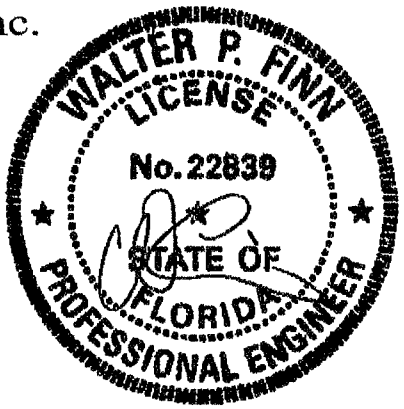
Permit # 31820

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
 Document ID 1V5F487-Z0210140604



Truss Fabricator.	**Anderson Truss Company**
Job Identification.	**14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL**
Truss Count.	**21**
Model Code	**Florida Building Code 2010**
Truss Criteria	**FBC2010Res/TPI-2007(STD)**
Engineering Software	**Alpine Software,Version 12.03.**
Structural Engineer of Record.	**The identity of the structural EOR did not exist as of**
Address	**the seal date per section 61G15-31.003(5a) of the FAC**
Minimum Design Loads	**Roof - 37.0 PSF @ 1.25 Duration**
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

04/10/2014

Notes

1. **Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1**
2. **The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.**
3. **As shown on attached drawings; the drawing number is preceded by: HCUSR9114**

Walter P. Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-

#	Ref	Description	Drawing#	Date
1	53125--A	27'3"8 Common	14100005	04/10/14
2	53126-A1	27'3"8 Common	14100009	04/10/14
3	53127-A2	27'3"8 Common	14100012	04/10/14
4	53128-AGE	27'3"8 Gable	14100008	04/10/14
5	53129--B1	30' Common	14100006	04/10/14
6	53130--B2	30' Common	14100003	04/10/14
7	53131--B3	37' Common	14100004	04/10/14
8	53132--B4	37' Common	14100005	04/10/14
9	53133--BGE	30' Gable	14100007	04/10/14
10	53134--BGE1	37' Common	14100010	04/10/14
11	53135--BGE2	37' Gable	14100006	04/10/14
12	53136--C	14'4" Mono	14100004	04/10/14
13	53137--CJ2	1'3"4 Jack	14100007	04/10/14
14	53138--CJ3	3' Jack	14100008	04/10/14
15	53139--CJ5	5' Jack	14100009	04/10/14
16	53140--EJ7	7' End Jack	14100010	04/10/14
17	53141-H7	14'4" Mono Hi	14100013	04/10/14
18	53142-H9	14'4" Mono Hi	14100001	04/10/14
19	53143-HJ7	9'10"13 Hip	14100011	04/10/14
20	53144-H11	14'4" Mono H	14100002	04/10/14
21	53145-H13	14'4" Mono H	14100003	04/10/14

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - A 27'3"8 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member.

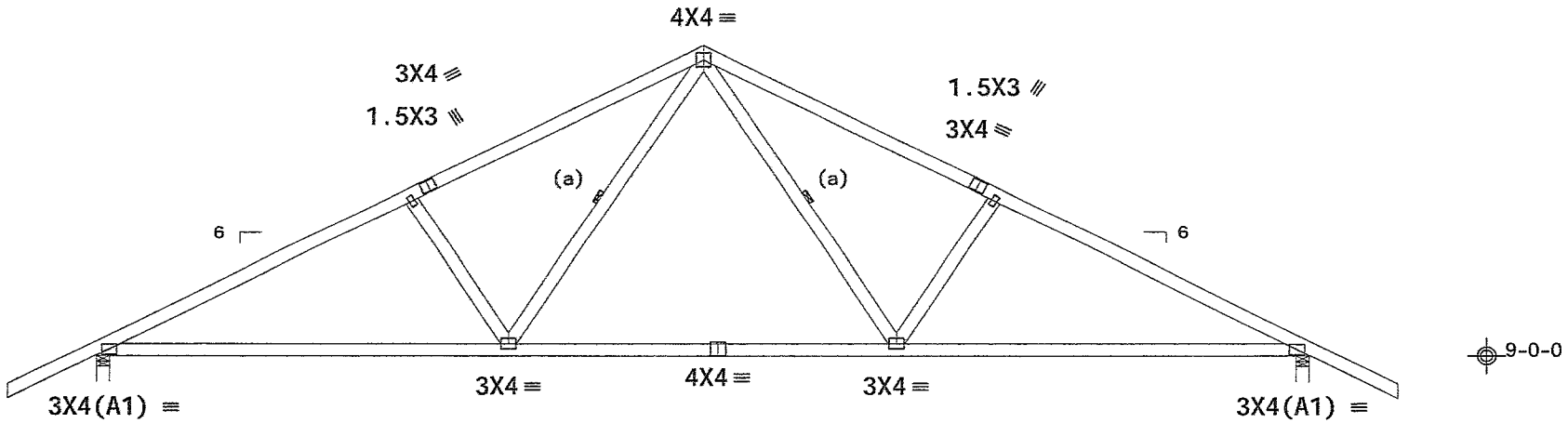
Bottom chord checked for 10.00 psf non-concurrent live load.

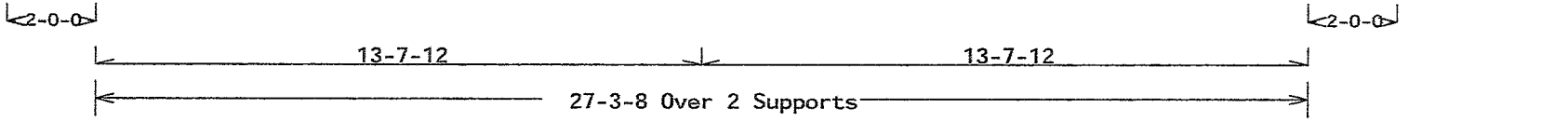
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=1227 U=51 W=3.5"
RL=129/-129

R=1228 U=51 W=3.5"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible] QTY:13 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating handling shipping installing and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this desi any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipping installation bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/10/2014

TC LL	20.0 PSF	REF	R9114- 53125
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100005
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	330983
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V5F487_Z02

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - A1 27'3"8 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

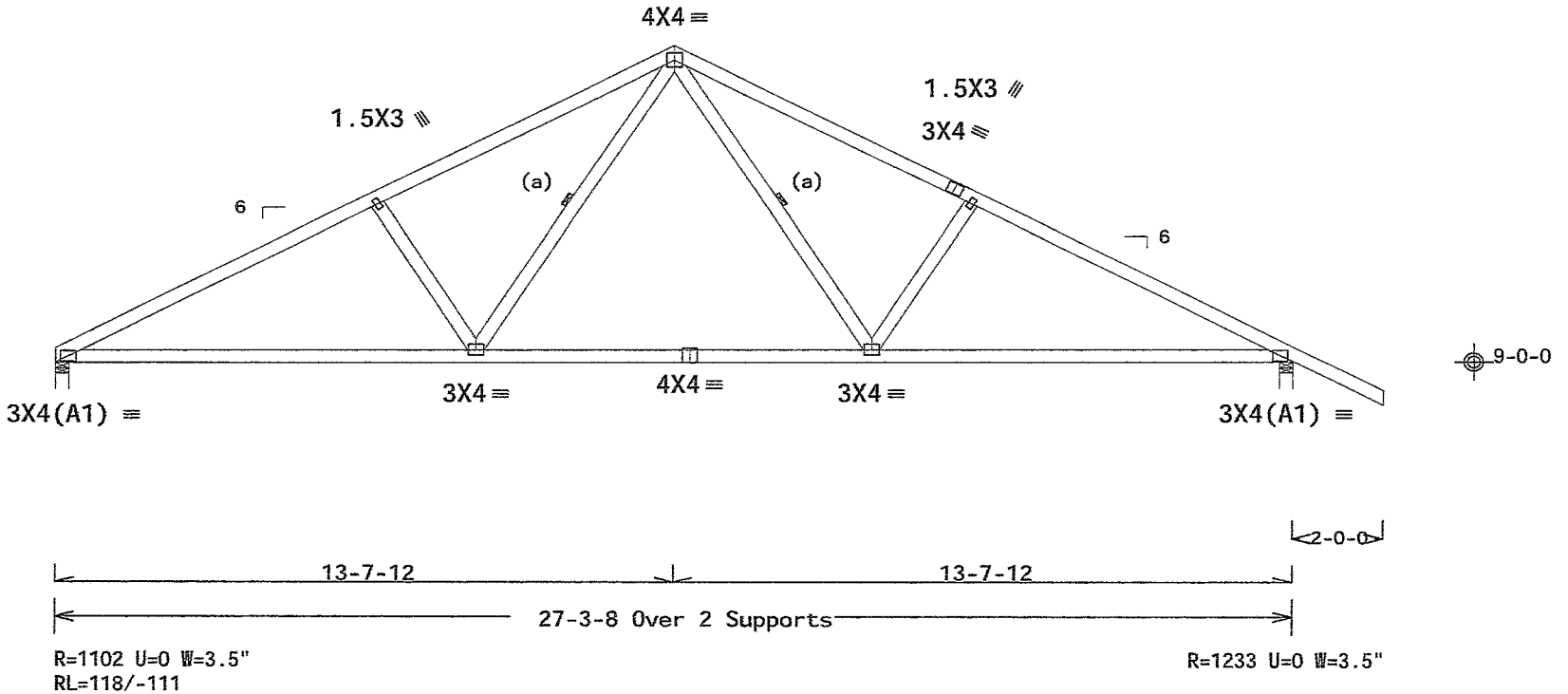
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 QTY:8 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/10/2014

TC LL	20.0 PSF	REF	R9114- 53126
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100009
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	331009
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V5F487_Z02

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - A2 27'3"8 Common Girder)

Top chord 2x4 SP M-30
Bot chord 2x6 SP M-26
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 56 plf at 0.00 to 56 plf at 13.65
TC- From 56 plf at 13.65 to 56 plf at 19.91
TC- From 56 plf at 19.91 to 56 plf at 29.29
BC- From 10 plf at 0.00 to 10 plf at 14.00
BC- From 10 plf at 14.00 to 10 plf at 27.29
BC- From 4 plf at 27.29 to 4 plf at 29.29
BC- 1127.72 lb Conc. Load at 1.23, 3.23, 5.23, 7.23 9.23,11.23,13.23,15.23,17.23,19.23,21.23,23.23,25.23

#1 hip supports 13-7-12 jacks with no webs.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

