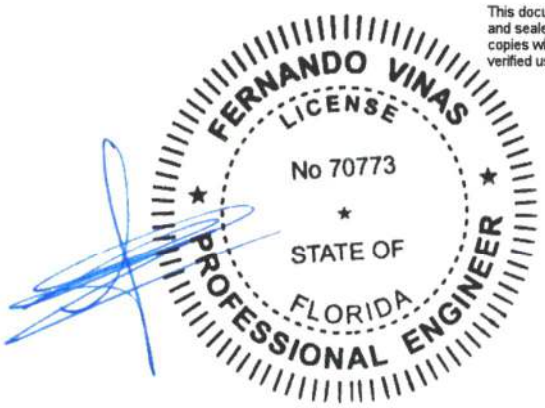

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07/14/2023

COA#0-278
Florida Certificate of Product Approval #FL1999

Site Information:	Page 1:
Customer: Trussco of Georgia	*Job Number:* 0842
Job Description: Fred Perry / Lear Job	
Address: LAKE CITY, FL	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	*IntelliVIEW Version:* 22.02.01 *JRef #:* 1XRc96160001
Wind Standard: ASCE 7-16 *Wind Speed (mph):* 115 *Building Type:* Closed	*Design Loading (psf):* 40.00

This package contains general notes pages, 6 truss drawing(s) and 4 detail(s).

Item	Drawing Number	Truss
1	194.23.1006.53160	T1
3	194.23.1007.08953	S1
5	194.23.1007.22897	T3
7	BRCLBSUB0119	
9	GABRST160118	

Item	Drawing Number	Truss
2	194.23.1007.06880	T1A
4	194.23.1007.21717	G1
6	194.23.1007.24990	G2
8	A11515ENC160118	
10	GBLLETIN0118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

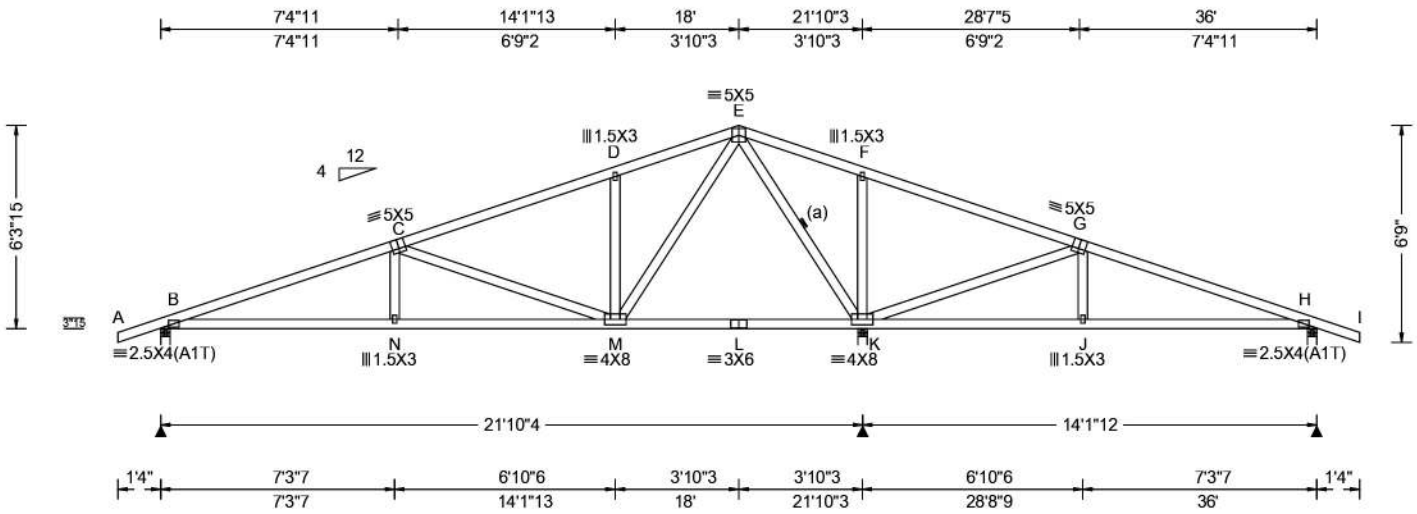
BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
FRT-PR = ProWood Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com.

