

## JULIUS LEE PE.

RE: 532574 - HOUSECRAFT - HOPKINS RES.

**1109 COASTAL BAY BLVD,  
BOYNTON BEACH, FL 33435**

### Site Information:

Project Customer: HOUSECRAFT HOMES Project Name: 532574 Model HOPKINS RES

Lot/Block: Subdivision:

Address: 2116 SW JUNCTION RD

City: COLUMBIA CTY State: FL

### Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: JOHN D. HARRINGTON License #: CGC038861

Address: 24113 NW OLD BELLAMY RD

City: HIGH SPRINGS, State: FL

### General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2010/TPI2007

Design Program: MiTek 20/20 7.3

Wind Code: ASCE 7-10 Wind Speed: 130 mph

Floor Load: N/A psf

Roof Load: 32.0 psf

This package includes 19 individual, dated Truss Design Drawings and 0 Additional Drawings  
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet  
conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

This document processed per section 16G15-23 003 of the Florida Board of Professionals Rules

**In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany  
this coversheet. The latest approval dates supersede and replace the previous drawings.**

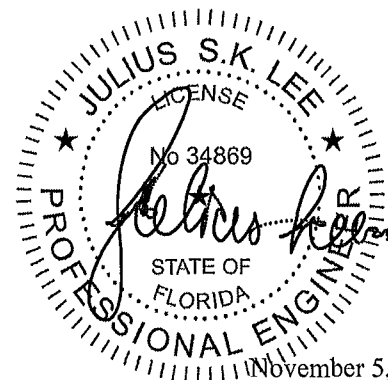
No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I7459040	CJ01	11/5/013	18	I7459057	T11	11/5/013
2	I7459041	CJ02	11/5/013	19	I7459058	T12	11/5/013
3	I7459042	CJ03	11/5/013				
4	I7459043	EJ01	11/5/013				
5	I7459044	EJ02	11/5/013				
6	I7459045	HJ01	11/5/013				
7	I7459046	HJ02	11/5/013				
8	I7459047	T01	11/5/013				
9	I7459048	T02	11/5/013				
10	I7459049	T03	11/5/013				
11	I7459050	T04	11/5/013				
12	I7459051	T05	11/5/013				
13	I7459052	T06	11/5/013				
14	I7459053	T07	11/5/013				
15	I7459054	T08	11/5/013				
16	I7459055	T09	11/5/013				
17	I7459056	T10	11/5/013				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2015.

**NOTE:** The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



November 5, 2013

Job 532574	Truss CJ01	Truss Type Jack-Open Truss	Qty 12	Ply 1	HOUSECRAFT - HOPKINS RES.  Job Reference (optional) ID IDL4KEa49EGgR2GNm4I8yMvd8-EOIF71G9Bh3Z_vtwretn4ieQEKTg4ypLYCOW_DyMI3v	17459040
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7,350 s Sep 27 2012 MITek Industries Inc. Tue Nov 05 08:11 16 2013 Page 1  
Builders FirstSource Lake City FL 32055

Scale = 1/8" = 1'-0"

Plate Offsets (X,Y): [2'-0"-6'-0", 0'-1'-2"]					
LOADING (psf)	SPACING	2'-0'-0"	CSI	DEFL	PLATES GRIP
TCLL 20.0	Plates Increase	1 25	TC 0.32	in (loc) l/defl L/d	MT20 244/190
TCDL 7.0	Lumber Increase	1 25	BC 0.06	Vert(LL) 0.00 8 >999 240	
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Vert(TL) 0.00 8 >999 180	
BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)	Horz(TL) 0.00 2 n/a n/a	
			Weight: 7 lb		FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

**REACTIONS** (lb/size) 2=184/0-7-10 (min 0-1-8) 5=-24/Mechanical 3=-21/Mechanical

Max Horz 2=67(LC 12)

Max Uplift 2=-138(LC 12), 5=-30(LC 2) 3=-27(LC 2)

Max Grav 2=225(LC 2), 5=24(LC 16), 3=22(LC 8)

**FORCES** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES** (7-9)

- 1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCCL=4.2psf BCDL=3.0psf h=18ft, Cat. II Exp C Encl. GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown. Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 2, 30 lb uplift at joint 5 and 27 lb uplift at joint 3.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- 8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P E License No. 34869 Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435

**LOAD CASE(S)** Standard

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 1'-0" oc purlins

BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



November 5, 2013

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.  
Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component.  
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB 89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job 532574	Truss CJ03	Truss Type Jack-Open Truss	Qty 8	Ply 1	HOUSECRAFT - HOPKINS RES	I7469042
Builders FirstSource, Lake City FL 32055		7.350 s Sep 27 2012 Mitek Industries, Inc. Tue Nov 05 08:11 18 2013 Page 1				
		ID: IDL4KEa49EGg8gR2GNm4I8yMvd8-An_OYIPIJJHDD1Jy3wFA7kmj862Yslb?W126yM18				

Scale = 1/20.3

Plate Offsets (X,Y) [2-0-4-4,0-0-4]					
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	PLATES GRIP
TCLL 20.0*	Plates Increase 1.25	TC 0.32	in (loc) 0.02	4-7 >999	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.13	Vert(TL) -0.03	4-7 >999	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.00	2 n/a	
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)			
					Weight: 19 lb FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

**REACTIONS** (lb/size) 3=79/Mechanical 2=253/0-7 10 (min 0-1-8) 4=23/Mechanical

Max Horz 2=162(LC 12)

Max Uplift 3=93(LC 12) 2=148(LC 12)

Max Grav 3=97(LC 2) 2=304(LC 2) 4=56(LC 3)

**FORCES** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-418/105

BOT CHORD 2-4=-352/610

**NOTES** (7-9)

1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C Encl GCpl=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone-C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 93 lb uplift at joint 3 and 148 lb uplift at joint 2.

6) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss

7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code

8) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.