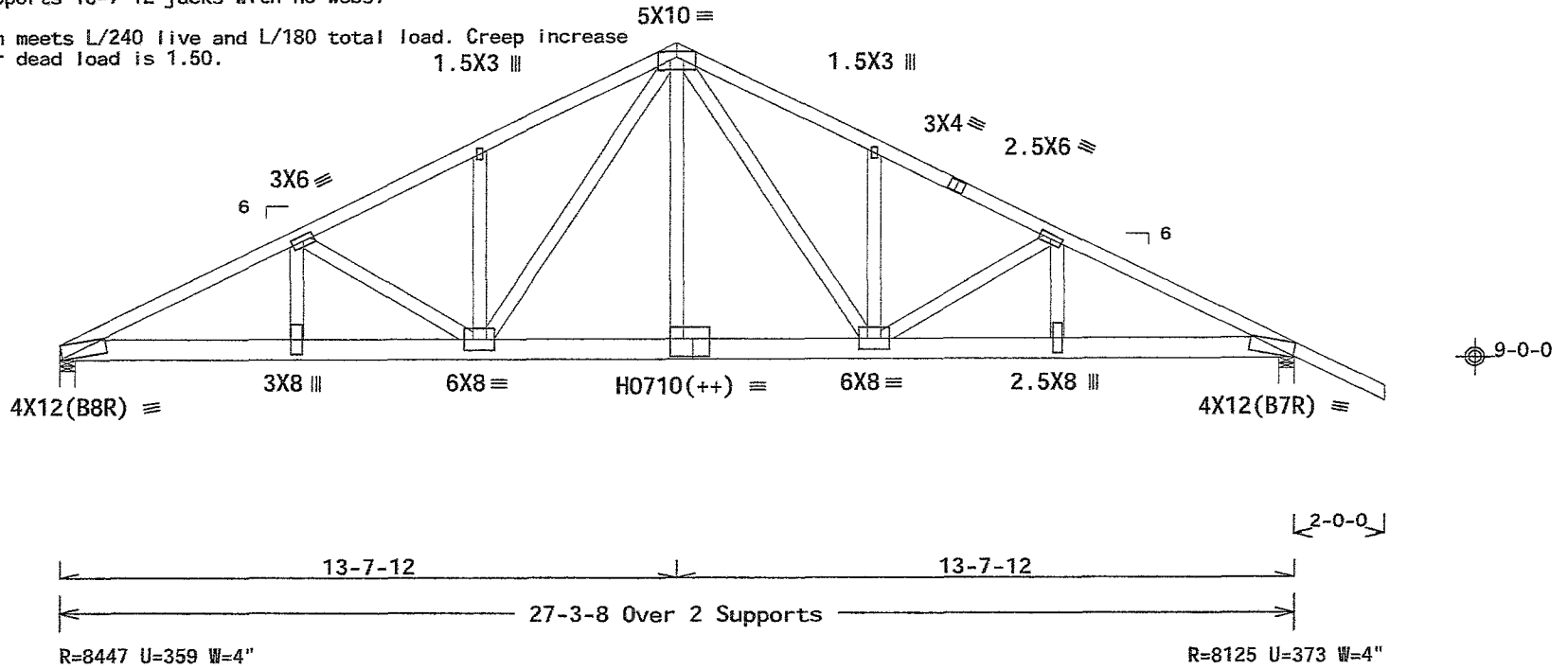
2 COMPLETE TRUSSES REQUIRED

Nail Schedule·0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord. 2 Rows @ 6.00" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
4" o.c. spacing of nails perpendicular and parallel to grain required in area over bearings greater than 4"

(++) - This plate works for both joints covered.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03.04 QTY:1 FL/-/5/-/-/R/- Scale =.25"/Ft.

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ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/10/2014

TC LL	20.0 PSF	REF	R9114- 53127
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100012
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	36969
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V5F487_Z02

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - AGE 27'3"8 Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
:Stack Chord SC1 2x4 SP #1-13B:Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

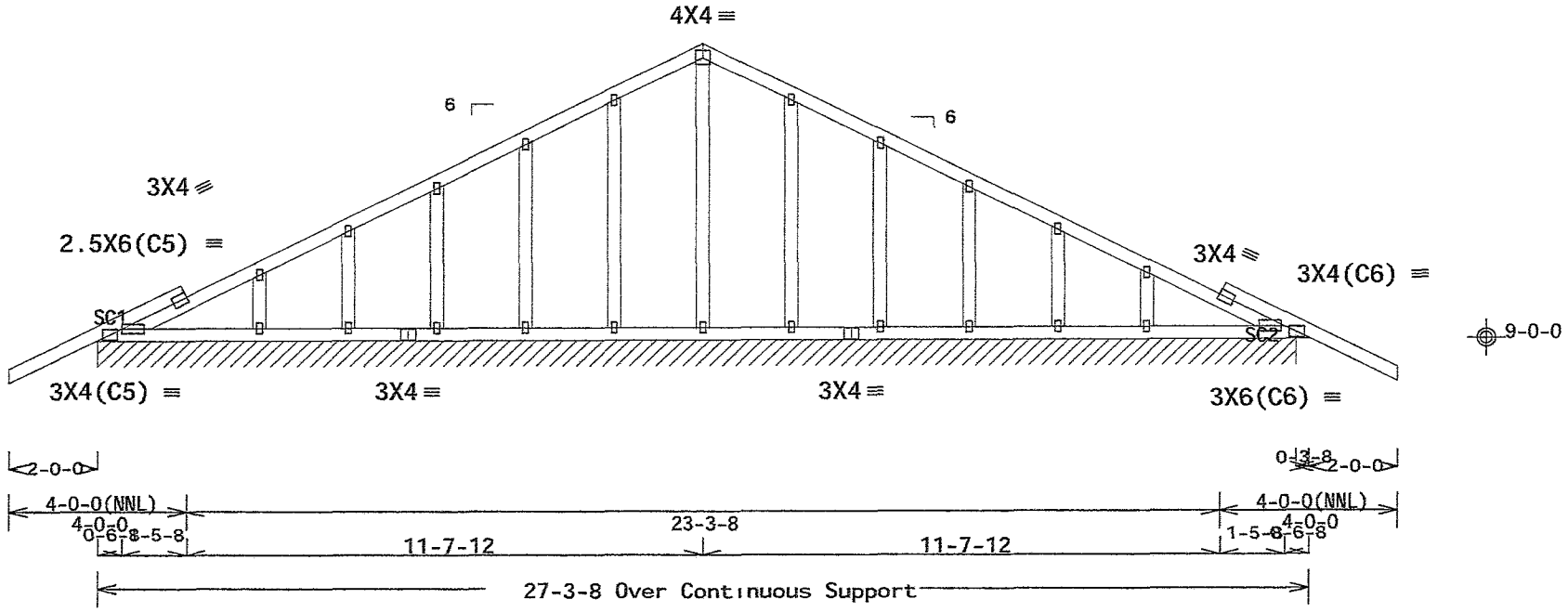
Right cantilever is exposed to wind

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=178 PLF U=19 PLF W=27-0-0
RL=10/-10 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04 0226.14 | QTY:1 | FL/-/5/-/-/R/- | Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 53128
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100008
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	330978
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V5F487_Z02

04/10/2014

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - B1 30' Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP 2850f-2.3E
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member.

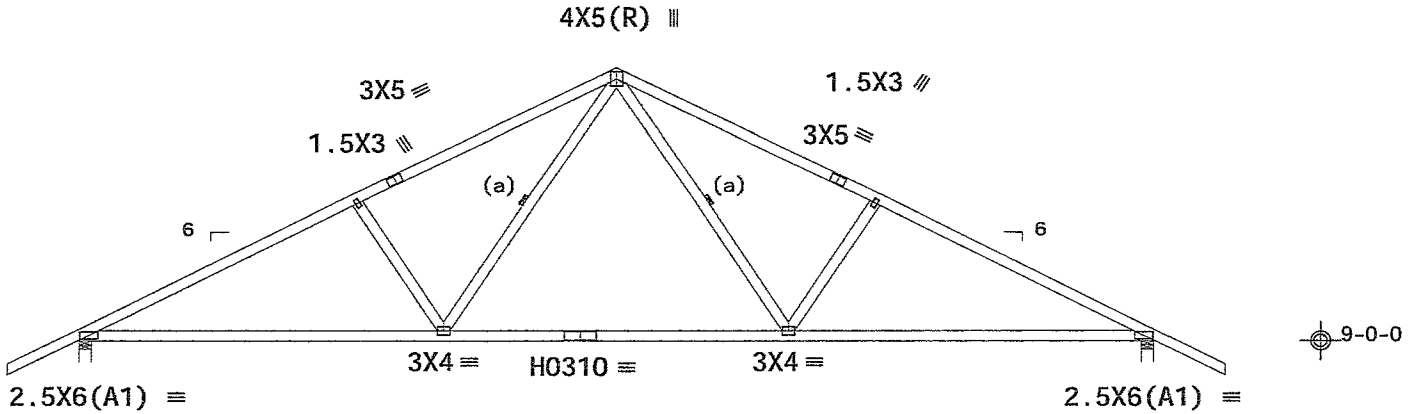
Bottom chord checked for 10.00 psf non-concurrent live load.

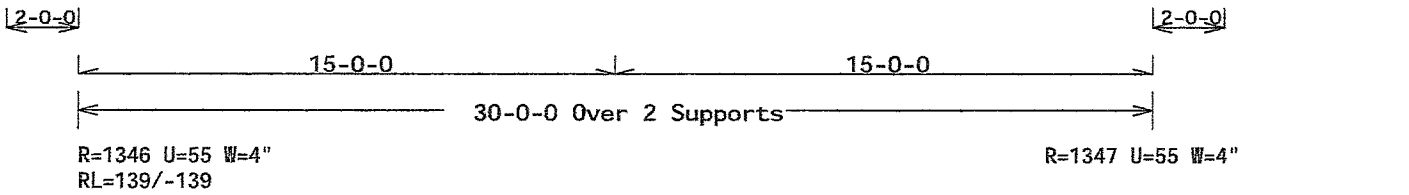
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=1346 U=55 W=4"
RL=139/-139

R=1347 U=55 W=4"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03.04.0226.14 QTY:7 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0278

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04/10/2014

TC LL	20.0 PSF	REF	R9114- 53129
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100006
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	330985
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V5F487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - B2 30' Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP M-30
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

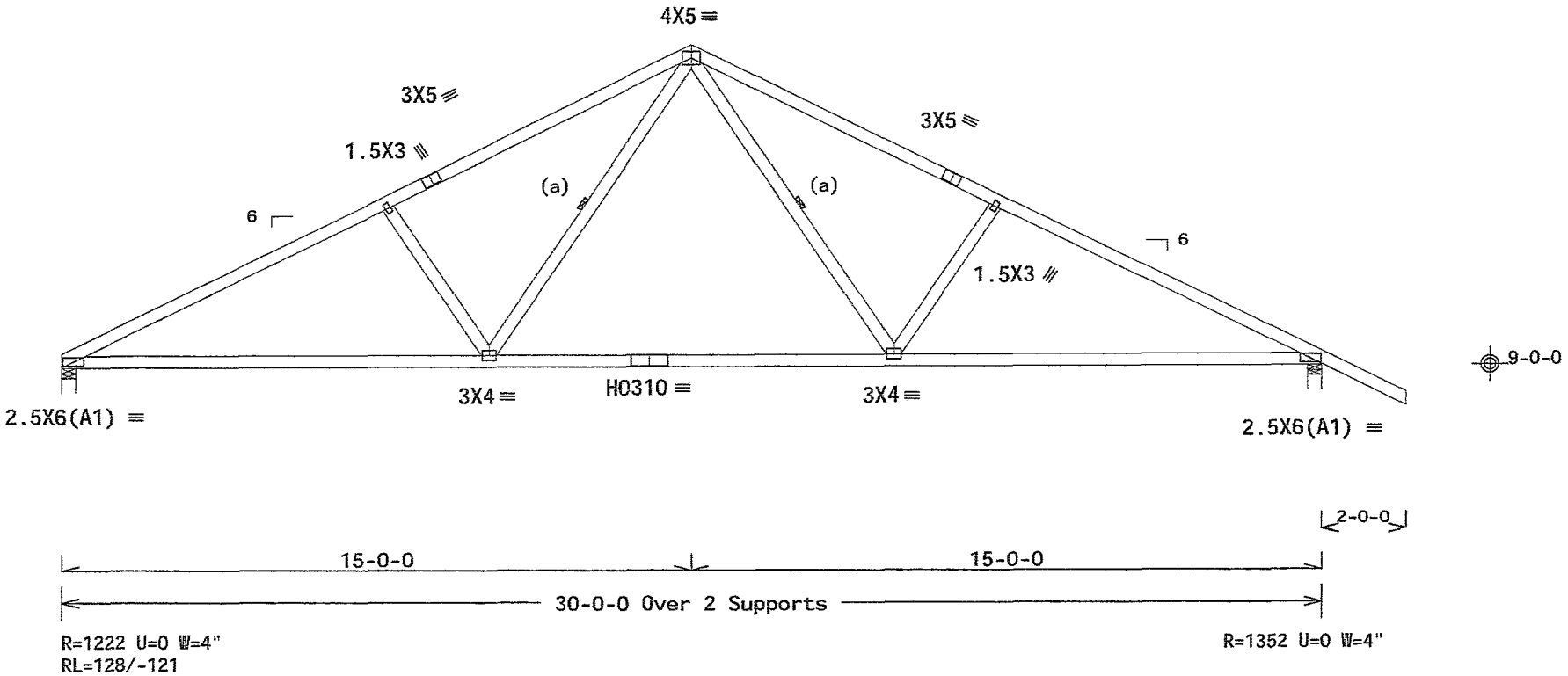
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave | FT/RT=10%(0%)/0(0) | 12.03.04 0326.14 | QTY:2 | FL/-/5/-/-/R/- | Scale =.25"/Ft.

