ALL WIND LOADS ARE IN ACCORDANCE WITH SECTION 1609, FLORIDA BUILDING CODE 7TH EDITION (2020)						
FLOOR AND ROOF LIVE LOADS						
UNINHABITABLE ATTICS:		20 PS	F			
HABITABLE ATTICS, BEDROOM:		30 PS	F			
ALL OTHER ROOMS:		40 PS	F			
GARAGE:		40 PS	F			
ROOFS:		20 PSF UNI	FORM			
	WI	ND DESIGN DATA				
ULTIMATE WIND SPEED:	130 M	PH				
NOMINAL (BASIC) WIND SPEED:	NOMINAL (BASIC) WIND SPEED:		PH			
RISK CATEGORY:		II				
WIND EXPOSURE:		В				
ENCLOSURE CLASSIFICATION:		ENCL	OSED			
INTERNAL PRESSURE COEFFICIENT:		0.18 +/-				
COMPONENTS AND CLADDING						
ROOFING ZONE 1:		17.7 PSF MAX.	-19.3 PSF MIN.			
ROOFING ZONE 2:		17.7 PSF MAX.	-22.5 PSF MIN.			
ROOFING ZONE 3:		17.7 PSF MAX.	-22.5 PSF MIN.			
ROOFING AT ZONE 2 OVERHANGS:		-32.7 PSF MIN.				
ROOFING AT ZONE 3 OVERHANGS:		-32.7 PSF MIN.				
STUCCO, CLADDING, DOORS AND WINDOWS						
ROOFING ZONE 4:		19.3 PSF MAX.	-20.9 PSF MIN.			
ROOFING ZONE 5:		19.3 PSF MAX.	-25.8 PSF MIN.			
9' WIDE O/H DR.:		16.9 PSF MAX19.2 PSF M				
16' WIDE O/H DR.: 16.3 PSF MAX18.1 PSF MIN						

**HIP ROOFS** 

20 PSF (REDUCIBLE)

40 PSF

40 PSF

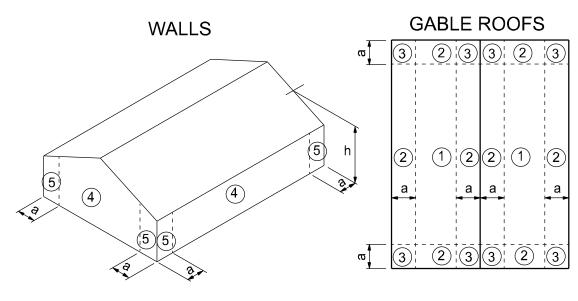
20 PSF

3000 PSI

ASTM A185

ASTM A615-40 40,000 PSI

ASTM A615-40 40,000 PSI



a: 10% of least horizontal dim. or 0.4h, whichever is smaller, but not less than either 4% of least horizontal dimension or 3 ft.

COMPONENTS AND CLADDING

## STRUCTURAL DESIGN CRITERIA

FLORIDA BUILDING CODE 7TH EDITION (2020) CODES: BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE SPECIFICATIONS FOR STRUCTURAL CONCRETE BUILDINGS BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES

NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, 2018 EDITION

APA PLYWOOD DESIGN SPECIFICATION

LIVE LOADS:

h: mean roof height, in feet.

RESIDENTIAL FLOOR, UNLESS OTHERWISE INDICATED **BALCONIES** 

ALL REINFORCING BARS

ALL STIRRUPS AND TIES

MORTAR TYPE "S" 1800 PSI

STAIRS

LIGHT PARTITIONS (DEAD LOAD), U.N.O.

WELDED WIRE FABRIC SHALL CONFORM TO

BEAMS, RAFTERS, JOIST, PLATES, ETC. U.N.O.

NO. 2 SOUTHERN YELLOW PINE (19% M.C.)

ASTM C90-99b, STANDARD WEIGHT UNITS, fm=1500 PSI

ALL BOLTS CAST IN CONCRETE: ASTM A36 OR ASTM A-307

ROOF DECK: PLYWOOD C-C/C-D, EXTERIOR, or OSB

FLOOR SHEATHING: T&G A-C GROUP 1 APA RATED (48/24)

WALL SHEATHING: PLYWOOD C-C/C-D, EXTERIOR OR OSB

WIND LOADS BASED ON FBC, SECTION 1609 WIND LOADS: WIND VELOCITY: 125 M.P.H., USE FACTOR: 1.0 (F.B.C.)

CONCRETE

ALL CONCRETE UNLESS OTHERWISE INDICATED STRENGTH

PEA GRAVEL CONCRETE FOR MASONRY CELLS ONLY (DO NOT USE FOR CONCRETE COLUMNS OR TIE BEAMS)

@ 28 DAYS **REINFORCING:** 

CONCRETE MASONRY

CONCRETE GROUT 3000 PSI UNITS: CONTINUOUS MASONRY INSPECTION IS REQUIRED DURING CONSTRUCTION ALL STRUCTURAL AND MISCELLANEOUS STEEL A36 36,000 PSI, U.N.O **STRUCTURAL** SHOP AND FIELD WELDS: E70XX ELECTRODES STEEL:

WOOD FRAMING:

VERSA LAM BEAM Fb = 2900 PSI (2.0E) WOOD COLS. PARALLAM 2.0E U.N.O. DESIGN LOADS:

**WOOD ROOF** TRUSSES:

TOP CHORD LIVE AND DEAD LOAD: BOTTOM CHORD DEAD LOAD:

SEE DRAWINGS FOR SPECIAL CONCENTRATED LOADS. DESIGN FOR NEW WIND UPLIFT AS PER SPECIFIED CODES, DEDUCTING A MAXIMUM OF 5 P.S.F. DEAD LOAD, BUT NOT EXCEEDING ACTUAL DEAD LOAD.

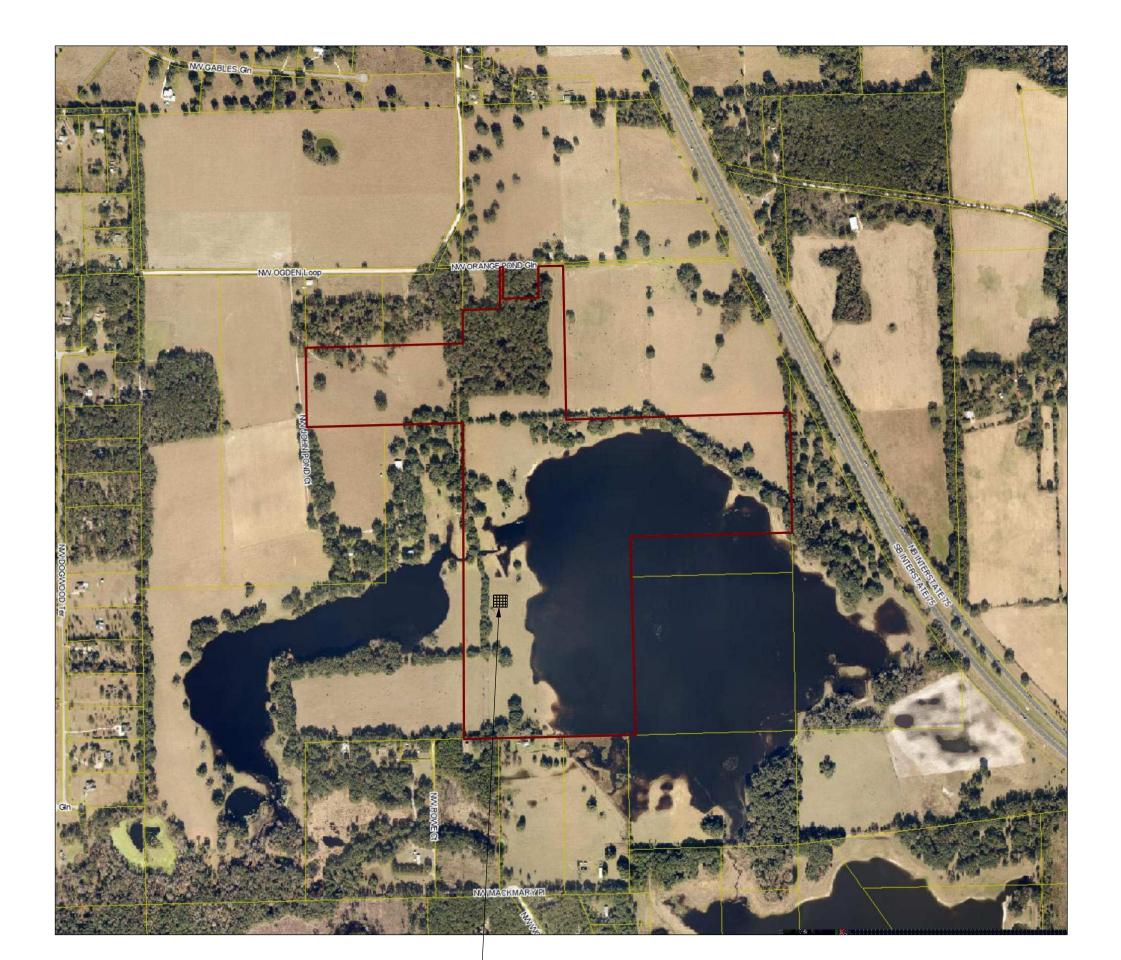
SOIL BEARING

ASSUMED ALLOWABLE SOIL BEARING PRESSURE AFTER COMPACTION: 1,500 PSF SEE SOILS REPORT AND SPECIFICATIONS FOR COMPACTION REQUIREMENTS IF SOIL CONDITIONS IN THE PROJECT DO NOT MEET OR EXCEED THE CAPACITY THE GENERAL CONTRACTOR SHALL CONTACT THE ENGINEER PRIOR TO FOUNDATION POUR FOR VERIFICATION OF FOUNDATION DESIGN.

