Permit # 31820

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837 Florida Engineering Certificate of Authorization Number 0 278 Florida Certificate of Product Approval # FL1999 Document ID 1V5F487-Z0210140604 Page 1 of 1

Truss Fabricator. Anderson Truss Company

Job Identification. 14-022A--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL

Truss Count. 21

Model Code Florida Building Code 2010 Truss Criteria FBC2010Res/TPI-2007(STD)

Engineering Software Alpine Software, Version 12.03.

Structural Engineer of Record.

The identity of the structural EOR did not exist as of the seal date per section 61G15-31.003(5a) of the FAC

Address Minimum Design Loads

Roof - 37.0 PSF @ 1,25 Duration

Floor = N/A

Wind - 120 MPH ASCE 7-10 -Closed

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-

#	Ref	Description	Drawi ng#	Date
1	53125	A 27'3"8 Common	14100005	04/10/14
2	53126-A	1 27'3"8 Common	14100009	04/10/14
3	53127-A	2 27'3"8 Common	14100012	04/10/14
4	53128-A	GE 27'3"8 Gable	14100008	04/10/14
5	53129	B1 30' Common	14100006	04/10/14
6	53130	B2 30' Common	14100003	04/10/14
7	53131	B3 37' Common	14100004	04/10/14
8	53132	B4 37' Common	14100005	04/10/14
9	53133	BGE 30' Gable	14100007	04/10/14
10	53134	BGE1 37' Common	14100010	04/10/14
11	53135	BGE2 37' Gable	14100006	04/10/14
12	53136	C 14'4" Mono	14100004	04/10/14
13	53137	CJ2 1'3"4 Jack	14100007	04/10/14
14	53138	CJ3 3' Jack	14100008	04/10/14
15	53139	CJ5 5' Jack	14100009	04/10/14
16	53140	EJ7 7' End Jack	14100010	04/10/14
17	53141-H	17 14'4" Mono Hi	14100013	04/10/14
18	53142-H	19 14'4" Mono Hi	14100001	04/10/14
19	53143-ł	IJ7 9'10"13 Hip	14100011	04/10/14
20	53144-H	111 14'4" Mono H	14100002	04/10/14
21	53145-H	113 14'4" Mono H	14100003	04/10/14



04/10/2014

Walter P. Finn -Truss Design Engineer-

1950 Marley Drive Haines City, FL 33844

Top chord 2x4 SP #1-13B Bot chord 2x4 SP #1-13B Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved $1/30/2013\ \ by\ ALSC$

(a) Continuous lateral restraint equally spaced on member

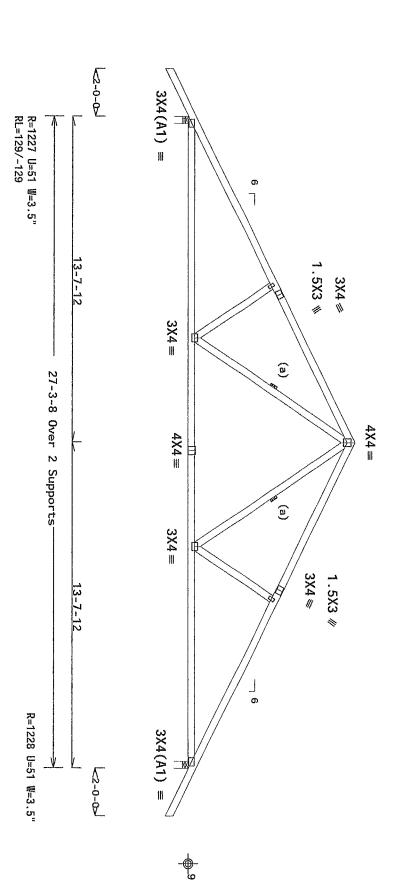
Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member

Truss passed check for 20 psf additional bottom chord live load in areas with $42"\mbox{-high}\ x\ 24"\mbox{-wide clearance}.$ design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



ALP I NE

ITW Building Components Group

Orlando FL, 32837 FL COA #0 278

** IMPORTANT** **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!

Design Crit: FBC2010Res/TPI-2007(STD)

FT/RT=10%(0%)/0(0)

PLT TYP.

Mave.

Trusses require oxtress care in fabricating fandling shipping installing and bracing fafer to a foliaw the leasts edition or BCSI (Building Component Safety information by FDI and MTDI) for sefect practices prior to performing these functions installers shall provide temporary bracing per BCSI (British once otherwise to probed shall have properly attached structural sheathing and bettom chord shall have a properly attached rigid colling Locations shown for permanent lateral restraints of weak shall have a properly attached rigid colling Locations shown for permanent lateral restraints of weak shall have a properly attached rigid colling Locations shown for permanent lateral restraints of weak shall have a properly attached rigid colling Locations shown for permanent lateral restraints. shall have a properly attached rigid ceiling Locations shown for permanent shall have bracing installed per BCSI sections 83 B7 or B10 as applicable

I'N Building Components Group Inc (INBEG) shall not be responsible for any deviation from this dost any failure to build the truss in conformance with AMSI/TPI 1 or for handling shipping installation bracing of trusses. Apply places to each face of truss and position as shown above and on the Joint Details unless noted otherwise. Refer to drawings 1809-2 for standard place positions. A seal on the drawing or cover page listing this drawing includes acceptance of professional engineering responsibility of the design shown. The suitability and use of this design for any structure the responsibility of the Building Designer per MSI/TPI 1 Sec 2. For more information see This job general notes page. ITM-BGS wew ithose con. TPI were tpinet or WICA wew shrindustry con. structure is This job s

12.03.0 CENS 04/10/2014 QTY:13 FL/-/5/-/-/R/-BC LL BC DL SPACING TC DL DUR. FAC. TC LL TOT.LD.

20.0 PSF

Scale = .25"/Ft. R9114- 53125

10.0 PSF 7.0 PSF

DRW HCUSR9114 14100005 HC-ENG WHK/WHK

DATE REF

04/10/14

0.0 PSF

SEQN-

330983

24.0" 1.25 37.0 PSF

JREF-

1V5F487_Z02

Top chord 2x4 SP #1-13B Bot chord 2x4 SP #1-13B Webs 2x4 SP #3-13B

Lumber grades designated 1/30/2013 by ALSC with "13B" use design values approved

(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load

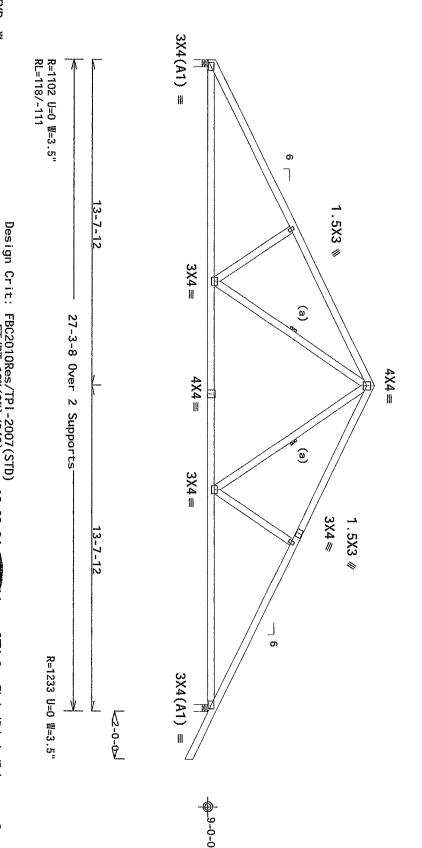
MMFRS loads based on trusses located at least 15.00 ft. from roof

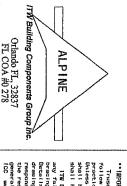
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

des i gn. Wind loads and reactions based on MWFRS with additional C&C member

Truss passed check for 20 psf additional bottom chord live load in areas with $42^{\prime\prime}-\text{high}~x~24^{\prime\prime}-\text{wide}$ clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!

IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

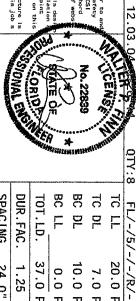
FT/RT=10%(0%)/0(0)

PLT TYP.

Wave

Trusses require extreme care in fabricating bandling shipping installing and bracing Refer to spriol to the latest edition of BCSI (Building Component Safety information by IPI and BTCA) for safety practices prior to performing these functions Installers shall provide temporary practing set BCSI liness noted otherwise top chord shall have properly attached structural shalling and bottom chord shall have a properly attached rigid calling Locations shown for permanent lateral restraint of webs shall have a properly attached rigid calling Locations shown for permanent afteral restraint of webs shall have a properly attached rigid calling Locations shown for permanent

bracing of trusses Apply plates to each foco of Betalis unless noted otherwise Rofer to drawing drawing or cover page listing this drawing indica responsibility of the Building bosom. The temporalibility of the Building besigner per All ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this des Ny failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping installation th face of truss and position as shown above and to drawings 160A-Z for standard plate positions This job s



04/10/2014	S. S. S.	NE	9 ;	**************************************	AN INTERNATION
SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL
24.0"	1.25	37.0 PSF	0.0 PSF	10.0 PSF	7.0 PSF
JREF- 1V5F487_Z02		SEQN- 331009	HC-ENG WHK/WHK	DRW HCUSR9114 14100003	DATE 04/10/14

20.0 PSF

REF

R9114- 53126

Scale = .25"/Ft

Lumber grades designated with "13B" use design values approved $1/30/2013\ \ \mbox{by ALSC}$ Top chord 2x4 SP M-30 Bot chord 2x6 SP M-26 Webs 2x4 SP #3-13B

Special loads

> COMPLETE TRUSSES REQUIRED

Nail Schedule 0.131"x3", min. nails Top Chord: 1 Row @12.00" o.c. Bot Chord. 2 Rows @ 6.00" o.c. (Eac Webs : 1 Row @ 4" o.c. (Each Row)

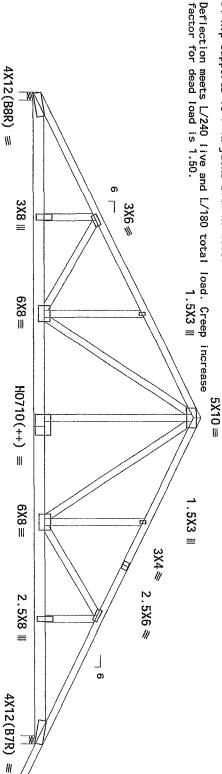
4" o.c. spacing of nails perpendicular and parallel to grain required in area over bearings greater than 4" Use equal spacing between rows and stagger nails n each row to avoid splitting.

(++) - This plate works for both joints covered

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on NWFRS.

#1 hip supports 13-7-12 jacks with no webs.



R=8447 U=359 W=4" 13-7-12 27-3-8 Over 2 Supports 3-7-12 R=8125 U=373 W=4" L2-0-0J

Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0)

PLT TYP.