ALPINE
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FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/10/2014

TC LL	20.0 PSF	REF	R9114- 53130
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100003
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	365369
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V5F487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - B3 37' Common)

Top chord 2x4 SP M-30
Bot chord 2x4 SP #1-13B :B3 2x4 SP 2850f-2.3E·
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

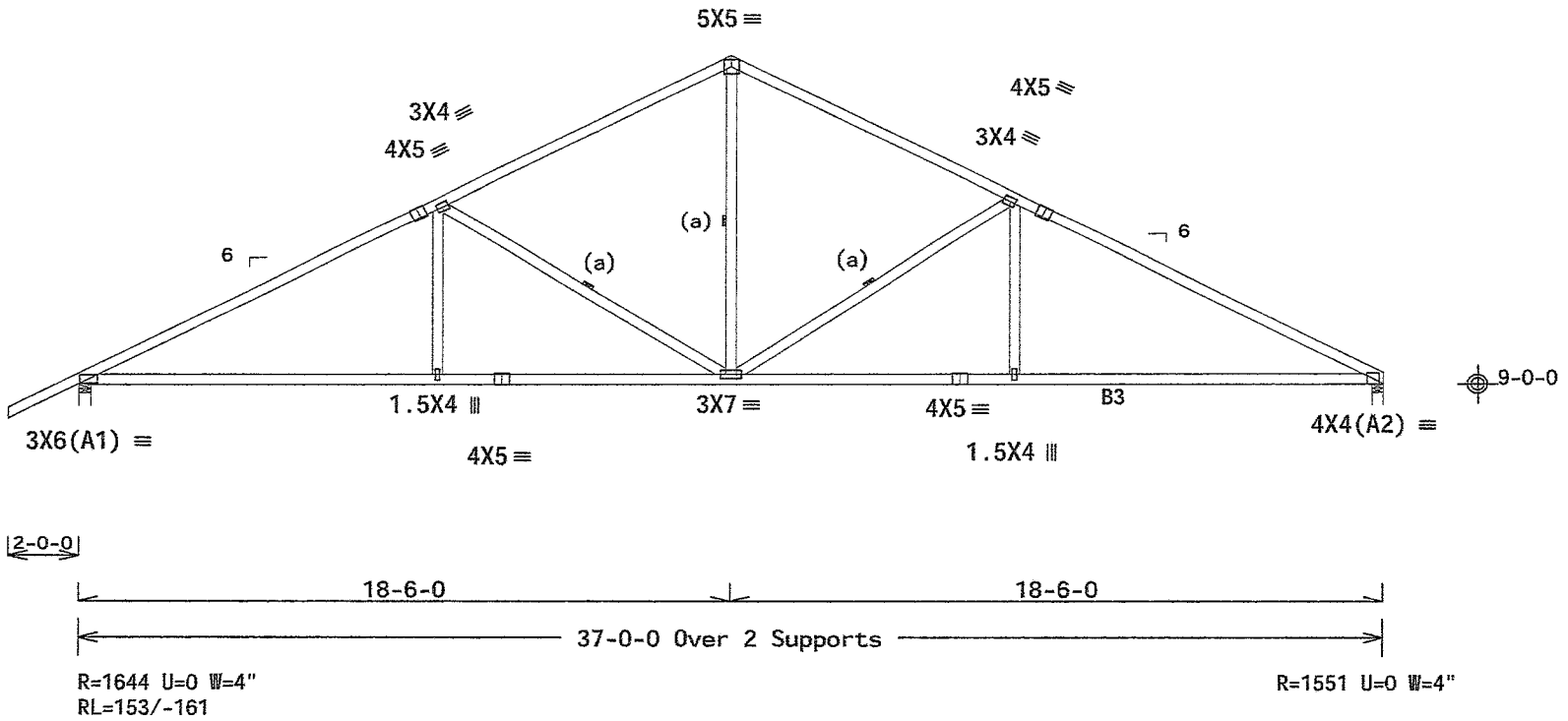
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5 0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 0226 14 QTY:15 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care n fabricating handling shipping installing and brac ng Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this de any failure to build the trusses in conformance with ANSI/TPI 1 or for handling shipping installatio bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to draw ngs 160A-Z for standard plate positions A seal on th drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com, TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/10/2014

TC LL	20.0 PSF	REF	R9114- 53131
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100004
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	365264
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V5F487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - B4 37' Common)

Top chord 2x4 SP #1-13B :T4 2x4 SP M-30:
Bot chord 2x4 SP 2850f-2.3E .B2 2x4 SP #1-13B.
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP M-26 supporting member.
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

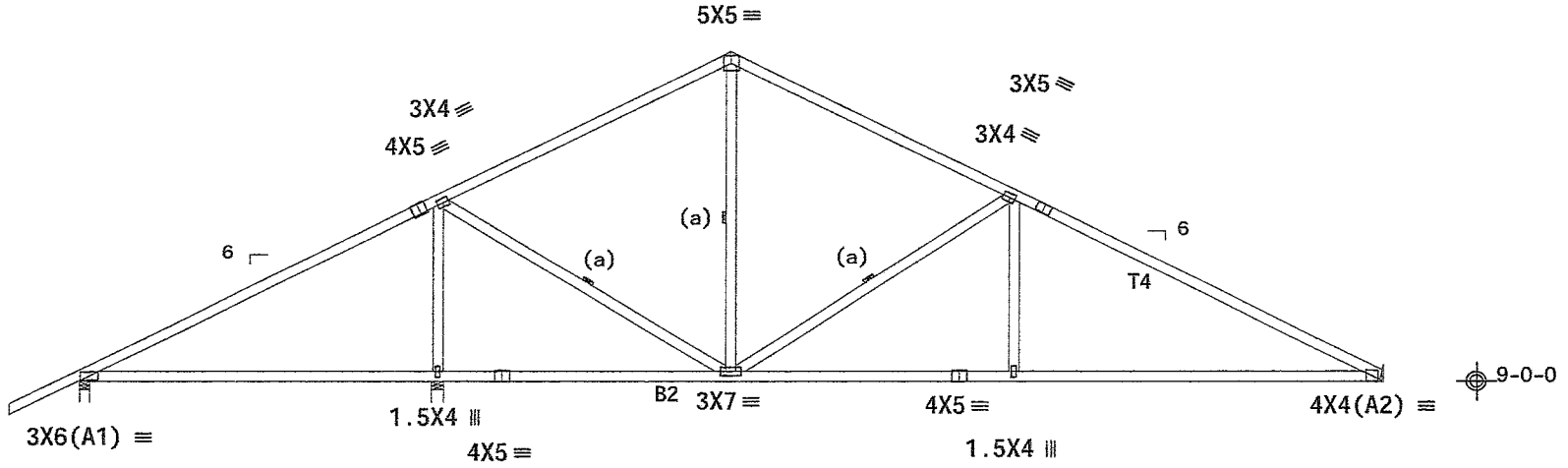
Wind loads and reactions based on MWFRS with additional C&C member design.

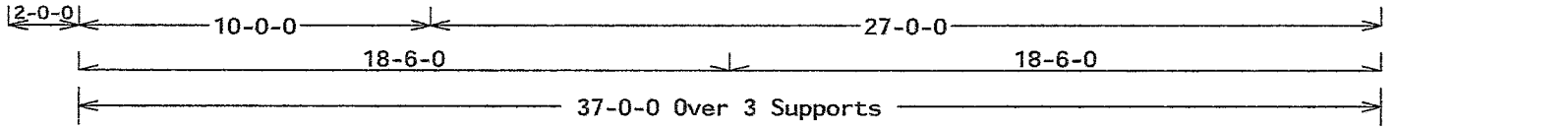
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

(a) Continuous lateral restraint equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=561 U=41 W=3.5"
RL=153/-161

R=1565 U=33 W=4"

R=1127 U=37
H=H1

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 QTY:13 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses require extreme care in fabricating handling shipping installing and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this des any failure to build the trusses in conformance with ANSI/TPI 1 or for handling shipping installation & bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/10/2014

TC LL	20.0 PSF	REF	R9114- 53132
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100005
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	365267
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V5F487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - BGE 30' Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
.Stack Chord SC1 2x4 SP #1-13B.:Stack Chord SC2 2x4 SP #1-13B:

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

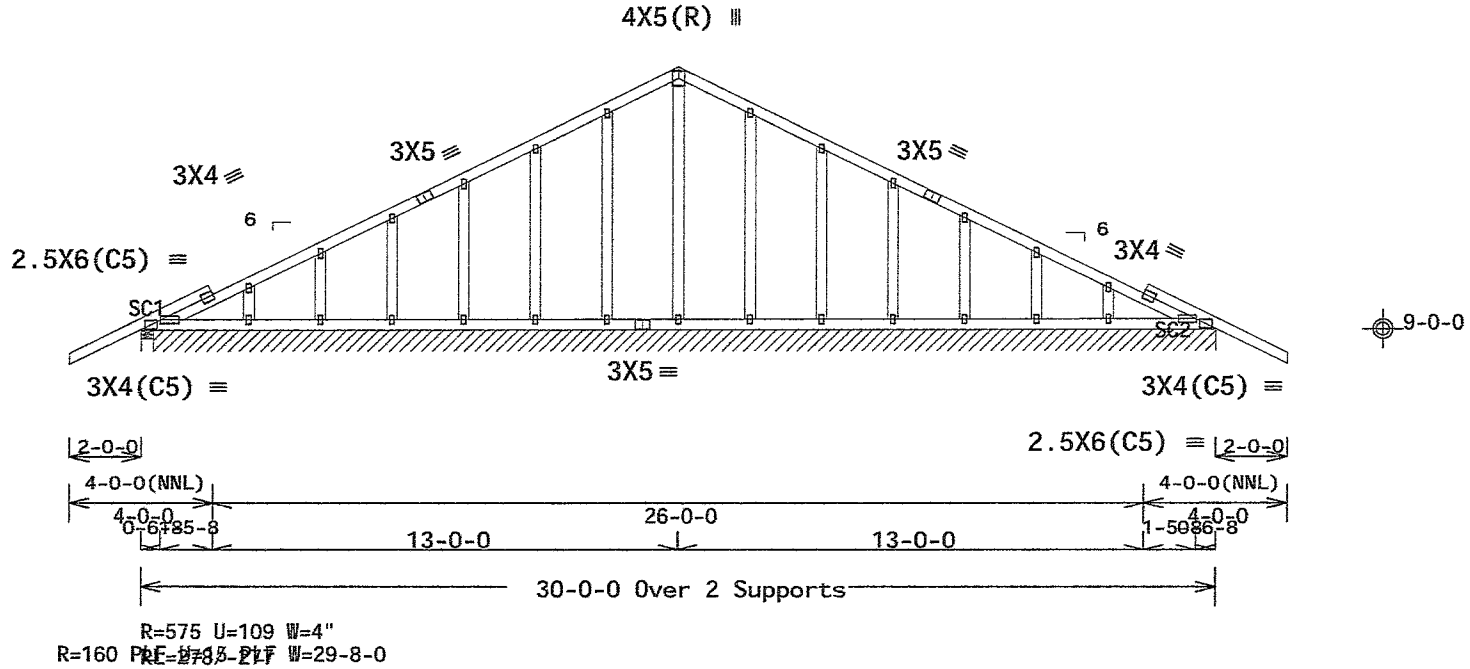
Wind loads and reactions based on MWFRS with additional C&C member design.