SEQN: 306525 COMN	Ply: 1	**Job Number:** 0842	Cust: R 9616 JRef: 1XRc96160001 T1
FROM: RER	Qty: 12	Fred Perry / Lear Job	DrwNo: 194.23.1006.53160
		Truss Label: T1	SSB / WHK 07/13/2023



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**				
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in	loc	L/defl	L/#	
TCDL:	10.00	Speed: 115 mph	Pf: NA Ce: NA	VERT(LL):	0.062 N	999	240	
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL):	0.127 N	999	180	
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL):	0.015 H	-	-	
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL):	0.031 K	-	-	
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0				
Soffit:	2.00	TCDL: 6.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI:	0.717			
Load Duration:	1.25	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI:	0.599			
Spacing:	24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI:	0.949			
		C&C Dist a: 3.60 ft	FT/RT/PT:20(0)/10(0)/2(0)					
		Loc. from endwall: Any	Plate Type(s):	VIEW Ver: 22.02.01.1115.14				
		GCpi: 0.18	WAVE					
		Wind Duration: 1.25						

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	826	/-	/-	/420	/33	/69
K	2001	/-	/-	/969	/19	/-
H	450	/-	/-	/219	/30	/-

Wind reactions based on C&C
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
K Brg Wid = 3.5 Min Req = 2.0 (Truss)
H Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, K, & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	33	-1535	E - F	888	0
C - D	63	-660	F - G	921	0
D - E	113	-639	G - H	290	-401

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - N	1406	0	N - M	1400	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - M	46	-898	E - K	26	-1338
M - E	1021	-35	K - G	47	-936

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
BC	75	0.15	35.85

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design & reactions.

Wind loading based on both gable and hip roof types.

Additional Notes

The calculated vertical deflections are LL: 0.06; DL: 0.08; CR: 0.08; for a total long term deflection of 0.20. Creep increase factor for dead load is 2.00.

Top Chord overhang(s) may be field trimmed.

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07/14/2023

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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155 Harlem Ave
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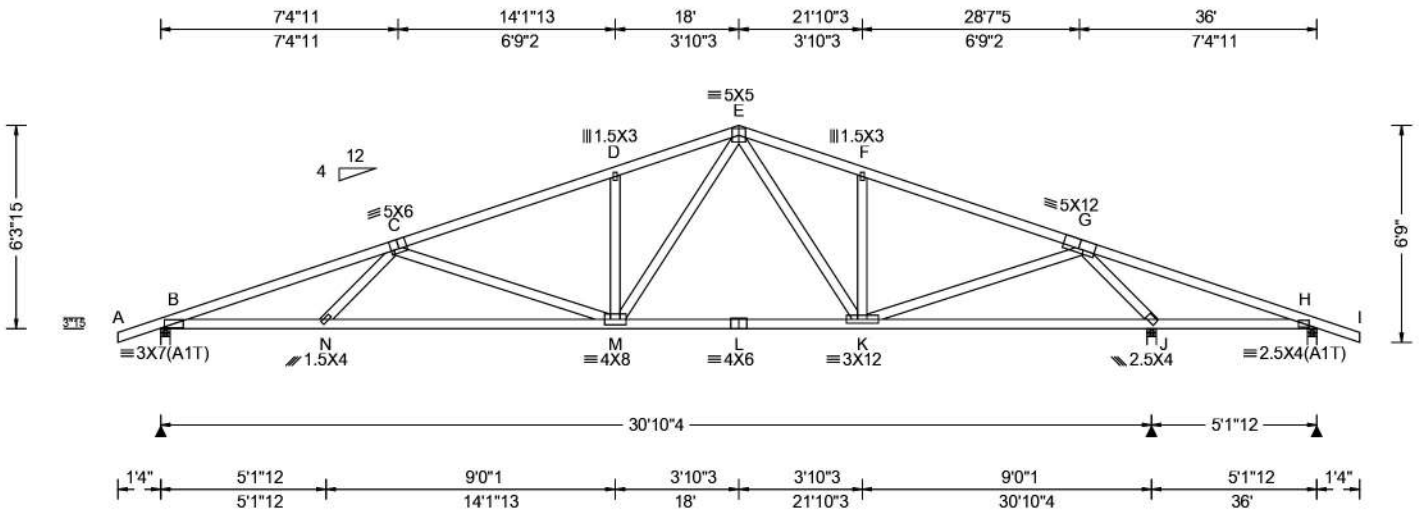
Glenview, IL 60025

