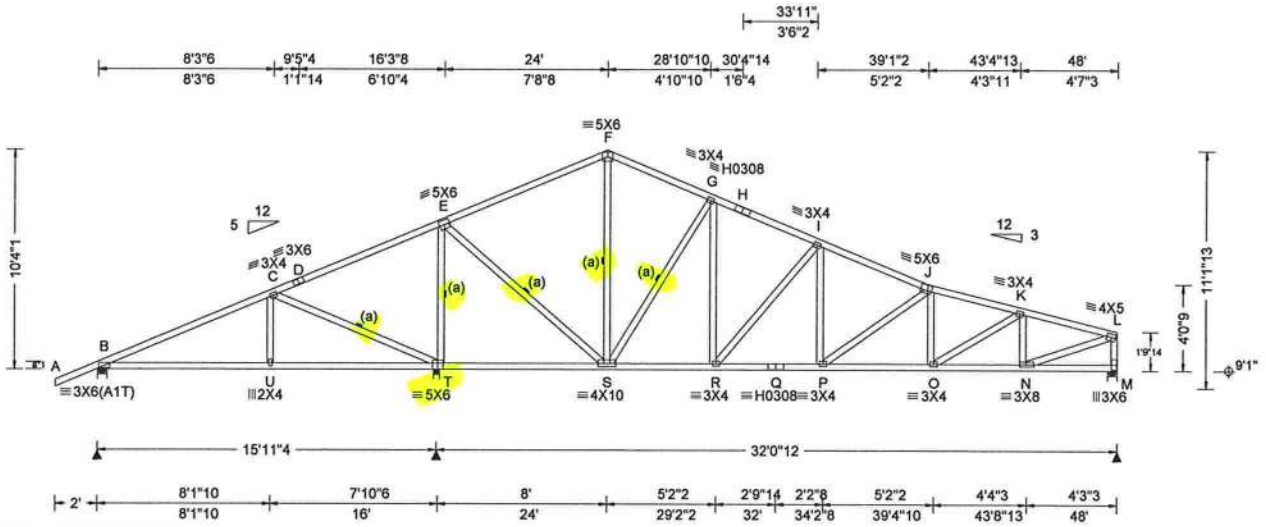
SEQN: 380567 / COMN FROM:	Ply: 1 Qty: 2	**Job Number:** 17556 :THE HART RESIDENCE **Truss Label:** A2	Cust: R 917 JRef: 1WVd9170001 T3 / DrwNo: 139.20.0509.34282 JB / DF 05/18/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 4.80 ft
Loc. from endwall: not in 6.50 ft
GCpi: 0.18
Wind Duration: 1.25

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Code / Misc Criteria
Bldg Code: FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.091	P	999	360
VERT(CL): 0.178	P	999	240
HORZ(LL): 0.026	F	-	-
HORZ(TL): 0.050	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.590
Max BC CSI: 0.349
Max Web CSI: 0.853

VIEW Ver: 18.02.01B.0321.11

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	560	/-	/-	/354	/103	/247
T	2611	/-	/-	/1356	/565	/-
M	1208	/-	/-	/654	/259	/-

Wind reactions based on MWFRS
B Brg Width = 5.5 Min Req = 1.5
T Brg Width = 3.5 Min Req = 3.1
M Brg Width = 5.5 Min Req = 1.5
Bearings B, T, & M are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	245	-482	G - H	440	-1151
C - D	696	-200	H - I	434	-1236
D - E	857	-195	I - J	578	-1766
E - F	281	-708	J - K	685	-2133
F - G	305	-664	K - L	581	-1826

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	378	-219	Q - P	1561	-388
T - S	295	-635	P - O	2043	-593
S - R	1061	-175	O - N	1794	-553
R - Q	1561	-388			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - T	269	-931	I - P	480	-156
T - E	599	-1992	P - J	250	-586
E - S	1501	-378	K - N	187	-529
S - G	340	-943	N - L	1831	-559
G - R	738	-255	L - M	395	-1166
R - I	325	-757			

Lumber

Top chord: 2x4 SP M-30;
Bot chord: 2x4 SP M-30;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

Refer to General Notes for additional information

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Top Chord overhang(s) may be field trimmed.

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COA#0-278

05/18/2020

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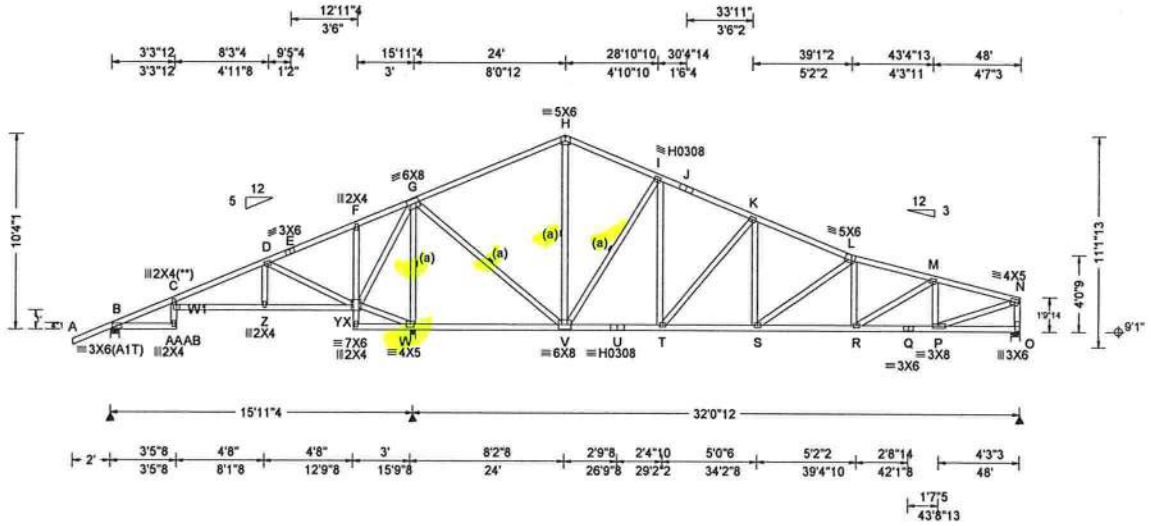
Orlando FL, 32821

SEQN: 380540 / COMN FROM:	Ply: 1 Qty: 5	**Job Number:** 17556 :THE HART RESIDENCE **Truss Label:** A3	Cust: R 917 JRef: 1WVd9170001 T5 / DrwNo: 139.20.0509.34468 JB / DF 05/18/2020