20 Gauge HS, Wave

Trusses require extreme care in febricating bandling shipping installing and bracing Befer to an follow the latest edition of BSSI (Building Component Safety Information by FIP) and BTDN for safety practices prior to performing these functions installers shall provide temporary bracing per BCSI whites noted otherwise top chord shall have properly attached structural shaeshing and bottom enord shall have a properly attached of circultural shaeshing and bottom enord while have a properly attached of circultural shaeshing and bottom enord whose shall have a properly attached of circultural shaeshing and sortion enorgy of the shall have a properly attached of circultural shaeshing and bottom enorgy of the shall have a properly attached of circultural shaeshing and sortion enorgy of the shall have a properly attached of circultural shaeshing and bottom enorgy of the shall have a properly attached of circultural shaeshing and bottom enorgy of the shall have a properly attached of circultural shaeshing and bottom enorgy of the shall have a properly attached structural shaeshing and bottom enorgy of the shall have a properly attached structural shaeshing and bottom enorgy of the shall have a properly attached structural shaeshing and bottom enorgy of the shall have a properly attached structural shaeshing and bottom enorgy of the shall have a properly attached structural shaeshing and bottom enorgy of the shall have a properly attached of circultural shaeshing and bottom enorgy of the shall have a properly attached of circultural shaeshing and bottom enorgy of the shall have a properly attached of circultural shaeshing and bottom enorgy of the shall have a properly attached of circultural shaeshing and bottom enorgy of circultural shaeshing and bottom enorgy of circultural shaeshing and ci ** IMPORTANT** **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!

shall have bracing installed per BCSI sections 83 87 or 810 as applicable

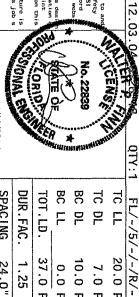
drawing or cover page listing this drawi responsibility solely for the design sho the responsibility of the Building Dasig ITM Building Components Group Inc (ITMBCG) shall not be responsible for any deviation from this des by failure to build the truss in conformanco with ANSI/TPI 1 or for handling shipping installation Apply plates to each face of tru d otherwise Refer to drawings 1

ITW Building Components Group Inc.

ALPINE

Orlando FL, 32837 FL COA #0 278

ANSI/TPI 1 Sec 2 F y structure is e This job s



1/301/	*	SE E	Y)	H11671	MINER
SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL
24,0"	1.25	37.0 PSF	0.0 PSF	10.0 PSF	7.0 PSF
JREF- 1V5F487_Z02		SEQN- 36969	HC-ENG SSB/WPF	DRW HCUSR9114 14100012	DATE 04/10/14

20.0 PSF

REF

Scale = .25"/Ft. R9114- 53127

Top chord 2x4 SP #1-13B Bot chord 2x4 SP #1-13B Webs 2x4 SP #3-13B :Stack Chord SC1 2x4 SP

#1-13B::Stack Chord SC2 2x4 SP #1-13B

lumber grades designated with "13B" use design values approved $1/30/2013~{
m by}$ ALSC

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

perpendicular to chord length. Splice top chord in notchable area using 3x6. to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at $24^{\prime\prime}$ o.c. intervals. Attach stacked top chord (SC)

> 120 mph wind, 15.00 ft mean hgt, ASCE 7-10, anywhere in roof, RISK CAT II, EXP B, wind DL=5.0 psf. GCpi(+/-)=0.18TC DL=3.5 psf, wind BC

Wind loads and reactions based on NWFRS with additional C&C member design.

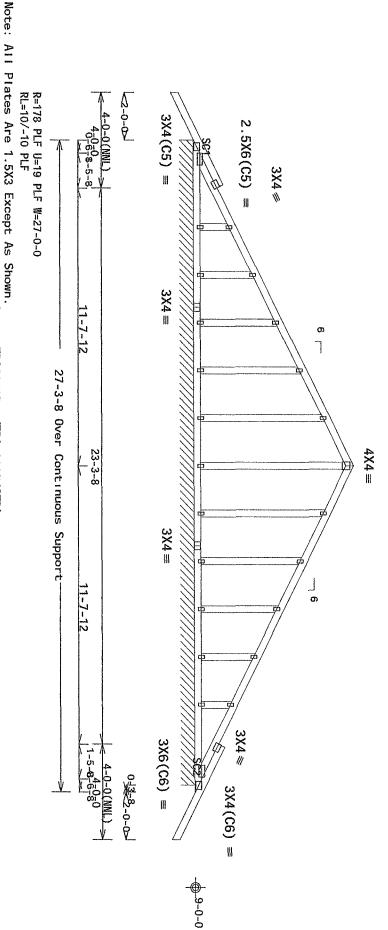
Right cantilever is exposed to wind

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more

In lieu of structural panels use purlins to brace TC @ 24"

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



ITW Building Components Group linc. Orlando FL, 32837 FL COA #0 278 ALPINE

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!

BORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Design Crit: FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)

12.03.04

017:1

FL/-/5/-/-/R/-

REF

Scale = .25"/Ft. R9114- 53128

PLT TYP.

Mave

Trusses require extreme care in fabricating handling, shipping installing and brasing Refer to a follow the latest edition of ECSI (Guilding Component Safety Information by TPI and MTCA) for safety practices prior to performing these functions installers shall provide temporary bracing par ECSI Miless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached a figlic calling Locations shown for paramental lateral restraint of well shall have a properly attached rigid calling Locations shown for paramental state at the stalling Locations shown for paramental state of the shall have a properly attached rigid calling Locations shown for paramental state of the shall have a properly attached rigid calling Locations shown for paramental state of the shall have a properly attached rigid calling Locations shown for paramental state of the shall have a properly attached rigid calling Locations shown for paramental states of the shall have a properly attached and parameters of the shall have a properly attached rigid calling Locations shown for paramental states of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the shall have a properly attached and parameters of the sh

ITW Building Components Group inc. (ITWBCG) shall not be responsible for any deviation from this any failure to be life the trues in conformance with AMS/IPP 1 or for handling shipping installar bracking of truesces. Apply pieces to each face of trues and position as shown above and on the John Details unless noted otherwise. Refer to drawings 160A.Z for standard plate positions. A seal on drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility and use of this design for any structume responsibility and use of this design for any structume responsibility of the design sheet page, AMS/IPP 1.5 and 2 professional engineering the responsibility of the design sheet page, AMS/IPP 1.5 and 2 professional engineering.

CENS, 04/10/2014 BC LL TC DL TC LL SPACING DUR. FAC. TOT.LD. 20.0 PSF

1.25 10.0 37.0 PSF 24.0" 0.0 7.0 PSF PSF PSF SEQN-DATE JREF-HC-ENG WHK/WHK DRW HCUSR9114 14100008 1V5F487_Z02 04/10/14 330978

Top chord 2x4 SP #1-13B Bot chord 2x4 SP 2850f-2.3E Webs 2x4 SP #3-13B

Lumber grades designated with 1/30/2013 by ALSC "13B" use design values approved

(a) Continuous lateral restraint equally spaced on member.

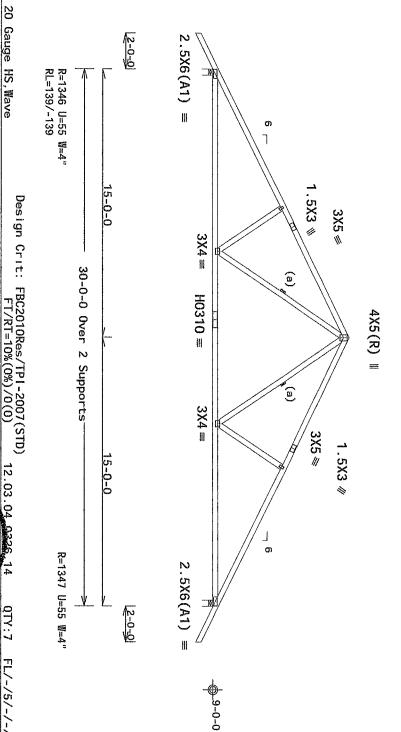
Bottom chord checked for 10.00 psf non-concurrent live load

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

des ign. Wind loads and reactions based on NMMFRS with additional C&C member

Truss passed check for 20 psf additional bottom chord live load in areas with $42"\hbox{-high}\ x\ 24"\hbox{-wide clearance.}$

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



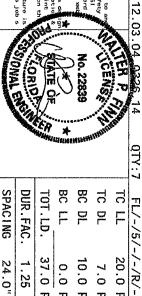
ITW Building Components Group Orlando FL, 32837 FL COA #0 278 ALPINE

** IMPORTANT** **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!

PLT TYP.

Trusses require extreme cere in fabricating bandling shipping installing and brocing Refer to a follow the latest edition of RCSI (Building Component Safety) Information by TPI and MTCA) for safety practices prior to performing these functions installers shall provide temporary bracing per BCSI whiles noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid celling Loadstons shown for permanent lateral restraint of wed shall have a properly attached rigid celling Loadstons shown for permanent lateral restraint of wed shall have a properly attached rigid celling.

I'W Building Components Group Inc. (INWCG) shall not be responsible for any deviation from any failure to build the trusts in conformance with AMS/IPFI 1 or for handling shipping instruction of trustees. Apply places to each face of trusts and position as above above and on the Details unless noted otherwise. Before to develop (60.2) for standard plate positions. A sea drawing income page itsing this drawing indicates acceptance of professional engineering responsibility of the design sham. The suitability and also of this design for any state the responsibility of the Building besigner per AMS/IPFI 1 50-2. For any en information see the responsibility of the Building besigner per AMS/IPFI 1 50-2. For any state the responsibility of the Building besigner per AMS/IPFI 1 50-2. For any state of the second page IIW-BCG www.ichaefe.



04/10/2014

	Ser.	ER	1 ************************************	MINA 	WHI.
SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	IC DL
24.0"	1.25	37.0 PSF	0.0 PSF	10.0 PSF	7.0 PSF
JREF- 1V5F487_Z02		SEQN- 330985	HC-ENG WHK/WHK	DRW HCUSR9114 14100006	DATE 04/10/14

20.0 PSF

REF

Scale = .1875"/Ft. R9114-53129

Top chord 2x4 SP #1-13B Bot chord 2x4 SP W-30 Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10.00 psf non-concurrent live load

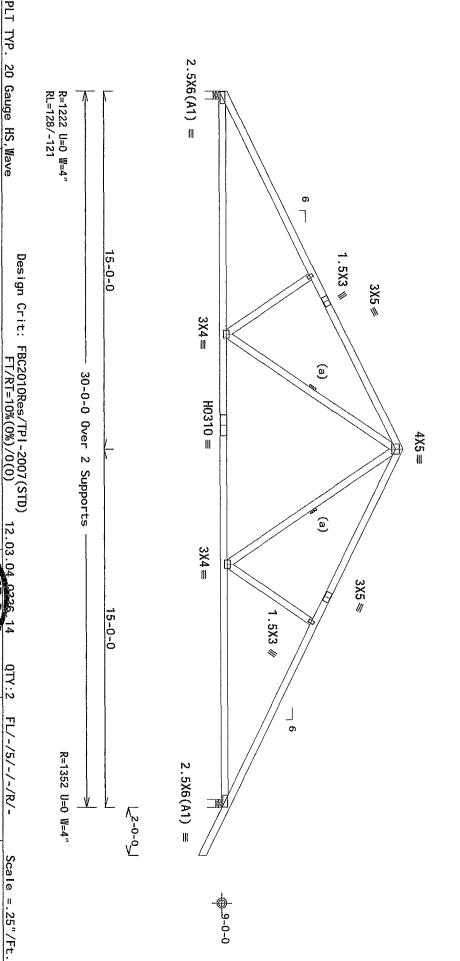
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

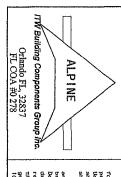
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCp:(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets $L/240\,$ live and $L/180\,$ total load. Creep increase factor for dead load is 1.50.