SEQN: 306572 COMN FROM: RER	Ply: 1 Qty: 3	**Job Number:** 0842 Fred Perry / Lear Job **Truss Label:** T1A		Cust: R 9616 JRef: 1XRc96160001 T7 DrwNo: 194.23.1007.06880 SSB / FV 07/13/2023



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 115 mph	Pf: NA Ce: NA	VERT(LL): 0.164 C 999 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.313 C 999 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.045 J - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL): 0.088 J - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 6.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.803
Load Duration:	1.25	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.990
Spacing:	24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.841
		C&C Dist a: 3.60 ft	FT/RT/PT:20(0)/10(0)/2(0)	
		Loc. from endwall: Any	Plate Type(s):	
		GCpi: 0.18	WAVE	VIEW Ver: 22.02.01.1115.14
		Wind Duration: 1.25		

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1311	/-	/-	/660	/39	/69
J	1912	/-	/-	/946	/-	/-
H	127	/-289	/-	/20	/90	/-

Wind reactions based on C&C
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
J Brg Wid = 3.5 Min Req = 1.9 (Truss)
H Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, J, & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	7	-3086	E - F	136	-1679
C - D	85	-2174	F - G	88	-1708
D - E	134	-2152	G - H	1173	-12

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - N	2874	0	L - K	1448	0
N - M	2756	-39	K - J	462	-35
M - L	1448	0	J - H	41	-1062

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
N - C	384	0	K - G	1241	0
C - M	81	-803	G - J	107	-2240
M - E	1017	-27			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
BC	59	0.15	35.85

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design & reactions.

Wind loading based on both gable and hip roof types.

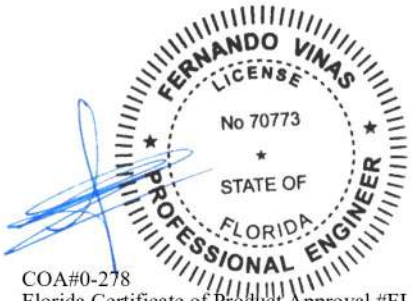
Additional Notes

Negative reaction(s) of -289# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

The calculated vertical deflections are LL: 0.16; DL: 0.03; CR: 0.03; for a total long term deflection of 0.08. Creep increase factor for dead load is 2.00.

Top Chord overhang(s) may be field trimmed.

WARNING! This truss is not symmetric, but its exterior geometry makes erection error more probable. It is imperative that this truss be installed properly.



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07/14/2023

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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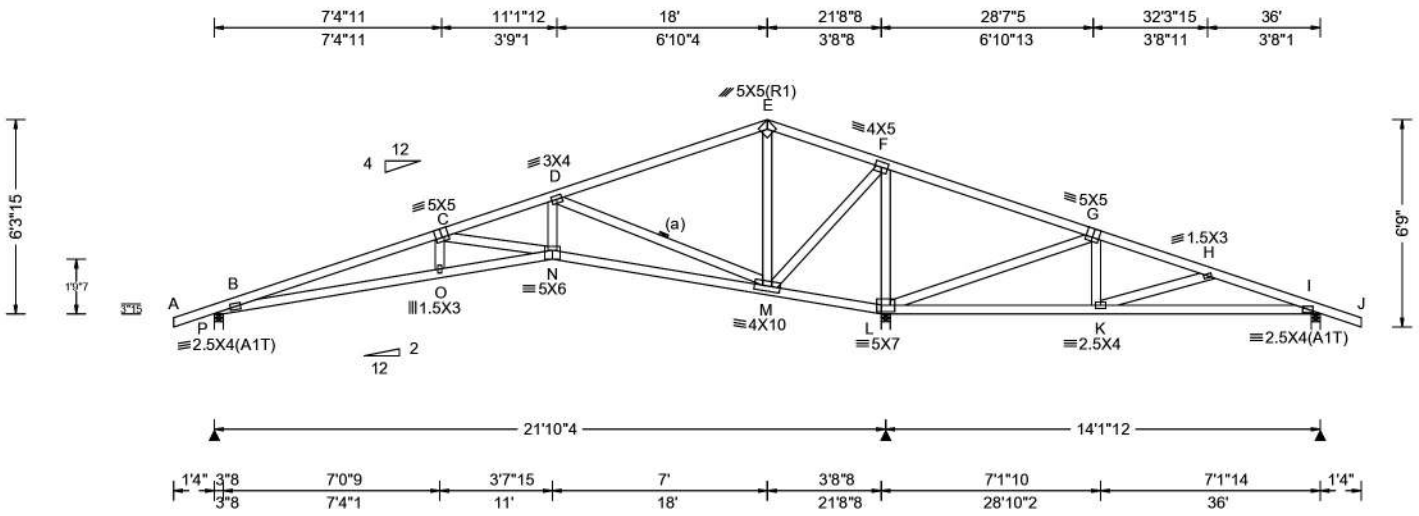
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 306569 COMN FROM: RER	Ply: 1 Qty: 8	**Job Number:** 0842 Fred Perry / Lear Job **Truss Label:** S1		Cust: R 9616 JRef: 1XRc96160001 T5 DrwNo: 194.23.1007.08953 SSB / FV 07/13/2023