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.097 AA 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.233 AA 807 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.043 C - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.102 C - -
NCBCLL: 10.00	Mean Height: 15.00 ft	**Code / Misc Criteria**	Creep Factor: 2.0
Soffit: 0.00	TCDL: 5.0 psf	Bldg Code: FBC 2017 RES	Max TC CSI: 0.607
Load Duration: 1.25	BCDL: 3.0 psf	TPI Std: 2014	Max BC CSI: 0.346
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.853
	C&C Dist a: 4.80 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 13.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE, HS	VIEW Ver: 18.02.01B.0321.11
	Wind Duration: 1.25		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	296	/-	/-	/157	/84	/247
W	2953	/-	/-	/1638	/631	/-
O	1208	/-	/-	/663	/239	/-

Wind reactions based on MWFRS
B Brg Width = 5.5 Min Req = 1.5
W Brg Width = 3.5 Min Req = 3.1
O Brg Width = 5.5 Min Req = 1.5
Bearings B, W, & O are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
C - D	863	-221	I - J	408	-1150
D - E	1385	-319	J - K	402	-1235
E - F	1466	-314	K - L	546	-1764
F - G	1501	-291	L - M	656	-2132
G - H	280	-713	M - N	560	-1825
H - I	275	-662			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
AA- Z	205	-728	T - S	1559	-360
Z - X	206	-739	S - R	2042	-564
W - V	357	-1198	R - Q	1793	-532
V - U	1059	-169	Q - P	1793	-532
U - T	1059	-169			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - X	210	-724	T - K	326	-757
X - W	396	-1322	K - S	481	-156
X - G	65	-388	S - L	250	-587
W - G	592	-2137	M - P	180	-529
G - V	1891	-435	P - N	1830	-538
V - I	336	-937	N - O	383	-1166
I - T	741	-257			

Lumber

Top chord: 2x4 SP M-30;
Bot chord: 2x4 SP M-30;
Webs: 2x4 SP #3; W1 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

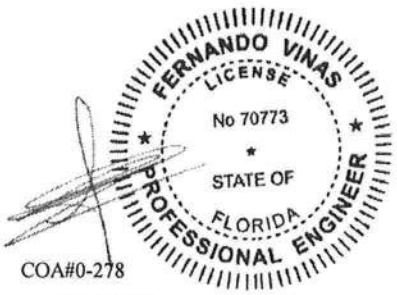
Wind loads based on MWFRS with additional C&C member design.

Additional Notes

Refer to General Notes for additional information

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Top Chord overhang(s) may be field trimmed.



COA#0-278

05/18/2020

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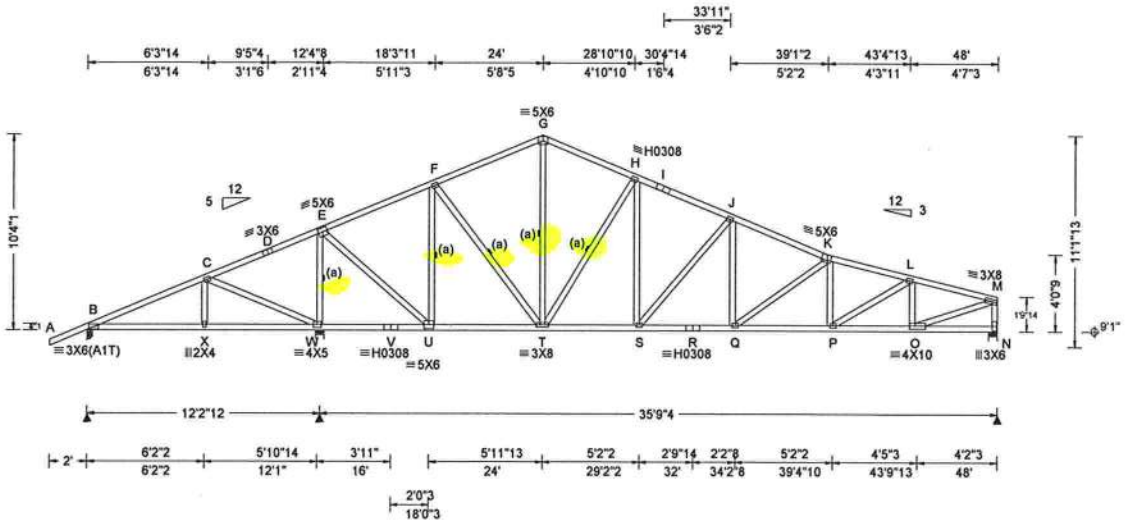
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 380574 / COMN FROM:	Ply: 1 Qty: 4	**Job Number:** 17556 :THE HART RESIDENCE **Truss Label:** A4	Cust: R 917 JRef: 1WVd9170001 T12 / DrwNo: 139.20.0509.34313 JB / DF 05/18/2020



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.122 Q 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.238 Q 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.032 G - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.062 G - -
NCBCLL: 10.00	Mean Height: 15.00 ft	**Code / Misc Criteria**	Creep Factor: 2.0
Soffit: 0.00	TCDL: 5.0 psf	Bldg Code: FBC 2017 RES	Max TC CSI: 0.409
Load Duration: 1.25	BCDL: 3.0 psf	TPI Std: 2014	Max BC CSI: 0.387
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.842
	C&C Dist a: 4.80 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 13.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE, HS	VIEW Ver: 18.02.01B.0321.11
	Wind Duration: 1.25		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	385	/-71	/-	/227	/42	/247
W	2580	/-	/-	/1378	/186	/-
N	1413	/-	/-	/753	/97	/-

Wind reactions based on MWFRS
B Brg Width = 3.0 Min Req = 1.5
W Brg Width = 5.5 Min Req = 2.7
N Brg Width = 5.5 Min Req = 1.7
Bearings B, W, & N are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	494	-173	H - I	595	-1684
C - D	909	-258	I - J	588	-1768
D - E	978	-245	J - K	731	-2294
E - F	256	-793	K - L	829	-2627
F - G	441	-1203	L - M	682	-2173
G - H	459	-1189			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - X	189	-429	T - S	1554	-319
X - W	190	-434	S - R	2048	-530
W - V	298	-737	R - Q	2048	-530
V - U	298	-737	Q - P	2529	-735
U - T	701	-73	P - O	2142	-654

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - W	207	-715	S - J	322	-747
W - E	616	-2192	J - Q	475	-156
E - U	1812	-474	Q - K	250	-584
U - F	316	-1022	P - L	450	-92
F - T	615	-145	L - O	222	-650
G - T	549	-188	O - M	2187	-663
T - H	345	-968	M - N	454	-1370
H - S	743	-253			

Lumber

Top chord: 2x4 SP M-30;
Bot chord: 2x4 SP M-30;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

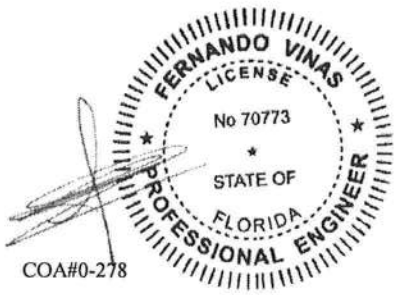
Wind loads based on MWFRS with additional C&C member design.