30 PSF

10 PSF

40 PSF



PROJECT LOCATION -PARCEL 07-3S-16-02029-008 (43421)

### **ABBREVIATIONS**

ALT. B.C. B.F. Bk Sh Bm. BOT. B.P. Brg. Cir. Clg. Col.	Beam Bottom Bypass door Bearing Circle Ceiling Column A/C Compressor Ceramic Tile Dryer Decorative Dedicated Outlet Double Diameter Disposal Distance Drawer Stack Dryer Vent Dishwasher Each Each Way Electrical Elevation Exterior Expansion Florida Bldg. Code	Flr. Fdn. Flr. Sys. F.Pl. Ft. Ftg. FX G.C. G.F.I. Hdr. Hgt. HB Int. K/Wall K.S. Lav. L.F. L.T. Max M.C. MICO Micro Mir. Monos. Opt. Pc. PLF	Floor Foundation Floor System Fireplace Foot / Feet Footing Fixed Galvanized General Contractor Ground Fault Interrupter Girder Truss Header Height Hose Bibb Interior Kneewall Knee Space Laundry Lavatory Linear Ft. Laundry Tub Masonry Maximum Medicine Cabinet Master Distribution Panel Manufacturer Microwave Minimum Microlam Mirror Monolithic Not to Scale Opening Optional Piece Pedestal Parallam Pounds per linear foot	Plt. Ht. Plt Sh. PSF P.T. Pwd. Ref. Req'd. Ref. S.F. Sh. S.P. S.Y.P. Thik'n. T.O.M. T.O.P. Trans. U.N. VB Vert. VTR W W/ W	Plate Height Plant Shelf Pounds per square foot Pressure Treated Powder Room Radius Refrigerator Required Room Round Rod and Shelf Smoke Detector Square Ft. Shelves Sheet Side Lights Spruce Pine Fir Square Southern Yellow Pine Tempered Thicken Top of Block Top of Masonry Top of Plate Transom Window Typical Under Cabinet Lighting Unless Noted Otherwise Vanity Base Vertical Versalam Vent through Roof Washer With Water Closet Wedge Anchor Wood Water Proof

### **INDEX OF SHEETS**

<u>SHEET</u> **DESCRIPTION COVER SHEET** FLOOR PLAN A-2 A-3

UPPER FLOOR PLAN ELEVATIONS FRONT AND REAR ELEVATIONS SIDES A-5 FOUNDATION PLAN

ROOF PLAN ELECTRICAL PLAN MAIN ELECTRICAL PLAN UPPER SECTIONS AND FRAMING DETAILS A-10 A-11 SHEARWALL DETAILS

NO. 28022

Brett A. Crews, P.E. 65592

Plans

Reviewed

for Code

No 65592

STATE OF

GENERAL PLAN NOTES

CONSTRUCTION DOCUMENTS THE CUSTOMER IS RESPONSIBLE FOR DELIVERING THE REQUIRED SETS OF CONSTRUCTION DOCUMENTS TO THE PERMIT ISSUING AUTHORITIES, FOR THE ISSUANCE OF CONSTRUCTION PERMITS. THE CONTRACTOR SHALL REVIEW THE CONSTRUCTION DOCUMENTS AND VERIFY ALL DIMENSIONS. ANY DIS-CREPANCIES SHALL BE REPORTED TO THE ARCHITECT PRIOR TO THE COMMENCEMENT OF ANY WORK OR FABRACATION OF ANY MATERIALS.

DO NOT SCALE OFF THESE PLANS

AMPLE DIMENSIONS ARE SHOWN ON THE PLANS TO LOCATE ALL ITEMS. SIMPLE ARITHMETIC MAY BE USED TO DETERMINE THE LOCATIONS OF THOSE ITEMS NOT DIMENSIONED.

CHANGES TO FINAL PLAN SETS

PLEASE DO NOT MAKE ANY STRUCTURAL CHANGES TO THESE PLANS WITHOUT CONSULTING WITH THE ARCHITECT. THE OWNER SHALL ASSUME ANY AND ALL LIABILITY FOR STRUCTURAL DAMAGE RESULTING FROM CHANGES MADE TO THE PLANS OR BY SUBSTITUTION OF MATERIALS DIFFERENT FROM SPECIFICATION ON THE PLANS.

INORGANIC ARSENICAL PRESSURE TREATED WOOD SOME FRAMING MATERIALS SPECIFIED FOR THE CONSTRUCTION OF YOUR PROJECT SUCH AS SILLS OR EXTERIOR FRAMING ARE PRESSURE TREATED. EACH PIECE IS CLEARLY MARKED FOR EASY IDENTIFICATION AND IS USUALLY GREENISH IN COLOR.

THIS WOOD HAS BEEN PRESERVED BY PRESSURE-TREATMENT WITH AN EPA-REGISTERED PESTICIDE CONTAINING INORGANIC ARSENIC TO PROTECT IT FROM INSECT ATTACK AND DECAY. EXPOSURE TO TREATED WOOD MAY PRESENT CERTAIN HAZARDS, THEREFORE, PRECAUTIONS SHOULD BE TAKEN BOTH WHEN HANDLING THE TREATED WOOD AND IN DETERMINING WHERE TO USE OR DISPOSE OF THE TREATED WOOD.

FOR FURTHER INFORMATION ON THE USE OF AND DISPOSAL OF INORGANIC ARSENIC PRESSURE TREATED WOOD, PLEASE REFER TO THE EPA MATERIAL SAFETY SHEET DEALING WITH THIS PRODUCT.

### PREFABRICATED WOOD TRUSSES

1. ALL PREFABRICATED WOOD TRUSSES SHALL BE SECURELY FASTENED TO THEIR SUPPORTING WALLS OR BEAMS WITH **HURRICANE CLIPS OR ANCHORS** 

- 2. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENERS" AS RECOMMENDED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.
- 3. TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPOR-TIONED ( WITH A MAXIMUM ALLOWABLE STRESS INCREASE FOR LOAD DURATION OF 25%) TO WITHSTAND THE LIVE LOADS GIVEN IN THE NOTES AND TOTAL DEAD LOAD. 4. BRIDGING FOR PRE-ENGINEERED TRUSSES SHALL BE AS REQUIRED BY THE TRUSS MANUFACTURER UNLESS

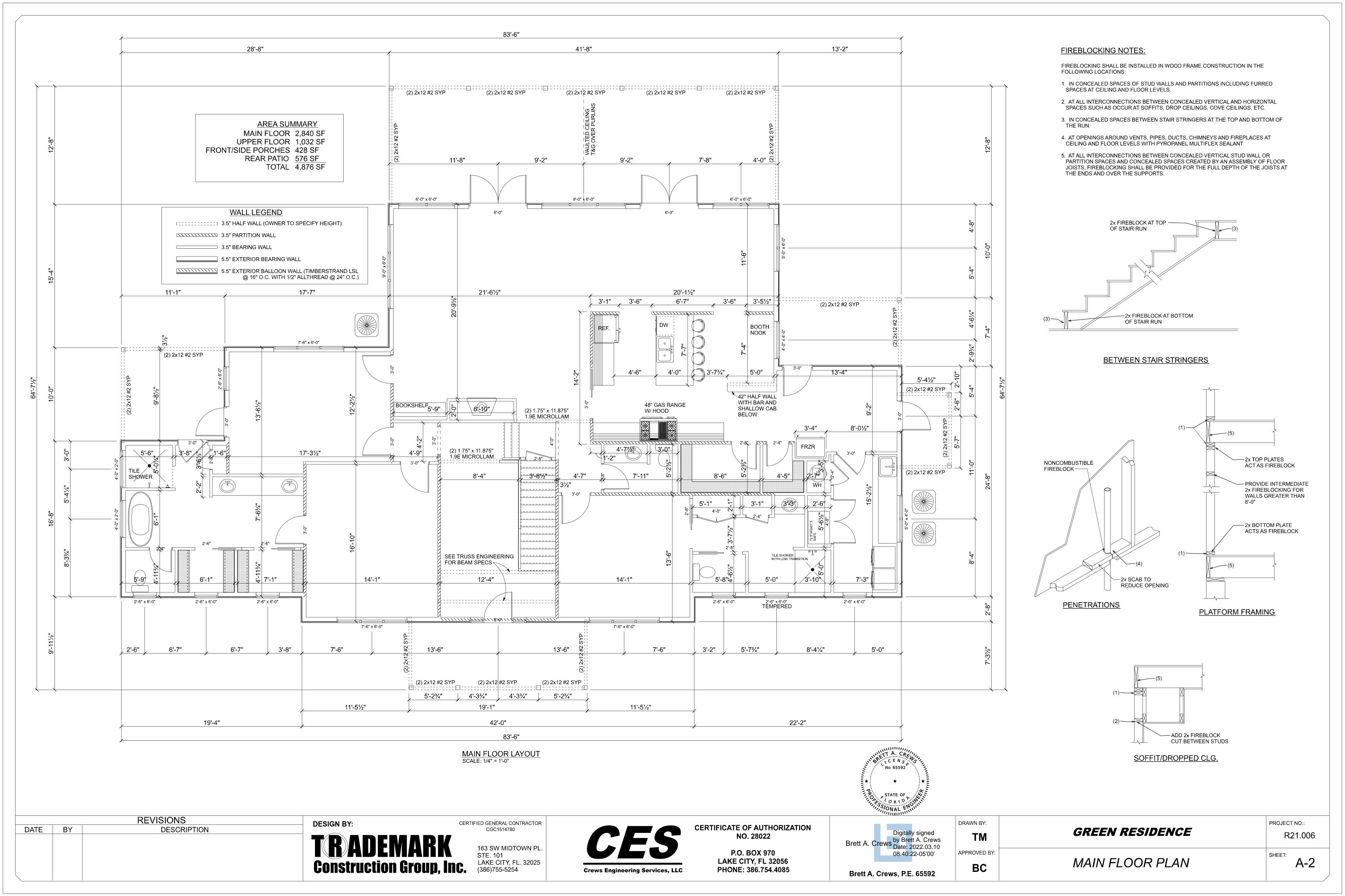
NOTED ON THE PLANS.