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 115 mph	Pf: NA Ce: NA	VERT(LL): 0.105 C 999 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.233 C 999 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.042 L - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL): 0.097 L - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 6.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.755
Load Duration:	1.25	BCDL: 6.0 psf	TPI Std: 2014	Max BC CSI: 0.994
Spacing:	24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.943
		C&C Dist a: 3.60 ft	FT/RT/PT:20(0)/10(0)/2(0)	
		Loc. from endwall: not in 9.00 ft	Plate Type(s):	
		GCpi: 0.18	WAVE	VIEW Ver: 22.02.01.1115.14
		Wind Duration: 1.25		

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
P	647	/-	/-	/324	/46	/69
L	2299	/-	/-	/1161	/-	/-
I	365	/-195	/-	/134	/50	/-

Wind reactions based on C&C
P Brg Wid = 3.5 Min Req = 1.5 (Truss)
L Brg Wid = 3.5 Min Req = 2.7 (Truss)
I Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings P, L, & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	0	-1714	F - G	1644	0
C - D	26	-806	G - H	991	-76
D - E	703	0	H - I	832	-368
E - F	668	0			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - O	1615	0	M - L	32	-1540
O - N	1596	0	L - K	24	-950
N - M	722	0	K - I	334	-767

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - N	5	-861	M - F	1352	0
N - D	638	0	L - F	18	-1629
D - M	3	-1331	L - G	6	-936
M - E	25	-706			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
BC	120	0.15	11.00
BC	56	11.00	21.71
BC	70	21.71	35.85

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

Wind

Wind loads based on MWFRS with additional C&C member design & reactions.

Wind loading based on both gable and hip roof types.

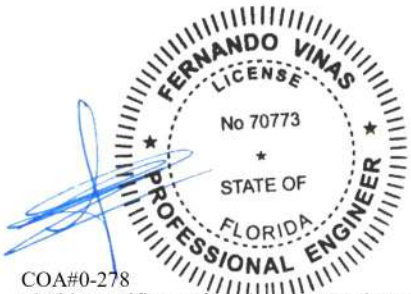
Additional Notes

Negative reaction(s) of -195# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

The calculated vertical deflections are LL: 0.11; DL: 0.15; CR: 0.15; for a total long term deflection of 0.38. Creep increase factor for dead load is 2.00.

Shim all supports to solid bearing.

Top Chord overhang(s) may be field trimmed.