WARNING READ AND FOLLOW ALL NOTES ON THIS SHEETI
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating handling shipping installing and bracing Refer to a follow the latest edition of BCSI (Building Component Safety Information by TPI and MTDA) for safety proctices prior to performing these functions Installiers shall provide temporary bracing per BCSI Midess nated otherwise top chord shall have properly attended structural sheathing and bottom chord shall have a properly attended rigid of sling Locations slown for permanent lateral restraint of we shall have a properly attended rigid colling Locations slown for permanent lateral restraint of we shall have a properly attended rigid colling Locations slown for permanent lateral restraint of we shall have a properly attended rigid colling Locations slown for permanent lateral restraint of we shall have a properly attended rigid colling Locations slown for permanent lateral restraint of we shall have a properly attended rigid colling Locations slown for permanent lateral restraint of we shall have a properly attended rigid colling Locations slown for permanent lateral restraint of we shall have a properly attended rigid colling Locations slown for permanent lateral restraint of we shall have a properly attended rigid colling Locations slown for permanent lateral restraint of we shall have a properly attended at the properly attended structural shall have a properly attended at the pro

ITW Building Components Group Inc. (ITWBOS) shall not be responsible for any deviation from this department on the little terms in conformance with ABSI/TPI 1 or for handling shipping installation practing of trusses. Apply plates Go each foce of truss and position as shown above and on the Joint Details unless ented otherwise Refer to dresings 1904.7 for standard plates positions. A seal or the processing of cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure the responsibility of the Building Designer Page (ARSI/TPI 1 Sec 2 For more information see. This justice is the responsibility of the Building Designer Page (ARSI/TPI 1 Sec 2 For more information see. This justice)



10.0

0.0

PSF PSF

DRW HCUSR9114 14100003

SEQN-

365369

37.0 PSF 1.25

24.0"

JREF-

1V5F487_Z02

20.0 PSF

R9114- 53130

7.0 PSF

REF

04/10/14

Top chord 2x4 SP M-30
Bot chord 2x4 SP #1-13B :B3 2x4 SP 2850f-2.3E:
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved $1/30/2013\ \ by\ ALSC$

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10.00 psf non-concurrent live load

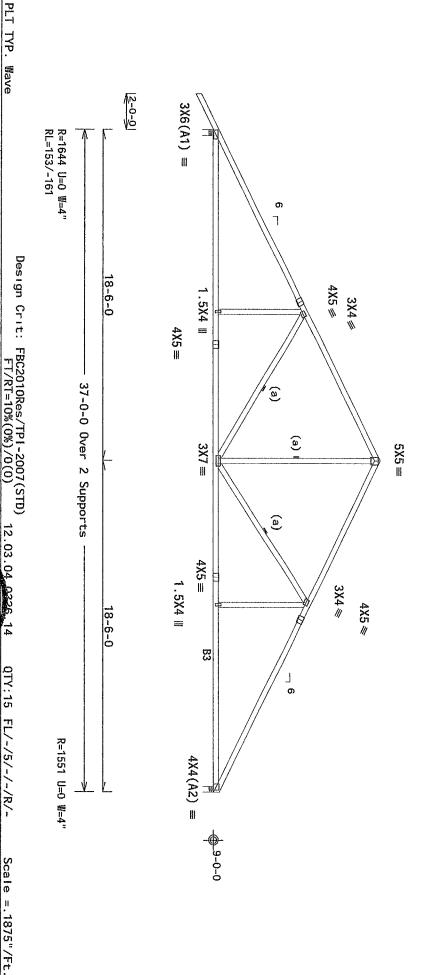
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5 0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on NWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



ALPINE
ALPINE

ALPINE

ALPINE

Truss

traitor

practice

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in febricating handling shipping installing and bracing Refer to a follow the latest edition of 8CS1 (Building Gosponent Safety Information by IP) and BIGN) for sarety practices prior to performing these functions. Installers that provide temporary bracing per BCS1 bliess noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly statched rigid occlining. Locations shown for permanent lateral restraint of well shall have a properly attached rigid occlining. Locations shown for permanent lateral restraint of well shall have a properly statched rigid occlining Locations shall have a prope

I'W Building Components Group inc (I'WBCC) shall not be responsible for any deviation from this edular increases apply places to each face of truss and position as shaping installation bracing of trusses. Apply places to each face of truss and position as shaping an apply places to each face of truss and position as shaped above and on the Joint Details unless noted otherwise. Refer to drawings 1604-2 for standard plate positions. A seel on the drawing or course page listing this drawing indicates acceptance of professional engineering reaponsibility soften by for the design shown. The suitability and use of this doesign for any structure the responsibility of the Building Designer per AMSI/TP1 1 Sec 2. For more information see. This joint page 178-800 was about that the page of the second page 178-800 was imposed our. TP1 was triant org WTCA was about distance on the page 178-800 was imposed our.

No. 22839

No. 22839

BC DL

BC LL

TOT.LD.

DUR. FACING

04/10/2014

SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL
24.0"	1.25	37.0 PSF	0.0 PSF	10.0 PSF	7.0 PSF
JREF- 1V5F487_Z02	FROM JAM	SEQN- 365264	HC-ENG SSB/WPF	DRW HCUSR9114 14100004	DATE 04/10/14

20.0 PSF

REF

R9114-

53131

Top chord 2x4 SP #1-13B :T4 2x4 SP M-30: Bot chord 2x4 SP 2850f-2.3E .B2 2x4 SP #1-13B Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved $1/30/2013\ \mbox{by ALSC}$

calculations. Conditions may exist that require different connecthan indicated. Refer to manufacturer publication for additional H = recommended connection based on manufacturer tested capacities and information. connections

These support conditions used at bearings indicated (H1) = HUS26 w/ (2)2x6 SP M-26 supporting member. (14) 0.148"x3" nails into supporting member, (4) 0.148"x3" nails into supported member.

Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

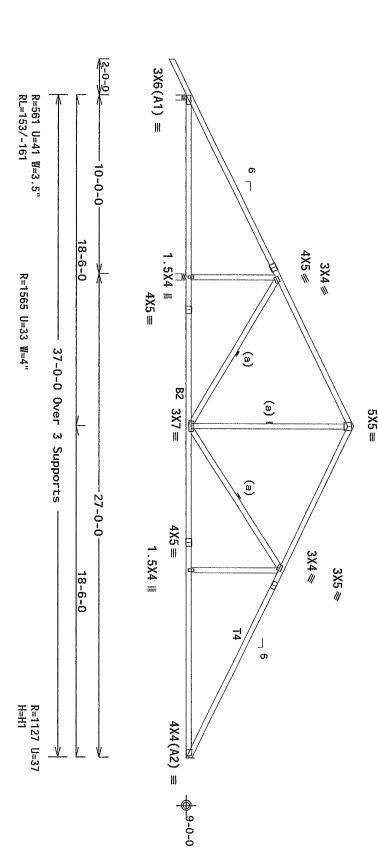
design. Wind loads and reactions based on MWFRS with additional C&C member

Hanger specified assumes connection to supporting chord is located minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

(a) Continuous lateral restraint equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load areas with $42^{\prime\prime}$ -high x $24^{\prime\prime}$ -wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0)

₹P.

Wave

** IMPORTANT

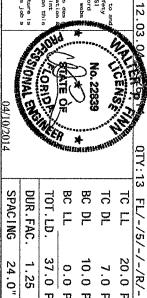
Trusses require extreme care in fabricating handling shipping installing and bracing Refer to an follow the latest edition of BCSI (Building Component Safety Information by TPI and BTCA) for safety practices prior to performing these functions installers shall provide temporary bracing per BCSI liness noted otherwise top chord shall have properly stached structural sheathing and bottom chord shall have a properly actached rigid colling Locations shown for permanent lateral restraint of webs shall have a properly actached rigid colling Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 87 or 810 as applicable

ITW Building Components Group Inc. (ITWBGC) shall not be responsible for any deviation from this doss any failure to build the trust in conformance with AMSI/TPI 1 or for handling shipping installation breating of trustees. Apply pieces to each focu of trust and position as shown above and on the Joint Decalls unless noted otherwise. Refer to drawings 1604-2 for standard plate positions. A seel on this drawing to cover page listing this drawing Indicates acceptance of professional engineering responsibility of the Building Designer has licability and use of this dosign for any structure if the responsibility of the Building Designer per AMSI/TPI 1 See 2 for more information see. This Job general notes page INU-DCC waw itselfer or West Committee of the Section of the Building Designer of the Section of the Building Designer of MSI/TPI 1 See 2 for more information see. This Job general notes page INU-DCC waw itselfer or y Structure is This job s

ITW Building Components Group

ALPINE

Orlando FL, 32837 FL COA #0 278



04/10/2014	The state of the s	O	OF PROPERTY.) 6	
SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL
24.0"	1.25	37.0 PSF	0.0 PSF	10.0 PSF	7.0 PSF
JREF- 1V5F487_Z02		SEQN- 365267	HC-ENG SSB/WPF	DRW HCUSR9114 14100005	DATE 04/10/14

20.0 PSF

REF R9114- 53132 Scale = .1875"/Ft.

Top chord 2x4 SP #1-138 Bot chord 2x4 SP #1-138 Webs 2x4 SP #3-138 Stack Chord SC1 2x4 SP

#1-13B.:Stack Chord SC2 2x4 SP #1-13B:

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, anywhere in roof, RISK CAT II, EXP B, wind DL=5.0 psf. GCpi(+/-)=0.18TC DL=3.5 psf, wind BC

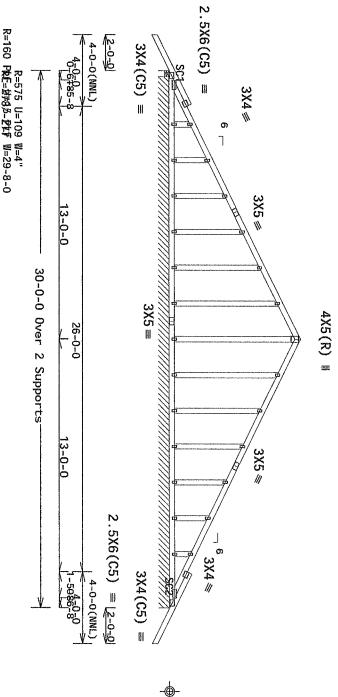
design. Wind loads and reactions based on MWFRS with additional C&C member

Truss spaced at 24.0° OC designed to support 2-0-0 top chord must outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched

In lieu of structural panels use purlins to brace TC @ 24"

Bottom chord checked for 10.00 psf non-concurrent live

Deflection meets $L/240\,$ live and $L/180\,$ total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X3 Except As Shown.

PLT TYP.

Wave

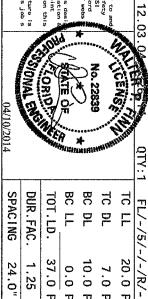
** IMPORTANT** **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET! Design Crit: FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)

Trusses require extreme care in febricating handling shipping installing and brasing Refer to a follow the latest edition of BCS1 (Suilding Component Safety Information by IPI and BTCA) for safety practices prior to performing these functions intestilets shall provide temporary bracing per BCS1 (Inless noted otherwise top chord shall have properly attended structural sheathing and bottom chord shall have a properly attended rigid colling Locations shown for permanent lateral restraint of webs shall have a properly attended rigid colling Locations shown for permanent lateral restraint of webs shall have a properly attended rigid colling Locations shown for permanent lateral restraint of webs shall have a properly attended rigid colling.

