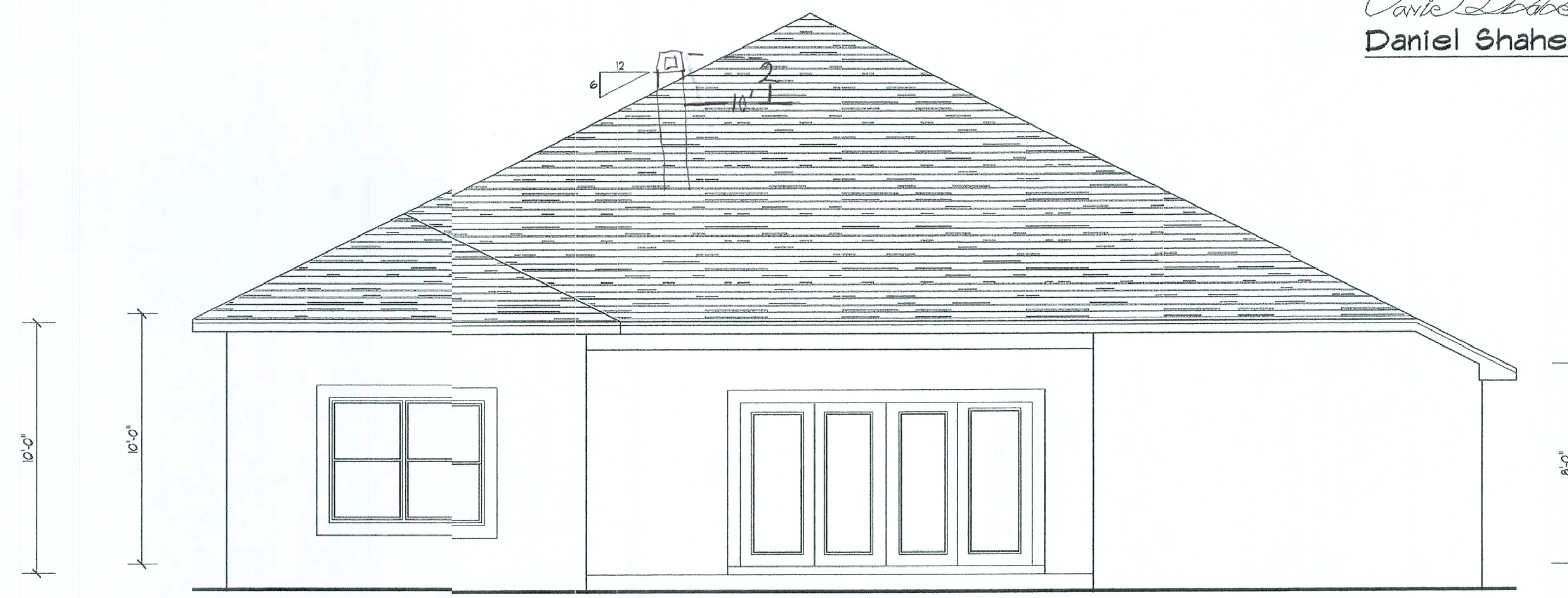


ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS



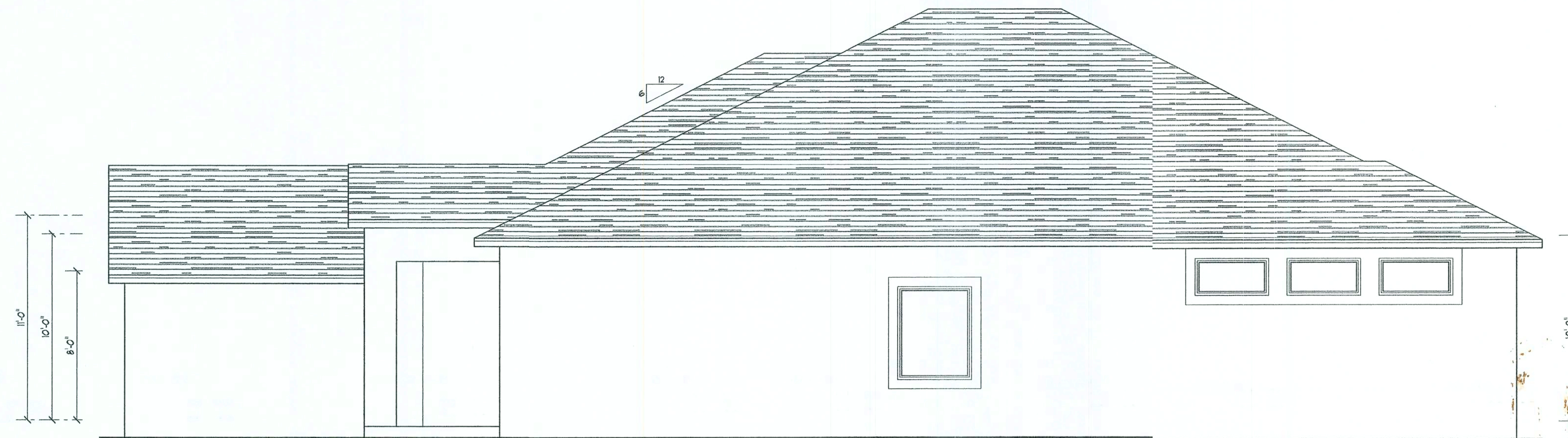
FRONT ELEVATION
SCALE: 1/4" = 1'



REAR ELEVATION
SCALE: 1/4" = 1'



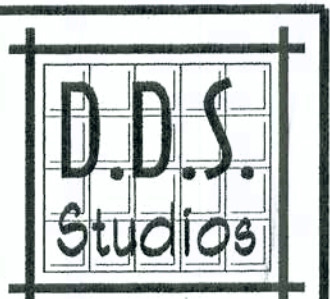
LEFT ELEVATION
SCALE: 1/4" = 1'



RIGHT ELEVATION
SCALE: 1/4" = 1'

Daniel Shaheen
Daniel Shaheen

February 05, 2004



ARCHITECTURAL
DESIGN

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(386) 754-0181

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POB 868, LAKE CITY, FL 32056
PH: (804) 754-5419
PH: (804) 754-6749
FLORIDA P.E. 53915

ENGINEERED BY:

A CUSTOM HOME BY DON REED CONSTRUCTION

MEHALKO RESIDENCE

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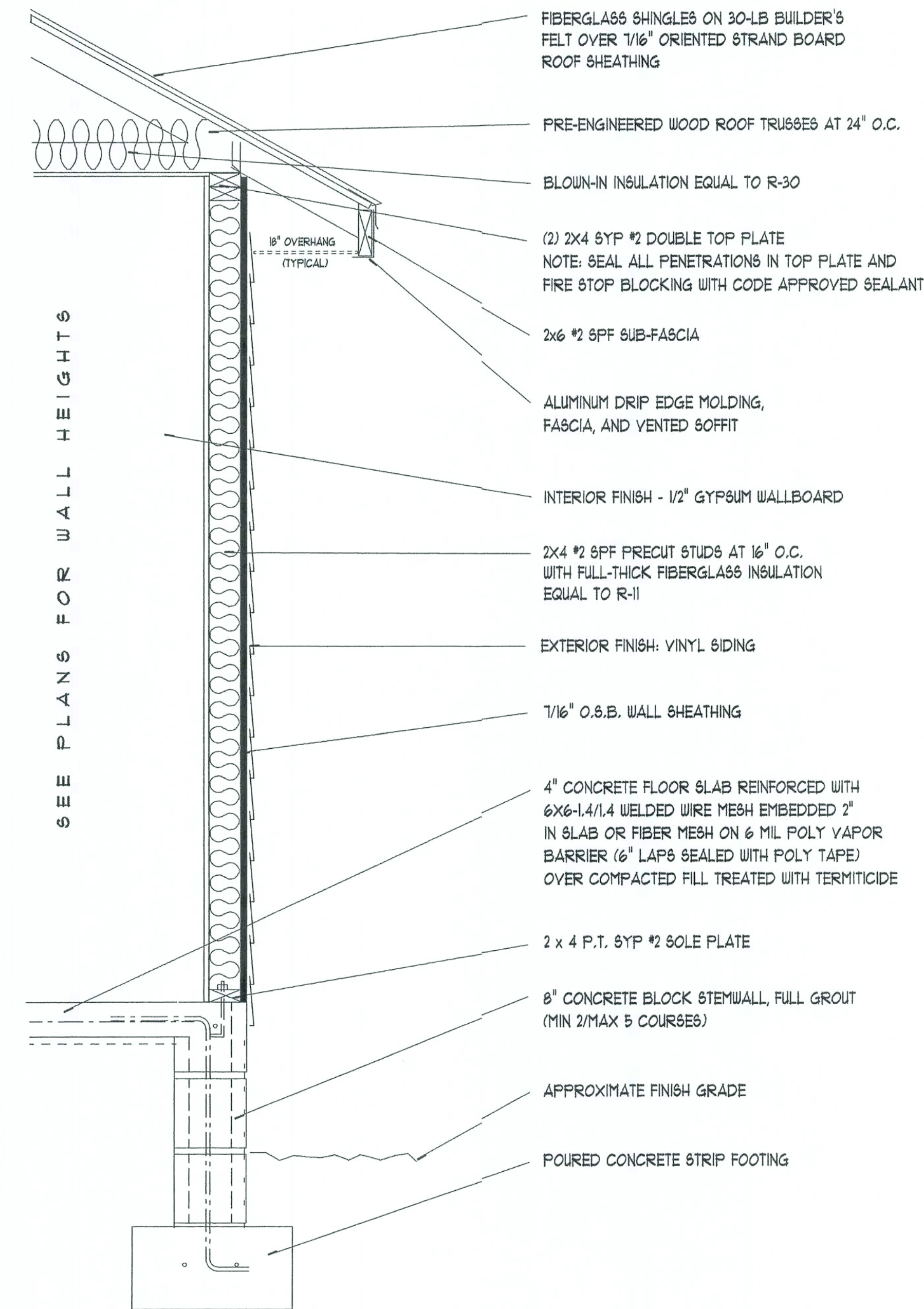
EXTERIOR ELEVATIONS

SHEET NUMBER
1 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances.
Contractor shall verify all dimensions prior to commencing construction.

21650 / 245

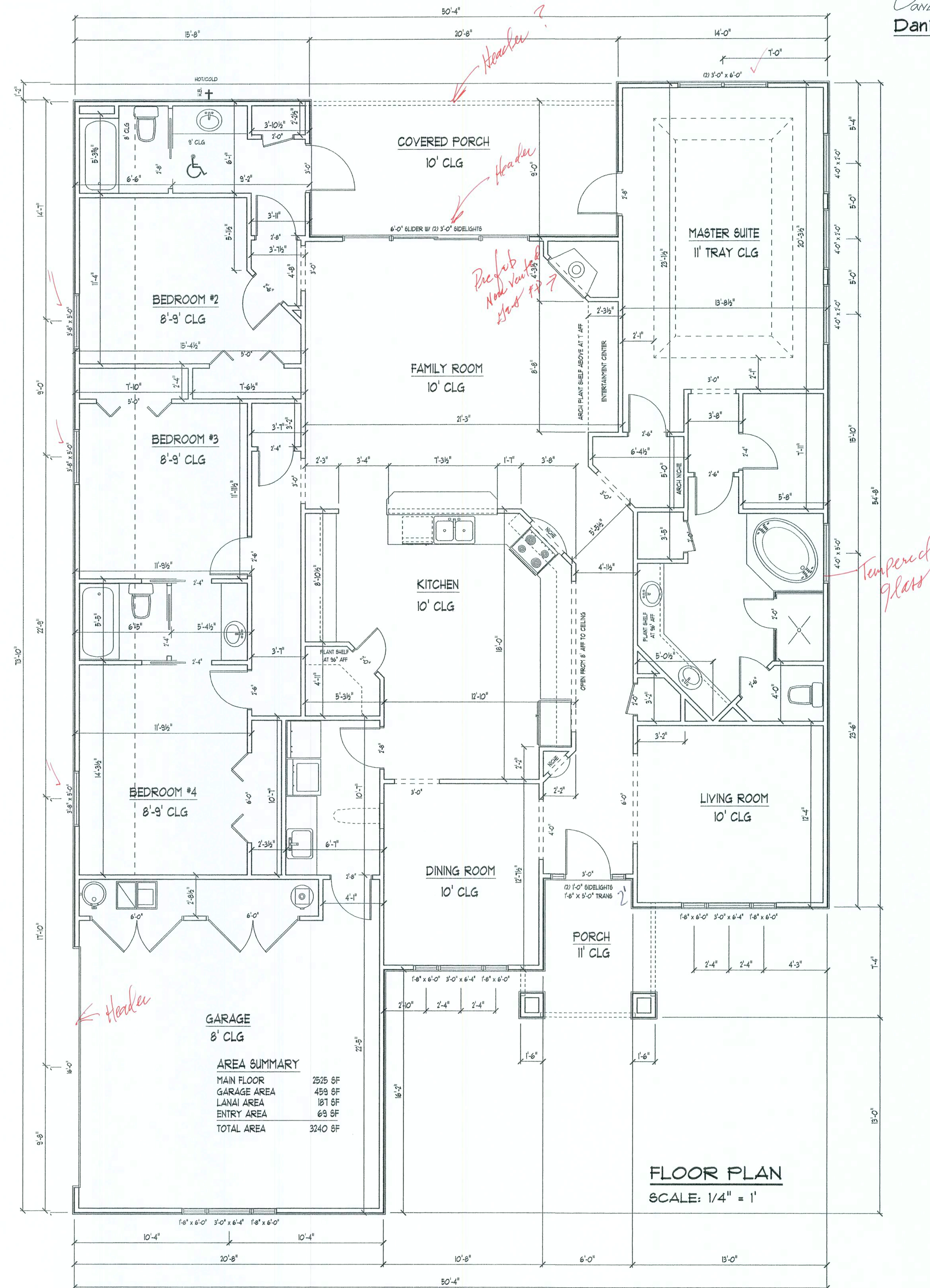
ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS



TYPICAL WALL SECTION

SCALE: 1" = 1'0"

REFER TO STRUCTURAL PAGE FOR
STRUCTURAL SPECIFICATIONS



Daniel Shaheen
Daniel Shaheen

February 05, 2004



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FLOOR PLAN

TYPICAL WALL SECTION

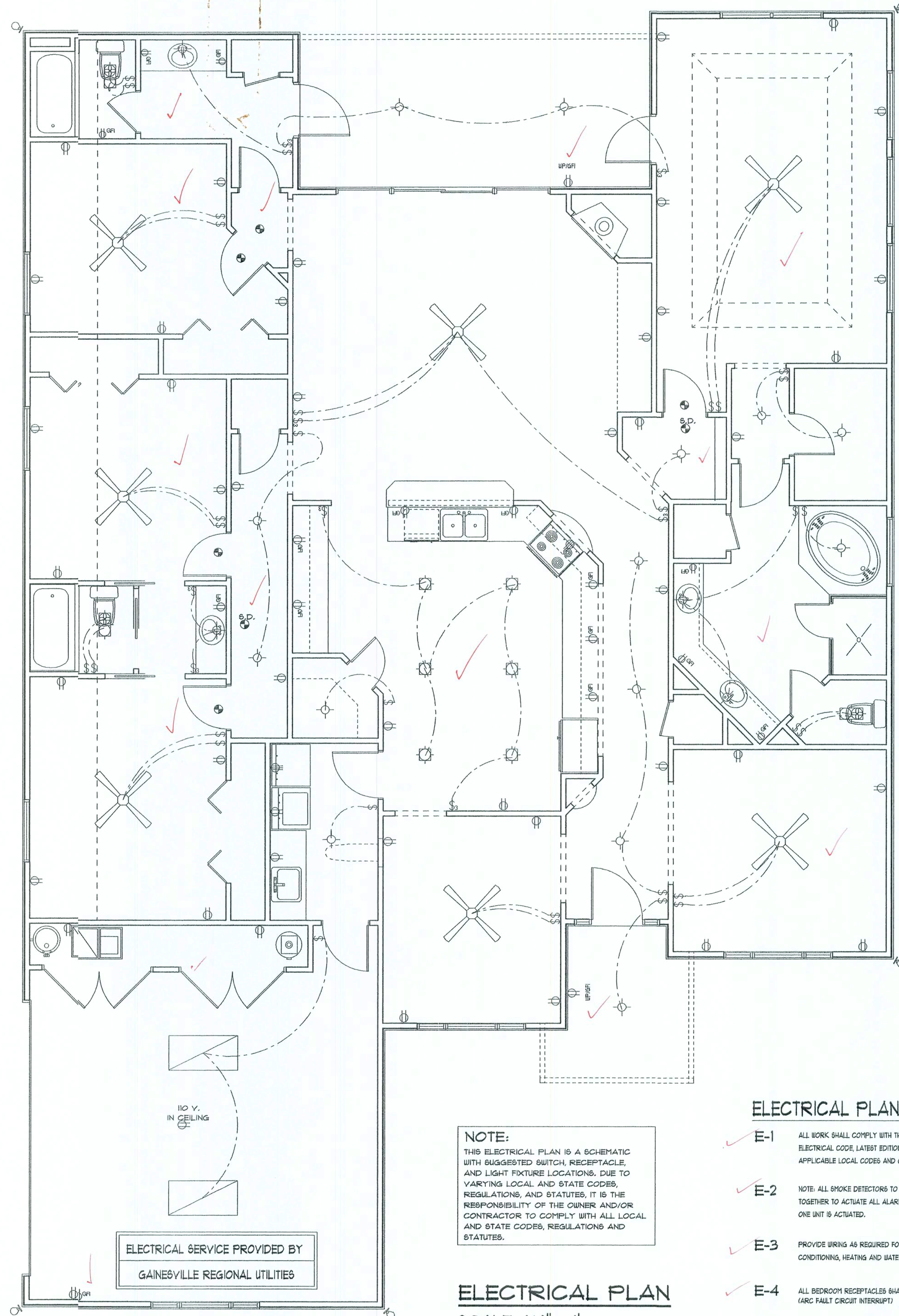
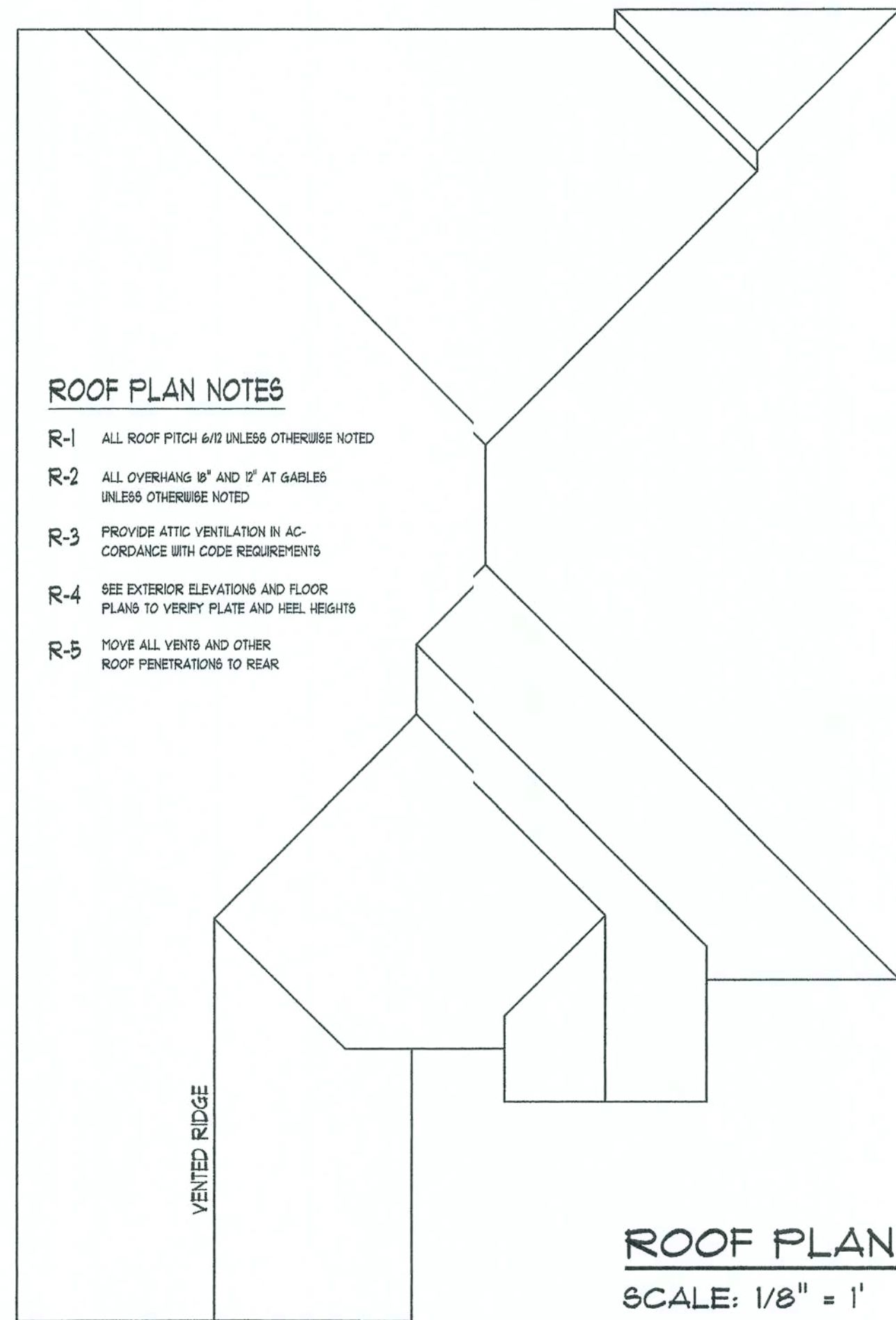
SHEET NUMBER

2 of 3

All work shall comply with
the standard building code,
and all applicable local
codes and ordinances.

Contractor shall verify all
dimensions prior to
commencing construction.

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

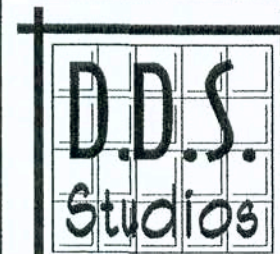


ELECTRICAL PLAN NOTES

- ✓ E-1 ALL WORK SHALL COMPLY WITH THE NATIONAL ELECTRICAL CODE, LATEST EDITION, AND ALL OTHER APPLICABLE LOCAL CODES AND ORDINANCES.
- ✓ E-2 NOTE: ALL SMOKE DETECTORS TO BE USED TOGETHER TO ACTIVATE ALL ALARMS IF ANY ONE UNIT IS ACTIVATED.
- ✓ E-3 PROVIDE WIRING AS REQUIRED FOR APPLIANCES, AIR CONDITIONING, HEATING AND WATER HEATING EQUIPMENT.
- ✓ E-4 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT).

Daniel Shahsen
Daniel Shahsen

February 05, 2004



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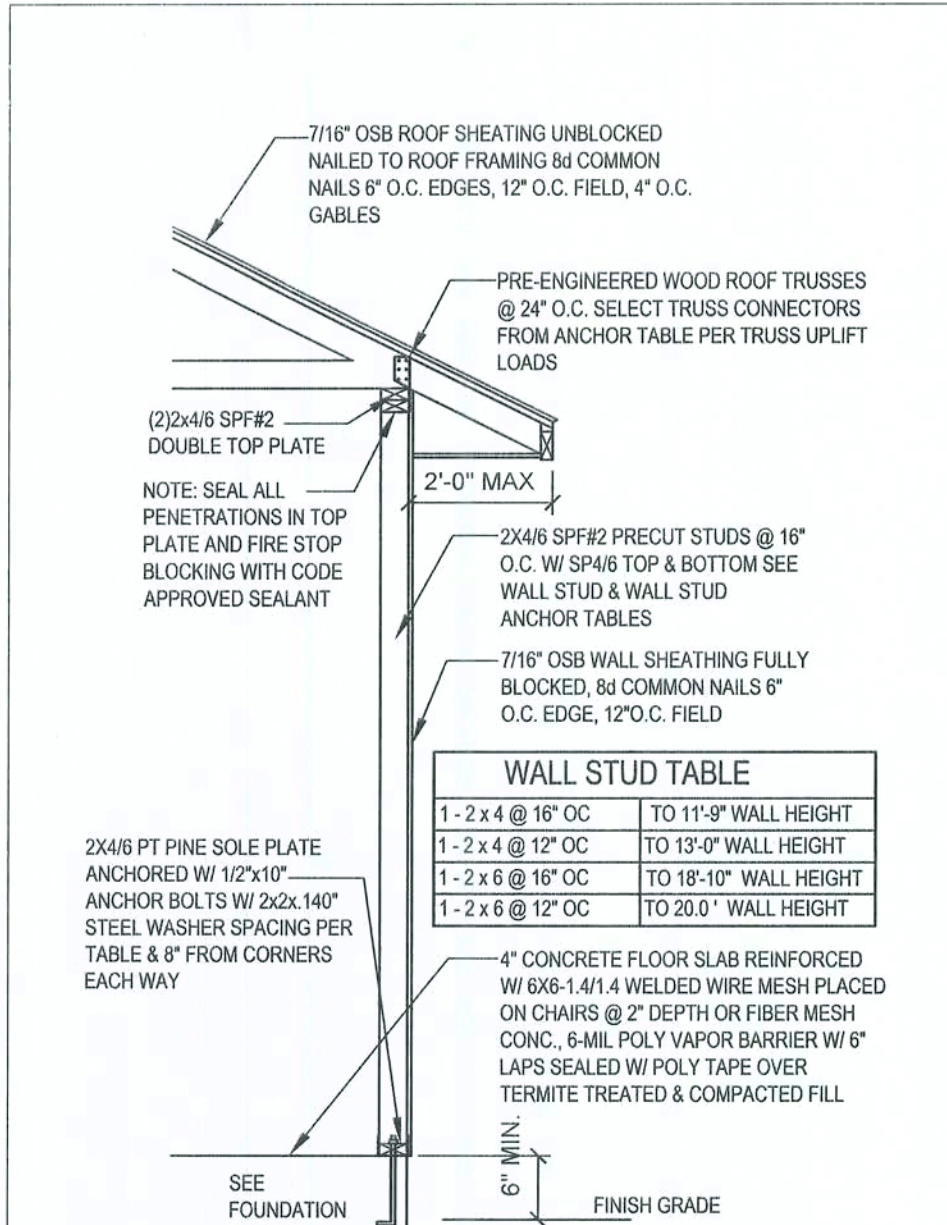
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ELECTRICAL PLAN

ROOF PLAN

SHEET NUMBER
3 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances.
Contractor shall verify all dimensions prior to commencing construction.