Additional Notes

Refer to General Notes for additional information

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Top Chord overhang(s) may be field trimmed.



COA#0-278

05/18/2020

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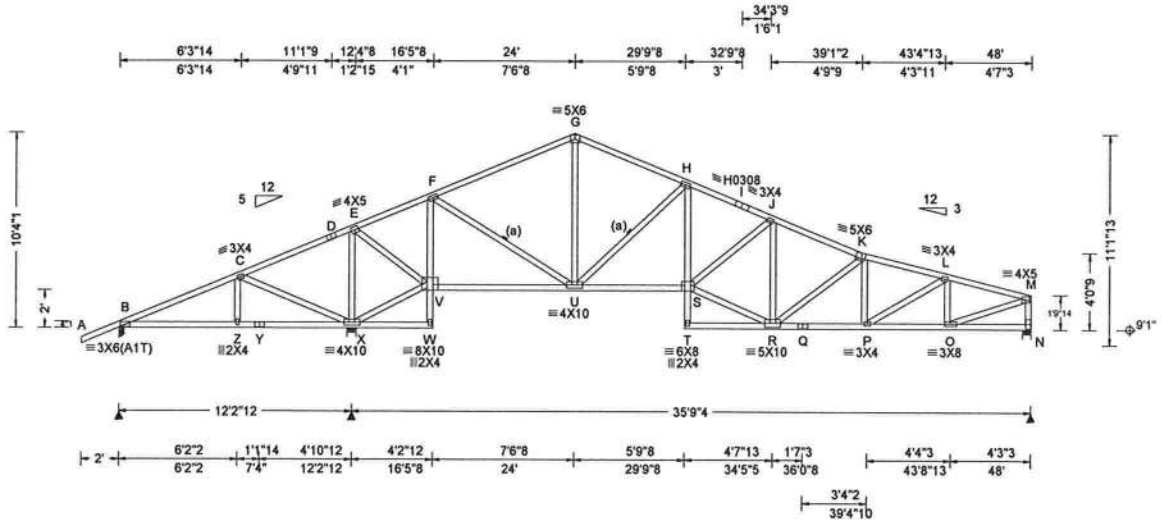
For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org

ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 380577 / COMN FROM:	Ply: 1 Qty: 8	**Job Number:** 17556 :THE HART RESIDENCE **Truss Label:** A5	Cust: R 917 JRef: 1WVd9170001 T11 / DrwNo: 139.20.0509.34251 JB / DF 05/18/2020



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 10.00
Soffit: 0.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.80 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.25

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Code / Misc Criteria
Bldg Code: FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.117	S	999	360
VERT(CL): 0.240	S	999	240
HORZ(LL): 0.037	G	-	-
HORZ(TL): 0.077	G	-	-

Creep Factor: 2.0
Max TC CSI: 0.491
Max BC CSI: 0.493
Max Web CSI: 0.843

VIEW Ver: 18.02.01B.0321.11

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	337	/-218	/-	/144	/48	/247
X	2638	/-	/-	/1499	/192	/-
N	1291	/-	/-	/724	/94	/-

Wind reactions based on MWFRS
B Brg Width = 3.0 Min Req = 1.5
X Brg Width = 5.5 Min Req = 2.7
N Brg Width = 5.5 Min Req = 1.5
Bearings B, X, & N are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	851	-226	H - I	711	-2065
C - D	1320	-358	I - J	699	-2102
D - E	1348	-338	J - K	698	-2002
F - G	424	-1170	K - L	790	-2332
G - H	447	-1141	L - M	655	-1967

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Z	227	-756	R - Q	2238	-695
Z - Y	227	-761	Q - P	2238	-695
Y - X	227	-761	P - O	1936	-628
U - S	1887	-442			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - X	221	-748	H - S	923	-312
X - E	437	-1587	S - R	1986	-562
X - V	462	-1319	J - R	90	-399
E - V	1447	-372	R - K	240	-563
V - F	444	-1380	L - O	213	-577
F - U	1106	-299	O - M	1977	-636
G - U	449	-131	M - N	438	-1249
U - H	424	-1229			

Lumber

Top chord: 2x4 SP M-30;
Bot chord: 2x4 SP M-30;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X6 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

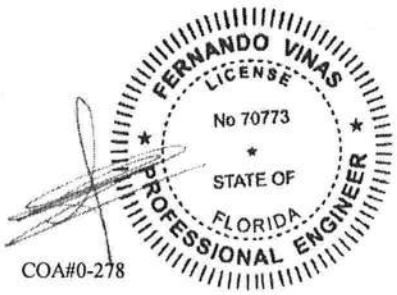
Additional Notes

Refer to General Notes for additional information

Negative reaction(s) of -218# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Top Chord overhang(s) may be field trimmed.



COA#0-278

05/18/2020

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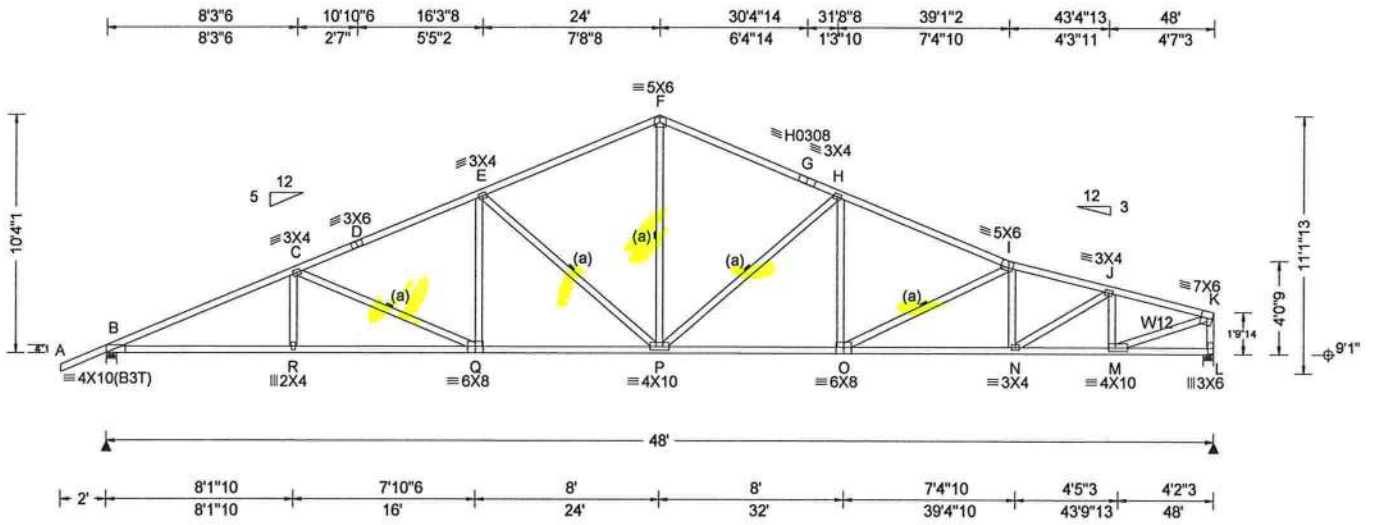
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 380536 / COMN FROM:	Ply: 1 Qty: 7	**Job Number:** 17556 :THE HART RESIDENCE **Truss Label:** A6	Cust: R 917 JRef: 1WVd9170001 T8 / DrwNo: 139.20.0509.34499 JB / DF 05/18/2020