9) Truss Design Engineer Julius Lee, PE. Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

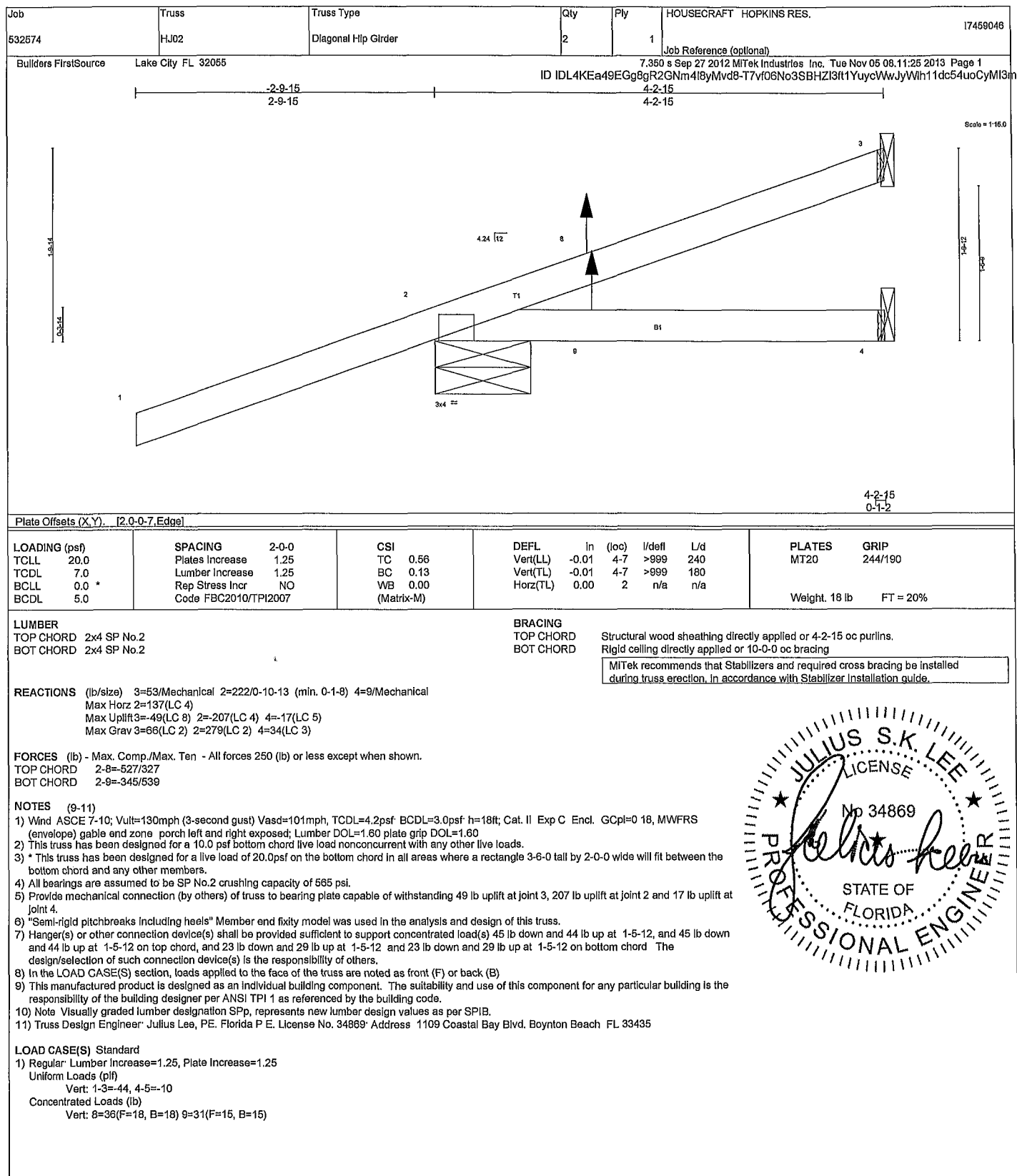
**LOAD CASE(S)** Standard

November 5, 2013

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17-7473 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,  
1109 Coastal Bay  
Boynton Beach, FL 33435

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Boynton Beach FL 33435

Job 532574	Truss T01	Truss Type Hip Truss	Qty 2	Ply 1	HOUSECRAFT HOPKINS RES.  Job Reference (optional)	I7459047
Builders FirstSource Lake City, FL 32055		7.350 s Sep 27 2012 MITek Industries Inc. Tue Nov 05 08:11:28 2013 Page 2 ID IDL4KEa49EGg8gR2GNm4IByMvd8-tiboe8QhMNasQIoEY95baE8KO9LxuG83I3IYPXyM3				
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code						
12) Note. Visually graded lumber designation SPP, represents new lumber design values as per SPIB.						
13) Truss Design Engineer: Julius Lee, PE: Florida P E License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435						
LOAD CASE(S) Standard						
1) Regular: Lumber Increase=1.25 Plate Increase=1.25						
Uniform Loads (plf)						
Vert: 1-3=-44, 3-7=-44, 7-9=-44, 2-8=-10						
Concentrated Loads (lb)						
Vert: 3=-69(F) 5=-69(F) 7=-141(F) 15=-210(F) 10=-210(F) 20=-69(F) 21=-69(F) 22=-69(F) 23=-69(F) 24=-69(F) 25=-69(F) 26=-69(F) 27=-69(F) 28=-22(F) 29=-22(F) 30=-22(F) 31=-22(F) 32=-22(F) 33=-22(F) 34=-22(F) 35=-22(F) 36=-22(F)						