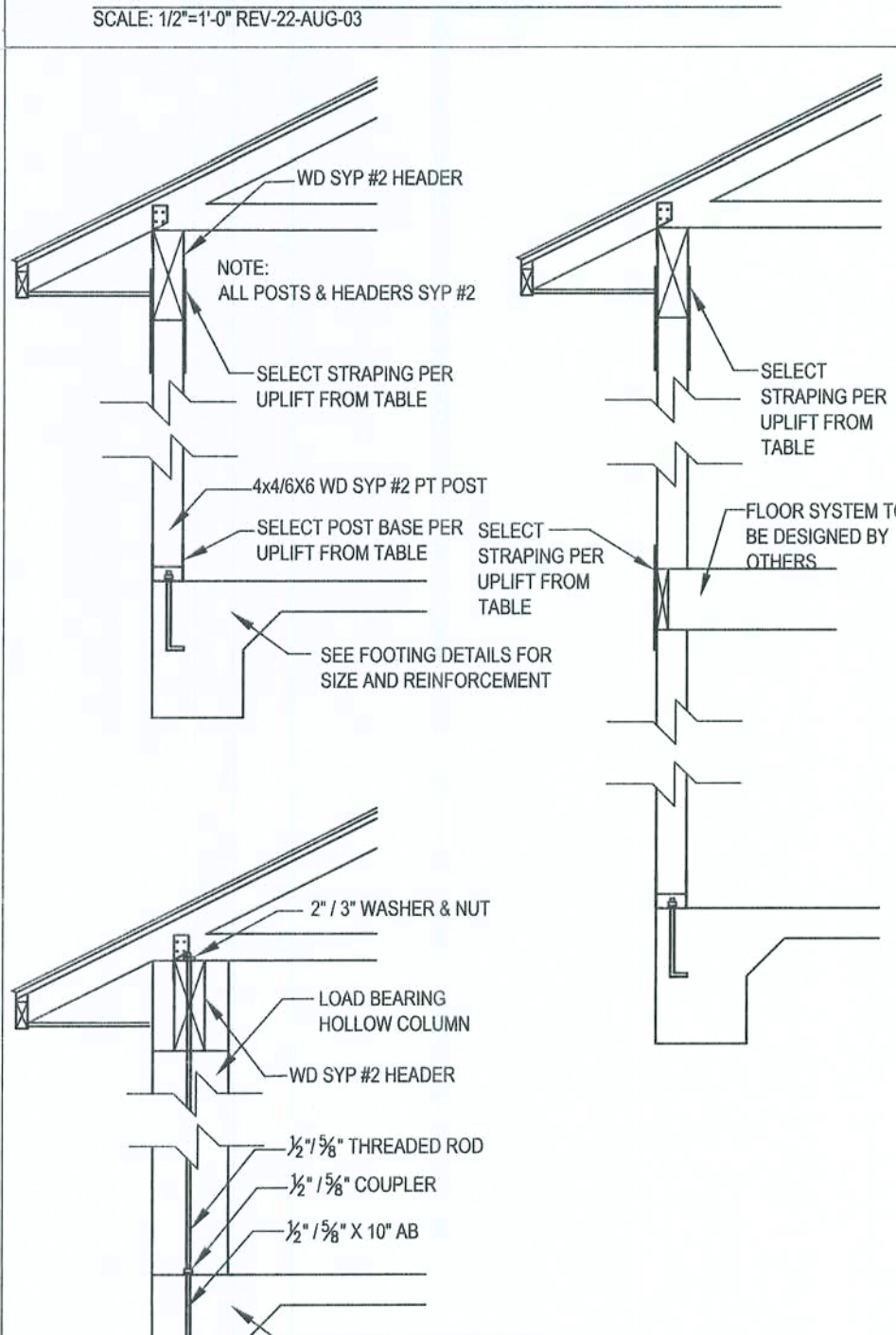


STUD ANCHOR TABLE

TYPICAL TRUSS UPLIFT & MAX 12' 0" WALL HEIGHT	ANCHOR BOLT SPACING	SP4 / SP6 SPACING	ALTERNATE STRAPPING SPACING
770 LB	48" O.C.	48" O.C.	NA
890 LB	48" O.C.	32" O.C.	NA
1270 LB	32" O.C.	16" O.C.	32" O.C.
1590 LB	24" O.C.	16" O.C.	16" O.C.
2280 LB	LTT101 W/ 5/8" X 7" WEDGE ANCHOR	NA	(2) HTS20 NAILED TO STUD PACK

NOTE: SP2 TOP & SP1 BOTTOM ALTERNATE FOR SP4/6

NOTE: MINIMUM ANCHOR BOLT SPACING FOR WALLS WITH A HEIGHT GREATER THAN 10'-0" AND LESS THAN 14'-0" SHALL BE 32" O.C.