I'W Building Components Group Inc. (ITMSCS) shall not be responsible for any deviation from this design any failure to build the truss in conformance with AMSI/TPI 1 or for handling shipping installation bracing of trusses. Apply places to each face of truss and position as shown above and on the Joint Details unless noted otherwise. Refer to drawings 160A-2 for standard place positions. A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building Designor per AMSI/TPI 1 Sec 2. For more information see. This job general notes page 118-503 www taboog com. TPI www.cpinatorg. BTCA www.sbc/industry.com structure is This jobs

ITW Building Components Group Orlando FL, 32837 FL COA #0 278

ALPINE



04/10/2014	- ER	O VI	O Numa) §	······································	No.
SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	TC LL
24.0"	1.25	37.0 PSF	0.0 PSF	10.0 PSF	7.0 PSF	20.0 PSF
JREF- 1V5F487_Z02		SEQN- 330970	HC-ENG WHK/WHK	DRW HCUSR9114 14100007	DATE 04/10/14	REF R9114- 53133

Scale = .1875"/Ft.

Lumber grades designated with "13B" use design values approved $1/30/2013\ \ \mbox{by ALSC}$

Truss spaced at 24.0" OC designed to support 2-3-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24° o.c. intervals. Attach stacked top chord (SC)

> (**) 1 plate(s) require special positioning. Refer plot details for special positioning requirements. to scaled plate

DL=5.0 psf. GCpi(+/-)=0.18 120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC

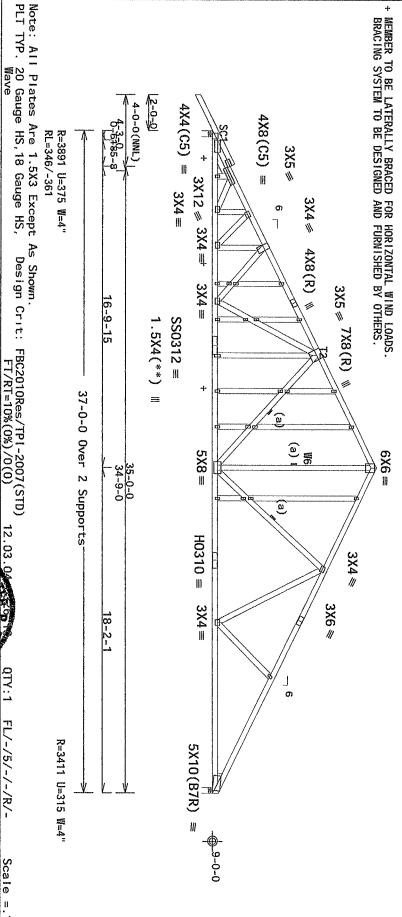
Wind loads and reactions based on MMFRS with additional C&C member design.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10.00 psf non-concurrent live load

factor for dead load is 1.50. Deflection meets L/240 live and L/180 total load. Creep increase



ITW Building Components Group Orlando FL, 32837 FL COA #0 278 ALPINE

** IMPORTANT** **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

12.03

0TY:1

FL/-/5/-/-/R/-

Scale = .1875"/Ft. R9114- 53134 04/10/14

DATE REF

SEQN-FROM

349476

JREF- 1V5F487_Z02

HC-ENG WHK/WHK

DRW HCUSR9114 14100010

Trusses require extreme care in fabricating thandling shipping installing and bracing Refer to a follow the latest edition of BCSI (Building Component Safety information by TPI and BTCA) for safety practices prior to performing these functions installers shall provide temporary breating per BCSI billies nated otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid continues to be account of the base of properly attached rigid continues to be account of the base of properly attached rigid continues to be account of the base of the properly attached rigid continues to be account for permanent lateral restraint of web shall have a properly attached rigid continue to be account for permanent lateral restraint of web shall have a properly attached rigid continue to be accounted to the properly attached rigid continues to be accounted to the properly attached rigid continues to be accounted to the properly attached rigid continues to the properly attached to the provide the properly attached attached to the properly attached to the provide the properly attached attached to the properly attached attached to the provide the prov

ITW Building Components Group Inc (ITWSCG) shall not be responsible for any deviation from this design any failure to build the trues in comformance with AMS//TPI 1 or for handling shipping installation bracing of truesses Apply plates to each foce of trues and position as shown above and on the Joint Betails unless noted otherwise Refer to drawings 160A-Z for standard plate positions. A soal on this drawings recover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is The suitability and use per ANSI/TPI 1 Sec 2 F

structure is This job s CENS ORIO)4/10/2014 BC LL BC DL TC DL TC LL SPACING DUR.FAC. TOT.LD. 20.0 PSF 24.0" 1.25 37.0 PSF 0.0 10.0 PSF 7.0 PSF PSF

Top chord 2x4 SP #1-13B Bot chord 2x4 SP #1-13B Webs 2x4 SP #3-13B 'Stack Chord SC1 2x4 SP

#1-13B: 'Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more

perpendicular to chord length. Splice top chord in notchable area using 3x6. Center plate on stacked/dropped chord interface, plate length to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24° o.c. intervals. Attach stacked top chord (SC)

> 120 mph wind, 15.00 ft mean hgt, ASCE 7-10, anywhere in roof, RISK CAT II, EXP B, wind DL=5.0 psf. GCpi(+/-)=0.18TC DL=3.5 psf, wind Located , wind BC

Wind loads and reactions based on MWFRS with additional C&C member

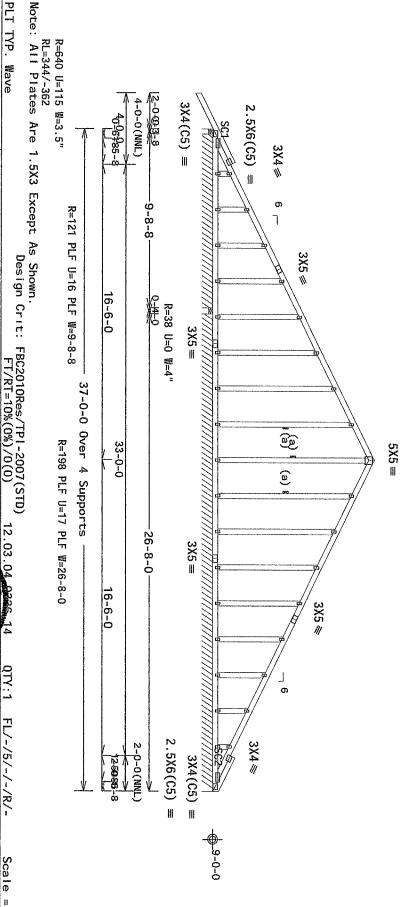
not be cut or notched. Truss spaced at 24.0" OC designed to support 2-3-0 top chord must

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10,00 psf non-concurrent live load

factor for dead load is 1.50. Deflection meets L/240 live and L/180 total load. Creep increase



ITW Building Components Group Orlando FL, 32837 FL COA #0 278 **ALPINE**

*IMPORTANT** **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!

12.03.04

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Scale = .1875"/Ft R9114- 53135

PLT TYP. Wave

Trusses require extreme care in fabricating handling shipping installing and bracing Refer to a follow the latest addition of 8051 (Suiling Component Safety Information by TPI and \$TCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per 8051 Unless noted otherwise top chord shall have properly attached structural shallnave a properly attached frigid calling Locations shown for permanent lateral restraint or web shall have a properly attached frigid calling Locations shown for permanent lateral restraint or web shall have a properly attached frigid calling Locations shown for permanent attached restraint or web shall have a properly attached frigid calling Locations shown for permanent attached restraint or web shall have a properly attached restraint or web shall have a properly attached frigid calling Locations shown for permanent lateral restraint or web shall have a properly attached frigid calling the shall have a properly attached frigid from the shall have a properly attached friging from the shall have a properly attached frigid from the shall

I'W Building Components Group Inc. (IWBCG) shall not be responsible for any day ation from this death for iure to build the truss in conformance that AMSI/TPI 1 or for handling shipping interselestic bracing of trusses. Apply places to each face of truss and position as shown above and on the Joint Decails unless noted otherwise Refer to drawings 160A-Z for standard plate positions. A seal on it drawing or cover page 11sting this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure the responsibility of the Building Designer per AMSI/TPI 1 Sec 2. For more information see. This jumperal notes page 11N-BCG was iterace on TPI www.tpinet.org WTAA waw abcindancy con

金田 CENS? No. 22839 CORIO 04/10/2014 SPACING ВС TC DL DUR. FAC. TC LL TOT.LD. 37.0 1.25 20.0 PSF 10.0 PSF 0.0 24.0" 7.0 PSF PSF PSF

SEQN-

REF-

1V5F487_Z02

HC-ENG SSB/WPF DRW HCUSR9114 14100006 DATE 쮸

04/10/14

Top chord 2x4 SP 2850f-2.3E :T2 2x4 SP #1-13B: Bot chord 2x4 SP #1-13B Webs 2x4 SP #3-13B

Lumber grades designated with 1/30/2013 by ALSC "13B" use design values approved

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

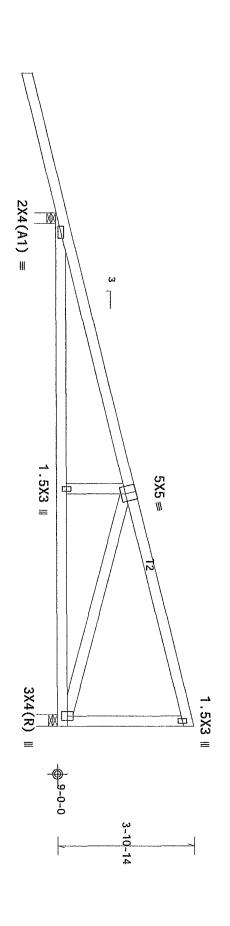
Top chord overhangs have been checked only for loads as indicated Overhangs not checked for man loads or long-term deflection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on NWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure

MWFRS loads based on trusses located at least 15.00 ft. from roof





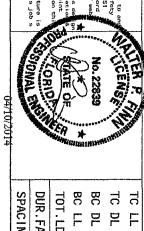
WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extrese care in fabricating handling shipping (notalling and bracing Refer to an follow the latest edition of BSS (Bailding Component Safety) Information by TPI and BTGA) for safety practices prior to performing these functions installers shall provide temporary bracing per BSS liniess noted otherwise top chard shall have properly attached structural shaething and bottom chard shall have a properly attached of colling Locations shall have a properly attached to the colling Locations shall have a properly attached to the colling Locations shall have a properly attached to the colling Locations shall have a properly attached to the colling Locations shall have a properly attached to the colling Locations shall have a properly attached to the colling Locations shall have a properly attached to the colling Locations shall have a properly attached to the colling Locations shall have a properly attached to the colling Locations shall have a properly attached to the colling Locations shall have a properly attached to the colling Locations shall have a properly attached to the colling Locations shall have a properly attached to the colling Locations shall have a properly attached to the

I'W Building Components Group Inc. (I'WESD) shall not be responsible for any day atton from this of any failure to build the truss in conformance with ABSI/TPI 1 or for handing shipping installation bracing of trusses. Apply plates to each face of truss and position as stoom above and on the Joint Details unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on drawing or cover page listing this downing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure.

ITW Building Components Group Orlando FL, 32837 FL COA #0 278

ALPINE



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	SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL
	24.0"	1.25	37.0 PSF	0.0 PSF	10.0 PSF	7.0 PSF
	JREF- 1V5F487_Z02	FROM JUNE	SEQN- 349480	HC-ENG WHK/WHK	DRW HCUSR9114 14100004	DATE 04/10/14

20.0 PSF

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Scale = .375"/Ft. R9114-53136

Top chord 2x4 SP 2850f-2.3E Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved $1/30/2013\ \mbox{by ALSC}$

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

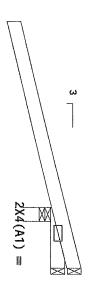
Top chord overhangs have been checked only for loads as indicated. Overhangs not checked for man loads or long-term deflection.