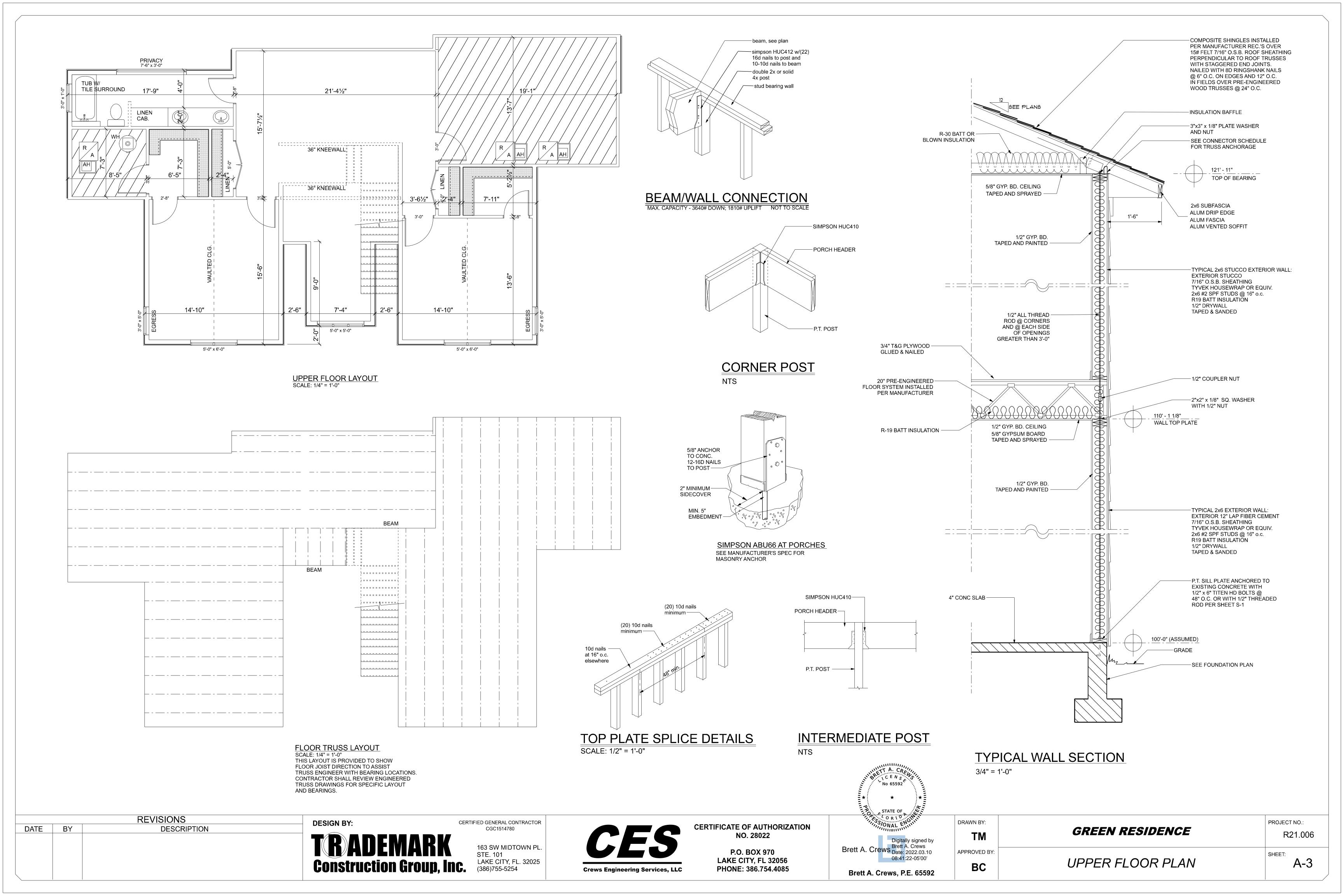
- 5. TRUSS ELEVATIONS AND SECTIONS ARE FOR GENERAL CONFIGURATION OF TRUSSES ONLY. WEB MEMBERS ARE NOT SHOWN, BUT SHALL BE DESIGNED BY THE TRUSS MANUFACTURER IN ACCORDANCE WITH THE FOLLOWING DESIGN LOADS:
- DESIGN SPECIFICATIONS FOR LIGHT WEIGHT METAL PLATE CONNECTED WOOD TRUSSES PER THE TRUSS PLATE INSTITUTE TPI LATEST EDITION.
- 7. PRE-ENGINEERED WOOD TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH SPECIFIED LOADS AND GOVERNING CODES . SUBMITTALS SHALL INCLUDE TRUSS FRAMING PLANS AND DETAILS SHOWING MEMBER SIZES, BRACING, ANCHORAGE, CONNECTIONS, TRUSS LOCATIONS, AND AND PERMANENT BRACING AND/OR BRIDGING AS REQUIRED FOR ERECTION AND FOR THE PERMANENT STRUCTURE. EACH SUBMITTAL SHALL BE SIGNED AND SEALED BY A FLORIDA REGISTERED STRUCTURAL ENGINEER. SUBMIT 3 COPIES FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.
- 8. THE TRUSS MANUFACTURER SHALL DETERMINE ALL SPANS WORKING POINTS, BEARING POINTS, AND SIMILAR CONDITIONS. TRUSS SHOP DRAWINGS SHALL SHOW ALL TRUSSES, ALL BRACING MEMBERS, AND ALL TRUSS TO TRUSS HANGERS.