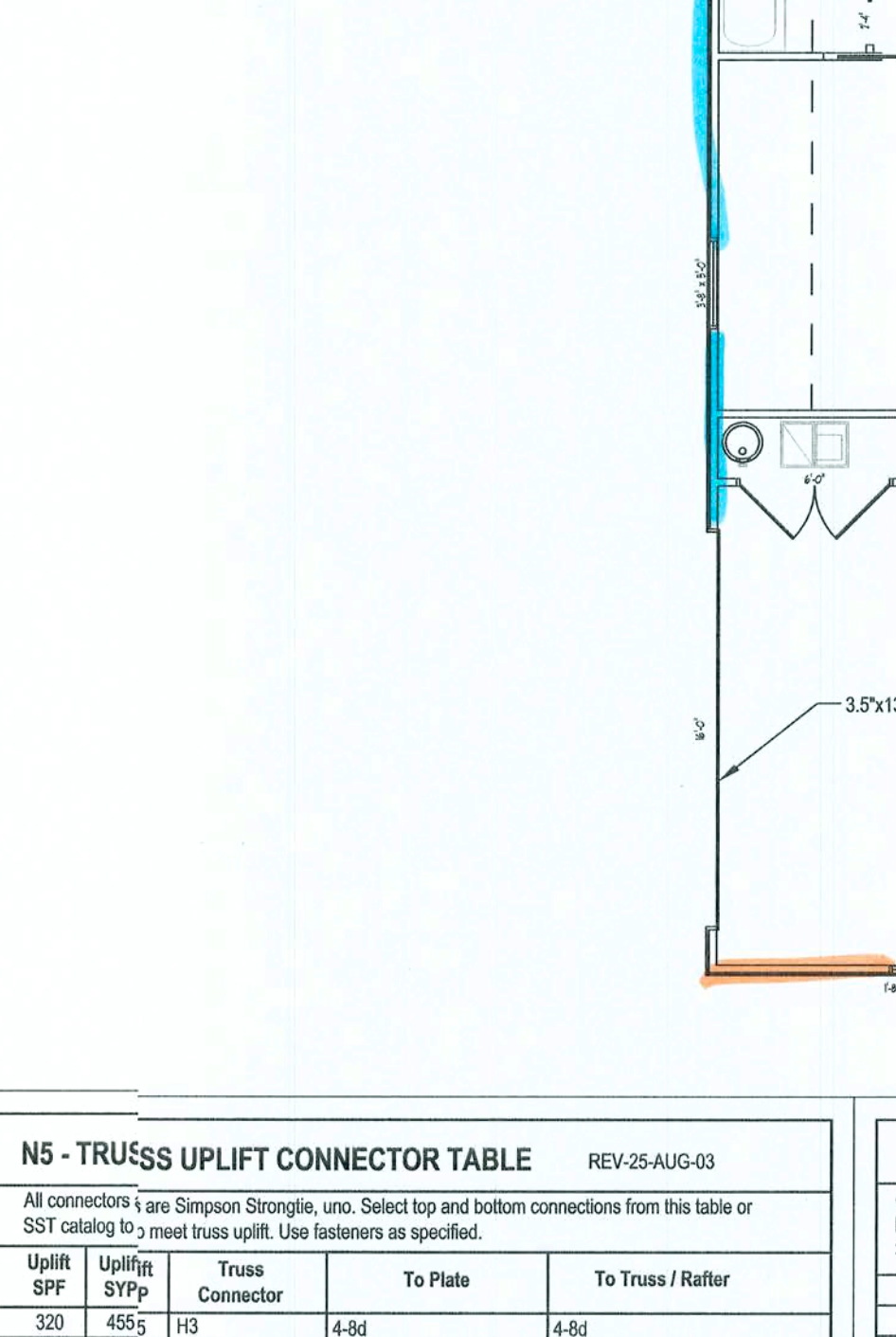
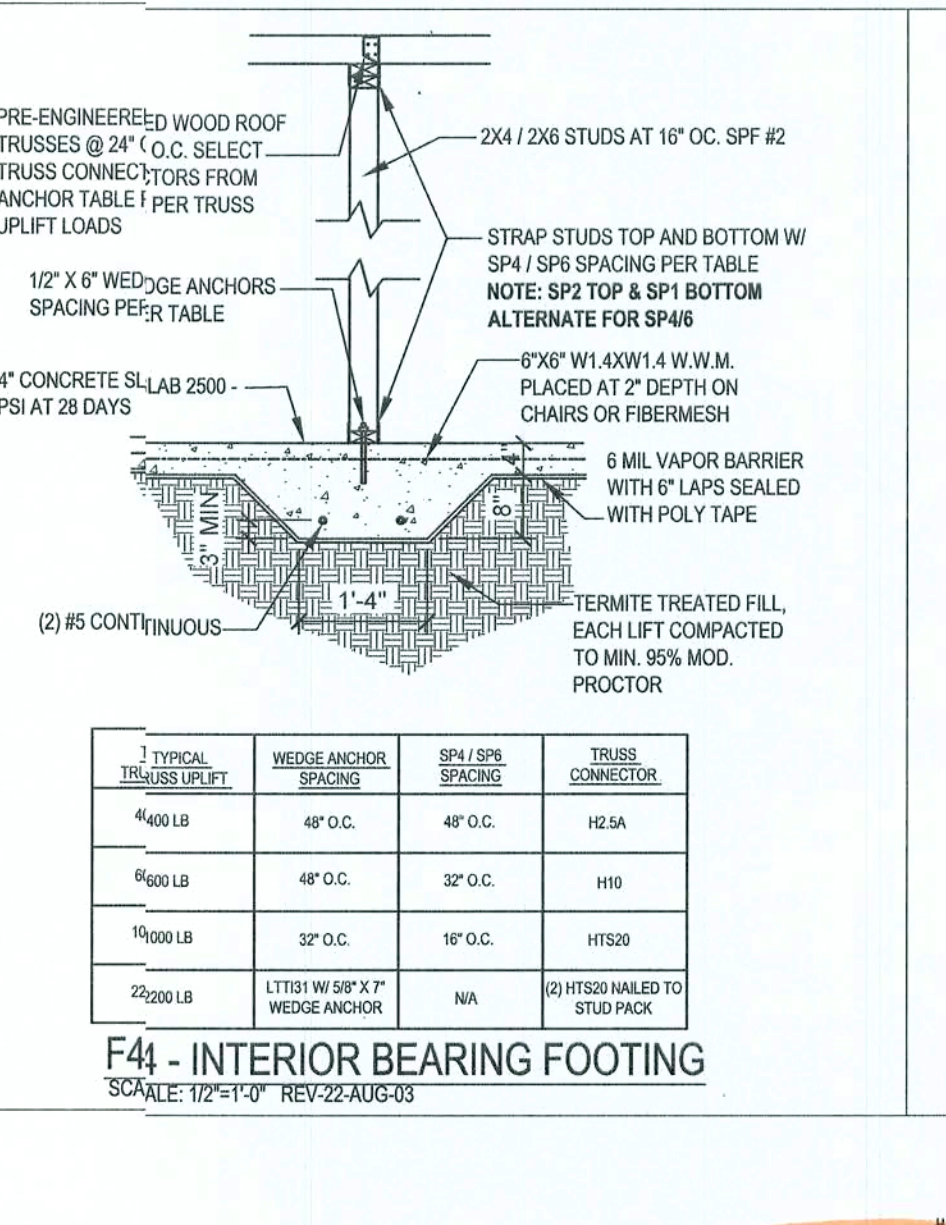
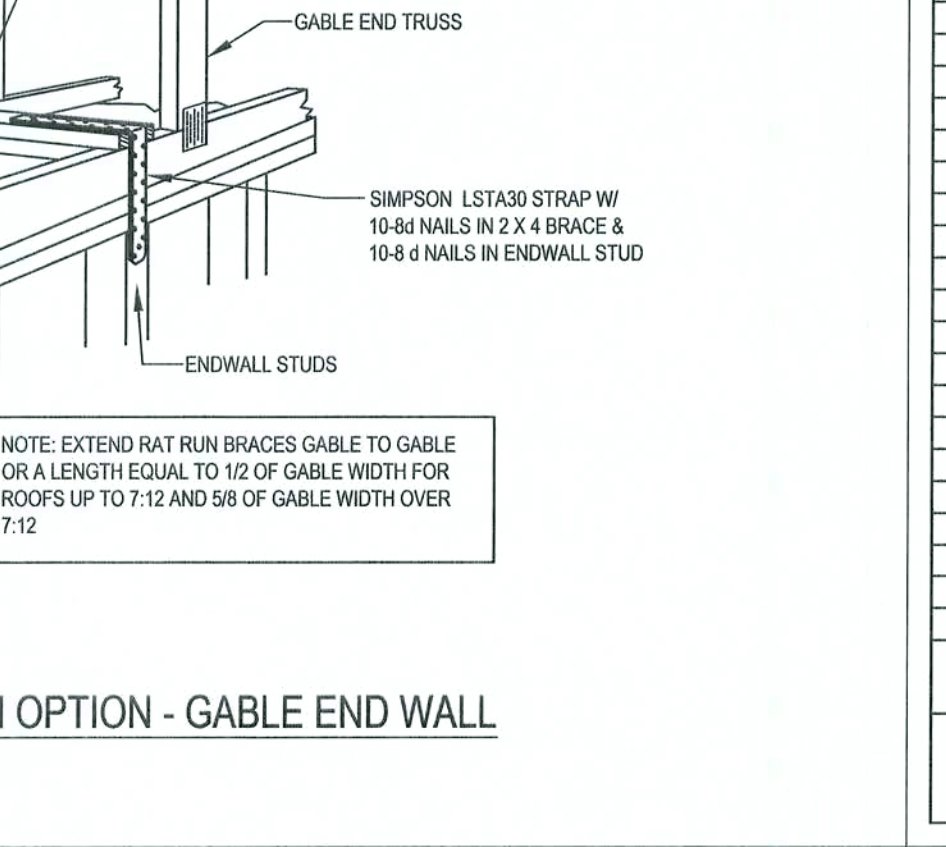
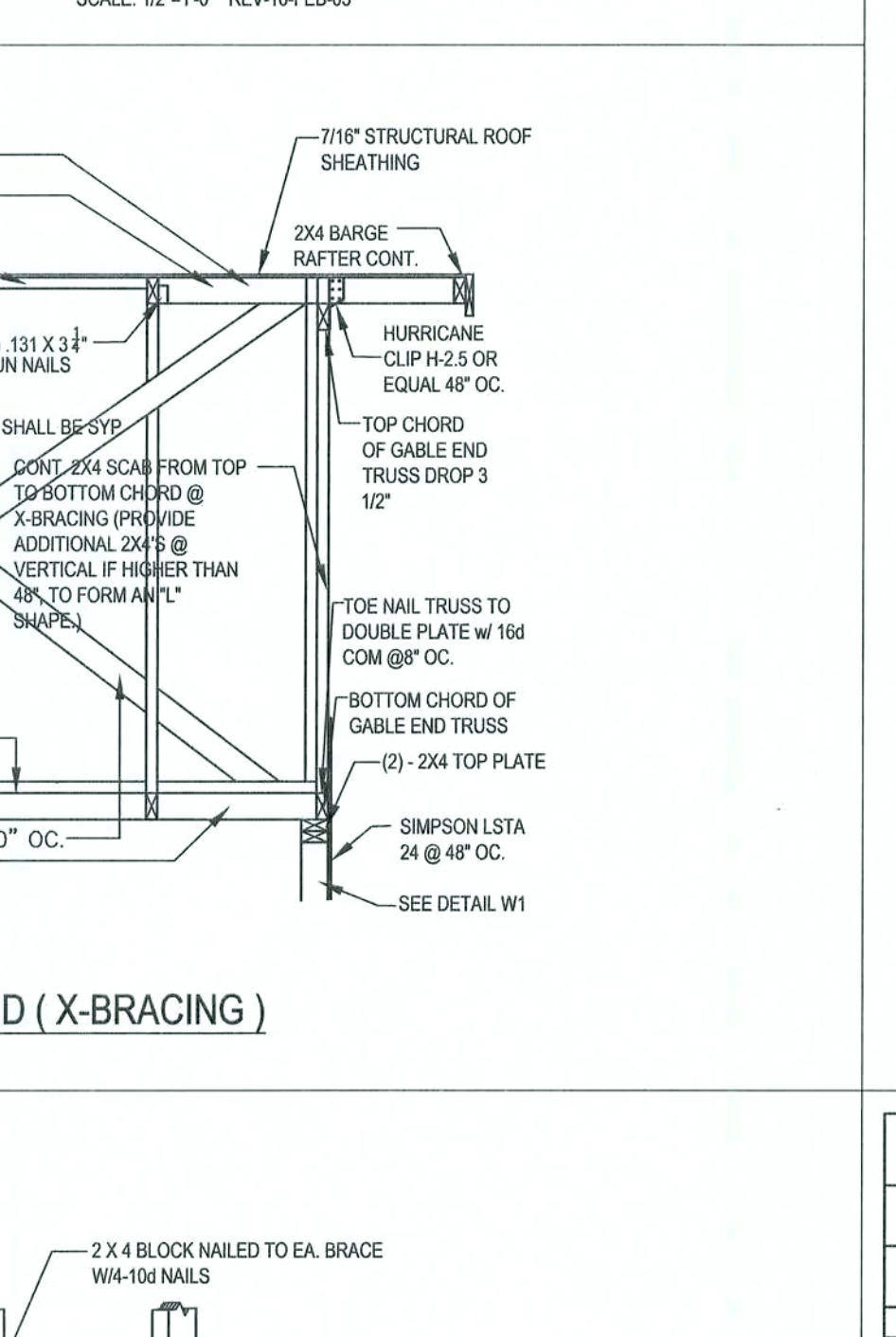
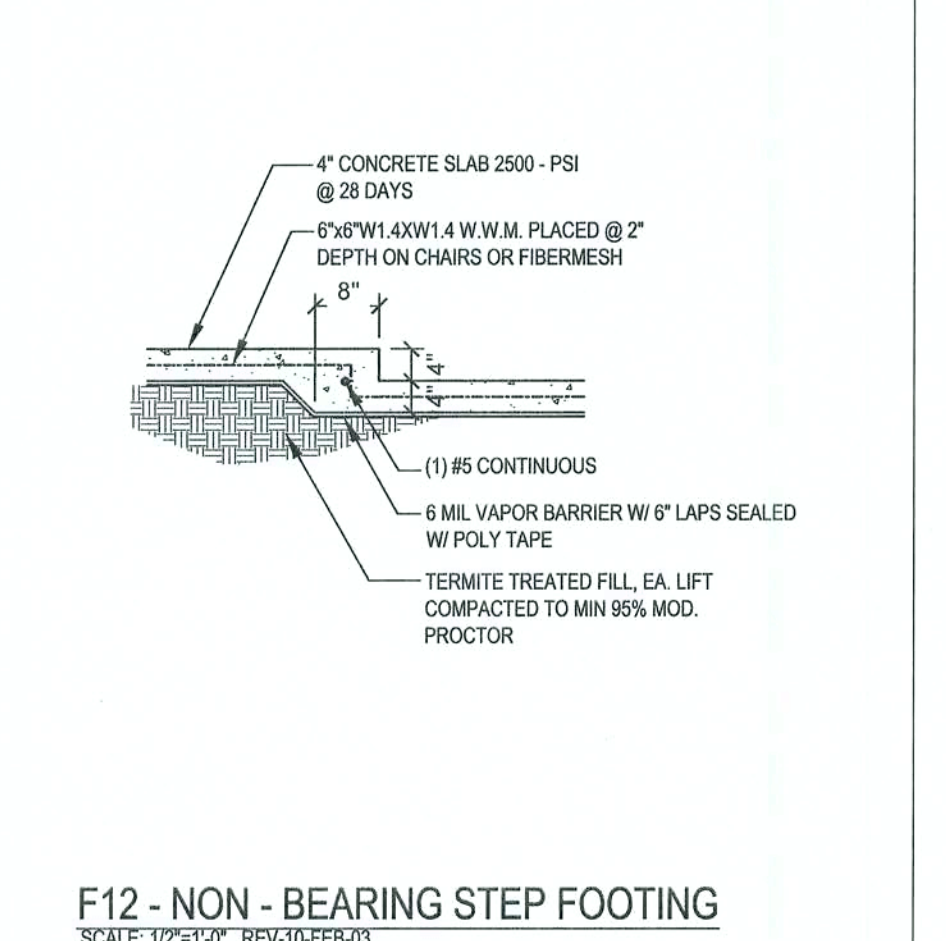