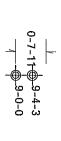
Negative reaction(s) of -249# NA case requires uplift connection. of -249# MAX. (See below) from a non-wind load

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on NWFRS with additional C&C member design.

R=-250 Rw=104 U=114





R=-134 Rw=61 U=58

3-10-12 1-3-4 Over 3 Supports R=707 U=242 W=3.5" RL=32

Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0)

PLT TYP.

Wave

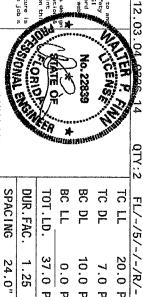
Trusses require extreme care in fabricating, handling shipping inestalling and bracing Refer to a follow the latest edition of BCSI (Building Component Safety) information by TPI and WTCA) for safety practices prior to performing these functions installers shall provide temporary bracing per BCSI linkess noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached structural stacking and bottom chord shall have a properly attached rigid colling Locations shown for permanent stackal restraint of was shall have a properly attached rigid colling Locations shown for permanent stackal restraint of was shall have a properly attached rigid colling Locations shown for permanent stackal restraint of was ** IMPORTANT** **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!

ITW Building Components Group Inc (ITRECC) shall not be responsible for any deviation from this day failure to build the truss in conformance with MRS/TPI 1 or for handling shipping installation bracing of trusses Apply places to seek face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to drawings 1004-Z for standard plastings A seal on drawing or cover page listing the drawing indicates acoptance of porfessional engineering responsibility softly for the design shown. The suitability and use of this dealign for any structure the responsibility of the Building Designer par AMS/TPI 1 Sec 2 for more information see This jumparal notes page. TW-BCG www. labelg.com. TPI www.tpinstorg. BTIA www.sbcindustry.com.

ITW Building Components Group

ALPINE

Orlando FL, 32837 FL COA #0 278



04/10/2014	N		O STATE OF THE STA	1 ×	INITER S	ALIMIN A
	SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	IC DL
	24,0"	1.25	37.0 PSF	0.0 PSF	10.0 PSF	7.0 PSF
	JREF- 1V5F487_Z02	FROM JANU	SEQN- 365311	HC-ENG SSB/WPF	DRW HCUSR9114 14100007	DATE 04/10/14

20.0 PSF

REF

53137

Scale = .5"/Ft R9114-

Top chord 2x4 SP 2850f-2.3E Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved $1/30/2013\ \mbox{by ALSC}$

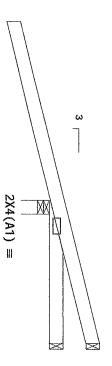
Bottom chord checked for 10.00 psf non-concurrent live load

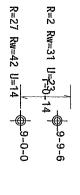
Top chord overhangs have been checked only for loads as indicated. Overhangs not checked for man loads or long-term deflection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on NWFRS with design. additional C&C member

Deflection meets $L/240\,$ live and $L/180\,$ total load. Creep increase factor for dead load is 1.50.







Design Crit: FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)

!MPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

PLT TYP.

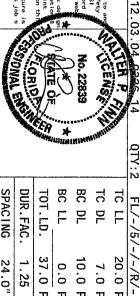
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Trusses require extreme care in fabricating bondling shipping installing and bracing Refer to a follow the latest edition of BCSI (Building Component Sefecy Information by ITP) and BTCB) for sefecy practices prior to performing these functions. Installers shall provide temporary breating per BCSI bliness noted otherwise top chord shall have properly attached structurel shaething and bottom chord shall have a properly attached rigid colling. Locations shown for permanent lateral restraint of web shall have a properly attached rigid colling. Locations shown for permanent lateral restraint of web shall have a properly attached rigid colling.

I'W Building Components Group Inc (ITWSCS) shall not be responsible for any deviation from this dept failure to build the truss in conformance with ANS/TPI 1 or for handling shipping instaliation bracing of trussess. Apply places to each face of truss and position as shown above and on the Joill betails unless noted otherwise. Refer to drawings 160A-2 for standard plaste positions. A scal on drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility of the Suitable shown. The suitability and use of this design for any structure the responsibility of the Suitable Designer per ANS/TPI 1 Sec 2 For more information see. This jegueral notes page ITW-BCC www invocay com. TPI www.tpinst.org. WICA www.scindarty.com.

TW Building Components Group Orlando FL, 32837 FL COA #0 278

ALPINE



04/10/2014	V		SER WAR		MINNA 	IMUI.
	SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL
,	24.0"	1.25	37.0 PSF	0.0 PSF	10.0 PSF	7.0 PSF
The second secon	JREF- 1V5F487_Z02	FROM JAME	SEQN- 365348	HC-ENG SSB/WPF	DRW HCUSR9114 14100008	DATE 04/10/14

20.0 PSF

REF

R9114- 53138

Scale =.5"/Ft.

Top chord 2x4 SP 2850f-2.3E Bot chord 2x4 SP #1-13B

Lumber grades designated with "138" use design values approved $1/30/2013\ \mbox{by ALSC}$

Bottom chord checked for 10.00 psf non-concurrent live load

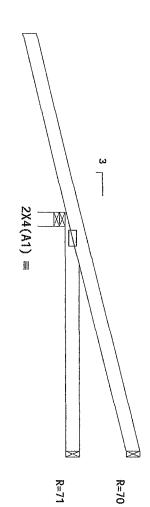
Top chord overhangs have been checked only for loads as indicated. Overhangs not checked for man loads or long-term deflection.

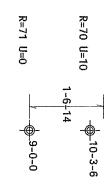
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

design. Wind loads and reactions based on NWFRS with additional C&C member

Deflection meets L/240 live and L/180 total load. Creep factor for dead load is 1.50. increase

NWFRS loads based on trusses located at least 7.50 ft. from roof edge.







Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0)

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

PLT TYP.

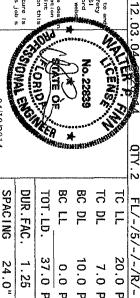
Mave

Trusses require extrame care in fabricating handling shipping installing and bracing Refor to a follow the latest edition of BCSI (Building Component Safety Information by TPI and BTDA) for safety practices prior to performing these functions installers shall provide temporary bracing per BCSI follows and otherwise top chord shall have properly attached structural steathing and bottom chord shall have properly attached injul calling Locations shown for permanent lateral restraint of webs shall have a properly attached rigid calling Locations shown for permanent lateral restraint of webs shall have a properly attached rigid calling Locations shown for permanent lateral restraint of webs shall have a properly attached rigid calling Locations shown for permanent values. **!MPORTANT**

ITW Building Components Group Inc (ITMBCG) shall not be responsible for any deviat on from this design feiture to build the truss in conformance with AMSI/TPI 1 or for handling as pping installation bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise. Refer to drawings 160h-2 for standard plate positions. A seal on the drawing or come page listing this drawing includes acceptance of professional engineering responsibility solely for the design shown. The saleability and use of this design for any structure the responsibility of the building Designer per AMSI/TPI 1 Sec 2. For more information sea. This job speamer integer page ITM-BCG www.itmbog.com. TPI www.tpinstorg.WTCA www.sbcindustry.com

TW Building Components Group line. Orlando FL, 32837 FL COA #0 278

ALP INE



2014		E) Marina Marina		W. Links	ARTIC
SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	TC LL
24.0"	1.25	37.0 PSF	0.0 PSF	10.0 PSF	7.0 PSF	20.0 PSF
JREF- 1V5F487_Z02	FROM JAM	SEQN- 365344	HC-ENG SSB/WPF	DRW HCUSR9114 14100009	DATE 04/10/14	REF R9114- 53139

Scale = .5"/Ft.

Top chord 2x4 SP 2850f-2.3E Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved $1/30/2013\ \mbox{by ALSC}$

Bottom chord checked for 10.00 psf non-concurrent live load

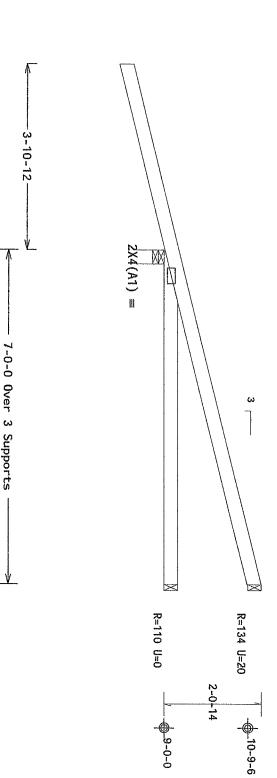
Top chord overhangs have been checked only for loads as indicated. Overhangs not checked for man loads or long-term deflection.

120 mph wind, 15.00 ft mean hgt, \overrightarrow{ASCE} 7-10, CLOSED bidg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. $\overrightarrow{GCpi(+/-)}$ =0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

NWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0)

12.03.0

20.0 PSF

REF

R9114- 53140

Scale =.5"/Ft.

7.0 PSF

DATE

04/10/14

R=564 U=106 W=3.5

PLT TYP.

Mave

Trusses require extress care in fabricating handling shipping installing and bracing Refer to a follow the lobast edition of BCSI (Building Component Safety Information by TP) and BTGA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI (Inless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached residuation shall have a properly attached right observed right of calling Locations shown for permanent lateral restraint of webs shall have a properly attached right occions 83 87 or 810 as applicable **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!

ITW Building Components Group Inc (ITWECG) shall not be responsible for any deviation from this deal any failure to build the truss in conformance with ARS//PH 1 or for handling shipping installation bracing of trusses. Apply places to each face of truss and position as shown above and on the Joint Bessits unless noted otherwise. Refer to drawings 160A-2 for standard place positions. A seal on thi drawing or come page listing this drawing includes acceptance of professional engineering responsibility of the Besider by Salving includes acceptance of this design for any structure the responsibility of the Building Busigner por ARS/IFPI 1 Sec 2. For more information see This job general notes page ITW-BCG www.itabog.com. TPI www.tpinstorg. WICA www.sbcindustry.com.

ITW Building Components Group

ALP INE

Orlando FL, 32837 FL COA #0 278

CENS YONAL ENGIN <u>14/10/2014</u> QTY:4 BC LL BC DL TC DL TC LL SPACING DUR. FAC. TOT.LD. FL/-/5/-/-/R/-

37.0 PSF

SEQN-

365303

0.0 PSF

10.0 PSF

DRW HCUSR9114 14100010 HC-ENG SSB/WPF

1.25

FROM

24.0"

JREF- 1V5F487_Z02

Top chord 2x4 SP M-30 Bot chord 2x4 SP 2850f-2.3E Webs 2x4 SP #3-13B

Lumber grades designated with 1/30/2013 by ALSC

"13B" use design values approved

Special loads 55588855588 From 193.48 134.13 329.15 109.71 From From -(Lumber From From From r Dur.Fac.=1.25 / F 54 plf at -3.90 t 27 plf at 7.00 t 4 plf at 7.00 t 4 plf at 0.00 170 plf at 0.29 160 plf at 7.03 8 lb Conc. Load at 3 lb Conc. Load at 5 lb Conc. Load at 11 lb Conc. Load at 0 to 54 plf at 7.0
0 to 27 plf at 14.0
0 to 4 plf at 0.0
0 to 20 plf at 0.0
0 to 20 plf at 7.0
0 to 160 plf at 7.0
0 to 160 plf at 14.0
0 to 160 plf at 14.0
0 to 9.06,11.06,13.06
0 to 9.06,11.06,13.06 Plate Dur.Fac.=1.25)
54 plf at 7.00
27 plf at 14.33
4 plf at 0.29
20 plf at 0.29
170 plf at 7.03
160 plf at 14.33

> 120 mph wind, 15.00 ft mean hgt, ASCE 7-10, within 4.50 ft from roof edge, RISK CAT II, wind BC DL=5.0 psf. GCpi(+/-)=0.18 CLOSED bldg, not located EXP B, wind TC DL=3.5 ps

Wind loads and reactions based on NWFRS

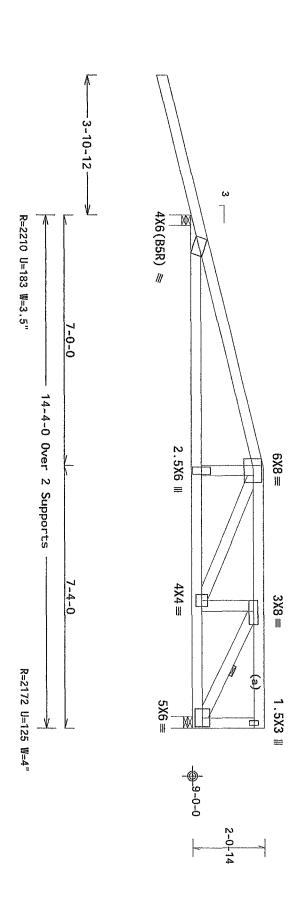
Right end vertical not exposed to wind pressure

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace all flat TC @ 0C.