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria			
TCLL:	20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in	loc	L/defl	L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.327	P	999	360
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.621	P	924	240
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.116	L	-	-
Des Ld:	40.00	EXP: C Kzt: NA		HORZ(TL): 0.221	L	-	-
NCBCLL:	10.00	Mean Height: 15.00 ft	**Code / Misc Criteria**	Creep Factor: 2.0			
Soffit:	0.00	TCDL: 5.0 psf	Bldg Code: FBC 2017 RES	Max TC CSI: 0.655			
Load Duration:	1.25	BCDL: 3.0 psf	TPI Std: 2014	Max BC CSI: 0.764			
Spacing:	24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.751			
		C&C Dist a: 4.80 ft	FT/RT:20(0)/10(0)				
		Loc. from endwall: Any	Plate Type(s):				
		GCpi: 0.18	WAVE, HS	VIEW Ver: 18.02.01B.0321.11			
		Wind Duration: 1.25					

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	2218	/-	/-	/1261	/481	/247
L	2072	/-	/-	/1032	/446	/-

Wind reactions based on MWFRS
B Brg Width = 5.5 Min Req = 2.6
L Brg Width = 5.5 Min Req = 2.4
Bearings B & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	2096	-4625	G - H	1573	-2911
C - D	1831	-3834	H - I	1989	-3831
D - E	1854	-3771	I - J	2268	-4230
E - F	1557	-2912	J - K	1756	-3284
F - G	1600	-2879			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - R	4190	-1870	P - O	3444	-1572
R - Q	4187	-1870	O - N	4112	-2110
Q - P	3432	-1497	N - M	3256	-1714

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - Q	424	-804	O - I	597	-732
Q - E	588	-182	I - N	244	-436
E - P	585	-1104	N - J	984	-451
F - P	1699	-844	J - M	587	-1037
P - H	686	-1121	M - K	3325	-1740
H - O	604	-285	K - L	1141	-2026

Lumber

Top chord: 2x4 SP M-30;
Bot chord: 2x4 SP M-30;
Webs: 2x4 SP #3; W12 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

Refer to General Notes for additional information

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Top Chord overhang(s) may be field trimmed.

FERNANDO VINAS LICENSE No 70773 STATE OF FLORIDA PROFESSIONAL ENGINEER

COA#0-278

05/18/2020

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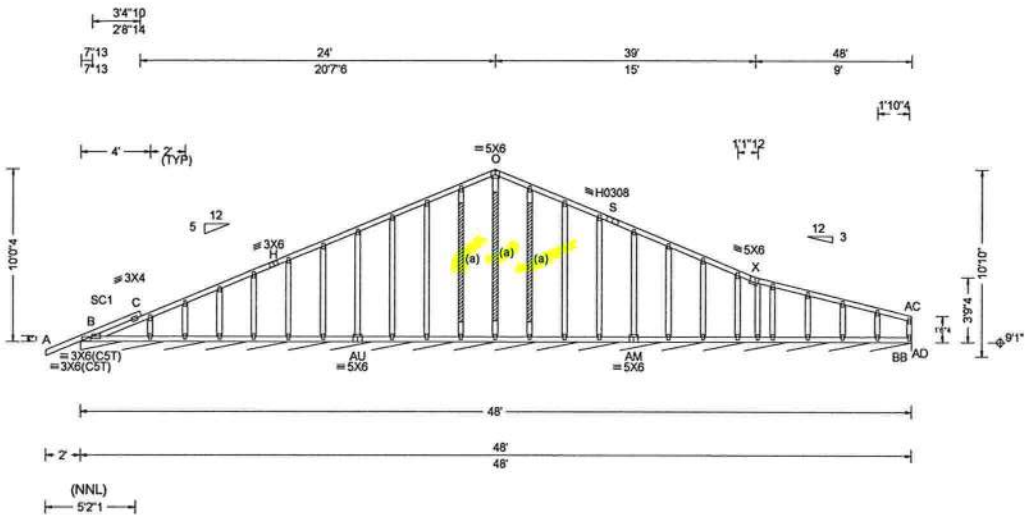
ALPINE AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 380544 / GABL FROM: Page 1 of 2	Ply: 1 Qty: 1	**Job Number:** 17556 :THE HART RESIDENCE **Truss Label:** A7	Cust: R 917 JRef: 1WVd9170001 T9 / DrwNo: 139.20.0509.34250 JB / DF 05/18/2020



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**	**▲Maximum Reactions (lbs), or *=PLF**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.80 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.25	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA **Code / Misc Criteria** Bldg Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.003 C 999 360 VERT(CL): 0.006 C 999 240 HORZ(LL): 0.009 X - - HORZ(TL): 0.012 X - - Creep Factor: 2.0 Max TC CSI: 0.269 Max BC CSI: 0.066 Max Web CSI: 0.154 VIEW Ver: 18.02.01B.0321.11	Gravity / Non-Gravity Loc R+ / R- / Rh / Rw / U / RL BB*84 /- /- /45 /19 /5 Wind reactions based on MWFRS BB Brg Width = 576 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# **Maximum Top Chord Forces Per Ply (lbs)** Chords Tens. Comp. Chords Tens. Comp. H - O 483 -134 O - S 483 -29

Lumber

Top chord: 2x4 SP M-30;
Bot chord: 2x4 SP M-30;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP M-30;

Bracing

(a) #3 or better scab reinforcement. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Plating Notes

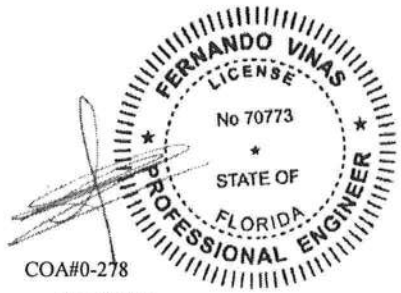
All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

See DWGS A14015ENC101014 & GBLLETIN0118 for more requirements.

Truss designed to support 8" maximum gable end overhang.