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Job 532574	Truss T03	Truss Type Hip Truss	Qty 1	Ply 1	HOUSECRAFT - HOPKINS RES.	17459049																																																		
Builders FirstSource Lake City FL 32055					Job Reference (optional)																																																			
<div style="display: flex; justify-content: space-between;"> <span>ID IDL4KEa49EGg8gR2GNm4i8yMvd8-EgOhhrUqAwC9WWhCLihmGirJJA9iZaaoSL0J4kyMI3a</span> <span>7.350 s Sep 27 2012 MITek Industries Inc. Tue Nov 05 08:11:33 2013 Page 1</span> </div>																																																								
Plate Offsets (X,Y): [2,0-2-10,0-1-8], [4,0-6-0,0-2-8], [6,0-6-0,0-2-8], [8,0-2-10,0-1-8]																																																								
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:15%;">LOADING (psf)</td> <td style="width:15%;">SPACING</td> <td style="width:15%;">2-0-0</td> <td style="width:15%;">CSI</td> <td style="width:15%;">DEFL</td> <td style="width:15%;">In (loc)</td> <td style="width:15%;">I/defl</td> <td style="width:15%;">L/d</td> <td style="width:15%;">PLATES</td> <td style="width:15%;">GRIP</td> </tr> <tr> <td>TCLL 20.0</td> <td>Plates Increase</td> <td>1.25</td> <td>TC 0.40</td> <td>Vert(LL) 0.17</td> <td>13</td> <td>&gt;999</td> <td>240</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCDL 7.0</td> <td>Lumber Increase</td> <td>1.25</td> <td>BC 0.50</td> <td>Vert(TL) -0.26</td> <td>13-15</td> <td>&gt;999</td> <td>180</td> <td></td> <td></td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Rep Stress Incr</td> <td>YES</td> <td>WB 0.28</td> <td>Horz(TL) 0.10</td> <td>8</td> <td>n/a</td> <td>n/a</td> <td></td> <td></td> </tr> <tr> <td>BCDL 5.0</td> <td>Code FBC2010/TPI2007</td> <td></td> <td>(Matrix-M)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> </table>							LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP	TCLL 20.0	Plates Increase	1.25	TC 0.40	Vert(LL) 0.17	13	>999	240	MT20	244/190	TCDL 7.0	Lumber Increase	1.25	BC 0.50	Vert(TL) -0.26	13-15	>999	180			BCLL 0.0 *	Rep Stress Incr	YES	WB 0.28	Horz(TL) 0.10	8	n/a	n/a			BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)						
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<p>REACTIONS (lb/size) 2=1008/0-7 10 (min 0-1-8) 8=1008/0-7-10 (min 0-1-8)        Max Horz 2=88(LC 11)        Max Uplift 2=267(LC 12), 8=267(LC 13)        Max Grav 2=1196(LC 2) 8=1196(LC 2)</p>																																																								
<p>FORCES (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.        TOP CHORD 2-3=2080/1160, 3-4=1736/1020, 4-5=1752/1097 5-6=1752/1097, 6-7=1736/1020, 7-8=2079/1160        BOT CHORD 2-16=891/1796, 15-16=891/1796, 14-15=628/1456 13-14=628/1456, 12-13=630/1457, 11-12=630/1457 10-11=900/1800, 8-10=900/1800        WEBS 3-15=394/308, 4-15=115/286, 4-13=172/413, 5-13=368/272 6-13=172/413 6-11=114/286, 7 11=394/308</p>																																																								
<p>NOTES (9-11)        1) Unbalanced roof live loads have been considered for this design.        2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf h=18ft; Cat. II Exp C, Encl GCPl=0.18 MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60        3) Provide adequate drainage to prevent water ponding        4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.        5) * This truss has been designed for a live load of 20 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.        6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.        7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 267 lb uplift at joint 2 and 267 lb uplift at joint 8        8) "Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss        9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.        10) Note: Visually graded lumber designation SP, represents new lumber design values as per SPIB.        11) Truss Design Engineer: Julius Lee PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435</p>																																																								
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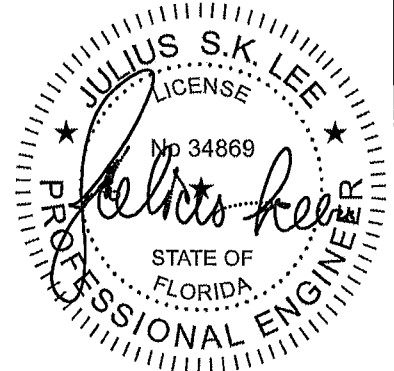


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Job 532574	Truss T05	Truss Type Hip Truss	Qty 2	Ply 1	HOUSECRAFT - HOPKINS RES.	I7459051																																																												
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<p>7.350 s Sep 27 2012 MITek Industries, Inc. Tue Nov 05 08:11:40 2013 Page 1</p> <p>ID:IDL4KEa49EGg8gR2GNm4l8yMvd8-X0JK9FZDX349sbjYFgJP3meQx?S0lkdq3xCBqqyMI3X</p> <p>Scale: 3/16"=1'</p>																																																																		
<p>Plate Offsets (X,Y): [2.0-0-10,Edge], [3.0-3-0,0-3-0], [5.0-6-0,0-2-8], [6.0-6-0,0-2-8], [8.0-3-0,0-3-0], [9.0-0-10,Edge], [11.0-3-0,0-3-0], [15.0-3-0,0-3-0]</p>																																																																		
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<p><b>REACTIONS</b> (lb/size) 2=1008/0-7 10 (min. 0-1-8) 9=1008/0-7 10 (min. 0-1-8)</p> <p>Max Horz 2=-115(LC 10)</p> <p>Max Uplift 2=-292(LC 12) 9=-292(LC 13)</p> <p>Max Grav 2=1196(LC 2) 9=1196(LC 2)</p>																																																																		
<p><b>FORCES</b> (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.</p> <p>TOP CHORD 2-3=-3756/2023, 3-4=-3486/1841 4-5=-2761/1433, 5-6=-2762/1461 6-7=-2761/1435</p> <p>7-8=-3501/1875, 8-9=-3775/2060</p> <p>BOT CHORD 2-15=-1694/3370, 14-15=-1401/3031 13-14=-914/2412, 12-13=-916/2413, 11-12=-1415/3037</p> <p>9-11=-1745/3393</p> <p>WEBS 3-15=-238/282, 4-15=-106/312, 4-14=-627/510, 5-14=-257/450 5-13=-248/627 6-13=-244/626, 6-12=-257/450, 7 12=-627/509, 7-11=-103/312 8-11=-237/279</p>																																																																		
<p><b>NOTES</b> (10-12)</p> <p>1) Unbalanced roof live loads have been considered for this design.</p> <p>2) Wind: ASCE 7 10, Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C, Encl GCpl=0 18' MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces &amp; MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60</p> <p>3) Provide adequate drainage to prevent water ponding</p> <p>4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads</p> <p>5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.</p> <p>6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi</p> <p>7) Bearing at joint(s) 2 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.</p> <p>8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 292 lb uplift at joint 9</p> <p>9) Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss</p> <p>10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code</p> <p>11) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.</p> <p>12) Truss Design Engineer: Julius Lee, PE: Florida P E License No. 34869 Address 1109 Coastal Bay Blvd Boynton Beach FL 33435</p>																																																																		
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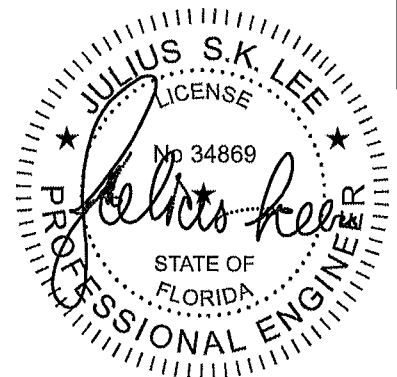
November 5, 2013