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04 | QTY:1 | FL/-/5/-/-/R/- | Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/10/2014

TC LL	20.0 PSF	REF	R9114- 53133
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100007
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	330970
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V5F487_Z02

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - BGE1 37' Common)

Top chord 2x4 SP 2850f-2.3E :T2 2x4 SP M-30.
Bot chord 2x4 SP 2850f-2.3E
Webs 2x4 SP #3-13B :W6 2x4 SP #2-13B:
'Stack Chord SC1 2x4 SP M-30':Lt Slider 2x4 SP #3-13B: BLOCK LENGTH = 1.344' .Rt Wedge 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Truss spaced at 24.0" OC designed to support 2-3-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

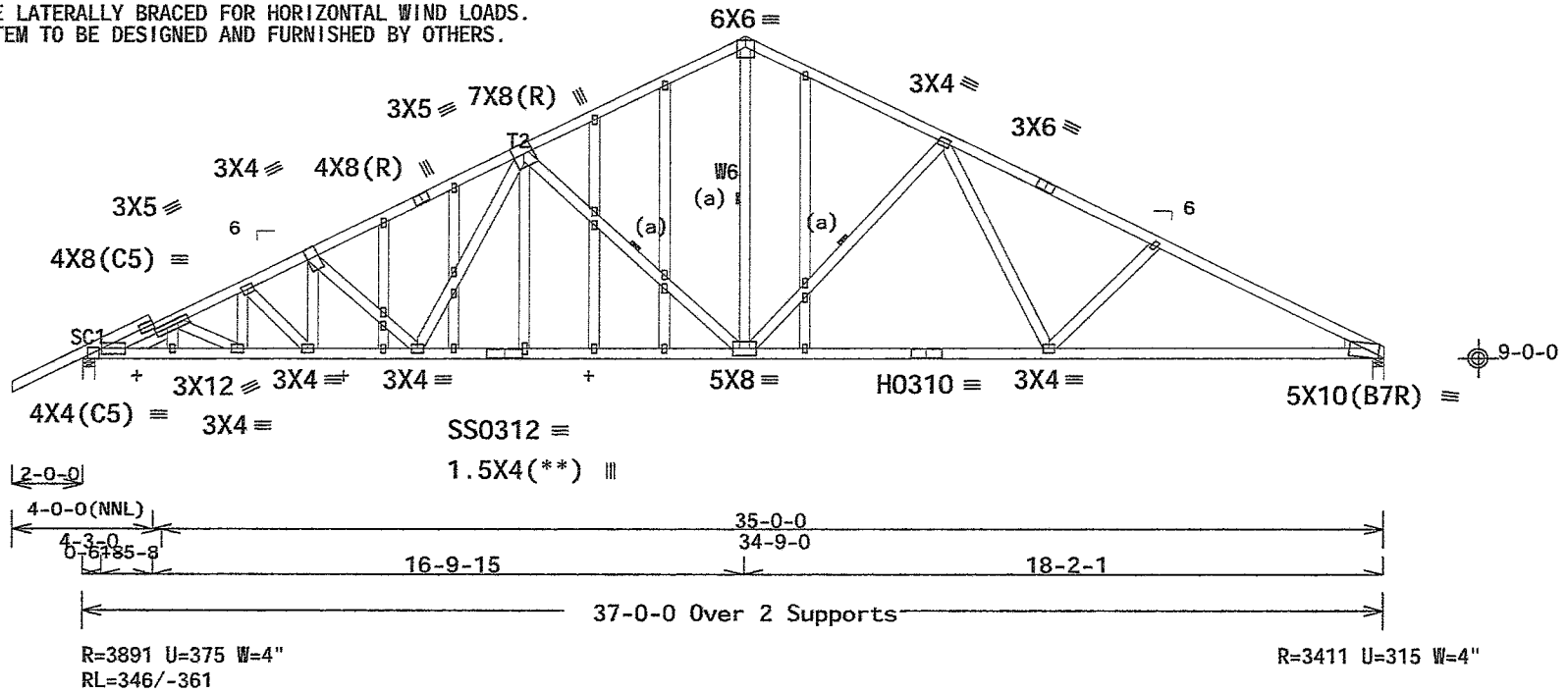
Wind loads and reactions based on MWFRS with additional C&C member design.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X3 Except As Shown.
PLT TYP. 20 Gauge HS,18 Gauge HS, Design Crit: FBC2010Res/TPI-2007(STD)
Wave FT/RT=10%(0%)/0(0) 12.03.04 QTY:1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/10/2014

TC LL	20.0 PSF	REF	R9114- 53134
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100010
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	349476
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V5F487_Z02

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - BGE2 37' Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
·Stack Chord SC1 2x4 SP #1-13B:·Stack Chord SC2 2x4 SP #1-13B:

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

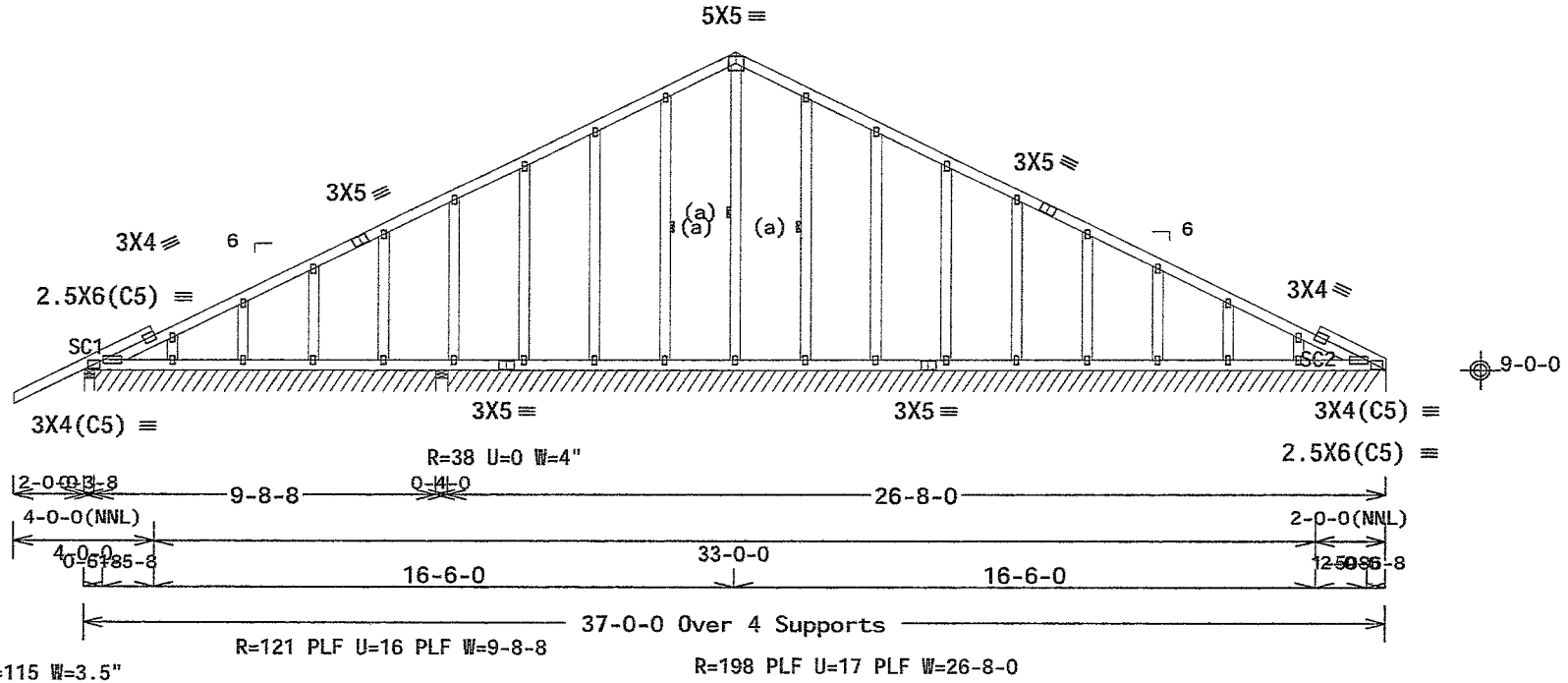
Truss spaced at 24.0" OC designed to support 2-3-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

(a) Continuous lateral restraint equally spaced on member.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=640 U=115 W=3.5"
RL=344/-362

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0336.14 | QTY:1 | FL/-/5/-/-/R/- | Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/10/2014

TC LL	20.0 PSF	REF	R9114- 53135
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100006
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	365261
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V5F487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - C 14'4" Mono)

Top chord 2x4 SP 2850f-2.3E :T2 2x4 SP #1-13B:
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