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load. Greep increase factor for dead load is 1.50.



WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS !NCLUDING INSTALLERS Design Crit: FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)

PLT TYP.

Wave

** IMPORTANT**

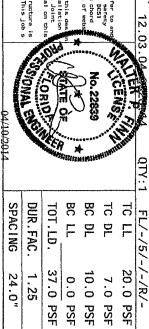
Trusses require extreme care in fabricating handling shipping installing and bracing Refer to a follow the latest edition of BCSI (Building Component Safety Information by TPI and \$TDA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI (Building Safety) attached structural sheathing and bottom chord shall have a properly attached of colling Locations shall have a properly attached rigid colling Locations shall have a properly attached structural shall have a properly attached rigid colling Locations shall have a properly attached structural shall have a properly attached structural shall have a properly attached shall have a properly attached structural shall have a properly attached shall have a p

ITW Building Components Group Inc (ITWBCG) shall not be responsible any failure to build the truss in conformance with AUS/ITP1 to right bracing of trusses Apply plates to each face of truss and position as Details unless noted otherwise Refer to drawings 160A-Z for standard drawing or cover page listing this drawing responsibility solely for the design shown the responsibility of the Building Designer

ITW Building Components Group

ALPINE

Orlando FL, 32837 FL COA #0 278



70014		EE,	Petersi	**************************************	W. WALL
SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL
24.0"	1.25	37.0 PSF	0.0 PSF	10.0 PSF	7.0 PSF
JREF- 1V5F487_Z02	FROM JIMW	SEQN- 365362	HC-ENG SSB/WPF	DRW HCUSR9114 14100013	DATE 04/10/14

Scale = .375"/Ft. R9114-53141

Top chord 2x4 SP 2850f-2.3E :T2 2x4 SP #1-13B: Bot chord 2x4 SP #1-13B Webs 2x4 SP #3-13B

Lumber grades designated with 1/30/2013 by ALSC "13B" use design values approved

Special loads From From From From From Lumber Dur. Fac.=1
54 plf at
54 plf at
54 plf at
20 plf at
20 plf at
20 plf at 1.25 / -3.90 -3.90 -3.90 0.00 0.29 14.00 44444 Plate Bur.Fac.=1.25)
54 pif at 9.00
54 pif at 14.33
4 pif at 0.00
20 pif at 0.29
50 pif at 14.00
20 pif at 14.00

Right end vertical not exposed to wind pressure.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf wind BC DL=5.0 psf. GCpi(+/-)=0.18

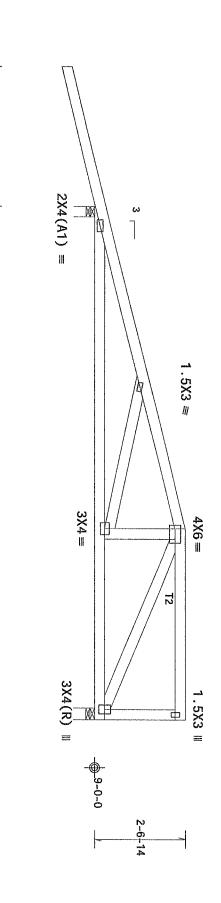
In lieu of structural panels use purlins to brace 0C. all flat TC @ 24"

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets $L/240\,$ live and $L/180\,$ total load. Creep increase factor for dead load is 1.50.

MMFRS loads based on trusses located at least 7.50 ft. from roof edge.

Wind loads and reactions based on MFRS.



PLT TYP. Wave **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS Design Crit: FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0) 12.03.0

R=1009 1

U=97

14-4-0 Over

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Supports

R=697 U=21 W=4"

5 - 4 - 0

3-10-12-

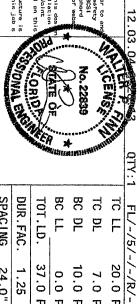
Trusses require extreme care in fabricating, handling shipping installing and bracing Refer to a follow the lettest edition of BCS1 (Building Component Softer) Information by TPI and BTCA) for safety practices prior to performing these functions installers shall provide temporary bracing per BCS1 (bliness noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached of siglide calling locations shown for permanent lateral restraint of webs shall have a properly attached rigid celling locations shown for permanent lateral restraint of webs shall have a properly attached rigid celling locations shown for permanent lateral restraint of webs shall have a properly attached rigid celling. **IMPORTANT**

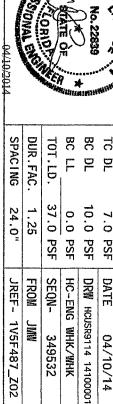
I'W Building Components Group Inc (I'NBGC) shall not be responsible for any deviation from any feliure to build the trues in conformance with MBCI/FPI in for handing shipping in bracing of truesses Apply plates to each face of trues and position as shamn above and on the Betails unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A ferming or cover page (isting this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and see of this design for any the responsibility of the Seid citied Dasigner for MSI/FPI 1 Sec 2. For more information see general notes page. ITM-BCS new introop com. TPI waw topinst org MTCA new secindarity com.

ITW Building Components Group

ALPINE

Orlando FL, 32837 FL COA #0 278





20.0 PSF

REF

Scale = .375"/Ft. R9114-53142

within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18 PLT TYP. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FABRICATOR TO REVIEW THIS DWG PRIOR TO CUTTING LUMBER TO VERIFY THAT ALL DATA, INCLUDING DIMENSIONS AND LOADS, CONFOTTO THE ARCHITECTURAL PLANS/SPECIFICATIONS AND FABRICATOR'S TRUSS LAYOUT. Overhangs not checked for Wind loads and reactions based on WWFRS Lumber grades designated with "13B" use design values approved $1/30/2013\ \ by\ ALSC$ Top chord 2x4 SP 2850f-2.3E Bot chord 2x4 SP #1-13B Webs 2x4 SP #3-13B Bottom chord checked for 10.00 psf non-concurrent live load ITW Building Components Group Top chord overhangs have been checked only for loads as indicated. ALPINE Wave man loads or long-term deflection. bracing of trusses Apply places to each face of truss Betails unless noted otherwise Refer to drawings 180 drawing cover page listing this drawing indicates a responsibility solely for the des makes the manufacture of the Building Designer per AMS/7P Trusses require extreme care in fabricating handling shipping installing and bracing Befor to a follow the latest edition of BCSI (Building Component Safety Informat on by TPI and BTDA) for safety practices prior to performing these functions installers shall provide temporary bracing per BCSI bliess noted otherwises top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached crigit of thing Locations shown for permanent lateral restraint of webs shall have a properly attached rigit of thing Locations shall have a properly attached rigit of the shall have a properly attached and the shall have a properly attached and the shall have a properly attached the shall have a properly attached and the shall have a shall have bracing installed per ** IMPORTANT** ITE Building Components Group Inc (ITEBCS) shall not be responsible for any deviation from this des y feilure to build the truss in conformance with AMSI/TPI 1 or for handling shipping installat or 5-6-2 **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS Design Crit: FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0) 2,12 e of truss and position as shown above and on the Jawings 160A-Z for standard plate positions. A seal CONFORM R=403 U=353 W=4.95 2X4(A1) =Structure This job Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Special loads 9-10-13 Over 3 Supports BC- From BC- From TC- -145.84 12.03.0 CENS BC LL BC DL TC DL TC LL 54 pif at 2 pif at 4 pif at 2 pif at DUR. FAC. TOT.LD. Dur.Fac.=1.25)
54 plf at 0.00
2 plf at 9 90
4 plf at 0.00
2 plf at 0.00
2 plf at 9.90 FL/-/5/-/-/R/-37.0 PSF 20.0 PSF 1.25 10.0 PSF 0.0 7.0 PSF 4X4 =PSF SEQN-DATE REF FROM HC-ENG SSB/WPF DRW HCUSR9114 14100011 Scale = .5"/Ft. R=219 U=103 R=59 U=0 R9114- 53143 04/10/14 365353 2-0-12 10-9-4

04/10/2014

SPACING

24.0"

JREF- 1V5F487_Z02

Top chord 2x4 SP 2850f-2.3E .T2 2x4 SP #1-13B. Bot chord 2x4 SP #1-13B Webs 2x4 SP #3-13B

Lumber grades designated with 1/30/2013 by ALSC "13B" use design values approved

Special loads -(Lumber From From From From From Tr Dur.Fac.=1.25 / P 54 plf at -3.90 t 54 plf at 11.00 4 plf at -3.90 t 20 plf at 0.00 t 50 plf at 0.29 t 20 plf at 14.00 t Plate 444444 Dur.Fac.=1.25)
54 pif at 11.00
54 pif at 14.33
4 pif at 0.09
20 pif at 0.29
50 pif at 14.00
20 pif at 14.00

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf wind BC DL=5.0 psf. GCpi(+/-)=0.18

Right end vertical not exposed to wind pressure.

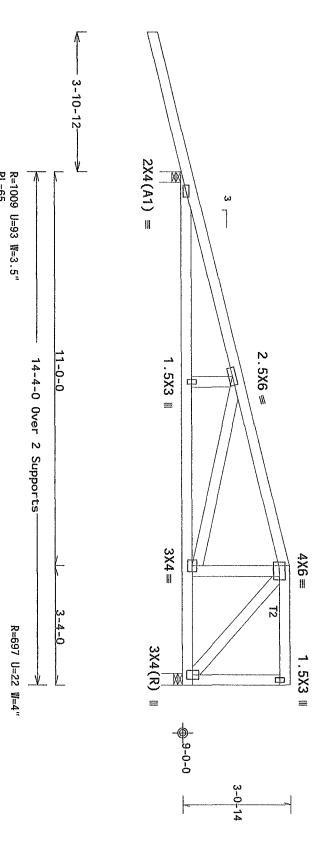
In fieu of structural panels use purlins to brace all flat TC @ 24" $\,$ 0C.