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Julius Lee PE  
1109 Coastal Bay  
Boynton Beach, FL 33435



Job 532574	Truss T07	Truss Type Hip Truss	Qty 1	Ply 1	HOUSECRAFT HOPKINS RES	I7459053																																				
Builders FirstSource, Lake City FL 32055					7.350 s Sep 27 2012 MITek Industries Inc. Tue Nov 05 08:11:44 2013 Page 1 ID IDL4KEa49EGg8gR2GNm4l8yMvd8-PnZr7ccjblbbLC0JUWNLDcp6McqDeQDQ_ZAPzbyMI3T																																					
<div style="display: flex; justify-content: space-between;"> <div style="width: 40%;"> <p>2-0-0 7-2-0 11-0-0 17-0-0 23-0-0 28-4-5 34-0-0 38-0-0</p> <p>2-0-0 7-2-0 3-10-0 6-0-0 6-0-0 5-4-5 5-7-11 2-0-0</p> </div> <div style="width: 55%; text-align: right;"> <p>Scale: 3/16"=1'</p> </div> </div>																																										
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:15%;">LOADING (psf)</td> <td style="width:15%;">SPACING</td> <td style="width:15%;">CS</td> <td style="width:15%;">DEFL</td> <td style="width:15%;">PLATES</td> <td style="width:15%;">GRIP</td> </tr> <tr> <td>TCLL 20.0</td> <td>2-0-0</td> <td>TC 0.76</td> <td>in (loc) l/defl L/d</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCDL 7.0</td> <td>Plates Increase 1.25</td> <td>BC 0.83</td> <td>Vert(LL) 0.35 15 &gt;999 240</td> <td></td> <td></td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Lumber Increase 1.25</td> <td>WB 0.87</td> <td>Vert(TL) -0.55 15 &gt;741 180</td> <td></td> <td></td> </tr> <tr> <td>BCDL 5.0</td> <td>Rep Stress Incr YES</td> <td>(Matrix-M)</td> <td>Horz(TL) 0.29 8 n/a n/a</td> <td></td> <td></td> </tr> <tr> <td></td> <td>Code FBC2010/TPI2007</td> <td></td> <td></td> <td>Weight: 190 lb</td> <td>FT = 20%</td> </tr> </table>							LOADING (psf)	SPACING	CS	DEFL	PLATES	GRIP	TCLL 20.0	2-0-0	TC 0.76	in (loc) l/defl L/d	MT20	244/190	TCDL 7.0	Plates Increase 1.25	BC 0.83	Vert(LL) 0.35 15 >999 240			BCLL 0.0 *	Lumber Increase 1.25	WB 0.87	Vert(TL) -0.55 15 >741 180			BCDL 5.0	Rep Stress Incr YES	(Matrix-M)	Horz(TL) 0.29 8 n/a n/a				Code FBC2010/TPI2007			Weight: 190 lb	FT = 20%
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<b>REACTIONS</b> (lb/size) 2=1001/0-7-10 (min. 0-1-8) 8=1011/0-7-10 (min. 0-1-8) Max Horz 2=-88(LC 10) Max Uplift 2=-285(LC 12) 8=-267(LC 13) Max Grav 2=1190(LC 2) 8=1202(LC 2)																																										
<b>FORCES</b> (lb) Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-3743/1938, 3-4=-3614/2004, 4-5=-1766/1104 5-6=-1766/1104, 6-7=-1742/1021 7-8=-2073/1153 BOT CHORD 2-16=-1601/3354 3-16=-222/261, 13-14=-628/1473, 12-13=-629/1462, 11-12=-629/1462, 10-11=-892/1792, 8-10=-892/1792 WEBS 14-16=-620/1501, 4-16=-1222/2367 4-14=-578/292, 4-13=-174/404, 5-13=-369/276, 6-13=-177/421, 6-11=-112/282, 7-11=-379/300																																										
<b>NOTES</b> (10-12) 1) Unbalanced roof live loads have been considered for this design 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCCL=4.2psf BCDL=3.0psf h=18ft, Cat. II Exp C, End. GCpl=0.18; MWFRS (envelope) and C-C Exterior(2) zone-C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula Building designer should verify capacity of bearing surface 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 265 lb uplift at joint 2 and 267 lb uplift at joint 8. 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 11) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB. 12) Truss Design Engineer: Julius Lee, PE, Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435																																										
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Julius Lee PE,  
 1109 Coastal Bay  
 Boynton Beach, FL 33435

Job 532574	Truss T09	Truss Type Scissor Truss	Qty 3	Ply 1	HOUSECRAFT - HOPKINS RES.	I7459055
Builders FirstSource, Lake City FL 32055					7.350 s Sep 27 2012 MITek Industries, Inc. Tue Nov 05 08:11:48 2013 Page 1 ID IDL4KEa49EGg8gR2GNm4l8yMyd8-IYoMr_fEaX50pqK4jMSHNSznDChaG07vA8c6NyMI3P	

Plate Offsets (X,Y): [2.0-0-10,Edge], [10-0-0-10,Edge], [12.0-3-0-0-3-0], [14.0-3-0-0-3-0]				
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.81 BC 0.79 WB 0.69 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) 0.62 13-14 >661 240 Vert(TL) -0.98 13-14 >416 180 Horz(TL) 0.63 10 n/a n/a	PLATES GRIP MT20 244/190  Weight: 162 lb FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 \*Except\*

WEBS B1 2x4 SP No.1  
2x4 SP No.3

**REACTIONS** (lb/size) 2=1006/0-7 10 (min 0-1-8) 10=1006/0-7-10 (min 0-1-8)

Max Horz 2=129(LC 10)

Max Uplift 2=302(LC 12) 10=302(LC 13)

Max Grav 2=1196(LC 2) 10=1196(LC 2)

**FORCES** (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown

TOP CHORD 2-3=-3798/2071 3-4=-3439/1805, 4-5=-3349/1818 5-6=-2545/1320, 6-7=-2545/1320

7-8=-3360/1844, 8-9=-3451/1832 9-10=-3813/2108

BOT CHORD 2-14=-1741/3408 13-14=-1374/2978, 12-13=-1385/2983 10-12=-1787/3429

WEBS 3-14=-318/356, 5-14=-95/366, 5-13=-779/616, 6-13=-933/1913, 7 13=-778/615, 7-12=-93/366, 9-12=-317/354

**NOTES** (9-11)

1) Unbalanced roof live loads have been considered for this design

2) Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C, Encl GCpl=0.18, MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) All bearings are assumed to be SP No 2 crushing capacity of 565 psi

6) Bearing at joint(s) 2 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 302 lb uplift at joint 2 and 302 lb uplift at joint 10.

8) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.

9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

10) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.

11) Truss Design Engineer Julius Lee, PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 4-7-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



November 5,2013

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Julius Lee PE,  
1109 Coastal Bay  
Boynton Beach FL 33435

Job 532574	Truss T11	Truss Type Hip Truss	Qty 1	Ply 1	HOUSECRAFT HOPKINS RES.	17459057																																						
Builders FirstSource, Lake City FL 32055					Job Reference (optional)																																							
<div style="text-align: right; font-size: small;">7,350 s Sep 27 2012 MITek Industries Inc. Tue Nov 05 08:11:51 2013 Page 1</div> <div style="text-align: center; font-size: x-small;">ID IDL4KEa49EGg8gR2GNm4l8yMvd8-i7UUT?h6xSTbgH3rOU?_74bQDROInn1Rb8NGjhyMI3M</div>																																												
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				Weight. 40 lb	FT = 20%																																							
<table border="1" style="width:100%; border-collapse: collapse; font-size: x-small;"> <tr> <td style="width:50%;"> <b>LUMBER</b>            TOP CHORD 2x4 SP No.2            BOT CHORD 2x4 SP No.2            WEBS 2x4 SP No.3         </td> <td style="width:50%;"> <b>BRACING</b>            TOP CHORD            BOT CHORD         </td> </tr> </table>							<b>LUMBER</b> TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3	<b>BRACING</b> TOP CHORD BOT CHORD																																				
<b>LUMBER</b> TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3	<b>BRACING</b> TOP CHORD BOT CHORD																																											
<div style="border: 1px solid black; padding: 5px; font-size: x-small;">       Structural wood sheathing directly applied or 6-0-0 oc purlins        Rigid ceiling directly applied or 10-0-0 oc bracing        MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.     </div>																																												
<b>REACTIONS</b> (lb/size) 2=314/0-7 10 (min. 0-1-8) 5=316/0-7-10 (min. 0-1-8) Max Horz 2=31(LC 12) Max Uplift 2=230(LC 8) 5=232(LC 9) Max Grav 2=376(LC 21) 5=379(LC 2)																																												
<b>FORCES</b> (lb) Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-337/330, 3-15=-276/320, 4-15=-276/320 4-5=-343/347 BOT CHORD 2-8=-261/270, 8-16=-265/271 7-16=-265/271 5-7=-269/275																																												
<b>NOTES</b> (11-13) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C, Encl. GCPI=0 18; MWFRS (envelope) porch left and right exposed Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 2 and 232 lb uplift at joint 5. 8) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3 lb down and 57 lb up at 3-0-0 and 3 lb down and 57 lb up at 4-2-0 and 49 lb down and 113 lb up at 5-4-0 on top chord and 7 lb down and 58 lb up at 3-0-0 and 3 lb down and 31 lb up at 4-2-0, and 7 lb down and 58 lb up at 5-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B) 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 12) Note Visually graded lumber designation SPP, represents new lumber design values as per SPIB. 13) Truss Design Engineer: Julius Lee PE: Florida P.E. License No. 34869, Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435																																												
<b>LOAD CASE(S)</b> Standard 1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-44 3-4=-44 4-6=-44, 9-12=-10 Concentrated Loads (lb) Vert: 3=2(F) 4=-7(F) 8=0(F) 7=0(F) 15=2(F) 16=-1(F)																																												



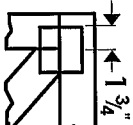
November 5, 2013

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.  
 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component.  
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

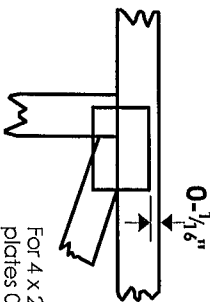
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 1109 Coastal Bay  
 Boynton Beach FL 33435

# Symbols

## PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated  
Dimensions are in ft-in-sixteenths  
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

**\* Plate location details available in Mitek 20/20 software or upon request.**

## PLATE SIZE

4 X 4

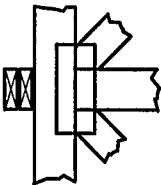
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots

## LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

## BEARING

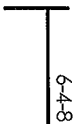


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

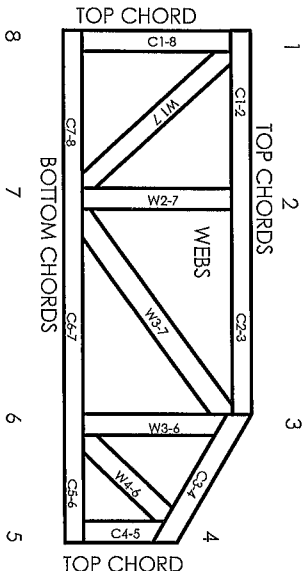
## Industry Standards:

ANSI/FP11. National Design Specification for Metal Plate Connected Wood Truss Construction  
DSB-89. Design Standard for Bracing.  
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

# Numbering System



dimensions shown in ft-in-sixteenths  
(Drawings not to scale)



**JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.**

**CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.**

## PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B,  
9730, 95-43, 96-31, 9667A  
NER-487, NER-561  
95110, 84-32, 96-67, ER-3907, 9432A

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# General Safety Notes

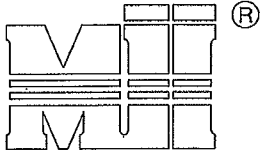
**Failure to Follow Could Cause Property Damage or Personal Injury**

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered
3. Never exceed the design loading shown and never stock materials on inadequately braced trusses
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other
6. Place plates on each face of truss at each joint and embed fully. Knots and ware of joint locations are regulated by ANSI/FP11
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/FP11
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified
13. Top chords must be sheathed or purlins provided at spacing indicated on design
14. Bottom chords require lateral bracing at 10 ft spacing, or less if no ceiling is installed, unless otherwise noted
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer
17. Install and load vertically unless indicated otherwise
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient
20. Design assumes manufacture in accordance with ANSI/FP11 Quality Criteria

August 10, 2010

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

ST - T-BRACE 2



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MiTek Industries, Chesterfield, MO Page 1 of 1