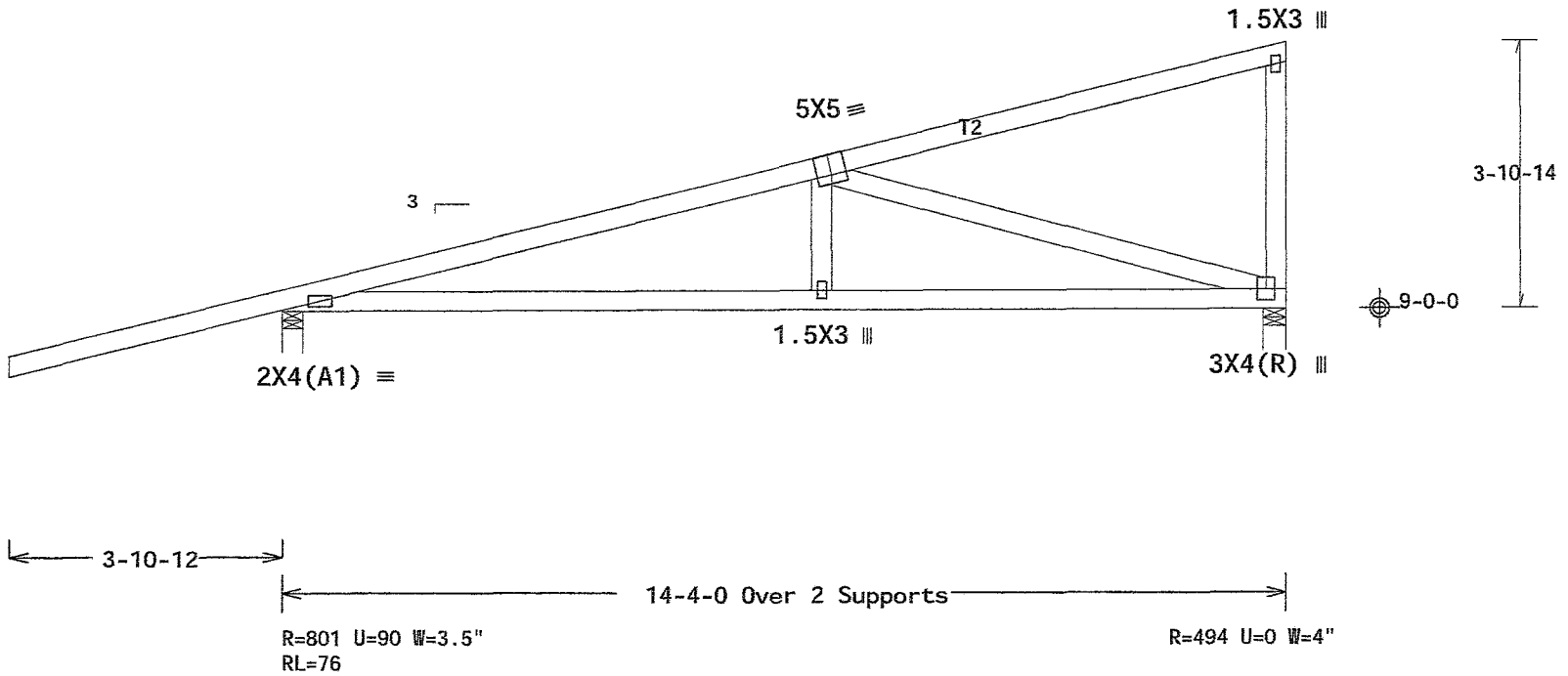
Top chord overhangs have been checked only for loads as indicated. Overhangs not checked for man loads or long-term deflection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.13 QTY:8 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/10/2014

TC LL	20.0 PSF	REF	R9114- 53136
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100004
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	349480
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V5F487_Z02

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - CJ2 1'3"4 Jack)

Top chord 2x4 SP 2850f-2.3E
Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10.00 psf non-concurrent live load.

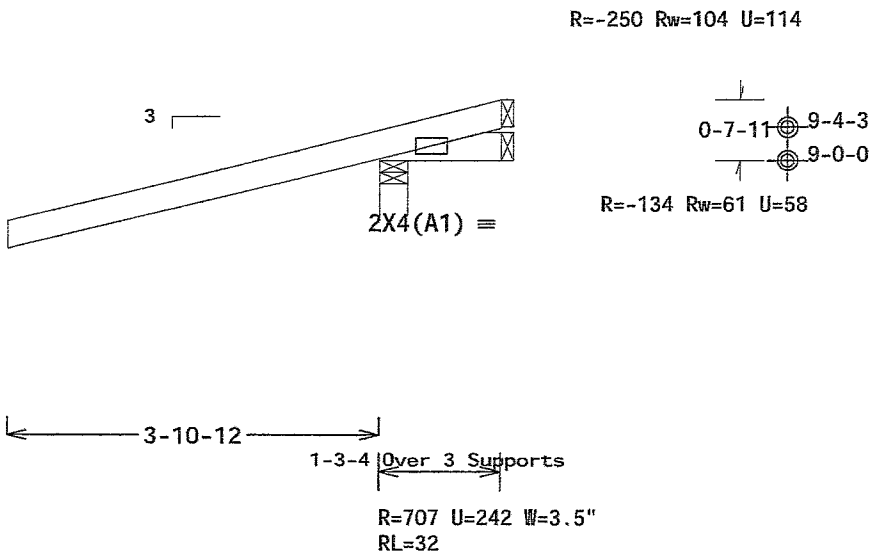
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Top chord overhangs have been checked only for loads as indicated. Overhangs not checked for man loads or long-term deflection.

Negative reaction(s) of -249# MAX. (See below) from a non-wind load case requires uplift connection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04 0326 14 | QTY:2 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/10/2014

TC LL	20.0 PSF	REF	R9114- 53137
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100007
BC LL	0.0 PSF		HC-ENG SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	365311
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V5F487_Z02

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - CJ3 3' Jack)

Top chord 2x4 SP 2850f-2.3E
Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

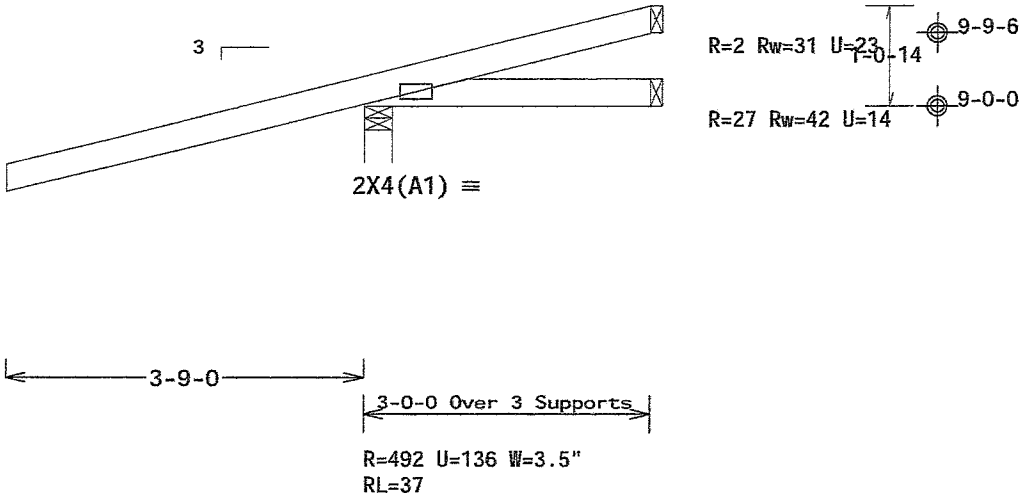
Bottom chord checked for 10.00 psf non-concurrent live load.

Top chord overhangs have been checked only for loads as indicated. Overhangs not checked for man loads or long-term deflection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04 0336 14 | QTY:2 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating handling shipping installing and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance w th ANSI/TPI 1 or for handling shipping installation & bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job s general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R9114- 53138
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100008
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	365348
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V5F487_Z02

04/10/2014

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - CJ5 5' Jack)

Top chord 2x4 SP 2850f-2.3E
Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10.00 psf non-concurrent live load.

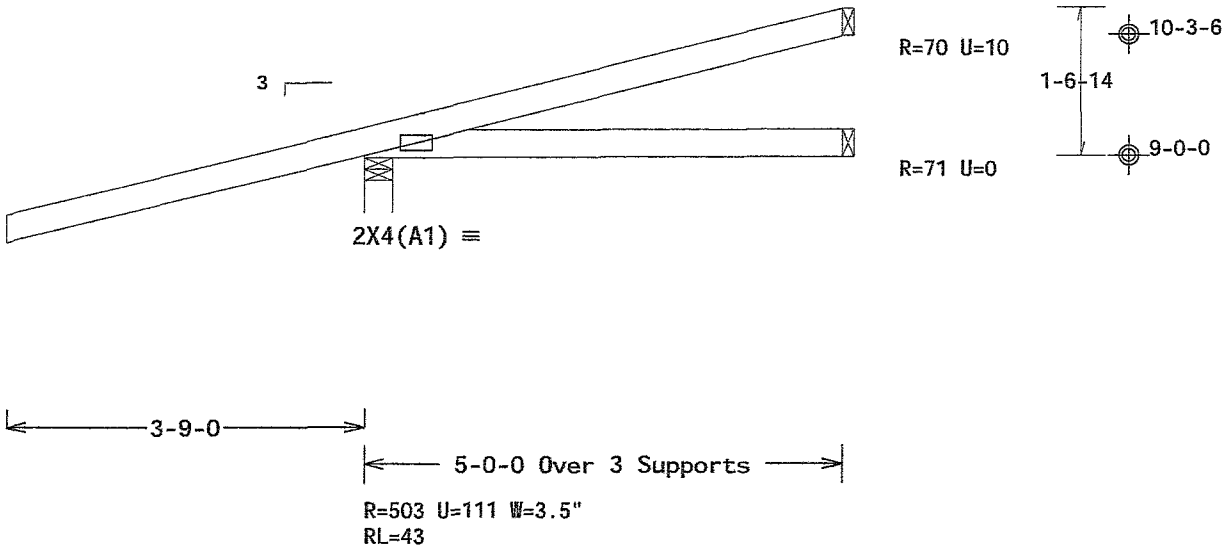
Top chord overhangs have been checked only for loads as indicated. Overhangs not checked for man loads or long-term deflection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04 [illegible]44 | QTY.2 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22639 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/10/2014

TC LL	20.0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	37.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R9114- 53139
DATE	04/10/14
DRW	HCUSR9114 14100009
HC-ENG	SSB/WPF
SEQN-	365344
FROM	JMW
JREF-	1V5F487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - EJ7 7' End Jack)

Top chord 2x4 SP 2850f-2.3E
Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10.00 psf non-concurrent live load.

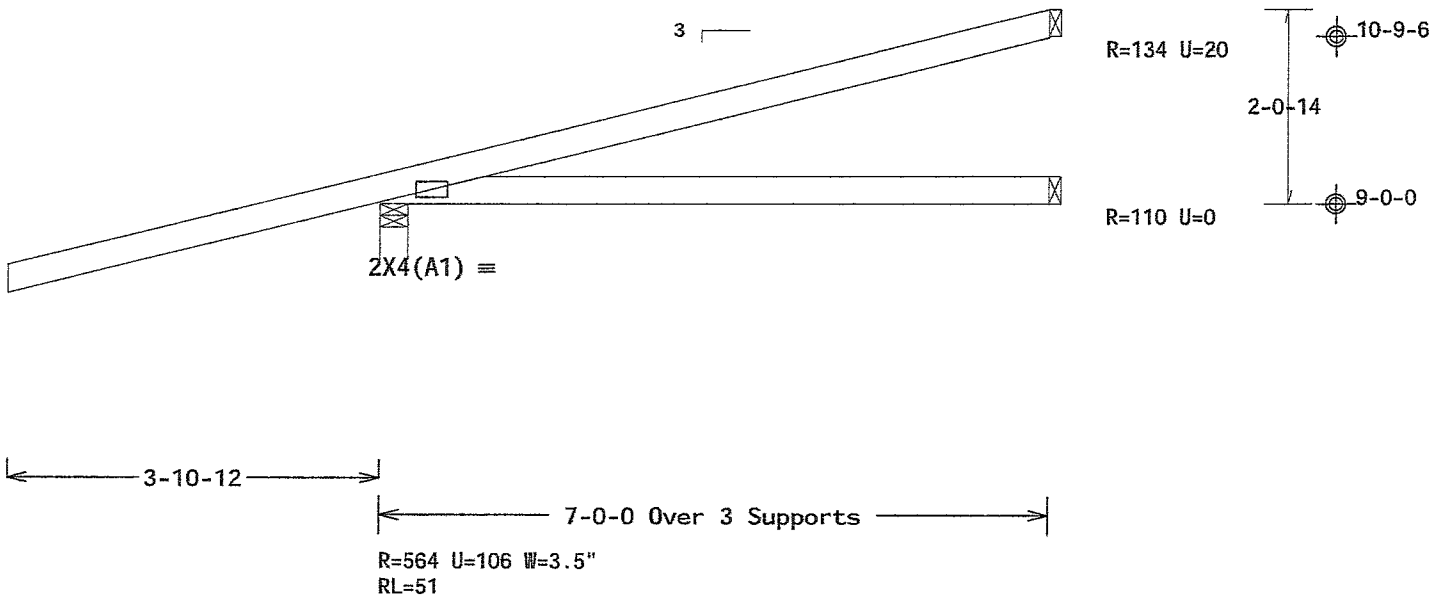
Top chord overhangs have been checked only for loads as indicated. Overhangs not checked for man loads or long-term deflection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 QTY:4 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/10/2014