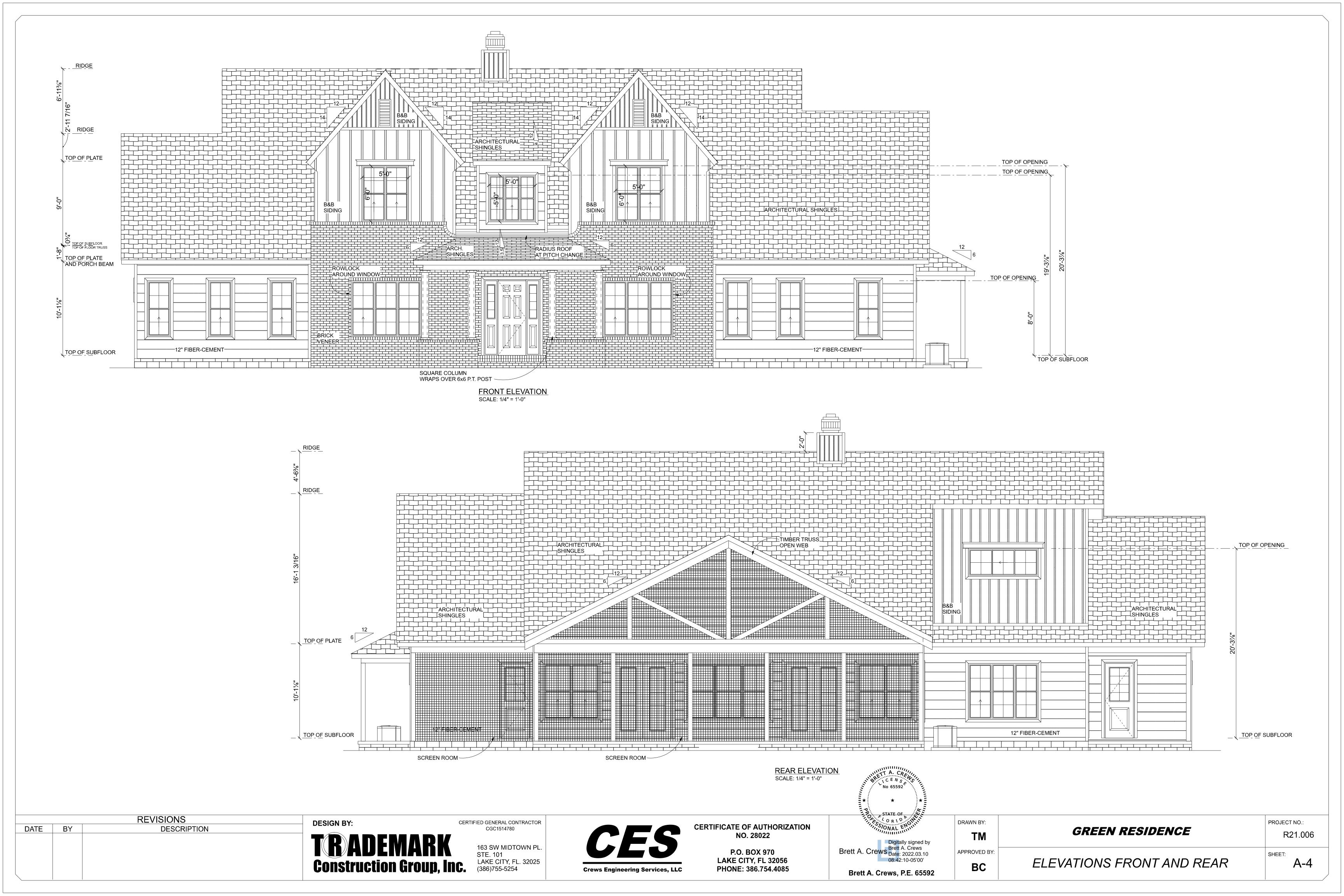
### FIELD REPAIR NOTES

- 1. MISSED LINTEL STRAPS FOR MASONRY CONSTRUCTION MAY BE SUBSTITUTED W/ (1) "SIMPSON MTSM16 TWIST STRAP W/ (4) 1/4" X 2 1/4" DIA. TITENS TO THE BOND BEAM BLOCK AND (7) 10d TO THE TRUSS FOR UPLIFTS OF 1000 LBS. OR LESS. USE (2) FOR 2000 LBS. OR LESS. OTHERS MAY BE SUBSTITUTED ON A CASE BY CASE BASIS. 2. MISSED "J" BOLTS FOR WOOD BEARING WALLS MAY BE SUB-
- STITUTED W/ 1/2" DIA. ANCHOR BOLTS SET IN 3/4" DIA. X 6" DEEP UNITEX "PROPOXY" 300 ADHESIVE BINDER FOLLOWING ALL MANUFACTURERS RECOMMENDATIONS (OR 1/2" X 6" RAWL STUD EXPANSION ANCHORS. ) 3. REGARDING MISSED REBAR IN VERTICAL FILLED CELLS:
- DRILL A 3/4" DIAMETER HOLE 6" DEEP AT THE LOCATION OF THE OMITTED REBAR, AND INSTALL A 32" LONG #5 BAR INTO THE EPOXY FILLED HOLE. USE A TWO PART EMBEDDEMENT EPOXY ( SIMPSON "EPOXY TIE SET", OR HILTI " 2 PART" EMBEDDMENT EPOXY ), MIXED PER MANUFACTURER'S INSTRUCTIONS. ASSURE THAT ALL DUST AND DEBRIS FROM DRILLING ARE REMOVED FROM THE HOLE BY BRUSHING AND AND USING COMPRESSED AIR PRIOR TO APPLYING THE EPOXY. ALLOW THE EPOXY TO CURE TO MANUFACTURER'S SPECIFICATIONS, THEN FILL THE CELL IN THE NORMAL WAY DURING BOND BEAM POUR.
- 4. HURRICANE STRAPS MAY BE SUBSTITUTED WITH A STRAP OF GREATER HOLDOWN VALUE OR GREATER UPLIFT VALUE IN THE FIELD WITHOUT VERIFICATION, PROVIDED ALL MANUFACTURERS INSTALLATION INSTRUCTIONS ARE FOLLOWED.
- 5. FOR MORTER JOINTS LESS THAN 1/4", PROVIDE (1) #5 VERT. IN CONC. FILLED CELL EACH SIDE OF THE JOINT (BAR DOES NOT HAVE TO BE CONT. TO FOOTING)

REVISIONS CERTIFIED GENERAL CONTRACTOR PROJECT NO.: **DESIGN BY:** DRAWN BY: **CERTIFICATE OF AUTHORIZATION** DATE BY **DESCRIPTION** CGC1514780 **GREEN RESIDENCE** Digitally signed by R21.006 TRADEMARK Brett A. Crews Brett A. Crews Date: 2022.03.10 163 SW MIDTOWN PL P.O. BOX 970 APPROVED BY: 08:39:40-05'00' STE. 101 SHEET: **COVER SHEET** LAKE CITY, FL. 32025 (386)755-5254 LAKE CITY, FL 32056 **Construction Group, Inc.** A-1 PHONE: 386.754.4085 **Crews Engineering Services, LLC** 











DATE BY DESCRIPTION

DESIGN BY:

CERTIFIED GENERAL CONTRACTOR
CGC1514780

TRADENARK

163 SW MIDTOWN PL.

STE. 101

LAKE CITY, FL. 32025
(386)755-5254

Crews Engineering Services, LLC

CERTIFICATE OF AUTHORIZATION NO. 28022

P.O. BOX 970 LAKE CITY, FL 32056 PHONE: 386.754.4085

Brett A. Cre	Digitally signed by Brett A. Crews Date: 2022.03.10 08:42:48-05'00'

Brett A. Crews, P.E. 65592

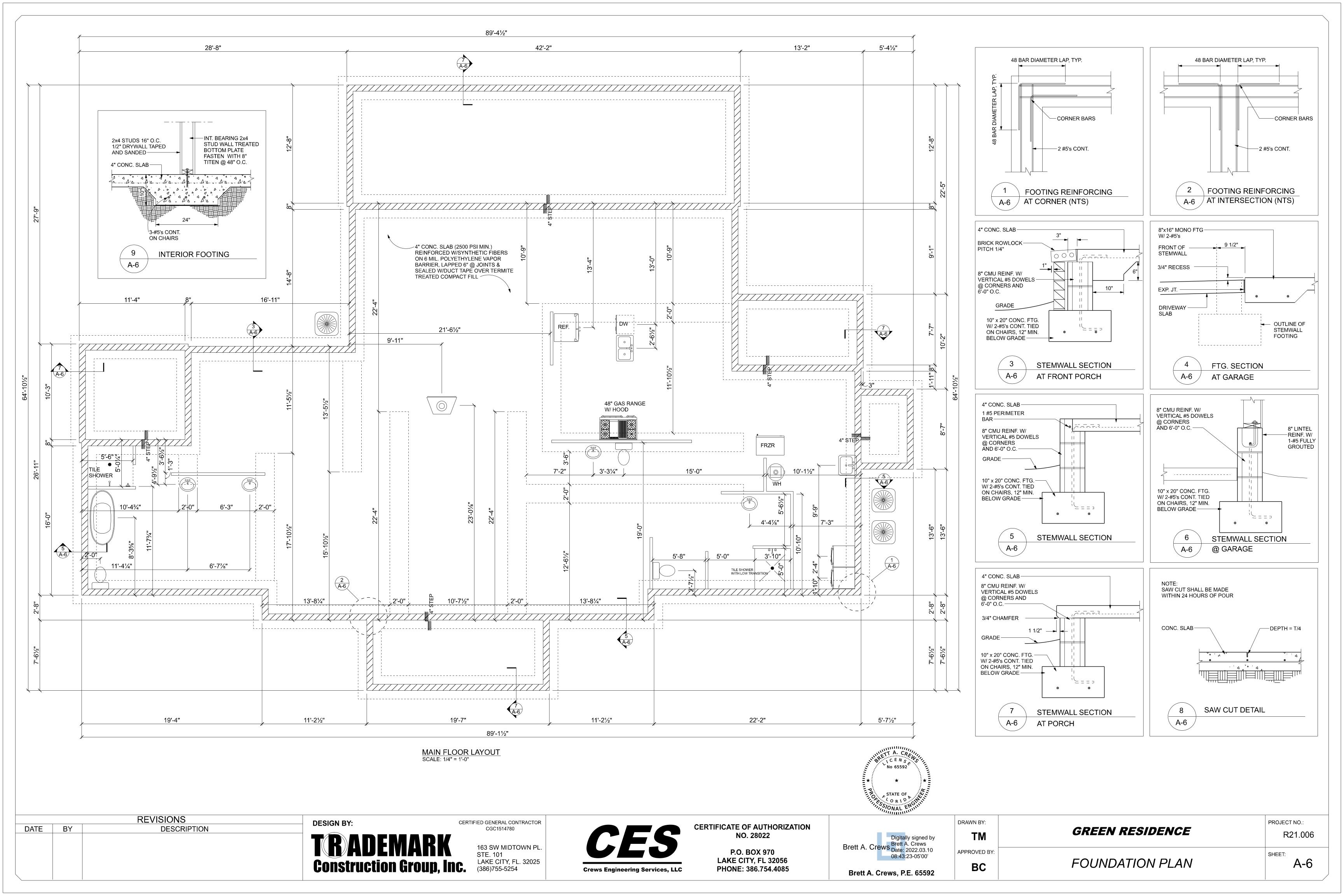
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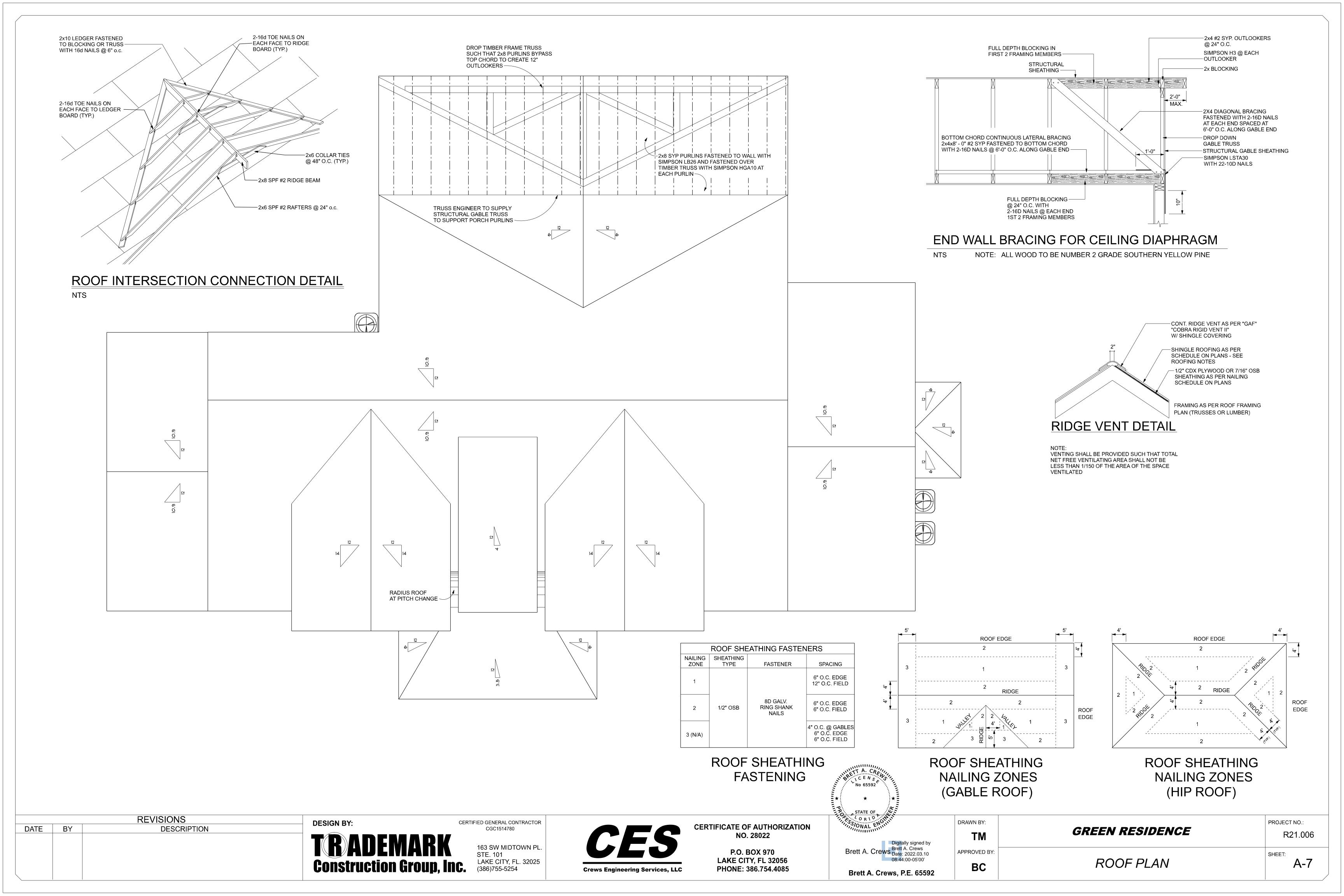
DRAWN BY:	
TM	
APPROVED BY:	
ВС	