COA#0-278
Florida Certificate of Product Approval #FL1999
07/14/2023

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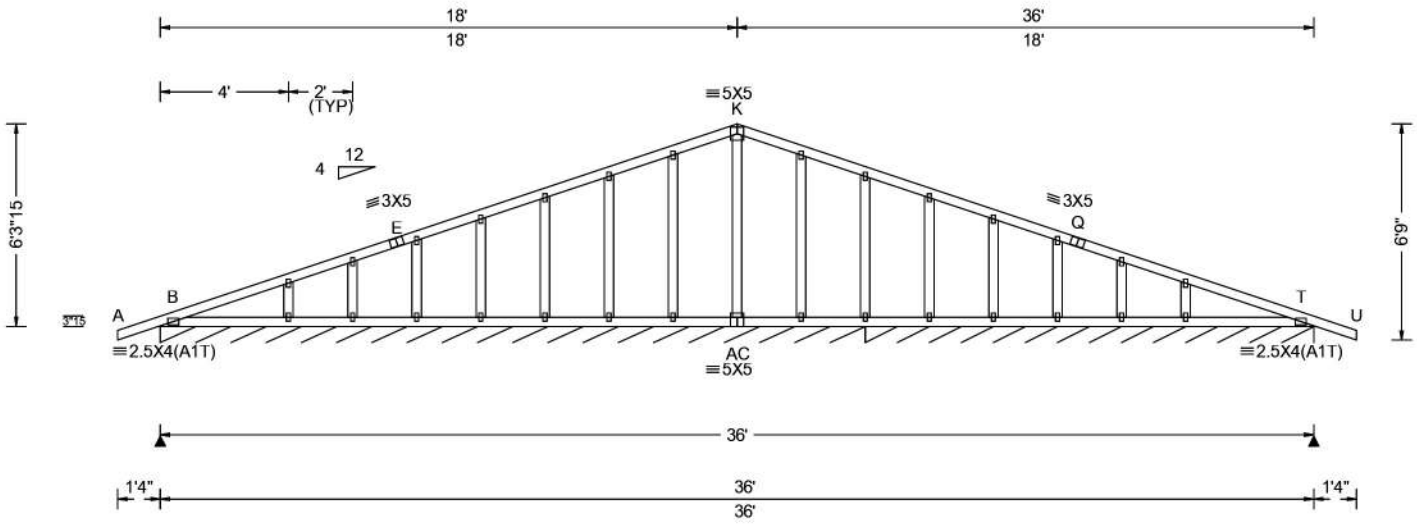
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 306575 GABL FROM: RER	Ply: 1 Qty: 2	**Job Number:** 0842 Fred Perry / Lear Job **Truss Label:** G1		Cust: R 9616 JRef: 1XRc96160001 T2 DrwNo: 194.23.1007.21717 SSB / FV 07/13/2023	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 115 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.60 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.25	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT/PT:20(0)/10(0)/2(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.005 T 999 240 VERT(CL): 0.010 T 999 180 HORZ(LL): -0.002 T - - HORZ(TL): 0.004 T - - Creep Factor: 2.0 Max TC CSI: 0.142 Max BC CSI: 0.120 Max Web CSI: 0.078 VIEW Ver: 22.02.01.1115.14

▲ Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	88	/-	/-	/45	/2	/3
T*	81	/-	/-	/44	/4	/-

Wind reactions based on C&C
B Brg Wid = 264 Min Req = -
T Brg Wid = 168 Min Req = -
Bearings B & AA are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Purlins

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
BC	75	0.15	35.85

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

Wind

Wind loads based on MWFRS with additional C&C member design & reactions.

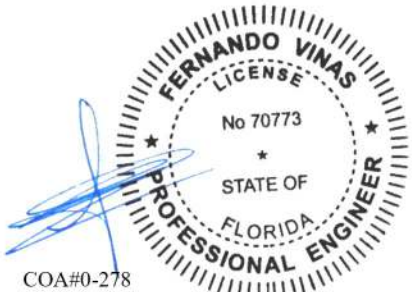
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A11515ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

The calculated vertical deflections are LL: 0.00; DL: 0.00; CR: 0.00; for a total long term deflection of 0.01. Creep increase factor for dead load is 2.00.

Top Chord overhang(s) may be field trimmed.



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Florida Certificate of Product Approval #FL1999
07/14/2023