COA#0-278

05/18/2020

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****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org

ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 380544 / GABL FROM: Page 2 of 2	Ply: 1 Qty: 1	**Job Number:** 17556 :THE HART RESIDENCE **Truss Label:** A7	Cust: R 917 JRef: 1WVd9170001 T9 / DrwNo: 139.20.0509.34250 JB / DF 05/18/2020

Additional Notes

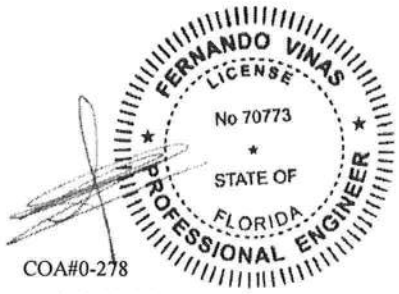
Refer to General Notes for additional information

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Top Chord overhang(s) may be field trimmed.



COA#0-278

05/18/2020

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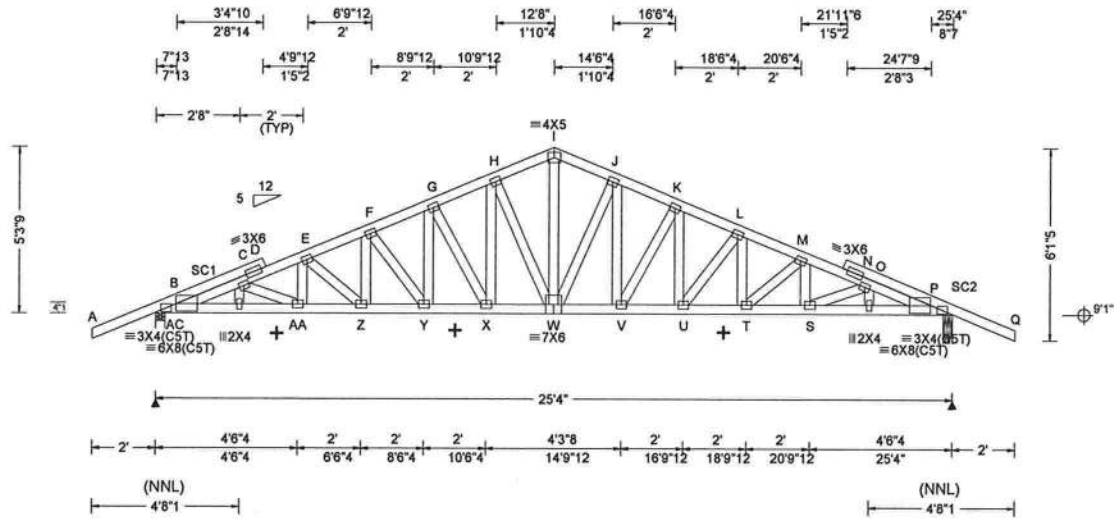
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 380982 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 17556 :THE HART RESIDENCE **Truss Label:** B1	Cust: R 917 JRef: 1WVd9170001 T2 DrwNo: 139.20.0732.47150 JB / FV 05/18/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.25

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Code / Misc Criteria
Bldg Code: FBC 2017 RES
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.192	V	999	360
VERT(CL): 0.379	V	783	240
HORZ(LL): 0.058	R	-	-
HORZ(TL): 0.115	R	-	-

Creep Factor: 2.0
Max TC CSI: 0.558
Max BC CSI: 0.933
Max Web CSI: 0.467

VIEW Ver: 18.02.01B.0321.11

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
AC	2312	/-	/-	/1022	/499	/307
P	2312	/-	/-	/1022	/499	/-

Wind reactions based on MWFRS
AC Brg Width = 3.5 Min Req = 2.7
P Brg Width = 3.5 Min Req = 2.7
Bearings AC & P are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	809	-1417	I - J	1642	-2864
B - D	1734	-3497	J - K	1826	-3343
C - D	821	-1302	K - L	2015	-3794
D - E	2358	-4661	L - M	2214	-4253
E - F	2212	-4253	M - N	2361	-4661
F - G	2014	-3794	N - O	798	-1262
G - H	1825	-3343	N - P	1750	-3537
H - I	1641	-2864	O - P	792	-1381

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B -AA	4549	-2036	W - V	2981	-1132
AA- Z	4297	-1910	V - U	3410	-1365
Z - Y	3826	-1615	U - T	3826	-1594
Y - X	3410	-1341	T - S	4297	-1872
X - W	2981	-1093	S - P	4550	-2073

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
E - Z	376	-598	W - J	594	-918
F - Y	435	-658	V - K	488	-836
G - X	488	-836	U - L	436	-658
H - W	594	-918	T - M	376	-598

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.	Gables	Tens.	Comp.
Z - F	435	-262	J - V	737	-421
Y - G	568	-363	K - U	568	-363
X - H	737	-421	L - T	435	-263
I - W	1740	-1018			

Lumber

Top chord: 2x4 SP M-30;
Bot chord: 2x4 SP M-30;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP M-30;
Stack Chord: SC2 2x4 SP M-30;

Special Loads

-----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	62 plf at	-2.00 to	62 plf at	27.33
BC: From	20 plf at	0.00 to	20 plf at	9.23
BC: From	80 plf at	9.23 to	80 plf at	16.10
BC: From	20 plf at	16.10 to	20 plf at	25.33

Plating Notes

All plates are 3X4 except as noted.

Loading

Truss designed to support 2-0-0 top chord outlookers and cladding load not to exceed 10.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS.

Additional Notes

Refer to General Notes for additional information

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Top Chord overhang(s) may be field trimmed.