GREEN RESIDENCE

R21.006

SHEET:
A-5





### **SHINGLE NOTES:**

DECK REQUIREMENTS:
ASPHALT SHINGLES SHALL BE FASTENED TO SOLIDLY SHEATHED DECKS.

# SLOPE: ASPHALT SHINGLES SHALL BE USED ONLY ON ROOF SLOPES OF 4:12 OR GREATER FOR ROOF SLOPES FROM 3:12 TO 4:12 DOUBLE UNDERLAYME

OR GREATER. FOR ROOF SLOPES FROM 3:12 TO 4:12, DOUBLE UNDERLAYMENT IS REQUIRED.

UNLESS OTHERWISE NOTED, UNDERLAYMENT SHALL CONFORM WITH ASTM D 226, TYPE 1, OR ASTM D 4869, TYPE 1.

SELF-ADHERING POLYMER MODIFIED BITUMEN SHEET: SELF ADHERING POLYMER MODIFIED BITUMEN SHALL COMPLY WITH ASTM D 1970.

ASPHALT SHINGLES:
ASPHALT SHINGLES SHALL HAVE SELF SEAL STRIPS OR BE INTERLOCKING, AND COMPLY WITH ASTM D 225 OR ASTM D 3462.

### FASTENERS:

FASTENERS FOR ASPHALT SHINGLES SHALL BE GALVANIZED, STAINLESS STEEL, ALUMINUM OR COPPER ROOFING NAILS, MINIMUM 12 GAUGE SHANK WITH A MINIMUM 3/8 INCH DIAMETER HEAD, OF A LENGTH TO PENETRATE THROUGH THE ROOFING MATERIAL AND A MINIMUM 3/4" INTO THE ROOF SHEATHING. WHERE ROOF SHEATHING IS LESS THAN 3/4" THICK, THE NAILS SHALL PENETRATE THROUGH THE SHEATHING.

### ATTACHMENT

ASPHALT SHINGLES SHALL BE SECURED TO THE ROOF WITH NOT LESS THAN FOUR FASTENERS PER STRIP SHINGLE OR TWO FASTENERS PER INDIVIDUAL SHINGLE. WHERE ROOFS LOCATED IN BASIC WIND SPEED OF 110 MPH OR GREATER, SPECIAL METHODS OF FASTENING ARE REQUIRED. UNLESS OTHERWISE NOTED, ATTACHMENT OF ASPHALT SHINGLES SHALL CONFORM WITH ASTM D 3161 OR M-DC PA 107-95.

UNDERLAYMENT APPLICATION: FOR ROOF SLOPES FROM 3:12 TO 4:12, UNDERLAYMENT SHALL BE A MINIMUM OF TWO LAYERS APPLIED AS FOLLOWS:

1. STARTING AT THE EAVE, A 19 INCH STRIP OF UNDERLAYMENT SHALL BE APPLIED PARALLEL WITH THE EAVE AND FASTENED SUFFICIENTLY TO STAY IN PLACE.

2. STARTING AT THE EAVE, 36 INCH WIDE STRIPS OF UNDERLAYMENT FELT SHALL BE APPLIED OVERLAPPING SUCCESSIVE SHEETS 19 INCHES AND FASTENED SUFFICIENTLY TO STAY IN PLACE

FOR ROOF SLOPED 4:12 AND GREATER, UNDERLAYMENT SHALL BE A MINIMUM OF ONE LAYER OF UNDERLAYMENT FELT APPLIED AS FOLLOWS:
STARTING AT THE EAVE, UNDERLAYMENT SHALL BE APPLIED SHINGLE FASHION PARALLEL TO THE EAVE, LAPPED 2 INCHES, AND FASTENED SUFFICIENTLY TO STAY IN PLACE.

### BASE AND CAP FLASHINGS:

BASE AND CAP FLASHING SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTALLATION INSTRUCTIONS. BASE FLASHING SHALL BE OF EITHER CORROSION RESISTANT METAL OF MINIMUM NOMINAL THICKNESS 0.019 INCH OR MINERAL SURFACE ROLL ROOFING WEIGHING A MINIMUM OF 77 LBS PER 100 SQUARE FEET. CAP FLASHING SHALL BE CORROSION RESISTANT METAL OF MINIMUM NOMINAL THICKNESS OF 0.019 INCH.

VALLEYS:
VALLEY LININGS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTALLATION
INSTRUCTIONS BEFORE APPLYING ASPHALT SHINGLES. VALLEY LININGS OF THE FOLLOWING
TYPES SHALL BE PERMITTED.

FOR OPEN VALLEYS LINED WITH METAL, THE VALLEY LINING SHALL BE AT LEAST 16 INCHES WIDE AND OF ANY OF THE CORROSION RESISTANT METALS IN TABLE 1507.3.9.2.
 FOR OPEN VALLEYS, VALLEY LINING OF TWO PLIES OF MINERAL SURFACE ROLL ROOFING SHALL BE PERMITTED. THE BOTTOM LAYER SHALL BE 18 INCHES AND THE TOP LAYER A MINIMUM OF 36 INCHES WIDE.

3. FOR CLOSED VALLEYS VALLEY LINING SHALL BE ONE OF THE FOLLOWING:

- 1. BOTH TYPES 1 AND 2 ABOVE, COMBINED.
- 2. ONE PLY OF SMOOTH ROLL ROOFING AT LEAST 36 INCHES WIDE AND COMPLYING WITH ASTM D 224.
- 3. SPECIALTY UNDERLAYMENT AT LEAST 36 INCHES WIDE AND COMPLYING WITH ASTM D 1970.

MATERIAL	MINIMUM THICKNESS (in)	GAGE	WEIGHT
	THICKINESS (III)		(LB)
COPPER			1
ALUMINUM	0.024		
STAINLESS STEEL		28	
GALVANIZED STEEL	0.0179	26 (zinc coated G90)	
ZINC ALLOY LEAD PAINTED TERNE	0.027		2 1/2 20

### -COMPOSITE SHINGLES INSTALLED PER MFGR. RECOMMENDATIONS OVER #15 FELT 1/2" O.S.B. ROOF SHEATHING INSTALLED PERPENDICULAR TO ROOF TRUSSES WITH STAGGERED END JOINTS. NAILED WITH 8d RINGSHANK NAILS @ 6" O.C. ON EDGES AND 12" O.C. IN FIELDS OVER ENG. WOOD TRUSSES @ 24" O.C. SEE PLAN SEE CONNECTOR SCHEDULE R-30 BATT OR -FOR TRUSS ANCHORAGE **BLOWN INSULATION** WITH INSULATION BAFFLE AT EAVE SEE ELEVATIONS TOP OF PLATE 1/2" OR 5/8" GYP. BD. CEILING TAPED AND SPRAYED -2x6 SUBFASCIA ALUM DRIP EDGE ALUM FASCIA VINYL VENTED SOFFIT 1'-6" 1/2" GYP. BD. TAPED AND PAINTED R-13 BATT INSULATION -6" VINYL SIDING 7/16" OSB WALL SHEATHING 1/2" ALL THREAD ROD FASTEN W/ 8d COMMON FROM FOUNDATION TO TOP PLATE, FASTENED @ 6" O.C. EDGES / 12" O.C. INT TO WITH NUT AND 3"x3" WASHER NO. 15 FELT 2 x 6 #2 SPF GRADE OR BTR. STUDS @ 16" O.C. P.T. PLATE ANCHORED PER SHEARWALL PLAN 4" CONC. SLAB (2500 PSI. MIN.)-REINFORCED WITH SYNTHETIC FIBERS ON 6 MIL. POLYETHYLENE VAPOR BARRIER, LAPPED 6" @ JOINTS AND SEALED WITH DUCT TAPE OVER TERMITE TREATED COMPACTED FILL 100'-0" (ASSUMED) TOP OF SLAB \_\_\_\_\_\_ -8" CMU STEMWALL REINF. WITH GRADE #5 DOWELS IN FULLY GROUTED CELLS @ CORNERS AND 4'-0" O.C. FOUNDATION PLAN — 12" MIN DISTANCE BELOW GRADE