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nailing Pattern

T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

Brace Size  
for One-Ply Truss

Specified Continuous  
Rows of Lateral Bracing

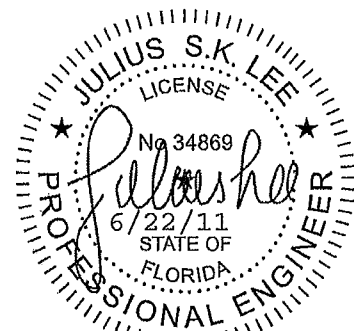
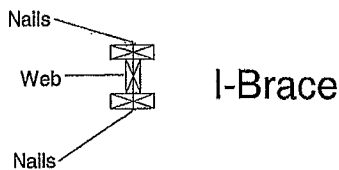
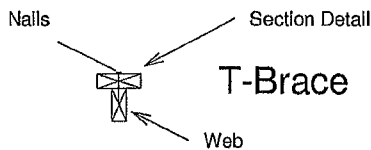
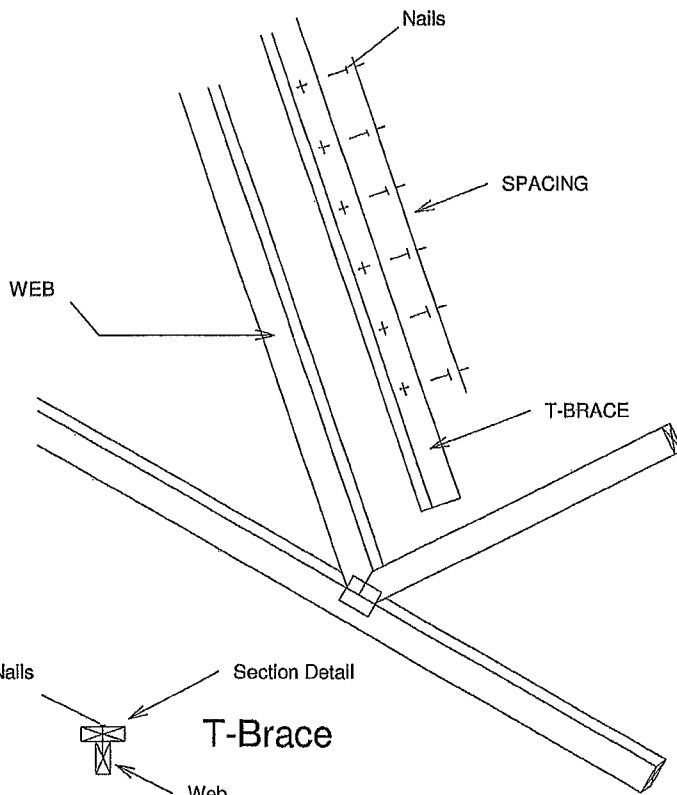
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size  
for Two-Ply Truss

Specified Continuous  
Rows of Lateral Bracing

Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.



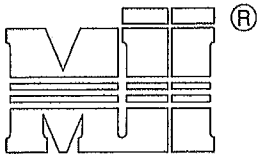
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JANUARY 1, 2009

## LATERAL TOE-NAIL DETAIL

ST-TOENAIL\_SP

MITek Industries, Chesterfield, MO Page 1 of 1



MITek Industries, Inc.

## NOTES

1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.)
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SYP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80 6	69.9	68 4	59.7
	.135	93 5	85 6	74.2	72.6	63.4
	.162	108 8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57 6	50.3
	.131	75.9	69.5	60.3	59 0	51.1
	.148	81.4	74 5	64.6	63.2	52.5

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.  
 APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED

## EXAMPLE

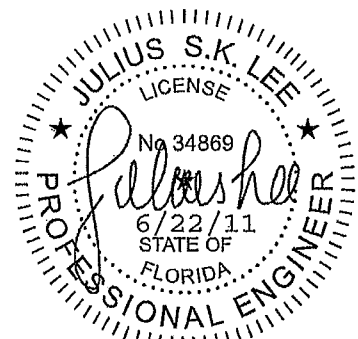
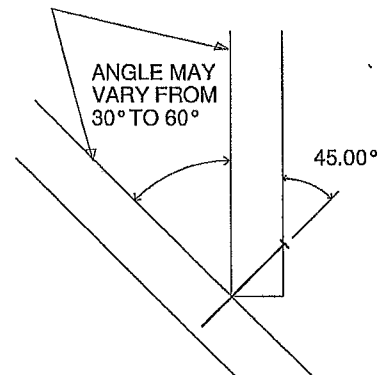
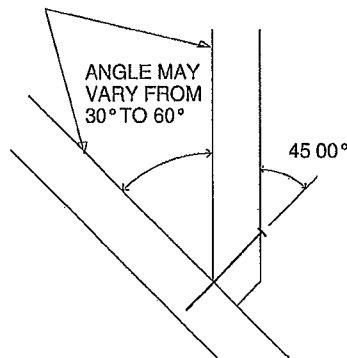
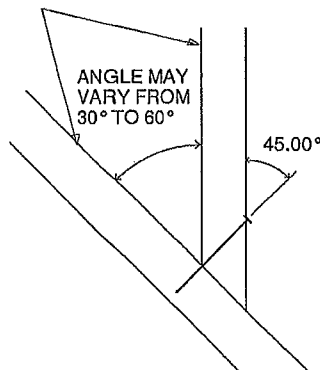
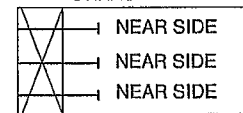
(3) - 16d NAILS (.162" diam. x 3.5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15

3 (nails) X 84 5 (lb/nail) X 1.15 (DOL) = 291 5 lb Maximum Capacity

SIDE VIEW

3 NAILS



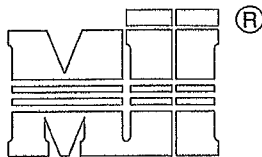
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FEBRUARY 14, 2012

# STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY-7-10

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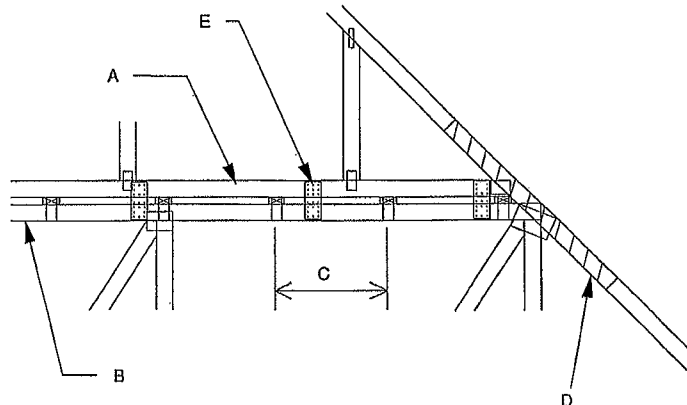