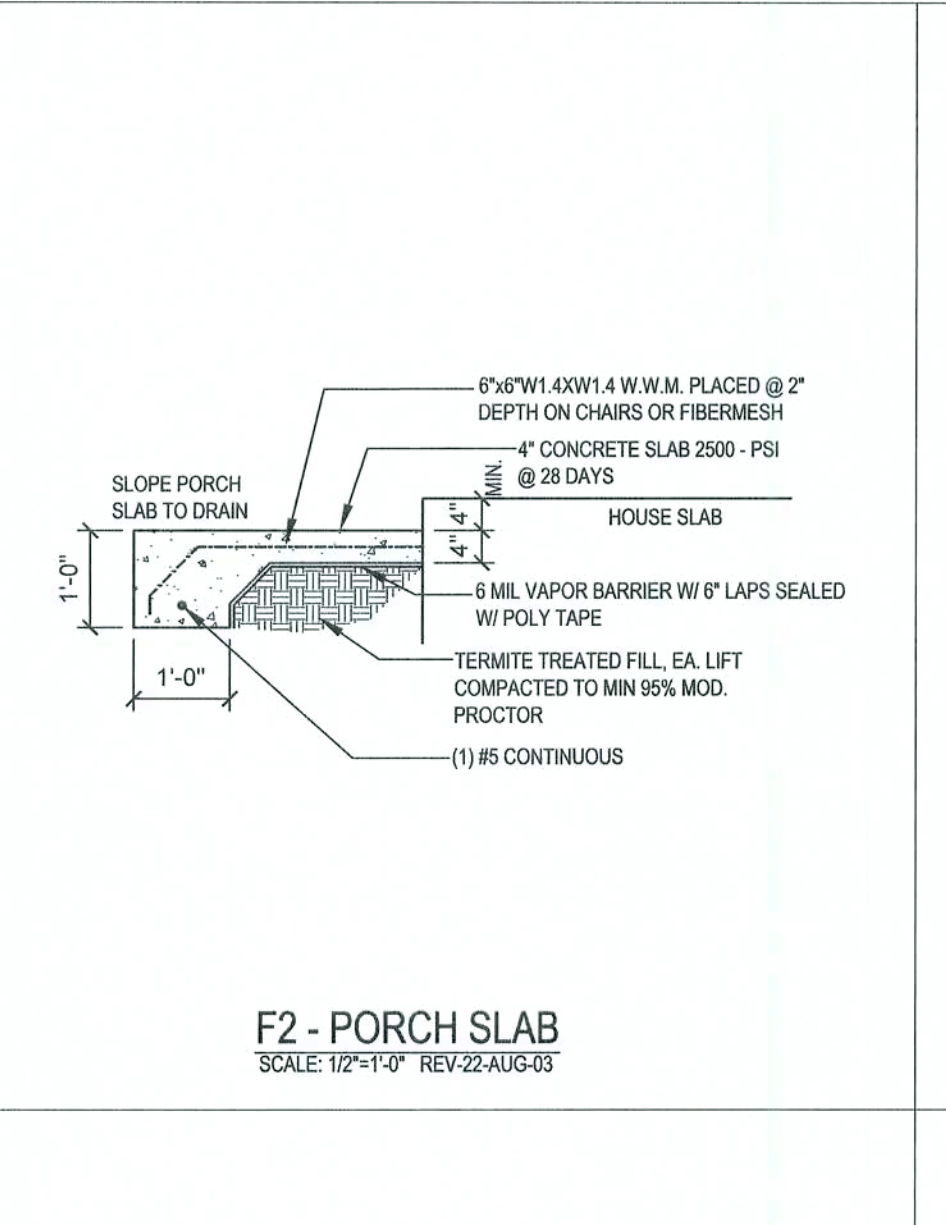
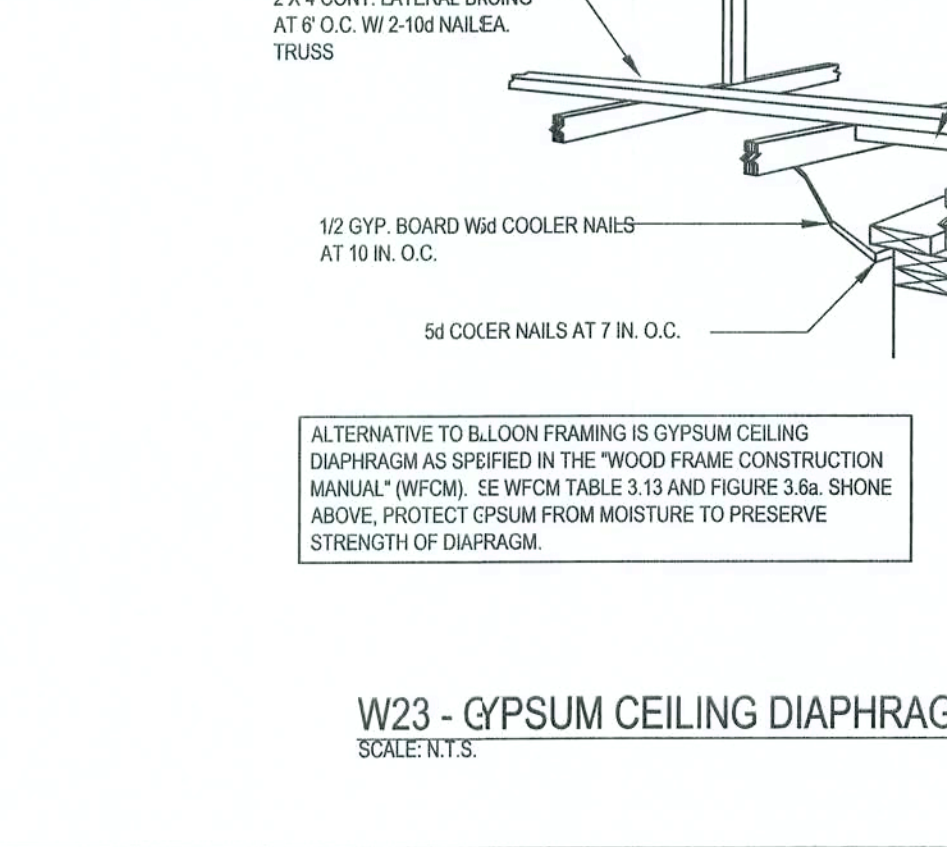
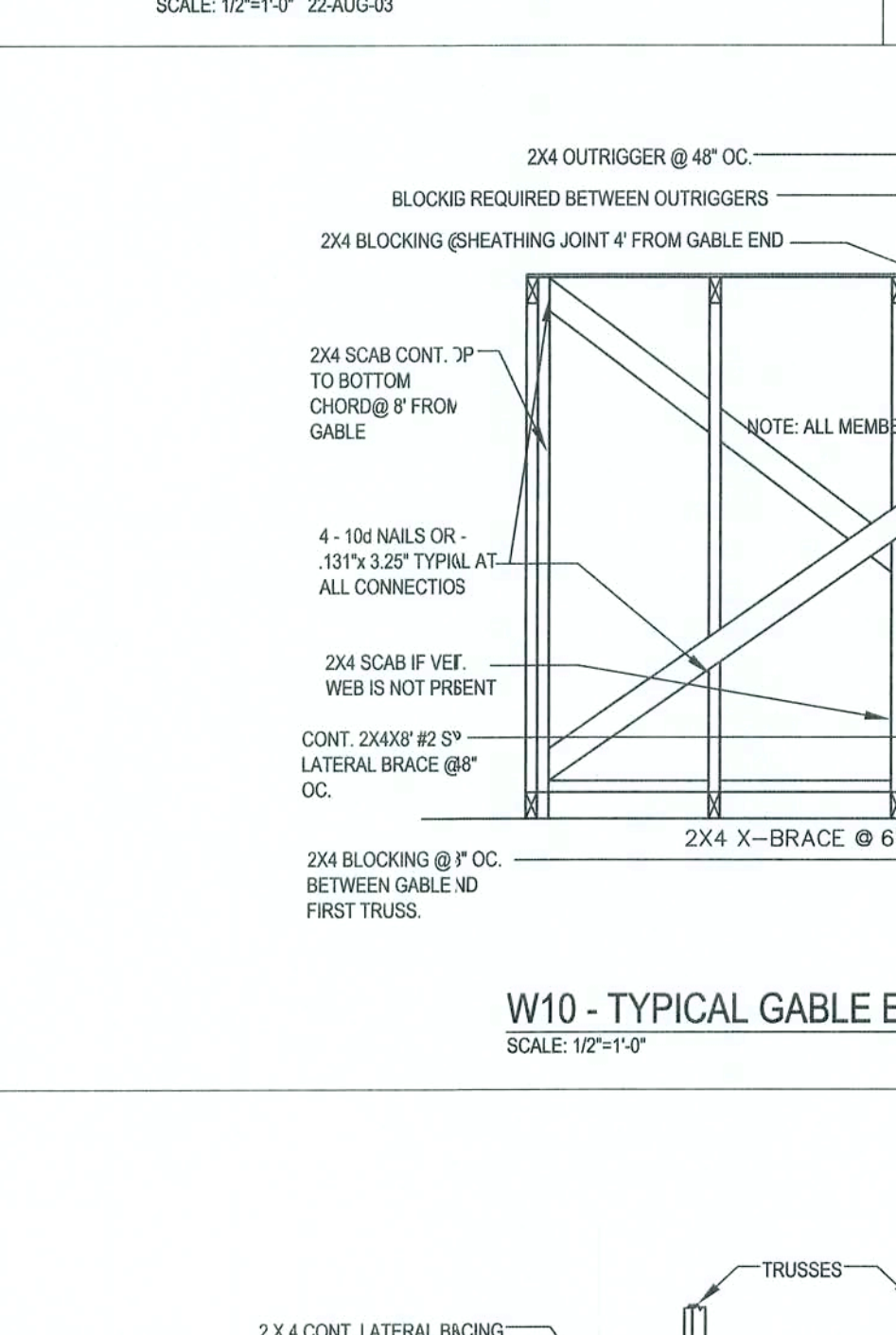
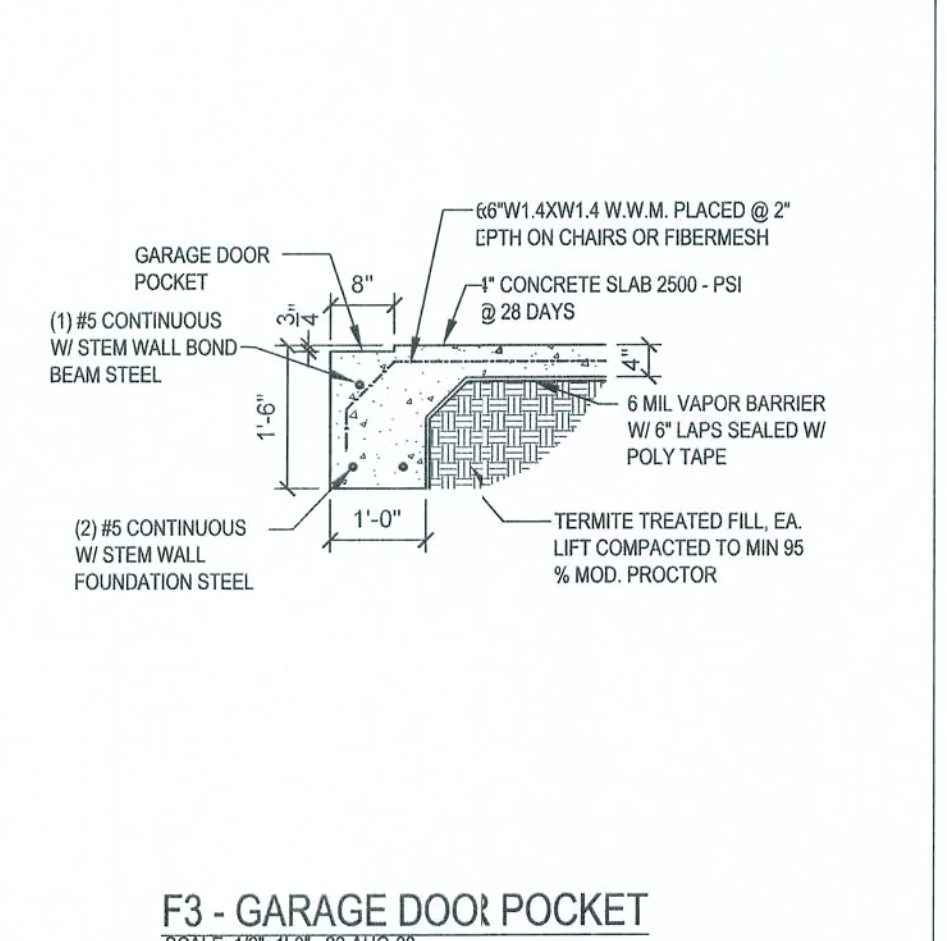
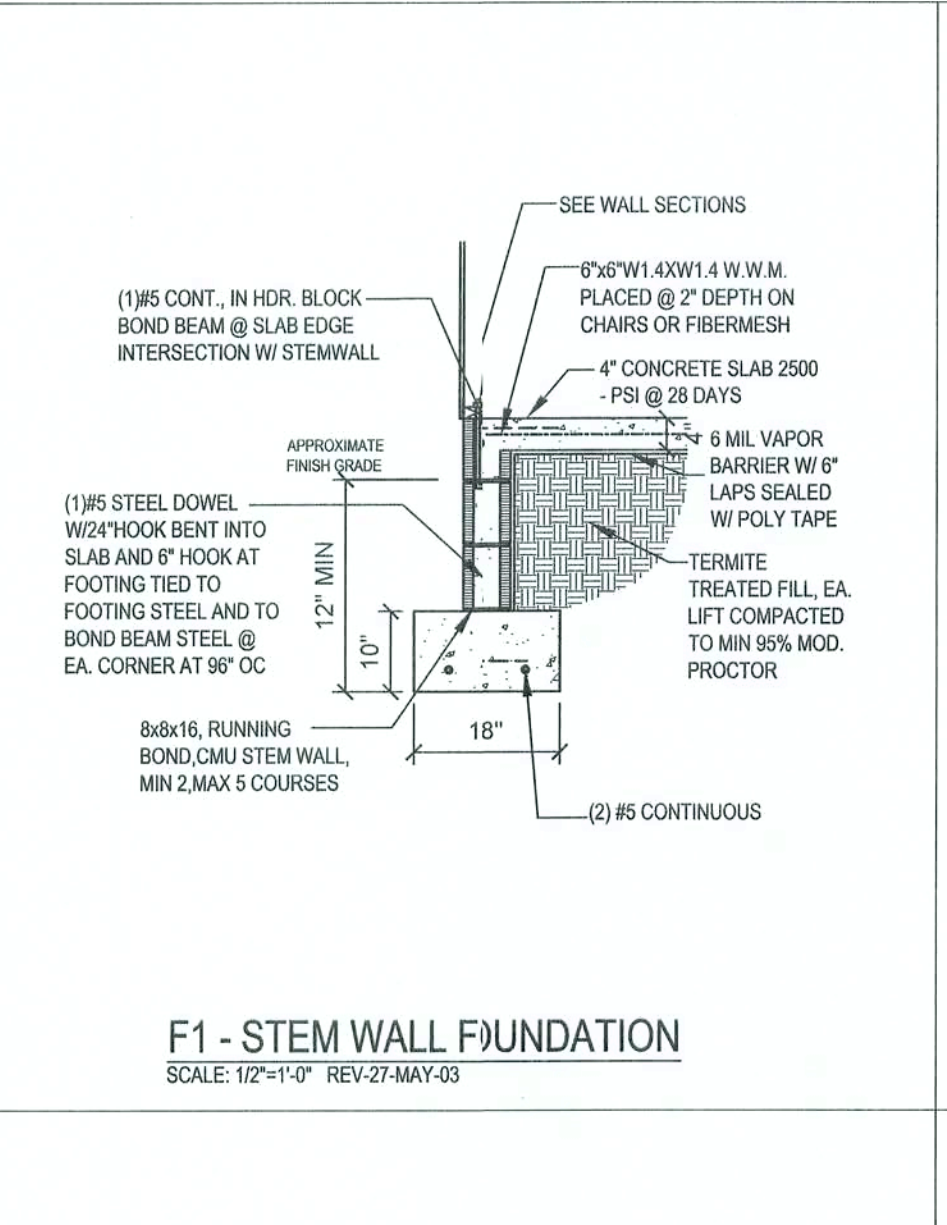