FERNANDO VINAS LICENSE No 70773 STATE OF FLORIDA PROFESSIONAL ENGINEER

COA#0-278

05/18/2020

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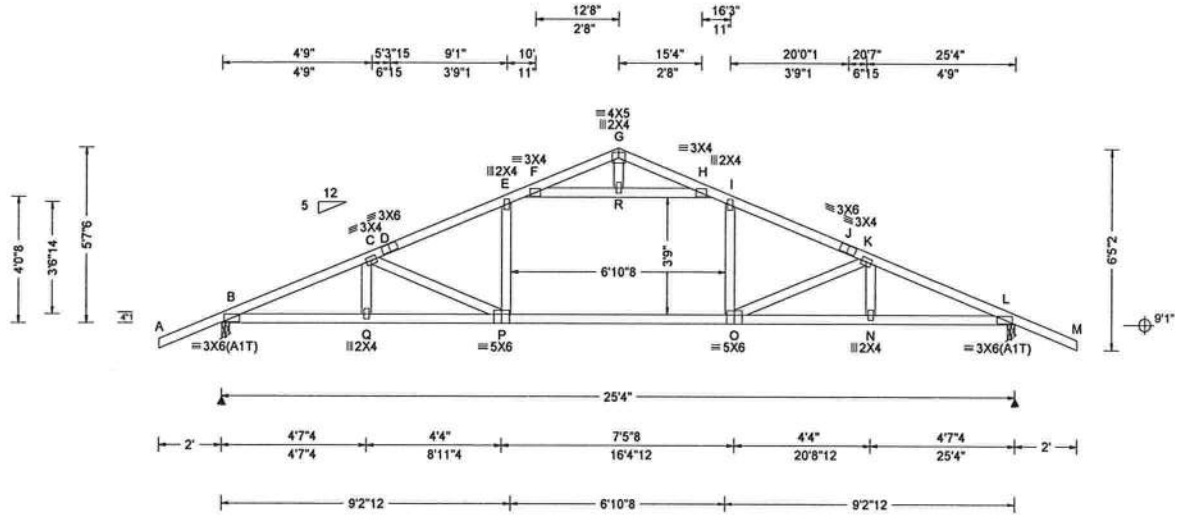
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 380522 / COMN FROM:	Ply: 1 Qty: 11	**Job Number:** 17556 :THE HART RESIDENCE **Truss Label:** B2	Cust: R 917 JRef: 1WVd9170001 T1 / DrwNo: 139.20.0509.34235 JB / DF 05/18/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.25

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Code / Misc Criteria
Bldg Code: FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.221	O	999	360
VERT(CL): 0.494	O	609	240
HORZ(LL): 0.079	E	-	-
HORZ(TL): 0.178	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.601
Max BC CSI: 0.892
Max Web CSI: 0.248

VIEW Ver: 18.02.01B.0321.11

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1404	/-	/-	/678	/269	/158
L	1404	/-	/-	/678	/269	/-

Wind reactions based on MWFRS
B Brg Width = 3.0 Min Req = 1.7
L Brg Width = 3.0 Min Req = 1.7
Bearings B & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	983	-2746	H - I	826	-2045
C - D	849	-2399	I - J	864	-2384
D - E	864	-2384	J - K	849	-2399
E - F	826	-2045	K - L	983	-2746

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Q	2498	-797	O - N	2500	-813
Q - P	2500	-797	N - L	2498	-813
P - O	2122	-580			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - P	255	-541	R - H	722	-2056
P - E	619	-102	I - O	619	-103
F - R	722	-2056	O - K	255	-541

Lumber

Top chord: 2x4 SP M-30;
Bot chord: 2x4 SP M-30;
Webs: 2x4 SP #3;

Loading

Attic room loading from 9-2-12 to 16-1-4: Live Load: 30 PSF. Dead Load: 5 PSF Ceiling: 0 PSF, Kneewalls: 0 PSF

Truss designed for sleeping room only. No waterbeds permitted. Provide information to contractor, architect, and bldg owner. Trusses to be visibly stamped to indicate 30.00 psf MAX LL.

Purlins

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

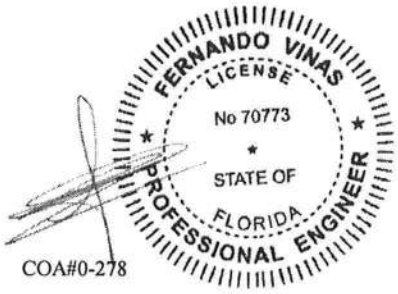
Wind

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

Refer to General Notes for additional information

Top Chord overhang(s) may be field trimmed.



COA#0-278

05/18/2020

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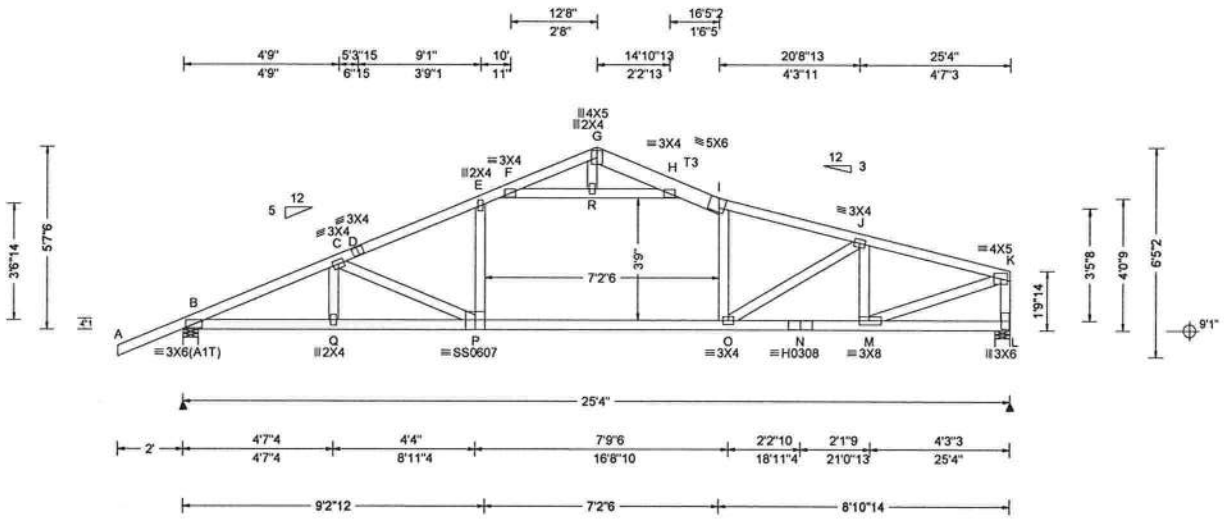
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 380563 / COMN FROM:	Ply: 1 Qty: 3	**Job Number:** 17556 :THE HART RESIDENCE **Truss Label:** B3	Cust: R 917 JRef: 1WVd9170001 T4 / DrwNo: 139.20.0509.34312 JB / DF 05/18/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.25

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Code / Misc Criteria
Bldg Code: FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, 18SS, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.255	I	999	360
VERT(CL): 0.560	I	539	240
HORZ(LL): -0.070	I	-	-
HORZ(TL): 0.154	I	-	-

Creep Factor: 2.0
Max TC CSI: 0.647
Max BC CSI: 0.978
Max Web CSI: 0.744

VIEW Ver: 18.02.01B.0321.11

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1418	/-	/-	/688	/123	/109
L	1274	/-	/-	/529	/132	/-

Wind reactions based on MWFRS
B Brg Width = 5.5 Min Req = 1.7
L Brg Width = 5.5 Min Req = 1.5
Bearings B & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	615	-2749	H - I	551	-2124
C - D	552	-2458	I - J	548	-2314
D - E	568	-2442	J - K	493	-1939
E - F	543	-2092			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Q	2498	-560	O - N	1920	-466
Q - P	2502	-561	N - M	1920	-466
P - O	2179	-459			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - P	127	-478	O - J	564	-81
P - E	648	-70	J - M	149	-654
F - R	499	-2189	M - K	1953	-473
R - H	509	-2234	K - L	336	-1218

Lumber

Top chord: 2x4 SP M-30; T3 2x6 SP #2;
Bot chord: 2x4 SP M-30;
Webs: 2x4 SP #3;

Loading

Attic room loading from 9-2-12 to 16-5-2: Live Load: 30 PSF. Dead Load: 5 PSF Ceiling: 0 PSF, Kneewalls: 0 PSF

Truss designed for sleeping room only. No waterbeds permitted. Provide information to contractor, architect, and bldg owner. Trusses to be visibly stamped to indicate 30.00 psf MAX LL.