TC LL	20.0 PSF	REF	R9114- 53140
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100010
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	365303
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V5F487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - H7 14'4" Mono Hip Girder)

Top chord 2x4 SP M-30
Bot chord 2x4 SP 2850f-2.3E
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 54 plf at -3.90 to 54 plf at 7.00
TC- From 27 plf at 7.00 to 27 plf at 14.33
BC- From 4 plf at -3.90 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 0.29
BC- From 170 plf at 0.29 to 170 plf at 7.03
BC- From 160 plf at 7.03 to 160 plf at 14.33
TC- 193.48 lb Conc. Load at 7.03
TC- 134.13 lb Conc. Load at 9.06,11.06,13.06
BC- 329.15 lb Conc. Load at 7.03
BC- 109.71 lb Conc. Load at 9.06,11.06,13.06

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

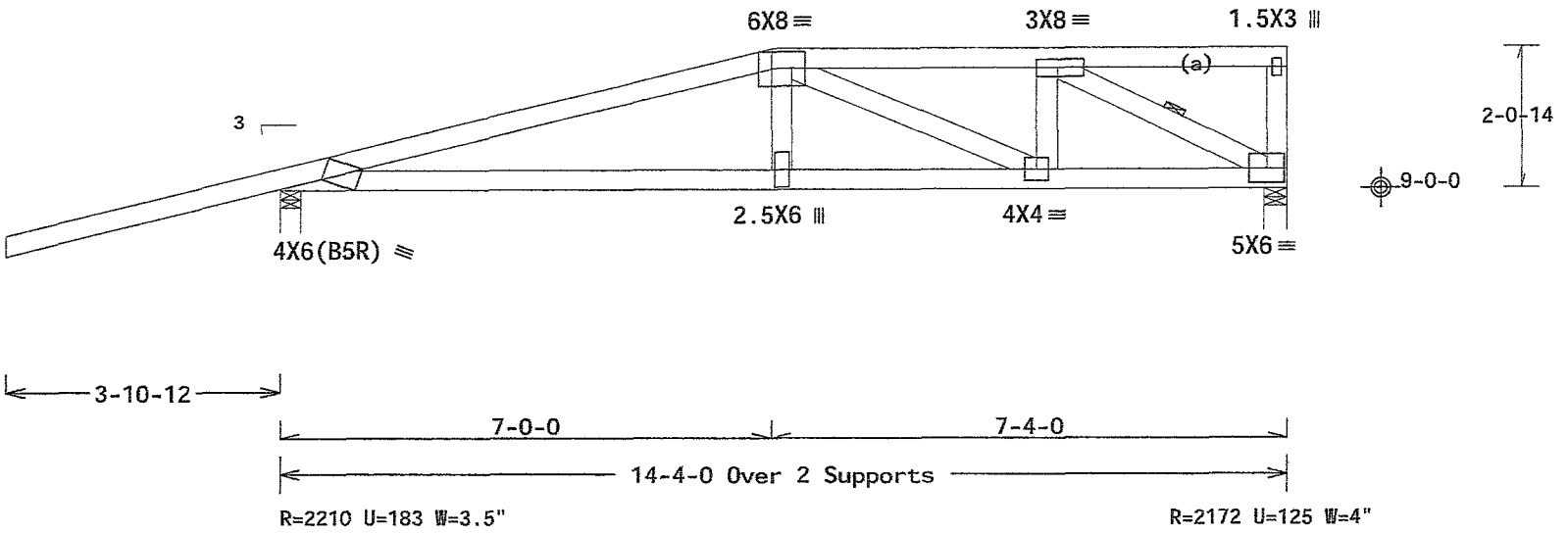
Right end vertical not exposed to wind pressure.

(a) Continuous lateral restraint equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible]44 QTY:1 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/10/2014

TC LL	20.0 PSF	REF	R9114- 53141
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100013
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	365362
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V5F487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - H9 14'4" Mono Hip)

Top chord 2x4 SP 2850f-2.3E :T2 2x4 SP #1-13B:
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC- From	54 plf at	-3.90 to	54 plf at	9.00
TC- From	54 plf at	9.00 to	54 plf at	14.33
BC- From	4 plf at	-3.90 to	4 plf at	0.00
BC- From	20 plf at	0.00 to	20 plf at	0.29
BC- From	50 plf at	0.29 to	50 plf at	14.00
BC- From	20 plf at	14.00 to	20 plf at	14.33

Wind loads and reactions based on MWFRS.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

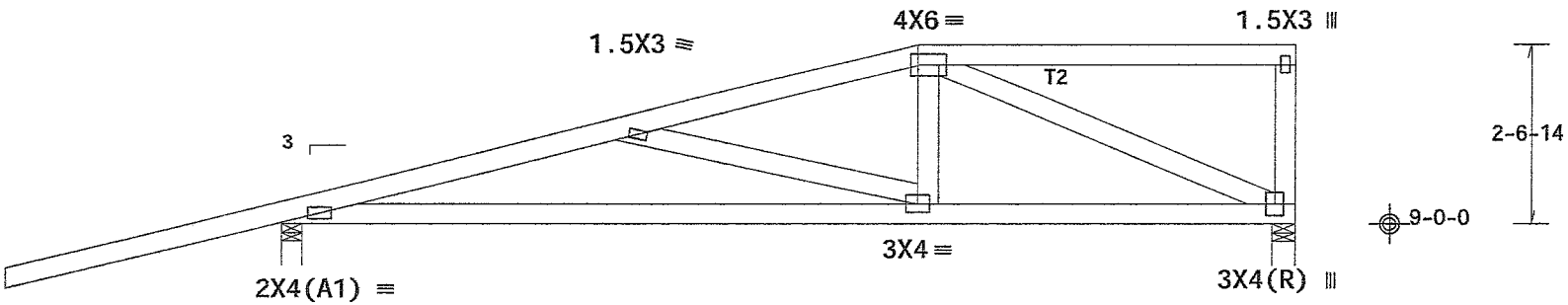
Right end vertical not exposed to wind pressure.

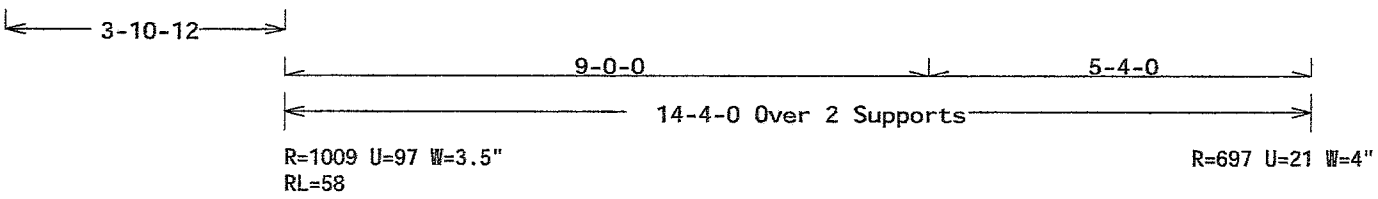
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 [illegible]13 QTY:1 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/10/2014

TC LL	20.0 PSF	REF	R9114- 53142
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100001
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	349532
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V5F487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - HJ7 9'10"13 Hip Jack Girder)

Top chord 2x4 SP 2850f-2.3E
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

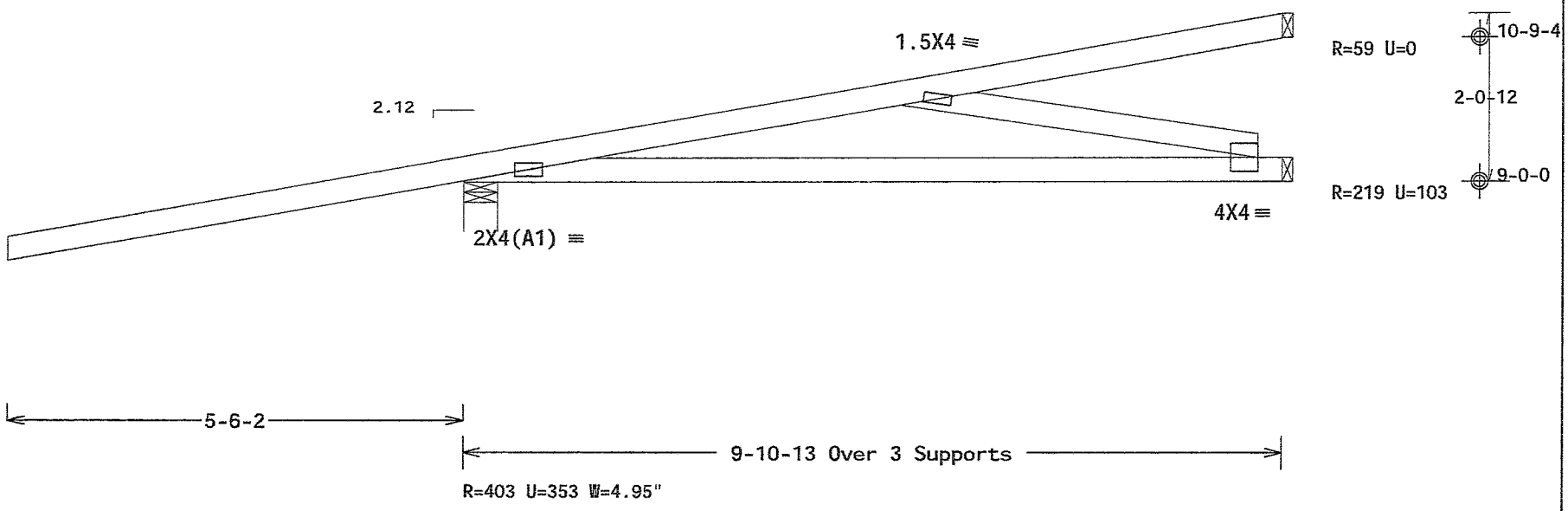
Bottom chord checked for 10.00 psf non-concurrent live load.

Top chord overhangs have been checked only for loads as indicated. Overhangs not checked for man loads or long-term deflection.