W12 - PORCH HEADER ANCHORS
SCALE: 1/2"=1'-0" REV-18-JUL-03

TYPICAL TRUSS UPLIFT	POST BASE ANCHOR	BETWEEN FLOOR STRAPPING	HEADER STRAPPING
555 LB	ABW4 W/ (8) 10x16 4x8	(2) LST1021 W/ (8) 10x16 EA	(2) LST1021 W/ (8) 10x16 EA
720 LB	ABW4 W/ (8) 10x16 4x8	(2) LST1021 W/ (8) 10x16 EA	(2) LST1021 W/ (8) 10x16 EA
2260 LB	ABW4 W/ (2) 10x16 4x8	(2) LST1021 W/ (8) 10x16 EA	(2) LST1021 W/ (8) 10x16 EA
2300 LB	ABW4 W/ (2) 10x16 4x8	(2) LST1021 W/ (8) 10x16 EA	(2) LST1021 W/ (8) 10x16 EA

HOLLOW COLUMN

TYPICAL TRUSS UPLIFT	POST BASE ANCHOR	BETWEEN FLOOR STRAPPING	HEADER STRAPPING
1900 LB	8" X 10" AB ATTACHED TO 7/8" THREADED ROD WITH 7/8" COUPLER	(2) LST1021 W/ (8) 10x16 EA	(2) LST1021 W/ (8) 10x16 EA
2300 LB	8" X 10" AB ATTACHED TO 7/8" THREADED ROD WITH 7/8" COUPLER	(2) LST1021 W/ (8) 10x16 EA	(2) LST1021 W/ (8) 10x16 EA



N5 - TRUSSES UPLIFT CONNECTOR TABLE REV-25-AUG-03

All connectors are Simpson Strong-Tie, Inc. Select top and bottom connections from this table or SST catalog to meet truss uplift. Use fasteners as specified.

Uplift SPF	Uplift SYP#	Truss Connector	To Plate	To Truss / Rafter
320	455	H3	4-8d	4-8d
245	350	H5A	3-8d	3-8d
535	800	H2.5A	5-8d	5-8d
620	720	H10	6-10d x 1 1/2"	6-10d x 1 1/2"
850	990	LTS12	9-8d x 1 1/2"	9-8d x 1 1/2"
1245	1450	HTS20	10-10d or 12-10d x 1 1/2"	10-10d or 12-10d x 1 1/2"
1265	1470	H16, H16.2	10-10d x 1 1/2"	2-10d x 1 1/2"
1765	2050	LG12	14-10d Sinker	16-16d Sinker
3655	4200	MG1	3/4" Thd. Rod	22-10d

SPF SYP# Strap Connector

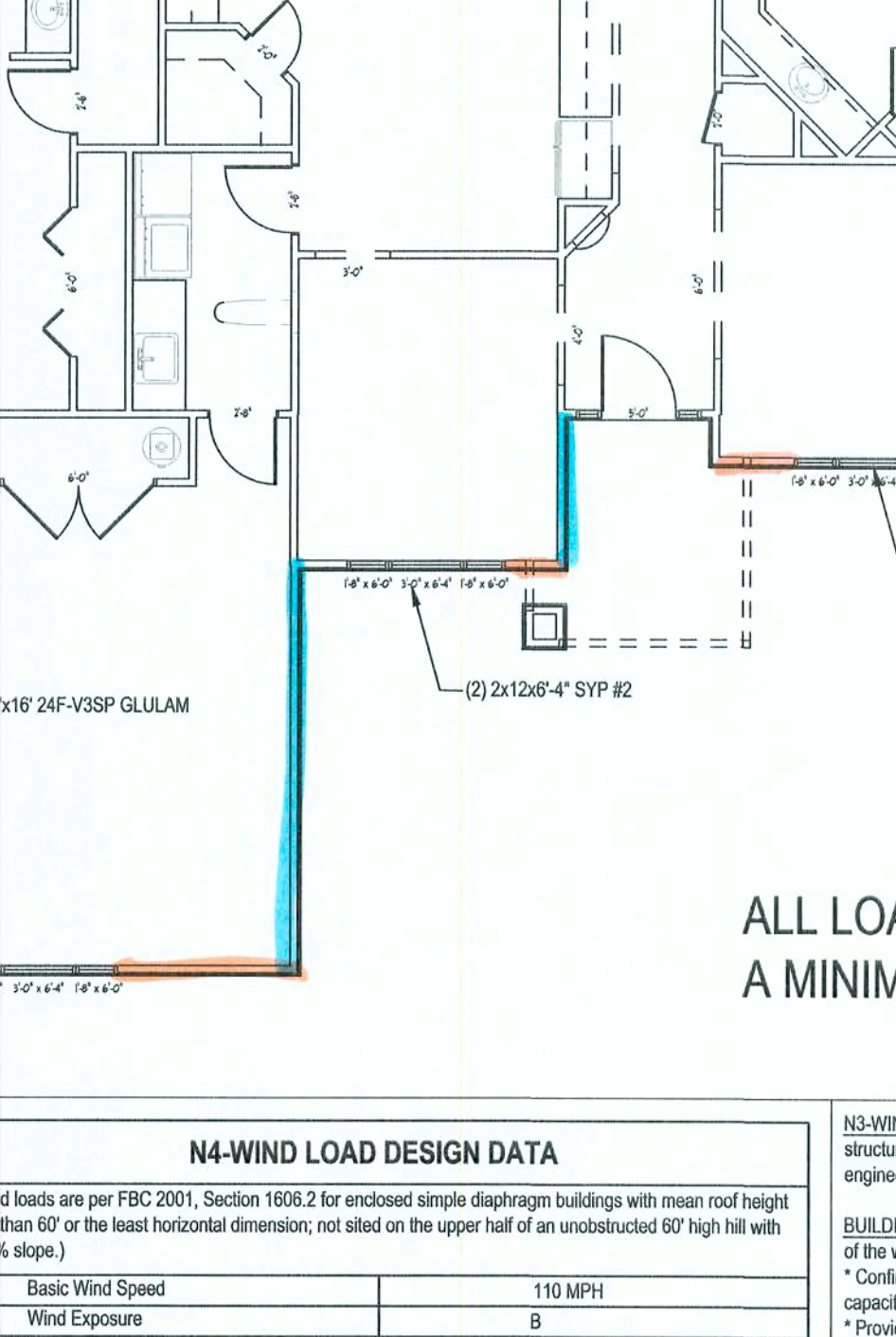
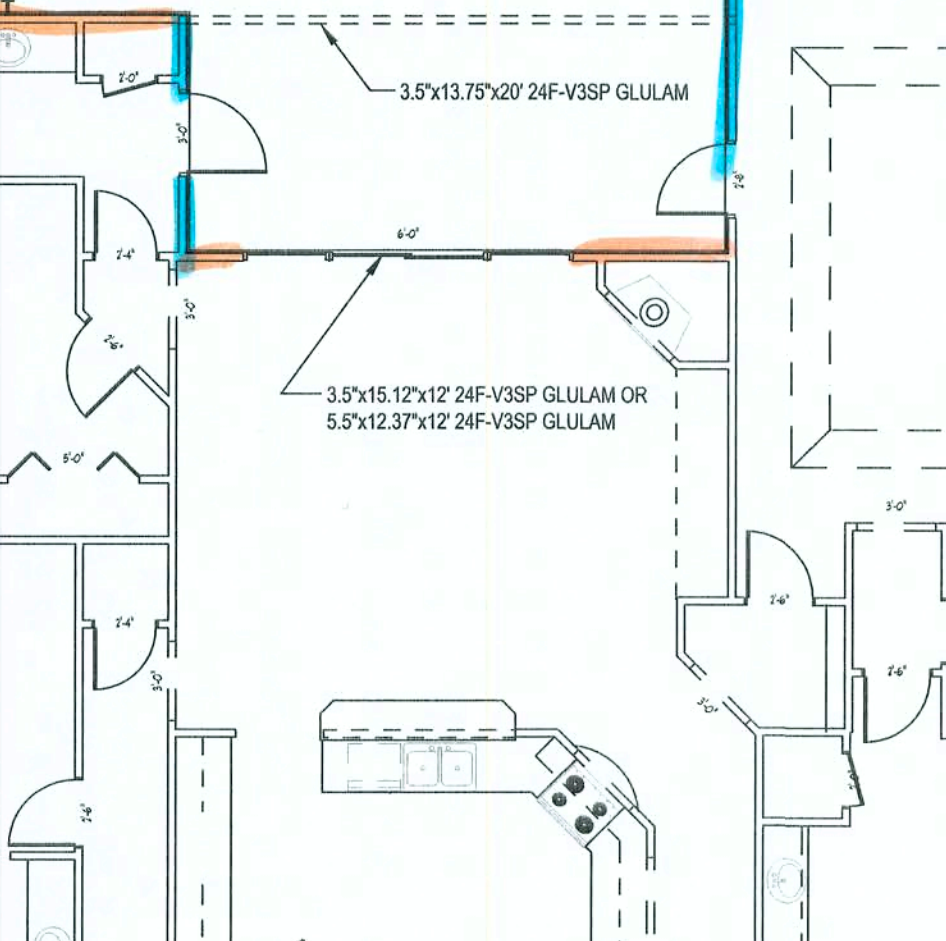
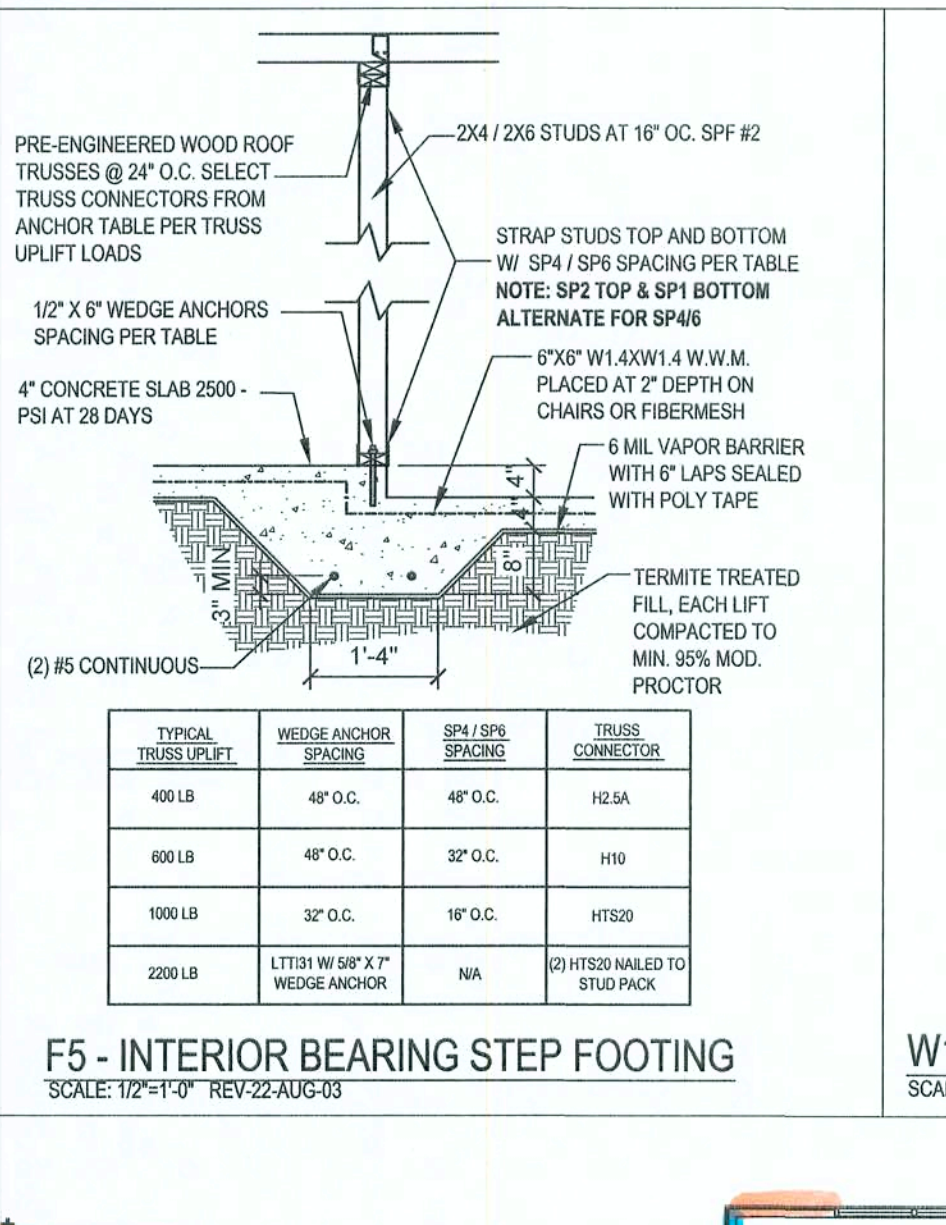
Uplift SPF	SYP#	Truss Connector	To One Member	To Other Member
760	885	SP4	6-10d x 1 1/2"	N/A
865	1005	CS20	9-8d or 7-10d	9-8d or 7-10d
1085	1265	LSTA18-24	7-10d	7-10d
1170	1390	SP14	12-10d x 1 1/2"	N/A
1420	1650	CS16	14-8d or 11-10d	14-8d or 11-10d