# TYPICAL WALL SECTION

3/4" = 1'-0"

# SEE PLAN SIMPSON H2.5 PER MANUFACTURER SEE ELEVATIONS TOP OF PLATE VINYL SOFFIT 2x6 SUBFASCIA OVER 1x FURRING 24" O.C. ALUM DRIP EDGE ALUM FASCIA VINYL VENTED SOFFIT SIMPSON HUC212-2 PER MANUFACTURER −P.T. 6x6 SIMPSON ABW66 FASTENED WITH 1/2" TITEN MIN. 5" EMBED AND 12 - 10D 3" TREATED NAILS TOP OF SLAB

### **UPLIFT CONNECTORS**

1. UPLIFT CONNECTORS SUCH AS HURRICANE CLIPS, TRUSS ANCHORS AND ANCHOR BOLTS ARE ONLY REQUIRED ON MEMBERS IN WALLS THAT ARE EXPOSED TO UPLIFT FORCES. INTERIOR LOAD BEARING WALLS ARE NOT ALWAYS EXPOSED TO UPLIFT FORCES. THE MEMBERS OF THESE WALLS WOULD NOT NEED TO HAVE CONNECTORS APPLIED. PLEASE CONSULT THE TRUSS ENGINEERING FOR THE LOCATION OF THESE WALLS.

### FIELD REPAIR NOTES

- 1. MISSED LINTEL STRAPS FOR MASONRY CONSTRUCTION MAY BE SUBSTITUTED W/ (1) "SIMPSON MTSM16 TWIST STRAP W/ (4) 1/4" X 2 1/4" DIA. TITENS TO THE BOND BEAM BLOCK AND (7) 10d TO THE TRUSS FOR UPLIFTS OF 1000 LBS. OR LESS. USE (2) FOR 2000 LBS. OR LESS. OTHERS MAY BE SUBSTITUTED ON A CASE BY CASE BASIS.
- 2. MISSED "J" BOLTS FOR WOOD BEARING WALLS MAY BE SUBSTITUTED W/ 1/2" DIA. ANCHOR BOLTS SET IN 3/4" DIA. X 6" DEEP UNITEX "PROPOXY" 300 ADHESIVE BINDER FOLLOWING ALL MANUFACTURERS RECOMMENDATIONS ( OR 1/2" X 6" RAWL STUD EXPANSION ANCHORS. )
- 3. REGARDING MISSED REBAR IN VERTICAL FILLED CELLS:
  DRILL A 3/4" DIAMETER HOLE 6" DEEP AT THE LOCATION OF
  THE OMITTED REBAR, AND INSTALL A 32" LONG #5 BAR INTO
  THE EPOXY FILLED HOLE. USE A TWO PART EMBEDDEMENT
  EPOXY ( SIMPSON "EPOXY TIE SET", OR HILTI " 2 PART"
  EMBEDDMENT EPOXY ), MIXED PER MANUFACTURER'S
  INSTRUCTIONS. ASSURE THAT ALL DUST AND DEBRIS FROM
  DRILLING ARE REMOVED FROM THE HOLE BY BRUSHING AND
  AND USING COMPRESSED AIR PRIOR TO APPLYING THE EPOXY.
  ALLOW THE EPOXY TO CURE TO MANUFACTURER'S SPECIFICATIONS,
  THEN FILL THE CELL IN THE NORMAL WAY DURING BOND BEAM
  POUR
- 4. HURRICANE STRAPS MAY BE SUBSTITUTED WITH A STRAP OF GREATER HOLDOWN VALUE OR GREATER UPLIFT VALUE IN THE FIELD WITHOUT VERIFICATION, PROVIDED ALL MANUFACTURERS
- INSTALLATION INSTRUCTIONS ARE FOLLOWED.

  5. FOR MORTER JOINTS LESS THAN 1/4", PROVIDE (1) #5 VERT.
  IN CONC. FILLED CELL EACH SIDE OF THE JOINT ( BAR DOES NOT HAVE TO BE CONT. TO FOOTING )

### **GENERAL NOTES:**

DATE

- 1. THE CONTRACTOR SHALL INDEMNIFY THE OWNER AGAINST ALL CLAIMS, WHETHER FROM PERSONAL INJURY OR PROPERTY DAMAGE, ARISING FORM EVENTS ASSOCIATED WITH THE WORK PERFORMED UNDER THE CONTRACT FOR THIS PROJECT.
- 2. THE CONTRACTOR AND/OR SUB-CONTRACTORS SHALL WARRANT ALL WORK FOR A PERIOD OF ONE YEAR FOLLOWING THE WORK DATE OF FINAL COMPLETION AND ACCEPTANCE BY THE OWNER DEFECTS IN MATERIALS, EQUIPMENT, COMPONENTS AND WORK-MANSHIP SHALL BE CORRECTED AT NO FURTHER COST TO THE OWNER DURING THE ONE YEAR WARRANTY PERIOD.
- 3. AT THE OWNER'S OPTION, A WARRANTY INSPECTION SHALL BE PERFORMED DURING THE ELEVENTH MONTH FOLLOWING THE COMMENCEMENT OF THE WARRANTY PERIOD, FOR THE PURPOSE OF DETERMINING ANY WARRANTY WORK THAT MAY BE REQUIRED. THE CONTRACTOR SHALL BE PRESENT DURING THIS INSPECTION IF REQUESTED BY THE OWNER.
- 4. THE CONTRACTOR SHALL PAY FOR ALL PERMITS, LICENSES, TESTS AND THE LIKE THAT MAY BE REQUIRED BY THE VARIOUS AUTHORITIES HAVING JURISDICTION OVER THIS PROJECT BE THEY CITY, COUNTY, STATE OR FEDERAL.

- THE OWNER SHALL FILE A "NOTICE OF COMMENCEMENT" PRIOR TO THE BEGINNING OF THE PROJECT AND THE CONTRACTOR(S) SHALL FILE "NOTICE TO OWNER" AND PROVIDE "RELEASE OF LIEN" FOR ALL PAYMENT REQUESTS PRIOR TO DISBURSEMENT OF ANY FUNDS.
- ANY AND ALL DISPUTES ARISING FROM EVENTS ASSOCIATED WITH THE CONSTRUCTION OF THIS PROJECT BETWEEN THE OWNER, CONTACTOR(S) AND SUPPLIERS SHALL BE RESOLVED THROUGH BINDING ARBITRATION.
- ALL WORK SHALL BE IN ACCORDANCE WITH APPLICABLE CODES AND LOCAL REGULATIONS, INCLUDING APPLICABLE ENERGY CODES. ALL COMPONENTS OF THE BUILDING SHALL MEET WITH THE MINIMUM ENERGY REQUIREMENTS OF THE BUILDING CODE. ANY DISCREPANCIES SHALL BE REPORTED TO THE ARCHITECT IN WRITING PRIOR TO THE COMMENCEMENT OF THE WORK.
- 8. ALL INSULATION SHALL BE LEFT EXPOSED AND ALL LABELS LEFT INTACT ON THE WINDOWS AND DOORS UNTIL INSPECTED BY THE BUILDING OFFICIAL.
- ALL WOOD IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED.

## CONSTRUCTION DOCUMENTS:

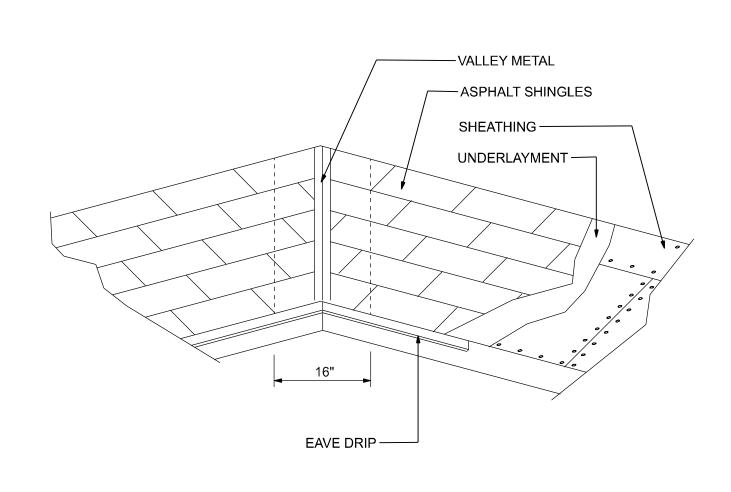
THE CUSTOMER IS RESPONSIBLE FOR DELIVERING THE REQUIRED SETS OF CONSTRUCTION DOCUMENTS TO THE PERMIT ISSUING AUTHORITY FOR THE ISSUANCE OF CONSTRUCTION PERMITS. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR REVIEWING THE PLANS AND VERIFYING ALL EXISTING CONDITIONS, ELEVATIONS, AND DIMENSIONS PRIOR TO COMMENCING CONSTRUCTION INCLUDING FABRICATION. ALL DISCREPANCIES SHALL BE REPORTED TO THE ARCHITECT/ENGINEER FOR RESOLUTION.