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FABRICATOR TO REVIEW THIS DWG PRIOR TO CUTTING LUMBER TO VERIFY THAT ALL DATA, INCLUDING DIMENSIONS AND LOADS, CONFORM TO THE ARCHITECTURAL PLANS/SPECIFICATIONS AND FABRICATOR'S TRUSS LAYOUT.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 0 plf at -5.51 to 54 plf at 0.00
TC- From 2 plf at 0.00 to 2 plf at 9 90
BC- From 0 plf at -5.51 to 4 plf at 0.00
BC- From 2 plf at 0.00 to 2 plf at 9.90
TC- -145.84 lb Conc. Load at 1.90
TC- 3.44 lb Conc. Load at 4.31
TC- 139.31 lb Conc. Load at 7.13
BC- -63.28 lb Conc. Load at 1.90
BC- 53.00 lb Conc. Load at 4.31
BC- 142.00 lb Conc. Load at 7.13

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04 [illegible] | QTY:1 | FL/-/5/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/10/2014

TC LL	20.0 PSF	REF	R9114- 53143
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100011
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN-	365353
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V5F487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - H11 14'4" Mono Hip)

Top chord 2x4 SP 2850f-2.3E .T2 2x4 SP #1-13B.
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC- From	54 plf at	-3.90 to	54 plf at	11.00
TC- From	54 plf at	11.00 to	54 plf at	14.33
BC- From	4 plf at	-3.90 to	4 plf at	0.00
BC- From	20 plf at	0.00 to	20 plf at	0.29
BC- From	50 plf at	0.29 to	50 plf at	14.00
BC- From	20 plf at	14.00 to	20 plf at	14.33

Wind loads and reactions based on MWFRS with additional C&C member design.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

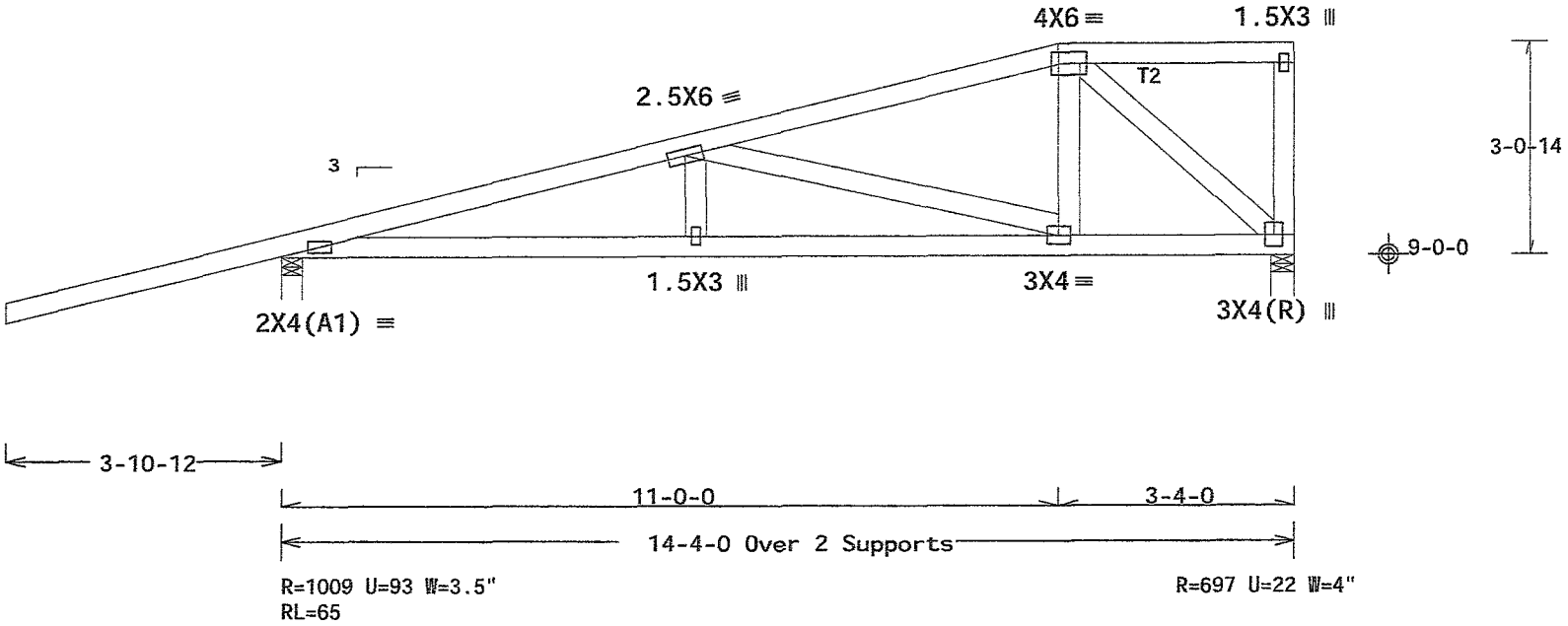
Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 12.03.04 [illegible] 13 | QTY:1 | FL/-/5/-/-/R/- | Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
04/10/2014

TC LL	20.0 PSF	REF	R9114- 53144
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100002
BC LL	0.0 PSF		HC-ENG WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	349538
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V5F487_Z02

(14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - H13 14'4" Mono Hip)

Top chord 2x4 SP #1-13B T1 2x4 SP 2850f-2.3E
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Top chord overhangs have been checked only for loads as indicated. Overhangs not checked for man loads or long-term deflection.

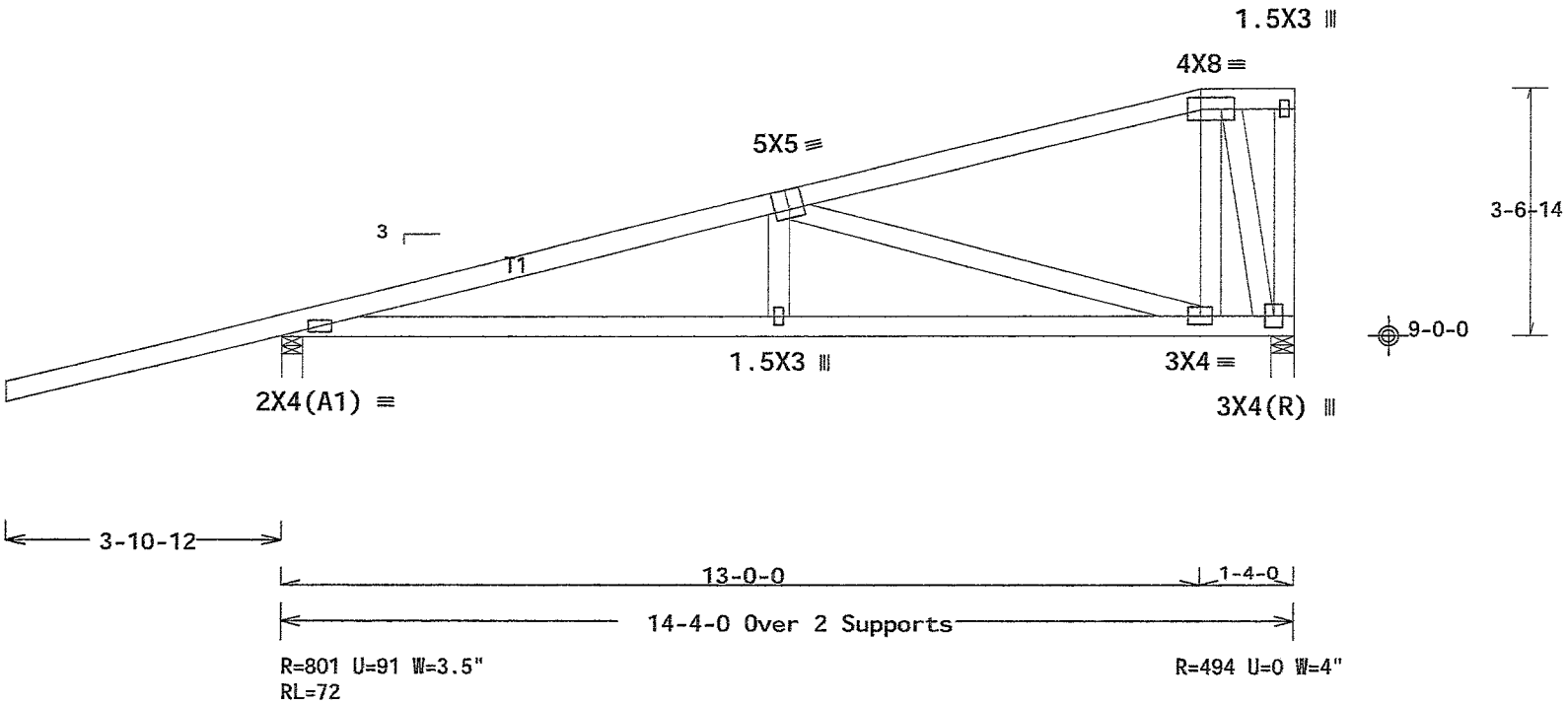
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 12.03.04 0000 13 QTY:1 FL/-/5/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

04/10/2014

TC LL	20.0 PSF	REF	R9114- 53145
TC DL	7.0 PSF	DATE	04/10/14
BC DL	10.0 PSF	DRW	HCUSR9114 14100003
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	349490
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V5F487_Z02

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement

Alternative reinforcement specified in chart below may be conservative For minimum alternative reinforcement, re-run design with appropriate reinforcement type

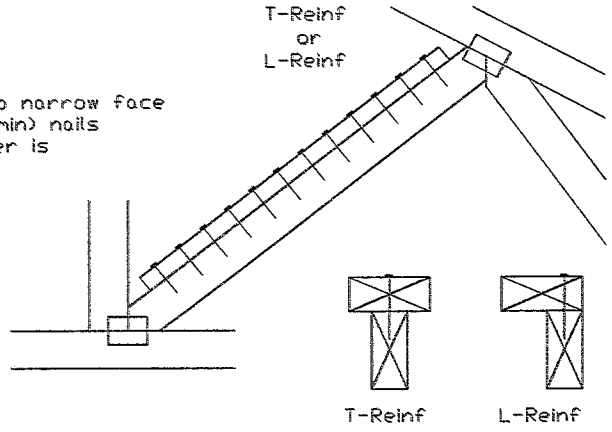
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf	Scab Reinf
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web Apply (1) scab to each face of web

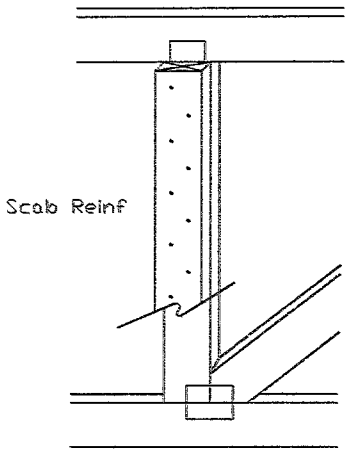
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face Attach with 10d (0.128"x3.0",min) nails at 6" oc Reinforcing member is a minimum 80% of web member length



Scab Reinforcement:

Apply scab(s) to wide face of web No more than (1) scab per face Attach with 10d (0.128"x3.0",min) nails at 6" oc Reinforcing member is a minimum 80% of web member length.



TW
Building Components Group Inc.

Earth City MO 63045

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14

TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 8/15/13
BC DL	PSF	DRWG BRCLBSUB0813
BC LL	PSF	
TOT LD	PSF	
DUR FAC.		
SPACING		

04/10/2014

Gable Stud Reinforcement Detail

ASCE 7-10 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace * Group A	(1) 1x4 "L" Brace * Group B	(1) 2x4 "L" Brace * Group A	(1) 2x4 "L" Brace * Group B	(2) 2x4 "L" Brace ** Group A	(2) 2x4 "L" Brace ** Group B	(1) 2x6 "L" Brace * Group A	(1) 2x6 "L" Brace * Group B	(2) 2x6 "L" Brace ** Group A	(2) 2x6 "L" Brace ** Group B
24" o.c.	SPF HF	#1 / #2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 11"	8' 3"	8' 7"	9' 9"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	6' 0"	6' 4"	8' 0"	8' 6"	10' 10"	11' 7"	12' 6"	13' 5"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	9' 3"	9' 9"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 8"	9' 5"	9' 10"	11' 2"	11' 7"	13' 3"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	7' 4"	7' 10"	9' 9"	10' 5"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	4' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	6' 2"	10' 5"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	8' 6"	9' 0"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades

Group A

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January 2012 ruling

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace space nails at 2" o.c in 18" end zones and 4" o.c between zones.