SPF SYP# Column Anchor

Uplift SPF	SYP#	Column Anchor	To Foundation	To Column / Truss
1160	1320	LTT19	3/4" x 16" AB	8-16d Sinker
1985	2310	LTT101	3/4" x 16" AB	16-10d x 1 1/2"
2385	2775	H20A	3/4" x 16" AB	2-3/4" x 8d
3690	4175	HTT16	3/4" x 16" AB	16-16d
1975	2300	ABU86	3/4" x 16" AB	12-16d

State Supporting Trusses: The builder is responsible for gravity loads, but you should put an extra 2x4 stud under truss bearing location for each 3000 lb ft reaction. Check the minimum bearing requirements of the truss and top plate SYP# for (SYP#2200000).

Manufacturer and Product number are listed for example not endorsement. An equivalent device of the same or other manufacturer can be substituted if they are shown listed in the example tables as long as it meets the required load capacities. Manufacturer's installation instructions must be followed to achieve rated loads. All connections except directly to the member shall be fastened and gasketed after installation. Loads are increased for wind direction. Strap uplift may be reduced proportionally to number of nails. See specification for minimum nail sizes (16d=16d, 18d=18d, 20d=20d, 22d=22d, 24d=24d, 26d=26d, 28d=28d, 30d=30d).



N4-WIND LOAD DESIGN DATA

(Wind loads are per FBC 2001, Section 1609.2 for enclosed simple diaphragm buildings with mean roof height less than 60' or the least horizontal dimension; not sited on the upper half of an unobstructed 60' high hill with >10% slope.)

Basic Wind Speed	110 MPH
Wind Exposure	B
Wind Importance Factor	1.0
Building Category	II
Internal pressure Coefficient	N/A (Enclosed)
Building not in the high velocity hurricane zone	
Building not in the wind-borne debris region	
Mean Roof Height	< 30 ft
Roof Angle	10-45 degrees
Components and Cladding Wind Pressures (FBC Table 1606.2 BAC)	

Zone Effective Wind Area (ft2)

Zone	Effective Wind Area (ft2)
1	100
2	21.8
3	23.8
4	18.5
5	21.8
6	29.1
7	18.5
8	22.8

Total Shear Wall Segments

Transverse	Longitudinal
45.0'	23.9'

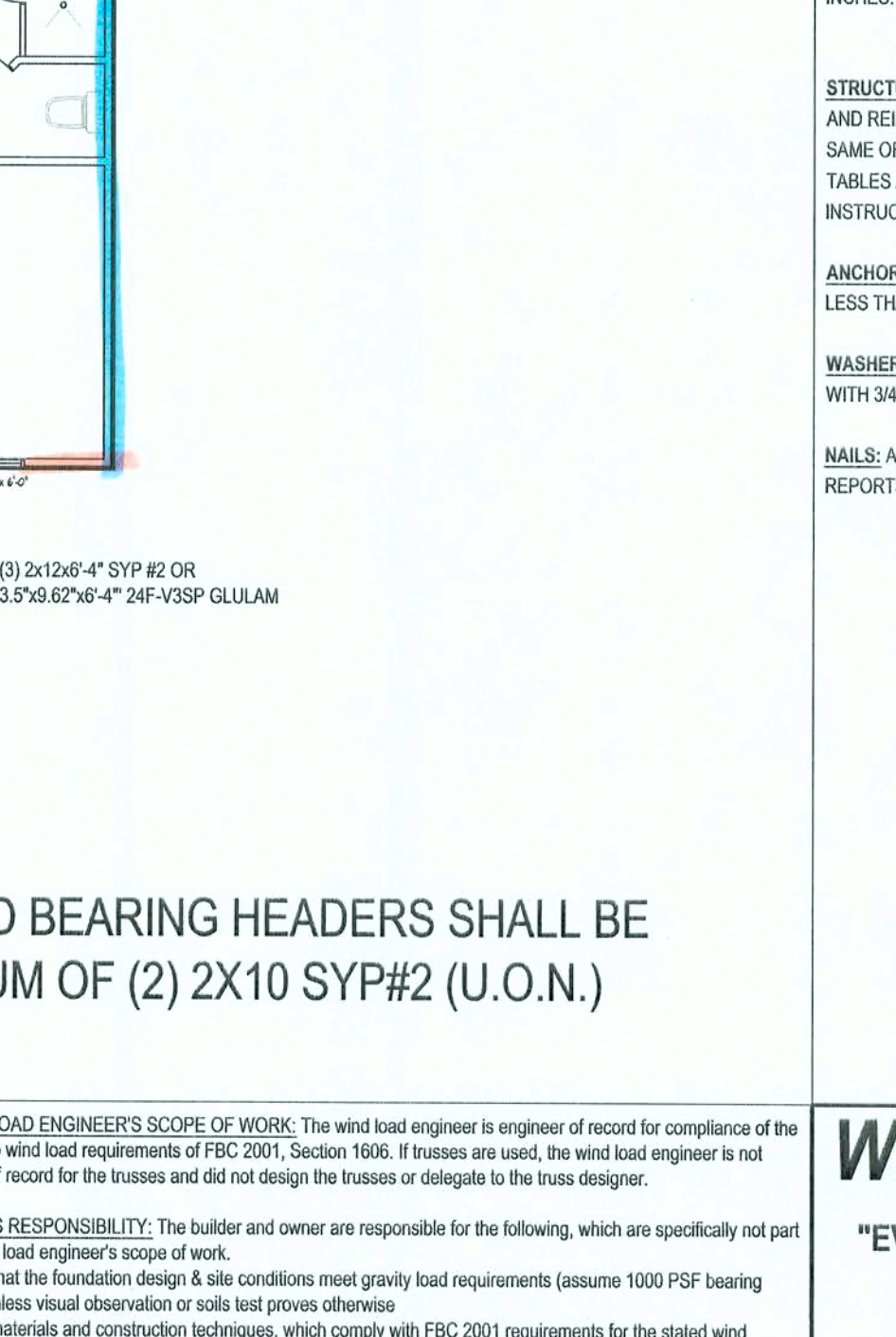
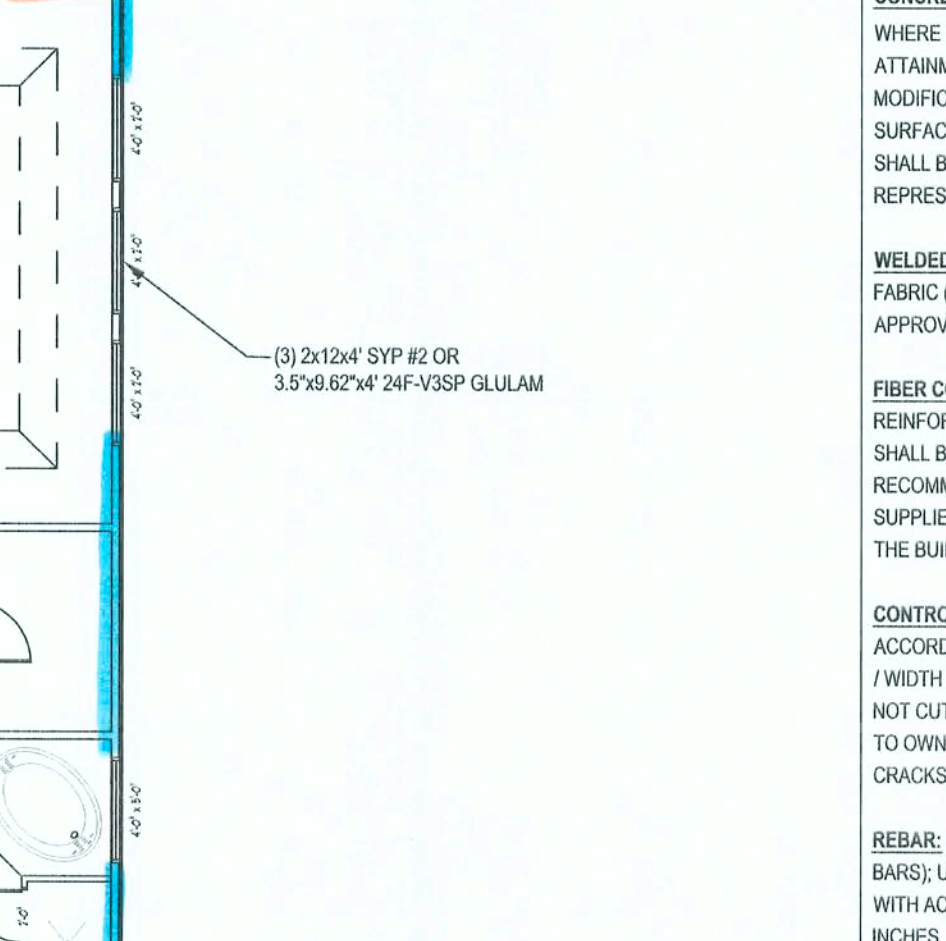
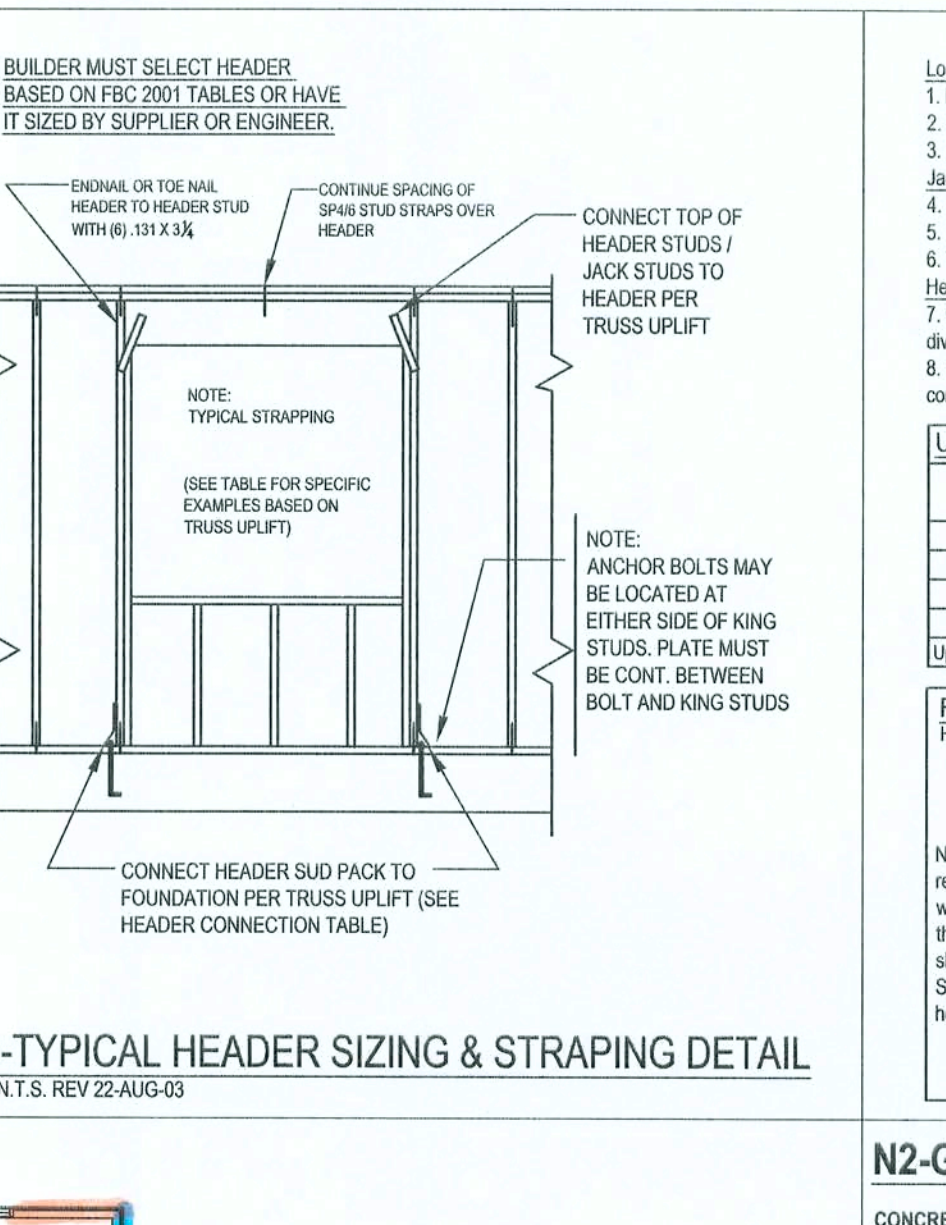
Required 45.0' 23.9'