### DO NOT SCALE THESE PLANS:

AMPLE DIMENSIONS ARE SHOWN ON THE PLANS TO LOCATE ALL ITEMS.
SIMPLE ARITHMATIC MAY BE USED TO DETERMINE THE LOCATION OF THOSE ITEMS NOT DIMENSIONED.

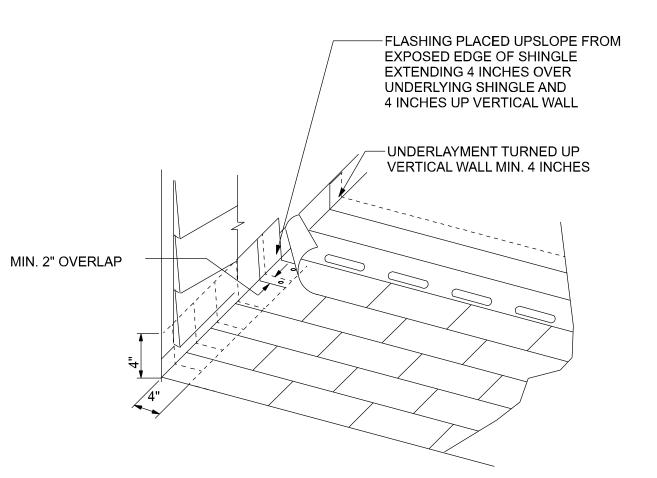
### **CHANGES TO PLAN SETS:**

PLEASE DO NOT MAKE ANY STRUCTURAL CHANGES TO THES PLANS WITHOUT CONSULTING WITH THE ARCHITECT/ENGINEER. THE OWNER SHALL ASSUME ANY AND ALL LIABILITY FOR STRUCTURAL DAMAGE RESULTING FROM CHANGES MADE TO THE PLANS OR BY SUBSTITUTION OF MATERIALS DIFFERENT FROM SPECIFICATIONS ON THE PLANS.



TYP. PORCH SECTION

SCALE: NTS



	REVISIONS	DESIGN BY:
BY	DESCRIPTION	DEGIGIT DT.
		TRADEMARK Construction Group,

CERTIFIED GENERAL CONTRACTOR CGC1514780

> 163 SW MIDTOWN PL. STE. 101 LAKE CITY, FL. 32025 (386)755-5254



CERTIFICATE OF AUTHORIZATION NO. 28022

> P.O. BOX 970 LAKE CITY, FL 32056 PHONE: 386.754.4085

Brett A. Crews
Digitally signed by
Brett A. Crews
Date: 2022.03.10
08:44:41-05'00'

Brett A. Crews, P.E. 65592

STATE OF

y APPF

TM
APPROVED BY:

DRAWN BY:

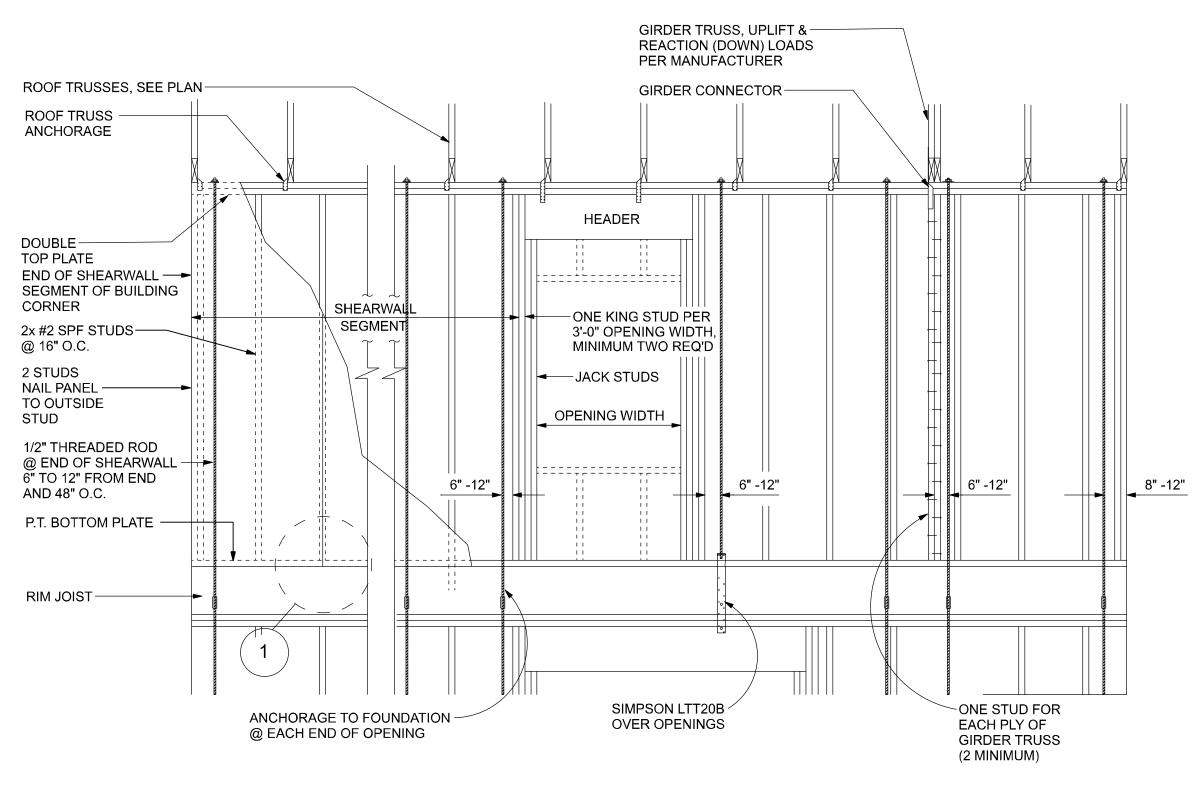
FOR FOOTING DETAILS

GREEN RESIDENCE

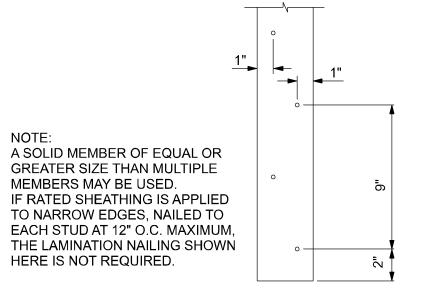
PROJECT NO.:
R21.006

SECTIONS AND FRAMING DETAILS

A-10

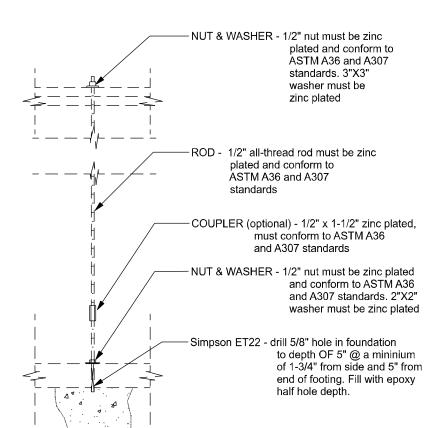


## SHEARWALL DETAILS SCALE: 1/2" = 1'-0"



END (TOP OR BOTTOM)

### GIRD. COL. DETAIL SCALE: 1/2" = 1'-0"



1. One all-thread rod 6" - 12" from each corner. 2. One all-thread rod at each end of shearwalls. 3. One all-thread rod at each end of opening headers. 4. Check sub-sheathing to top plate connection for horizontal transfer capability. 5. If necessary, add all-thread rods to girders individually to exclude the from average uplift plf. 6. Check sole plate to slab connection, additional anchors may be required for lateral and shear load transfer.

ALLOWABLE VALUES					
Connection Type	Allowable Value				
Foundation / S.Y.P. Top Plate	3840 lbs.				
Foundation / Spruce-Pine-Fir Top Plate	3840 lbs.				
Lintel or Bond Beam / S.Y.P. Top Plate	3840 lbs				
Lintel or Bond Beam / Spruce-Pine-Fir Top Plate	3840 lbs				

### Placement at slab level:

Corners

When presetting the all-thread rod at a building corner, the rod should be placed 8 to 12 inches away from the corner so it does not set under the corner framing members. When a all-thread rod is specified at a building corner, it may be placed on either side of the corner. Header ends

When presetting the all-thread rod at a header end, the rod should be placed 8 to 12 inches away from the header end so it does not

fall under the stud pack framing members. Top Connections

Top connections made at corners and header ends shall be made within 2 inches of the framing pack. A nut and 3X3 washer shall be applied to the top plates and tightened securely.

Intermediate Coupler Connections

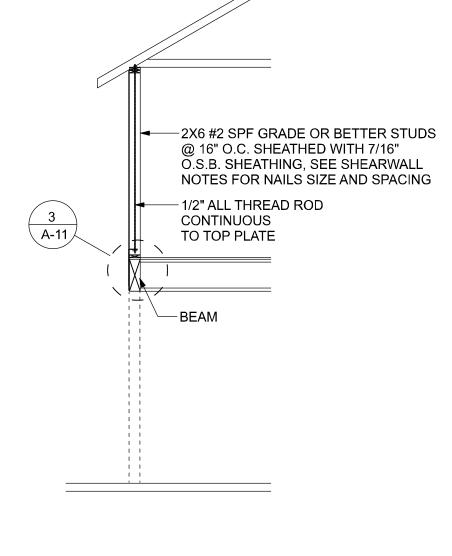
When using the rod coupler, care should be taken to ensure full and equal thread engagement. This is easily achieved by threading the coupler all the way onto the rod, then standing the two rods end to end, then threading the coupler back over the rod joint so each rod is halfway into the coupler.