MITek Industries, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E  
MAX MEAN ROOF HEIGHT = 30 FEET  
MAX TRUSS SPACING = 24" O.C.  
CATEGORY II BUILDING  
EXPOSURE B or C  
ASCE 7-10  
DURATION OF LOAD INCREASE : 1 60

DETAIL IS NOT APPLICABLE FOR TRUSSES  
TRANSFERING DRAG LOADS (SHEAR TRUSSES).  
ADDITIONAL CONSIDERATIONS BY BUILDING  
ENGINEER/DESIGNER ARE REQUIRED

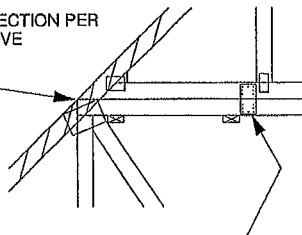
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH
- D - 2 X 4'-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
  2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



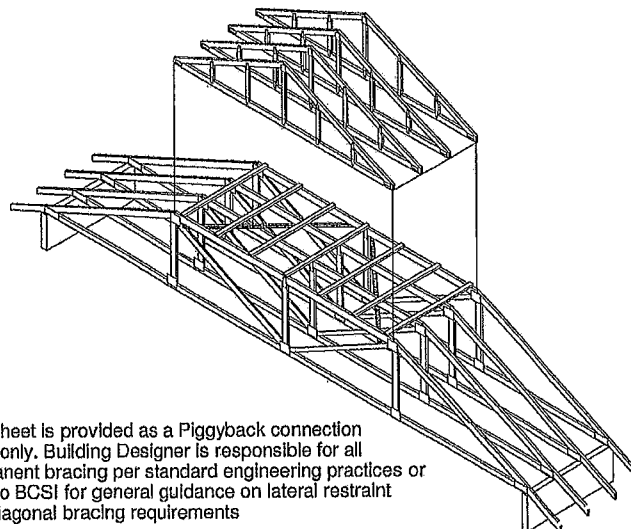
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING

SCAB CONNECTION PER  
NOTE D ABOVE

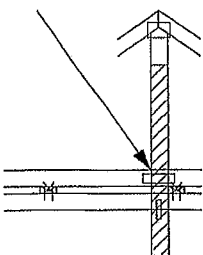


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE



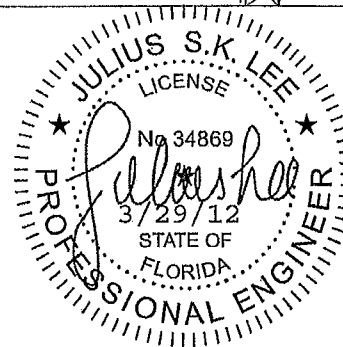
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements

VERTICAL WEB TO  
EXTEND THROUGH  
BOTTOM CHORD  
OF PIGGYBACK

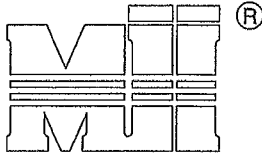


FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB.

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN



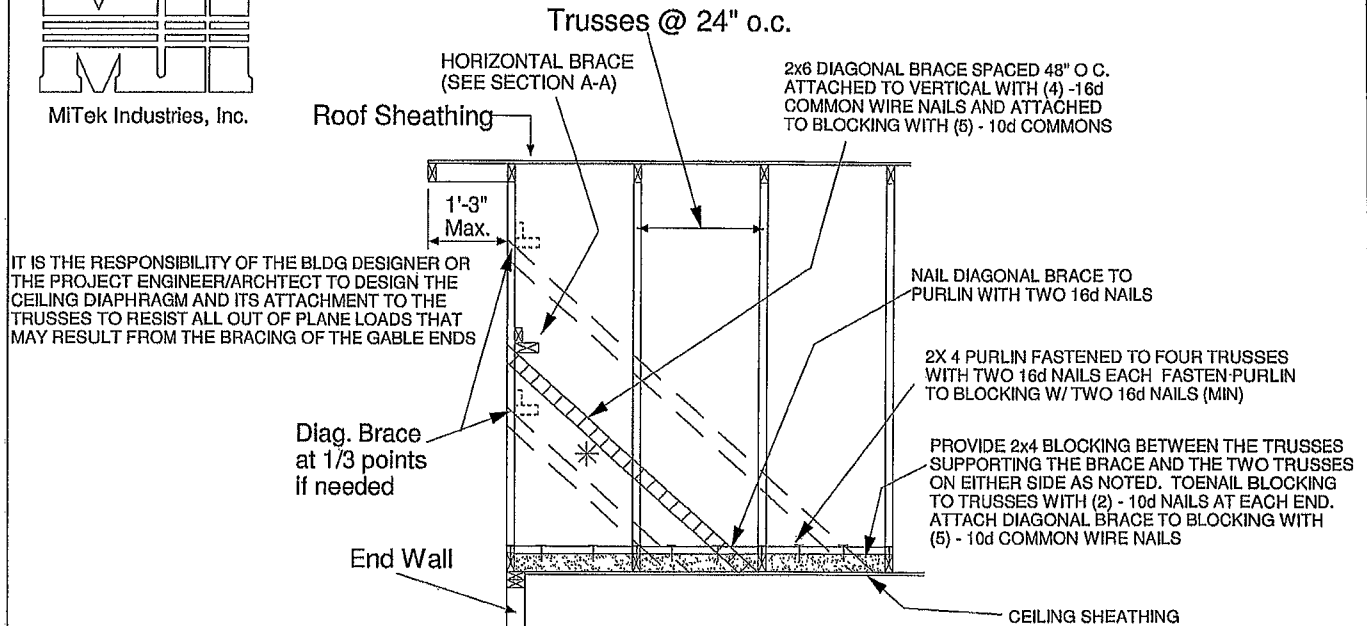
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## ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



## BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

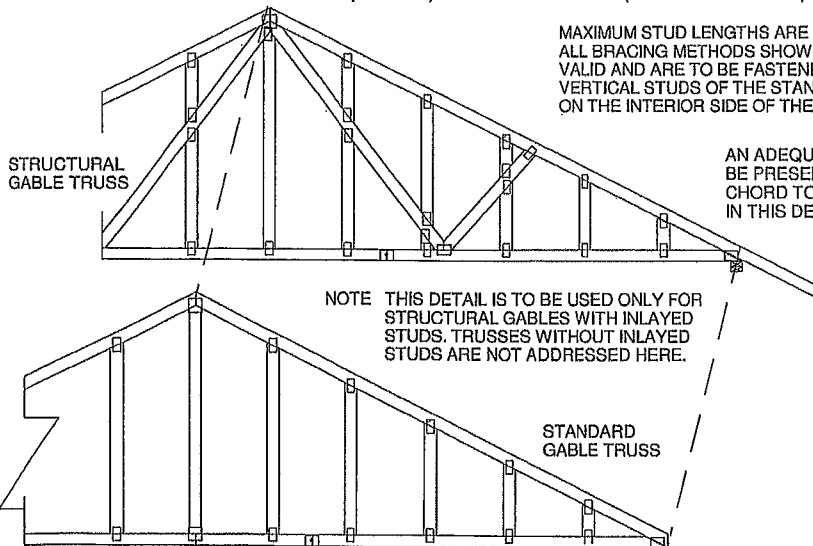
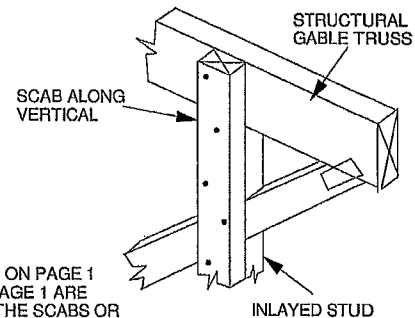
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED:

METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

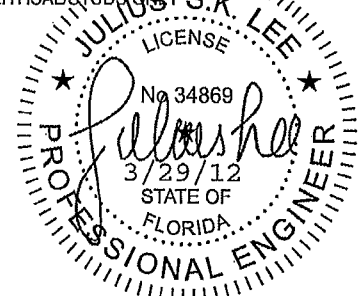
METHOD 2: ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

## NAILING SCHEDULE:

- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (.131" X 3") NAILS SPACED 6" O.C
- FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (.131" X 3") NAILS SPACED 6" O.C (2X 4 STUDS MINIMUM)



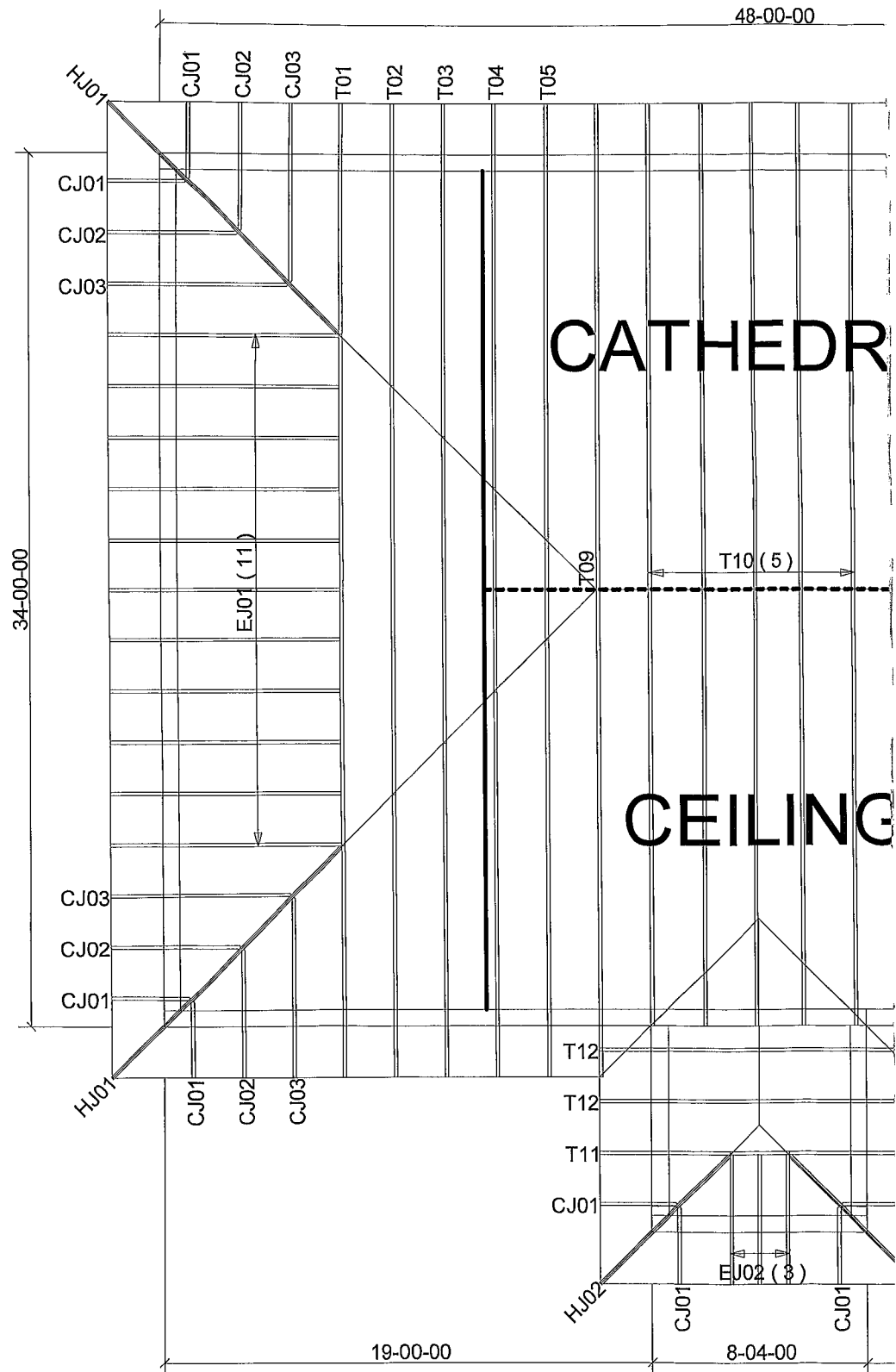
AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY.



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# 6/12 PITCH -



MITEK PLATE APPROVAL #'s 2197.2 - 2197.4, WEYERH