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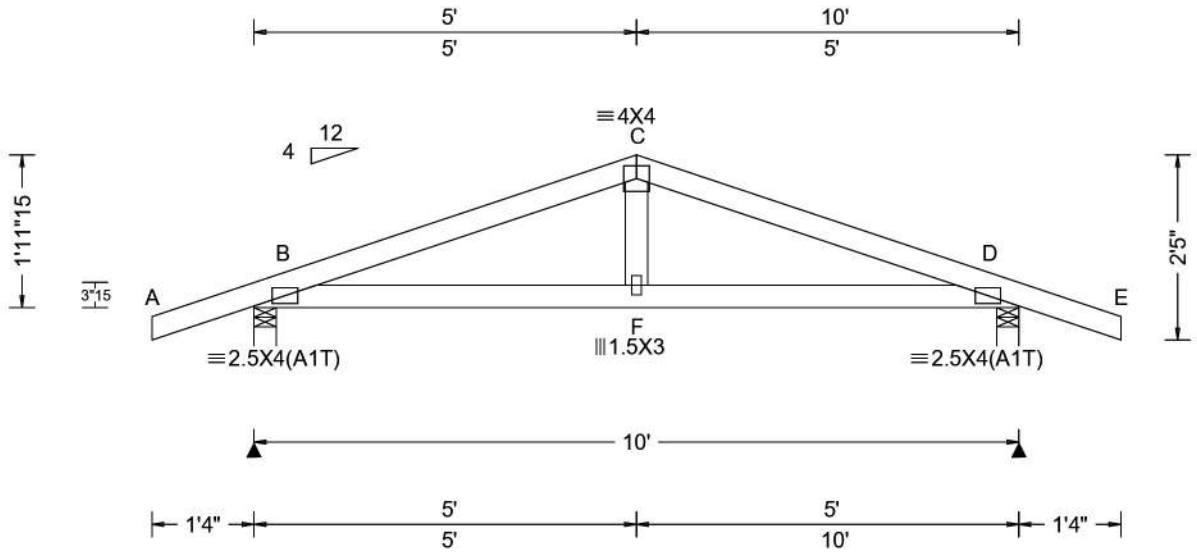
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 217149 COMN FROM: RER	Ply: 1 Qty: 5	**Job Number:** 0842 Fred Perry / Lear Job **Truss Label:** T3		Cust: R 9616 JRef: 1XRc96160001 T3 DrwNo: 194.23.1007.22897 SSB / FV 07/13/2023	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 115 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 6.0 psf
BCDL: 6.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.25

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT/PT:20(0)/10(0)/2(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.014	F	999	240
VERT(CL): 0.028	F	999	180
HORZ(LL): 0.005	D	-	-
HORZ(TL): 0.009	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.207
Max BC CSI: 0.285
Max Web CSI: 0.078

VIEW Ver: 22.02.01.1115.14

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	493	/-	/-	/247	/193	/24
D	493	/-	/-	/247	/193	/-

Wind reactions based on C&C
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & D are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	215	-717	C - D	215	-717

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - F	649	-123	F - D	649	-123

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
BC	75	0.15	9.85

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

Wind

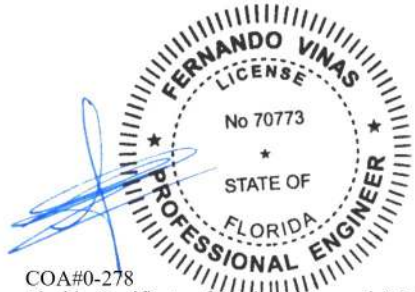
Wind loads based on MWFRS with additional C&C member design & reactions.

Wind loading based on both gable and hip roof types.

Additional Notes

The calculated vertical deflections are LL: 0.01; DL: 0.01; CR: 0.01; for a total long term deflection of 0.04. Creep increase factor for dead load is 2.00.

Top Chord overhang(s) may be field trimmed.