Retro-fits In the case of an all thread rod misplacement, the rod may be epoxied into the concrete.

Sole plate to slab connection:

The slab level sole plate shall be connected to the slab with the connectors specified and at the spacing specified within the design documents. All-thread rods shall be placed as per the design specifications. All-thread rods with a nut and washer at the sole plate will qualify as a sole plate connection but may require other anchors intermediate of the all-thread rod locations to qualify the specified spacing requirements.

System Tightening:
On multiple story applications, the all-thread rod system shall be rechecked for proper tension just before the walls are veneered. This will allow the all-thread rod system to compensate for the buildings dead load compression.



-2X6 #2 SPF GRADE OR BETTER STUDS @ 16" O.C. SHEATHED WITH 7/16" O.S.B. SHEATHING, SEE SHEARWALL NOTES FOR NAILS SIZE AND SPACING - 1/2" ALL THREAD ROD CONTINUOUS TO TOP PLATE -2X6 #2 SPF GRADE OR BETTER STUDS @ 16" O.C. SHEATHED WITH 7/16" O.S.B. SHEATHING, SEE SHEARWALL NOTES FOR NAILS SIZE AND SPACING

2 STORY SECTION SCALE: 1/4" = 1'-0"

**HEADER SIZE** 

#2 GRADE OR

BETTER

(2) 1 3/4" x 11 1/4" LVL - 2.0E

(2) 1 3/4" x 11 1/4" LVL - 2.0E

(2) 1 3/4" x 11 1/4" LVL - 2.0E

(2) 2x8

(2) 2x10

(2) 2x12

2 STORY SECTION SCALE: 1/4" = 1'-0"

**CONNECTOR AT** 

EACH END OF

OPENING

1/2" ALL THREAD ROD

ANCHORAGE TO

FOUNDATION @ EACH

END OF OPENING

1/2" ALL THREAD ROD

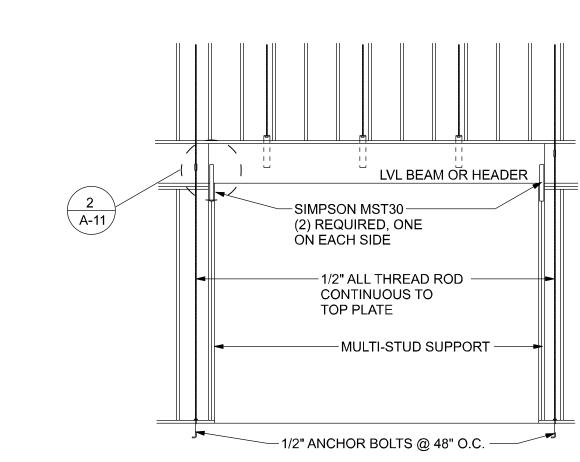
NOTES

CONSULT TRUSS ENGINEERING FOR UPLIFT

SEE FRAMED HEADER CONNECTION REQ.

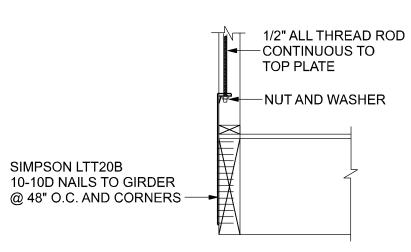
SEE FRAMED HEADER CONNECTION REQ.

ONE PER FULL HEIGHT KING STUD



# LOAD PATH AT OPENINGS

SCALE: 1/4" = 1'-0" NOTE: USE FOR OPENINGS OVER 6'-4" WIDE



6" - 12"

MAX.

GIRDER/WALL BRG. DTL.

1/2" ALL THREAD ROD —

/SCALE: 3/4" = 1'-0"

## BEAM/WALL HOLD DOWN /SCALE: 3/4" = 1'-0"

-SIMPSON MST30

ON EACH SIDE

-BEARING PER

-MULTI STUD OR

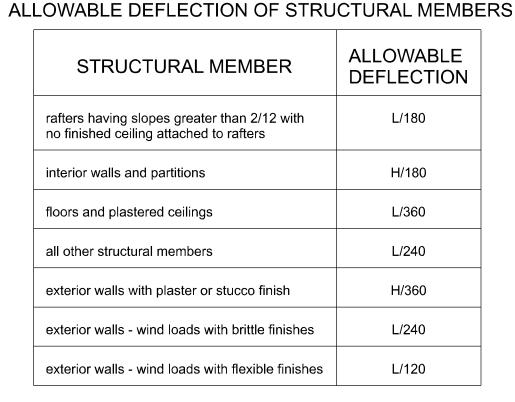
SOLID POST OF

EQUAL SIZE

**MANUFACTURER** 

GIRDER

(2) REQUIRED, ONE



# WINDSTORM 7/16" O.S.B. FULL HEIGHT SHEATHING **→** 3/8"

DOUBLE NAIL EDGE SPACING TOP AND BOTTOM PLATE UPLIFT CAPACITY = 474 plf (TABLE 305S1 SSTD10-99)

SHEATHING IN LIEU OF ALL THREAD

OPTIONAL NAILING PATTERN WITH WINDSTORM

CLEAR

**OPENING** 

WIDTH

0' - 3'

>3' - 6'

>6' - 9'

>9' - 12'

>12' - 15'

>15' - 18'

LOCATION

TRUSS/RAFTER CONNECTION

HEADER CONNECTION

TOP PLATE TO STUDS

STUD TO BEAM (header)

BEAM TO POST

POST TO GIRDER

SHEARWALL NOTES:

MAKE

SIMPSON

SIMPSON

SIMPSON

SIMPSON

SIMPSON

SIMPSON

1. ALL SHEARWALLS SHALL BE TYPE 2 SHEARWALLS AS DEFINED BY STD 10-99 305.4.3.

OPENING CONNECTION REQUIREMENTS

**END BEARING** 

1.5"

3"

4.5"

LSTA30

LSTA30

LSTA30

HD2A

OPTIONAL HOLD DOWN CONNECTORS

| SPH4/SPH6 | 48" O.C.

MODEL SPACING

EACH

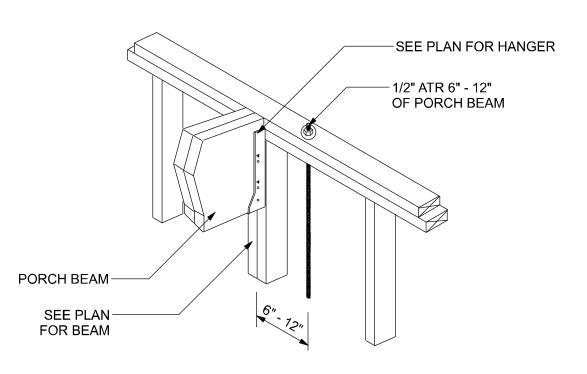
2. THE WALL SHALL BE ENTIRELY SHEATHED WITH 7/16" O.S.B. INCLUDING AREAS ABOVE AND BELOW

3. ALL SHEATHING SHALL BE ATTACHED TO FRAMING ALONG ALL FOUR EDGES WITH JOINTS FOR ADJACENT PANELS OCCURING OVER COMMON FRAMING MEMBERS OR ALONG BLOCKING.

4. NAIL SPACING SHALL BE 6" O.C. EDGES AND

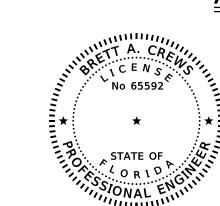
12" O.C. IN THE FIELD. TYPE 2 SHEARWALLS ARE DESIGNED FOR THE OPENING IT CONTAINS. MAXIMUM HEIGHT OF OPENING SHALL BE 5/6 TIMES THE WALL HEIGHT. THE MINIMUM DISTANCE BETWEEN OPENINGS SHALL BE THE WALL HEIGHT/3.5 ie. FOR 8'-0" WALLS - (2'-3").

OPENING WIDTH	SILL PLATES	16d TOE NAILS EACH END
UP TO 6'-0"	(1) 2x4 OR (1) 2x6	1
> 6' TO 9'-0"	(3) 2x4 OR (1) 2x6	2
> 9' TO 12'-0"	(5) 2x4 OR (2) 2x6	3



# ALL THREAD @ PORCH BEAM





	REVISIONS	DESIGN BY:	CERTIFIED GENERAL CONTRACTOR		CERTIFICATE OF AUTHORIZATION		DRAWN BY:		PROJECT NO.:
DATE BY	DESCRIPTION		CGC1514780	CEC	CERTIFICATE OF AUTHORIZATION NO. 28022	Digitally signed by	TM	GREEN RESIDENCE	R21.006
		TRADEMARK	163 SW MIDTOWN PL.			Brett A. Crews Date: 2022.03.10			
			STE. 101 LAKE CITY, FL. 32025		P.O. BOX 970 LAKE CITY, FL 32056	08:45:43-05'00'	APPROVED BY:	SHEARWALL AND	SHEET:
		Construction Group, In	(386)755-5254	Crews Engineering Services, LLC	PHONE: 386.754.4085	Brett A. Crews, P.E. 65592	BC	SECTION DETAILS	A-11
						DICE / 1. 010W3, 1.L. 0000L			