Actual 53.1' 113.0'

2'-min for 0'-0" wall 2'-10" min for 10'-0" wall

3.17A & 3.17B with table 3.17E adjustment for types II shear wall (or equivalent calculation)

REV-27-Jun-03



N3-WINDLOAD ENGINEER'S SCOPE OF WORK: The wind load engineer is engineer of record for compliance of the structure to wind load requirements of FBC 2001, Section 1606. If trusses are used, the wind load engineer is not engineer of record for the trusses and did not design the trusses or delegate to the truss designer.

BUILDER'S RESPONSIBILITY: The builder and owner are responsible for the following, which are specifically not part of the wind load engineer's scope of work.

- * Confirm that the foundation design & site conditions meet gravity load requirements (assume 1000 PSF bearing capacity unless visual observation or soils test proves otherwise)
- * Provide materials and construction techniques, which comply with FBC 2001 requirements for the stated wind velocity and design pressures.
- * Provide a continuous load path from roof to foundation. If you believe the plan omits a continuous load path connection, call the wind load engineer immediately.
- * Verify the truss engineering includes truss design, placement plans, temporary and permanent bracing details, truss-to-truss connections, and load reactions for all bearing locations.
- * Select uplift connections, walls, columns, and footings based on truss engineering bearing locations and reactions, including interior bearing walls.
- * Size headers for gravity loads; headers sized by the builder for gravity loads will also satisfy wind loads.

DOCUMENT CONTROL AND PRIORITY: Structural requirements on S-1 control unless the building code or architectural sheets have more stringent requirements. Non-structural requirements on architectural sheets control. Specific requirements take precedence over general requirements. Revision control is by the latest signature date and is the responsibility of the builder.

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DIMENSIONS: Stud dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

WINDLOAD ENGINEER: Mark Disoway, PE No 53915

CERTIFICATION: The attached plans and "Windload Engineering", sheet S-1, comply with FBC 2001, Section 1606 wind loads, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location. This drawing is not valid for construction unless raised seal is affixed.

REV-06-OCT-02

Load Bearing Header Sizing Methods (BY BUILDER)

1. Determine header size from FBC 2001, Tables 2306.3 A, B, C, or 2308.5.
2. Use supplier published data or Southern pine span tables.
3. For engineered lumber beams have suppliers engineer size item.
4. Look up jack studs from FBC 2001, Tables 2306.3 A, B, C, or 2308.5.
5. Use one jack stud for every 3000 lb vertical load.
6. Total king plus jack studs = studs needed to be there if no opening was there.
7. Calculate the uplift at each end of the header by summing the moments of all truss uplifts and dividing by the length of the header.
8. Select header connections from table below or mfg. catalog to connect header to stud (top connection) and stud to foundation (bottom connection).

Header Uplift Connections (BY BUILDER)

Uplift, lb.	Top Connector	Bottom Connector
< 800	End nail or toe nail w/ 13"x3.25"	SP4, 6-10d
< 1500	LSTA12	755
< 1750	LSTA18	1055
< 2500	LSTA18	2110
< 3885	LSTA18 16-10d	3480
		HTT16, 16-16d, 3/4" 10" AB
		4175

Uplift greater than 3885 requires engineering design

FBC2001, TABLE 2308.3A
Header Spans for Exterior Bearing Walls Supporting Roof/Ceiling (20psf+20psf)

Header Span (ft)	Bulling Width / Truss Span (ft)		
	10	20	30
2-2x4	5-6	1-3/2	1-2-10
2-2x6	6-5	1-4-8	1-4-2
2-2x8	6-11	1-5-11	2-5-4
2-2x10	6-5	2-7-3	2-6-2
2-2x12	9-8	2-8-5	2-7-6
3-2x8	6-4	1-7-5	1-6-8
3-2x10	10-4	1-8-1	2-9-2
3-2x12	12-2	2-10-7	2-9-5
4-2x8	9-2	1-8-4	1-7-6
4-2x10	11-4	1-10-6	1-9-5
4-2x12	14-1	1-12-2	2-10-11

NOTES: NU = Number of jack studs required to support each end. Building width is measured perpendicular to the ridge. For widths between those shown, spans may be interpolated. Spans are based on uniform loads on header.

N2-GENERAL NOTES:

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE $F_c = 3000$ PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE ENGINEER, OWNER OR HIS REPRESENTATIVE.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14, FB = 65SI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTHS SHALL BE 1/2 INCH TO 2 INCHES IN LENGTH. DOSAGE AMOUNTS SHALL BE FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. SYNTHETIC FIBERS SHALL COMPLY WITH ASTM C 1116. THE MANUFACTURER OR SUPPLIER SHALL PROVIDE CERTIFICATION OF COMPLIANCE WITH ASTM C 1116 WHEN REQUESTED BY THE BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI ALL LAPS SPICES 40" DB (25' FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLUCED IN ACCORDANCE WITH ACI 315-85 WITH ACI 315-86 UNLESS NOTED OTHERWISE. ALL TENSION DEVELOPMENT LENGTHS SHALL BE 23 INCHES.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS SHALL BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 5" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/16"; WITH 5/8" BOLTS TO BE 5" x 3" x 9/16"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/16"; WITH 7/8" BOLTS TO BE 3" x 3" x 9/16"; NO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

REV-22-AUG-03

WINDLOAD ENGINEERING

"EVERYTHING YOU NEED FOR YOUR BUILDING PERMIT"

Mark Disoway P.E.

POB 868, Lake City, FL 32056 Phone: (386) 754-5419

Fax: (702) 543-7241 Email: windloadengineering@bellsouth.net

Location: Lot #17 Emerald Lakes S/D Columbia County, Florida

Mehalko Residence

Builder: Don Reed Construcion

Designer: DDS

Approved: FLPE#53915

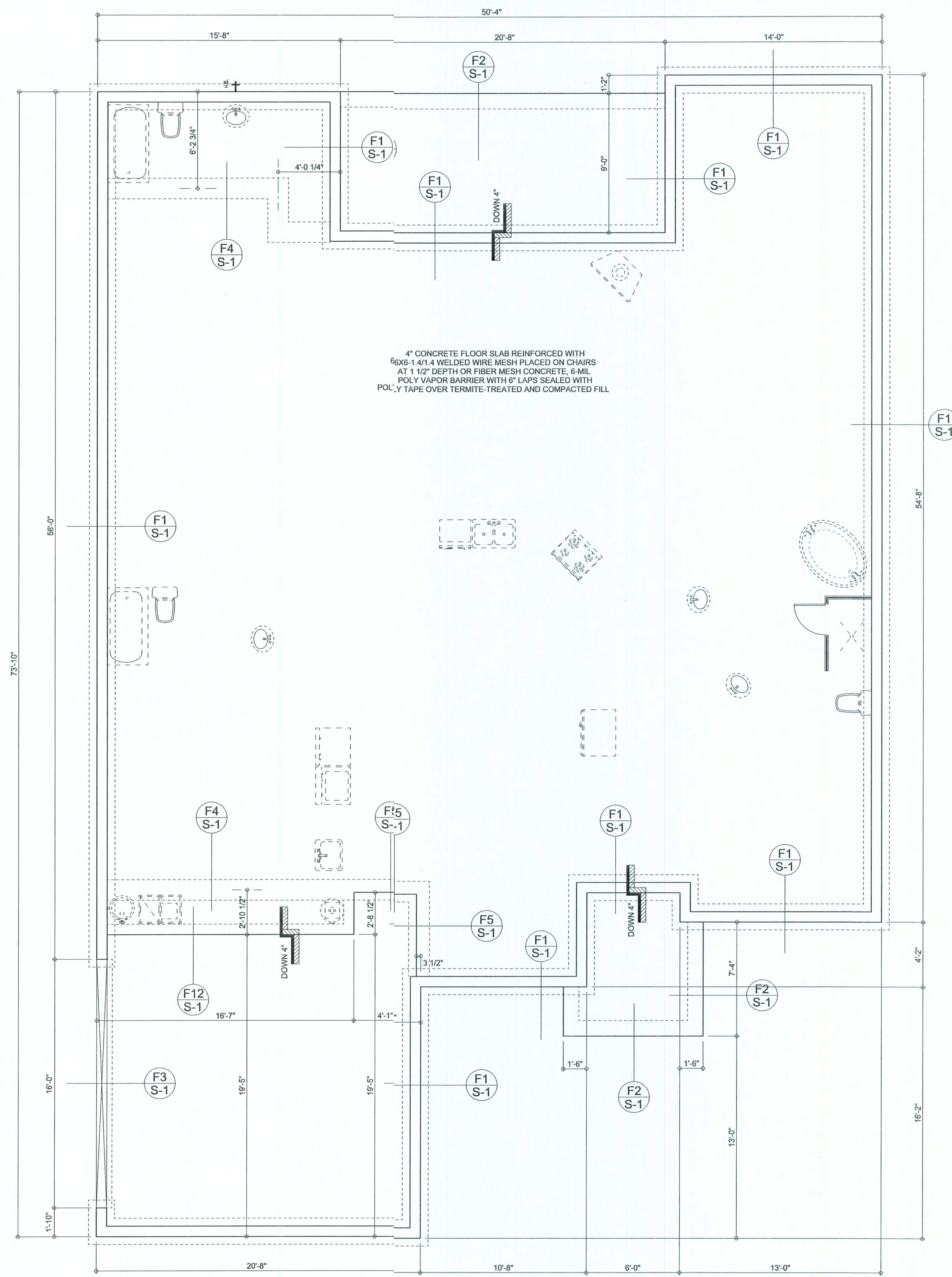
Revisions:

Sheet S-1 of 2 Sheets

Windload Engineering

Job # 402173

24F004



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

DETAIL #
F1
S-1
SHEET #

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway,
P.E. #53915, POB 868, Lake City, FL
32065, 386-754-5419

DIMENSIONS:
Scaled dimensions supercede scaled
dimensions. Refer all questions to
Mark Disoway, P.E. for resolution.
Do not proceed without clarification.

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form or manner without first the express written
permission and consent of Mark Disoway.

CERTIFICATION: These plans and
"Windload Engineering" Sheet S-1, attached,
comply with Florida Building Code 2001,
Section 1605 wind loads, to the best of my
knowledge.

LIMITATION: This design is valid for one
building at specified location. In case of conflict,
structural requirements, scope of work, and
builder responsibilities on sheet S-1 control.

MARK DISOWAY
P.E. 53915

24 FEB 04

SEAL

Don Reed
Construction

Mehalko
Residence

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