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

NWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Wind loads and reactions based design. 9 NNVFRS with additional C&C member



PLT TYP. Wave **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS Design Crit: FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)

** IMPORTANT**

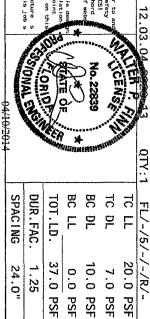
shall have a properly attached rigid ceiling Loc shall have bracing installed per BCSI sections B3 Trusse require extreme ears in fabricating bandling shipping installing and breing Befor to a follow the latest edition of BS3 (Bulling Component Safety) information by Piplinds WIDN, for service practices prior to performing these functions install bers shall be proprily attached significant of weight blacks noted otherwise top choose shall have proprily attached of jid one illing installing and bettem about the shall have a property attached of jid one illing installing and bettem about the shall have a property attached of jid one illing installing and bettem about the property attached of jid one illing installing and bettem about the property attached of jid one illing installing and bettem about the property attached at Locations shown for permanent lateral B3 B7 or B10 as applicable

drawing or cover page listing this drawing responsibility solely for the design shown the responsibility of the Building Designer ITW Building Components Group inc any failure to build the truss in con bracing of trusses Apply plates to Details unless noted otherwise Rei th AMSI/TPI 1 or for hand

ITW Building Components Group Inc

ALPINE

Orlando FL, 32837 FL COA #0 278



1.25 37.0 PSF 10.0 PSF 24.0" 0.0 7.0 PSF PSF DATE FROM SEQN-DRW HCUSR9114 14100002 HC-ENG JREF-1V5F487_Z02 門大学芸 04/10/14 349538

罕

Scale = .375"/Ft. R9114-53144

Top chord Bot chord chord 2x4 SP chord 2x4 SP Webs 2x4 SP #1-13B #1-13B #3-13B T1 2x4 SP 2850f-2.3E

Lumber grades designated with "13B" 1/30/2013 by ALSC use design values approved

In fieu of structural panels use purlins to brace all flat TC @ $24^{\prime\prime}$ OC.

Deflection meets L/240 live and factor for dead load is 1.50.

L/180 total load. Creep increase

Top chord overhangs have been checked only for loads as indicated. Overhangs not checked for man loads or long-term deflection.

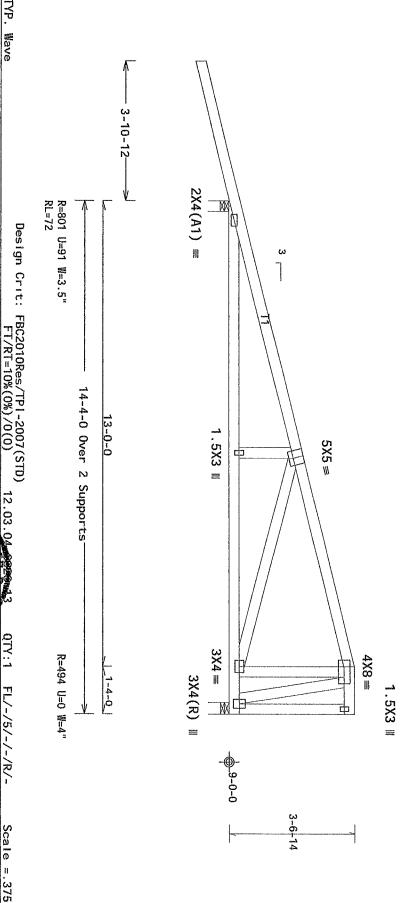
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

design. Wind loads and reactions based on NWTRS with additional C&C member

Right end vertical not exposed to wind pressure

Bottom chord checked for 10.00 psf non-concurrent live load

NWFRS loads based on trusses located at least 15.00 ft. from roof



PLT TYP.

Have

!MPORTANT

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

CENS

FL/-/5/-/-/R/-

Scale = .375"/Ft. R9114- 53145

20.0 PSF

7.0 PSF PSF

> DATE REF

04/10/14

BC LL BC DL TC DL TC LL

0.0 PSF

HC-ENG WHK/WHK DRW HCUSR9114 14100003

SEQN-

349490

10.0

TW Building Components Group

ITW Building Components Group inc. (ITWGC) shall not be responsible for any deviation from this deal any failure to build the truss in conformance with AMSI/TPI 1 or for handling shipping installation bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise. Refer to drawings 160k-7 for standard plate positions. A seal on the training or come page 11sting this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure the responsibility of the Building Designar par NASI/TPI 1 Sec 2. For more information see. This job general notes page 11%-BCG www. Itabag com. TPI www.tpinet.org. WTCA www.sccilous.try.com.

04/10/2014

SPACING DUR. FAC TOT.LD.

24.0"

JREF- 1V5F487_Z02

1.25 37.0 PSF

FROM

Trusses require extreme care in fabricating bandling shipping inestiling and been of Riger to a follow the latest edition of BSS (Guit ding Component Safety Information by TPI) and BTO) for safety practices prior to performing these functions on lets lets shall provide temperary breeing per BSS (Unless noted otherwise top days shall have properly stateded structural Sweathing and bottom chord shall have a properly attached of figure editing Locations shown for permanent lateral estatistic of easis shall have a properly attached figure editing Locations shown for permanent lateral estatistic of easis shall have a properly attached figure editing Locations shown for permanent lateral estatistic of easis

shall have a properly attached rigid ceiling Locations shown for permanent shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

ALP I NE

Orlando FL, 32837 FL COA #0 278

Reinforcing Member

is specified on a truss design but an alternative web This detail is to be used when a Continuous Lateral Restraint (CLR) reinforcement method is desired.

Notes:

shown on single ply sealed designs to L-reinforecement or scab reinforcement This detail is only applicable for changing the specified CLR T-reinforcement or

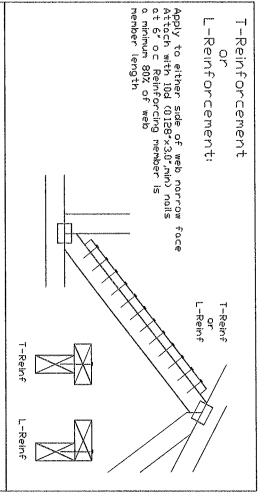
Alternative reinforcement specified in chart below may be conservative reinforcement type for minimum alternative reinforcement, re-run design with appropriate

2-2×6(*)	50 XX 65	n rows	N N X X X X X X X X X X X X X X X X X X
1-2×6 2-2×4(米)	10 10 X X 60 4	1 row 2 rows	(N) (N)
1-2×4 2-2×4	N N X X X X X X X X X X X X X X X X X X	2 rows	PXS OF PXA
Scab Reinf	Alternative Reinforecement I- or L- Reinf Scab Reinf	Specified CLR Restraint	Web Member Size

T-reinforcement, L-reinforcement, or scab reinforcement to be species and grade or better than web member unless specified otherwise on Engineer's sealed design. SOME

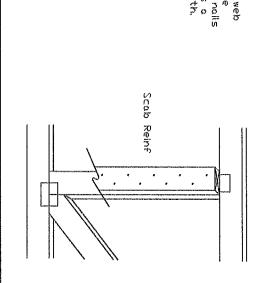
 \Re face of web Center scab on wide face of web Apply (1) scab to each

SUBSTITUTION



9000 Reinforcement:

Apply scab(s) to wide face of web No more than (1) scab per face Attach with 10d (0128"×30",min) nails at 6" oc Reinforcing member is a minimum 80% of web member length.



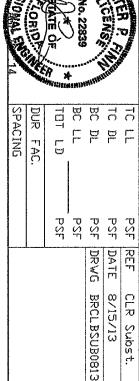


noting of trusses.

sed on this drowing or cover page listing this drowing, indicates acceptance of professional interring responsibility solely for the design show. The suitability ond use of this drowing any structure is the responsibility of the Balding Designer per ANSI/TRI 1 Sec2.

for more information see this job's general notes page and these web sites ITYECs wereliebocked; it was the strong SECs wereliebocked; it's was the second of the

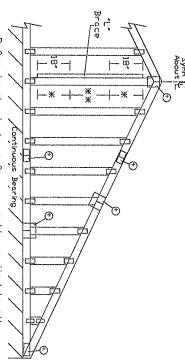
Earth City MO 63045



ASCE 7 - 1012 12 0 E C 9 9 Wind Speed, 15' Mean Height, Gable Stud Reinforcement Enclosed, Detail Exposure \Box スNさ

100 mph Wind Speed, 15 Mean Height, Partially Enclosed, Expasure C, Kzt = 1.00 100 mph Wind Speed 15' Mean Height, Enclosed, Expasure D, Kzt = 1.00

		\	10))	×		G	a	b	1 (1	2		\ \	∕ ∈	? r	~ -	tı	C	0	l			9	r	16) -	lk	voterments.	
		16	<u></u>	,	C	Э,	C				16	- > "	,	C),	<u>_</u>	1		í	2	4	//	(0	. C	-		Spacing	55
]	(/)			7	7 7	1		(\(\frac{1}{\tau}\))			<u> </u>	7	1	1				3	7	シてて)	cing Species	D×4
	Standard	Stud	#3	3#	#1	Standard	Stud	£#3	3# / #5	Standard	Stud	#3	#"	#1	Standard	Stud	#3	2# / 1#	Standard	Stud	ä	#2	##	Standard	Stud	#.	#1 / #2	Grade	Brace
	δ. Ωί	5 9"	5' 9"	6' 1"	6' 2"	5′ 9″	5′ 9″	5, 9,	6'1"	ာ သု	5′ 3′	ပၢ သူ	5′ 6.	5 8,	5' 3'	5′ 3′	5' 3"	5 6,	4' 7"	4' 7'	4 7"	4' 10"	4' 11"	4 7"	4' 7'	4' 7"	4 10"	Braces	5
	ο, ού	9, 9,	9, 9,	10′ 4″	10′ 5″	10, 5,	10′ 2″	10' 2'	10' 4"	7' 4"	8' 6"	8, 6,	9' 5'	9′ 5′	93.	9' 3'	9' 3"	9, 5,	6, 0,	6 11'	6' 11'	Ľ, α	φ ω	φ,	8, 1,	7' 9"	8 2,	Group A	(D) 1×4 'L'
	9, 0,	10′ 5′	10' 5"	10′ 8 ′	10 9	10' 7"	10' 7"	10′ 7″	10′ 8″	7′ 10″	9 0,	9, 0,	9, 9,	9' 10"	9' 7"	9' 7"	9' 9"	9, 9,	6' 4"	7 4'	7' 4"	8, 6,	8' 7"	8' 4'	8, 4,	8' 3'	8, 9,	Group B	Brace *
	111′ 3′	12′ 0 ′	12′ 0′	12 2	12° 3°	12, 0,	12' 0"	15, 0 ,	12, 5,	ð, ð,	10′ 11″	10 11.	11' 1"	11, 5,	10′ 11″	10' 11"	10 11"	11, 10	ο, ο,	9' 3"	9'3"	9′8″	3, 3,	9' 7"	9, 7,	9' 7"	9, 8,	Group A	(1) 2×4 °L
Symm	12, 1.	12′ 6′	12′ 6″	12' 8"	12′ 9′	12, 6,	12' 6"	12, 6,	12, 8,	10' 5'	11' 4'	11' 4"	11' 6'	11' 7'	11' 4"	11' 4'	11 4*	11' 6'	8, 6,	9′ 10′	9' 10"	10′ 1″	10′ 1″	9' 11'	9' 11'	9' 11'	10' 1'	Group B	2x4 'L' Brace *
7	14' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0'	4′ 0°	14' 0"	13′ 2°	13′ 0″	13' 0"	13' 0"	13′ 2″	13′ 3′	13' 6"	13' 8"	13′ 0″	13' 2'	10′ 10 ″	11 5,	11 5	11 6*	11' 7"	11 5	11 5	11 57	11 6	Group A	(2) 2×4 'L'
	14' 0"	14 0"	14' 0"	14 0"	14' 0"	14' 0"	14 0"	14' 0"	14' 0"	13 7	13 7'	13' 7"	13′ 9′	13 10"	13' 7'	13′ 7′	13' 7"	13 9"		11' 10'	11′ 10′	12 0,	12 1	11 10"	11' 10"	11' 10'	15 0	Group B	Brace ** (1)
	14' 0"	14′ 0″	14' 0"	14' 0"	14' 0"	14' 0"	14 0,	14' 0'	14' 0"	14 0"	14 0.	14' 0"	14' 0'	14' 0"	14' 0"	14' 0"	14' 0"	14′ 0*	12, 6,	14 0"	14′ 0″	14′ 0′	14' 0'	14' 0"	14 0'	14' 0"	14' 0"	⊅	(1) 2×6 (L'
	14' 0"	14' 0'	14' 0'	14' 0"	14' 0"	14' 0"	14' 0'	14' 0"	14' 0"	14′ 0′	14' 0"		14, 0,	14, 0,	14' 0'	14′ 0″	14 0"	14' 0"	13, 2,		14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14′ 0″	Group B	Brace *
	14' 0"	14' 0"	14' 0'	14′ 0′	14' 0'	14′ 0′		14' 0"	14' 0'	14' 0"	14' 0"		14′ 0′	14′ 0′	14 0"	14' 0"	14′ 0′	14' 0"	14' 0"	14' 0'	14' 0"	14' 0"	14' 0"	14 0"	14' 0'	14' 0"	14 0"	Group A	(2) 2x6 'L' Brace
	14' 0"	14' 0"	14' 8"	14' 0"	14' 0"	14, 0,	14' 0"	14' 0"	14' 0"	14' 0"	14 0"	14 0"	14' 0"	14. 0.	14' 0"	14' 0"	14' 0"		14′ 0″		14' 0"	14' 0"	14' 0"	14' 0"		14' 0"	14' 0"	Group B	Broce **