Purlins

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

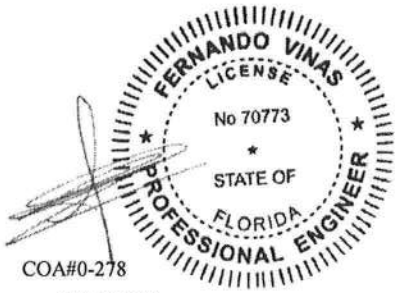
Wind

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

Refer to General Notes for additional information

Top Chord overhang(s) may be field trimmed.



COA#0-278

05/18/2020

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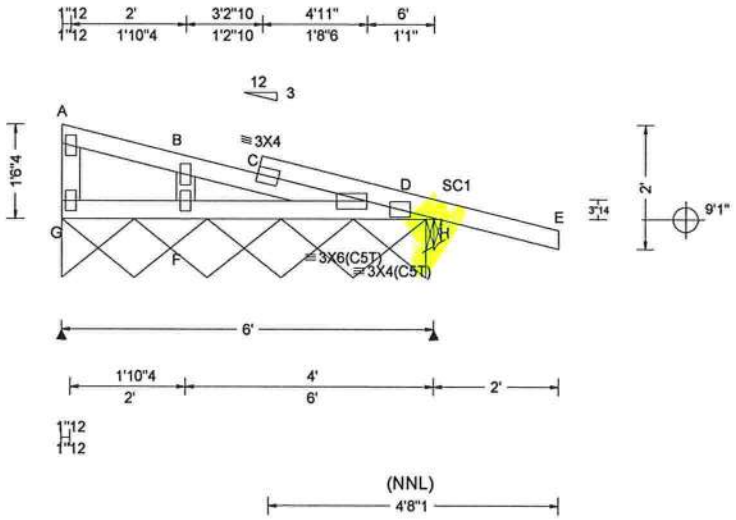
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 380533 / GABL FROM:	Ply: 1 Qty: 2	**Job Number:** 17556 :THE HART RESIDENCE **Truss Label:** C1	Cust: R 917 JRef: 1WVd9170001 T7 / DrwNo: 139.20.0509.34484 JB / DF 05/18/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.25

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Code / Misc Criteria
Bldg Code: FBC 2017 RES
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): -0.009	C	999	360
VERT(CL): 0.014	C	999	240
HORZ(LL): 0.003	C	-	-
HORZ(TL): 0.004	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.682
Max BC CSI: 0.066
Max Web CSI: 0.199

VIEW Ver: 18.02.01B.0321.11

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
G*	88	/-	/-	/37	/82	/53
H	576	/-	/-	/321	/631	/-

Wind reactions based on MWFRS
G Brg Width = 70.5 Min Req = -
H Brg Width = 1.5 Min Req = 1.5
Bearings G & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
C - D	271	-477

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
G - F	382	-3	F - D	382	-3

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
B - F	522	-378

Lumber

Top chord: 2x4 SP M-30;
Bot chord: 2x4 SP M-30;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP M-30;

Plating Notes

All plates are 2X4 except as noted.

Loading

Truss designed to support 2-0-0 top chord outlookers and cladding load not to exceed 10.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

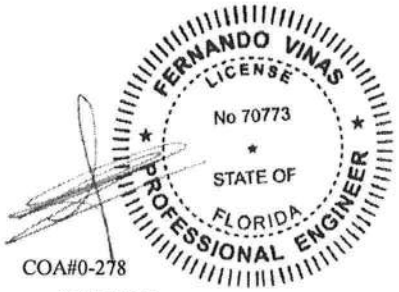
Additional Notes

Refer to General Notes for additional information

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Top Chord overhang(s) may be field trimmed.



COA#0-278

05/18/2020

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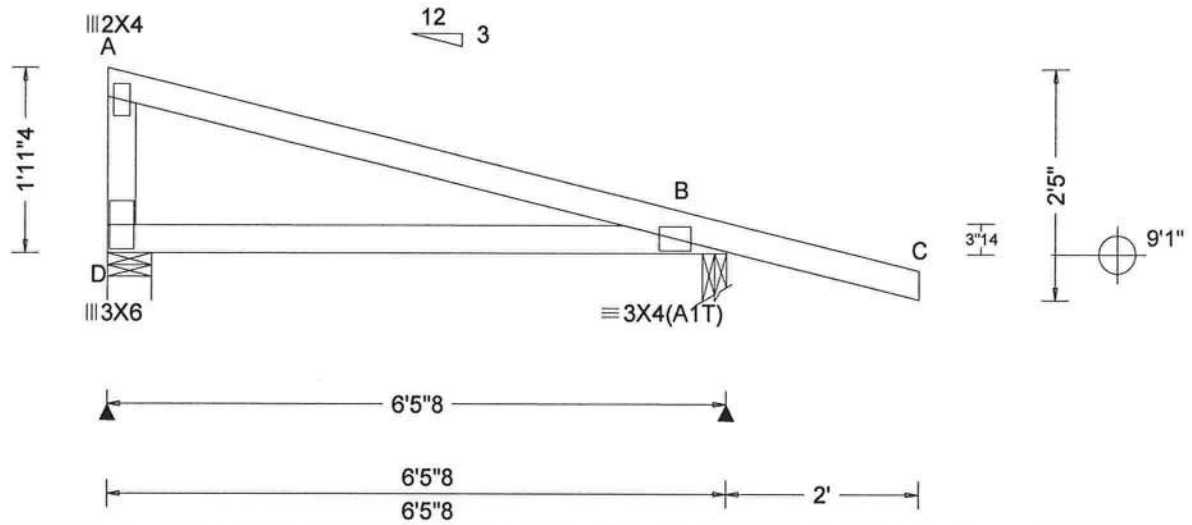
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 380530 / MONO FROM:	Ply: 1 Qty: 28	**Job Number:** 17556 :THE HART RESIDENCE **Truss Label:** C2	Cust: R 917 JRef: 1WVd9170001 T6 / DrwNo: 139.20.0509.34283 JB / DF 05/18/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.25

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Code / Misc Criteria
Bldg Code: FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.035	D	999	360
VERT(CL): 0.071	D	999	240
HORZ(LL): -0.010	D	-	-
HORZ(TL): 0.021	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.310
Max BC CSI: 0.225
Max Web CSI: 0.244

VIEW Ver: 18.02.01B.0321.11

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D	255	/-	/-	/125	/60	/60
B	408	/-	/-	/185	/99	/-

Wind reactions based on MWFRS
D Brg Width = 5.5 Min Req = 1.5
B Brg Width = 3.0 Min Req = 1.5
Bearings D & B are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP M-30;
Bot chord: 2x4 SP M-30;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

Refer to General Notes for additional information

Top Chord overhang(s) may be field trimmed.