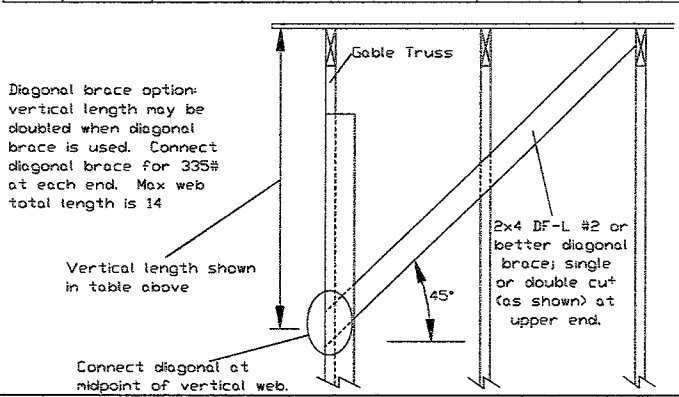
** For (2) "L" braces. space nails at 3" o.c in 18" end zones and 6" o.c between zones.

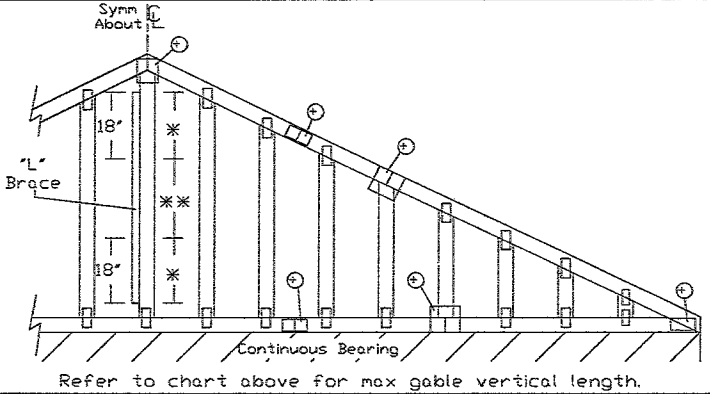
"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0" but less than 11' 6"	2X4
Greater than 11' 6"	2.5X4

+ Refer to common truss design for peak splice and heel plates.

Refer to the Building Designer for conditions not addressed by this detail





Refer to chart above for max gable vertical length.

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14

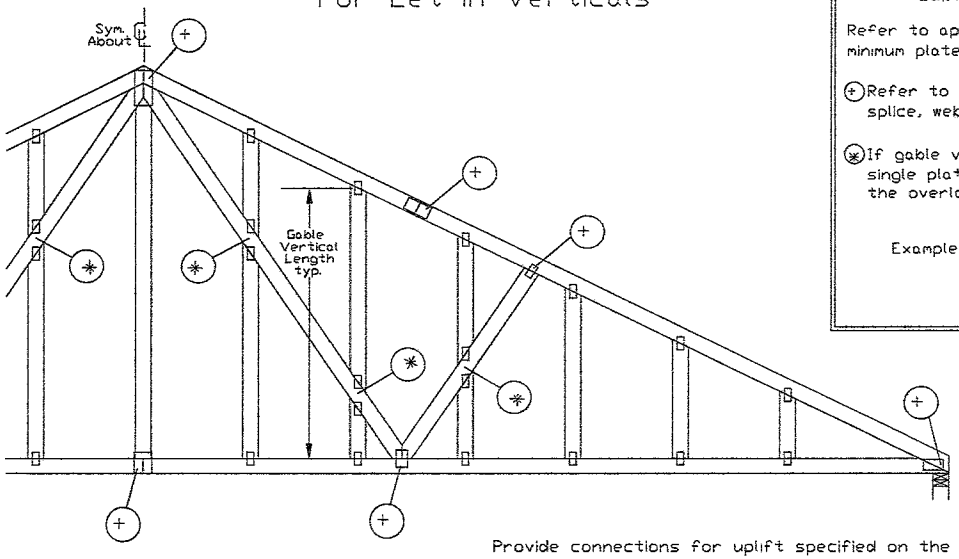
MAX TOT LD 60 PSF

MAX SPACING 24 0"

REF ASCE7-10-GAB12015
DATE 2/14/12
DRWG A12015ENC100212

04/10/2014

Gable Detail
For Let-in Verticals

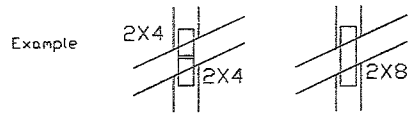


Gable Truss Plate Sizes

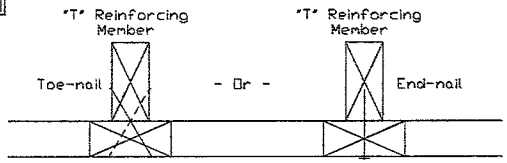
Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs

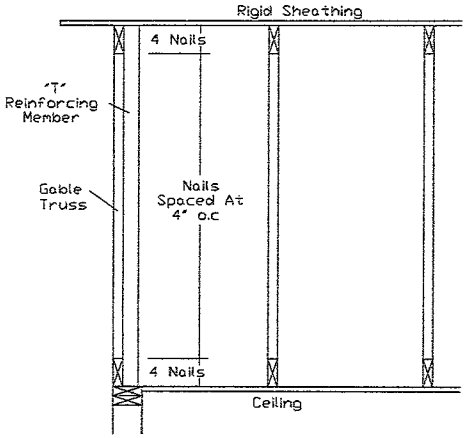
⊕ Refer to Engineered truss design for peak splice, web and heel plates

⊛ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web



"T" Reinforcement Attachment Detail





Provide connections for uplift specified on the engineered truss design

Attach each "T" reinforcing member with

End Driven Nails
- 10d Common (0.148"x 3.", min) Nails at 4" o.c plus
- (4) nails in the top and bottom chords

Toenailed Nails
- 10d Common (0.148"x3" min) Toenails at 4" o.c plus
- (4) toenails in the top and bottom chords

This detail to be used with the appropriate ITW gable detail for ASCE wind load

ASCE 7-98 Gable Detail Drawings
A130 5980109, A120i5980109, A11015980109 A100i5980109
A13030980109 A12030980109 A11030980109 A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109, A12015020109 A11015020109, A10015020109 A14015020109,
A13030020109, A12030020109 A11030020109, A10030020109 A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109, A12015050109, A11015050109, A10015050109 A14015050109
A13030050109, A12030050109 A11030050109 A10030050109, A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212 A12015ENC100212, A14015ENC100212, A16015ENC100212
A18015ENC100212, A20015ENC100212, A20015END100212, A20015PED100212,
A11530ENC100212 A12030ENC100212 A14030ENC100212 A16030ENC100212,
A18030ENC100212 A20030ENC100212 A20030END100212, A20030PED100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length

To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate ITW gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member

Web Length Increase w/ "T" Brace

"T" Reinf Mbr Size	"T" Increase
2x4	30 %
2x6	20 %

Example
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24'o.c SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

Building Components Group Inc.

Earth City MO 63045

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14

MAX TOT LD	60 PSF
DUR FAC	ANY
MAX SPACING	24.0"

REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLLETIN0212

04/10/2014

ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft Mean Hgt, ASCE 7-10, Part Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf

Lateral chord bracing requirements
Top Continuous roof sheathing
Bot Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail

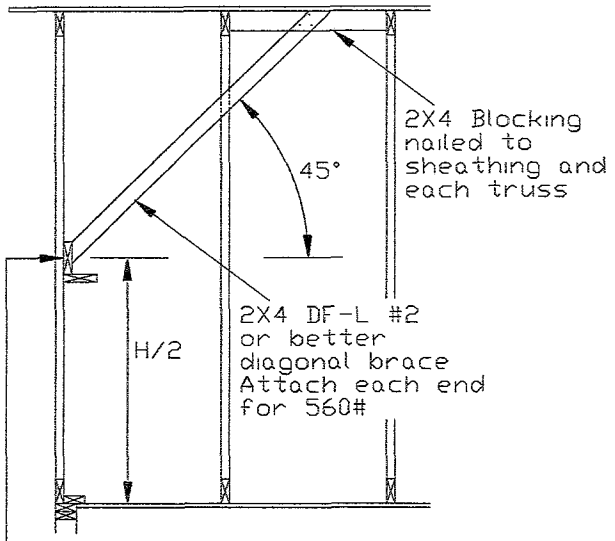
Nails 10d box or gun (0.128"x3",min) nails

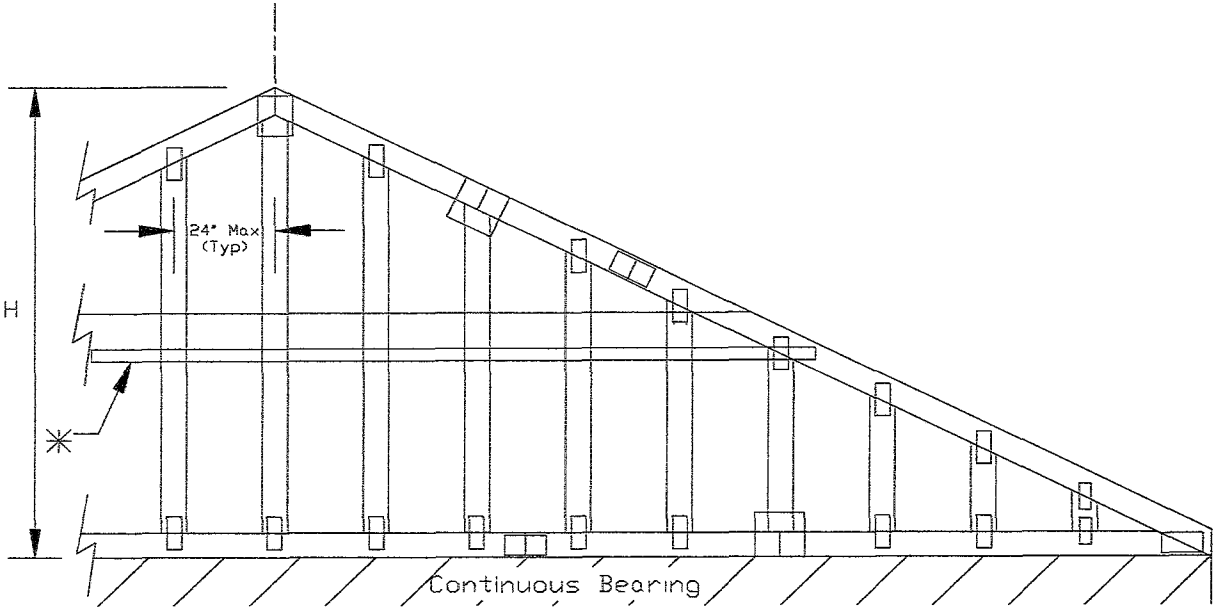
H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC100212)

H Greater than 7'6" to 12'0" max
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC100212)

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min) nails @ 6" oc





Building Components Group Inc.

Earth City MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10 as applicable Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2
For more information see this job's general notes page and these web sites:
ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.org; ICC: www.iccsafe.org

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MAX TOT LD 60 PSF

MAX SPACING

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DATE	2/14/12
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04/10/2014