Attach "L" braces with 10d (0.128"x3.0" min) noits. So. Pine lumber design values based on the ALSC January 2012 ruling Sable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang. Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Wind Load deflection criterion is L/240.

Gable Truss Detail Notes

(x4 Braces shall be SRB (Stress-Rated Board) **For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group values may be used with these grades.

Douglas Fir-Larch

Southern Pine***

Bouglas Fir-Larch #3 Stud

Southern Pine***

Stenderd

Stud

Standard

Group B Hem-Fir #1 & Btr

Spruce-Pine-Fir #1 / #2 | Standard #3 | Stud

Bracing Group Species

and Grades

Group A

Ιţ

100

"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

No Splice

£X 4¥

diagonal brace for 335# at each end. Max web total length is 14 doubted when diagonal brace is used. Connect Diagonal brace option: vertical length may be

Jable Truss

in table above Vertical length shown

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(as shown) at brace; single or double cut better diagonal

upper end.

X

DF-L #2 or

Connect diagonal at midpoint of vertical web.

INPERTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS. だのかので ŧο chart above for max aldag tical

Refer to the Building Designer for conditions

t Refer to common truss design for

peak splice and heel plates.

Greater than 4 0' Greater than 11' 6'

less than II' 6' Vertical Length

not addressed by this detail

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of SES, (Sudding Comparent Safety). Information, by IPI and WICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per SEJ, liviness noted otherwise top chard shall have properly ettached structural sheething and bottom chard shall have a properly attached right celling. Locations shall not be promoted that it is shall be compared to the properly attached shall have properly ettached structural sheething and bottom of webs shall have bracing installed per SEJ sections SS, 37 or SEJ, as applicable. Apply plates to each face of truss and position as shown below and on the Joint Details unless noted otherwise. Refer to drawings 150A-Z for standard plate positions. CEN S CORIO 0.22839 × length ĭ N ×

MAX 5 PSF DATE 70 DRWG A12015ENC100212 2/14/12 ASCE7-10-GAB12015

04/10/2014

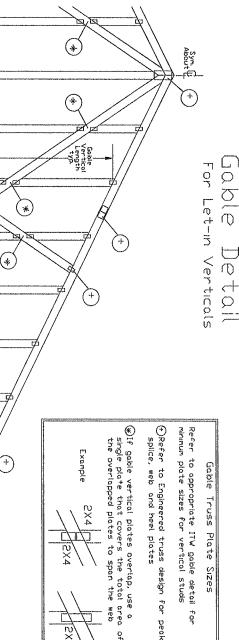
SPACING

240,

Earth City MO 63045

ITV Balading Components Group Inc. shell not be responsible for any deviation from this drawing, any failure to balld the truss in conformance with ANX./PI i or far handling, shipping, instabilation 8 has bracing of trusses. A seek on this growing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability ond use of this drawing for only structure is the responsibility of the balading Designer per ANSI/TPI I Sec.2. For more information see this job's general notes page and those web sites.

ITVSIG wealthoogocon IPI, wentchistoring WICLA wealsochootstryorg ITCL wealcase.

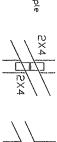


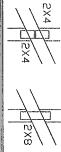
Gable Truss Plate Sizes

minimum plate sizes for vertical studs Refer to appropriate ITW gable detail

splice, web and heel plates

※If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web Example T 4XG





Provide connections for uplift specified on the engineered truss design Attach each 'T' reinforcing member with snld

End Driven Nails 110d Common (0.148'x 3.',min) Nails at 4' oc (4) nails in the top and bottom chards

Toenailed Nails 10d Common (0.148'x3' min) Toenails at 4' a.c plus (4) toenails in the top and bottom chords

This detail to be used with the appropriate ITW gable detail for ASCE wind load

Reinforcing Member

Noits

Rigid Sheathing

Truss

Nails Spaced 4° o.c

₽

ASCE 7-98 Gable Detail Drawings A13030980109 A12030980109 A11030980109 A130 5980109, A12015980109, A11015980109 A10015980109 A10030980109

ASCE 7-02 Gable Detail Drawings A13030020109, A12030020109 A11030020109, A10030020109 A14030020109 A13015020109, A12015020109 A11015020109, A10015020109 A14015020109,

ASCE 7-10 Gable Detail Drawings ASCE 7-05 Gable Detail Drawings A13030050109, A12030050109 A11030050109 A10030050109, A14030050109 A13015050109, A12015050109, A11015050109, A10015050109 A14015050109

A18030ENC100212 A20030ENC100212 A20030END100212, A20030PED100212 A11530ENC100212 A12030ENC100212 A14030ENC100212 A18015ENC100212, A20015ENC100212, A20015END100212, A20015PED100212, A11515ENC100212 A12015ENC100212, A14015ENC100212, A16015ENC100212 A16030ENC100212

Nois

Ceiling

See appropriate ITV gable detail for maximum unreinforced gable vertical length

"T" Reinforcement Attachment Detail "T" Reinforcing 5 'T' Reinforcing

End-nail

Web Length Increase w/ Ϋ́ Size Increase 30 %

specie, and grade of the "L" reinforcing member 'T' reinforcing member material must match size, length is 14' from top to bottom chord Maximum allowable "I" reinforced gable vertical

Brace

appropriate ITW gable detail).

To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on

Gable Vertical = 24°oc SP #3	Mean Roof Height = 30 ft, Kzt = 100	ASCE 7-10 Wind Speed = 120 mph	comple 2x6 20 %
	= 100	-	

Maximum 'T' Reinforced Gable Vertical Length $130 \times 8'$ 7' = 11' 2' (1) 2x4 "L" Brace Length = 8' 7" 'T' Brace Increase (From Above) = 30% = 130 'T' Reinforcing Member Size = 2x4



Earth City MO 63045

WARNINGS READ AND FOLLOW ALL NOTES ON THIS DRAWING
***TANDS** FURNISH THIS DRAWING TO ALL NOTES ON THIS DRAWING

Trusses require extreme care in fabricating, handling, shipping, installing and bracing, Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly officered rigid cellip. Lacations shown for permanent therein restraint of webs shall have bracing installed per BCSI sections B3. B7 or B10 as applicable Apply plates to each face of truss and position as shown door peak on the Joint Betails, unless noted otherwise. Refer to drawings 1604-2 for standard plate positions.

ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing any failure to build the truss in conformace with MSI/IPT i, or for handling, shipping, installations feature of professional engineering responsibility salely for the design shown. The suitability and use of this drawing or cover page listing estimating the suitability and use of this drawing for only structure is the responsibility of the Building Besigner per MSI/IPT I Sec.2 for more information see this job's general notes page and these web sites.

ITVBCS www.thucog.com; IPI www.tpinst.org; VIICM www.spichedustry.org, ICC www.iccsefe.org



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RITT

LET-IN VERT

FAC 디 SPACING PZ-60 PSF 74 0 * GBLLETIN0212 2/16/12

MAX

MAX DUR

Common Residential ASCE 10: Dable Fra Vina 120 mph, 8 Mean Height, Bracing Requirements Closed, Exposure 1 Stiffeners

120 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp C, or 100 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp D, or 100 mph, 30ft Mean Hgt, ASCE 7-10, Part Enclosed, Exp C, Kzt = 100, Wind TC DL=50 psf, Wind BC DL=50 psf

I I

Lateral chord bracing requirements Top Continuous roof sheathing Bot Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail

Nails 10d box or gun (0128"x3",min) nails

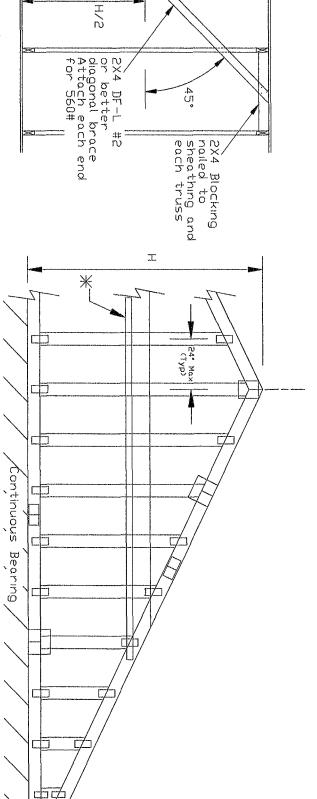
Less than 4'6" — no stud bracing required

Greater than 4'6" to 7'6" in length provide a 2x6 stffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC100212)

Greater than 7'6" to 12'0" max provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC100212)

I

★ Optional 2x L-reinforcement attached
to stiffback with 10a box or gun
(0128" x 3", min > nails @ 6" oc





N X S

#10 Stiffback

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VARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING: INSTALLERS.

attached to each Stud w/ (4) 10d box or gun (0123" \times 3", min) nails

Trusses require extreme care in fabricating, handling, shipping, installing and bracking. Refer to and follow the latest edition of SLIXI Chalding Component Safety information, by TP and WTLD for safety practices prior to performing these functions. Installers shall provide theoporary bracking per SLXI thieses noted otherwise top chard shall have properly attached structural sheathing and bottom chard shall have a properly attached rigid ceiling. Locations shown for personent attend restraint of webs shall have bracing installed per SLXI sections 33, 37 or 380 as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted atherwise. Refer to drawings 160A-Z for standard plate positions.

ITW statisting Components Group Inc. shall not be responsible for any deviation from this drowing, only failure to build the truss on conformance with MSU/FPI I, or for handling, shipping, installation 8 broking of trusses. A seal on this drowing or cover page listing this drowing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability on use of this drowing for any structure is the responsibility of the Building Besigner per ANSI/TPI 1 Sec.2 for more information see this job's general notes page and these web sites



×	AX TOT LD 60 PSF	50	P V T	DATE	DATE 2/14/12 DRWG GABRST100212
×	TOT LD 6	00	PSF		
×	AX SPACING				

REF

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WHALER