FERNANDO VINAS
LICENSE
No 70773
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

COA#0-278

05/18/2020

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ALPINE
AN ITW COMPANY
6750 Forum Drive
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Gable Stud Reinforcement Detail

ASCE 7-10: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Stud	4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Standard	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"
	SP DFL	#1	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
		Stud	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
		Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Douglas Fir-Larch	Southern Pine***
#3	#3
Stud	Stud
Standard	Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch	Southern Pine***
#1	#1
#2	#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' braces space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) 'L' braces space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

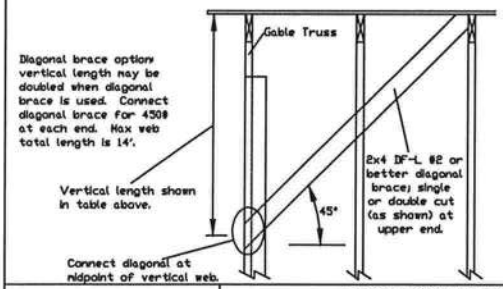
'L' bracing must be a minimum of 80% of web member length.

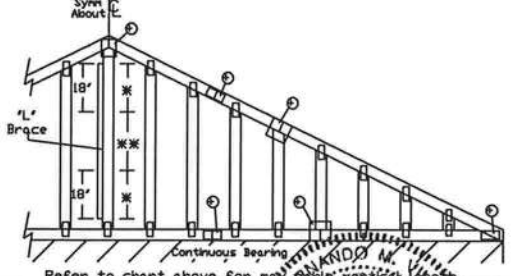
Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





Refer to chart above for max. gable vertical length.

ALPINE
AN ITW COMPANY
13723 Riverport Drive
Suite 200
Maryland Heights, MO 63043

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ALPINE: www.alpineitw.com TPI: www.tpinst.org; SBCA: www.sbcindustry.org; ICC: www.iccsafe.org

FERNANDO M. VIDAL LICENSE NUMBER
No 70773
STATE OF FLORIDA
PROFESSIONAL ENGINEER
10/1/2020

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

REF	ASCE7-10-GABI4015
DATE	10/01/14
DRWG	A14015ENC101014

COA#0-278

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

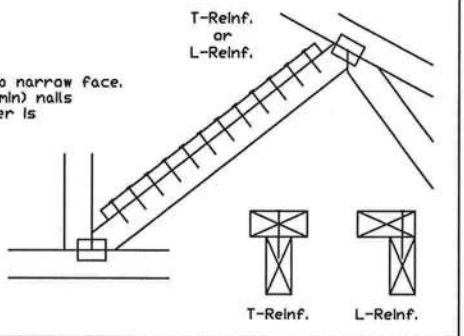
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4 (#)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6 (#)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(#) Center scab on wide face of web. Apply (1) scab to each face of web.

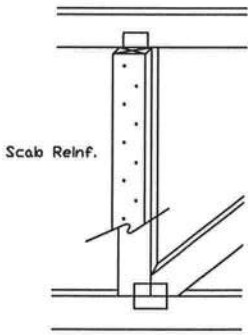
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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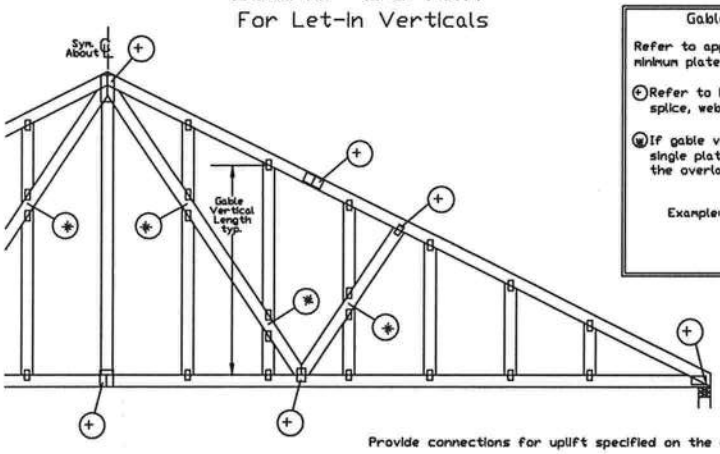
13723 Riverport Drive
Suite 200
Maryland Heights, MO 63043

FERNANDO M. VINAS
LICENSE
No 70773
STATE OF FLORIDA
PROFESSIONAL ENGINEER
03/18/2020

TC LL	PSF	REF	CLR Subst.
TC DL	PSF	DATE	01/02/19
BC DL	PSF	DRWG	BRCLBSUB0119
BC LL	PSF		
TOT. L.D.	PSF		
DUR. FAC.			
SPACING			

COA#0-278

Gable Detail
For Let-In Verticals



Gable Truss Plate Sizes

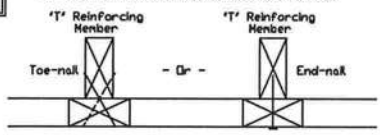
Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

Ⓦ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example: 2X4 2X4 2X8

'T' Reinforcement Attachment Detail



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

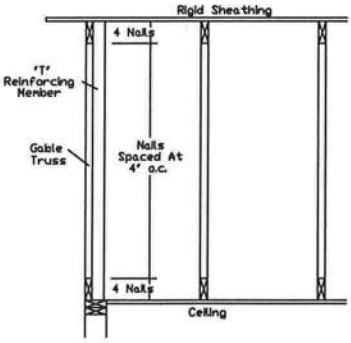
Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24'o.c. SP #3
'T' Reinforcing Member Size = 2x4
'T' Brace Increase (From Above) = 30% = 1.30
(1) 2x4 'L' Brace Length = 8' 7"
Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"



Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:
10d Common (0.148"x 3.",min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118, A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118, S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118, S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118, S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118, S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

FERNANDO VINAS LICENSE No 70773 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/18/2020
COA#0-278

ALPINE AN ITW COMPANY
13723 Riverport Drive
Suite 200
Maryland Heights, MO 63043

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-I for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpinebtw.com/ TPI: www.tpinst.org/ SBCA: www.sbcindustry.org/ ICC: www.iccsafe.org

MAX. TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX. SPACING	24.0"

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118