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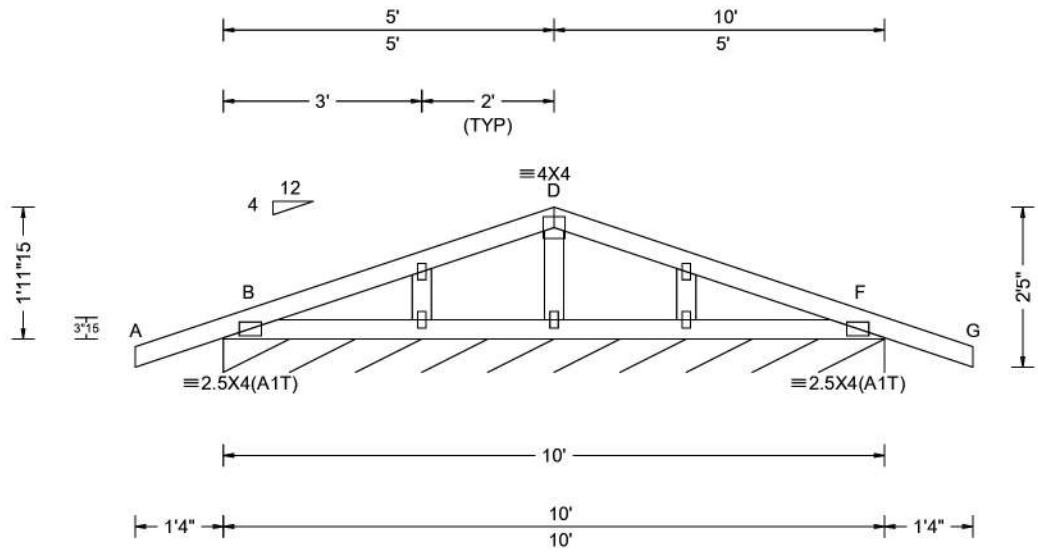
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 217147 GABL FROM: RER	Ply: 1 Qty: 1	**Job Number:** 0842 Fred Perry / Lear Job **Truss Label:** G2		Cust: R 9616 JRef: 1XRc96160001 T4 DrwNo: 194.23.1007.24990 SSB / FV 07/13/2023



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 115 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 6.0 psf BCDL: 6.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.25	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT/PT:20(0)/10(0)/2(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 F 999 240 VERT(CL): 0.004 F 999 180 HORZ(LL): -0.001 F - - HORZ(TL): 0.002 F - - Creep Factor: 2.0 Max TC CSI: 0.119 Max BC CSI: 0.056 Max Web CSI: 0.043 VIEW Ver: 22.02.01.1115.14

▲ Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
F*	97	/-	/-	/45	/37	/2

Wind reactions based on C&C
F Brg Wid = 120 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Purlins

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
BC	75	0.15	9.85

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

Wind

Wind loads based on MWFRS with additional C&C member design & reactions.

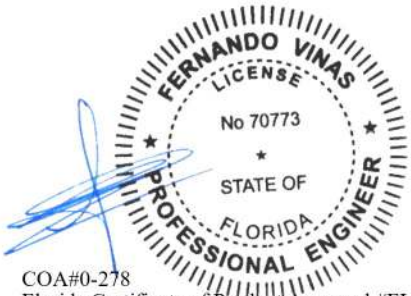
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A11515ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

The calculated vertical deflections are LL: 0.00; DL: 0.00; CR: 0.00; for a total long term deflection of 0.01. Creep increase factor for dead load is 2.00.

Top Chord overhang(s) may be field trimmed.



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AN ITW COMPANY
155 Harlem Ave
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Glenview, IL 60025

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

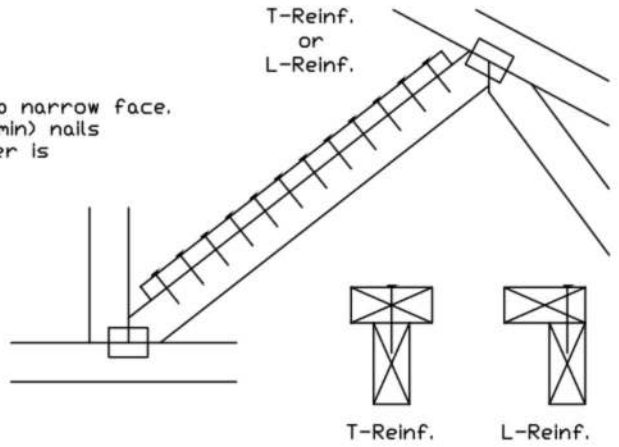
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

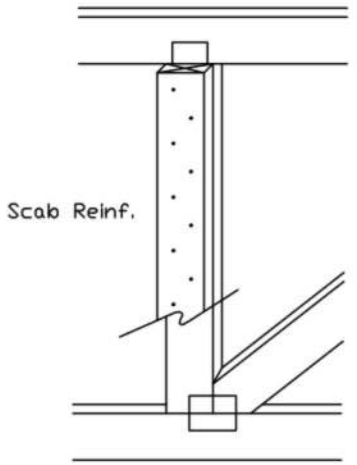
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

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No 70773
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.	07/14/2023	
SPACING		

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